Mr. Tracey Garland, GIT
New York State Department of Environmental Conservation
Division of Environmental Remediation
625 Broadway, 11th Floor
Albany, New York 12233-7014

Arcadis of New York, Inc.
One Lincoln Center
110 West Fayette Street
Suite 300
Syracuse
New York 13202
Phone: 315 446 9120
Fax: 315 449 0017
www.arcadis.com

Date: September 18, 2026
Our Ref: 30337970
Subject: 2026 Post-Construction Monitoring Report
Binghamton Court Street Former Manufactured Gas Plant Site
NYSDEC Site No. 7-04-031

Dear Mr. Garland,

On behalf of the New York State Electric & Gas Corporation (NYSEG), this letter summarizes the 2026 post-remediation monitoring completed at the Binghamton Court Street Former Manufactured Gas Plant (MGP) site, Operable Unit No. 1 (OU-1) and Operable Unit No. 2 (OU-2), located in Binghamton, New York (the Site). Arcadis of New York, Inc. (Arcadis) conducted the 2026 monitoring activities in accordance with the May 2023 Site Management Plan (SMP) prepared by GEI Consultants, Inc.

For reference, interim remedial measures (IRMs) at OU-1 were completed from 2000 to 2001, 2003 to 2006, and 2011 to 2012, and the New York State Department of Environmental Conservation (NYSDEC) issued a Satisfactory Completion Letter in May 2023. The IRM for OU-2 was completed in 2018-2019, and the NYSDEC issued a Decision Document in June 2021 identifying the selected remedy as No Further Action with Monitoring. In accordance with the May 2023 SMP, annual Periodic Review Reports (PRRs) were submitted to the Department in October 2024 and October 2025. In accordance with the SMP and a letter from the NYSDEC to NYSEG dated October 20, 2025, the next PRR is due on October 15, 2030.

Post-construction monitoring was initiated by Arcadis in 2020 (i.e., “Year 1”); 2026 represents “Year 7” of post-construction monitoring. Site locations are shown on Figure 1.

2026 Monitoring and Gauging

Post-construction monitoring activities are generally used to assess the performance and effectiveness of the remedy. The 2026 monitoring and gauging program consisted of the following:

- Gauging to assess the presence/absence of non-aqueous phase liquid (NAPL);
- Water-level measurements to evaluate groundwater flow conditions;
- A site-wide inspection to assess the condition and effectiveness of the site cover system;
- Annual sheen monitoring at OU-2 to assess the presence of MGP-related sheens; and
- Annual riverbank monitoring at OU-2 to assess bank stability and erosion at the former Transloading Area.

Monitoring and gauging activity details are presented below. OU-1 and OU-2 IRM areas and monitoring well locations are shown on Figure 2.

Tracey Garland, GIT
New York State Department of Environmental Conservation
September 18, 2026

Annual NAPL and Water-Level Gauging

Arcadis conducted NAPL and water level gauging on April 21, 2026. NAPL gauging is performed to document locations where NAPL, if present, is accumulating. This information is used to assess the performance of the NAPL barrier wall and identify potential changes in subsurface NAPL migration. Groundwater elevations are measured to evaluate groundwater flow conditions and identify changes in flow patterns that could affect remedy performance. NAPL gauging results and groundwater elevation information are presented below.

NAPL Gauging Activities and Results

Field personnel used an oil-water interface probe and, as necessary, a weighted tape to measure and confirm groundwater levels, gauge accumulated NAPL, and measure the depth to bottom at each monitoring well, recovery well, and piezometer.

The 2026 NAPL gauging results for NAPL barrier wall recovery wells RW-1 through RW-22 and NAPL monitoring wells NMW-1 and NMW-2 are presented in Table 1. Gauging results for the remaining upgradient, side gradient, and sentinel wells are presented in Table 2. For reference, previous gauging results from June 2024 to April 2026 are included in Tables 1 and 2.

NAPL gauging results were generally consistent with those from previous monitoring events. NAPL was not present at a measurable thickness (i.e., recoverable quantity) at any monitoring location. Trace amounts of DNAPL, observed as blebs on the interface probe, were detected at RW-6. This condition was first observed in 2022 and has persisted during subsequent monitoring events, including those conducted in November 2023 and June 2024. These observations suggest that a limited quantity of DNAPL has entered the barrier wall at this location.

The NAPL barrier wall was designed to prevent mobile NAPL, if any, from migrating beyond the Site by acting as a capillary break. The wall intercepts migrating NAPL, causing DNAPL to sink to the bottom of the barrier and LNAPL to accumulate at the water table within the wall. This process reduces NAPL saturation and allows for recovery and proper disposal of any accumulated product. No recoverable amounts of NAPL have been observed in any of the gauged wells since post-IRM gauging began in 2020.

The 2026 gauging results indicate that significant NAPL migration into the barrier wall is not occurring and that the barrier wall is performing as designed.

Groundwater Elevation and Flow

Field personnel conducted synoptic water-level measurements in conjunction with NAPL gauging. Groundwater elevation measurements were taken from surveyed marks on the top of the inner well casings and converted to elevations. Groundwater elevation data are summarized in Tables 1 and 2. Water table and sand-and-gravel unit potentiometric surface maps for the April 2026 monitoring event are included as Figures 3 and 4, respectively. For comparison, water table and potentiometric surface maps from the 2002 RI are included as Attachment 1.

When comparing the 2002 and 2026 maps, the following should be considered:

- The 2002 maps were drawn using a greater number of data points; many of the previous monitoring wells and piezometers were removed during and following remedial construction activities.

- The 2002 water table map represents water table elevations prior to installing the NAPL barrier wall. The NAPL barrier wall is comprised of a permeable gravel-filled trench keyed into low-permeability till, with several short segments composed of low-permeability jet-grout panels. A high-density polyethylene (HDPE) curtain lines the downgradient side of the gravel-filled portions of the barrier, extending below the water table. The grout panels and HDPE curtain serve as barriers to groundwater flow. As a result, the gravel-filled trench, grout panels, and HDPE curtain alter local groundwater flow patterns.

Therefore, both historical groundwater flow patterns and inferred groundwater flow effects associated with the NAPL barrier wall were considered when preparing Figures 3 and 4.

The April 2026 configuration of the water table (Figure 3) is similar to that mapped in 2002. Groundwater is mounded atop the underlying silt-and-clay unit near the center of the Site. This groundwater moves radially away from the mound and either 1) spills off the eastern and western edges of the silt-and-clay unit or 2) enters the gravel-filled portions of the NAPL barrier wall, where groundwater moves downward beneath the HDPE curtain and into the underlying sand-and-gravel unit. Downgradient of the NAPL barrier wall and beneath Court Street, shallow groundwater generally moves south toward the flood wall and downward beneath the flood wall, eventually discharging into the Susquehanna River.

The April 2026 configuration of the sand-and-gravel unit potentiometric surface (Figure 4) is also similar to that mapped in 2002. The relatively high permeability of the sand-and-gravel unit results in a potentiometric surface that slopes gently southward toward the river. Flow in the sand-and-gravel unit is primarily horizontal, except near the river where groundwater moves upward, discharging into the Susquehanna River.

The groundwater elevation data indicate that flow patterns remain generally consistent with historical conditions and that the NAPL barrier wall continues to perform as intended.

Site Inspection

In accordance with the SMP, Arcadis conducted a site inspection to evaluate site usage, general site conditions, and the condition of the cover system. As documented on the Site Inspection Form (Attachment 2), the asphalt and gravel cover system were intact, with no evidence of intrusive activities.

Monitoring well and piezometer surface completions were inspected and found to be in satisfactory condition, with the exception of off-site shallow monitoring well MW01-17S. The protective cover and PVC riser were damaged; however, the well remained accessible, and gauging activities yielded reliable data. The well will be repaired once current activities at the off-site location are completed.

Sheen Monitoring

The SMP requires periodic visual monitoring for sheens within the OU-2 sediment removal areas. The occurrence of sheens may indicate a change in subsurface conditions and that the remedy may not be performing as designed. Quarterly sheen monitoring was conducted in 2021 and 2022, followed by semi-annual sheen monitoring in 2023 and 2024 and annual sheen monitoring beginning in 2025. Annual sheen monitoring will be conducted through 2030.

Arcadis conducted the annual sheen monitoring in April 2026 from the Tompkins Street Bridge. Photographs documenting river conditions at the beginning and end of the monitoring event are included in the Sheen and Riverbank Monitoring Memorandum (Attachment 3). No sheens were observed during the 2026 monitoring event,

indicating that the remedy is performing as designed. In accordance with the SMP, sheen monitoring will be conducted on an annual basis. The next monitoring event is scheduled for 2027.

Riverbank Monitoring

To facilitate river access during OU-2 IRM construction activities, a temporary Transloading Area was constructed approximately 1,000 feet upstream of the OU-2 IRM areas (Figure 1). The Transloading Area was subsequently restored following completion of the 2019 IRM in accordance with the Transloading Area Restoration Plan.

Transloading Area monitoring activities include periodic inspection for visual evidence of erosion, settlement, or soil instability within the restored riverbank area, in accordance with the Transloading Area Restoration Plan. During the May 2022 riverbank inspection, erosion was noted at the upstream end of the restored riverbank area and documented in the 2022 Transloading Area Restoration Monitoring Report, which was submitted to the Department on December 22, 2022. The riverbank erosion area is shown on Figure 5.

During the riverbank inspection completed in April 2026, no evidence of further erosion, settlement, or soil instability was observed within the restored bank area. Specifically, the area where erosion was previously observed appeared unchanged from previous inspections, and no evidence of continued erosion or bank instability was observed. Photographs from the April 2026 inspection are included in the Sheen and Riverbank Monitoring Memorandum (Attachment 3).

