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ENGINEERING REPORT FOR WASTEWATER SETTLING PONDS

SYRACUSE CHINA LANDFILL Town of Salina, Ouondaga County, New York

July 10, 2000

Prepared for:

THE PFALTZGRAFF CO. AND SYRACUSE CHINA/LIBBEY, INC.



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1.0 INTRODUCTION

This Engineering Report for Wastewater Settling Ponds (Report) has been prepared by Remedial Engineering, P.C. (Remedial Engineering) on behalf of The Pfaltzgraff Co. (Pfaltzgraff) and Syracuse China/Libbey, Inc. (Syracuse China) to describe the settling pond design at the Syracuse China site. This Report has been prepared in accordance with the New York State Department of Environmental Conservation (NYSDEC) Bureau of Water Permits guidance document titled *Industrial Wastewater Treatment Facilities*. A letter of endorsement from Syracuse China has been included as Appendix A. Syracuse china reserves the right, with NYSDEC approval, to modify the details of pond construction as site conditions allow.

1.1 Site Description

The Syracuse China manufacturing plant (facility) is located in the Town of Salina, Onondaga County, New York, which is located north of the City of Syracuse. A site location map is included as Figure 1. The settling ponds are located within the Syracuse China Landfill (site). The Syracuse China Landfill and settling ponds are located north of the facility. The site is bordered by wetlands and Factory Avenue to the north, CSX rail lines and the facility border the site to the south, and undeveloped Syracuse China property lie to the east and west. A site plan is included as Figure 2.

1.2 Regulatory History

The Syracuse China facility was constructed in the 1920's and has been manufacturing china since that time. Pfaltzgraff purchased the Syracuse China facility in 1989. The NYSDEC listed the site on the Registry of Inactive Hazardous Waste Disposal Sites with a classification of 2 in March 1991. Pfaltzgraff sold the property to a subsidiary of Libbey, Inc. in 1995. Based on the results of a Remedial Investigation and Focused Feasibility Study (RI/FS), a Record of Decision (ROD) was issued for the site in March 1996. Pfaltzgraff is required by the ROD to perform remediation of the landfill, wetlands north of the landfill and the existing settling ponds as well as the "reconstruction of the settling ponds, as necessary to maintain the current wastewater discharge." The final design for existing settling pond and landfill remediation was approved (in March 2000) with the exception of the design for new settling ponds, which is required to be designed in accordance with NYSDEC State Pollutant Discharge Elimination System (SPDES) guidelines. Wastewater discharges from the settling ponds are regulated by SPDES permit number NY0100137 issued to Syracuse China. The SPDES permit was renewed April 1, 1999 and expires on April 1, 2004. A copy of the current SPDES permit is included in Appendix B.

2.0 DESCRIPTION OF INDUSTRY AND SOURCES OF WASTEWATER

The two main sources of flow into the settling ponds are storm water and facility wastewater. Each of these sources is described in further detail below.

2.1 Storm Water

Storm water flows to the settling ponds from the facility, the landfill, and from a sewer from the Village of Lyncourt. Facility storm water is carried by a series of culverts and catch basins to two main storm-water lines, a 24-inch concrete pipe and a 36-inch vitreous clay pipe. These two pipes discharge into a manhole north of the facility. A 36-inch pipe carries storm water from this manhole under the CSX tracks to a swale that flows into the existing settling ponds. This storm-water collection system provides drainage for approximately 22 acres of the manufacturing facility located south of the CSX tracks. The facility stormwater drainage area consists of largely impermeable surfaces such as paved parking lots and the building roof. The new settling ponds will continue to accept storm-water flow from the facility area.

Some storm water flow from the landfill enters the settling ponds via overland flow. This flow originates from the east face of the western portion of the landfill and from the eastern portion of the landfill which surrounds the settling ponds. The area of the landfill which currently drains to the settling ponds is estimated to be no more than 7 acres. Subsequent to closure of the landfill and construction of the new settling ponds, storm-water runoff from the landfill will no longer flow to the settling ponds. Landfill runoff will be decreased due to vegetation, and diversion swales will be constructed to carry storm water to the north side of the landfill and discharge under Factory Avenue via a drainage swale which runs through the wetlands. The new settling ponds will be constructed with a perimeter berm above the new grade in the eastern portion of the site and therefore, the new ponds will not receive overland flow from that area.

Stormwater flow is also received from a Village of Lyncourt sewer. The quality and volume of stormwater from this culvert is not well defined but the drainage area of this sewer consists of residential property and impermeable surfaces such as paved roads. However, please note that the Village of Lyncourt, Onondaga County and New York State are working toward separation

of the sewer from Syracuse China's system. Until it has been physically separated, the new settling ponds will continue to accept storm-water flow from this source.

2.2 Description of Manufacturing Processes and Facility Wastewater

Syracuse China manufactures dinnerware (ware) for domestic and commercial use at its facility. Activities conducted as part of the primary manufacturing processes includes:

- raw material batching;
- ware forming, finishing and drying;
- ware firing;
- ware cleaning;
- decal application;
- glaze batching and application; and
- overglaze decorating.

Activities conducted as part of the secondary manufacturing processes include:

- decorative design studio;
- decal production;
- color preparation and ware decorating;
- analytical laboratory activities;
- plaster mold making;
- operation of building utility systems (boilers, air-conditions, vacuum pumps, etc.);
- equipment cleaning and washdown;
- personnel activities (cafeteria, restrooms, showers, etc.); and
- storm water collection and drainage.

Nearly all of these operations and activities result in wastewater discharges.

Wastewater discharged from the facility enters through the metered municipal water supply, or as precipitation. Figure 3 is a schematic which shows the types and approximate amounts of

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Syracuse China's wastewater. Approximately 234,000 gallons per day (gpd) of water enters the facility through the metered municipal water supply. Of this quantity, approximately 29,750 gpd are discharged from various processes and activities to the sanitary sewer through the metered sanitary sewer outfall. Approximately 197,200 gpd of water, which enters the facility from the municipal water system, are discharged through Outfall 001. In addition to the portion of the municipal water supply which is discharged though Outfall 001, approximately 150,000 gpd of storm water and ground water are discharged through Outfall 001. A total of approximately 380,000 gpd of water are discharged and carried off site.

The processes which discharge to the storm sewer through Outfall 001 include:

- raw material batching;
- ware forming and finishing;
- ware cleaning;
- plaster mold making; and
- utility systems (non-contact water from air-conditioners and vacuum pumps).

Wastewater from these operations (except for non-contact cooling water from utility systems which will be discharged directly to Outfall 001) drains to the wastewater treatment system for removal of total suspended solids (TSS) prior to discharge to SPDES Outfall 001. The wastewater from these processes is currently filtered in one of the two primary filter presses which remove clay, calcined alumina, and aluminum hydrate (suspended solids). The filtrate from these filter presses is then discharged through a three-stage effluent tank and to the settling ponds. The discharge (overflow point) from the settling pond is SPDES Outfall 001.

The following includes a brief description of the processes which discharge water to the settling ponds and/or SPDES Outfall 001.

Plaster Mold Making: Plaster molds are formed at Syracuse China as the original patterns for the ware to be produced. In the mold making shop, water is used to mix the plaster, form the molds, and to clean equipment. The plaster mold making shop is equipped with a settling basin which is used as a source for recycle water, as well as a point for the settling of suspended solids

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prior to discharge of the wastewater to the wastewater treatment system for the removal of suspended soils. Approximately 1,450 gpd of water are used in the plaster mold making shop and an average of 1,200 gpd are discharged through the wastewater treatment system located in the facility and to the settling ponds.

Raw Material Batch Filter Press: In the raw material batching area, raw materials including calcined alumina, ground quartz, clay and feldspathic rock are brought in on railcars and loaded into raw material silos. Wastewater generated from the batching area results from minor leaks of precipitation into the batching area around the railcar garage, leaks to floor drains inside the batch storage area, and from water sealed pumps inside the batching area. The outside area drains discharge directly to the facility storm sewer which leads to SPDES Outfall 001. The floor drain and pump sealing waters are directed back to the facility's wastewater treatment system.

The raw materials are transferred from the raw material silos into batch mixing facilities where they are blended. The blended materials are then filter pressed to form a moldable clay raw material. The filtrate from the raw material filter presses is discharged to the wastewater treatment system. Of 14,700 gpd of water from the municipal water system typically used in the batching operations, approximately 4,700 gallons remain in the raw clay. Approximately 10,000 gpd are discharged through the wastewater treatment system located in the facility and to the settling ponds.

Ware Forming, Finishing and Drying: In the ware forming and finishing area, the moldable clay raw material is extruded and cut into shapes which are then rolled to form the dinnerware shapes (plates, bowls, saucers, etc.). After the ware is formed, the edge of the ware is scraped with steel scrapers. The formed ware then passes through a dryer which heats the dinnerware shapes in order to evaporate moisture. As the ware leaves the dryer, it is placed on racks and allowed to air dry for a period of time.

On a typical day, approximately 20,000 gallons of water are used in the ware forming and finishing area. Of the water (20,000 gpd from wet finishing plus 4,700 gpd in the raw clay) which enters the ware forming and finishing area, approximately 3,550 gpd are evaporated in the ware dryer, and approximately 1,000 gpd remain in the ware transferred to the firing kilns.

Wastewater from one automated flatware forming and finishing system and several manualfinishing stations is discharged directly to a central cistern where it is collected from treatment in the wastewater treatment system. Seven manual cup edge finishing stations in the finishing area are equipped with water recycling clarifiers which allow reuse of the edge finishing water. The water in these recycle systems is returned to the process in-feed once each shift. Approximately 20,000 gpd of wastewater are discharged from the ware forming and finishing area through the wastewater treatment system located in the facility and to the settling ponds.

Biscuit Ware Cleaning: In the biscuit ware cleaning area, the fired ware is cleaned with water in a vibratory finisher to remove calcined alumina which was applied to the surface of the ware to prevent it from sticking to other pieces of ware during firing.

Approximately 26,500 gpd of water are used in the biscuit ware cleaning area. The water is passed through a hydrocyclone to remove the calcined alumina (course solids). The wastewater from the hydrocyclone is discharged through the wastewater treatment system located in the facility and to the settling ponds.

Building Utilities: Building utilities at the Syracuse China facility which discharge wastewater include boiler systems, air-conditioning systems, air compressors, and vacuum pumps. The boiler systems are used for heating and drying. Boiler blowdown is discharged to the sanitary sewer.

Vacuum pumps which are used in decorating systems and in ware forming account for approximately 30 percent of the 140,000 gpd non-contact cooling water flow. The remaining non-contact cooling water was used to cool air-conditioners and air compressors throughout the plant. The waste cooling water from the air-conditioners, air compressors and vacuum pumps is discharged to the settling ponds and SPDES Outfall 001 without passing through the wastewater treatment system located in the facility.

Storm and Ground-Water Collection: An average of approximately 150,000 gpd of storm water and ground water are discharged through the settling ponds and SPDES Outfall 001 without passing through the wastewater treatment system located in the facility. This flow is seasonally variable.

Equipment Cleaning and Washdown: As part of system operations, manufacturing equipment is periodically rinsed with water from utility hoses to remove accumulations of dust and dirt. Flows of wastewater generated in this manner have been included in the wastewater discharges associated with each activity described above.

Facility wastewater will continue to be discharged to the new settling ponds at the same rates, the same quality, and with the same pre-treatment (filter press) as current operations.

3.0 TREATMENT OBJECTIVE

The treatment objective for wastewater effluent from the settling ponds is dictated by compliance with the current SPDES permit as provided in Appendix B for Outfall 001. The current SPDES permit (#NY0100137) was issued on April 1, 1999 and includes the following discharge limits:

Parameter	Discharge Limit
flow	monitor only
temperature	90 degrees Fahrenheit
total dissolved solids (TDS)	600 milligrams per liter (mg/ ℓ)
total suspended solids (TSS)	20 mg/ <i>l</i>
5-day biochemical oxygen demand (BOD5)	21 mg/ ℓ (monitoring action level only)
oil and grease	15 mg/ℓ
total boron	840 micrograms per liter $(\mu g/\ell)$ (monitoring action level only)
total lead	70 μg/ℓ

The primary design consideration for the new settling ponds will be TSS under high flow or facility upset conditions. Under normal facility operating conditions, the existing data shows that the current discharge meets the monitored SPDES limits prior to discharge into the settling ponds as can be seen in Table 1. There are no other discharge limits or reported constraints on the settling pond operations beyond those specified in the SPDES permit.

As described in Section 5.0, Remedial Engineering collected settling pond influent samples between May 18, 1998 and June 15, 1998. Table 1 provides a summary of this data.

4.0 EXISTING TREATMENT FACILITIES

The existing wastewater treatment system located in the facility is designed to remove TSS from the wastewater before it is discharged to the settling ponds and through SPDES Outfall 001. The wastewater streams which were described in Section 2.2, vary with regard to flow and TSS concentration. All wastewater which discharges through SPDES Outfall 001 must have a TSS concentration equal to or less than 20 milligrams per liter (mg/ ℓ). A schematic of the wastewater treatment system located in the facility is included in Figure 4.

The treated plant wastewater leaves the facility and flows directly to the settling ponds. There are four former and existing settling ponds and one out of service "sludge pond" at the site. Figure 2 is a site plan which shows the layout of the settling ponds and Figure 5 includes a schematic of the current settling pond treatment scenario. Settling Ponds 2, 3 and 4 are currently in use. Settling Pond 1 and the Sludge Pond were formerly used to polish wastewater, and store sludge, respectively, but are currently inactive. All settled material in each of the settling ponds and the sludge pond will be excavated and placed under the landfill cap as part of the site remediation.

A 36-inch pipe carries facility wastewater and stormwater under the CSX tracks to a swale which flows into the existing settling ponds. Wastewater currently flows through Settling Pond 3, into Settling Pond 4, and into Settling Pond 2, all of which are interconnected with 15-inch diameter pipe. The effluent water from Settling Pond 2 discharges through a 15-inch pipe (SPDES Outfall 001) and then flows through a natural channel within the northern wetlands and then off-site through a culvert under Factory Avenue.

The settling ponds are excavated into native soil below existing grade and currently contain a significant volume of sediment. Volumes were estimated based on reports and data obtained from Syracuse China and site surveys. The maximum original volumes (prior to accumulation of sediment) of the currently active settling Ponds 2, 3 and 4 are approximately 450,000 gallons, 400,000 gallons, and 385,000 gallons, respectively, totaling approximately 1.2 million gallons. The current conditions of Settling Ponds 2, 3 and 4 are estimated to be:

- Settling Pond 2: 8 feet of sludge located below approximately 5.5 feet of standing water;
- Settling Pond 3: 10 feet of sludge located below approximately 0.5 feet of standing water; and
- Settling Pond 4: 6 feet of sludge located below approximately 6.5 feet of standing water.

5.0 WASTEWATER CHARACTERISTICS

Remedial Engineering performed a wastewater evaluation in May-June 1998 to characterize quality of wastewater entering the settling ponds. Wastewater flow quantities were collected from May to August 1998. A portable sampler and flow monitor were installed inside a 36-inch diameter pipe in the effluent manhole. Historical data and other information pertaining to wastewater flow and quality was also obtained from Syracuse China personnel. Characteristics of the wastewater flow are described below.

5.1 SPDES Sampling Results Provided by Syracuse China

SPDES Outfall 001 sampling results were provided by Syracuse China for the period from January 1990 to December 1999. The parameters analyzed included: flow, lead, pH, TSS and TDS. Table 2 is a summary of Syracuse China's SPDES sampling results. It should be noted that the limits for certain parameters in the previous SPDES permit (effective date September 1, 1985) have been modified. The previous SPDES permit limits were as follows:

Parameter	Discharge Limit
flow	monitor only
total dissolved solids (TDS)	600 milligrams per liter (mg/ ℓ)
total suspended solids (TSS)	30 mg/l
total lead	0.2 mg/ℓ

5.2 Flow Data

Wastewater flow recorded by Remedial Engineering (May - August, 1998) was generally in the range of 300 to 400 gallons per minute (gpm) which is consistent with historical flow data received from Syracuse China personnel. Syracuse China's flow data indicate flows ranging from less than 100 gpm to over 400 gpm with flows generally falling in the 200 to 400 gpm range.

Storm-water flow from the facility, the Village of Lyncourt sewer and the landfill area can make a significant contribution to the total flow entering the Settling Ponds. The storm water flows for 1-year and 10-year storms (used in calculations) were estimated using flow data and rainfall data collected by Remedial Engineering. Daily rainfall data was obtained during the study period **REMEDIAL ENGINEERING, P.C.** 10 *PASTTOLE.75.R3* from the National Oceanic and Atmospheric Administration (NOAA) for the Syracuse area. The rainfall data was plotted against the daily maximum recorded flow and a best-fit line was applied to the data. The 24-hour rainfall for various frequency storms was obtained from *Urban Hydrology for Small Watersheds*. The design frequency storm 24-hour rainfall was correlated to a maximum flow (on the basis of the data collected May - August, 1998) using a best-fit line for the 1-year and 10-year storms. Appendix C includes rainfall and storm flow data.

A copy of Remedial Engineering's flow data collected between May 15, 1998 and August 24, 1998 is included in Appendix D and flow data provided by Syracuse China is included in Table 2.

5.3 Chemical Analysis

Settling pond influent samples collected with the portable sampler installed by Remedial Engineering were analyzed for TSS, TDS, total lead and dissolved lead. Table 1 summarizes the results of the chemical analysis of the influent wastewater during this limited time period.

5.4 Facility Upset

Another consideration for the settling pond design was the potential for facility "upset." An upset condition could occur should one of the filter presses in the facility fail and wastewater discharged to the settling ponds without prior treatment. Data for two upset scenarios were provided by Syracuse China personnel.

The first scenario was the failure of the process wastewater filter press which treats process wastewater from ware forming, finishing, and drying; biscuit ware cleaning; and plaster mold making and general facility wastewater from equipment cleaning, spillage and some minor wet processes. This sample was collected in November 1998 and the TSS concentration in the sample was 23,811 mg/ ℓ . A second upset sample was described to simulate the failure of the production filter press. This scenario was provided by Syracuse China personnel in March 2000 which indicated a TSS concentration of 407,810 mg/ ℓ suspended solids and was used for the calculations as the "worst-case scenario."

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5.5 Particle Size

A particle distribution curve developed from a simulated facility upset sample provided by Syracuse China indicated a mean particle diameter of 14.5 micrometers (μ m). Another particle distribution curve provided by Syracuse China personnel indicated a mean particle diameter of 1.3 μ m. Particles smaller than 10 μ m are generally considered unsettleable (Metcalf & Eddy, 1972). Approximately 55% of the facility flow particles were considered unsettleable for the design calculations based on the particle size distributions. Wastewater particle size distributions are included in Appendix E. The settling ponds adequately remove TSS associated with storm water.

The settling pond influent data collected during the recent field activities and the historical effluent data provided by Syracuse China indicate that treatment in the settling ponds is not necessary to meet the discharge limits in the SPDES permit for routine facility discharges; however, the ponds provide for the necessary polishing of plant wastewaters that may occasionally contain higher suspended solid concentrations as a result of production upsets and/or wastewater processing equipment breakdowns. The settling pond design parameters and calculations are discussed in detail in the following sections.

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6.0 DESIGN PARAMETERS

The intent of this new settling pond design is to meet current design standards with volumes similar to the existing settling ponds. The new settling ponds are designed to treat combined wastewater/storm water flow from the sources described in Section 2.0. In accordance with discussions held with NYSDEC's Bureau of Water Permits, two scenarios were evaluated for the settling pond design:

- Case 1: base facility discharge and a 10-year storm flow; and
- Case 2: base facility discharge with a facility upset and 1-year storm flow.

The data for storm water and facility wastewater were discussed in Section 5.0. The following parameters were used to evaluate the projected performance of a pond design similar in volume to the existing ponds.

Parameter	Value
base facility flow	264 GPM;
base facility flow TSS concentration	7 mg/ <i>l</i> ;
facility upset flow	25 GPM (for 1.25 hours);
facility upset flow TSS concentration	407,810 mg/ ℓ (for failure of the production wastewater filter press);
1-year storm flow	4,450 GPM;
1-year storm flow TSS concentration	45 mg/l;
10-year storm flow	7,179 GPM;
10-year storm flow TSS concentration	45 mg/ <i>l</i> ;
average particle size (facility upset)	1.3 to 14.5 μm;
average particle size (typical storm water)	1.0 to 2,000 µm

An average of 2.25 feet of freeboard is provided for design conditions and the design allows for the removal of one settling pond from service for maintenance at any time.

Calculations performed using the above parameters result in failure under facility upset conditions as shown in Table 3. The results of Table 3 are discussed in Section 7.3.

7.0 TREATMENT SYSTEM DESIGN

As discussed in Section 3.0, the treatment objectives for the new settling ponds are compliance with the existing SPDES permit (# NY0100137) under normal, facility upset and storm flow conditions. Calculations were performed with consideration to meeting the SPDES limits as well as handling the production upset flows as requested by the Bureau of Water. The important parameters for the calculations were described in Section 6.0. A general settling pond description is provided below. Pond treatment performance calculations are discussed in Section 7.2. Section 7.3 details the facility's operational response requirements for facility upset conditions. The evaluations of the ponds treatment performance in this section are based on a 3-pond configuration in which one settling pond can always be taken out of service for maintenance, with an approximate maximum volume of 1.25 million gallons, similar to the existing settling pond system.

7.1 General Settling Pond Description

The new settling pond system consists of 3 settling ponds connected by pipes with effluent weirs and downstream headwalls. Each settling pond is approximately 170 feet long by 60 feet wide at the bottom with a maximum water depth of 6.5 feet. The operating water level is 4.25 feet with an average of 2.25 feet of freeboard. The approximate particle flow path is 160 feet in each settling pond. A perforated floating baffle will be installed in each of the three ponds to evenly distribute flow through the ponds and maximize settling of TSS. The baffles will each be approximately 60 feet wide and 4 feet in depth. They will be capable of floating within a 1 foot range. The baffles will have the manufacturer's standard perforations, which allow 30 gpm of flow per square foot of baffle beyond which the baffles would be overtopped. Each baffle will be installed 1/3 the length of the pond from the inlet (approximately 60 feet). A concrete weir structure will be installed in each of the three ponds to evenly distribute effluent flow and control pond water levels. The weirs will be installed at the effluent of each pond and will flow to the 24-inch HDPE pipes which carry wastewater to the next pond or to the outfall swale. The settling ponds will be constructed into native soils, however, the depth of excavation will be limited by depth of groundwater. The bottom of the settling ponds will start one foot above mean high ground-water elevation and will be underlain with a geogrid and a 12-inch sand

drainage subbase and geosynthetic clay liner overlain by 12-inches of gravel. The side slopes of the settling ponds will be lined with a geosynthetic clay mat overlain by 6-inches of gravel. Each settling pond has a 15-foot wide emergency spillway which discharges to a swale carrying wastewater out to the SPDES outfall. If wastewater flows through all three settling ponds under normal conditions, it will flow out of Settling Pond 3 into an effluent swale which carries the pond discharge to the SPDES outfall 001. Bypass piping will be installed for Settling Ponds 1 and 2 so any pond can be removed from service for maintenance. If Settling Pond 3 is removed from service, water will flow out of the bypass headwall (Headwall No. 4) in Settling Pond 2 and flow into the spillway swale to SPDES outfall 001. A schematic of the new settling ponds is shown on Figure 6. Table 4 includes a list of the equipment/structures required for the new settling ponds design. Appendix F includes a description of the possible flow scenarios for the new settling ponds and it also contains Drawings 5 and 12 from the *Final Remedial Design Syracuse China Landfill* dated April 2000 which show the latest design revision for proposed settling ponds including the appropriate construction details. Appendix G includes the specifications for the significant elements of the new settling ponds.

7.2 Settling Pond Design Calculations

Initially, a calculation was performed to determine the smallest particle which can settle 100% in the proposed settling ponds. This calculation is included in Table 5. The settling velocity of the smallest particle which will settle 100%, assuming two-pond operation, is 0.00056 feet per second (fps) or 0.017 centimeters per second (cm/sec). Using Figure 3.2, the settling velocity vs. particle diameter chart in *Unit Operations and Processes in Environmental Engineering* (Reynolds, 1982) included as Appendix H, particles ranging in size from 0.001 cm to 0.06 cm (10 μ m to 600 μ m) will settle at this velocity depending on the specific gravity of the particle. Therefore, the smallest particle which would theoretically settle in the settling ponds has a diameter of 10 μ m. This is consistent with the *Wastewater Engineering* reference also included in Appendix H which indicates that particles smaller than 10 μ m in diameter are generally unsettleable. As discussed in Section 5.0, average particle size in a facility upset discharge appears to range from 1.3 to 14.5 μ m, based on samples and data provided by Syracuse China. Stormwater particles generally range in size from 1.0 to 2,000 μ m based on published data.

The TSS concentrations were calculated for Case 1 and Case 2, as defined in Section 6.0. The final TSS concentrations were calculated by using a mass balance equation and assuming the sample is collected as a 6-hour composite as required by the SPDES permit. The TSS calculations are included as Table 3. The calculated TSS concentrations at the SPDES outfall in 6-hour composite samples were as follows:

- Case 1: 11 mg/ ℓ ; and
- Case 2 (failure of one of the facility production wastewater filter presses): $257 \text{ mg/}\ell$;

These calculations indicate that the SPDES permit limit for TSS of 20 mg/ ℓ can be met for the conditions in Case 1. However, the SPDES permit limit for TSS cannot be met for the facility upset condition (Case 2) without additional treatment.

The weir loading rate was calculated by dividing the flow rate by the length of the weir. Average flow into the ponds is approximately 468,000 gpd and the weir length is equal to the width of the pond (65 feet). The calculated overflow rate is approximately 7,200 gpd per foot of weir. Recommended weir loading rates are 15,000 to 20,000 gpd per foot (Reynolds, 1982), therefore, the weir length is more than adequate for normal facility flow. This calculation is included as Table 6.

The flow capacity of the emergency spillways was calculated to confirm that the spillways can pass the 25-year storm. The emergency spillways are 6 inches deep and 15 feet wide and can carry a flow of 8,734 gpm which is greater than the 25-year storm flow of 8,680 gpm. This calculation is included as Table 7.

The pond piping was designed to carry the 25-year storm (8,680 gpm). Calculations indicate that a 24-inch diameter HDPE pipe will carry 16,515 gpm which is adequate for the 25-year storm. The head loss due to friction for the pipe was also calculated and was nearly negligible at 0.35 feet. These calculations are included on Table 8.

7.3 Facility Operational Response Requirements for Upset Condition

Since the solids in the Syracuse China wastewater are generally unsettleable and mixing is not adequate to meet the SPDES TSS limits during any significant plant upset, additional treatment is necessary to achieve compliance with the SPDES permit. If a facility upset occurred, Settling Pond 1 has a detention time of approximately 3 hours which would allow Syracuse China operations personnel to isolate Settling Pond 1 so facility upset wastewater could be contained to receive additional treatment. Once Settling Pond 1 is isolated by closing stop gates located in Distribution Boxes 1 and 2, normal facility flow can be diverted into Settling Ponds 2 and 3. During a facility upset, a flocculant would be added to the isolated settling pond to aid in settling of TSS in the wastewater. Syracuse China's current SPDES permit includes the use of an approved flocculant, Amerfloc, 482 manufactured by Ashland Chemical Company. During a facility upset, a flocculant would be added either in the facility effluent tank and/or the isolated pond. Addition of flocculant will be performed by Syracuse China as they are obligated to operate the new settling ponds, in accordance with the SPDES permit, at the completion of pond construction.

8.0 EFFLUENT CHARACTERISTICS

Parameter	Anticipated Concentration	Permit Limit
TSS (Case 1)	11 mg/ℓ	20 mg/ <i>l</i>
TSS (Case 2)	20 mg/ℓ	20 mg/ℓ
TDS	200 - 300 mg/ℓ	600 mg/ℓ
Total lead	0 - 0.024 mg/ <i>l</i>	.07 mg/ℓ

The anticipated settling pond effluent concentrations are as follow:

TSS is the only SPDES parameter of concern for the settling pond effluent. The other parameters currently meet the SPDES limits prior to discharge into the settling pond under all operating conditions. The addition of flocculant will be necessary to meet the TSS limits in Case 2. If a filter press fails, Settling Pond 1 could be isolated so the upset wastewater containing high TSS concentrations would be retained until sufficient settling occurred. Normal facility flow could be diverted to Settling Ponds 2 and 3 while upset wastewater would be retained in Settling Pond 1. Diversion of flows and the addition of the appropriate flocculant would be the responsibility of Syracuse China operations personnel as they will operate the new settling ponds once the pond construction is complete.

9.0 OPERATIONS AND MAINTENANCE

Pond operations and maintenance will entail three distinct elements:

- effluent sampling;
- pond inspections; and
- sludge removal.

Each of these three elements is described below.

Pond effluent sampling will occur on a weekly basis in accordance with the facility's SPDES permit which was issued April 1, 1999. Samples will be collected from a new Outfall 001 which will be located at the discharge from newly constructed Pond No. 3.

Formal pond inspections will be performed on a routine basis (every three months initially, but not less than annually thereafter) by Syracuse China to prevent any deterioration to the ponds or the associated pond structures. Inspections will include visual observations of soil berms, pond inlet and outlet structures, pond stop gates, floating baffles and pond piping. Any deterioration of these pond elements will be documented and repaired as soon as possible. Completed pond repairs will also be documented.

After the ponds have been operational for a sufficient period of time, sludge will accumulate within the ponds and will require removal. Sludge removal may be facilitated by the isolation of each individual pond to allow safe access. Future cleaning will be performed by Syracuse China, as required, utilizing an outside contractor utilizing a hydraulic dredge or other suitable device. If appropriate, removed sediments will be dewatered on site, chemically characterized, and appropriately disposed.

10.0 CONCLUSIONS

Remedial Engineering has prepared this engineering to present the design of new settling ponds similar in volume to existing settling ponds at the Syracuse China site in the Town of Salina, Onondaga County, New York. This report has been prepared in accordance with the NYSDEC's Bureau of Water guidelines as well as discussions with Bureau of Water personnel. This report satisfies the requirements of a Phase 1 submission as well as a Phase 2 submission since the engineering design drawings (Appendix F) and appropriate specifications (Appendix G) are included.

