NEW YORK STATE | Department of Environmental Conservation

60-Day Advance Notification of Site Change of Use

Physical Alteration, Transfer of Certificate of Completion, and/or Ownership Required by 6NYCRR Part 375-1.11(d) and 375-1.9(f)

SUBMITTAL INSTRUCTIONS:

Please submit via Site Control Dropbox as described below, OR, if file size permits, by email to DERSiteControl@dec.ny.gov. Print to pdf before submitting.

You may submit your document(s) via ground mail at the address below however please – DO NOT submit both electronic and ground mail.

a.) VIA SITE CONTROL DROPBOX:

Request an Invitation

In the "Title" field, please include the following: "Change of Use – Site Name, Site # 828178 / 828013.

After uploading files, an automated email will be sent to the submitter's email address with a link to verify the status of the submission. Please do not send a separate email to confirm receipt.

Packages submitted through third-party file transfer services will not be accepted.

b.) VIA GROUND MAIL:

Save the COU form w/attached file(s) and cover letter (optional) to an external storage device (e.g., thumb drive, flash drive). Do NOT include any paper.

Mail the external storage device to the following address:
Chief, Site Control Section
Division of Environmental Remediation
625 Broadway, 12th Floor
Albany, NY 12233-7020

Section I: Property Information	
Site Name: Xerox Webster Landfill Site	DEC Site # 828178 / 828013
Site Address: 800 Phillips Road, Webster, NY 14580	

Section II: Contact Information Person Submitting Notification	
Name: Wendi Latko	
Address1: 800 Phillips Road, Bldg. 0111-04A	
Address 2: Webster, New York 14580	
Phone: 585-422-9988	E-mail: wendi.latko@xerox.com

Section III: Type of Change and Date
☐ Change of Ownership
☐ Change of Remedial Parties
☐ Transfer of Certificate of Completion
☑ Other (e.g., any physical alteration or other change of use)
Proposed Date of Change (mm/dd/yyyy) September 1, 2025

Section IV: Description of Proposed Change (Required)

Please provide a brief narrative of the proposed changes(s) indicated above. Attach maps, drawings, and/or parcel information as needed. If "other" the description must explain and advise the DEC how such change may or may not affect the site's proposed, ongoing, or completed remedial program (attach additional sheets if needed).

TM Montante Solar Development LLC is proposing construction of a photo-voltaic solar array within a 20-acre portion of the Xerox Property (Site ID No. 828178 / 828013) A description of the project can be found in Attachment A. The boundaries of the proposed solar development are shown in attached Figure 1.

Section V: Certification Statement

Where the change results in a change in ownership or in responsibility for the proposed, ongoing, or completed remedial program for the site, the following certification must be completed (by owner or designated representative: see § 375-1.11(d)(4)(i):

I hereby certify that the prospective purchaser and/or remedial party has been provided a copy of any order, agreement, Site Management Plan, or State Assistance Contract regarding the Site's remedial program as well as a copy of all approved remedial work plans and reports.

Name: _____ _____

(Signature) (Date)

_____

(Print Name)

Address1:_____

Address2:_____

Phone:_____ Email :_____

Section VI: Contact Information for New Owner, Remedial Party, or CoC Holder

If the site will be sold or there will be a new remedial party, identify the prospective owner(s) or party(ies) along with contact information. If the site is subject to an Environment Easement, Deed Restriction, or Site Management Plan subject to periodic certification of institutional controls/engineering controls (IC/ECs), indicate who will be the certifying party (attach additional sheets if needed).

☐ Prospective Owner ☐ Prospective Remedial Party ☐ Prospective Owner Representative

Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

Cert. Party Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

Section VII: Agreement to Notify DEC After Transfer

If Section VI applies, i.e., all or part of the site will be sold, in accordance with § 375-1.11(d)(4)(ii) 30 days after the transfer a letter to notify the DEC of the completion of the transfer must be provided. If the current owner is also the CoC holder for the site, the CoC should be transferred to the new owner using DEC's approved "Notice of Transfer of COC" forms found at Initial Notice And Transfer Of Certificate Of Completion - NYSDEC. This form has its own filing requirements at §375-1.9(f).

Signing below indicates that theses notices will be provided to the DEC within the specified timeframes as follows:

Within 30 days of the sale of the site, I agree to submit to the DEC:

1. The name and contact information for the new owner(s) per §375-1.11(d)(4)(ii)
2. The name and contact information for any owner representative; and
3. A Notice of Transfer using the DEC form Initial Notice And Transfer Of Certificate Of Completion - NYSDEC.