Summary and Conclusions

Based on the 2026 post-construction monitoring results:

- There is no evidence that NAPL is migrating on-site or off-site. No recoverable quantities of NAPL were observed in the NAPL barrier wall recovery wells or in monitored off-site wells during the 2026 monitoring event. Further, recoverable quantities of NAPL have not been observed in the NAPL barrier wall recovery wells since they were installed (i.e., in 2006).
- Post-remediation groundwater flow patterns remain generally consistent with pre-remediation conditions; except near the NAPL barrier wall, where shallow groundwater in the fill and silt-and-clay unit generally enters the gravel-filled portions of the trench and moves downward into the sand-and-gravel unit as designed. Consistent with pre-remediation conditions, groundwater within the sand-and-gravel unit flows southward and discharges into the Susquehanna River. Shallow groundwater downgradient of the NAPL barrier wall also discharges to the Susquehanna River.
- The Site cover system is intact with no evidence of intrusive activities.
- No sheens were observed within the OU-2 IRM areas.
- The restored riverbank area appeared stable, and the area of minor erosion documented during the May 2022 inspection appeared unchanged.

Data collected and observations made during the 2026 post-remediation monitoring event demonstrate that the various remedial components are performing as designed.

Schedule

Consistent with the monitoring and reporting requirements presented in the SMP:

Tracey Garland, GIT
New York State Department of Environmental Conservation
September 18, 2026

- Site-wide inspections and NAPL monitoring will continue on an annual basis, with the next event tentatively scheduled for Q2 2027.
- Sheen monitoring will be conducted on an annual basis, with the next event tentatively scheduled for Q2 2027.
- Transloading Area riverbank stability monitoring will be completed concurrently with each sheen monitoring event.
- The next annual monitoring report will be submitted in Q4 2027.
- The next groundwater sampling event will occur in 2030 (i.e., post-construction Year 11).

Please contact Levia Terrell of NYSEG at 607.423.1652 or lterrell@nyseg.com with any questions or comments.

Sincerely,
Arcadis of New York, Inc.



Joe Bistrovich
Senior Environmental Engineer

Email: joe.bistrovich@arcadis.com
Direct Line: 315.671.9697
Mobile: 315.427.4585

CC. Levia Terrell, NYSEG
Keith White, PG, Arcadis
Mark Gravelding, PE, Arcadis

Enc: Table 1 – NAPL Recovery Well Gauging and Groundwater Elevation Summary
Table 2 – Upgradient, Side-Gradient, and Sentinel Well Gauging and Groundwater Elevation Summary

Figure 1 – Site Map
Figure 2 – OU-1, OU-2 IRM Areas, and Monitoring Well Locations
Figure 3 – Water Table Map
Figure 4 – Sand and Gravel Potentiometric Surface Map
Figure 5 – Transloading Area

Attachment 1 – Select RI Figures
Attachment 2 – Site Inspection Form
Attachment 3 – Sheen and Riverbank Monitoring Memorandum

Tables

Table 1
NAPL Recovery Well Gauging and Groundwater Elevation Summary
2026 Post-Construction Monitoring Report
NYSEG - Binghamton Former MGP Site
Binghamton, New York

Well ID	Top of Casing (ft AMSL)	6/20/2024					4/22/2025					4/21/2026				
		DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)	Comments/ Observations	DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)	Comments/ Observations	DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)	Comments/ Observations
Barrier Wall Recovery Wells																
RW-1	841.76	10.56	51.39	831.20	--		9.62	51.11	832.14	--		9.95	51.50	831.81	--	
RW-2	841.97	10.89	51.58	831.08	--		9.83	51.58	832.14	--		9.27	51.61	832.70	--	
RW-3	842.01	10.88	15.89	831.13	--		9.84	15.86	832.17	--		9.23	15.98	832.78	--	
RW-4	842.99	11.90	15.82	831.09	--		10.86	15.39	832.13	--	No bolts	10.21	15.83	832.78	--	Soft bottom
RW-5	842.97	11.85	48.51	831.12	--		10.83	48.57	832.14	--	No bolts	10.25	48.48	832.72	--	
RW-6	843.57	12.20	48.37	831.37	0.3'	Blebs	11.25	48.28	832.32	--		10.55	48.33	833.02	blebs	Soft bottom
RW-7	843.58	12.21	18.04	831.37	--		11.27	17.77	832.31	--		10.55	17.94	833.03	--	
RW-8	843.49	12.13	17.98	831.36	--		11.19	17.25	832.30	--		10.50	17.97	832.99	--	
RW-9	843.53	12.17	52.87	831.36	--		11.24	51.74	832.29	--		10.51	52.88	833.02	--	
RW-10	843.08	11.71	16.30	831.37	--		10.78	15.76	832.30	--		10.02	16.25	833.06	--	
RW-11	843.92	12.57	39.70	831.35	--	Soft bottom	11.65	39.57	832.27	--		10.89	39.82	833.03	--	Sediment
RW-12	844.78	13.35	47.86	831.43	--		12.55	47.72	832.23	--		11.69	47.85	833.09	--	
RW-13	844.78	13.36	18.47	831.42	--		12.54	17.86	832.24	--		11.69	18.60	833.09	--	
RW-14	845.80	14.39	17.09	831.41	--		13.58	17.03	832.22	--		12.70	17.01	833.10	--	
RW-15	845.77	14.35	48.82	831.42	--		13.53	48.82	832.24	--		12.69	48.81	833.08	--	
RW-16	848.38	16.93	49.57	831.45	--		16.16	48.53	832.22	--		15.29	49.57	833.09	--	
RW-17	848.34	16.91	24.44	831.43	--		16.12	24.45	832.22	--		15.25	24.46	833.09	--	
RW-18	845.85	14.37	41.86	831.48	--		13.57	41.84	832.28	--		12.69	41.83	833.16	--	
RW-19	845.87	14.38	19.18	831.49	--		13.66	19.13	832.21	--		12.70	19.25	833.17	--	
RW-20	841.76	10.53	15.09	831.23	--		9.54	13.13	832.22	--	Missing brace in box (middle bolt)	9.90	14.89	831.86	--	Soft
RW-21	841.99	10.67	15.77	831.32	--		9.63	15.68	832.36	--	Bolts stripped	9.20	15.75	832.79	--	
RW-22	841.97	10.58	51.60	831.39	--		9.60	50.80	832.37	--		8.95	51.75	833.02	--	Soft bottom
NAPL Recovery Wells																
NMW-1[2]	849.94	18.54	40.75	831.40	--		17.69	40.48	832.25	--		16.94	41.01	833.00	--	Soft
NMW-2	846.12	14.93	43.34	831.19	--		14.07	40.48	832.05	--		13.33	43.34	832.79	--	

Acronyms and Abbreviations:

AMSL = Feet Above Mean Sea Level
DTB = depth to bottom
DTW = depth to water
ft = feet
NAPL = non-aqueous phase liquid
TIC = Top of Inner Casing
TR = trace
-- = NAPL not present

Notes:

1. Elevations referenced to the NAVD 88 Datum.
2. Not included as part of the monitoring well network.

Table 2
Upgradient, Side-Gradient, and Sentinel Well Gauging and Groundwater Elevation Summary
2025 Periodic Review Report
NYSEG - Binghamton Former MGP Site
Binghamton, New York

Well ID	Ground Surface Elevation (ft AMSL)	Measuring Point Elevation (ft AMSL)[2]	6/20/2024				4/22/2025				4/21/2026			
			DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)	DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)	DTW (ft-TIC)	DTB (ft-TIC)	Groundwater Elevation (ft AMSL)	NAPL Thickness (ft)
Upgradient and Side-Gradient Wells														
PZ93-1	844.70	848.00	11.22	12.25	836.78	--	10.04	12.18	837.96	--	9.95	12.24	838.05	--
MW93-06D	844.20	845.98	14.24	67.23	831.74	--	13.49	67.18	832.49	--	12.52	67.21	833.46	--
MW93-05D	844.90	847.16	15.41	59.11	831.75	--	14.52	59.14	832.64	--	13.75	59.12	833.41	--
MW97-14D	845.90	844.88	12.85	37.70	832.03	--	11.89	37.76	832.99	--	11.25	37.76	833.63	--
MW97-14S	845.90	844.87	12.95	18.84	831.92	--	11.93	18.82	832.94	--	11.27	18.90	833.60	--
MW01-17D	861.50	860.79	29.05	58.90	831.74	--	28.37	58.71	832.42	--	27.31	52.92	833.48	--
MW01-17S[3]	861.70	860.86	29.36	36.59	831.50	--	28.63	32.33	832.23	--	27.75	32.22	833.11	--
Sentinel Wells														
MW97-07S	849.33	848.97	17.89	25.82	831.08	--	16.99	25.78	831.98	--	16.36	25.81	832.61	--
MW-01-03-R	--	846.56	14.46	111.05	832.10	--	13.52	111.12	833.04	--	12.63	111.02	833.93	--
PZ03-01D	848.10	847.61	16.21	46.67	831.40	--	15.36	46.69	832.25	--	14.70	46.65	832.91	--
PZ03-09A	851.19	850.86	19.90	12.68	830.96	--	19.06	22.37	831.80	--	18.43	22.45	832.43	--
PZ03-09D	851.19	850.81	19.62	47.51	831.19	--	18.72	47.09	832.09	--	18.05	47.52	832.76	--
Surface Water Elevation														
MP-1 (Tompkins St. Bridge)	--	858.19	27.51	--	830.68	--	26.32	--	831.87	--	25.90	--	832.29	--