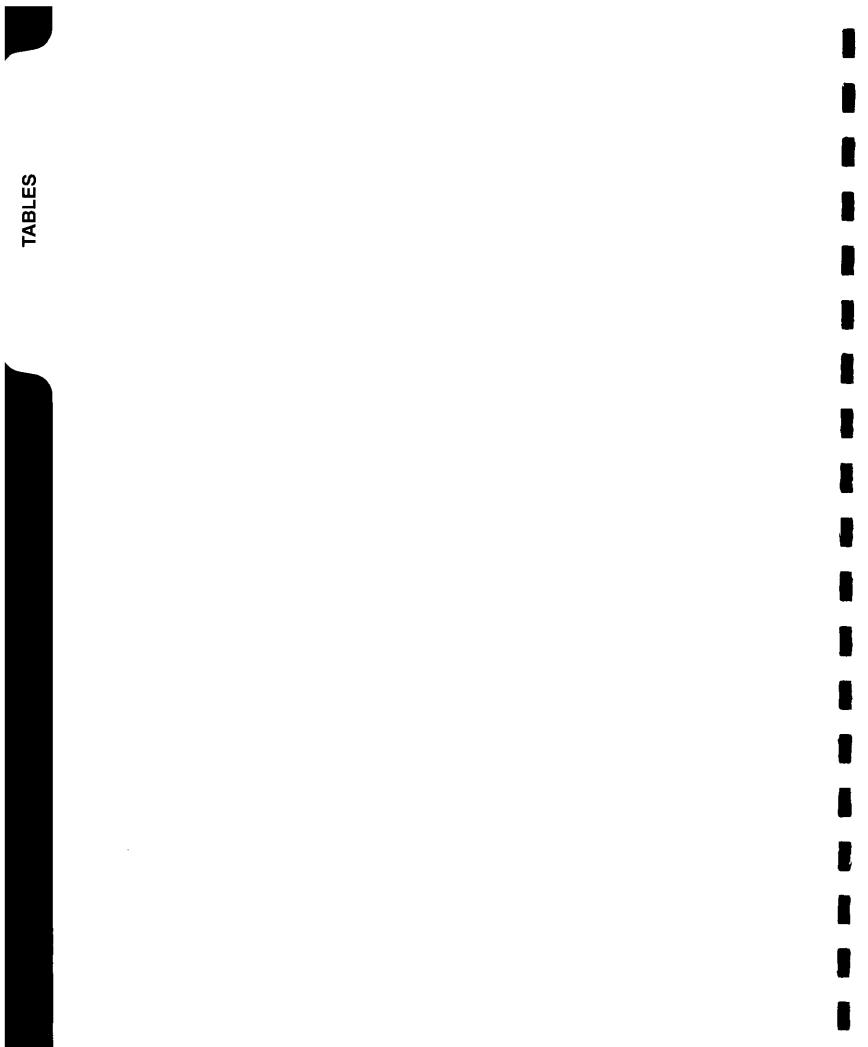
The conclusions of this report are as follows:

- the new settling ponds have approximately the same volume as the existing operating settling ponds;
- the primary means of treatment of the pond influent is sedimentation;
- wastewater effluent from the settling ponds will meet SPDES limits under "normal" flow conditions (Case 1) which includes a 10-year storm plus base facility flow; and
- wastewater effluent from the settling pond will meet SPDES limits under facility upset conditions if the upset flow is retained in Settling Pond 1 and/or flocculant is added to the wastewater by the operator to settle additional TSS.

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Page 1 of 1

Day	Influent TDS (mg/l)	Influent TSS (mg/l)	Effluent TSS (mg/l)	Influent Toal Lead (mg/l)	Influent Dissolved Lead (mg/l)
05/18/1998	280	20	-	0.009	0.006
05/18/1998	250	10	-	0.009	0.004
05/18/1998	250	3	-	0.008	0.005
05/19/1998	220	2	-	0.009	0.005
05/19/1998	300	5	-	0.007	0.007
05/19/1998	250	3	-	0.042	0.006
05/19/1998	240	1	-	0.013	0.005
05/20/1998	320	2	-	0.012	0.008
05/21/1998	260	14	-	0.008	0.002
05/21/1998	190	5	-	0.007	<0.001
05/21/1998	260	3	-	0.007	0.006
05/22/1998	240	4	-	0.006	0.004
05/22/1998	260	5	-	0.006	0.005
05/23/1998	270	1	-	0.006	0.005
05/24/1998	270	2	-	0.004	0.005
05/25/1998	280	4	-	0.007	0.007
05/26/1998	260	5	-	0.008	0.005
05/27/1998	230	6	3	0.008	<0.001
05/28/1998	210	5	-	0.003	0.003
05/29/1998	200	10	-	0.012	<0.001
05/30/1998	220	<1	-	0.004	0.001
05/31/1998	220	52	-	0.004	0.005
06/01/1998	230	7	-	0.006	<0.001
06/02/1998	210	9	-	0.024	0.002
06/03/1998	240	6	-	0.007	0.002
06/04/1998	480	3	-	0.006	<0.001
06/05/1998	320	15	-	0.005	0.004
06/06/1998	260	3	-	0.13	0.003
06/07/1998	270	3	-	0.005	0.003
06/08/1998	270	1	-	0.002	0.003
06/10/1998	350	3	-	0.004	<0.001
06/11/1998	250	4	-	<0.001	<0.001
06/12/1998	250	5	-	0.002	<0.001
06/13/1998	200	7	-	<0.001	<0.001
06/14/1998	190	4	-	<0.001	<0.001
06/15/1998	220	4	-	<0.001	0.001
	SPDES limit = $600 \text{ mg}/\ell$	SPDES limit = 20 mg/ ℓ		SPDES limit = 0.07 mg/ℓ	

Notes:

SPDES = State Pollutant Discharge Elimination System

TDS = Total Dissolved Solids

TSS = Total Suspended Solids

 $mg/\ell = Milligrams per liter$

- = Sample not collected on that date by Syracuse China.

1. Effluent TSS information supplied by Syracuse China Company.

Detection limit for lead is 0.001 mg/ ℓ .

2. SPDES limits from permit effective 4/1/99.

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mits No Limit 0.2 $6.0.90$ 30 445,100 0.0256 7.6 4.7 4.7 ,800 0.021 7.9 7 442,500 0.001 7.9 7 4.7 ,800 0.012 7.9 7 442,500 0.012 7.9 7 4.7 ,800 0.012 7.9 7 442,500 0.012 7.3 2.5 5.5 7.3 2.5 5.5 5.5 2.7 4.7 4.7 4.7 4.7 4.7 2.5 5.5 2.7 4.7 4.5 5.5 2.7	Sample Date	Flow (GPD)	Lead (mg/l)	pH	TSS (mg/l)	TDS (mg/l)
	SPDES Permit Limits Effective 9/1/85	No Limit	0.2	6.0-9.0	30	600
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/2/90	465,100	0.056	7.6	5	570
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/9/90	472,800	0.024	8.3	47	340
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/16/90	458,200	0.008	8.2	S	370
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/23/90	442,500	0.021	7.9	7	360
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/31/90	488,200	0.015	7.3	25	280
	2/6/90	515,500	0.034	8.0	13	540
	2/13/90	490,900	0.008	2.S	12	390
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2/20/90	435,300	0,014	۲. / ۱. /	2 10	44U
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	06/9/2 Arii 717	491,100 411,200	0.012	7.8	و ا	380
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/13/90	505,900	0.023	8.1	45	400
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/20/90	540,800	0.037	7.7	25	370
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/27/90	445,300	0.037	8.1	43	300
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/3/90	ł	0.071	7.8	14	300
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/11/90	ł	0.025	3.0	10	360
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/1//90		0.030	8.0 0.7	2	360
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/1/90		0.010	7.9	17	390
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/8/90	1	0.023	7.7	23	310
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/15/90	ł	0.005	7.9 2 2	25	330
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	00/0C/S		0.006	8.0 7.4))))	33U 480
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/5/90	1	0.004	8.0	9 [380
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/12/90	•	0.007	7.9	6	300
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/19/90	1	0.007	7.7	1 U 1	320
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0/20/90 7/10/90		0.014	7.9	ພບ	260
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7/17/90	1	0.026	7.8	16	340
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/7/90		0.02	7.4	2 6	350
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/21/90	.,+00	<0.001	0.4 7.8	4 F	310
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9/4/90	1	0.014	7.4	7	400
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9/11/90	1	0.016	7.6	4	250
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0/18/90	469,500	0.008	7.5	J (J	350 190
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/2/90	*	0.005	7.5	. س	250
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/9/90	•	0.008	7.7	2	270
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/16/90	348,200	0.007	7.8	o 7	340
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/30/90	440,900 362.300	0.039	0.0 7.6	و 18	300
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11/6/90	381,300	0.022	6.5	10	200
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11/14/90	358,900	<0.001	8.0	œ	400
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11/20/90	388,400	0.003	7.9	× ~1	290
356,500 0.015 8.9 3 0.019 8.1 7 369,600 0.017 8.3 4 406,600 0.004 7.7 4	12/4/90	386.200	0.097	7.7	30 7	200 240
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	12/11/90	356,500	0.015	8.9	υ	200
369,600 0.004 7.7 4	12/18/90		0.019	8.1	. 7	580
	1/17/01	406 600	0.017	7.7	14	055

Table 2. Summary of SPDES Sampling Results (Pond Effluent) Provided by Syracuse China. Pfaltzgraff Co.,

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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Sample Date SPDES Permit Limits Effective 9/1/85 1/22/91	Flow (GPD) No Limit 380,400	Lead (mg/ℓ) 0.2 0.020	рН 6.0-9.0 7.7	TSS (mg/ℓ) 30 6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/22/91	380,400	0.020	7.7	, 0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/2/91 2/7/91	295.000	<0.016	7.7 7.7	ს, ს
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2/15/91	369,600	0.007	7.7	<u>^</u>
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2/20/91	490,697	0.005	7.9	ور
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2/27/91	410,900	<0.001	7.8	ົ່
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/6/91		0.003	8 .1	و (
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/12/91	329,800	0.005	7.8	س ،
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/19/91	380,400	0.014	ð.1	10
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3/26/91	266,278	<0.100		~ 7
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/4/91	ł	0.011	8.1	6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/10/91	353,200	0.072	ţ	18
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/17/91	369,000	0.002	7.6	^1
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4/24/91	396,232	0.010	7.2	△
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/1/91	359,000	<0.020	8.3	80
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/7/91	329,400	0.014	7.5	сı
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/14/91	287,200	<0.020	7.6	6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/21/91	259,300	0.011	7.8	ω
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5/28/91	286,800	<0.020	8.0	6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/5/91	286,800	0.009	7.8	4
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/13/91	280,000	I	1	ł
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/18/91	344,500	ł		ł
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/19/91	338,700	<0.020	8.1	6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/20/91	353,400	0.038	8.4	10
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6/26/91	342,400	<0.020	8.4	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7/3/91	271,700	0.014	8.0	دى
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7/9/91	283,000	<0.020	8.2	<1.0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7/16/91		0.010	8.0	ه س و
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7/25/91	155,900	<0.020		S
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/8/91	384,000	0.028	8.3	<1.0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/13/91	410,300	0.024	7.8	4
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/20/91	401,600	<0.020	8.1	4
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8/28/91	358,700	0.110	7.8	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9/5/91	1,	0.020	8.0	6
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9/10/91	1	0.004		رب ا
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9/18/91	1	<0.020	7.3	1
<pre><0.020 </pre> <	9/27/91	390,000	0.011	7.8	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/2/91	1	<0.020	7.7	<0.5
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/9/91	1	0.060	7.6	دى
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10/15/91	ł	< 0.020		<0.5
390,000 <0.020 0.020 0.020 0.020 0.021 0.024 0.027 0.020	10/23/91	1	0.005	8.1	15 52
	10/20/01	000 005	<0.000	0.1 7 Q	7
0.020 <0.020 1 380,000 0.024 0.027 <0.020 0.020	16/67/01	MON'NES	<0.020	1.4	
	11/6/91	ł	0.020	7.9	21
1 0.020 1 380,000 0.024 0.027 1 <0.020	11/13/91	1	<0.020	7.7	12
380,000 0.024 0.027 0.020	11/19/91	ł	0.020	7.9	39
0.027 0.020	11/27/91	380,000	0.024	7.6	6
<0.020	12/4/91	1 ·	0.027	7.4	ω
0.020	12/12/91	1	<0.020	7.7	6
	12/18/01			1 ·	. (

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10/17/07	10/7/92	10/6/92	20/1/01	9/24/92	26/81/6	9/9/92	9/4/92	8/26/92	8/18/92	8/11/92	7/31/92	7/30/92	7/15/92	7/9/92	7/8/92	2/1021 2/1021	6/25/02	6/18/07	26/18/02 76/01/0	6/11/02	6/11/92	6/5/92	5/29/92	5/22/92	5/15/92	5/8/92	4/30/92	4/24/92	4/3/92	3/2/192	3/25/92	3/5/92	3/4/92	2/27/92	2/26/92	2/18/07	20/91/5	2/5/92	1/30/92	1/30/92	1/21/92	1/17/92	1/7/92	1/3/92	Effective 9/1/85	SPDFS Permit I imits	Sample Date Flow (GPD) Lead (mg/l)
	ł	006,875		511,500	323,100	382,300	415,900	327,500	425,100	375,900	ł	354,500	194,500	432,000		470 200	472,000	421 040	400,000	411,300		412,210		416,900	ł	**	ł	ł		ł	454,742	1	267,750	340,200	295.800	070,000	106 UUU	306,150	1	207,300	1	1	!	380,000	No Limit		Flow (GPD)
	0.008		0.007	0.011		0.018	0.012	0.009	0.094	0.011	•	0.004	0.023	ł	0.044	<0.005			800.0 	ł	0.017		0.008		0.001	<0.020	0.037	0.005	0.020	0.013		-0.015	1	<0.020		0.020	0.021		0.018	ł	0.034	0.037	<0.020	0.060	0.2		Lead (mg/ℓ)
		4	8.1	° o	•	00		8.2	7.9	7.6	7.8	I	7.6	7.8	7.7	بر 8		i.	د x 	1	8.0	°	8.5	7.7	د: 8	8.0	1	1 3	7.0 7.0	 		7.7		7.8	i	7.7	7.8		7.8	1	7.8	7.7	8.4	7.8	6.0-9.0	,	pH
	- 4	•	_	<u>`</u> u	۰	2	ω	<u>^</u>	4	<u>^</u>	1	1	10	1	ω.	ب		4	₽	1	4	1	Сı	<0.5	6	<0.5	T	2	<u>^</u> =	: <u>^</u>	1	≙	ł	13	¦ •	1 7	0		14	:	υ	10 12	13	و 13	30	, (TSS (mg/l)
	334		0/7	067	81	340	291	280	296	200	1	310	150	1	480	225	ł		300		016	1	220	283	280	330	290	310	300	400		360	1	453		240	512 087	81	290	ł	243	320	285	352 240	600	(TDS (mg/l)

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DEMEDIAI ENGINEEDING	6/15/94	6/1/94	5/26/94	5/12/94	4/28/94	4/22/94	3/31/94	3/24/94	2/17/94	1/13/94	12/21/93	12/16/93	11/16/93	10/20/93	10/2/03	56/Z/6	8/19/93	7/28/93	7/7/93	6/22/93	6/8/93	5/25/93	5/12/93	5/5/03 5/41 4/23	4/0/93	4/1/93	3/23/93	3/4/93	2/9/93	2/2/93	1/27/93	1/19/93	t6/t1/l	26/C1/71	12/1/92	11/25/92	11/24/92	11/20/92	11/19/92	11/11/92	11/10/92	11/6/92	11/5/92	10/29/92	10/15/92	Effective 9/1/85	SPDES Permit Limits	Sample Date
	444,200	403,500	371,100	378,500	386,300	417,400	385,812	515,314	462,400	398,500		419,608	435.800		400,700		505,000	389,600	430,300	467,600	387,200	387,280	415,400	403,100	394,300	394,300	444,800	421,700	380,400	371,200	414,700	381,500	417,400	167 500	427,300	402,100	1	:	!	382,400	1	371,000	,	362.000	105 000	:	No Limit	Flow (GPD)
	0.006	<0.1	<0.1	0.008	0.007	0.016	0.006	0.005	0.016	0.010	0.007	I	0.018	0.007	0.007	0.010	<0.003	0.004	<0.015	0.004	0.007	0.012	0.005	0.000	0.003	0.018	0.006	0.001	ł	0.011	I	0.010	0.020	0.008	0.007		0.007	1	0.008		0.004		0.011	0.010	0 007		0.2	Lead (mg/ℓ)
	8.1	7.98	7.91	8.15	8.1	7.96	7.69	7.68	7.77	7.71	7.9	1	7.3	8.1	7.7		, o o	8.2	7.9	8	8.1	7.9	00 i	× 1	¢ .]	1 / 2	7.9	7.6		7.9		7.5		8.02 7.8	8.1	7.6	8.1	7.8	ł	7.8	1	7.8		.9	7.6 8		6.0-9.0	pH
	3.5	9.0	7.0	10.5	8.0	5.3	4.3	5.3	4.0	8.0	≙	1	4	2 0	/ در	7 ~	• 🛆	1	ω	1	80	S	<u>^</u> ·	1 1	<u>v</u> 0	n	27	دى	ł	د ب		<u>^</u>	; ;	۰ I	يه و		≙	1	2	ł	4	1	2	7 .	4		30	TSS (mg/ℓ)
D167701E 76	229	237	263	360	328	414	479	542	262	207	300	ł	240	250	260 200	09C	250	220	200	260	180	290	290	105 00t	4/0	340	370	320	I	290	1	360		200 200	320		380	i	570	!	200	1	316	250			600	TDS (mg/l)

 Table 2. Summary of SPDES Sampling Results (Pond Effluent) Provided by Syracuse China. Pfaltzgraff Co.,

 Syracuse China Landfill; Town of Salina, New York

REMEDIAL ENGINEERING, P.C.

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Sample Date	Flow (GPD)	Lead (mg/l)	рН	TSS (mg/l)	TDS (mg/l)
SPDES Permit Limits Effective 9/1/85	No Limit	0.2	6.0-9.0	30	600
6/21/94	415,500	0.005	7.5	3.5	258
7/14/94	489,000	0.008	7.82	<1	276
8/9/94	423,700	0.057	7.87	3,5	260
8/17/94	400,500				
9/15/94	392,000	0.002	7.88	2.5	262
9/28/94	376,400	0.020	7.62	2.5	206
10/11/94	396,400	<0.1	7.81	6.0	226
10/18/94		<0.1	7.68	<1	279
11/9/94	463,023	0.008	7.53	1.3	281
11/29/94	408,314	0.012	7.97	5.3	280
1/95	392,862	0.008	8.0	5.1	514
2/95	388,460	0.007	8.2	6.8	357
3/95	408,006	0.007	7.8	3	347
4/95	424,841	0.007	7.8	6	288
5/95	415,800	0.008	7.6	<1	258
6/95	394,824	0.003	7.8	3	266
7/95	381,200	0.002	8.0	2.67	293
8/95	405,900	0.029	8.0	3.3	255
9/95	396,200	0.010	7.9	2	248
10/95	374,100	0.035	8.0	<1	248
11/95	428,300	0.010	8.0 7.9	1.5	338
12/95	399,700	0.010	7.9 7.9	1.75	410
01/96	580,400	0.019	7.9	8	410 796
02/96	398,400	0.078	7.8	2	530
03/96	405,558	0.011	7.8	2 4	460
04/96	403,358	0.032	7.8	8	290
04/90	387,458	0.005	7.8	1	290 440
	•	0.005	8	<1	
06/96	432,800		° 7.7	3	340
07/96	387,300	0.009			210
08/96	393,100	0.015	7.6	5	210
09/96	398,400	0.018	7.8	4	220
10/96	410,200	0.015	7.6	2	250
11/96	392,300	0.011	7.7	2	400
12/96	302,100	0.017	7.6	1	460
01/97	389,800	0.015	7.2	<1	560
02/97	377,200	0.011	7.4	2	310
03/97	390,000	0.01	7.3	3	350
04/97	402,600	0.024	7.4	4	290
05/97	399,800	0.035	7.6	7	270
06/97	374,900	0.011	7.6	5	290
07/97	308,200	0.006	7.4	2	250
08/97	384,700	0.021	7.6	5	250
09/97	400,100	0.012	7.6	3	170
10/97	398,400	0.018	7.5	4	220
11/97	378,600	0.01	7.6	7	360
12/97	388,800	0.017	7.5	2	240
01/98	390,000	0.014	7.7	3	420
02/98	403,200	0.016	7.8	5	270
03/98	396,700	0.01	7.6	<1	400

 Table 2. Summary of SPDES Sampling Results (Pond Effluent) Provided by Syracuse China. Pfaltzgraff Co.,

 Syracuse China Landfill; Town of Salina, New York
 Page 5

Sample Date	andfill; Town of Sa Flow (GPD)	Lead (mg/ <i>l</i>)	pН	TSS (mg/l)	Page 6 of TDS (mg/l)
SPDES Permit Limits Effective 9/1/85	No Limit	0.2	6.0-9.0	30	600
04/98	363,400	0.014	7.5	3	340
05/98	352,100	0.011	7.4	3	180
06/98	399,400	0.011	7.6	4	230
07/98	402,300	0.025	7.6	4	230
08/98	401,100	0.017	7.5	6	240
09/98	379,400	0.009	7.4	4	220
10/98	399,100	0.003	7.1	6	250
11/98	401,200	0.016	7	7	170
12/98	389,000	0.033	6.9	5	270
01/99	400,000	0.011	6.5	3	630
02/99	348,000	0.011	7.5	12	340
03/99	402,500	0.008	7.6	3	580
SPDES Permit Limits Effective 4/1/99	No Limit	0.07	6.5-8.5	20	600
04/99	436,845	0.016	7.3	7	370
05/99	412,301	0.012	7.2	6	280
06/99	494,389	0.026	7.5	4	230
07/99	404,654	0.026	7.8	4	300
08/99	356,511	0.011	7.6	4	250
09/99	358,530	0.015	6.9	7	220
10/99	380,269	0.011	7.1	6	240
11/99	420,700	0.054	7.4	5	240
12/99	437,072	0.021	7.4	9	280

Table 2. Summary of SPDES Sampling Results (Pond Effluent) Provided by Syracuse China.	Pfaltzgraff Co.,
Syracuse China Landfill; Town of Salina, New York	Page 6

GPD = Gallons per day.

 $mg/\ell = Milligrams$ per liter.

pH is given in standard units.

TSS = Total suspended solids.

TDS = Total dissolved solids.

---- = Parameter not tested.

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New TOFK.		Page 1 of			
Case 1 Base Facility Flow, 10-Year Stor	m	Case 2 Base Facility Flow, 1-Year Storm, Facility (Production Upset)			
Base Plant Flow (gpm)	264	Base Plant Flow (gpm)	264		
Base Plant Flow TSS Concentration (mg/ℓ)	7	Base Plant Flow TSS Concentration (mg/ ℓ)	7		
Non-Settable Percentage TSS	55	Non-Settable Percentage TSS	55		
TSS Concentration After Pond Settling	3.9	TSS Concentration After Pond Settling	3.9		
10-Year Storm Flow (gpm)	7,179	1-Year Storm Flow (gpm)	4,450		
10-Year Storm Flow TSS Concentration (mg/l)	45	1-Year Storm Flow TSS Concentration (mg/ ℓ)	45		
Non-Settable Percentage TSS	25	Non-Settable Percentage TSS	25		
TSS Concentration After Pond Settling	11	TSS Concentration After Pond Settling	11		
		Plant Upset Flow (gpm) for 1.25 Hours	25		
		Plant Upset Flow TSS Concentration (mg/l)	407,810		
		Non-Settable Percentage TSS	55		
		TSS Concentration After Pond Settling	224,296		
Concentration of Flow Exiting Ponds (mg/ℓ) when Base and 10-Year Flows Contribute (Mass Balance of 2 Flows)	11	Concentration of Flow Exiting Ponds (mg/ ℓ) when Base, 1-Year and Plant Upset Flows all Contribute (First 1.25 Hours) (Mass Balance of 3 Flows)	1,194		
		Concentration of Flow Exiting Ponds (mg/l) when Base and 1-Year Flows all Contribute (Final 4.75 Hours) (Mass Balance of 2 Flows)	11		
6-Hour Composite Concentration (mg/l) (Base and 10-Year Storm Flow Contribute entire 6 Hours)	11	6-Hour Composite Concentration (mg/l) (Upset Flow Only Contributes for 1.25 Hours, the 1-Year and Base Flow Contribute for the Remaining 4.75 Hours)	257		
SPDES TSS Limit	20	SPDES TSS Limit	20		

Table 3. Calculation of Final Concentrations for Pond Effluent. Pfaltzgraff Co., Syracuse China Landfill; Salina, New York. Page 1 of 1

1-year storm flow extrapolated from existing storm flow data (Appendix C).

10-year storm flow taken from existing storm flow data (Appendix C).

Storm flow TSS concentration taken from 5/31/98 storm (minus the average plant flow TSS) (see Table 1).

Non-settable percentage of TSS in stormwater estimated based on typical storm water particle size distributions.

Upset flow and time based on information provided by Syracuse China in 3/00.

Upset flow TSS concentrations taken from samples collected by Syracuse China in 3/00.

Quantity (approximate)	Unit	Equipment or Structure	Size
360	feet	High density polyethylene (HDPE) SDR 32.5 pipe	36-inch diameter
. 3	ponds	Wastewater settling ponds (includes fill materials, geotextiles, geosynthetic clay liner and rip rap)	417,000 gallons (approximate) each 1.25 million gallons total
450	feet	High density polyethylene (HDPE) pressure pipe	24-inch diameter
5	headwalls	Concrete headwalls	headwall for 24-inch pipe
3	weirs	Concrete overflow weirs	65 feet long
3	baffles	Floating baffles	65 feet long
3	spillways	Emergency spillways (includes geotextile and riprap)	15 feet wide, 6 inches deep
2	swales	Swales to carry water from emergency spillways and Pond 3 outlet to SPDES outfall	2 feet deep at center and 8 feet wide a top (approximate)

 Table 4. List of Structures and Equipment for New Wastewater Settling Ponds. Pfaltzgraff Co., Syracuse China

 Landfill; Town of Salina, New York
 Page 1 of 1

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·····	Units	Formula	
Length of Settling Basin (L)	feet	given	315
Width of Settling Basin (W)	feet	given	60
Depth of Settling Basin (average) (H)	feet	given	6.50
Flow into Settling Basin (Q)	cfs	given	10.5
Horizontal Velocity of Particles (V_h)	fps	V _h ≈Q/(H*W)	0.0269
Detention Time (t)	seconds	t=L/V _h	11,700
Settling Velocity (V ₀)	fps	Vo=H/t	0.00056

 Table 5. Calculation of Smallest Particle which can Settle 100%. Pfaltzgraff Co.; Syracuse China Landfill, Town of Salina, New York.

 Page 1 of 1

Length of settling basin taken from Roux Associates, Inc. Drawing no. 57701034- the particle flow path through the basin.

Width of settling basin taken from Roux Associates, Inc. Drawing no. 57701034- the dimension perpendicular to the particle flow path through the basin.

Depth of settling basin taken from Roux Associates, Inc. Drawing no. 57701034.

The flow into the settling basin is the 1-year storm flow plus the average daily facility flow. 1-year storm flow taken from "Daily Precipitation vs. Maximum Flow Data" graph is included in Appendix C. The 1-year, 24-hour rainfall for Syracuse extrapolated from a graph based on data in TR-55 Manual- Urban Hydrology for Small Watersheds. The maximum daily facility flow taken from data obtained from Syracuse China.

cfs = Cubic feet per second.

fps = Feet per second.

Assumptions:

- 1. Ideal rectangular settling basin.
- 2. Settling of discrete particles occurs.
- 3. Even distribution of flow entering and leaving basin.
- 4. Settling basin zones consist of entrance, outlet and sludge zones.
- 5. Uniform distribution of particles throughout the depth of entrance zone.
- 6. Particles that enter sludge zone remain there and particles that enter outlet zone are removed.

Reference: Unit Operations and Processes in Environmental Engineering, Tom D. Reynolds, 1982 (included in Appendix H.).

Weir Calculation						
Name	Variable	Unit	Equation	Facility Flow		
acility Flow	Q	gpd	Given	468,000		
Weir Length Weir Loading Rate	L W	feet gpd/foot	Given Q / L	65 7,200		

Table 6. Weir Loading Rate Calculation. The Pfaltzgraff Co., Syracuse China Landfill; Salina, New York.

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Page 1 of 1

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65 feet of weir (width of pond) is adequate for normal facility flow. Recommended weir loading is 15,000-20,000 gpd/foot.

			Emergency Spillway Calculation	
Name	Variable	Unit	Formula	Result 6-inch deep 15 feet wide weir
eight of Water Flowing Over Wier	Н	feet	Given	0.5
eight of Weir	Y	feet	Given	0.5
ehbock Coefficient	Cı	-	$C_1 = [0.6035 + 0.08138(H/Y) + (0.000295/Y)] * [1 + (0.00361/H)]^{3/2}$	0.686
idth of Weir	b	feet	Given	15
cceleration of Gravity	g	feet/sec ²	Constant	32.2
ow Over the Weir	Q _{cfs}	cfs	$Q=(2/3)*C1*b*(2g)^{1/2}*(H)^{3/2}$	19
ow Over the Weir	Q _{gpm}	gpm	Q _{gom} =Q _{cfs} *448.831	8,734

Table 7. Emergency Spillway Flow Capacity Calculation. The Pfaltzgraff Co, Syracuse China Landfill; Salina, New York.

The spillway must be able to pass the 25-year storm flow which is 8,680 gpm, so the spillway size is adequate.

 $feet/sec^2 = feet per second squared.$

cfs = cubic feet per second.

gpm = gallons per minute.

Spillway equations from the Civil Engineering Reference Manual, Sixth Edition, Michael Lindeburg, P.E., 1997, pages 5-7 and 5-8.

Page 1 of 1

•			8 , 2		, ,		0
Pipe Flow Calculation			<u>, </u>				
				Result	Result	Result	
Name	Variable	Unit	Formula	(24-inch)	(18-inch)	(19-inch)	
Inside Diameter of the Pipe	D	feet	Given	2	1.5	1.583	
Hazen Williams Coefficient	Ci	-	Reference 1, page 21-24	130	130	130	
Slope of the Pipe	S	ft/ft	Given (Pond Grading Plan)	0.016	0.016	0.016	
Flow	Q	cfs	$Q=0.432*C_1*D^{2.63}*S^{0.54}$	36.796	17.267	19.894	
Flow	Q	gpm	$Q_{snm} = Q_{cfs} * 448.831$	16,515	7,750	8,929	

Table 8. Pipe Flow and Friction Loss Calculations. The Pfaltzgraff Co, Syracuse China Landfill; Salina, New York.

The pipe must be able to pass the 25-year storm flow which is 8,680 gpm.

A 19-inch inside diameter pipe is needed to pass the 25-year storm. A 24-inch (inside diameter) pipe is proposed to be conservative.

Friction Loss Calculation					
				Result	
Name	Variable	Unit	Formula	(24-inch)	
Length of Pipe	L	feet	Given (Pond Grading Plan)	32	
Flow	Q	cfs	Given (25-Year Storm)	19	
Hazen Williams Coefficient	С	-	Reference 1, page 21-24	130	
Inside Diameter of the Pipe	D	feet	Given (pipe sizing calculation)	1.686	
Head Loss Due to Friction	h _f	feet	$h_{f} = (4/727/D^{4.87})^{*}(L)^{*}(Q/C_{1})$	0.35	_

sf = square feet.

ft/ft = foot per foot.

cfs - cubic feet per second.

gpm = gallons per minute

The equations for pipe flow and friction loss (Hazen Williams) is taken from the Civil Engineering Handbook, page 21-24.

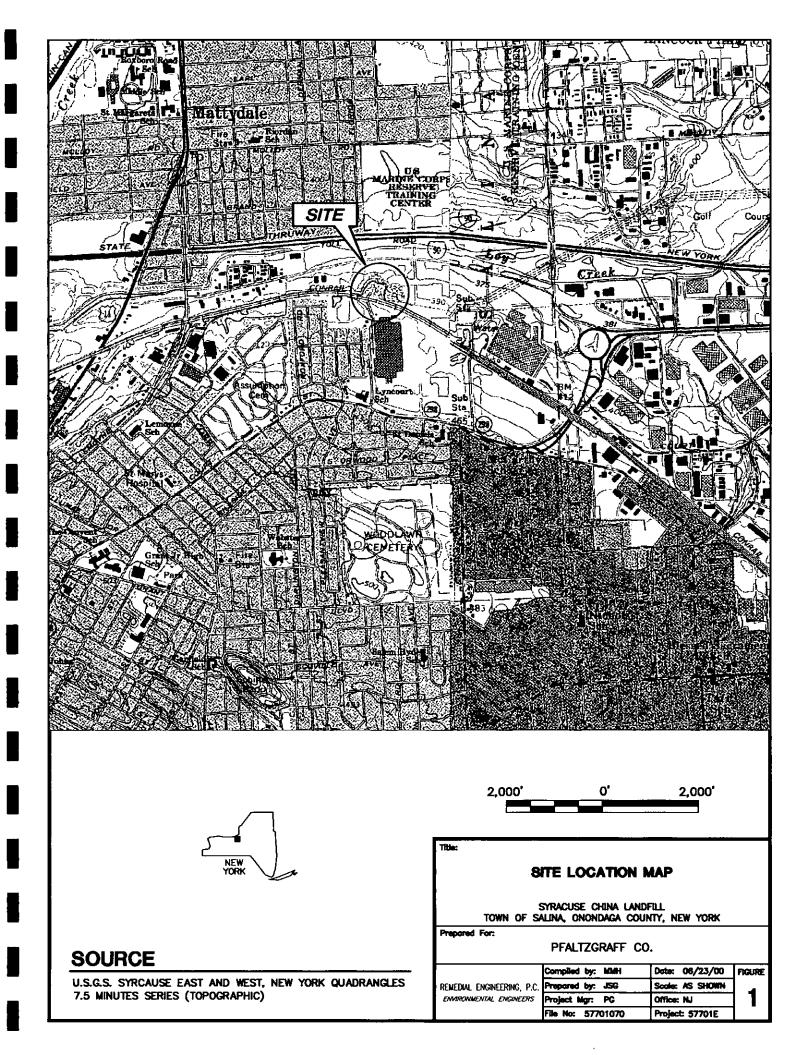
Page 1 of 1

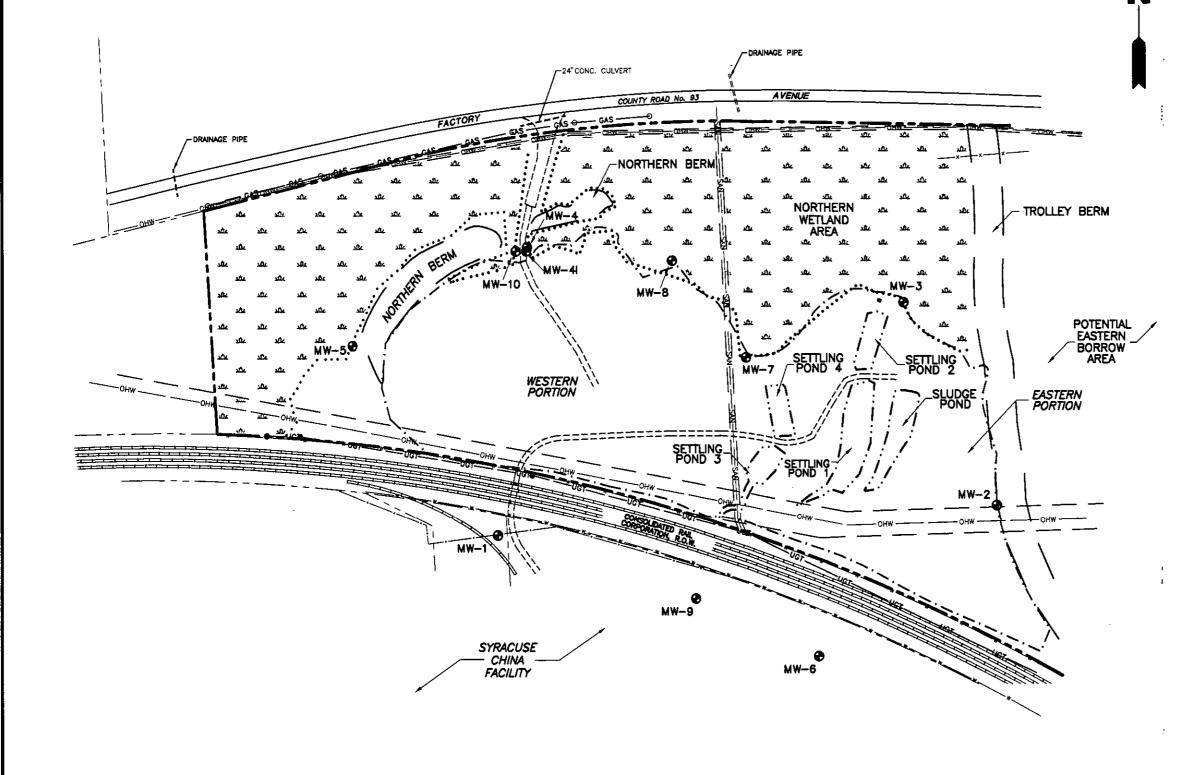
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LEGEND

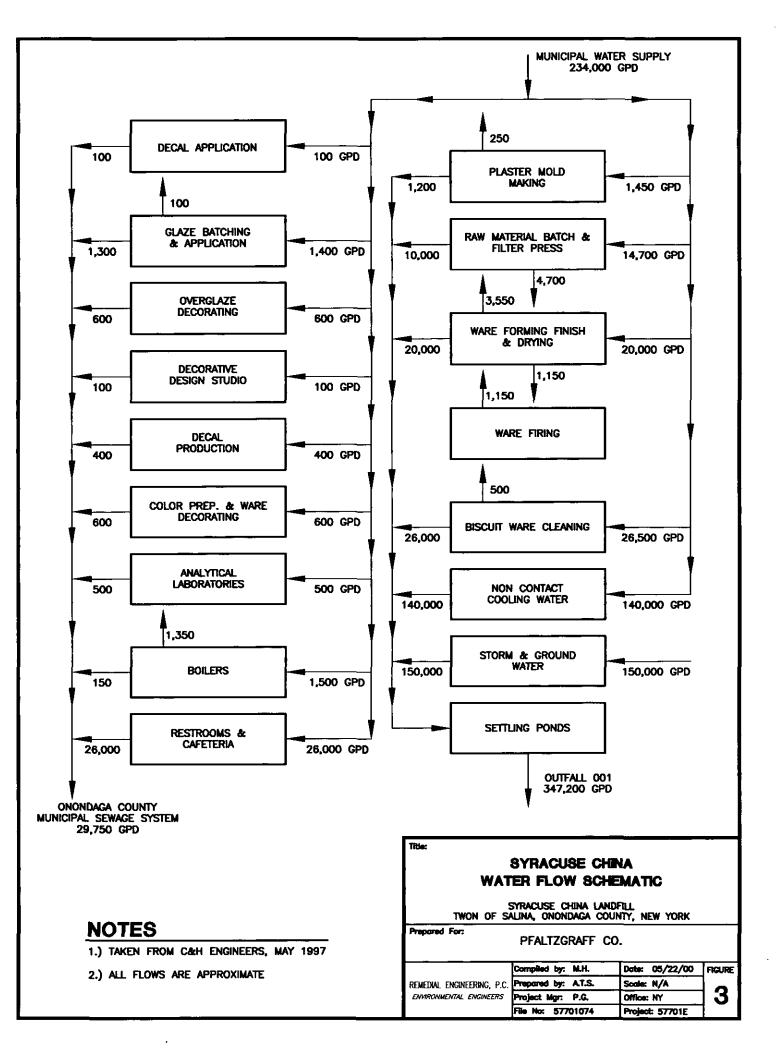
	PROPERTY LINE
_ , _ , _ , _ , _	EXISTING LANDFILL LIMIT
	EDGE OF POND
• • • • • • • • • • • • • •	WETLAND BOUNDARY
	EASEMENT
भूमर भूमर भूमर	WETLANDS
~	EXISTING ACCESS ROAD
	BERM LIMIT
xx	FENCE
	RAILROAD TRACKS
	APPROXIMATE WATERLINE LOCATION
онw	OVERHEAD ELECTRICAL TRANSMISSION WIRES
\$AN	SUBSURFACE SANITARY SEWER
CAS	MARKED GAS LINE LOCATION
UGT•	MARKED UNDERGROUND TELEPHONE LINE (FIBER OPTIC) LOCATION
● ^{MW-3}	GROUND-WATER MONITORING WELL LOCATION AND DESIGNATION

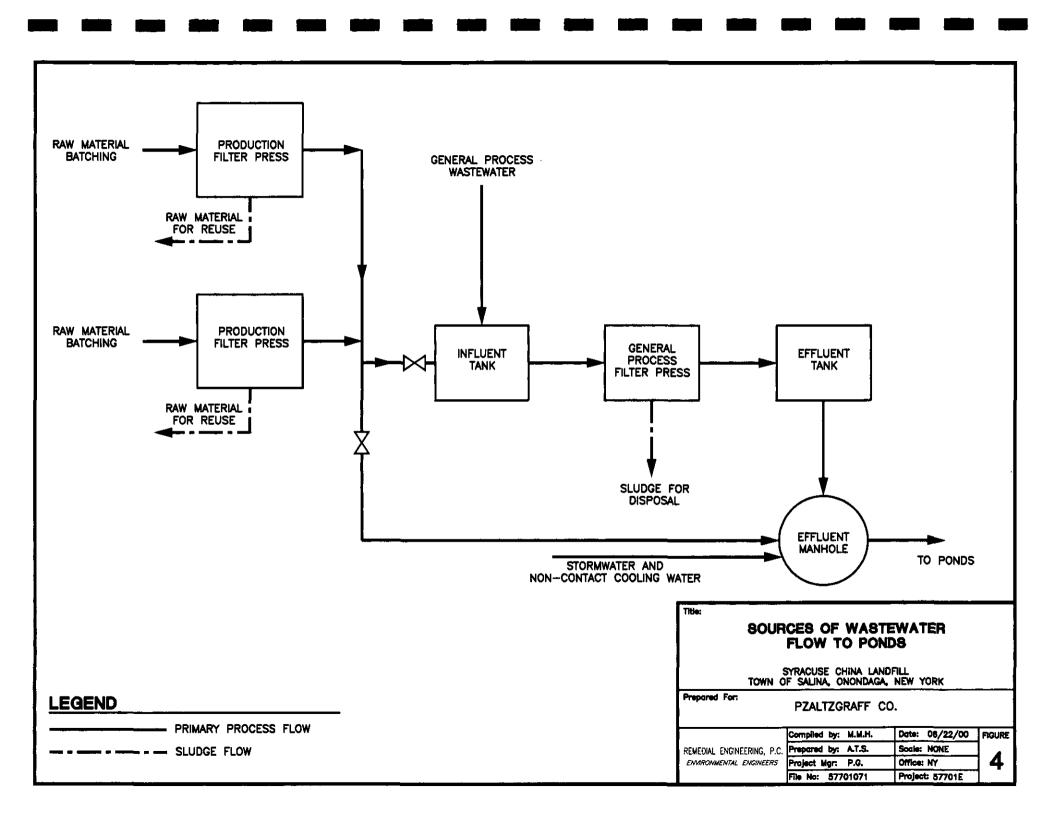
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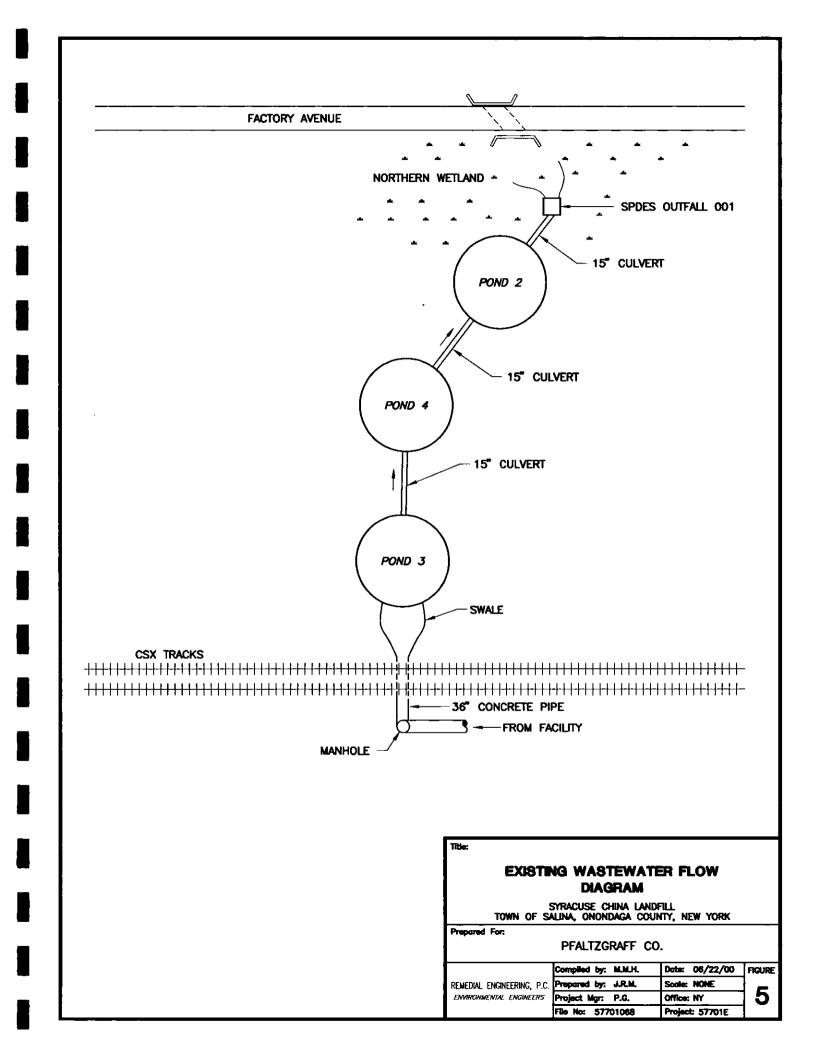
- 1. BASE MAP INFORMATION BASED ON PLAN ENTITLED:, "MAP OF PARTIAL BOUNDARY & TOPOGRAPHIC SURVEY", PARCEL OF LAND, PART OF MILITARY LOT 19, TOWN OF SALINA, JANUARY 20, 1998, SCALE 1=50', PROJECT 90006, PREPARED BY RYAN SURVEY, PORTER BUILDING, NORTHERN LIGHTS OFFICE PARK, SYRACUSE, NY 13220-3225.
- 2. WETLAND BOUNDARIES LOCATED AS FLAGGED BY TERRESTRIAL ENVIRONMENTAL SPECIALISTS, INC. (DECEMBER, 1990) REGULATED WETLANDS EXIST NORTH OF DELINEATION LINE.
- 3. SUBSURFACE SANITARY SEWER IS DIVIDING LINE BETWEEN EASTERN AND WESTERN PORTIONS OF LANDFILL.

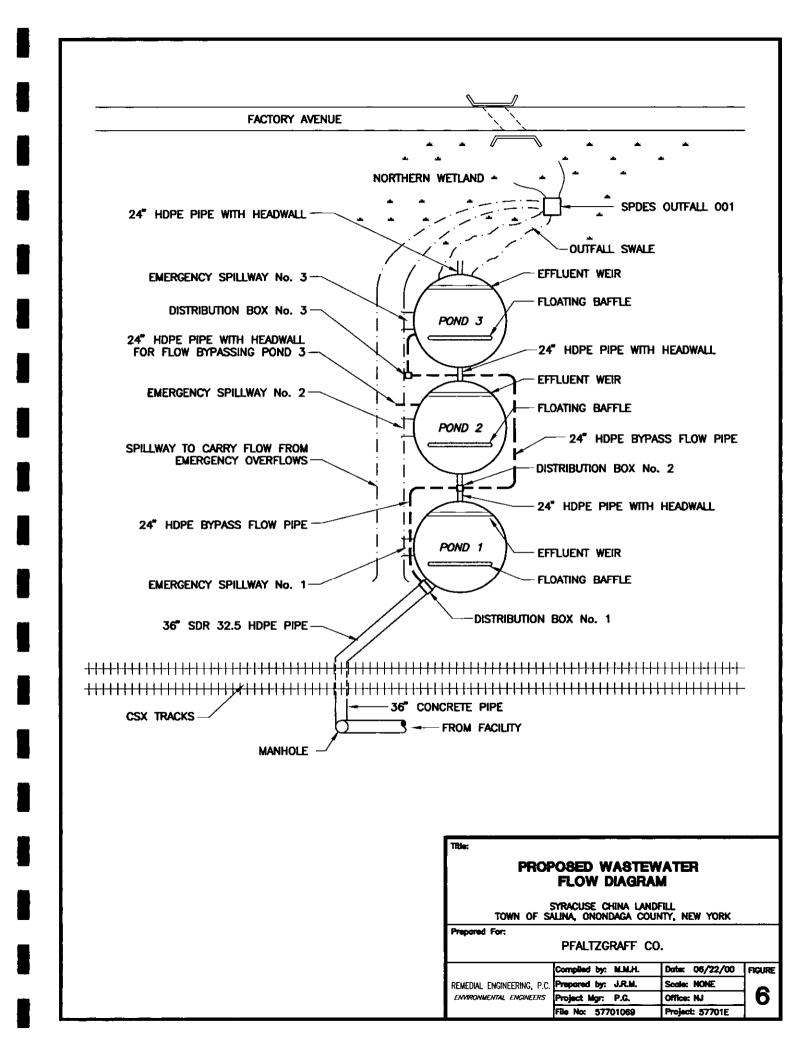
200'	0'	200'

Title:							
	SITE PLAN						
	SYRACUSE CHINA LANDI ALINA, ONONDAGA COUM						
Prepared For: PFALTZGRAFF CO.							
	Complied by: M.M.H.	Dote: 06/22/00	FIGURE				
REMEDIAL ENGINEERING, P.C.	Prepared by: J.S.G.	Socie: AS SHOWN					
ENVIRONMENTAL ENGINEERS	Project Mgr: P.G.	Office: NJ	2				
	File No: 57701009	Project: 57701E					









APPENDICES

APPENDICES

APPENDIX A

APPENDIX A

APPENDIX A

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LETTER OF ENDORSEMENT FROM SYRACUSE CHINA

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Robert D. St. John, CIH, CHMM Manager – Environment, Health and Safety

July 20, 2000

Mr. Shayne A. Mitchell Environmental Engineer NYSDEC Division of Water Bureau of Water Permits, Room 314 50 Wolf Road Albany, New York 11788

Re: Remedial Engineering P.C. Engineering Report for Wastewater Settling Ponds

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Dear Mr. Mitchell:

Syracuse China Company, a Unit of Libbey Inc. has reviewed the submittal "Engineering Report for Wastewater Settling Ponds" prepared by Remedial Engineering, P.C., and endorses the design concept. The report details the proposed upgrading/reconstruction of Syracuse China's existing wastewater settling ponds (SPDES Permit Number NY0100137) as part of the remedial action at the Syracuse China Landfill Inactive Hazardous Waste Disposal Site (Site No. 7-34-053). The report describes the construction of three lined settling ponds, with a total capacity of approximately 1.25 million gallons. Please note that the detailed design drawings present the proposed design concept, however, Syracuse China Company reserves the right, with NYSDEC approval, to modify the details of pond construction as site conditions allow.

If you require additional information, please call me at (419) 727-2493.

Very truly yours,

Robert D Alt

Robert D. St. John CIH, CHMM Manager – Environment, Occupational Safety and Health .

APPENDIX B

APPENDIX B

APPENDIX B

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SPDES PERMIT

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New York State Department of Environmental Conservation

Division of Environmental Permita, Region 7 615 Erie Boulevard West, Syracuse, New York 13204-2400 Phone: (315) 426-7438 FAX: (315) 426-7425



April 1, 1999

Philip Harvard Env. Manager Syracuse China Company P.O. Box 4820 Syracuse, NY 13221-4820

RE: State Pollutant Discharge Elimination System (SPDES) NY 0100137 DEC #7-3115-00160/00011

Dear Mr. Harvard:

Enclosed please find your permit with the effective date changed to April 1, 1999.

Included is a SPDES Permit fact sheet.

Unless otherwise specified this permit will become effective immediately unless you petition, pursuant to ECL Section 17-0907, that you be given an opportunity to be heard in connection with this determination and where applicable.

Should you have any questions feel free to call.

Sincerely,

Robert A. Torba Deputy Permit Administrator

RAT/eab

cc: Robert Hannaford, Albany Steve Eidt, Region 7 Shayne Mitchell, Albany

Received Time

1-20-2 (1/89)

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION State Pollutant Discharge Elimination System (SPDES) **DISCHARGE PERMIT**

Special Conditions (Part I)

Industrial Code: 3262	SPDES Number: NY - 0100137
Discharge Class (CL): 01	DEC Number: 7-3115-160/1-0
Toxic Class (TX): T	Effective Date (EDP): 4 /01 / 99
Major Drainage Basin: 07	Expiration Date (ExDP): 4 / 01 /2004
Sub Drainage Basin: 02	Modification Date(s):
Water Index Number: 0-66-12-P154-3	Attachment(s): General Conditions (Part II) Date: 11/90
Compact Area:	

This SPDES permit is issued in compliance with Title 8 of Article 17 of the Environmental Conservation Law of New York State and In compliance with the Clean Water Act as amended, (33 U.S.C. Section 1251 et. seq.) (hereafter referred to as "the Act").

PERMITTEE NAME AND ADDRESS

Attention: Vice President of Mfg.

Name:	Syracuse China Company		_		
Street:	PO Box 4820				
	Syracuse	State:	NY	Zip Code:	13221-4820
is authorized to	discharge from the facility described below:				

FACILITY NAME AND ADDRESS

and:

Name:	Syracuse	China Compa	пу				
Location (C,T,V	: Syracuse	(C)			County: Ono:	ndaga	
Facility Address	: 2900 Cour	t Street_	_				
City:	Syracuse		Sta	ate:	NY Zip Cod	e: 13221	
NYTM - E:			NYTM - N:		4.		
From Outfall No	.: 001	at Latitude:	<u>43</u> 05'	33"	& Longitude:	760 07'	49"
into receiving w	aters known as:	Ley Creek			Class	a: B	
(list other Outfalls,	Receiving Waters	s & Water Classifi	cations)				
002 -	Lev Creek	. Class: F	3				

in accordance with the effluent limitations, monitoring requirements and other conditions set forth in Special Conditions (Part I) and General Conditions (Part II) of this permit.

DISCHARGE MONITORING REPORT (DMR) MAILING ADDRESS

Mailing Name:	Syracuse	China	Company				•
Street:	P.O. Box	4820			_		······································
City:	Svracuse			State:	NY	Zin Code:	13221-4820
Responsible Of				 e marce.	A1 4		TOURT TOTO

This permit and the authorization to discharge shall expire on midnight of the expiration date shown and the permittee shall not discharge after the expiration date unless this permit has been renewed, or extended pursuant to law. To be authorized to discharge beyond the expiration date, the permittee shall apply for a permit renewal no less than 180 days prior to the expiration date shown above. DISTRIBUTION:

Permit Administrator:	
Robert A. Torba	
Address;	
615 Erie Blvd. W., Syracuse,	NY 13204

Received Time Apr. 12. 10:38AM Print Time

Apr. 12, 10:45AM

SPE	DES PER	MIT NUM	IBER NY	0100137
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PERMIT LIMITS, LEVELS AND MONITORING DEFINITIONS

	BIIIID, BETEBBIII BIIII				·····		<u>.</u>		
OUTFALL	WASTEWATER TYPE		RECEI	VING WATER		EI	FECTIVE		EXPIRING
	This cell describes the type of wastewater authorized for include process wastewater, storm water, non-contact c		This cell lists classifi which the listed outf		talė to		ate this page in effect.		fate this page is nger in effect
PARAMETER	R MINIMUM		MAXIMUM		UN	TS S.	AMPLE FREQ.	sÆ	AMPLE TYPE
pH	The minimum level that must be maintained at all tim	he minimum level that must be maintained at all times. The maximum level that may not be exceeded at any time. SU							
PARA- METER	CALCULATED LIMIT	COMPLIANCE LEVE	EL ACTION LEVEL	UNITS	MDL AND	PQL	SAMPLE FREQUENCY		SAMPLE TYPE
	Daily Avg. and Daily Max. are defined below. The D calculated limit is the limit that has been derived based d on the assumptions and rules in place at the time the d permit is written. Examples of these assumptions w include receiving water hardness, pH and temperature; d rates of other discharges to the receiving stream; c conservatism of substances in the environment; etc. If the the assumptions or rules change, the calculated limit c may, after due process, change. The Calculated Limit c is developed without consideration of what level is q technologically achievable or what can be quantitated analytically. If a calculated limit is not included in this a column, but a compliance level is included in the next column, the calculated limit is the compliance level.	defined below. All determinations of complia with substance specific discharge limits are made comparing monitoring res to the compliance level. T compliance level is develo considering what can be quantituted analytically or level is technologically achievable in the permitte	Type II Action Levels are by monitoring requirements, like as defined below, that trigger what additional monitoring e's and permit	include units of flow, pH, Temperature, mass or concentration. Examples include SU, °F, µg/), lbs/d, p cte.	detection 1 practical qu limits the	imits and antitation nust make bleeffort te when in the vater using ensitiv		ith, Ierly, Y.	Examples include grab, 24 hour composite and 3 grab sample3 collected over a 6 hour period.

DAILY DISCHARGE.: The discharge of a pollutant measured during a calendar day or any 24-hour period that reasonably represents the calendar day for the purposes of sampling. For pollutants expressed in units of mass, the 'daily discharge' is calculated as the total mass of the pollutant discharged over the day. For pollutants with limitations expressed in other units of measurement, the 'daily discharge' is calculated as the pollutant over the day.

DAILY MAX .: The highest allowable daily discharge.

DAILY AVG.: The highest allowable average of daily discharges over a calendar month, calculated as the sum of all daily discharges measured during a calendar month divided by the number of daily discharges measured during that month.

ACTION LEVELS: Routine Action Level monitoring results, if not provided for on the Discharge Monitoring Report (DMR) form, shall be appended to the DMR for the period during which the sampling was conducted. If the additional monitoring requirement is triggered as noted below, the permittee shall undertake a short-term, high-intensity monitoring program for the parameter(s). Samples identical to those required for routine monitoring purposes shall be taken on each of at least three consecutive operating and discharging days and analyzed. Results shall be expressed in terms of both concentration and mass, and shall be submitted no later than the end of the third month following the month when the additional monitoring requirement was triggered. Results may be appended to the DMR or transmitted under separate cover to the same address. If levels higher than the Action Levels are confirmed, the permittee is not authorized to discharge any of the listed parameters at levels which may cause or contribute to a violation of water quality standards.

 TYPE I:
 The additional monitoring requirement is triggered upon receipt by the permittee of any monitoring results in excess of the stated Action Level.

 TYPE II:
 The additional monitoring requirement is triggered upon receipt by the permittee of any monitoring results that show the stated action level exceeded for four of six consecutive samples, or for two of six consecutive samples by 20 % or more, or for any one sample by 50 % or more.