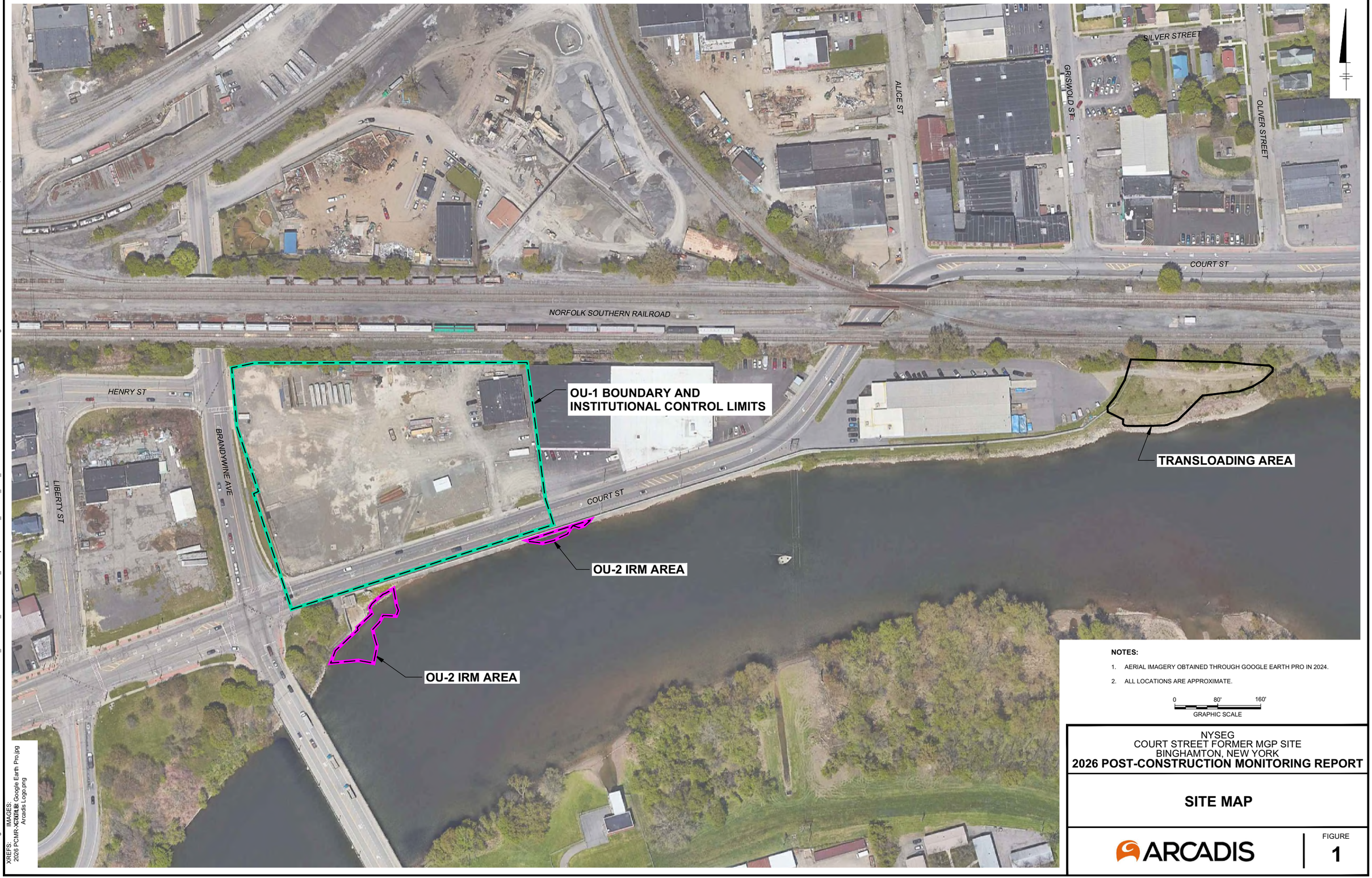
Acronyms and Abbreviations:

AMSL = Feet Above Mean Sea Level
DTB = depth to bottom
DTW = depth to water
ft = feet
NAPL = non-aqueous phase liquid
TIC = Top of Inner Casing
NA = not accessible
NM = not measured
St. = street

Notes:

1. NAPL gauging and water level data collected by Arcadis on the dates indicated.
2. All elevations referenced to feet above mean sea level, North American Vertical Datum of 1988. Monitoring Well network was resurveyed in 2021.
3. During the 2026 gauging event, the MW01-17S cover and riser were discovered to be damaged by ongoing construction.

Figures



NOTES:

1. AERIAL IMAGERY OBTAINED THROUGH GOOGLE EARTH PRO IN 2024.
2. ALL LOCATIONS ARE APPROXIMATE.

0 80' 160'
GRAPHIC SCALE

NYSEG
COURT STREET FORMER MGP SITE
BINGHAMTON, NEW YORK
2026 POST-CONSTRUCTION MONITORING REPORT

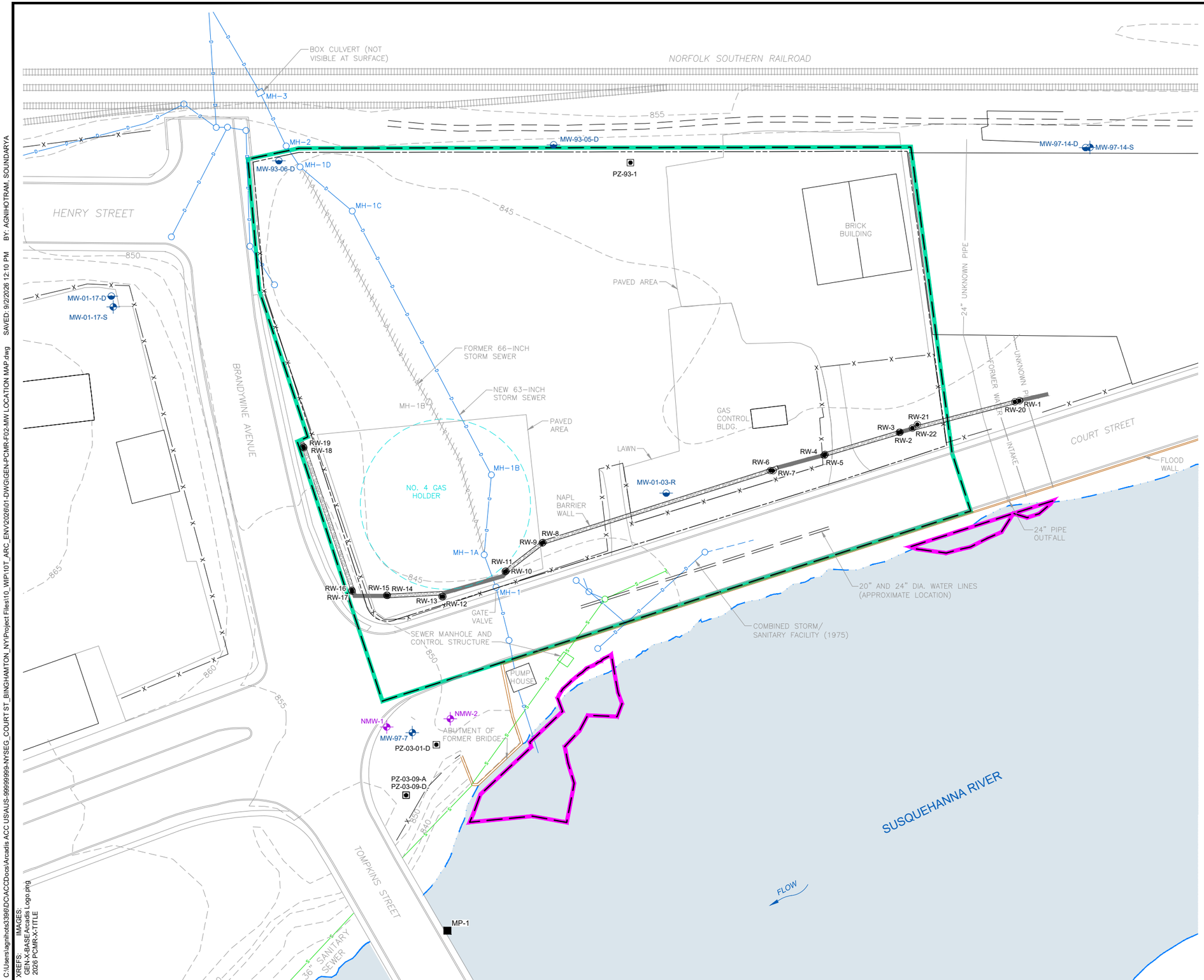
SITE MAP

ARCADIS

FIGURE
1

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IMAGES:
2026 PCMR-XCTRL1
Google Earth Pro.jpg
Arcadis Logo.png



NOTES:

1. BASE MAP PROVIDED BY NYSEG (JUNE 12, 1997).
2. GROUND SURFACE CONTOURS DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303; FLOWN DECEMBER 2, 1973 AND MAPPED APRIL 1, 1974.
3. BASEMAP REFERENCE DATUMS:
   HORIZONTAL: NORTH AMERICAN VERTICAL DATUM OF 1983 (NAD83), NEW YORK STATE PLANE CENTRAL ZONE 3102.
   VERTICAL: NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD29).
4. STORM SEWER LOCATION DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, ENTITLED: PRELIMINARY REPORT, COMPREHENSIVE STORM DRAINAGE, EXISTING FACILITIES, PREPARED BY VERNON 0. SHUMAKER, CONSULTING ENGINEER, VESTAL, NEW YORK, DATE NOT PROVIDED.
5. ALL LOCATIONS ARE APPROXIMATE. SITE PLAN DEPICTS BOTH HISTORICAL AND CURRENT SITE FEATURES, INCLUDING MONITORING WELLS.
6. OU-2 CONSISTS OF SEDIMENTS IMPACTED BY THE SITE WHICH WERE REMEDIATED IN 2018/2019.