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315-455-4575

Part I, Page 2 of 12

91-20-28 (1/98)

FINAL

Part I, Page 3 of 1

PERMIT LIMITS, LEVELS AND MONITORING

OUTFALL NUMBER	WASTEWATER TYPE	RECEIVING WATER	EFFECTIVE	EXPIRINC
001	Outfall 01A, noncontact cooling water, storm water nunoff and groundwater.	Ley Creek	EDP	EDP + S Years

PARAMETER	MININ	IUM	MA	XIMUM	U	INITS	SAM	PLE FREQU	ENCY	S	AMPLE TYPE	FOOTNUTES
pH 6.5		5	8.5		. <u>.</u>	SU		2/Wock		Grab		
PARAMETER		CALCULA	TED LIMIT	STATUS		MONITORING ACTION LEVEL			MDL	PQL	SAMPLE	SAMPLE
		Daily Avg.	Daily Max.			TYPEI	TYPE II UNITS		(µg/l)	(µ g/l)	FREQUENCY	TYPE
Flow								GPD			Continuous	Recorder
Temperature								٩F			2/Week	Grab
Total Dissolved Solids								mg/l			Weekly	6 hr. composite
Total Suspended Solid	\$		1	Mittate 243				mg/l			Weekly	6 hr. composite
BOD5						21		ന്യ/ി			Quarterly	6 hr. composite
Oil & Grease				Monau III			Ì	mg∕1	<u> </u>		Monthly	Grab
Boron, Total						840		μg/l	<u> </u>		Quarterly	6 hr. composite
Lead, Total						L		μ <u>ε</u> /1		1	2/Month	6 hr. composite

SPECIAL CONDITION:

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The permittee must report both the concentration (in mg/l or µg/l) and the mass loading (in lbs/day) on the Discharge Monitoring Reports for all parameters except flow, pli, temperature and set solids. This requirement applies to all outfalls.

DEFINITIONS:

Recorder - A flow measurement system that continuously measures and displays the instantaneous flow rate, and records the cumulative discharge volume versus time on paper and/or electron-

Totalizer - A flow measurement system that continuously measures and displays the instantaneous flow rate and the cumulative discharge volume.

SPDES PERMIT NUMBER NY 0100137

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FINAL91-20-2aa(3/98)

PERMIT LIMITS, LEVELS AND MONITORING

OUTFALL NUMBER	WASTEWATER TYPE	RECEIVING WATER	EFFECTIVE	EXPIRING	ŭ
01A	Dinnerware production contact wastewaters - raw material batching, ware forming and firing, ware cleaning and plaster mold making.	Outfall 001	July 1, 1999	EDP + S years	65:50

PARAMETER	MINI	MUM	м		l	INITS	SAM	PLE FREQU	ENCY	S	AMPLE TYPE	FOOTNOTES (FN	
pH	Mo	nitor		Monitor		su	<u> </u>	Monthly		L	Grah		
PARAMET	ER	CALCULA	TED LIMIT	TED LIMIT		MONITORING ACTION LEVEL			MDL	PQL	SAMPLE	SAMPLE	FN
		Daily Avg.	Daily Max.	Sala a		TYPEI	TYPE II	UNITS	(µg/l)		FREQUENCY	TYPE	
Flow		1		Monder				GPD			Weekly	Totalizer	
Fotal Dissolved Solids				Monsorill				mg/l	[Monthly	Grab	
Fotal Suspended Solids					HIRI	40		mg/l			Monthly	Grab	

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91-20-7aa(3/98)

OUTFALL NUMBER

PARAMETER

pН

Total Dissolved Solids

Total Suspended Solids

PARAMETER

SPDES PERMIT NUMBER NY 0100137

MDL

(µg/l)

UNITS

GPD

mg/l

mg/l

PQL

(µg/l)

SAMPLE

FREQUENCY

Weckly

Monthly

Monthly

Part I, Page 5 of 12

SAMPLE

TYPE

Totalizer

Grab

Grab

		Ľ	SFDES PERMIT NUMBER		art I, rage 5 of 12
	PERMIT LIMITS,	LEVELS A	AND MONITORING		· ·
	WASTEWATER TYPE		RECEIVING WATE	R EFFECTIVE	EXPIRING
Ceramic material preparat	ion contact wastewaters.		Outfail 001	July 1, 1999	EDP + 5 years
MINIMUM	MAXIMUM	UNITS	SAMPLE FREQUENCY	SAMPLE TYPE	FOOTNOTES (FN)
Monitor	Monitor	<u>su</u>	Monthly	Grab	

MONITORING ACTION LEVEL

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TYPEI

FINAL

01B

Flow

CALCULATED LIMIT

Daily Avg.

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Daily Max.

Dhihi

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Time
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10:38AM

315-455-4575

FN

91-20-2 ma(3/98)

SPDES PERMIT NUMBER NY 0100137

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OUTFALL NUMBER			WASTE	WASTEWATER TYPE						RECEIVING WATER			NO
102	Storm water	runoff.	<u> </u>					Storm sewer tri	b. to Ley C	Creek	EDP	EDP + 5 Years	
PARAMETER	MINIM	IUM	м	AXIMUM	U	JNITS	SAM	MPLE FREQUE	ENCY	SAMPLE TYPE		FOOTNOTES (FN)	
рН	6.0)		9.0		su	<u> </u>	Monthly		Grab			
PARAMET	ER	CALCULA	TED LIMIT	I I SNEORUTA DOMPLIANCE		MONITO ACTION	LEVEL	UNITS	MDL (µg/l)	PQL (µg/1)	SAMPLE FREQUENCY	SAMPLE TYPE	FN
 Flow	<u></u>	Daily Avg.	Daily Max.				TYPEII	GPD			Monthly	Estimate	+
Settleable Solids		<u> </u>						ml/l			Monthly	Grab	+
Oil & Grease		<u> </u>						 mg/l	 		Monthly	Grab	+

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b)

SPDES PERMIT NUMBER NY0100137 Part I, Page 7 of 12

SCHEDULE OF COMPLIANCE

The permittee shall comply with the following schedule.

Outfall Number(s)	Compliance Action	Due Date
001, 01A, 01B and 002	A short term monitoring program shall be performed to determine the presence of the following parameters:	October 1, 1999
	Outfall 001 - dissolved oxygen, total aluminum, ammonia (as N) and MBAS.	
	Outfall 01A - total lead and phthalates (EPA 625).	
	Outfall 01B - total lead and phthalates (EPA 625).	
	Outfall 002 - TDS, TSS and total lead.	
	This program must include, at a minimum, the collection and analysis of one representative effluent sample per day for 3 consecutive discharge days. Analytical methodologies shall achieve detection limits comparable to those utilized by this Department.	
	Number(s) 001, 01A,	Number(s) Compliance Action 001, 01A, 01B and 002 A short term monitoring program shall be performed to determine the presence of the following parameters: Outfall 001 - dissolved oxygen, total aluminum, ammonia (as N) and MBAS. Outfall 01A - total lead and phthalates (EPA 625). Outfall 01B - total lead and phthalates (EPA 625). Outfall 002 - TDS, TSS and total lead. This program must include, at a minimum, the collection and analysis of one representative effluent sample per day for 3 consecutive discharge days. Analytical methodologies shall achieve detection limits comparable to those utilized by this

The above compliance actions are one time requirements. The permittee shall comply with the above compliance actions to the Department's satisfaction once. When this permit is administratively renewed by NYSDEC letter entitled "SPDES NOTICE/RENEWAL APPLICATION/PERMIT", the permittee is not required to repeat the submission. The above due dates are independent from the effective date of the permit stated in the letter of "SPDES NOTICE/RENEWAL APPLICATION/PERMIT".

The permittee shall submit a written notice of compliance or non-compliance with each of the above schedule dates no later than 14 days following each elapsed date, unless conditions require more immediate notice under terms of the General Conditions (Part II), Section 5. All such compliance or non-compliance notification shall be sent to the locations listed under the section of this permit entitled RECORDING, REPORTING AND ADDITIONAL MONITORING REQUIREMENTS. Each notice of <u>non-compliance</u> shall include the following information:

- 1. A short description of the non-compliance;
- 2. A description of any actions taken or proposed by the permittee to comply with the elapsed schedule
- requirements without further delay and to limit environmental impact associated with the non-compliance;
- 3. A description or any factors which tend to explain or mitigate the non-compliance; and
- 4. An estimate of the date the permittee will comply with the elapsed schedule requirement and an assessment * of the probability that the permittee will meet the next scheduled requirement on time.

 c) The permittee shall submit copies of any document required by the above schedule of compliance to NYSDEC Regional Water Engineer at the location listed under the section of this permit entitled RECORDING, REPORTING AND ADDITIONAL MONITORING REQUIREMENTS and to the Bureau of Water Permits, Room 314, 50 Wolf Road, Albany, N.Y. 12233-3505, unless otherwise specified in this permit or in writing by the Department. BMPinit2.99

6.

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SPECIAL CONDITIONS - BEST MANAGEMENT PRACTICES

- 1. The permittee shall develop a Best Management Practices (BMP) plan to prevent, or minimize the potential for, release of significant amounts of toxic or hazardous pollutants to the waters of the State through plant site runoff; spillage and leaks; sludge or waste disposal; and storm water discharges including, but not limited to, drainage from raw material storage. Completed BMP plans shall be submitted by October 1, 1999 to the Regional Water Engineer at the address shown on the Recording, Reporting and Additional Monitoring Requirements. The BMP plan shall be implemented within 6 months of submission, unless a different time frame is approved by this Department.
- 2. Subsequent modifications to or renewal of this permit does not reset or revise the deadline set forth in (1) above, unless a new deadline is set explicitly by such permit modification or renewal.
- 3. The permittee shall review all facility components or systems (including material storage areas; in-plant transfer, process and material handling areas; loading and unloading operations; storm water, erosion, and sediment control measures; process emergency control systems; and sludge and waste disposal areas) where toxic or hazardous pollutants are used, manufactured, stored or handled to evaluate the potential for the release of significant amounts of such pollutants to the waters of the State. In performing such an evaluation, the permittee shall consider such factors as the probability of equipment failure or improper operation, cross-contamination of storm water by process materials, settlement of facility air emissions, the effects of natural phenomena such as freezing temperatures and precipitation, fires, and the facility's history of spills and leaks. For hazardous pollutants, the list of reportable quantities as defined in 40 CFR, Part 117 may be used as a guide in determining significant amounts of potential releases.

The review shall address all substances present at the facility that are listed as toxic pollutants under Section 307(a)(1) of the Clean Water Act or as hazardous pollutants under Section 311 of the Act or that are identified as Chemicals of Concern by the Industrial Chemical Survey.

- 4. Whenever the potential for a significant release of toxic or hazardous pollutants to State waters is determined to be present, the permittee shall identify Best Management Practices that have been established to minimize such potential releases. Where BMPs are inadequate or absent, appropriate BMPs shall be established. In selecting appropriate BMPs, the permittee shall consider typical industry practices such as spill reporting procedures, risk identification and assessment, employee training, inspections and records, preventive maintenance, good housekeeping, materials compatibility and security. In addition, the permittee may consider structural measures (such as secondary containment and erosion/sediment control devices and practices) where appropriate.
- 5. Development of the BMP plan shall include sampling of waste stream segments for the purpose of toxic "hot spot". identification. The economic achievability of effluent limits will not be considered until plant site "hot spot" sources have been identified, contained, removed or minimized through the imposition of site specific BMPs or application of internal facility treatment technology. For the purposes of this permit condition a "hot spot" is a segment of an industrial facility; including but not limited to soil, equipment, material storage areas, sewer lines etc.; which contributes elevated levels of problem pollutants to the wastewater and/or storm water collection system of that facility. For the purposes of this definition, problem pollutants are substances for which treatment to meet a water quality or technology requirement may, considering the results of waste stream segment sampling, be deemed unreasonable. For the purposes of this definition, an elevated level is a concentration or mass loading of the pollutant in question which is sufficiently higher than the concentration of that same pollutant at the compliance monitoring location so as to allow for an economically justifiable removal and/or isolation of the segment and/or B.A.T. treatment of wastewaters emanating from the segment.
 - The BMP plan shall be documented in narrative form and shall include any necessary plot plans, drawings or maps. Other documents already prepared for the facility such as a Safety Manual or a Spill Prevention, Control and Countermeasure (SPCC) plan may be used as part of the plan and may be incorporated by reference. USEPA guidance for development of storm water elements of the BMP is available in the September 1992 manual "Storm Water Management for Industrial Activities," USEPA

BMPinit2.00

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SPECIAL CONDITIONS - BEST MANAGEMENT PRACTICES

Office of Water Publication EPA 832-R-92-006 (available from NTIS, (703)487-4650, order number PB 92235969). A copy of the BMP plan shall be maintained at the facility and shall be available to authorized Department representatives upon request. As a minimum, the plan shall include the following BMP's:

a. BMP Committee	e. Inspections and Records	i. Security
b. Reporting of BMP Incidents	f. Preventive Maintenance	j. Spill prevention & response
c. Risk Identification & Assessment	g. Good Housekeeping	k. Erosion & sediment control
d. Employee Training	h. Materials Compatibility	1. Management of runoff

7. The BMP plan shall be reviewed annually and shall be modified whenever: (a) changes at the facility materially increase the potential for significant releases of toxic or hazardous pollutants, (b) actual releases indicate the plan is inadequate or (c) a letter from the Regional Water Engineer highlights inadequates in the plan.

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WTCpg2.99

SPDES PERMIT NUMBER NY0100137

Part I, Page 10 of 12

WATER TREATMENT CHEMICAL (WTC) REQUIREMENTS

New or increased use of a WTC requires Department review and authorization before it can be used and discharged. At a minimum, the permittee must notify the Department in writing of its intent to change WTC use. The Department will review that submittal and determine if a formal SPDES permit modification is necessary or whether WTC review and authorization may proceed outside of the formal permit administrative process. The majority of WTC authorizations do not require formal SPDES permit modification. WTCs which are used in closed systems and cannot contact wastewater effluents or WTCs which are discharged to municipal STP are not subject to SPDES permit review. WTCs include, but are not limited to, conditioners, corrosion or scale inhibitors, flocculants, biocides, fungicides, molluscicides, and sequestrants. Questions concerning the use in discharge of a new WTC or increased levels of an authorized WTC should be directed to the Department staff person who developed your SPDES permit. If you are not sure who that is, contact the Department staff person who normally inspects your facility.

Generic WTC Usage Requirements

WTC usage shall not exceed the usage rate reported by the permittee or authorized below, whichever is less.

• The permittee shall maintain a logbook of all WTC use, noting for each chemical the time, amount and location of each dosage. Additional guidance concerning necessary logbook content and other applicable requirements can be found in the general conditions (Part II) of the SPDES permit. The logbook must also document that adequate process controls are in place to ensure that excessive levels of WTCs are not used and subsequently discharged.

• The permittee shall provide an annual report, attached to the December DMR, containing the following information for each outfall: the current list of WTCs authorized for use and discharge by the Department, for each WTC the amount in pounds used during the year, and any other pertinent information.

The discharge shall not cause or contribute to a violation of water quality or an exceedance of AWQC.

Generic Prohibitions

WTCs which contain measurable levels of phosphorus are not permitted for discharge within the Great Lakes Basin or tributary to ponded waters outside the Basin unless the permittee can clearly demonstrate that no acceptable alternative exists. WTCs containing microorganisms cannot be approved unless a formal SPDES permit modification application is submitted.

List of WTCs Authorized for Use and Discharge							
WTC Manufacturer. Name & Function : Ashland Chemical Company, Drew Division, Amerfloc, 482, Flocculant							
Affected Outfall(s): 001	Avg/Max Daily Dosage : 19 / lbs/day						

- Authorized WTCs must either be identified above or in a letter sent to the permittee by the Department. In cases where a WTC is listed above and in a letter from the Department, the more recent document will control.

Received Time A

Apr. 12. 10:38AM

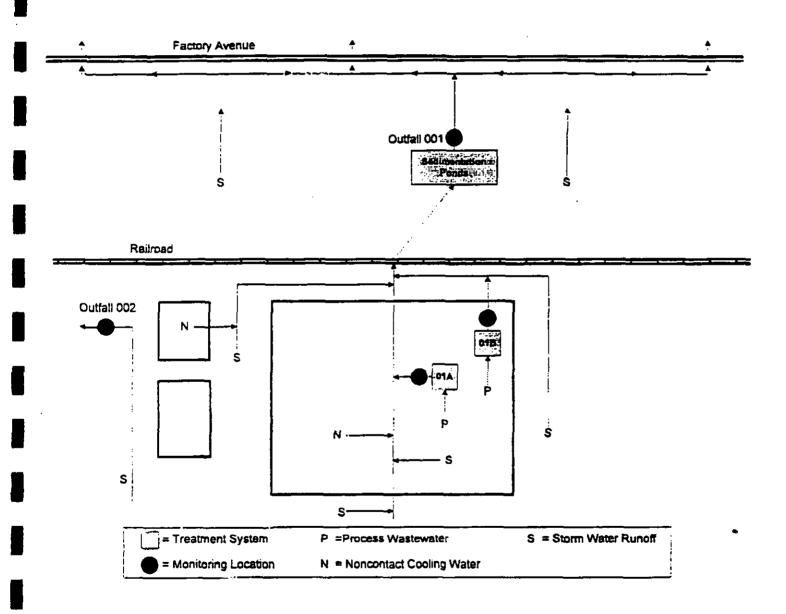
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MONITORING LOCATIONS

The permittee shall take samples and measurements, to comply with the monitoring requirements specified in this permit, at the location(s) specified below:

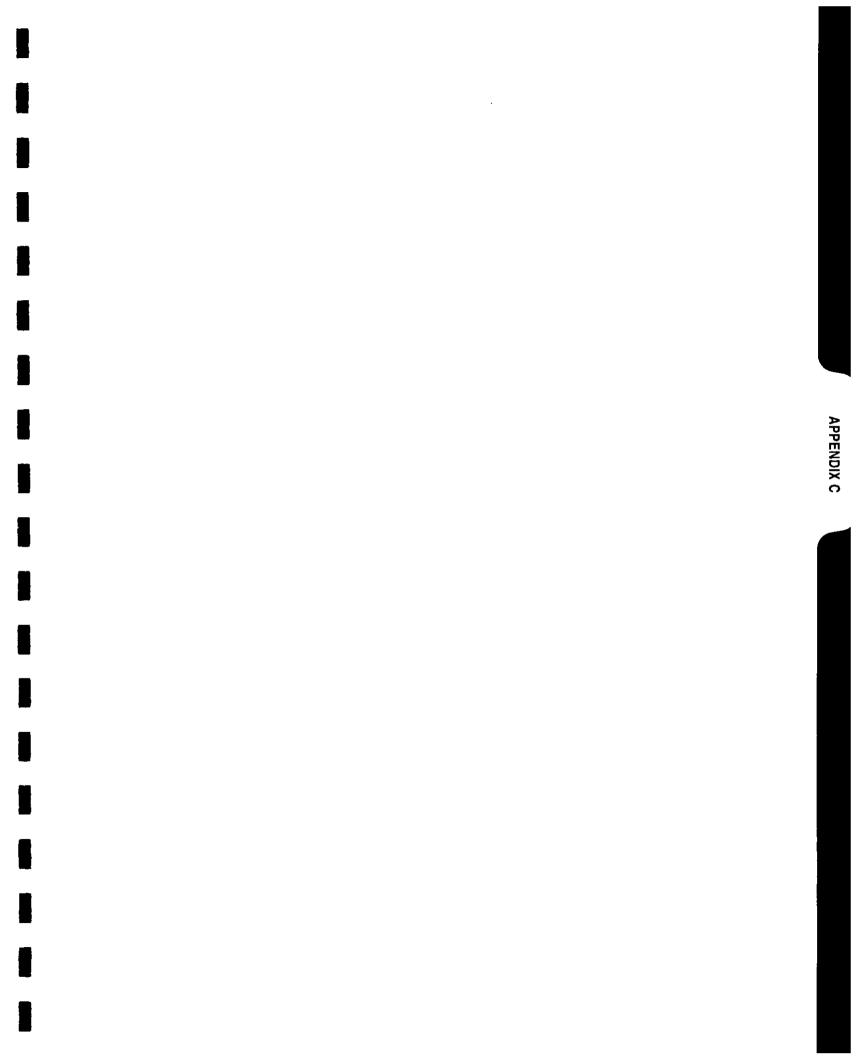


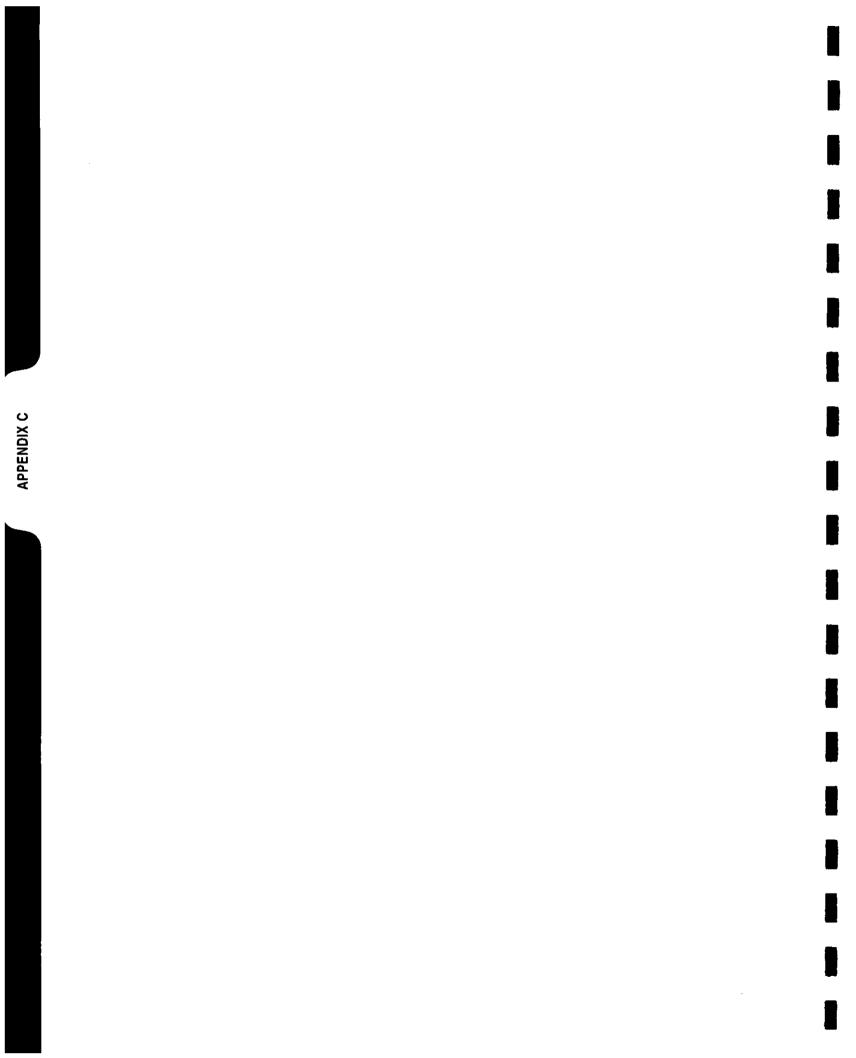
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91-	- 20-21 (5/94)				SPDES No.: NY	01001	.37			
					Part 1, Page	12 of	12			
RE	CORDING,	REPORTI	NG AND ADDITIONAL	MONITORING REQUIREMENTS						
a)	The permittee shall also refer to the General Conditions (Part II) of this permit for additional information concerning monitoring and reporting requirements and conditions.									
b)	The monitoring information required by this permit shall be summarized, signed and retained for a period of three years from the date of the sampling for subsequent inspection by the Department or its designated agent. Also;									
	[X] (If box is checked) monitoring information required by this permit shall be summarized and reported by submitting completed and signed Discharge Monitoring Report (DMR) forms for each <u>1</u> month reporting period to the locations specified below. Blank forms are available at the Department's Albany office listed below. The first reporting period begins on the effective date of this permit and the reports will be due no later than the 28th day of the month following the end of each reporting period.									
	Send the e	original (to	p sheet) of each DMR p	age to:						
	Divi Bur 50 \	ision of Wat reau of Wat Nolf Road	Environmental Conserv ter ershed Compliance Pro ork 12233-3506							
	Pho	one: (518) 4	457-3790							
	Send the f	Ігві сору (second sheet) of each [OMR page to:						
	Reg Reg 615	jional Wate gion 7 5 Erie	Environmental Conserv r Engineer Boulevard West New York 1320							
c)				port" (form 92-15-7) shall be submi ounty Health Department or Environm			dove.			
d)		liance with onditions (F		ermit shall be reported to the Departr	ment as prescribed	in the attaci	hed			
e)			onducted according to n specified in this permi	test procedures approved under 40 (L	CFR Part 136, unles	s other test				
f)	under 40 C	CFR Part 13		frequently than required by this perm permit, the results of this monitoring Monitoring Reports.			ved			
g)			nitations which require a this permit.	iveraging of measurements shall utiliz	ze an arithmetic me	an unless				
h)	Unless oth measurem	erwise spe ents and sa	cified, all Information re- ampling carried out duri	corded on the Discharge Monitoring ing the most recently completed repo	Report shall be bas stling period.	ed upon				
i)	certificates laboratory to the Envi	of approvi which has ironmental	al pursuant to section fix been issued a certificate Laboratory Accreditatio	ed by this permit for which the State (ve hundred two of the Public Health L e of approval. Inquiries regarding lat n Program, New York State Health D ences, The Nelson A. Rockefeller Stat	aw shall be conduct coratory certification spartment Center for	cted by a In should be or Laborato	sent ries			





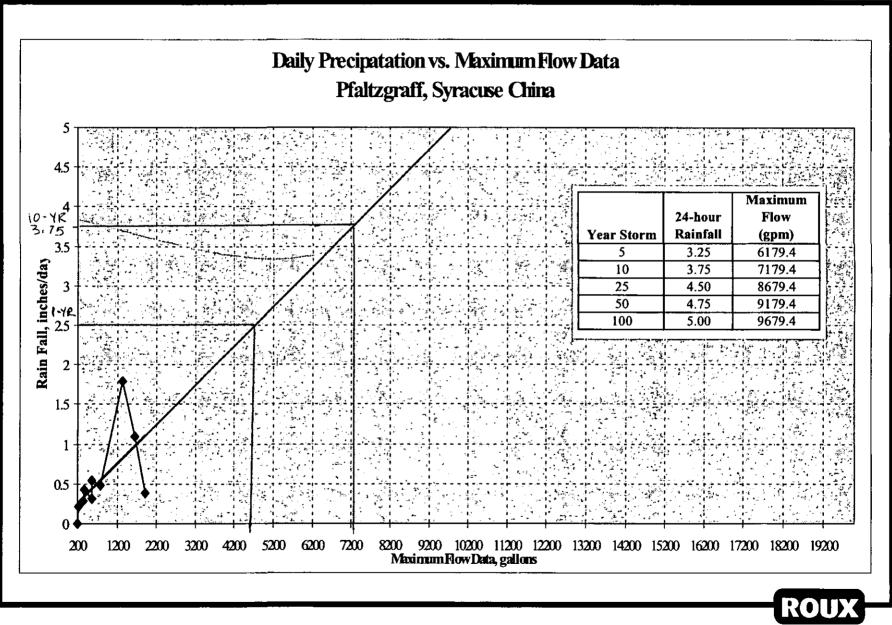
APPENDIX C

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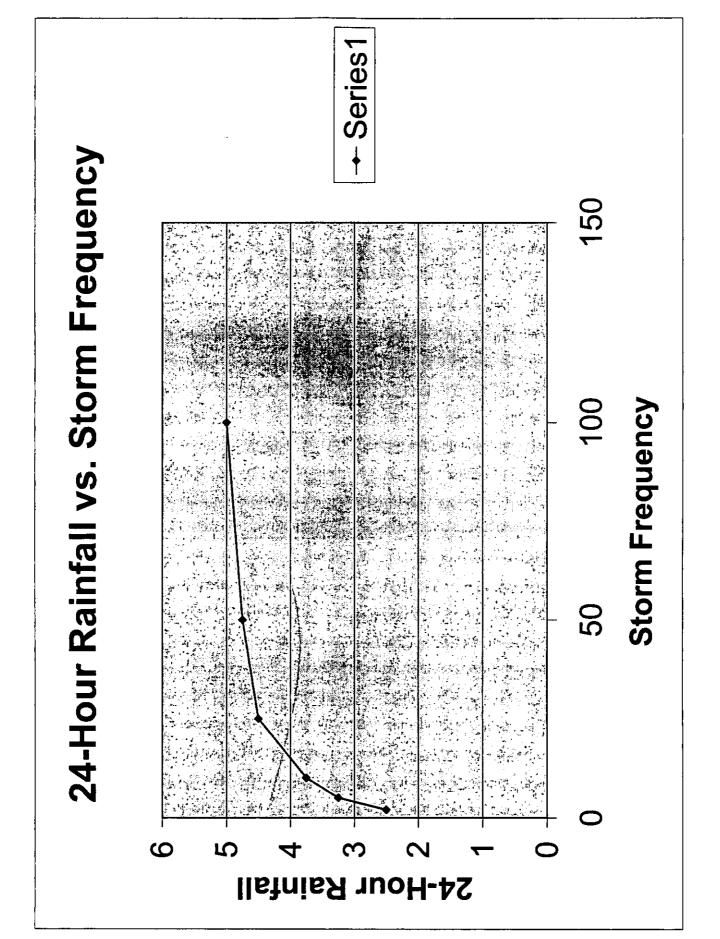
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RAINFALL AND STORM WATER FLOW INFORMATION



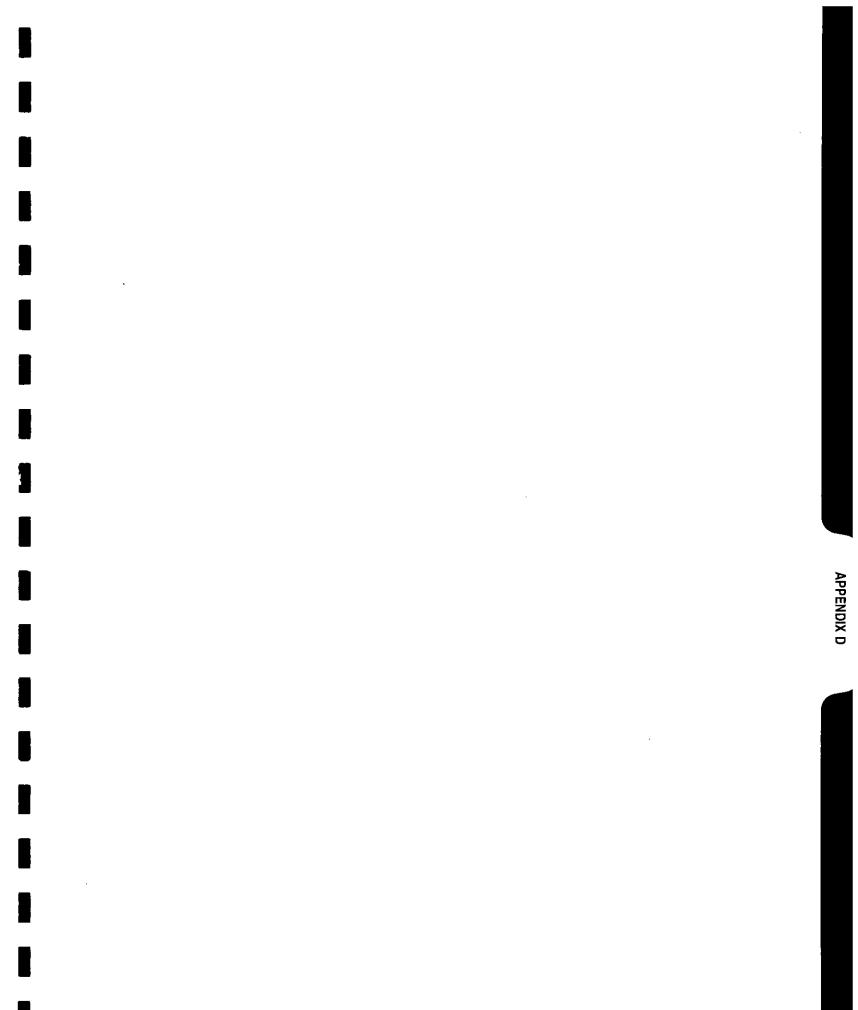
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REMEDIAL ENGINEERING, P.C.