0 40' 80'
GRAPHIC SCALE

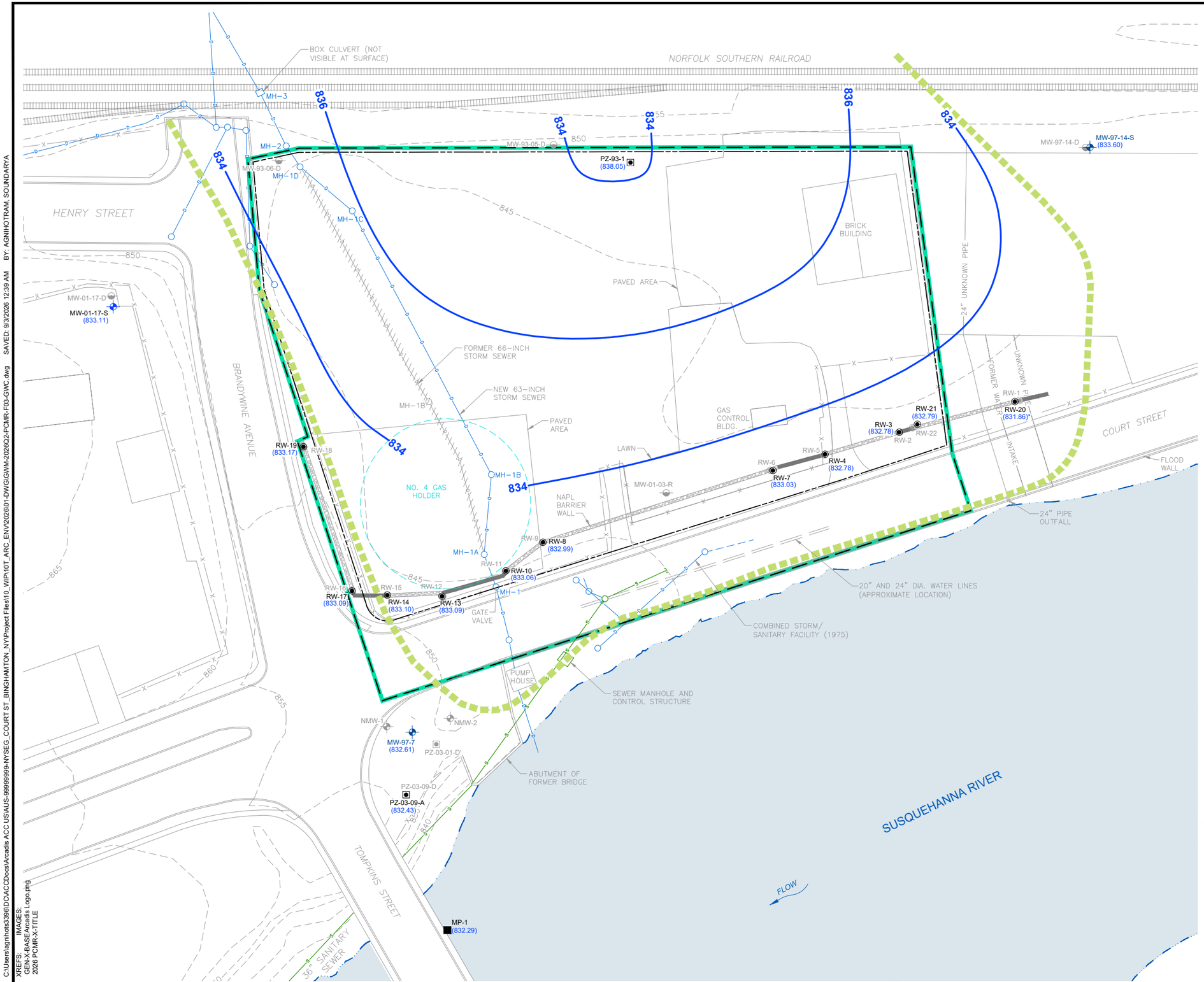
NYSEG
COURT STREET FORMER MGP SITE
BINGHAMTON, NEW YORK
2026 POST-CONSTRUCTION MONITORING REPORT

MONITORING WELL LOCATIONS

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FIGURE
2

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2026 PCMR-X-TITLE



LEGEND:

- NYSEG PROPERTY LINES AND ENVIRONMENTAL EASEMENT LIMITS
- OPERABLE UNIT BOUNDARY AND INSTITUTIONAL CONTROL LIMITS
- 840 GROUND SURFACE ELEVATION (FT AMSL)
- FENCE
- RAILROAD TRACK
- FLOOD WALL
- SANITARY SEWER LINE
- STORM SEWER LINE
- ABANDONED STORM SEWER LINE
- HISTORIC FEATURE
- EDGE OF SILT
- NAPL BARRIER WALL
- JET GROUT PANEL
- SHALLOW MONITORING WELL
- DEEP MONITORING WELL
- NAPL MONITORING WELL
- PIEZOMETER
- RECOVERY WELL
- RIVER STAFF GAUGE
- 838 WATER TABLE CONTOUR (SEE NOTE 6)
- (838.05) GROUNDWATER ELEVATION (SEE NOTE 6)

NOTES:

1. BASE MAP PROVIDED BY NYSEG (JUNE 12, 1997).
2. GROUND SURFACE CONTOURS DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303; FLOWN DECEMBER 2, 1973 AND MAPPED APRIL 1, 1974.
3. GROUND SURFACE AND BASEMAP REFERENCE DATUMS:VERTICAL: NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD29) HORIZONTAL: NORTH AMERICAN DATUM OF 1983 (NAD83), NEW YORK STATE PLANE, CENTRAL ZONE 3102
4. STORM SEWER LOCATION DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, ENTITLED: PRELIMINARY REPORT, COMPREHENSIVE STORM DRAINAGE, EXISTING FACILITIES, PREPARED BY VERNON 0. SHUMAKER, CONSULTING ENGINEER, VESTAL, NEW YORK, DATE NOT PROVIDED.
5. ALL LOCATIONS ARE APPROXIMATE. SITE PLAN DEPICTS BOTH HISTORICAL AND CURRENT SITE FEATURES, INCLUDING MONITORING WELLS.
6. GROUNDWATER ELEVATIONS REFERENCED TO MEAN SEA LEVEL USING NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
7. * WATER LEVEL ANOMALOUS BASED ON HISTORICAL DATA AND THEREFORE NOT USED IN CONTOURING.

0 40' 80'
GRAPHIC SCALE

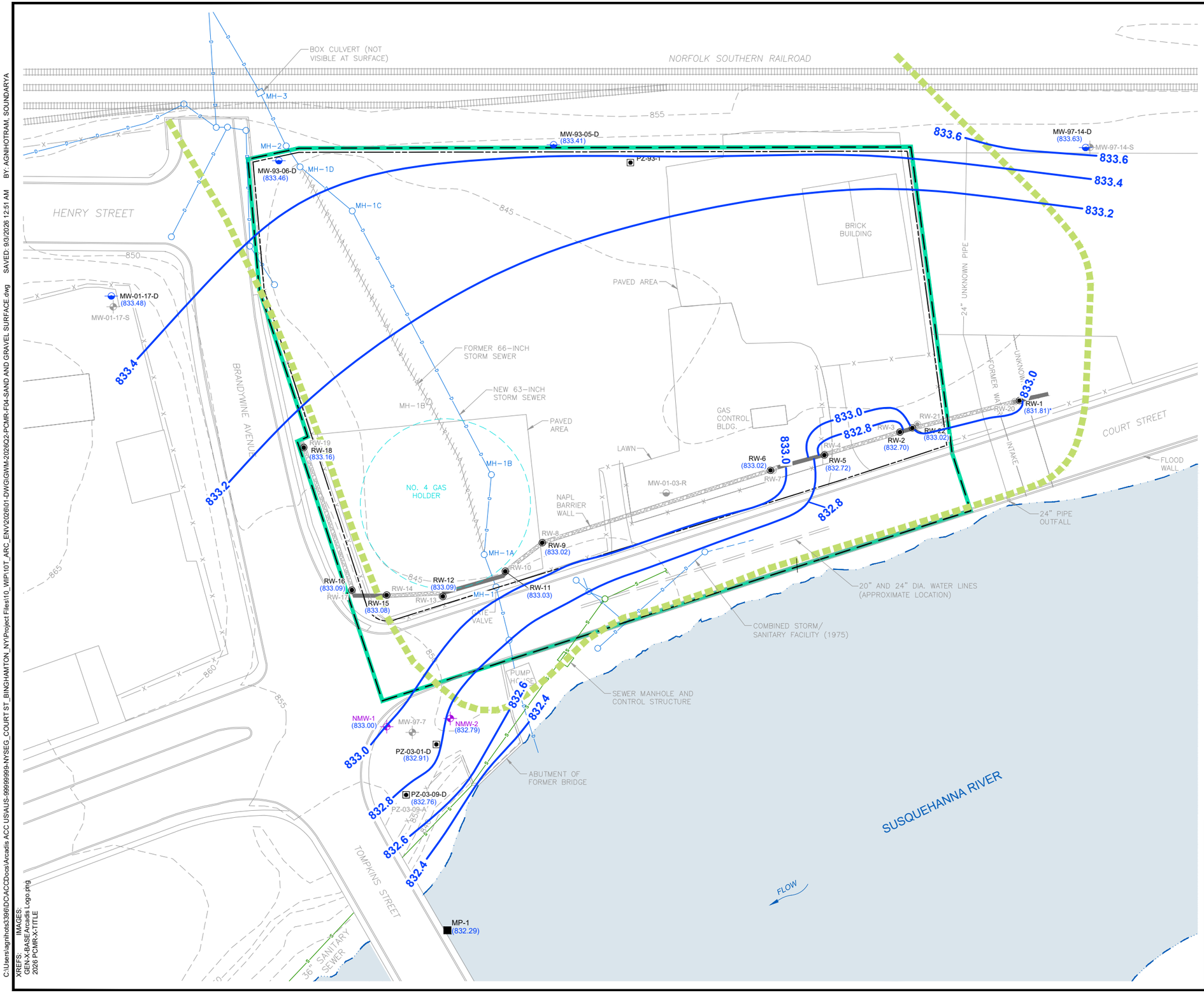
NYSEG
COURT STREET FORMER MGP SITE
BINGHAMTON, NEW YORK
2026 POST-CONSTRUCTION MONITORING REPORT

WATER TABLE MAP - APRIL 21, 2026

ARCADIS

FIGURE 3

C:\Users\agnihotr33960\DOCACCDocs\Arcadis ACC USAUS-99999999-NYSEG_COURT ST_BINGHAMTON_NY\Project Files\10_WIP\10T_ARC_ENV\2026\01-DWG\GWM-2026Q2-PCMR-F03-GWC.dwg SAVED: 9/3/2026 12:39 AM BY: AGNIHOTRAM, SOUNDARYA
XREFS: IMAGES: GEN-X-BASE Arcadis Logo.png 2026 PCMR-X-TITLE



LEGEND:

- NYSEG PROPERTY LINES AND ENVIRONMENTAL EASEMENT LIMITS
- OPERABLE UNIT BOUNDARY AND INSTITUTIONAL CONTROL LIMITS
- 840 GROUND SURFACE ELEVATION (FT AMSL)
- FENCE
- RAILROAD TRACK
- FLOOD WALL
- SANITARY SEWER LINE
- STORM SEWER LINE
- ABANDONED STORM SEWER LINE
- HISTORIC FEATURE
- EDGE OF SILT
- NAPL BARRIER WALL
- JET GROUT PANEL
- SHALLOW MONITORING WELL
- DEEP MONITORING WELL
- NAPL MONITORING WELL
- PIEZOMETER
- RECOVERY WELL
- RIVER STAFF GAUGE
- 833.6 POTENTIOMETRIC CONTOUR (DASHED WHERE INFERRED) (SEE NOTE 6)
- (833.63) GROUNDWATER ELEVATION (SEE NOTE 6)

NOTES:

1. BASE MAP PROVIDED BY NYSEG (JUNE 12, 1997).
2. GROUND SURFACE CONTOURS DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303; FLOWN DECEMBER 2, 1973 AND MAPPED APRIL 1, 1974.
3. GROUND SURFACE AND BASEMAP REFERENCE DATUMS:VERTICAL: NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929 HORIZONTAL: NORTH AMERICAN DATUM (NAD) OF 1983, NEW YORK STATE PLANE, CENTRAL ZONE 3102.
4. STORM SEWER LOCATION DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, ENTITLED: PRELIMINARY REPORT, COMPREHENSIVE STORM DRAINAGE, EXISTING FACILITIES, PREPARED BY VERNON 0. SHUMAKER, CONSULTING ENGINEER, VESTAL, NEW YORK, DATE NOT PROVIDED.
5. ALL LOCATIONS ARE APPROXIMATE. SITE PLAN DEPICTS BOTH HISTORICAL AND CURRENT SITE FEATURES, INCLUDING MONITORING WELLS.
6. GROUNDWATER ELEVATIONS REFERENCED TO MEAN SEA LEVEL USING NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
7. * WATER LEVEL ANOMALOUS BASED ON HISTORICAL DATA AND THEREFORE NOT USED IN CONTOURING.

0 40' 80'
GRAPHIC SCALE

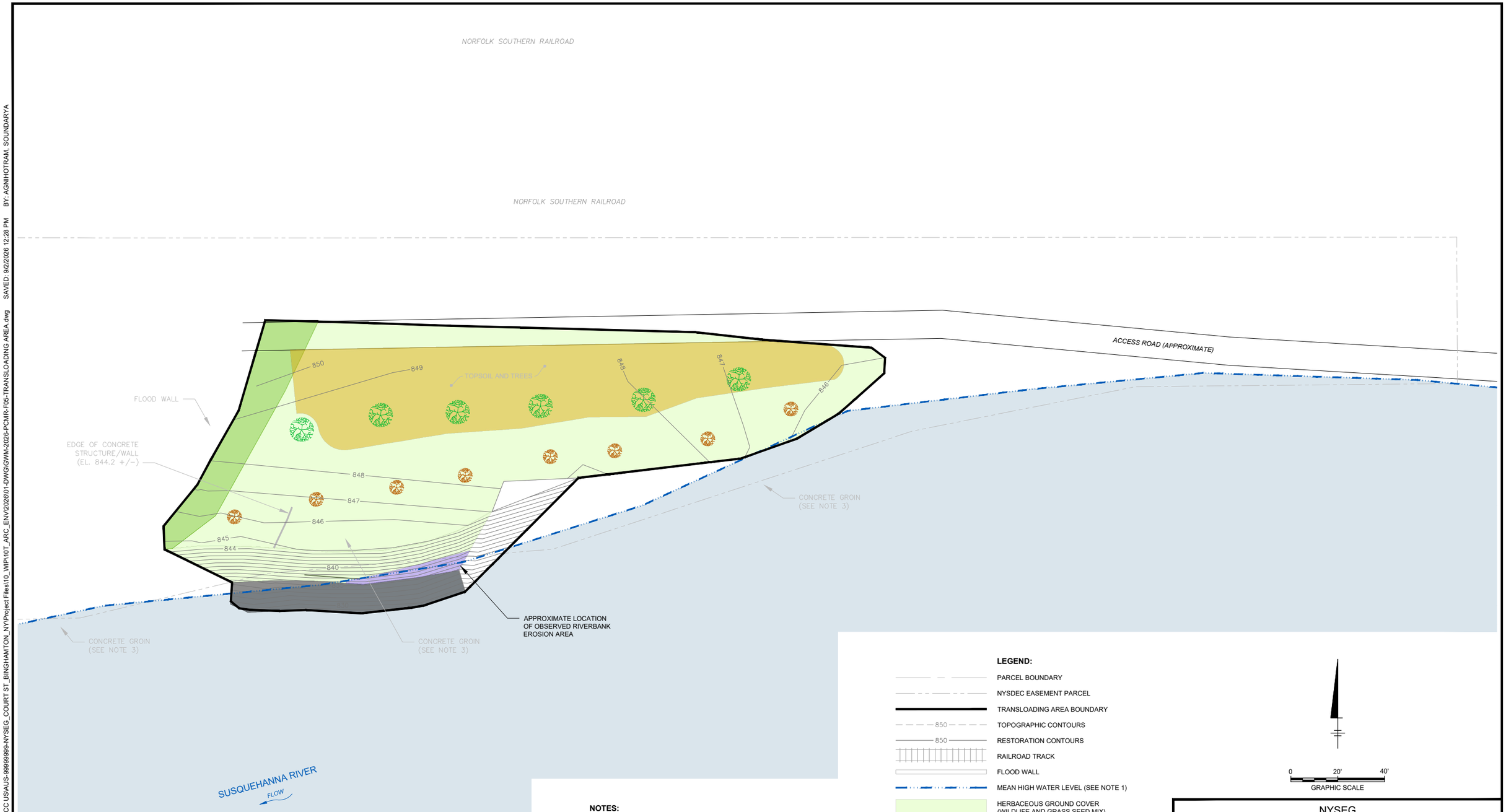
NYSEG
COURT STREET FORMER MGP SITE
BINGHAMTON, NEW YORK
2026 POST-CONSTRUCTION MONITORING REPORT

SAND AND GRAVEL POTENTIOMETRIC SURFACE MAP - APRIL 21, 2026

ARCADIS

FIGURE
4

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XREFS:
IMAGES:
GEN-X-BASE Arcadis Logo.png
2026 PCMR-X-TITLE



NOTES:

1. MEAN HIGH WATER LEVEL DELINEATED IN THE FIELD BY ARCADIS IN OCTOBER 2017, IN ACCORDANCE WITH RGL 05-05.
2. TREE LOCATIONS ARE APPROXIMATE.
3. CONCRETE GROINS NOT ENCOUNTERED IN THE FIELD AND ARE SHOWN PER NYSDEC'S REQUEST. LOCATIONS ARE APPROXIMATE BASED ON DRAWING FROM SOUTHERN NEW YORK FLOOD CONTROL PROJECT, BINGHAMTON NY SECTION NO. 3, PLAN AND PROFILE, FILE NO. 6406-44, ISSUED 10/22/1940, REVISED 8/1/41.
4. HORIZONTAL DATUM (US SURVEY FEET): NORTH AMERICAN DATUM OF 1983 (NAD83), NEW YORK STATE PLANE COORDINATE SYSTEM, NEW YORK CENTRAL ZONE, FIPSZONE 3102.

LEGEND:

- PARCEL BOUNDARY
- NYSDEC EASEMENT PARCEL
- TRANSLOADING AREA BOUNDARY
- 850 TOPOGRAPHIC CONTOURS
- 850 RESTORATION CONTOURS
- RAILROAD TRACK
- FLOOD WALL
- MEAN HIGH WATER LEVEL (SEE NOTE 1)
- HERBACEOUS GROUND COVER (WILDLIFE AND GRASS SEED MIX)
- HERBACEOUS GROUND COVER (FLOOD WALL SEED MIX)
- NYSDEC RESEEDED AREA
- SILVER MAPLES
- RED MAPLES
- RIPRAP