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APPENDIX D

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FLOW DATA COLLECTED BY REMEDIAL ENGINEERING BETWEEN MAY 1998 AND AUGUST 1998

		American Sigma	InSight	3 040
Date	Time	Flow 1	THOTAHC	5.040
-		(gpm)		
15/MAY/98	05:00p.m	n. 408.441		
15/MAY/98	06:00p.t			
15/MAY/98	07:00p.t			
15/MAY/98	08:00p.t			
15/MAY/98	09:00p.r			
15/MAY/98	10:00p.r	n. 424.686		
15/MAY/98	11:00p.r			
16/MAY/98	12:00a.r			
16/MAY/98	01:00a.m			
16/MAY/98 16/MAY/98	02:00a.m 03:00a.m			
16/MAY/98	04:00a.m			
16/MAY/98	05:00a.m			
16/MAY/98	06:00a.m			
_16/MAY/98	07:00a.m			
16/MAY/98	08:00a.m	n. 256.789		
L 6/MAY/98	09:00a.m			
16/MAY/98	10:00a.m			
6/MAY/98	11:00a.m			
L6/MAY/98 16/MAY/98	12:00p.m			
_16/MAY/98	01:00p.m 02:00p.m			
.6/MAY/98	02:00p.π 03:00p.π			
6/MAY/98	04:00p.m			
16/MAY/98	05:00p.π			
6/MAY/98	06:00p.π	n. 313.159		
6/MAY/98	07:00p.m			
16/MAY/98	08:00p.m			
16/MAY/98 6/MAY/98	09:00p.m			
16/MAY/98	10:00p.m 11:00p.m			
<u>1</u> 7/MAY/98	12:00a.m			
7/MAY/98	01:00a.m			
7/MAY/98	02:00a.m			
17/MAY/98	03:00a.m	. 317.430		
7/MAY/98	04:00a.m			
7/MAY/98	05:00a.m			
17/MAY/98 17/MAY/98	06:00a.m			
7/MAY/98	07:00a.m 08:00a.m			
17/MAY/98	09:00a.m			
17/MAY/98	10:00a.m			
7/MAY/98	11:00a.m			
7/MAY/98	12:00p.m	. 293.568		
17/MAY/98	01:00p.m			
7/MAY/98	02:00p.m			
7/MAY/98	03:00p.m			
T7/MAY/98 17/MAY/98	04:00p.m			
7/MAY/98	05:00p.m 06:00p.m			
7/MAY/98	07:00p.m	. 256.985		
17/MAY/98	08:00p.m			
MAY/98	09:00p.m			
7/MAY/98	10:00p.m	. 254.952		
17/MAY/98	11:00p.m	. 256.748		
📲/MAY/98	12:00a.m	. 278.082		

Flow Daton Syrateuse Channe Friteiling 5/15/98 - 5/35/95

20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 20/MAY/98 21/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98 22/MAY/98	01:00p.m. 02:00p.m. 03:00p.m. 05:00p.m. 05:00p.m. 06:00p.m. 07:00p.m. 09:00p.m. 10:00p.m. 11:00p.m. 12:00a.m. 01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 10:00a.m. 10:00a.m. 10:00a.m. 11:00p.m. 02:00p.m. 02:00p.m. 01:00p.m. 02:00p.m. 03:00p.m. 04:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00p.m. 10:00a.m. 01:00a.m. 02:00a.m. 01:00a.m. 02:00a.m. 01:00a.m.	382.557 319.289 332.964 409.524 411.445 444.890 432.497 436.081 417.672 409.383 403.997 417.492 428.251 425.100 365.179 357.295 344.378 367.861 388.971 418.392 397.622 349.857 401.390 351.177 369.907 0.0000 0.0000 0.000 0.000 0.0000 0.0000 0.0000 0.0000
22/MAY/98	03:00a.m.	0.000
22/MAY/98	04:00a.m.	0.000
22/MAY/98	05:00a.m.	0.000
22/MAY/98	06:00a.m.	0.000
22/MAY/98	07:00a.m.	0.000
22/MAY/98	08:00a.m.	0.000
22/MAY/98	09:00a.m.	356.414
22/MAY/98	10:00a.m.	284.232
22/MAY/98	11:00a.m.	345.576

l l l min a santa

25/MAY/98	01:00p.m.	212.974
_25/MAY/98	02:00p.m.	219.972
25/MAY/98	03:00p.m.	220.839
25/MAY/98	04:00p.m.	227.195
25/MAY/98	05:00p.m.	226.774
25/MAY/98	06:00p.m.	223.742
25/MAY/98	07:00p.m.	227.275
25/MAY/98	08:00p.m.	223.537
_25/MAY/98	09:00p.m.	215.547
25/MAY/98	10:00p.m.	213.626
25/MAY/98	11:00p.m.	219.786
26/MAY/98	12:00p.m.	225.223
26/MAY/98	01:00a.m.	356.736
26/MAY/98	02:00a.m.	372.046
26/MAY/98	03:00a.m.	275.278
26/MAY/98	04:00a.m.	237.311
26/MAY/98	05:00a.m.	215.768
26/MAY/98	06:00a.m.	227.779
_26/MAY/98	07:00a.m.	243.147
26/MAY/98	08:00a.m.	390.798
🗣 6/MAY/98	09:00a.m.	397.608
26/MAY/98	10:00a.m.	403.769
E 6/MAY/98	11:00a.m.	380.270
26/MAY/98	12:00p.m.	366.380
26/MAY/98	01:00p.m.	384.127
26/MAY/98	02:00p.m.	344.423
6/MAY/98	03:00p.m.	391.437
26/MAY/98	04:00p.m.	380.643
26/MAY/98	05:00p.m.	351.620
6/MAY/98	06:00p.m.	382.233
6/MAY/98	07:00p.m.	414.464
26/MAY/98 ∎6/MAY/98	08:00p.m.	370.401
	09:00p.m.	405.070
6/MAY/98	10:00p.m.	372.495
26/MAY/98	11:00p.m.	376.796
27/MAY/98	12:00a.m.	333.212
7/MAY/98	01:00a.m.	285.228
7/MAY/98	02:00a.m.	382.085
27/MAY/98	03:00a.m.	322.360
7/MAY/98	04:00a.m.	326.705
7/MAY/98	05:00a.m.	308.274
27/MAY/98	06:00a.m.	272.876
27/MAY/98	07:00a.m.	346.327
7/MAY/98	08:00a.m.	420.627
27/MAY/98	09:00a.m.	430.178
27/MAY/98	10:00a.m.	412.418
7/MAY/98	10:00a.m. 11:00a.m.	
		417.624
7/MAY/98	12:00p.m.	347.699
27/MAY/98	01:00p.m.	352.073
7/MAY/98	02:00p.m.	337.589
7/MAY/98	03:00p.m.	373.905
27/MAY/98	04:00p.m.	417.210
27/MAY/98	05:00p.m.	396.804
7/MAY/98	06:00p.m.	432.214
27/MAY/98	07:00p.m.	459.021
27/MAY/98	08:00p.m.	369.695
7/MAY/98	09:00p.m.	438.396
7/MAY/98	10:00p.m.	442.067
27/MAY/98	11:00p.m.	438.647
28/MAY/98	12:00a.m.	406.001
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28/MAY/98	01:00a.m.	318.535
28/MAY/98	02:00a.m.	330.218
28/MAY/98 28/MAY/98 28/MAY/98	03:00a.m. 04:00a.m.	305.358 297.110
28/MAY/98	05:00a.m.	270.984
28/MAY/98	06:00a.m.	257.200
28/MAY/98	07:00a.m.	323.368
28/MAY/98	08:00a.m.	419.429
28/MAY/98	09:00a.m.	370.591
28/MAY/98	10:00a.m.	372.887
28/MAY/98	11:00a.m.	372.855
28/MAY/98	12:00p.m.	386.485
28/MAY/98	01:00p.m.	369.110
28/MAY/98	02:00p.m.	334.654
28/MAY/98	03:00p.m.	358.936
28/MAY/98	04:00p.m.	381.491
28/MAY/98	05:00p.m.	368.381
28/MAY/98	06:00p.m.	461.860
28/MAY/98	07:00p.m.	459.400
28/MAY/98	08:00p.m.	442.819
28/MAY/98	09:00p.m.	442.928
28/MAY/98	10:00p.m.	477.253
28/MAY/98	11:00p.m.	418.806
29/MAY/98	12:00a.m.	389.459
29/MAY/98	01:00a.m.	331.060
29/MAY/98	02:00a.m.	322.591
29/MAY/98	03:00a.m.	353.428
29/MAY/98	04:00a.m.	308.839
29/MAY/98	05:00a.m.	355.098
29/MAY/98	06:00a.m.	297.165
29/MAY/98	07:00a.m.	317.979
29/MAY/98	08:00a.m.	430.807
29/MAY/98	09:00a.m.	422.281
29/MAY/98	10:00a.m.	334.988
29/MAY/98	11:00a.m.	454.624
29/MAY/98	12:00p.m.	425.203
29/MAY/98	01:00p.m.	405.295
29/MAY/98	02:00p.m.	573.310
29/MAY/98 29/MAY/98	03:00p.m. 04:00p.m.	586.530
29/MAY/98	05:00p.m.	371.358 369.566
29/MAY/98	06:00p.m.	418.581
29/MAY/98	07:00p.m.	415.151
29/MAY/98	08:00p.m.	380.768
29/MAY/98	09:00p.m.	394.331
29/MAY/98	10:00p.m.	423.199
29/MAY/98	11:00p.m.	391.090
30/MAY/98	12:00a.m.	347.545
30/MAY/98	01:00a.m.	234.142
30/MAY/98	02:00a.m.	233.130
30/MAY/98	03:00a.m.	288.828
30/MAY/98	04:00a.m.	312.767
30/MAY/98	05:00a.m.	302.352
30/MAY/98	06:00a.m.	258.928
30/MAY/98	07:00a.m.	268.772
30/MAY/98	08:00a.m.	203.034
30/MAY/98	09:00a.m.	206.461
30/MAY/98	10:00a.m.	219.988
30/MAY/98	11:00a.m.	218.260
30/MAY/98	12:00p.m.	220.820

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30/MAY/98	01:00p.m.	221.421	
30/MAY/98	02:00p.m.	270.239	
30/MAY/98	03:00p.m.	273.451	
3 0/MAY/98	04:00p.m.	253.844	
30/MAY/98	05:00p.m.	211.953	
30/MAY/98	06:00p.m.	200.510	
B0/MAY/98	07:00p.m.	207.656	
30/MAY/98	08:00p.m.	205.360	
30/MAY/98	09:00p.m.	197083	
30/MAY/98	10:00p.m.	197.193	
30/MAY/98	11:00p.m.	195.516	
31/MAY/98	12:00a.m.	197.000	
B1/MAY/98	01:00a.m.	198.798	
B1/MAY/98	02:00a.m.	196.537	
31/MAY/98	03:00a.m.	215.450	
B1/MAY/98	04:00a.m.	226.626	
B1/MAY/98	05:00a.m.	233.814 236.537	
31/MAY/98	06:00a.m.	224.911	
.31/MAY/98	07:00a.m. 08:00a.m.	204.640	
B1/MAY/98 B1/MAY/98	09:00a.m.	194.935	
31/MAY/98	10:00a.m.	201.599	
B1/MAY/98	11:00a.m.	215.181	
B1/MAY/98	12:00p.m.	209.271	
31/MAY/98	01:00p.m.	219.892	
_31/MAY/98	02:00p.m.	227.423	
81/MAY/98	03:00p.m.	224.626	
31/MAY/98	04:00p.m.	224.253	
_31/MAY/98	05:00p.m.	500.944	
1/MAY/98	06:00p.m.	887.915	
B1/MAY/98	07:00p.m.	608.306	
31/MAY/98	08:00p.m.	562.118	
# 1/MAY/98	09:00p.m.	411.278	
1/MAY/98	10:00p.m.	350.843	
31/MAY/98	11:00p.m.	296.169	
_01/JUN/98	12:00a.m.	289.965	
1/JUN/98	01:00a.m.	415.980	
🕶 1/JUN/98	02:00a.m.	412.518	
01/JUN/98	03:00a.m.	373.356	
1/JUN/98	04:00a.m.	317.269	
1/JUN/98	05:00a.m.	312.507	
01/JUN/98	06:00a.m.	303.534	
1/JUN/98	07:00a.m.	305.416	
1/JUN/98 01/JUN/98	08:00a.m.	470.872 426.950	
_01/JUN/98	09:00a.m. 10:00a.m.	357.269	
1/JUN/98	11:00a.m.	398.891	
1/JUN/98	12:00p.m.	367.671	
01/JUN/98	01:00p.m.	394.571	
1/JUN/98	02:00p.m.	364.576	
1/JUN/98	03:00p.m.	401.441	
01/JUN/98	04:00p.m.	394.507	
$\Delta 1/JUN/98$	05:00p.m.	423.109	
1/JUN/98	06:00p.m.	386.896	
01/JUN/98	07:00p.m.	454.290	
01/JUN/98	08:00p.m.	429.401	
1/JUN/98	09:00p.m.	410.465	
1/JUN/98	10:00p.m.	441.377	
01/JUN/98	11:00p.m.	399.206	• • • •
1 2/JUN/98	12:00a.m.	346.266	

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02/JUN/98 02/JUN/98	01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 09:00a.m. 10:00a.m. 11:00a.m. 12:00p.m. 01:00p.m. 02:00p.m. 03:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 06:00p.m. 08:00p.m.	294.002 315.159 310.470 301.507 314.254 293.626 307.654 470.056 385.040 411.699 437.873 382.628 414.534 350.785 400.192 390.332 384.134 336.381 429.834 373.382 430.367 408.201
02/JUN/98 02/JUN/98 03/JUN/98	10:00p.m. 11:00p.m. 12:00a.m. 01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 09:00a.m. 10:00a.m. 11:00a.m. 12:00p.m. 01:00p.m. 02:00p.m. 03:00p.m. 03:00p.m. 05:00p.m. 05:00p.m. 05:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 06:00p.m. 07:00p.m. 06:00p.m. 09:00p.m. 09:00p.m. 10:00p.m.	408.301 412.302 402.530 319.126 297.647 275.468 279.196 257.149 266.164 342.400 443.577 387.866 383.999 379.172 366.997 375.925 388.280 368.237 355.262 379.625 418.886 395.426 399.386 383.071 412.376
03/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98 04/JUN/98	11:00p.m. 12:00a.m. 01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 08:00a.m. 09:00a.m. 10:00a.m. 11:00a.m. 12:00p.m.	420.190 332.672 234.196 226.932 363.143 299.224 303.701 332.338 263.190 454.287 384.898 358.262 401.611 398.008

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04/JUN/98	01:00p.m.	405.321
_04/JUN/98	02:00p.m.	349.253
04/JUN/98	03:00p.m.	347.965
04/JUN/98	04:00p.m.	363.895
04/JUN/98	05:00p.m.	402.311
04/JUN/98	06:00p.m.	423.366
04/JUN/98	07:00p.m.	422.782
04/JUN/98	08:00p.m.	414.509
04/JUN/98	09:00p.m.	419.323
04/JUN/98	10:00p.m.	446.881
04/JUN/98	11:00p.m.	415.928
_05/JUN/98	12:00a.m.	326.448
05/JUN/98	01:00a.m.	255.363
05/JUN/98	02:00a.m.	227.368
05/JUN/98	03:00a.m.	389.590
D5/JUN/98	04:00a.m.	265.978
05/JUN/98	05:00a.m.	228.203
05/JUN/98	06:00a.m.	340.624
_05/JUN/98	07:00a.m.	344.401
05/JUN/98	08:00a.m.	452.717
05/JUN/98	09:00a.m.	413.989
05/JUN/98	10:00a.m.	388.322
5/JUN/98	11:00a.m.	435.015
05/JUN/98	12:00p.m.	370.048
05/JUN/98	01:00p.m.	427.230
_05/JUN/98	02:00p.m.	365.006
05/JUN/98	03:00p.m.	322.055
05/JUN/98	04:00p.m.	394.205
_05/JUN/98	05:00p.m.	352.523
5/JUN/98	06:00p.m.	310.782
5/JUN/98	07:00p.m.	394.475
05/JUN/98	08:00p.m.	328.532
■ 5/JUN/98	09:00p.m.	382.730
5/JUN/98	10:00p.m.	403.172
05/JUN/98	11:00p.m.	345.598
_06/JUN/98	12:00a.m.	283.410
6/JUN/98	01:00a.m.	216.115
5 6/JUN/98	02:00a.m.	214.101
06/JUN/98	03:00a.m.	225.714
6/JUN/98	04:00a.m.	223.405
6/JUN/98	05:00a.m.	214.384
06/JUN/98	06:00a.m.	212.345
📫 6/JUN/98	07:00a.m.	204.656
6/JUN/98	08:00a.m.	200.770
0 6/JUN/98	09:00a.m.	189.873
<u>0</u> 6/JUN/98	10:00a.m.	179.831
6/JUN/98	11:00a.m.	181.283
9 6/JUN/98	12:00p.m.	180.309
06/JUN/98	01:00p.m.	182.179
6/JUN/98	02:00p.m.	187.022
6/JUN/98	03:00p.m.	191.797
06/JUN/98	04:00p.m.	193.926
6/JUN/98	05:00p.m.	195.558
6/JUN/98	06:00p.m.	192.671
U6/JUN/98	07:00p.m.	175.755
06/JUN/98	08:00p.m.	172.595
6/JUN/98	09:00p.m.	175.354
6/JUN/98	10:00p.m.	170.097
06/JUN/98	11:00p.m.	173.816
1 7/JUN/98	12:00a.m.	169.727

07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98	01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 09:00a.m. 10:00a.m. 11:00a.m. 12:00p.m. 01:00p.m. 02:00p.m. 03:00p.m.	168.382 190.490 196.823 207.518 208.333 208.295 200.716 173.800 167.633 173.250 170.771 166.053 184.847 182.872 189.951
07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 07/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98	04:00p.m. 05:00p.m. 06:00p.m. 07:00p.m. 08:00p.m. 10:00p.m. 11:00p.m. 12:00a.m. 01:00a.m. 02:00a.m. 03:00a.m. 04:00a.m.	192.963 194.864 177.275 167.621 172.046 167.463 164.252 167.566 181.331 320.253 318.451 223.174 209.358
08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98	05:00a.m. 06:00a.m. 07:00a.m. 08:00a.m. 10:00a.m. 11:00a.m. 12:00p.m. 01:00p.m. 02:00p.m. 03:00p.m.	357.398 247.601 242.787 420.559 339.625 323.397 365.420 388.325 371.195 368.985 354.658
08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 08/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98	04:00p.m. 05:00p.m. 06:00p.m. 07:00p.m. 08:00p.m. 09:00p.m. 10:00p.m. 11:00p.m. 12:00a.m. 01:00a.m. 02:00a.m. 03:00a.m.	383.854 390.547 346.636 386.581 324.393 363.336 407.151 399.212 398.525 270.779 266.803 238.278
09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98 09/JUN/98	03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 08:00a.m. 09:00a.m. 10:00a.m. 11:00a.m. 12:00p.m.	238.278 232.844 209.644 192.738 395.101 395.500 387.516 370.873 369.197 395.133

09/JUN/98	01:00p.m.	401.624
_09/JUN/98	02:00p.m.	369.441
09/JUN/98	03:00p.m.	339.201
09/JUN/98	04:00p.m.	339.988
09/JUN/98	05:00p.m.	342.946
09/JUN/98	06:00p.m.	373.276
0 9/JUN/98	07:00p.m.	310.971
89/ MU T/98	08:00p.m.	395.564
⊒ 09/JUN/98	09:00p.m.	359.379
09/JUN/98	10:00p.m.	359.427
= 09/JUN/98	ll:00p.m.	367.736
_10/JUN/98	12:00a.m.	239.534
10/JUN/98	01:00a.m.	234.887
■10/JUN/98	02:00a.m.	219.172
10/JUN/98	03:00a.m.	229.054
10/JUN/98	04:00a.m.	227.038
10/JUN/98	05:00a.m.	228.274
10/JUN/98	06:00a.m.	206.175
_10/JUN/98	07:00a.m.	223.062
10/JUN/98	08:00a.m.	406.624
10/JUN/98	09:00a.m.	343.260
	10:00a.m.	
10/JUN/98		345.669
10/JUN/98	11:00a.m.	368.250
10/JUN/98	12:00p.m.	344.500
10/JUN/98	01:00p.m.	349.928
10/JUN/98	02:00p.m.	304.397
10/JUN/98	03:00p.m.	405.266
1 0/JUN/98	04:00p.m.	376.651
_10/JUN/98	05:00p.m.	397.760
10/JUN/98	06:00p.m.	407.598
💶 0/JUN/98	07:00p.m.	433.836
10/JUN/98	08:00p.m.	417.958
L0/JUN/98	09:00p.m.	410.266
10/JUN/98	10:00p.m.	404.187
_10/JUN/98	11:00p.m.	400.031
11/JUN/98 11/JUN/98	12:00a.m.	281.859
1/JUN/98	01:00a.m.	238.063
¶1/JUN/98	02:00a.m.	223.691
11/JUN/98	03:00a.m.	232.844
1/JUN/98	04:00a.m.	211.722
1/JUN/98	05:00a.m.	204.113
11/JUN/98	06:00a.m.	228.544
🛋 1/JUN/98	07:00a.m.	354.186
1/JUN/98	08:00a.m.	428.803
11/JUN/98	09:00a.m.	408.911
_11/JUN/98	10:00a.m.	341.896
1/JUN/98	11:00a.m.	382.968
1/JUN/98	12:00p.m.	384.291
11/JUN/98	01:00p.m.	410.199
1/JUN/98	02:00p.m.	327.809
1/JUN/98	03:00p.m.	381.481
11/JUN/98	04:00p.m.	389.879
1/JUN/98	05:00p.m.	399.032
1/JUN/98	06:00p.m.	412.598
1/JUN/98		
	07:00p.m.	421.414
11/JUN/98	08:00p.m.	432.815
1/JUN/98	09:00p.m.	430.843
■1/JUN/98	10:00p.m.	426.096
11/JUN/98	11:00p.m.	411.175
2/JUN/98	12:00a.m.	358.743

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12/JUN/98	01:00a.m.	271.935	
12/JUN/98	02:00a.m.	334.085	
		298.652	
12/JUN/98	03:00a.m.		
12/JUN/98	04:00a.m.	270.310	
12/JUN/98	05:00a.m.	222.580	
12/JUN/98	06:00a.m.	270.978	
12/JUN/98	07:00a.m.	511.565	
12/JUN/98	08:00a.m.	415.694	
12/JUN/98	09:00a.m.	525.847	
12/JUN/98	10:00a.m.	489.325	
12/JUN/98	11:00a.m.	571.358	
12/JUN/98	12:00p.m.	455.649	
12/JUN/98	01:00p.m.	411.602	
12/JUN/98	02:00p.m.	340.935	
12/JUN/98	03:00p.m.	340.001	
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12/JUN/98	04:00p.m.	348.736	
12/JUN/98	05:00p.m.	379.028	
12/JUN/98	06:00p.m.	397.690	
12/JUN/98	07:00p.m.	412.004	
• •	-	325.690	
12/JUN/98	08:00p.m.		
12/JUN/98	09:00p.m.	398.104	
12/JUN/98	10:00p.m.	391.517	
12/JUN/98	11:00p.m.	389.468	
13/JUN/98	12:00a.m.	351.203	
13/JUN/98	01:00a.m.	217.583	
13/JUN/98	02:00a.m.	219.230	
13/JUN/98	03:00a.m.	232.908	
13/JUN/98	04:00a.m.	223.556	
13/JUN/98	05:00a.m.	233.307	
13/JUN/98	06:00a.m.	234.116	
13/JUN/98	07:00a.m.	228.859	
13/JUN/98	08:00a.m.	249.213	
13/JUN/98	09:00a.m.		
		297.878	
13/JUN/98	10:00a.m.	283.416	
13/JUN/98	11:00a.m.	285.292	
13/JUN/98	12:00p.m.	265.435	
13/JUN/98	01:00p.m.	255.852	
13/JUN/98	02:00p.m.	217.034	
13/JUN/98	03:00p.m.	192.179	
13/JUN/98	04:00p.m.	183.360	
13/JUN/98	05:00p.m.	172.839	
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· ·	06:00p.m.	179.002	
13/JUN/98	07:00p.m.	189.074	
13/JUN/98	08:00p.m.	170.421	
13/ JUN/98	09:00p.m.	172.126	
13/JUN/98	10:00p.m.	185.435	
13/JUN/98			
	11:00p.m.	185.220	
14/JUN/98	12:00a.m.	178.620	
14/JUN/98	01:00a.m.	178.684	
14/JUN/98	02:00a.m.	196.428	
14/JUN/98	03:00a.m.	197.119	
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	04:00a.m.	192.231	
14/JUN/98	05:00a.m.	191.534	
14/J UN/98	06:00a.m.	203.648	
14/JUN/98	07:00a.m.	187.246	
14/JUN/98	08:00a.m.	170.762	
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	09:00a.m.	181.067	
14/JUN/98	10:00a.m.	178.668	
14/JUN/98	11:00a.m.	179.956	
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• 14/JUN/98	01:00p.m.	185.034
$_{14}/JUN/98$	02:00p.m.	186.623
14/JUN/98	03:00p.m.	189.938
14/JUN/98	04:00p.m.	186.164
_14/JUN/98	05:00p.m.	182.114
14/JUN/98	06:00p.m.	186.055
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14/JUN/98	10:00p.m.	186.553
14/JUN/98	11:00p.m.	207.139
_15/JUN/98	12:00a.m.	219.539
15/JUN/98	01:00a.m.	380.906
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1 5/JUN/98	09:00a.m.	430.795
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11//JUL/98 11//JUL/98
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19/JUL/98	01:00p.m.	0.000	
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19/JUL/98	08:00p.m.	0.000	
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19/JUL/98	10:00p.m.	0.000	
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_20/JUL/98	12:00a.m.	0.000	
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20/JUL/98	03:00a.m.	0.000	
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20/JUL/98	01:00p.m.	0.000	
2 0/JUL/98	02:00p.m.	323.959	
20/JUL/98	03:00p.m.	341.478	
20/JUL/98	04:00p.m.	307.998	
_20/JUL/98	05:00p.m.	306.119	
20/JUL/98	06:00p.m.	316.951	
🗣0/JUL/98	07:00p.m.	306.170	
20/JUL/98	08:00p.m.	256.638	
🗰 0/JUL/98	09:00p.m.	304.372	
0/JUL/98	10:00p.m.	314.973	
	11:00p.m.	327.781	
	12:00a.m.	304.507	
1/JUL/98	01:00a.m.	206.972	
2 1/JUL/98	02:00a.m.	205.726	
21/JUL/98	03:00a.m.	202.806	
1/JUL/98	04:00a.m.	209.727	
■1/JUL/98	05:00a.m.	213.199	
21/JUL/98	06:00a.m.	258.890	
_ 1/JUL/98	07:00a.m.	272.327	
1/JUL/98	08:00a.m.	340.678	
21/JUL/98	09:00a.m.	333.089	
<u>21/JUL/98</u>	10:00a.m.	300.216	
1/JUL/98	11:00a.m.	391.350	
2 1/JUL/98	12:00p.m.	399.604	
21/JUL/98	01:00p.m.	354.594	
1/JUL/98	02:00p.m.	346.279	
1/JUL/98	03:00p.m.	350.849	
21/JUL/98	04:00p.m.	326.349	
_ 21/JUL/98	05:00p.m.	382.631	
1/JUL/98	06:00p.m.	328.038	
2 1/JUL/98	07:00p.m.	389.757	
<u>2</u> 1/JUL/98	08:00p.m.	304.590	
1/JUL/98	09:00p.m.	358.570	
1/JUL/98	10:00p.m.	360.715	
21/JUL/98	11:00p.m.	336.625	
■2/JUL/98	12:00a.m.	303.672	