0 20' 40'
GRAPHIC SCALE

NYSEG
COURT STREET FORMER MGP SITE
BINGHAMTON, NEW YORK
2026 POST-CONSTRUCTION MONITORING REPORT

TRANSLOADING AREA

ARCADIS

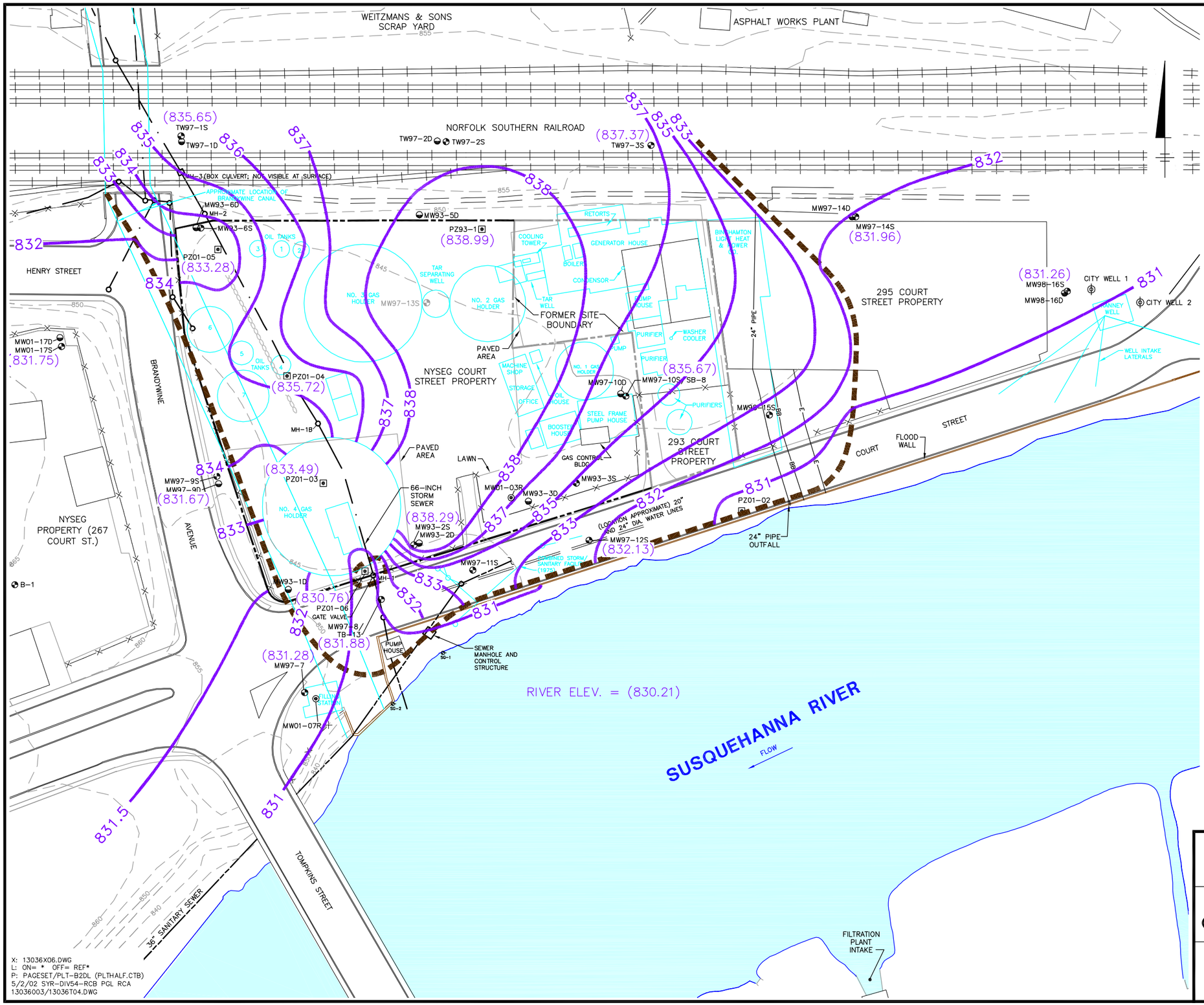
FIGURE
5

C:\Users\agnihotra3396\DCACCDocs\Arcadis ACC US\AUS-99999999-NYSEG_COURT ST_BINGHAMTON_NY\Project Files\10_WIP\10T_ARC_ENV\2026\01-DWG\GWM-2026-PCMR-F05-TRANSLOADING AREA.dwg SAVED: 9/2/2026 12:28 PM BY: AGNIHOTRAM, SOUNDARYA

XREFS: IMAGES:
GEN-X-BASE\Arcadis Logo.png
X-SP
X-PARCEL
X-CONTOURS
2026 PCMR-X-TITLE

Attachment 1

Select RI Figures



LEGEND

- FLOOD WALL
- RAILROAD TRACK
- FENCE
- SITE PROPERTY LINES (APPROXIMATE)
- SEWER LINE (APPROXIMATE)
- 840 GROUND SURFACE ELEVATION CONTOUR (FT. AMSL)
- HISTORICAL FEATURE
- BURIED CONCRETE WALL
- MONITORING WELL (SHALLOW)
- MONITORING WELL (DEEP)
- MONITORING WELL (BEDROCK)
- PIEZOMETER
- CITY MONITORING WELL
- STAFF GAUGE
- DECOMMISSIONED MONITORING WELL
- (831.26) WATER TABLE ELEVATION (FT. AMSL)
- 832 WATER TABLE ELEVATION CONTOUR (FT. AMSL) DASHED WHERE INFERRED
- EDGE OF SILT

NOTES:

1. BASE MAP PROVIDED BY NYSEG (JUNE 12, 1997).
2. SURFACE ELEVATIONS DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, FLOWN DECEMBER 2, 1973 AND MAPPED APRIL 1, 1974.
3. ALL ELEVATIONS ARE REFERENCED TO MEAN SEA LEVEL USING NATIONAL GEODETIC VERTICAL DATUM OF 1929, HORIZONTAL DATUM: NAD 83 NEW YORK STATE CENTRAL 3102.
4. STORM SEWER LOCATION DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, ENTITLED: PRELIMINARY REPORT, COMPREHENSIVE STORM DRAINAGE, EXISTING FACILITIES. PREPARED BY VERNON O. SHUMAKER, CONSULTING ENGINEER, VESTAL, NEW YORK, DATE NOT PROVIDED.
5. ALL SURFACE SOIL SAMPLING, SOIL BORING, TEST BORING, MONITORING WELL, CITY WELL, AND TEMPORARY MONITORING WELL LOCATIONS SURVEYED BY HAWK ENGINEERING, P.C., BINGHAMTON, N.Y.

0 100' 200'

GRAPHIC SCALE

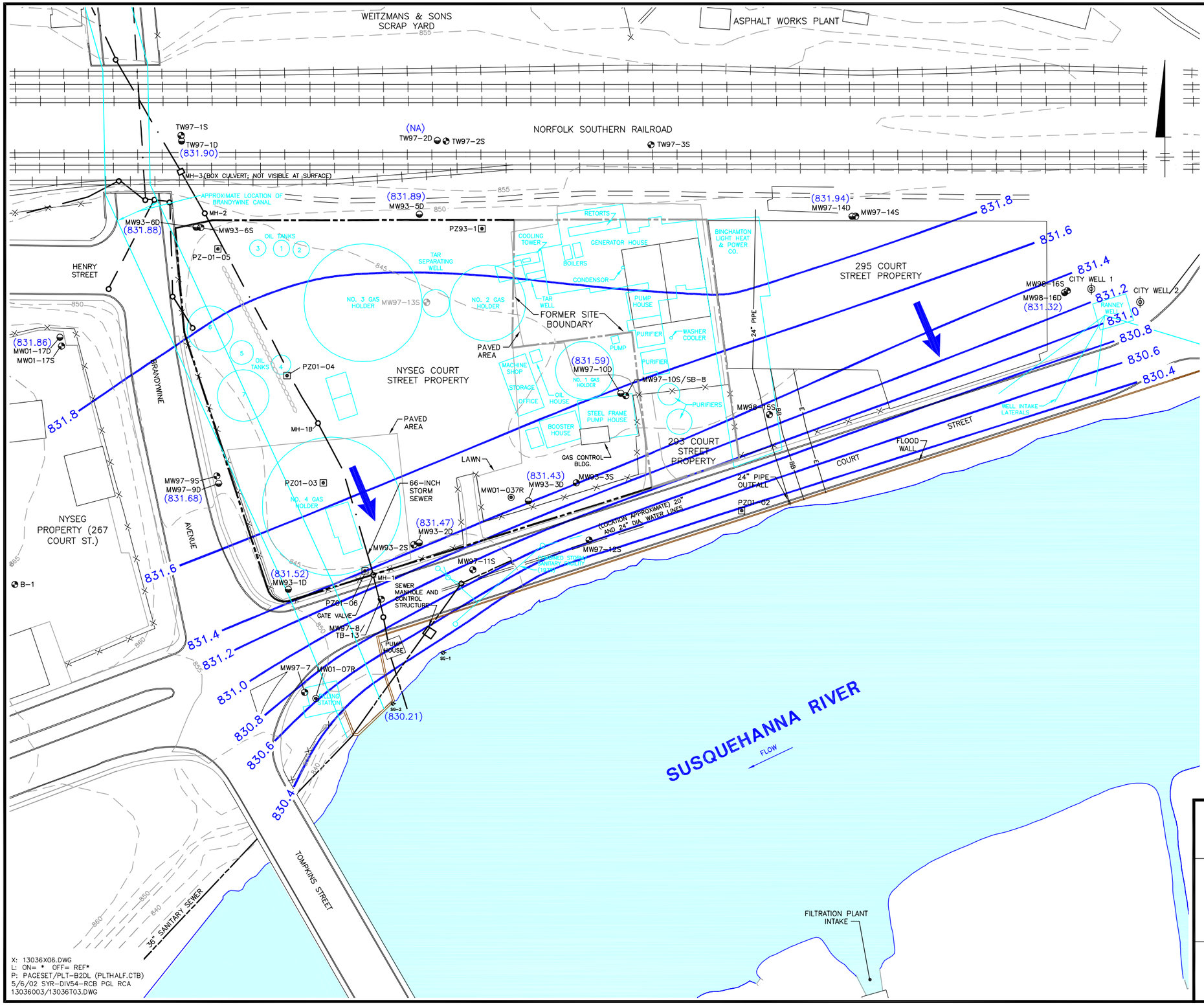
X: 13036X06.DWG
L: ON= * OFF= REF*
P: PAGESET/PLT-B2DL (PLTHALF.CTB)
5/2/02 SYR-DIV54-RCB PGL RCA
13036003/13036T04.DWG

NEW YORK STATE ELECTRIC & GAS CORP.
BINGHAMTON, NEW YORK
COURT STREET SITE
PHASE II SRI

WATER TABLE ELEVATION CONTOUR MAP - OCTOBER 2, 2001

BLASLAND, BOUCK & LEE, INC.
engineers & scientists

FIGURE 11



LEGEND

- FLOOD WALL
- RAILROAD TRACK
- FENCE
- SITE PROPERTY LINES (APPROXIMATE)
- SEWER LINE (APPROXIMATE)
- 840 GROUND SURFACE ELEVATION CONTOUR (FT. AMSL)
- HISTORICAL FEATURE
- BURIED CONCRETE WALL
- MONITORING WELL (SHALLOW)
- MONITORING WELL (DEEP)
- MONITORING WELL (BEDROCK)
- PIEZOMETER
- CITY MONITORING WELL
- STAFF GAUGE
- DECOMMISSIONED MONITORING WELL
- (831.68) DEEP GROUNDWATER POTENTIOMETRIC ELEVATION IN FEET ABOVE MEAN SEA LEVEL (NA = NOT AVAILABLE)
- 831.8 DEEP GROUNDWATER POTENTIOMETRIC ELEVATION CONTOUR (FT. AMSL)
- GROUNDWATER FLOW DIRECTION

NOTES:

1. BASE MAP PROVIDED BY NYSEG (JUNE 12, 1997).
2. SURFACE ELEVATIONS DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, FLOWN DECEMBER 2, 1973 AND MAPPED APRIL 1, 1974.
3. ALL ELEVATIONS ARE REFERENCED TO MEAN SEA LEVEL USING NATIONAL GEODETIC VERTICAL DATUM OF 1929, HORIZONTAL DATUM: NAD 83 NEW YORK STATE CENTRAL 3102.
4. ALL INVESTIGATION LOCATIONS SURVEYED BY HAWK ENGINEERING, P.C. BINGHAMTON, N.Y. EXCEPT THE FOLLOWING SEDIMENT PROBING AND SAMPLING LOCATIONS:
   - LOCATIONS WITH AN "SS" OR "SF" PREFIX;
   - LOCATIONS WITH A "-2" SUFFIX; AND
   - SEDIMENT SAMPLING LOCATIONS SED-1 AND SED-2.
5. ONLY WELLS INTERPRETED TO BE SCREENED IN THE SAND AND GRAVEL UNITS WERE USED FOR THESE CONTOURS.
6. CONTOUR INTERVAL = 0.2 FEET.
7. STORM SEWER LOCATION DIGITIZED FROM CITY OF BINGHAMTON MAP, SHEET 303, ENTITLED: PRELIMINARY REPORT, COMPREHENSIVE STORM DRAINAGE, EXISTING FACILITIES. PREPARED BY VERNON O. SHUMAKER, CONSULTING ENGINEER, VESTAL, NEW YORK, DATE NOT PROVIDED.

0 100' 200'

GRAPHIC SCALE

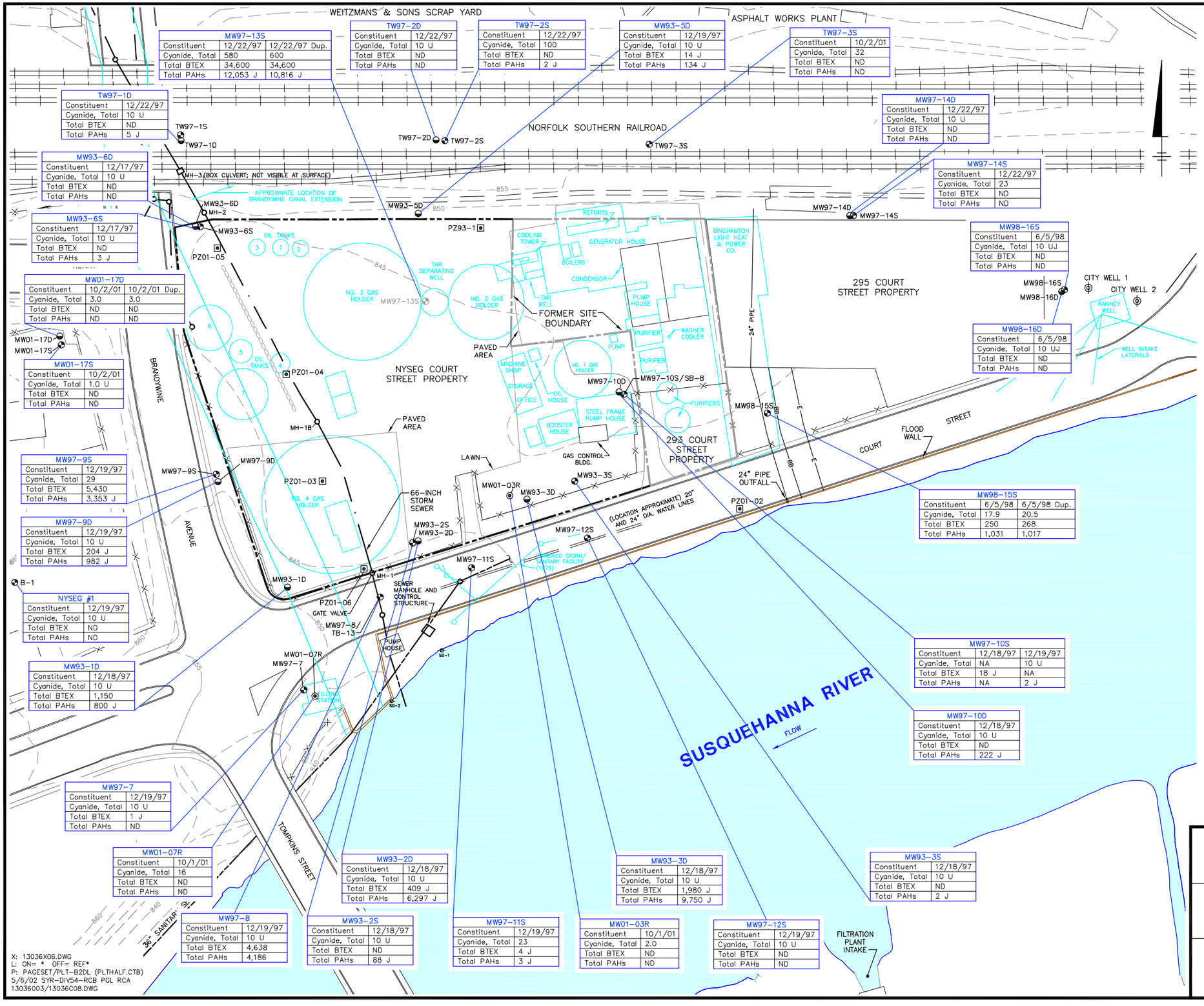
X: 13036X06.DWG
L: ON= * OFF= REF*
P: PAGESET/PLT-B2DL (PLTHALF.CTB)
5/6/02 SYR-DIV54-RCB PGL RCA
13036003/13036T03.DWG