22/TTT /00	01:00a.m.	233.721	
22/JUL/98			
22/JUL/98	02:00a.m.	304.748	
22/JUL/98	03:00a.m.	307.157	
22/JUL/98	04:00a.m.	388.842	
22/JUL/98	05:00a.m.	298.902	
22/JUL/98	06:00a.m.	341.064	
22/JUL/98	07:00a.m.	334.246	
22/JUL/98	08:00a.m.	435.711	
22/JUL/98	09:00a.m.	355.641	
22/JUL/98	10:00a.m.	333.719	
22/JUL/98	11:00a.m.	346.713	
22/JUL/98	12:00p.m.	329.708	
22/JUL/98	01:00p.m.	337.049	
22/JUL/98	02:00p.m.	350.734	
22/JUL/98	03:00p.m.	367.495	
22/JUL/98	04:00p.m.	339.015	
22/JUL/98	05:00p.m.	390.737	
22/JUL/98	06:00p.m.	395.268	
22/JUL/98	07:00p.m.	355.224	
22/JUL/98	08:00p.m.	377.499	
22/JUL/98	09:00p.m.	351.148	
22/JUL/98	10:00p.m.	402.215	
22/JUL/98	11:00p.m.	1912.418	
23/JUL/98	12:00a.m.	487.551	
23/JUL/98	01:00a.m.	352.905	
23/JUL/98	02:00a.m.	318.763	
23/JUL/98	03:00a.m.	288.009	
23/JUL/98	04:00a.m.	287.562	
23/JUL/98	05:00a.m.	358.223	
23/JUL/98	06:00a.m.	546.014	
23/JUL/98	07:00a.m.	613.303	
23/JUL/98	08:00a.m.	403.001	
23/JUL/98	09:00a.m.	778.165	
23/JUL/98	10:00a.m.	444.238	
23/JUL/98	11:00a.m.	354.032	
23/JUL/98	12:00p.m.	309.134	
23/JUL/98	01:00p.m.	345.868	
23/JUL/98	02:00p.m.	283.073	
23/JUL/98	03:00p.m.	0.000]
23/JUL/98	04:00p.m.	0.000	
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	05:00p.m.	0.000	1
23/JUL/98	06:00p.m.	0.000)
23/JUL/98	07:00p.m.	0.000	
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23/JUL/98	09:00p.m.	0.000	
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23/JUL/98	11:00p.m.	0.000	
24/JUL/98	12:00a.m.	0.000	
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28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98 28/JUL/98	02:00a.m. 03:00a.m. 04:00a.m. 05:00a.m. 06:00a.m. 07:00a.m. 08:00a.m. 10:00a.m. 11:00a.m.	255.328 262.978 258.475 252.646 266.720 275.253 338.347 354.154 335.848 344.234
28/JUL/98	12:00p.m.	318.551
28/JUL/98	01:00p.m.	315.863
28/JUL/98	02:00p.m.	340.438
28/JUL/98	03:00p.m.	355.933
28/JUL/98	04:00p.m.	342.037
28/JUL/98	05:00p.m.	373.523
28/JUL/98	06:00p.m.	352.060
28/JUL/98	07:00p.m.	357.555
28/JUL/98	08:00p.m.	330.938
28/JUL/98	09:00p.m.	378.838
28/JUL/98	10:00p.m.	380.036
28/JUL/98	11:00p.m.	364.042
29/JUL/98	12:00a.m.	335.113
29/JUL/98	01:00a.m.	238.972
29/JUL/98	02:00a.m.	218.415
29/JUL/98	03:00a.m.	241.551
29/JUL/98	04:00a.m.	287.245
29/JUL/98	05:00a.m.	348.537
29/JUL/98	06:00a.m.	321.159
29/JUL/98	07:00a.m.	561.019
29/JUL/98	08:00a.m.	399.194
29/JUL/98	09:00a.m.	367.903
29/JUL/98	10:00a.m.	321.637
29/JUL/98	11:00a.m.	331.792
29/JUL/98	12:00p.m.	305.037

29/JUL/98	01:00p.m.	320.436	
29/JUL/98	02:00p.m.	286.471	
29/JUL/98	03:00p.m.	314.626	
2 9/JUL/98	04:00p.m.	301.742	
29/JUL/98	05:00p.m.	312.632	
29/JUL/98	06:00p.m.	340.312	
29/JUL/98	07:00p.m.	375.643	
29/JUL/98	08:00p.m.	309.253	
29/JUL/98	09:00p.m.	370.517	
29/JUL/98	10:00p.m.	357.035	
	11:00p.m.	376.625	
_30/JUL/98	12:00a.m.	326.291	
30/JUL/98	01:00a.m.	251.741	
■30/JUL/98	02:00a.m.	236.480	
30/JUL/98	03:00a.m.	255.922	
	04:00a.m.	332.261	
B0/JUL/98	05:00a.m.	286.448	
30/JUL/98	06:00a.m.	311.434	
30/JUL/98	07:00a.m.	305.949	
B0/JUL/98	08:00a.m.	349.231	
30/JUL/98	09:00a.m. 10:00a.m.	320.237	
30/JUL/98	11:00a.m.	350.056 319.434	
B0/JUL/98	12:00p.m.	334.837	
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30/JUL/98	04:00p.m.	315.760	
_30/JUL/98	05:00p.m.	315.410	
80/JUL/98	06:00p.m.	309.825	
30/JUL/98	07:00p.m.	309.411	
30/JUL/98	08:00p.m.	288.298	
0/JUL/98	09:00p.m.	343.925	
80/JUL/98	10:00p.m.	325.889	
30/JUL/98	11:00p.m.	323.060	
_ 31/JUL/98	12:00a.m.	327.983	
1/JUL/98	01:00a.m.	262.586	
31/JUL/98	02:00a.m.	267.872	
31/JUL/98	03:00a.m.	258.042	
1/JUL/98	04:00a.m.	260.887	
■1/JUL/98	05:00a.m.	257.904	
31/JUL/98	06:00a.m.	337.952	
■1/JUL/98 ■1/JUL/98	07:00a.m. 08:00a.m.	321.297 325.488	
31/JUL/98	09:00a.m.	323.448	
	10:00a.m.	293.363	
1/JUL/98	11:00a.m.	324.203	
J1/JUL/98	12:00p.m.	283.747	
31/JUL/98	01:00p.m.	297.428	
1/JUL/98	02:00p.m.	320.503	
1/JUL/98	03:00p.m.	332.772	
31/JUL/98	04:00p.m.	282.206	
_ 21/JUL/98	05:00p.m.	322.334	
1/JUL/98	06:00p.m.	314.703	
3 1/JUL/98	07:00p.m.	302.718	
<u>3</u> 1/JUL/98	08:00p.m.	261.607	
1/JUL/98	09:00p.m.	289.178	
1/JUL/98	10:00p.m.	274.463	
31/JUL/98	11:00p.m.	265.868	
1/AUG/98	12:00a.m.	246.313	

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01/AUG/98 09:00a.m. 213.944 01/AUG/98 10:00a.m. 213.944 01/AUG/98 10:00a.m. 239.255 01/AUG/98 11:00a.m. 235.866 01/AUG/98 01:00p.m. 245.357 01/AUG/98 02:00p.m. 238.87 01/AUG/98 02:00p.m. 245.357 01/AUG/98 03:00p.m. 257.707 01/AUG/98 04:00p.m. 245.957 01/AUG/98 05:00p.m. 232.012 01/AUG/98 05:00p.m. 232.012 01/AUG/98 06:00p.m. 207.618 01/AUG/98 07:00p.m. 207.83 01/AUG/98 09:00p.m. 218.199 01/AUG/98 01:00a.m. 207.83 02/AUG/98 01:00a.m. 207.83 02/AUG/98 01:00a.m. 227.619 02/AUG/98 01:00a.m. 232.92 02/AUG/98 02:00a.m. 221.406 02/AUG/98 03:00a.m. 230.29 02/AUG/98 03:00a.m. 230.29 02/AUG/98 09:00a.m. 299.142 <t< th=""></t<>
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03/AUG/98	01:00p.m.	308.560
_03/AUG/98	02:00p.m.	321.984
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-03/AUG/98	04:00p.m.	280.764
03/AUG/98	05:00p.m.	325.411
03/AUG/98	06:00p.m.	343.848
03/AUG/98	07:00p.m.	366.840
03/AUG/98	08:00p.m.	300.740
03/AUG/98	09:00p.m.	300.820
03/AUG/98	10:00p.m.	356.704
0 3/AUG/98	11:00p.m.	343.623
_04/AUG/98	12:00a.m.	282.729
04/AUG/98	01:00a.m.	236.046
•04/AUG/98	02:00a.m.	268.206
04/AUG/98	03:00a.m.	347.805
04/AUG/98	04:00a.m.	304.054
04/AUG/98	05:00a.m.	363.506
04/AUG/98	06:00a.m.	284.849
04/AUG/98	07:00a.m.	337.762
04/AUG/98	08:00a.m.	347.696
04/AUG/98	09:00a.m.	335.591
04/AUG/98	10:00a.m.	301.517 315.394
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04/AUG/98 04/AUG/98	12:00p.m. 01:00p.m.	278.605
04/AUG/98	02:00p.m.	324.982
04/AUG/98	03:00p.m.	321.306
04/AUG/98	04:00p.m.	279.280
_04/AUG/98	05:00p.m.	311.967
04/AUG/98	06:00p.m.	333.616
4/AUG/98	07:00p.m.	322.459
04/AUG/98	08:00p.m.	286.419
D 4/AUG/98	09:00p.m.	332.193
04/AUG/98	10:00p.m.	327.366
04/AUG/98	11:00p.m.	338.546
05/AUG/98	12:00a.m.	285.562
05/AUG/98	01:00a.m.	247.787
05/AUG/98	02:00a.m.	247.739
05/AUG/98	03:00a.m.	258.774
05/AUG/98 05/AUG/98	04:00a.m. 05:00a.m.	235.060 262.111
05/AUG/98	06:00a.m.	268.364
■5/AUG/98	07:00a.m.	305.489
05/AUG/98	08:00a.m.	332.656
05/AUG/98	09:00a.m.	348.601
_05/AUG/98	10:00a.m.	322.973
05/AUG/98	11:00a.m.	339.779
5/AUG/98	12:00p.m.	311.813
05/AUG/98	01:00p.m.	306.446
5/AUG/98	02:00p.m.	330.048
5/AUG/98	03:00p.m.	330.231
05/AUG/98	04:00p.m.	276.935
95/AUG/98	05:00p.m.	308.338
5/AUG/98	06:00p.m.	315.872
05/AUG/98	07:00p.m.	334.133
05/AUG/98	08:00p.m.	280.706
5/AUG/98 5/AUG/98	09:00p.m. 10:00p.m.	292.765
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■6/AUG/98	12:00a.m.	338.077
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07/AUG/98 10: 07/AUG/98 11: 08/AUG/98 12: 08/AUG/98 01: 08/AUG/98 02: 08/AUG/98 03: 08/AUG/98 04: 08/AUG/98 05: 08/AUG/98 05: 08/AUG/98 07: 08/AUG/98 08: 08/AUG/98 09: 08/AUG/98 10: 08/AUG/98 11:	00p.m. 00p.m. 00a.m. 00a.m. 00a.m.	231.614 227.876 216.851 223.964 213.376

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08/AUG/98	01:00p.m.	211.487	
_08/AUG/98	02:00p.m.	203.272	
08/AUG/98	03:00p.m.	211.715	
•08/AUG/98	04:00p.m.	202.267 220.753	
08/AUG/98	05:00p.m. 06:00p.m.	218.203	
08/AUG/98	07:00p.m.	214.952	
08/AUG/98	08:00p.m.	227.086	
08/AUG/98	09:00p.m.	210.463	
08/AUG/98	10:00p.m.	215.042	
08/AUG/98	11:00p.m.	212.637 202.293	
09/AUG/98 09/AUG/98	12:00a.m. 01:00a.m.	202.293	
D9/AUG/98	02:00a.m.	206.763	
09/AUG/98	03:00a.m.	196.798	
09/AUG/98	04:00a.m.	234.549	
09/AUG/98	05:00a.m.	248.982	
09/AUG/98	06:00a.m.	245.032	
09/AUG/98 09/AUG/98	07:00a.m. 08:00a.m.	247.007 226.893	
09/AUG/98	09:00a.m.	208.838	
09/AUG/98	10:00a.m.	201.975	
9/AUG/98	11:00a.m.	208.250	
9/AUG/98	12:00p.m.	214.875	
09/AUG/98	01:00p.m.	226.797	
09/AUG/98 09/AUG/98	02:00p.m. 03:00p.m.	229.051 243.847	
09/AUG/98	04:00p.m.	235.355	
_09/AUG/98	05:00p.m.	215.893	
9/AUG/98	06:00p.m.	224.176	
9/AUG/98	07:00p.m.	225.104	
09/AUG/98	08:00p.m.	223.951	
9/AUG/98 9/AUG/98	09:00p.m. 10:00p.m.	215.996 217.448	
09/AUG/98	11:00p.m.	338.642	
<u>1</u> 0/AUG/98	12:00a.m.	313.470	
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1 0/AUG/98	02:00a.m.	294.939	
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10/AUG/98	06:00a.m.	304.057	
■ 0/AUG/98	07:00a.m.	337.178	
0/AUG/98	08:00a.m.	347.140	
10/AUG/98	09:00a.m.	342.592	
10/AUG/98 0/AUG/98	10:00a.m. 11:00a.m.	302.727	
0/AUG/98	12:00p.m.	339.108 276.267	
10/AUG/98	01:00p.m.	313.220	
0/AUG/98	02:00p.m.	336.686	
0/AUG/98	03:00p.m.	326.702	
10/AUG/98	04:00p.m.	336.221	
10/AUG/98 0/AUG/98	05:00p.m.	355.583	
10/AUG/98	06:00p.m. 07:00p.m.	438.236 1640.658	
10/AUG/98	08:00p.m.	570.316	
0/AUG/98	09:00p.m.	346.112	
0/AUG/98	10:00p.m.	328.022	
10/AUG/98	11:00p.m.	318.034	
1/AUG/98	12:00a.m.	308.344	

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11/AUG/98	01:00a.m.	312.953	
11/AUG/98	02:00a.m.	572.496	
11/AUG/98	03:00a.m.	354.825	
11/AUG/98	04:00a.m.	320.240	
11/AUG/98	05:00a.m.	319.277	
11/AUG/98	06:00a.m.	288.169	
11/AUG/98	07:00a.m.	333.684	
11/AUG/98	08:00a.m.	349.992	
11/AUG/98	09:00a.m.	335.052	
11/AUG/98	10:00a.m.	340.617	
11/AUG/98	11:00a.m.	355.535	
11/AUG/98	12:00p.m.	360.699	
11/AUG/98	-		
	01:00p.m.	349.199	
11/AUG/98	02:00p.m.	294.291	
11/AUG/98	03:00p.m.	306.623	
11/AUG/98	04:00p.m.	294.573	
11/AUG/98	05:00p.m.	328.430	
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11/AUG/98	07:00p.m.	300.367	
11/AUG/98	08:00p.m.	268.129	
11/AUG/98	09:00p.m.	275.163	
11/AUG/98	10:00p.m.	320.413	
11/AUG/98	11:00p.m.	345.309	
12/AUG/98	12:00a.m.	273.281	
12/AUG/98	01:00a.m.	349.790	
12/AUG/98	02:00a.m.	279.450	
12/AUG/98	03:00a.m.	259.953	
12/AUG/98	04:00a.m.	358.156	
12/AUG/98	05:00a.m.	312.285	
12/AUG/98	06:00a.m.	277.636	
12/AUG/98	07:00a.m.	328.744	
12/AUG/98	08:00a.m.	380.197	
12/AUG/98	09:00a.m.	342.904	
12/AUG/98	10:00a.m.	314.623	
12/AUG/98	11:00a.m.	305.367	
12/AUG/98	12:00p.m.	320.751	
12/AUG/98	01:00p.m.	311.492	
12/AUG/98	02:00p.m.	290.161	
12/AUG/98	03:00p.m.	302.214	
12/AUG/98	04:00p.m.	276.926	
12/AUG/98	05:00p.m.	316.389	
12/AUG/98	06:00p.m.		
12/AUG/98	07:00p.m.	353.444 276.788	
12/AUG/98	08:00p.m.		
12/AUG/98		253.870	
12/AUG/98	09:00p.m.	263.026	
12/AUG/98	10:00p.m.	267.098	
	11:00p.m.	268.781	
13/AUG/98	12:00a.m.	262.795	
13/AUG/98	01:00a.m.	268.858	
13/AUG/98	02:00a.m.	347.012	
13/AUG/98	03:00a.m.	255.344	
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13/AUG/98	06:00a.m.	263.129	
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13/AUG/98	08:00a.m.	303.511	
13/AUG/98	09:00a.m.	269.138	
13/AUG/98	10:00a.m.	272.436	
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13/AUG/98	12:00p.m.	303.553	

	01:00p.m.	318.454
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13/AUG/98	10:00p.m.	322.138
13/AUG/98	11:00p.m.	303.293
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5/AUG/98	01:00a.m.	225.088
1 5/AUG/98	02:00a.m.	221.045
15/AUG/98	03:00a.m.	223.020
5/AUG/98	04:00a.m.	219.217
5/AUG/98	05:00a.m.	222.114
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💼 5/AUG/98	07:00a.m.	299.012
5/AUG/98	08:00a.m.	229.523
1 5/AUG/98	09:00a.m.	213.408
<u>1</u> 5/AUG/98	10:00a.m.	235.709
5/AUG/98	11:00a.m.	239.556
■ 5/AUG/98	12:00p.m.	238.744
15/AUG/98	01:00p.m.	241.685
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15/AUG/98	04:00p.m.	249.942
15/AUG/98	05:00p.m.	246.085
5/AUG/98	06:00p.m.	248.404
T 5/AUG/98	07:00p.m.	249.480
15/AUG/98	08:00p.m.	234.636
5/AUG/98	09:00p.m.	232.385
5/AUG/98	10:00p.m.	232.237
15/AUG/98	11:00p.m.	228.120
6/AUG/98	12:00a.m.	233.952

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16/AUG/98	01:00a.m.	225.926	
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16/AUG/98	03:00a.m.	242.209	
16/AUG/98	04:00a.m.	239.116	
16/AUG/98	05:00a.m.	245.038	
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	08:00a.m.	233.326	
16/AUG/98			
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16/AUG/98	06:00p.m.	255.563	
16/AUG/98	07:00p.m.	253.799	
16/AUG/98	08:00p.m.	247.855	
16/AUG/98	09:00p.m.	251.291	
	_		
16/AUG/98	10:00p.m.	248.060	
16/AUG/98	11:00p.m.	240.973	
17/AUG/98	12:00a.m.	237.636	
17/AUG/98	01:00a.m.	325.199	
17/AUG/98	02:00a.m.	357.767	
17/AUG/98	03:00a.m.	312.780	
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		307.179	
17/AUG/98	05:00a.m.	308.926	
17/AUG/98	06:00a.m.	315.314	
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17/AUG/98	10:00a.m.	297.644	
17/AUG/98			
	11:00a.m.	338.421	
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17/AUG/98	01:00p.m.	315.291	
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17/AUG/98	05:00p.m.	307.092	
17/AUG/98	06:00p.m.	296.635	
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17/AUG/98	08:00p.m.	263.100	
17/AUG/98	09:00p.m.	301.086	
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17/AUG/98	11:00p.m.	255.909	
18/AUG/98	12:00a.m.	241.589	
18/AUG/98	01:00a.m.	244.107	
18/AUG/98			
· · · · · · · · · · · · · · · · · · ·	02:00a.m.	259.574	
18/AUG/98	03:00a.m.	236.817	
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18/AUG/98		371.449	
	11:00a.m.	356.197	
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18/AUG/98	11:00p.m.	218.938
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1 9/AUG/98	02:00a.m.	207.662
19/AUG/98	03:00a.m.	207.932
19/AUG/98	04:00a.m.	233.563
19/AUG/98	05:00a.m.	220.663
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19/AUG/98	10:00a.m.	312.853
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1 9/AUG/98	07:00p.m.	330.411
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20/AUG/98	05:00a.m.	241.824
20/AUG/98	06:00a.m.	230.747
🚅 0/AUG/98	07:00a.m.	228.078
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20/AUG/98	09:00a.m.	310.445
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20/AUG/98	07:00p.m.	307.599
_20/AUG/98	08:00p.m.	363.291
0/AUG/98	09:00p.m.	
0/AUG/98		304.564
20/AUG/98	10:00p.m.	253.902
20/AUG/98	11:00p.m.	256.728
1/ AUG/ 98	12:00a.m.	254.117

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21/AUG/98	01:00a.m.	251.503
21/AUG/98	02:00a.m.	268.309
21/AUG/98	03:00a.m.	295.508
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21/AUG/98	09:00p.m.	279.858
21/AUG/98	10:00p.m.	255.116
21/AUG/98	11:00p.m.	254.509
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22/AUG/98	03:00a.m.	226.276
22/AUG/98	04:00a.m.	225.149
22/AUG/98	05:00a.m.	216.956
22/AUG/98 22/AUG/98 22/AUG/98 22/AUG/98 22/AUG/98 22/AUG/98 22/AUG/98	06:00a.m. 07:00a.m. 08:00a.m. 09:00a.m. 10:00a.m. 11:00a.m.	214.917 202.209 205.777 208.603 204.149 213.138
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22/AUG/98	02:00p.m.	221.218
22/AUG/98	03:00p.m.	230.606
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22/AUG/98	06:00p.m.	231.704
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23/AUG/98	10:00a.m.	192.809
23/AUG/98	11:00a.m.	207.197
23/AUG/98	12:00p.m.	229.504

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23/AUG/98	01:00p.m.	229.141	
_23/AUG/98	02:00p.m.	228.364	
22/AUG/98	03:00p.m.	225.043	
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24/AUG/98	01:00a.m.	251.901	
24/AUG/98	02:00a.m.	245.286	
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24/AUG/98	04:00a.m.	262.606	
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_24/AUG/98	07:00a.m.	323.206	
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24/AUG/98	09:00a.m.	318.596	
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24/AUG/98	01:00p.m.	312.834	
24/AUG/98	02:00p.m.	292.659	
24/AUG/98	03:00p.m.	307.683	
24/AUG/98	04:00p.m.	276.897	
_24/AUG/98	05:00p.m.	325.603	
24/AUG/98	06:00p.m.	293.048	
4/AUG/98	07:00p.m.	316.762	
24/AUG/98	08:00p.m.	1258.876	
🚅 4/AUG/98	09:00p.m.	1133.954	
24/AUG/98	10:00p.m.	1328.154	
24/AUG/98	11:00p.m.	255.089	
 25/AUG/98	12:00a.m.	38.539	
25/AUG/98	01:00a.m.	38.539	
2 5/AUG/98	02:00a.m.	67.096	
_25/AUG/98	03:00a.m.	38.539	
5/AUG/98	04:00a.m.	38.539	
5/AUG/98	05:00a.m.	38.539	
25/AUG/98	06:00a.m.	38.539	
1 5/AUG/98	07:00a.m.	38.539	- Inaccurate Readings Meter dislodged in Channel
5/AUG/98	08:00a.m.	38.539	Hala
25/AUG/98	09:00a.m.	38.539	Meter dislodard in channel
25/AUG/98	10:00a.m.	38.539	See an craning
5/AUG/98	11:00a.m.	38.539	
5/AUG/98	12:00p.m.	38.53 <u>9</u>	
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APPENDIX E



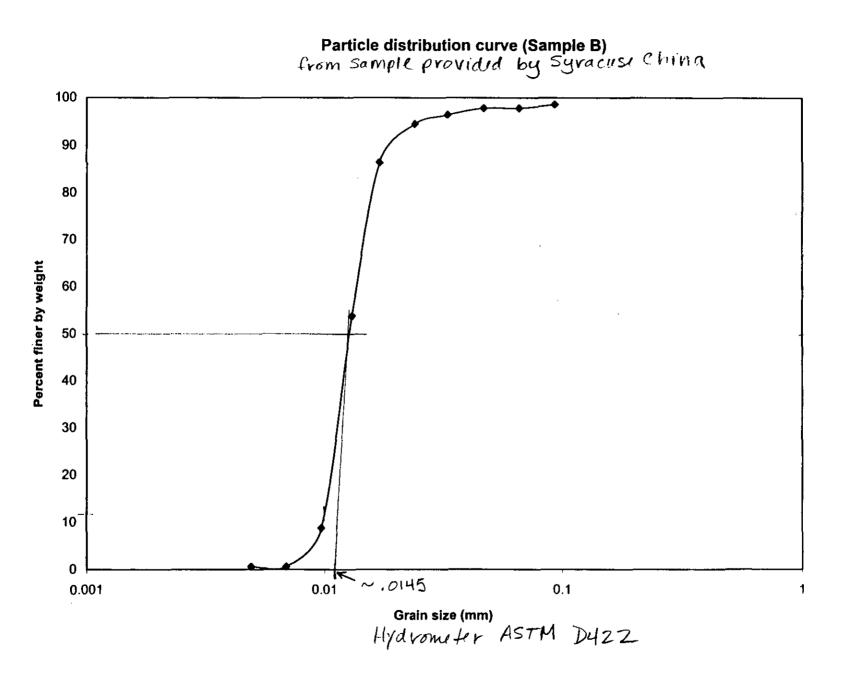
APPENDIX E

APPENDIX E

WASTEWATER INFLUENT PARTICLE SIZE DISTRIBUTIONS

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.0145 MM = 14.5 MM

Hydrometer Analysis

Mass in suspension = 24 g Assumed specific unit weight Gs = 2.75

Time (min)	Hydrometer Reading, R	Temperature (°C)	Percent finer by weight	Grain size (mm)
0.25	24	20	98.6	0.0930
0.5	23.8	20	97.8	0.0660
1	23.8	20	97.8	0.0467
2	23.5	20	96.4	0.0330
4	23	20	94.5	0.0240
8	21	20	86.4	0.0170
15	13	20	53.7	0.0130
30	2	20	8.8	0.0097
60	0	20	0.6	0.0069
120	0	20	0.6	0.0049

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SediGraph 5100 VS.07

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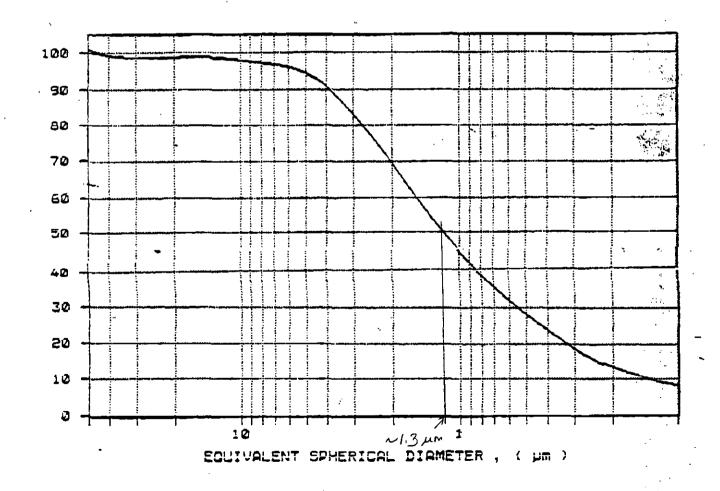
sample for phil

PAGE 2

4

SAMPLE DIRECTORY/NUMBER: RAWMATLS/133 UNIT NUMBER: 1 START 13:37:22 03/05/99 SAMPLE ID: waste water solids **REPRT 12:26:24 02/14/00** SUBMITTER: SYRACUSE CHINA TOT RUN TIME 0:59:32 OPERATOR: Cary Casler SAM DENS: 3.9800 g/cc SAMPLE TYPE: ALUMINA LIQ DENS: 0.9941 g/cc LIQUID TYPE: Water LIQ VISC: 0.7243 CP ANALYSIS TEMP: 34.9 deg C BASELINE/FULL SCALE: 140/ 102 kilocounts/sec RUN TYPE: High Speed

CUMULATIVE MASS PERCENT FINER VS. DIAMETER



sample for phil

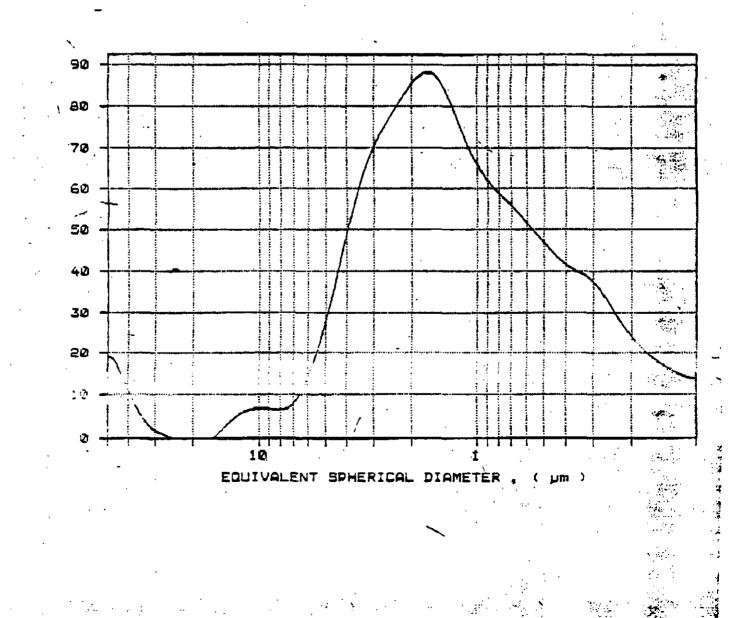
SediGraph 5100 V3.07

%/decade

HSS

SAMPLE DIRECTORY NUMBER: RAWMATLS/193 SAMPLE ID: waste water solids SUBMITTER: SYRACUSE CHINA OPERATOR: Cary Casler SAMPLE TYPE: ALUMINA LIQUID TYPE: Water ANALYSIS TEMP: 34.9 deg C BASELINE/FULL SCALE: 140/ 102 kilocounts/sec





START 13:37:22 03/05/99 REPRT 12:26:24 02/14/00 TOT RUN TIME SAM DENS: LIQ DENS: LIQ VISC: 0.7243 cp

UNIT NUMBER: 1

0:59:32 3,9800 g/cc 0.9941 g/cc RUN TYPE: High Speed

PAGE

SediGraph 5100 V3.07



PAGE 1 UNIT NUMBER: 1

SAMPLE DIRECTORY/NUMBER: RAWMATLS/133 START 19:37:22 03/05/99 REPRT 12:26:24 02/14/00 SAMPLE ID: waste water solids SUBMITTER: SYRACUSE CHINA TOT RUN TIME 0:59:32 OPERATOR: Cary Casler SAM DENS: 3,9800 g/cc SAMPLE TYPE: ALUMINA LIQ DENS: 0.9941 LIQUID TYPE: Water g/cc ANALYSIS TEMP: 34.9 deg C LIQ VISC: 0.7243 CP BASELINE/FULL SCALE: 140/ 102 kilocounts/sec RUN TYPE: High Speed

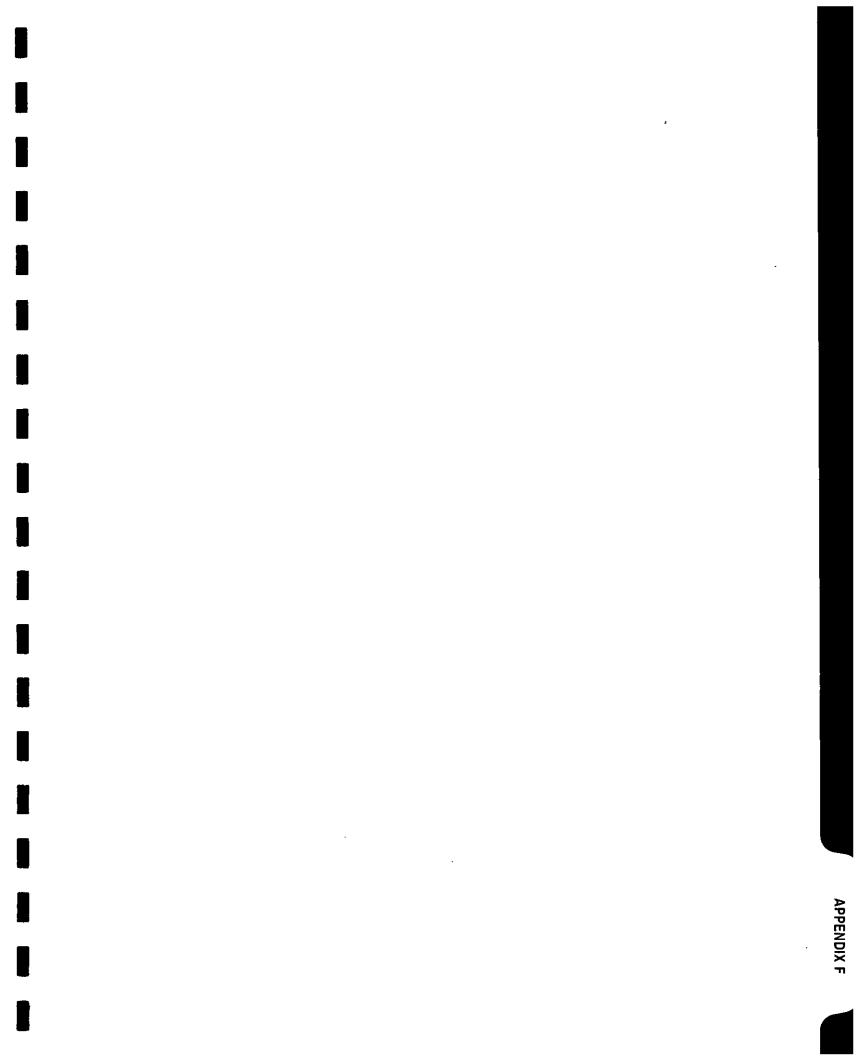
sample for phil

50.00 fm STARTING DIAMETER: ENDING DIAMETER: 0.10 fm REYNOLDS NUMBER: 0.39 FULL SCALE MASS X: 100

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MASS DISTRIBUTION

MEDIAN DI.	AMETER: 1.	19 fm	MODAL DIAM	
DIAMETER (fin)	Cumulative Mass Finer (%)			
3.89 2.73 2.05 1.58 1.19 0.84 0.55 0.32 0.13	90.0 80.0 70.0 60.0 50.0 40.0 30.0 20.0 10.0			
	-			,



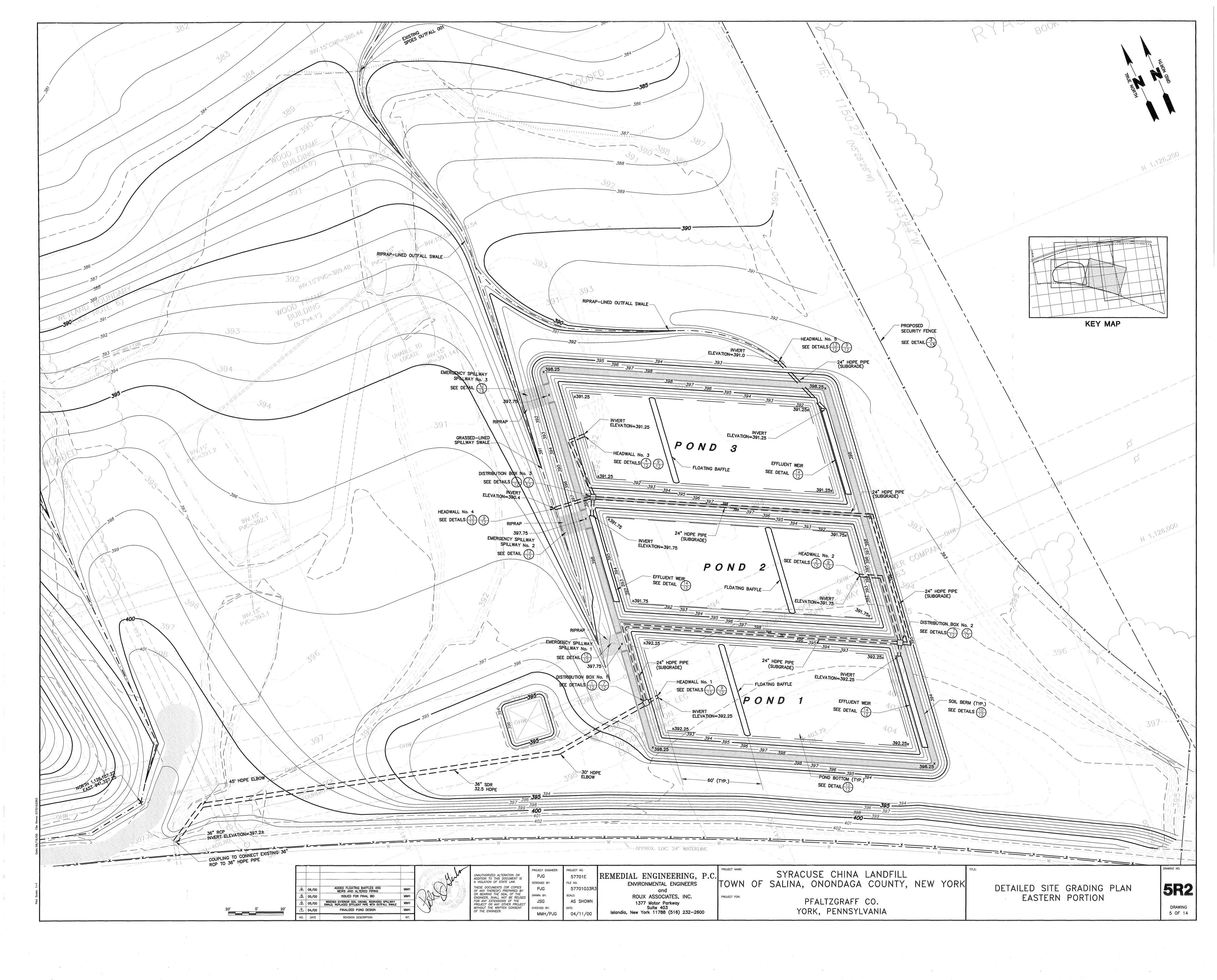
APPENDIX F

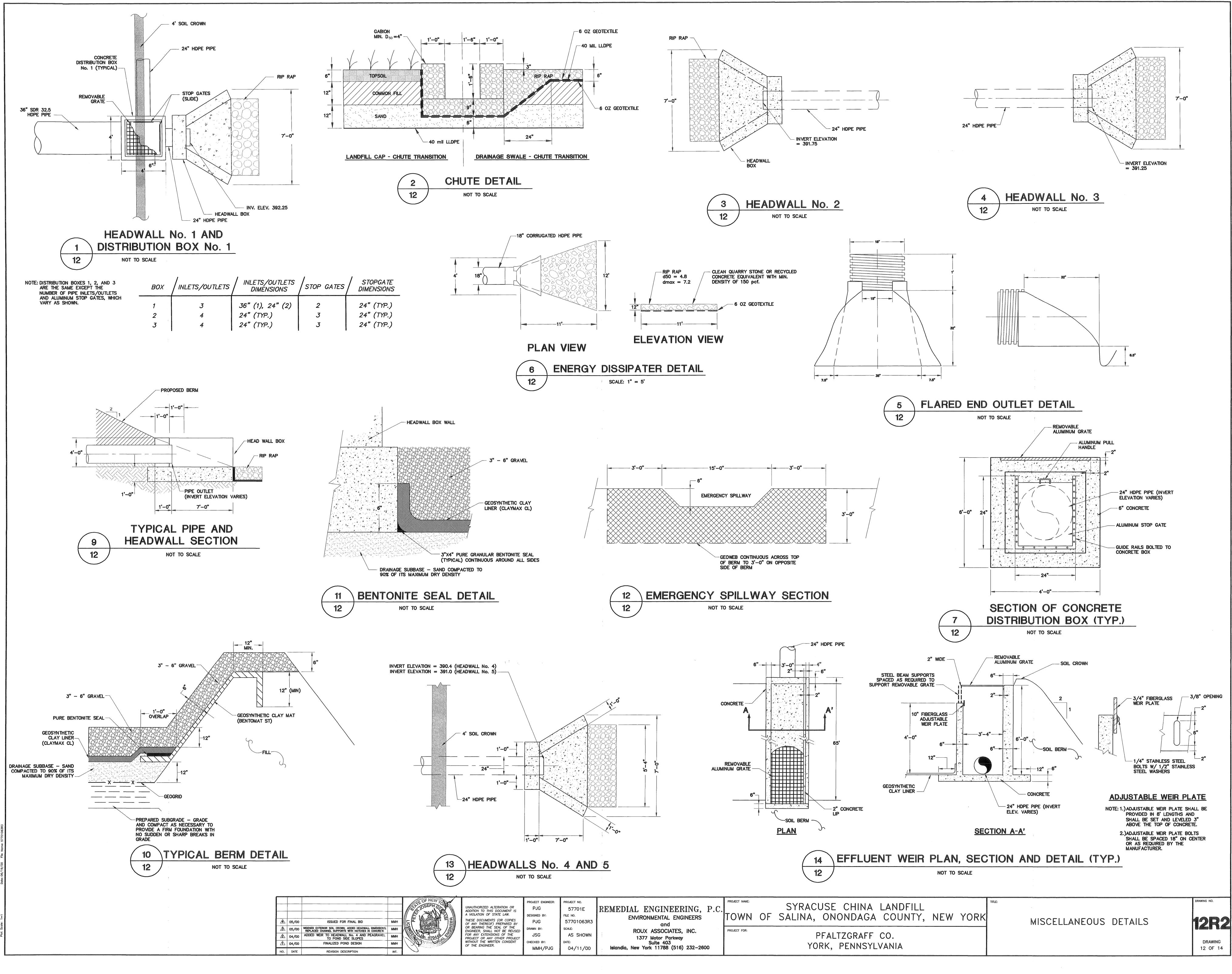
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NEW WASTEWATER SETTLING POND DESIGN DRAWINGS

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Flow Scenarios for Proposed Wastewater Settling Ponds

1. Normal Flow

Wastewater will enter the ponds through Headwall No. 1 and flow through Pond 1, flow through Headwall No. 2 and Pond 2, flow through Headwall No. 3 and Pond 3, through Headwall No. 5 and out to SPDES Outfall 001. The stop gates to the bypass piping in Distribution Boxes 1, 2, and 3 will be closed.

2. Facility Upset Flow

Wastewater will flow through Pond 1 for the retention time of approximately 3 hours. After approximately 3 hours, the stop gate in Distribution Box No. 1 will be closed to stop flow to Pond 1. The stop gate in Distribution Box No. 2 will be closed to stop flow from Pond 1 to Pond 2. Once the facility upset is mitigated, the stop gate in Distribution Box No. 1 for the bypass piping to Pond 2 will be opened so normal facility wastewater will flow into Pond 2 and through Pond 3 and out to SPDES outfall 001 via the outfall swale. The facility upset wastewater will remain isolated in Pond 1 until sufficient flocculent is added to aid in setting.

3. Removal of Pond 1 for Maintenance

The stop gate in Distribution Box No. 1 controlling flow to into Pond 1 will be closed. The stop gate in Distribution Box No. 1 controlling flow into the bypass piping to Pond 2 will be open. Wastewater will flow through Headwall No. 2 into Pond 2, through Headwall No. 3, through Pond 3, through Headwall No. 5 and out to SPDES Outfall 001 via the outfall swale.

4. Removal of Pond 2 for Maintenance

Wastewater will flow through Headwall No. 1 and through Pond 1. The stop gate in Distribution Box No. 2 controlling flow into Pond 2 will be closed and the stop gate in Distribution Box No. 2 controlling flow through the bypass piping to Pond 3 will be open. Wastewater will flow through Bypass Headwall No. 3, through Pond 3, through Headwall No. 5 and out to SPDES Outfall 001 via the outfall swale.

5. Removal of Pond 3 for Maintenance

Wastewater will flow through Headwall No. 1, through Pond 1, through Headwall No. 2 and through Pond 2. The stop gate in Distribution Box No. 3 controlling flow into Pond 3 will be closed and effluent will flow through Headwall No. 4 into the spillway swale which carries the effluent to SPDES Outfall 001.

APPENDIX G

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SPECIFICATIONS FOR POND DESIGN

SECTION 02225 OFF-SITE FILL MATERIALS

PART 1 - GENERAL

1.01 DESCRIPTION

- A. This section describes the Work activities related to the selection and use of fill materials obtained from off-site sources. In the event fill material from on-site sources are not acceptable for use as fill material or if fill material, in addition to on-site soil is required to perform the Work, fill materials from off-site sources may be utilized. Fill material from on-site sources are described in Section 02223. Off-site fill material shall meet any requirements specified in Section 02223 for on-site fill material in addition to the requirements specified herein. All fill materials shall be placed to the specified limits and grades as shown on the Drawings.
- B. All off-site fill materials shall be staged (Section 01500), as necessary, in accordance with these Specifications and Section 02223, and placed as shown on the Drawings.
- C. Off-Site fill materials include the following:
 - 1. common fill;
 - 2. crushed stone;
 - 3. riprap;
 - 4. pea gravel; and
 - 5. sand.

1.02 SUBMITTALS

A. In accordance with Section 01300, Contractor shall submit the following:

- 1. Gradation test results and certification from suppliers, that all off-Site fill materials meet the requirements of this Specification, and that they are free from contaminants. Certification must be received and approved prior to delivery of off-Site fill materials.
- 2. Three five-pound samples of all off-Site fill materials, if requested by the Engineer.
- 3. The Contractor shall submit the results of all testing required by the CQAP prior to proceeding with subsequent work.

PART 2 - PRODUCTS

2.01 COMMON FILL

- A. Common fill shall be well-graded granular material from fine to coarse, containing not more than 50 percent material passing a 200 sieve, with a maximum size of two (2) inches, obtained from approved natural deposits and unprocessed except for the removal of unacceptable material and stones larger than the maximum size permitted. It shall be substantially free from loam and other organic matter, trash, debris, and other fine substances. Vegetation, masses of roots, or individual roots debris, and other fine substances. Vegetation, masses of roots, or individual roots more than 18 inches long or more than 1/2 inch in diameter shall not be permitted.
- B. Common fill shall be used for backfill in excavated areas and the landfill cap barrier protection layer as shown on the Drawings.
- C. Common fill used in the landfill cap barrier protection layer shall have a permeability of less than 10⁻⁵ cm/sec.
- D. Common fill shall be used to construct the berms of the settling ponds as shown on the Drawings.

2.02 CRUSHED STONE

- A. Crushed stone shall be angular stone ranging from 1 to 3 inches in size for access roads and 3 to 6 inches in size for the settling ponds. The stone shall consist of clean, hard and durable particles free from dirt, vegetable, or other objectionable matter, and free from an excess of soft, thin elongated, laminated or disintegrated pieces.
- B. Crushed stone shall be used for the construction of the permanent access roads as shown on the Drawings.
- C. Crushed stone shall be placed in the interior of the settling ponds as shown on the Drawings.

2.03 RIPRAP

- A. Riprap shall have a mean spherical diameter of 4.8 inches, with stone sizes ranging from one (1) inch to 7.2 inches.
- B. Riprap shall consist of field stone, recycled concrete or rough unhewn quarry stone of approximately rectangular shape. The stone shall be hard and angular, of such quality that it will not disintegrate on exposure to water or weathering, and free from dirt, sand, clay and rock fines. The specific gravity of the individual stones shall be at least 2.5.

- C. Riprap shall be reasonably well-graded stone composed primarily of the larger stone sizes specified above, but shall have a sufficient mixture of the smaller specified sizes to fill the progressively smaller voids between stones.
- D. Riprap shall be used for the construction of drainage energy dissipation areas, drainage swales and in the Gabions installed as part of the cap drainage chute as shown on the Drawings.
- E. Riprap shall be used in the drainage swale which flows from the settling pond outlet to the new SPDES Outfall 001 as shown on the Drawings.

2.04 PEA GRAVEL

A. Pea gravel for pipe bedding shall be 3/8 inch nominal diameter, naturally rounded aggregate as follows:

<u>Standard Sieve Opening</u>	Percent Passing by Weight	
1/2"	100	
1/4"	0-15	
No. 200	0-1	

The gravel shall consist of clean, hard, and durable particles free from dirt, vegetable, or other objectionable matter, and free from an excess of soft, thin elongated, laminated or disintegrated pieces.

B. The pea gravel shall be installed as bedding for the corrugated drainage piping beneath the Site entrance to Factory Avenue as shown on the Drawings.

2.05 SAND

A. Sand: Natural river or bank sand; washed, free of silt, clay, loam, friable or soluble materials, or organic matter; graded in accordance with ANSI / ASTM C136, within the following limits:

<u>Sieve Size</u>	Percent Passing	
No. 4	100	
No. 14	10 to 100	
No 50	5 to 90	
No 100	4 to 30	
No. 200	0 to 1	

B. Sand shall be used in the barrier protection layer and shall have a permeability of greater than 10⁻² centimeters/second when used as the cap drainage layer shown on the Drawings.

C. Sand shall be used as the drainage subbase in the settling ponds as shown on the Drawings.

PART 3 - EXECUTION

3.01 OFF-SITE FILL MATERIALS

- A. All off-site fill materials shall be placed as shown on the Drawings and tested in a qualified independent laboratory in accordance with the CQAP Test results shall then be used to perform in-place density testing in the field.
- B. All off-site fill materials shall be consolidated, sloped and graded within the cap footprint as specified herein and as shown on the Drawings.
- C. All off-site fill material used outside of the cap limits shall be placed in 12-inch lifts and compacted to 90 percent of the Standard Proctor Density.
- D. The compaction requirements of this section are expected to minimize the amount of settlement that occurs; however, if significant settlement does occur following compaction, the Engineer may direct the Contractor to employ settlement monitoring by topographical survey methods. Refer to Section 01050 for survey requirements.
- E. In accordance with the CQAP and the SAP, any fill material provided from off-Site sources shall be analyzed to certify that the materials are free from contaminants.

3.02 FINAL GRADING

A. Final grading shall be performed as indicated on the Drawings, to the lines, and grades shown or as modified during placement of the waste within $\pm 2/10$ of a foot. The tolerance for cap thickness above the geomembrane shall be $\pm 1/10$ of a foot. The grading shown in the Drawings is based upon current estimations of waste removal depths and areas. Final grading may be altered based on actual conditions encountered during remedial activities. During the process of grading, the subgrades shall be maintained in such condition that the subgrade will be well drained at all times.

(END OF SECTION)

SECTION 02275 GEOTEXTILES

PART 1 - GENERAL

1.01 DESCRIPTION

- A. The Contractor shall furnish all labor, equipment and materials necessary for installation of geotextiles for use in silt fences, erosion control, and reinforcement/protection of various parts of the Work including:
 - 1. Geomembrane cushion layer;
 - 2. Wastepiles;
 - 3. Stabilized construction entrance;
 - 4. Equipment decontamination pad;
 - 5. Permanent access road;
 - 6. Drainage swales;
 - 7. Emergency spillways; and
 - 8. Settling ponds.

1.02 QUALITY ASSURANCE

- A. Each roll of geotextile shall be labeled by the manufacturer with the following information:
 - 1. Manufacturer's name;
 - 2. Product identification;
 - 3. Lot number;
 - 4. Roll number; and
 - 5. Roll dimensions.

- B. A competent laboratory shall be maintained by the producer of the fabric at the point of manufacture to ensure quality control for all products identified in this specification. That laboratory/manufacturer shall maintain records of its quality control results and provide, prior to shipment, a manufacturer's certificate which lists the guaranteed "minimum average roll value" properties as defined by the Federal Highway Administration and in the CQAP, for the type of geotextile to be delivered and shall provide written certification signed by a responsible party that the materials actually delivered have property "minimum average roll values" which meet or exceed all property values guaranteed for that type of geotextile. The certificate shall also include:
 - 1. Name of manufacturer;
 - 2. Chemical composition;
 - 3. Product identification/description;
 - 4. Statement of compliance to requirements of this specification section; and
 - 5. Signature of legally authorized official attesting to the information required.
- C. The Contractor shall be required to perform shear tests in accordance with the CQAP for the interface between the geotextile and the liner cushion layer soils which shall consist of northern wetland soil and overexcavated soil from beneath the waste. The minimum factor of safety resulting from these tests shall be 1.5.

1.03 SUBMITTALS:

- A. In accordance with Section 01300, Contractor shall submit the following:
 - 1. Samples and manufacturer's product literature for all geotextiles.
 - 2. Complete written instructions for storage, handling, installation, and joining/seaming of all geotextiles.
 - 3. Manufacturer's Quality Control/Quality Assurance manuals for all products, and any other manufacturer's literature regarding testing protocols and procedures to be followed in the field.
 - 4. Manufacturer's Certificate as outlined in paragraph 1.02(B) of this section.
 - 5. Results of all testing as defined in the CQAP.

PART 2 - PRODUCTS

2.01 WOVEN AND NON-WOVEN GEOTEXTILES

- A. Six-ounce woven geotextile shall be provided for use in wastepiles, stabilized construction entrance, equipment decontamination pad, and permanent access road.
- B. Six-ounce non-woven geotextile shall be provided for use beneath drainage swales.
- C. 10-ounce non-woven geotextile shall be provided for use with the geomembrane cushion layer. The geotextile shall meet the following requirements:

1.	grab strength	257 lbs
2.	tear strength	120 lbs
3.	puncture strength	175 lbs
4.	burst strength	530 psi
5.	minimum mass per unit over	10 oz.
6.	apparent opening size	140 (U.S. Sieve)

D. The properties of the geotextiles shall be established by testing as required by the CQAP.

2.02 GEOGRID

A. The geogrid for the emergency spillways and below the sand drainage layer in the settling ponds shall be Tensar BX1100 or Engineer-approved equal.

PART 3 - EXECUTION

3.01 HANDLING AND INSTALLATION

- A. The Contractor shall handle all geotextiles in a manner that they are not damaged in any way.
- B. All geotextiles shall be provided in rolls wrapped with protective covering to protect from ultraviolet light, precipitation, dirt, puncture, cutting or any other damage. If any special handling of the geotextile is required, it shall be so marked on the top surface of the geotextile (e.g., "This Side Up").
- C. Geotextiles shall not be exposed to precipitation prior to installation.
- D. Erosion control and turf reinforcement blankets shall be installed in the ground with staples and in accordance with the manufacturer's printed installation instructions.

- E. All geotextiles shall be installed, overlapped and seamed in accordance with manufacturer's printed recommendations.
- F. No horizontal seams shall be allowed on side slopes (i.e., seams shall be along, not across, the slope), except as part of a patch. All sewing shall be performed using polymeric thread with chemical resistance properties equal to or exceeding those of the geotextile.

3.02 PLACEMENT OF MATERIALS OVER GEOTEXTILE

- A. The Contractor shall place all materials over geotextile to prevent damage to or slippage of the geotextiles.
- B. No traffic or construction equipment shall be allowed directly on the geotextiles All machinery utilized in the placement shall be low ground pressure or otherwise specifically suited for the installation of these materials.
- C. During placement of the geomembrane cushion layer, a minimum thickness of one foot of material shall be placed between the geotextile materials and any construction equipment.

3.03 REPAIRS

- A. Geotextiles will be rejected at any time if they are determined by the Engineer to be defective, to have deteriorated, or if they are damaged.
- B. In the event that any damage occurs to the geotextiles at any time, the damaged portion shall either be repaired by methods approved by the Engineer or the geotextile replaced at no additional expense to the Owner.

(END OF SECTION)

SECTION 02623

HIGH DENSITY POLYETHYLENE (HDPE) PIPING AND ACCESSORIES

PART 1 – GENERAL

1.01 SCOPE OF WORK

- A. The Contractor shall furnish all labor, equipment materials and appurtenances necessary to install and test piping as shown on the Drawings, and specified herein.
- B. The Contractor shall be responsible for the proper connection of pipes to existing pipes, and to connecting all sections of supplied pipe.

1.02 SUBMITTALS

A. In accordance with Section 01300, Contractor shall submit information showing that the pipe and accessories to be supplied are in conformance with this Specification.

PART 2 – PRODUCTS

2.01 GENERAL

- A. All pipe and fittings shall be marked with the manufacturer's name or trademark, size, class, and the date of manufacture.
- B. All pipe shall be made from high density polyethylene resin compound qualified as Type III, Grade P33, Category 4, Class C by ASTM D1248 with a minimum density of 0.947 grams per cubic centimeter. The pipe shall have a long-term hydrostatic strength of 1,600 psi when tested and analyzed by ASTM D-2837. These requirements shall be certified by the pipe manufacturer.
- C. The corrugated HDPE pipe to be installed for swale drainage on the north side of the landfill shall be 18-inch diameter N-12 pipe as manufactured by Advanced Drainage Systems, London, Ohio or Engineer's-approved equal. Minimum wall thickness shall be 0.035 inches.
- D. The corrugated HDPE pipe to be installed from the outfall at the railroad tracks to the pond inlet shall be Model number MLSCO-ppd[®]/305 36-inch diameter SDR 32.5 pipe as manufactured by M.L. Sheldon Plastics Corp., New York, New York or Engineer's-approved equal.
- E. The corrugated HDPE pipe to be installed inside the ponds shall be Sure-Lok[®] WT 24-inch diameter pipe as manufactured by Hancor, Inc. or Engineer's-approved equal.

 F. Flared end outlets shall be model number 1810NP as manufactured by Advanced Drainage Systems, London, Ohio or Engineer's-approved equal. It shall fit an 18-inch diameter
 REMEDIAL ENGINEERING, P.C. 02623-1 PAST701E.75 corrugated HDPE pipe, be 42 inches wide at the mouth and 25 inches long from the pipe connection to the mouth end.

PART 3 – EXECUTION

3.01 GENERAL HANDLING

- A. Special care in handling shall be exercised during delivery, storage and installation of pipe to avoid cutting, kinking or other damage. Damaged pipe will be rejected and shall be replaced at the Contractor's expense.
- B. Ropes, fabric or rubber protected clings and straps shall be used when handling pipes. Chains, cables or hooks inserted into the pipe ends shall not be used. Two slings spread apart shall be used for lifting each length of pipe. Pipe shall not be dropped onto rocky or unprepared ground. Other proper tools and methods such as filled cutting shall be used for the handling of pipe and in accordance with manufacturer's printed installation instructions.

3.02 STORAGE OF PIPE

- A. Stacking of pipe shall be limited to a height that will not cause excessive deformation of the bottom layer of pipes under anticipated temperature conditions.
- B. Pipe shall be stored in such a manner as to keep the interior free from dirt, and in a location away from heavy machinery and trucks.

3.03 PLACEMENT OF PIPE

- A. Placement of pipe shall be performed in accordance with the manufacturer's printed information to the lines and grades shown on the Drawings.
- B. At the time of pipe placement, the pipe shall be examined carefully for defects, and should any pipe be discovered to be defective after being placed, it shall be removed and replaced with new pipe at the Contractor's expense.
- C. All pipe shall be carefully lowered into the trench so as to prevent dirt and other foreign substances from entering the pipe.
- D. All pipes shall be protected from ultraviolet light and from freezing conditions during placement.
- E. Whenever pipe placement is stopped at the end of the day, or for any other reason, the end of the pipe shall be securely closed to prevent entrance of water or any foreign matter.
- F. Piping shall be tested in accordance with Paragraph 3.01 prior to placing the backfill around the pipe unless otherwise permitted by the Engineer.

- G. The Contractor shall place backfill around the pipe in a manner that will not compromise the integrity of the pipe, and the pipe shall be secured to prevent displacement of the pipe any movement of the pea gravel placement. Pipe backfill shall be in accordance with the Drawings and Section 02225 of the Specifications.
- H. Underground warning tape shall read "Caution Pipeline Buried Below," and shall be buried shallow, approximately eighteen inches under final grade in all trenches where pipes have been buried.

3.04 JOINING METHODS

A. Lengths of pipe shall be assembled into suitable installation lengths with soil and water tight split couplers. All joints including tees and elbows shall be installed with soil and water tight connections. Pipe connections shall be performed in accordance with the procedures recommended by the manufacturer, by qualified personnel.

3.05 FLUSHING AND TESTING

- A. Following installation of a section of pipe, the Contractor shall thoroughly clean all pipe by flushing with water or other means to remove all dirt and other obstructions which may have entered the pipe during the construction. Pipelines shall be flushed at a rate of at least 2.5 feet per second until water runs clear, and no obstructions remain. All pipes shall be secured as necessary so that pipe movement does not occur during the flushing activities.
- B. After flushing is complete, in order to test the pipe to verify that no obstructions remain and also to determine that the pipe has not been crushed, the Contractor shall place a spherical object with a diameter equal to at least ninety percent of the inside diameter of the pipe, at the upgradient end of the pipe and pull the object through the length of the pipe. If the object does not exit the downgradient end of the pipe, the Contractor shall either remove the obstructions or replace the pipe at no additional cost to the Owner.
- C. A pressure test shall be performed in the 36-inch diameter SDR 32.5 HDPE pipe as specific in AWWA C600. The pressure shall be maintained at 80 psi for 12 hours. Any discrepancies to the manufacturer's recommendations on test pressures shall be brought to the attention of the Engineer. Any material showing seepage or the slightest leakage shall be repaired and/or replaced as directed by the Engineer and retested at no additional expense to the Owner.
- D. A pressure test shall be performed in the 24-inch diameter HDPE pipe as specific in ASTM D3212 with the exception that joints be tested using 5 psi and the test apparatus shall meet ASTM C1103 requirements.