NEW YORK STATE ELECTRIC & GAS CORP.
BINGHAMTON, NEW YORK
COURT STREET SITE
PHASE II SRI

SAND AND GRAVEL UNIT GROUNDWATER POTENTIOMETRIC ELEVATION CONTOUR MAP - OCTOBER 1, 2001

BBL BLASLAND, BOUCK & LEE, INC. engineers & scientists

FIGURE 12


WEITZMANS & SONS SCRAP YARD
ASPHALT WORKS PLANT
NORFOLK SOUTHERN RAILROAD
MW97-13S
Constituent | 12/22/97 | 12/22/97 Dup.
Cyanide, Total | 580 | 600
Total BTEX | 34,600 | 34,600
Total PAHs | 12,053 J | 10,816 J
TW97-2D
Constituent | 12/22/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | ND
TW97-2S
Constituent | 12/22/97
Cyanide, Total | 100
Total BTEX | ND
Total PAHs | 2 J
MW93-5D
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | 14 J
Total PAHs | 134 J
TW97-3S
Constituent | 10/2/01
Cyanide, Total | 32
Total BTEX | ND
Total PAHs | ND
TW97-1D
Constituent | 12/22/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | 5 J
MW97-14D
Constituent | 12/22/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | ND
MW97-14S
Constituent | 12/22/97
Cyanide, Total | 23
Total BTEX | ND
Total PAHs | ND
MW93-6D
Constituent | 12/17/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | ND
MW93-6S
Constituent | 12/17/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | 3 J
MW01-17D
Constituent | 10/2/01 | 10/2/01 Dup.
Cyanide, Total | 3.0 | 3.0
Total BTEX | ND | ND
Total PAHs | ND | ND
MW01-17S
Constituent | 10/2/01
Cyanide, Total | 1.0 U
Total BTEX | ND
Total PAHs | ND
MW98-16S
Constituent | 6/5/98
Cyanide, Total | 10 UJ
Total BTEX | ND
Total PAHs | ND
MW98-16D
Constituent | 6/5/98
Cyanide, Total | 10 UJ
Total BTEX | ND
Total PAHs | ND
MW98-15S
Constituent | 6/5/98 | 6/5/98 Dup.
Cyanide, Total | 17.9 | 20.5
Total BTEX | 250 | 268
Total PAHs | 1,031 | 1,017
MW97-9S
Constituent | 12/19/97
Cyanide, Total | 29
Total BTEX | 5,430
Total PAHs | 3,353 J
MW97-9D
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | 204 J
Total PAHs | 982 J
NYSEG #1
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | ND
MW93-1D
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | 1,150
Total PAHs | 800 J
MW97-10S
Constituent | 12/18/97 | 12/19/97
Cyanide, Total | NA | 10 U
Total BTEX | 18 J | NA
Total PAHs | NA | 2 J
MW97-10D
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | 222 J
MW97-7
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | 1 J
Total PAHs | ND
MW01-07R
Constituent | 10/1/01
Cyanide, Total | 16
Total BTEX | ND
Total PAHs | ND
MW93-2D
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | 409 J
Total PAHs | 6,297 J
MW93-3D
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | 1,980 J
Total PAHs | 9,750 J
MW93-3S
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | 2 J
MW97-8
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | 4,638
Total PAHs | 4,186
MW93-2S
Constituent | 12/18/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | 88 J
MW97-11S
Constituent | 12/19/97
Cyanide, Total | 23
Total BTEX | 4 J
Total PAHs | 3 J
MW01-03R
Constituent | 10/1/01
Cyanide, Total | 2.0
Total BTEX | ND
Total PAHs | ND
MW97-12S
Constituent | 12/19/97
Cyanide, Total | 10 U
Total BTEX | ND
Total PAHs | ND
TW97-1S
TW97-1D
TW97-2D
TW97-2S
TW97-3S
MW97-14D
MW97-14S
MW93-6D
MW93-6S
MW93-5D
MH-2
MH-3 (BOX CULVERT; NOT VISIBLE AT SURFACE)
APPROXIMATE LOCATION OF BRANDYWINE CANAL EXTENSION
PZ93-1
PZ01-05
PZ01-04
PZ01-03
PZ01-02
PZ01-06
MW01-17D
MW01-17S
MW01-03R
MW01-07R
MW97-7
MW97-8/TB-13
MW97-9S
MW97-9D
MW97-10D
MW97-10S/SB-8
MW97-11S
MW97-12S
MW93-1D
MW93-2S
MW93-2D
MW93-3D
MW93-3S
MW98-15S
MW98-16S
MW98-16D
MH-1
MH-1B
B-1
SG-1
SG-2
RETORTS
GENERATOR HOUSE
COOLING TOWER
BOILERS
CONDENSOR
BINGHAMTON LIGHT HEAT & POWER CO.
TAR SEPARATING WELL
TAR WELL
PUMP HOUSE
PURIFIER
WASHER COOLER
PUMP
NO. 1 GAS HOLDER
NO. 2 GAS HOLDER
NO. 3 GAS HOLDER
NO. 4 GAS HOLDER
OIL TANKS
MACHINE SHOP
STORAGE
OFFICE
OIL HOUSE
STEEL FRAME PUMP HOUSE
BOOSTER HOUSE
GAS CONTROL BLDG.
PAVED AREA
LAWN
295 COURT STREET PROPERTY
NYSEG COURT STREET PROPERTY
293 COURT STREET PROPERTY
FORMER SITE BOUNDARY
24" PIPE
24" PIPE OUTFALL
66-INCH STORM SEWER
(LOCATION APPROXIMATE) 20" AND 24" DIA. WATER LINES
SEWER MANHOUSE AND CONTROL STRUCTURE
GATE VALVE
COMBINED STORM/SANITARY FACILITY (1925)
PUMP HOUSE
FILLING STATION
CITY WELL 1
CITY WELL 2
RANNEY WELL
WELL INTAKE LATERALS
FLOOD WALL
FILTRATION PLANT INTAKE
SUSQUEHANNA RIVER
FLOW
BRANDYWINE AVENUE
COURT STREET
TOMPKINS STREET
36" SANITARY SEWER
855
850
845
840
860
865
855
850
845
840
860
865
855
850
845
840
860
865

Attachment 2

Site Inspection Form

Binghamton (Court Street) Former MGP Site
Binghamton, Broome County, New York
Site-Wide Inspection Form

Date: 4/21/2026
Personnel: S. Mack
Time of Arrival: 0830
Time of Departure: 1130

Weather Conditions: Sunny, Partly Cloudy
Temperature: 30 F
Wind Speed: 5 mph
Wind Direction (from): North

Inspection Checklist	Yes	No	Comments
Cover System			
Intrusive Activities Being Performed?			
- Trenching?		X	
- Excavation?		X	
- Tunneling?		X	
- Saw cutting?		X	
Signs of Previous Intrusive Activities Performed?			
- New drainage feature?		X	
- Evidence of a new underground utility?		X	
- New grass/vegetation/asphalt?		X	
- Other (e.g., cracking, potholes, depressions)		X	
Monitoring Well Condition			
Groundwater monitoring needs to be performed this year?		X	
Covers secure?	X		All secure except RW-20 (missing brace)
Casing in need of repair?		X	
Concrete surface seal intact?	X		
Settling in area around well?		X	
Well obstructed?		X	
Ponded water above well?		X	
Well screen silted in?		X	
Well in need of redevelopment?		X	

General Comments/Suggested Action Items:

Construction of new gas station occurring on nearby property where MW-01-17-S/D are located, MW-01-17-S appears to have been damaged during construction - Arcadis was able to gauge the well, but the measuring point will need to be resurveyed when the well is repaired - recommended after construction is completed.

Attachment 3

Sheen and Riverbank Monitoring Memo

Memo

SUBJECT
Sheen/Riverbank Monitoring – April 2026
NYSEG Binghamton Court Street Former MGP Site
NYSDEC Site No. 7-04-031

TO
Levia Terrell
NYSEG
18 Link Drive
Binghamton, New York 13902-5224

DATE
September 10, 2026

OUR REF
NA

DEPARTMENT
Environment

PROJECT NUMBER
30337970

COPIES TO
Mark Gravelding, Arcadis
David Cornell, Arcadis

NAME
Joe Bistrovich
315.671.9697
joe.bistrovich@arcadis.com

This memorandum presents a summary of the April 21, 2026 visual sheen monitoring and qualitative riverbank monitoring event conducted at Operable Unit No. 2 (OU-2) of the New York State Electric & Gas Corporation (NYSEG) Binghamton Court Street Former Manufactured Gas Plant (MGP) Site.

Sheen Monitoring Requirements

During the New York State Department of Environmental Conservation's (NYSDEC's) review of the August 2020 Draft Focus Feasibility Study for OU-2, NYSDEC requested that NYSEG conduct periodic visual sheen monitoring of the Susquehanna River to evaluate the effectiveness of the 2019 sediment removal interim remedial measure (IRM).

In accordance with the Site Management Plan (SMP), sheen monitoring is conducted from the Tompkins Street Bridge for a duration of 30 minutes. If sheens are observed, the observer will attempt to determine the potential source of the sheens and quantify the frequency and magnitude of sheens. For reference, the first visual sheen monitoring event was conducted in February 2021. Sheen monitoring was performed quarterly in 2021 and 2022, semiannually in 2023 and 2024, and annually beginning in 2025. As summarized below, sheens have not been observed since the first monitoring event in February 2021.

Riverbank Monitoring Requirements

In accordance with the 2022 Restoration Monitoring Report recommendations, riverbank monitoring of the restored transloading area is conducted at the same frequency as sheen monitoring activities and consists of a qualitative riverbank stability assessment that includes observations for erosion, sloughing, settlement, or other indicators of soil instability.

Levia Terrell
NYSEG
September 10, 2026

Sheen Monitoring Observations

No sheens have been observed since the start of sheen monitoring in February 2021 and none were observed during the April 2026 monitoring event. Photographs documenting river conditions at the start and end of the monitoring period are included in the attached Photograph Log.

Bank Stability Observations

During the May 2022 inspection, riverbank erosion was noted near the upstream end of the restored bank area. The NYSDEC conducted an inspection in March 2023 and documented erosion/sloughing in an email dated March 30, 2023. During the May 2023 inspection, Arcadis confirmed NYSDEC's observations and noted that, while there were exposed areas with little to no vegetation, the riverbank appeared stable and the existing root mass of the remaining vegetated portions of riverbank would continue to stabilize riverbank soils.

During the April 2026 inspection, the previously identified erosion area appeared stable with no evidence of new or significant erosion, sloughing, or settlement observed. Existing riverbank vegetation continues to encroach into and fill the affected area, providing additional stabilization. Riverbank photographs are included in the attached Photograph Log.

Summary

Based on observations during the April 2026 sheen and riverbank inspection:

- No sheens were observed during the monitoring event, and no sheens have been observed since sheen monitoring began in February 2021.
- No new or significant erosion, sloughing, or settlement was observed within the restored riverbank area. The previously documented erosion area appeared stable and unchanged from prior inspections, and vegetation continues to provide additional stabilization.
- The next sheen monitoring and riverbank inspection is tentatively scheduled for the second quarter (Q2) 2027.

Enc.: Photograph Log

Attachment 1

Photograph Log

Photograph Log

NYSEG
Binghamton Court St. Former MGP Site Operable Unit No. 2
Sheen Monitoring/Riverbank Stability Inspection



Photograph: 1

Description:
OU-2 Remedial Area 1, facing upstream, taken from Tompkins Street Bridge

Location:
Binghamton Court Street OU-2

Date: 4/21/2026

Time: 10:25



Photograph: 2

Description:
OU-2 Remedial Area 1, facing upstream, taken from Tompkins Street Bridge

Location:
Binghamton Court Street OU-2

Date: 4/21/2026

Time: 10:55

Photograph Log

NYSEG
Binghamton Court St. Former MGP Site Operable Unit No. 2
Sheen Monitoring/Riverbank Stability Inspection



Photograph: 3

Description:
Transloading Area restored riverbank, upstream end facing downstream.

Location:
Binghamton Court Street OU-2

Date: 4/21/2026



Photograph: 4

Description:
Transloading Area restored riverbank, upstream end.

Location:
Binghamton Court Street OU-2

Date: 4/21/2026

Photograph Log

NYSEG
Binghamton Court St. Former MGP Site Operable Unit No. 2
Sheen Monitoring/Riverbank Stability Inspection



Photograph: 5

Description:
Transloading Area restored riverbank, upstream end to top of bank.

Location:
Binghamton Court Street OU-2

Date: 4/21/2026