(END OF SECTION)

SECTION 02800 GEOSYNTHETIC CLAY LINER SYSTEM

PART 1 - GENERAL

1.01 DESCRIPTION:

- A. This Work includes the manufacture, supply and installation of a geosynthetic clay liner (GCL) system, located as shown on the Drawings.
- B. The Work includes furnishing all labor, materials, transportation, supervision, tools and construction machinery necessary to install and test the GCL as described in the Contract Documents.
- C. All procedures, operations, and methods shall be in strict accordance with the Contract Documents and Manufacturer's 'Quality Assurance Manual', unless otherwise authorized in writing by the Engineer. If there is a discrepancy between the Contract Documents and the Quality Assurance Manual, the Contractor shall bring the discrepancy to the attention of the Engineer for clarification.

1.02 DEFINITIONS:

- A. The following definitions are defined and capitalized for the purposes of this Section 02800 only, and shall have the meaning defined herein when used in the capitalized form only.
 - 1. Installer: The individual or firm, retained by the Contractor, who shall be responsible for field handling, stoning, placing, seaming and all other Site aspects of the GCL installation.
 - 2. Manufacturer: The individual or firm, retained by the Contractor, who shall be responsible for production of GCL rolls.

1.03 MISCELLANEOUS REQUIREMENTS:

A. Installation shall be performed under the direction of a single field engineer, employed by the Installer, who shall remain on Site and be in responsible charge throughout the GCL installation (including subgrade acceptance, GCL layout, and repairs) and all other activities contracted with the Installer. This field engineer shall have installed or supervised the installation of a minimum of 1,000,000 square feet of GCL. B. Quality assurance testing shall be carried out by the Manufacturer to demonstrate that the sodium bentonite and GCL meet the criteria stated in paragraph 2.03 of this section. Quality assurance testing shall be performed for each shipment of bentonite to the Manufacturer's plant, and for every 100,000 square feet of manufactured GCL.

1.04 SUBMITTALS:

The Manufacturer shall submit the following information in accordance with Section 01300 - "Submittals":

- 1. Origin (bentonite supplier's name, bentonite origin) of the bentonite used in the GCL.
- 2. Reports on the tests conducted to verify the quality of the bentonite and geotextiles used to manufacture the GCL rolls delivered to the Site.
- 3. Test data demonstrating that the permeability of the GCL is not adversely influenced by multiple desiccation/rehydration cycles and multiple freeze/thaw cycles. The data provided shall be from bench scale tests so as to avoid the masking effects of artificial consolidation that occurs from flexible wall permeability testing.
- 4. Copies of quality control certificates for each roll of GCL. The quality control certificate shall include:
 - a) Roll numbers and identification;
 - b) Certification that GCL properties conform to the properties listed in paragraph 2.03 of this section, as measured using the test method specified.
 - c) The signature of a responsible party employed by the GCL Manufacturer, such as the production manager.
- 5. Prior to installation, the Installer shall submit a panel layout drawing. Each field panel shall be given an identification code, agreed upon by the Installer and Engineer, that shall be as simple and logical as possible. The drawing shall show the location and number of all panels, and expected overlap 'seams'. The Engineer/Owner's Representative shall review the panel layout drawing and verify that it is consistent with the accepted state of practice. No panels may be placed in the field without the Engineer's approval. In addition, Installer shall not deviate from the Engineer's approved plan without the Engineer's prior written approval.
- 6. Copies of the Manufacturers "Quality Assurance" manual.
- 7. Qualifications and Work experience of:
 - a. Installer
 - b. Field engineer
- 8. Documentation that Installer is certified by the GCL Manufacturer.

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- 9. All written results of testing.
- 10. Final engineers report by Installer.
- 11. Record Drawings by Installer.

PART 2 - PRODUCTS

2.01 GENERAL:

A. The GCL shall consist of a layer of natural sodium bentonite clay encapsulated between two geotextiles, and shall comply with the all of the manufacturing processes and physical/chemical criteria listed in this section. For reasons of strength, performance, and integrity, the GCL shall be manufactured by mechanically bonding the geotextiles using a needlepunching or stitch-bonding process.

2.02 GCL:

A. The GCL shall be as follows or an Engineer's approved equal.

Product	<u>Manufacturer</u>
Settling Pond Berm: Bentomat ST	CETCO Arlington Heights, Illinois
Settling Pond Floor: Claymax 600 CL	CETCO Fairmount, Georgia

B. The Installer shall be certified by the Manufacturer for the installation of Manufacturer's GCL.

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2.03 PERFORMANCE REQUIREMENTS:

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A. GCL properties shall conform to the following:

Property	Test Method	<u>Minimum Value</u>
Bentonite Swell Index	ASTM D5890	24 ml/2g minimum
Hydrated Internal Shear Strength	ASTM D5321	500 psf typical
Bentonite mass/unit area	ASTM D5993	0.75 lb./sq. ft.
Permeability (maximum)	ASTM D5084	5x10 ⁻⁹ cm/sec
Thickness	ASTM D1777	.2 in.
Grab Strength	ASTM D4632	90 lbs.
Grab elongation	ASTM D4632	15%
Wide width tensile	ASTM D4595	35 lbs./in.
Peel strength	ASTM D4632	15 lbs.
Puncture resistance	ASTM D4833	115 lbs.
GCL Index Flux	ASTM D5887	$1X10^{-8} \text{ m}^{3}/\text{m}^{2}/\text{sec.}$

B. Woven geotextile properties shall conform to the following:

Property	Test Method	<u>Minimum Value</u>
Grab strength	ASTM D4632	90 lbs.
Grab elongation	ASTM D4632	15%
Wide width strength	ASTM D4595	55 lbs./in.
Puncture Resistance	ASTM D4833	20 lbs.
Thickness	ASTM D5199	.5 mm
Mass per unit area	ASTM D5262	0.95 lb/ft ²

C. Non-woven geotextile shall conform to the following:

Property	Test Method	<u>Minimum Value</u>
Grab strength	ASTM D4632	6 lbs.
Grab elongation	ASTM D4632	150%
Wide width strength	ASTM D4595	15 lbs./in.
Puncture Resistance	ASTM D4833	57 lbs.
Thickness	ASTM D5199	3.5 mm
Mass per unit area	ASTM D5261	6 oz/sq. yd.

D. Bentonite properties shall conform to the following:

<u>Property</u>	Test Method	<u>Minimum Value</u>
Moisture content	ASTM D4643	25% maximum
Water absorption	ASTM E946	800% minimum
Fluid loss	ASTM D5891	18 ml. maximum
Fluid swell	ASTM D5890	24 ml. minimum

PART 3 - EXECUTION

3.01 UNLOADING AND HANDLING:

- A. The Contractor shall unload the materials at the Site in accordance with the Manufacturer's instructions, shall handle the material with care, shall use adequate equipment and shall take all precautions necessary to prevent damaging the GCL.
- B. Upon delivery at the Site, the Installer, in the presence of the Engineer, shall conduct a surface inspection of all rolls for defects and for damage. This inspection shall be conducted without unrolling rolls unless, in the Engineer's opinion, defects or damages are found or suspected.
- C. All flaws in the materials shall be brought to the attention of the Engineer. Rolls which have severe flaws shall be rejected and shall be removed from the Site.
- D. Rolls, which in the opinion of the Engineer have minor repairable flaws, shall be repaired in accordance with this section.

3.02 STORAGE:

A. The Contractor shall be responsible for providing that the material is protected from dirt, shock, theft, vandalism, passage of vehicles, moisture, and all other sources of damage. All stored GCL materials and granular bentonite shall be covered with plastic sheeting until they are used.

3.03 GENERAL INSTALLATION REQUIREMENTS:

- A. The Contractor shall layout and install the GCL in accordance with the Specifications and Contract Drawings and Manufacturer's 'Quality Assurance' manual, and shall ensure that the Installer supervises all portions of the installation.
- B. The Contractor shall be responsible for any damage to the GCL as a result of its Work. Repairs will be approved by the Engineer before any further installation Work is done.

3.04 FIELD PANEL PLACEMENT:

- A. The Installer shall ensure that each field panel is given an identification code consistent with the layout plan.
- B. GCL placement shall not proceed during precipitation, in the presence of excessive moisture (e.g., fog, dew), in an area of ponded water, in the presence of excessive winds, or at an ambient temperature below 40°F.
- C. The Contractor shall unwrap and install only as much GCL in one working day as can be covered with gravel. In no case shall the GCL be exposed to the elements at the end of the day.
- D. The surface upon which the GCL is installed shall be prepared and compacted in accordance with the Contract Documents and the manufacturer's recommendations. All surfaces to be lined shall be smooth and free of vegetation, void spaces, ice, abrupt elevation changes, standing water, cracks larger than ¹/₄ inch in width, debris, roots, sticks, sharp rocks and rocks larger than two inches.
- E. The use of equipment capable of freely suspending the GCL roll is required. A spreader bar and core pipe are also required for supporting the roll and allowing it to unroll freely. The core bar and spreader bar shall not bend or flex excessively when a full roll is lifted.
- F. If a GCL is chosen that is made up of a non-woven and woven geotextile, GCL panels shall be placed with the non-woven geotextile facing down. On sloped areas exceeding a steepness of 4H:IV, the long dimension of all panels shall be oriented parallel to the slope, and the ends of these panels shall be secured in an anchor trench. Panels shall be placed from the highest elevation to the lowest within the area to be lined, to facilitate drainage in the event of precipitation. Panels shall be placed free of tension or stress yet without wrinkles or folds. It is not permissible to stretch the GCL in order to fit a designated area. Panels shall not be dragged across the subgrade into position except where necessary to obtain the correct overlap for adjacent panels.
- G. Any leading edge of panels left uncovered shall be protected at the end of the working day with a waterproof sheet which is adequately secured with sandbags or other ballast.
- H. The Contractor is responsible for ensuring the following:
 - 1. Any equipment used does not damage the GCL by handling, traffic, leakage of hydrocarbons or other means.
 - 2. The prepared surface underlying the GCL has not deteriorated since previous acceptance, and is still acceptable immediately prior to GCL placement.
 - 3. All personnel working on the GCL do not smoke, wear damaging shoes, or engage in other activities which could damage the GCL.

- 4. Adequate temporary loading and/or anchoring, not likely to damage the GCL, has been placed to prevent uplift by wind.
- 5. Direct contact with the GCL is minimized.

3.05 SEAMING:

- A. The placement of all panels shall conform to the panel layout drawing approved by the Engineer.
- B. The GCL panels shall be placed with overlaps. The overlap area shall be clean and free of moisture, dust, soil, debris of any kind and foreign material.
- C. A 6-inch to 9-inch overlap shall exist at all seam locations. The edges of the GCL panels shall be adjusted to smooth out any wrinkles or creases in order to maximize contact between the overlapping panels.
- D. After the overlying panel is placed, its edge shall be pulled back to expose the overlap zone. Any soil or debris present in the overlap zone or entrapped in the geotextiles shall be removed.
- E. If the GCL is constructed of a woven and non-woven geotextile, the GCL seam shall be formed by executing a bentonite-enhanced overlap to ensure that a continuous seal is achieved between panels.
- F. A fillet of granular bentonite shall then be placed in a continuous manner along the overlap zone at a minimum rate of one-quarter pound per linear foot. The use of a watering can or line Chalker is required to improve the uniformity and consistency of the bentonite enhancement.
- G. In corners and odd-shaped geometric locations, the number of seams shall be minimized. No horizontal seam shall be less than 5 ft. from the toe of the slope, or areas of potential stress concentrations, unless otherwise authorized. On gently sloping areas (gentler than 4H:IV) where seams may be placed across the slope, overlaps shall be 'shingled' so as to prevent flow into the seam.
- H. Any procedures used to temporarily bond adjacent panels together shall be in accordance with Manufacturer's instructions and shall not damage the GCL.

3.06 PENETRATIONS:

- A. Penetrations shall be sealed in accordance with Manufacturer's directions. Seals shall begin with a small notch being made around the circumference of the pipe, into the subgrade.
- B. Sodium bentonite shall then be packed around the pipe in the notch and on adjacent areas so that the pipe is encased by a pure bentonite seal.

- C. The GCL shall then be placed over the penetration and slit into a 'pie' configuration where the pipe is to protrude. This procedure will create a snug fit between the GCL and the pipe once the laps are trimmed.
- D. Sodium bentonite shall then be spread around the cut edges of the GCL against the pipe and over adjacent areas.
- E. A GCL collar shall then be cut in a manner similar to that made on the main GCL panel, and fit around the pipe, with additional sodium bentonite applied into any gaps that may remain.

3.07 COVER PLACEMENT

- A. The cover soil shall consist of gravel as specified in Section 02225. Cover soils shall be free of foreign matter that could damage the GCL.
- B. Soil cover shall be placed over the GCL using construction equipment that minimizes stresses on the GCL. A minimum thickness of 1 foot of cover should be maintained between the equipment tires/tracks and the GCL at all times during the covering process. This thickness recommendation does not apply to frequently trafficked areas or roadways, for which a minimum thickness of 2 feet (600 mm) is required.
- C. Soil cover should be placed in a manner that prevents the soil from entering the GCL overlap zones. Cover soil shall be pushed up slopes, not down slopes, to minimize tensile forces on the GCL.

3.08 DEFECTS AND REPAIRS:

- A. All areas of the GCL shall be visually examined by the Installers field engineer for identification of defects, holes, undispersed raw materials, large wrinkles and any sign of contamination by foreign matter. The surface of the GCL shall be clean at the time of examination.
- B. Any damage in the form of cuts or tears in the GCL and shall be identified and repaired by the Installer. The damaged area shall be cleaned of all dirt and debris. A patch of GCL shall be cut to fit over the damaged area, and shall extend one foot in all directions around the entire damaged area. Granular bentonite shall then be placed around the perimeter of the affected area at the rate of one-half pound per linear foot, and the patch shall be placed over the damage. A fast-setting adhesive such as wood glue can be used to keep the patch in position during geosynthetic membrane placement.

3.09 SYSTEM ACCEPTANCE:

- A. The Installer, Manufacturer, and Contractor shall retain all ownership and responsibility for the GCL until acceptance by the Engineer. The liner system shall be accepted by the Engineer when:
 - 1. The installation is completed;

- 2. Verification of the adequacy of all repairs, including associated testing is complete;
- 3. All documentation of installation is completed including an Engineer's final report completed by the Installer;
- 4. Certification, including Record Drawings, sealed by a registered professional engineer, have been received by the Engineer. Record Drawings shall identify panel layout, locations of imperfections, repairs, and any other appropriate observations.
- 5. Final cover material has been placed above the GCL as shown in the Drawings.

3.10 GUARANTEE:

- A. Manufacturer shall guarantee the materials supplied for the installation of the GCL for a period of twenty years from the date of final installation, and shall be responsible to replace any materials which are defective during this period.
- B. Installer shall guarantee the installation of the GCL for a period of ten years from the date of submittal of engineers report. Installer shall be responsible for the repair and installation of any defects which occur during this period.

(END OF SECTION)

SECTION 11282

ALUMINUM STOP GATES

PART 1: GENERAL

1.01 SCOPE OF WORK

A. The Contractor shall furnish all labor, materials, equipment and incidentals required to install stop gates, guiderails and appurtenances as shown on the Drawings and as specified herein.

1.02 SUBMITTALS

- A. Copies of all materials required to establish compliance with these Specifications shall be submitted in accordance with the provisions of Section 01300. Submittals shall include at least the following:
 - 1. Dimensions of all components
 - 2. Installation instructions
 - 3. Descriptive literature, bulletins, and/or catalogs of the equipment
 - 4. Complete assembly drawings

PART 2: PRODUCTS

2.01 MATERIALS AND EQUIPMENT

- A. The stop gates shall be sized for 24-inch diameter pipe and shall be Model AR-5 as manufactured by Waterman Industries, Inc. of Memphis, Tennessee or Engineer's-approved equal.
- B. Stop gates and appurtenances shall consist of the following materials:

Frames and Slides: Aluminum ASTM B-209 and ASTM B-211 Alloy 6061-T6

Fasteners and Anchor Bolts: Stainless steel ASTM A-193, 18-8 or stainless steel ASTM A-276 Type 304 or 316.

Flush Bottom Seals and "J" Bulb Seals: Rubber ASTM D-2000 BC610-615

Finish: Mill finish on aluminum with standard shopcoat paint on lift

2.02 FRAME AND GUIDES

A. The gate frame shall be a rigid, welded unit, composed of the guide rails, cross bars, and headrails with a clear opening the same size as the waterway, unless otherwise specified. They shall be flatback type.

The guides shall be of extruded aluminum incorporating a dual slot design. The primary slot will accept the plate of the slide (disc) and the secondary slot will be sufficiently wide to accept the reinforcing ribs of the disc. The guides shall be designed for maximum rigidity, having a weight of not less than three pounds per foot. The guides will be of sufficient length to support two-thirds (2/3) the height of the slide, when the gate if fully open.

On self-contained gates, where the guides extend above the operating floor, they shall be sufficiently strong so that no further reinforcing will be required.

Additional members will be added to the frame as specified for flushbottom closure, spigots, and "J" bulb seals. Holes will be provided for anchor bolts for flatback ad spigotback models.

2.03 SLIDE COVER (DISC)

A. The slide cover shall be plate reinforced with structural aluminum shapes welded to the plate. The slide cover shall not deflect more than 1/360 of the span of the gate under maximum head. Reinforcing ribs shall extend to the guides so that the seating surface of the guide is reinforced.

2.04 HANDLE

A. The slide gates shall have a slot grip handle for removal in accordance with the manufacturer's recommendations.

PART 3: EXECUTION

3.01 GENERAL

- A. Prior to installation, protect stored stop gates, frames, guides and all appurtenances from damage due to exposure to sunlight, heat, dirt, debris, freezing and thawing, vandalism, etc.
- B. The stop gates and appurtenances shall be installed in full accordance with the manufacturer's recommendations, by mechanics skilled in the installation of this type or work.
- C. After installation, all stop gates and appurtenances shall be tested by flushing water through the system to check for leaks around stop gates in the closed position. If

any stop gate proves to be defective, it shall be replaced or otherwise made acceptable to the Engineer.

D. All materials shall be carefully inspected for defects in workmanship and materials. All operating mechanisms shall be operated to check their proper functioning, and all nuts and bolts checked for tightness. Stop gates and other equipment which do not operate easily, or are otherwise defective, shall be repaired or replaced at no additional cost to the Owner.

(END OF SECTION)

SECTION 11284

FLOATING BAFFLES

PART 1: GENERAL

1.01 SCOPE OF WORK

A. The Contractor shall furnish all labor, materials, equipment and incidentals required to install floating baffles and appurtenances as shown on the Drawings and as specified herein.

1.02 SUBMITTALS

- A. Copies of all materials required to establish compliance with these Specifications shall be submitted in accordance with the provisions of Section 01300. Submittals shall include at least the following:
 - 1. Dimensions of all components
 - 2. Installation instructions
 - 3. Descriptive literature, bulletins, and/or catalogs of the equipment
 - 4. Complete assembly drawings

PART 2: PRODUCTS

2.01 MATERIALS AND EQUIPMENT

A. The floating baffles shall be custom designed factory prefabricated hydraulic barrier curtains manufactured and delivered on site in a state of completion such that no further fabrication, other than to connect individual completed sections, is required for installation. The floating baffles shall utilize virgin quality elastomeric geomembrane materials and exclusively inplant dielectric and thermal seaming processes throughout. The floating baffles shall be designed for ease of installation in a new or operating lagoon without requiring de-watering.

2.02 HYDRAULIC CURTAIN MATERIAL

A. The floating baffle shall be the Director 1TM Model as manufactured by Environetics, Inc. of Lockport, Illinois or Engineer's - approved equal. The floating baffles shall be fabricated from polyester reinforced polymeric alloy membrane material ENV-3602-12-XR-5TM. Curtain materials shall possess outstanding seam strength, excellent performance in a wide range of chemicals, superior U.V. resistance and weathering qualities.

2.03 UPPER AND LOWER TENSION MEMBERS

- A. The floating baffles shall have (2) two tension members consisting of a ¼" diameter vinyl coated stainless steel aircraft cable dielectrically seamed in a hem under the flotation collar and ¼" diameter galvanized proof-coil chain; dielectrically seamed in a hem at the bottom edge of the hydraulic curtain. Both tension members shall attach to ¼" x 2" x 12" (min.) stainless steel bolt-through end-connect plates with 3/8"diameter material pullout stops.
- B. The floating baffles shall have means for attaching concrete weights directly to the lower tension member to achieve a total ballast weight of not less than 3.5 lbs. per linear foot. Bottom anchors shall be retrievable by means of factory installed 3/8" diameter Polypropylene anchor lifting lines which extend from each specified bottom anchor point to a brass grommet in the flotation collar above the bottom anchor. Ballast attached directly to the curtain material will not satisfy the bottom anchoring. Tension members sewn to the baffle will not be acceptable.

2.04 FLOTATION COLLAR

A. The flotation collar shall be constructed using either 6" or 10" diameter by 8' long polystyrene foam logs dielectrically sealed in a chamber of the specified baffle curtain material. The flotation material shall be closed cell polystyrene foam (1 lb. per cubic foot minimum foam density) providing a minimum buoyancy of 60 lbs. per cubic foot. External, mechanically attached flotation that may require a spare part inventory will not be acceptable.

2.04 BAFFLE SECTIONS

A. The floating baffles will be manufactured in the minimum number of sections required that allow convenient installation and efficient shipping in standard size crates. When a baffle exceeds the maximum practical size, the baffle shall be manufactured in sections which are joined together on site with a mechanical stainless steel bolt-through connector.

PART 3: EXECUTION

3.01 GENERAL

- A. The floating baffles shall be installed in the locations on the contract drawings.
- B. The floating baffles shall be installed in accordance with the manufacturer's drawings, instructions and recommendations.

(END OF SECTION)

APPENDIX H

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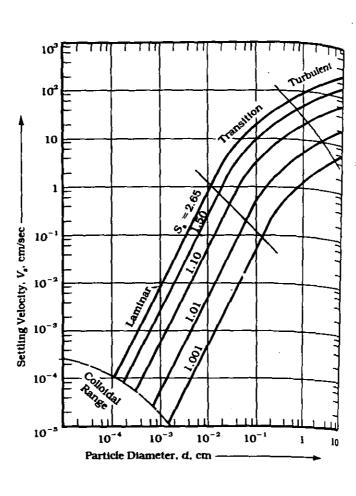
PAGES FROM UNIT OPERATIONS AND PROCESSES IN ENVIRONMENTAL ENGINEERING AND WASTEWATER ENGINEERING

Unit Operations and Processes in Environmental Engineering

Tom D. Reynolds

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Figure 3.2. Type I Settling of Spheres in Water at 10°C Adapted from "Water Treatment" by T. R. Camp in Handbook of Applied Hydraulics, 2nd ed., ed. C. V. Davis. Copyright © 1952 by McGraw-Hill Book Co., Inc. Reprinted by permission.

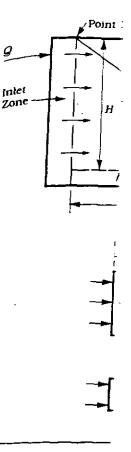


- 4. There are three zones in the basin: (1) the entrance zone, (2) the outlet zone, and (3) the sludge zone.
- 5. There is a uniform distribution of particles throughout the depth of the entrance zone.
- 6. Particles that enter the sludge zone remain there and particles that enter the outlet zone are removed.

Figure 3.3 shows an ideal rectangular settling basin aa length, L, a width, W, and a depth, H. V_0 is the settling velocity of the smallest particle size that is 100 percent removed. When a particle of this size enters the basin at the water surface, point 1, it has a trajectory as shown and

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Figure 3.3. Ideal Re.



intercepts the sludge stream end. The dete divided by the settlin

$$t=\frac{H}{V_0}$$

The detention time, t. by the horizontal velo

$$t = \frac{L}{V}$$

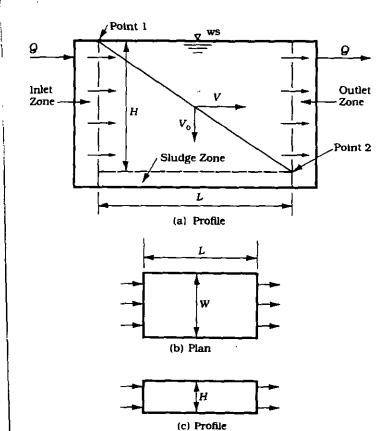
The horizontal veloci: vided by the cross-sec

$$V \approx \frac{Q}{HW}$$

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Figure 3.3. Ideal Rectangular Settling Basin



intercepts the sludge zone at point 2, which is at the downstream end. The detention time, $t_{\rm s}$ is equal to the depth, $H_{\rm s}$ divided by the settling velocity, $V_{\rm o}$, or

$$t = \frac{H}{V_0} \tag{3.13}$$

The detention time, t, is also equal to the length, L, divided by the horizontal velocity, V, or

$$t = \frac{L}{V} \tag{3.14}$$

The horizontal velocity, V, is equal to the flow rate, Q, divided by the cross-sectional area, HW, or

$$V = \frac{Q}{HW}$$
(3.15)

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Combining Eqs. (3.14) and (3.15) to eliminate V gives

$$t = \frac{LWH}{Q}$$

Since LWH equals the basin volume. \forall , the detention time, t, is equal to the basin volume, Ψ , divided by the flow rate Q, or

$$t = \frac{\Psi}{Q} \tag{3.17}$$

Equating Eqs. (3.16) and (3.13) gives

$$\frac{LWH}{Q} = \frac{H}{V_0}$$
(3.18)

Rearranging yields

$$V_{\rm o} = \frac{Q}{LW} \tag{3.19}$$

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$$V_{\rm o} = \frac{Q}{A_{\rm p}} = \text{overflow rate, gal/day-ft}^2$$
 (3.20)

where A_p is the plan area of the basin. Equation (3.20) shows that the overflow rate is equivalent to the settling velocity of the smallest particle size that is 100 percent removed.

The previous fundamentals also apply to an ideal circular settling basin, shown in Figure 3.4. The horizontal velocity, V, is given by

$$V = \frac{Q}{2\pi r H}$$

From inspection of Figure 3.4.

$$\frac{dh}{dr} = \frac{V_0}{V}$$

Substituting Eq. (3.21) into Eq. (3.22) gives

$$\frac{dh}{dr} = \frac{2\pi r H V_0}{Q}$$

Rearranging Eq. (3.23) and setting the integration limit yields

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Integrating gives

$$H = \frac{2\pi H V_0}{Q} \left[\frac{r}{2} \right]$$

or

Figure 3.4. Ideal Cir

(3.16)

Entrance Zone --

Sludge Zone -

 $\int_0^H dh = \frac{2\pi H V}{Q}$

(3.22)

(3.21)

(3.23)



 $H=\frac{\pi H V_0}{Q} (r_0^2)$

Wastewater Engineering

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Metcalf & Eddy, Inc.

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solids include those derived from toilets, sinks, baths, laundries, garbage grinders, and water softeners. Typical data on the daily per capita quantities of dry solid material derived from these and the aforementioned sources are reported in Table 7.5. ŧ

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TABLE 7-5 ESTIMATE OF THE COMPONENTS OF TOTAL SOLIDS IN WASTEWATER

Component	Dry weight, gpcc
Water supplies and ground water, assumed to	
have little hardness	12.7
Feces (solids, 23%)	20.5
Urine (solids, 3.7%)	43.3
Toilet (including paper)	20.0
Sinks, baths, laundries, and other sources of	
domestic wash waters	86.5
Ground garbage	30.0
Water softeners	*
Total for domestic sewage from separate	
sewerage systems, excluding contribution	
from water softeners	213.0
Industriai wastes	200.0‡
Total for industrial and domestic wastes	
from separate sewerage system	413.0
Storm water	25.0†
Total for industrial and domestic wastes	
from combined sewerage system	438.0

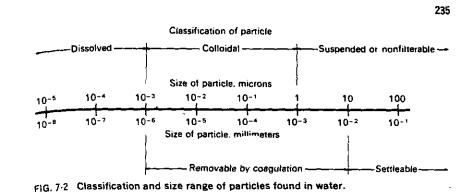
* Variable. † Will vary with the season.

‡ Will vary with the type and size of industries,

Analytically, the total solids content of a wastewater is defined as all the matter that remains as residue upon evaporation at 103 to 105°C. Matter that has a significant vapor pressure at this temperature is lost during evaporation and is not defined as a solid. Total solids, or residue upon evaporation, can be classified as either suspended solids or filterable solids by passing a known volume of liquid through a filter. The filter is commonly chosen so that the minimum diameter of the suspended solids is about 1 micron (μ). The suspended-solids fraction includes the settleable solids that will settle to the bottom of a cone-shaped container (called an Imhoff cone) in a 60-min period. Settleable solids are an approximate measure of the quantity of sludge that will be removed by sedimentation.

The filterable-solids fraction consists of colloidal and dissolved

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solids. The colloidal fraction consists of the particulate matter with an approximate diameter range from 1 millimicron $(m\mu)$ to 1μ (see Fig. 7.2). The dissolved solids consist of both organic and inorganic molecules and ions that are present in true solution in water. The colloidal fraction cannot be removed by settling. Generally biological oxidation or coagulation followed by sedimentation is required to remove these particles from suspension.

Each of the categories of solids may be further classified on the basis of their volatility at 600°C. The organic fraction will oxidize and be driven off as gas at this temperature, and the inorganic fraction remains behind as ash. Thus the terms "volatile suspended solids" and "fixed suspended solids" refer, respectively, to the organic and inorganic (or mineral) content of the suspended solids. At 600°C, the decomposition of inorganic salts is restricted to magnesium carbonate, which decomposes into magnesium oxide and carbon dioxide at 350°C. Calcium carbonate, the major component of the inorganic salts, is stable up to a temperature of 825°C. The volatile-solids analysis is applied most commonly to sewage sludges to measure their biological stability. The solids content of a medium-strength sewage may be classified approximately as shown in Fig. 7.3.

Turbidity, a measure of the light-transmitting properties of water, is another test used to indicate the quality of waste discharges and natural waters with respect to colloidal matter. Colloidal matter will scatter or absorb light and thus prevent its transmission.

Temperature

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The temperature of wastewater is commonly higher than that of the water supply, because of the addition of warm water from households and industrial activities. As the specific heat of water is much greater

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