Name: _____ _____

(Signature) (Date)

_____

(Print Name)

Address1:_____

Address2:_____

Phone:_____Email:_____

Continuation Sheet (if needed for multiple owners, representatives, or remedial parties)

☐ Prospective Owner ☐ Prospective Remedial Party ☐ Prospective Owner Representative

Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

☐ Prospective Owner ☐ Prospective Remedial Party ☐ Prospective Owner Representative

Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

☐ Prospective Owner ☐ Prospective Remedial Party ☐ Prospective Owner Representative

Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

☐ Prospective Owner ☐ Prospective Remedial Party ☐ Prospective Owner Representative

Name:_____

Address:1_____

Address2:_____

Phone:_____ Email:_____

NEW YORK STATE | Department of Environmental Conservation

Instructions for Completing the 60-Day Advance Notification of Site Change of Use, Transfer of Certificate of Completion (CoC), and/or Ownership Form

Please submit via Site Control Dropbox Request an Invitation

In the "Title" field, please include the following: "Change of Use – Site Name, Site #

OR, if file size permits, by email to DERSiteControl@dec.ny.gov

Section I	**Description**
Site Name	Official DEC site name. (see http://www.dec.ny.gov/cfmx/extapps/derexternal/index.cfm?pageid=3)
DEC Site ID No.	DEC site identification number.

Section II	**Contact Information of Person Submitting Notification**
Name	Name of person submitting notification of site change of use, transfer of certificate of completion and/or ownership form.
Address1	Street address or P.O. box number of the person submitting notification.
Address2	City, state and zip code of the person submitting notification.
Phone	Phone number of the person submitting notification.
E-mail	E-mail address of the person submitting notification.

Section III	**Type of Change and Date**
Check Boxes	Check the appropriate box(s) for the type(s) of change about which you are notifying the Department. Check all that apply.
Proposed Date of Change	Date on which the change in ownership or remedial party, transfer of CoC, or other change is expected to occur.

Section IV	**Description**
Description	For each change checked in Section III, describe the proposed change. Provide all applicable maps, drawings, and/or parcel information. If "Other" is checked in Section III, explain how the change may affect the site's proposed, ongoing, or completed remedial program at the site. Please attach additional sheets, if needed.

Section Certification

This section must be filled out if the change of use results in a change of ownership or responsibility for the proposed, ongoing, or completed remedial program for the site. When completed, it provides DEC with a certification that the prospective purchaser has been provided a copy of any order, agreement, or State assistance contract as well as a copy of

Name	The owner of the site property or their designated representative must sign and date the certification statement. Print owner or designated representative's name on the line provided below the signature.
Address	Owner or designated representative's street address or P.O. Box
Address	Owner or designated representative's city, state and zip
Phon	Owner or designated representative's phone
E-Mail	Owner or designated representative's E-

Section Contact Information for New Owner, Remedial Party, and CoC Holder (if a CoC was issued)

Fill out this section only if the site is to be sold or there will be a new remedial party. Check the appropriate box to indicate whether the information being provided is for a Prospective Owner, CoC Holder (if site was ever issued a COC), Prospective Remedial Party, or Prospective Owner Representative. Identify the prospective owner or party and include contact information. A Continuation Sheet is provided at the end of this form for additional

Name	Name of Prospective Owner, Prospective Remedial Party or Prospective Owner
Address	Street address or P.O. Box number for the Prospective Owner, Prospective Remedial Party, or Prospective Owner Representative.
Address	City, state and zip code for the Prospective Owner, Prospective Remedial Party, or Prospective Owner Representative.
Phon	Phone number for the Prospective Owner, Prospective Remedial Party or Prospective Owner Representative.
E-Mail	E-mail address of the Prospective Owner, Prospective Remedial Party or Prospective Owner Representative.

If the site is subject to an Environmental Easement, Deed Restriction, or Site Management Plan requiring periodic certification of institutional controls/engineering controls (IC/EC), indicate who will be the certifying party(ies). Attach additional sheets, if needed.

Certifying Party Name	Name of Certifying
Address	Certifying Party's street address or P.O. Box
Address	Certifying Party's city, state and zip
Phone	Certifying Party's Phone
E-Mail	Certifying Party's E-mail

Section VII Agreement to Notify DEC After Property Transfer/Sale

This section must be filled out for all property transfers of all or part of the site. If the site also has a CoC, then the CoC shall be transferred using DEC's form found at http://www.dec.ny.gov/chemical/54736.html

Filling out and signing this section of the form indicates you will comply with the post transfer notifications within the required timeframes specified on the form. If a CoC has been issued for the site, the DEC will allow 30 days for the post transfer notification so that the "Notice of CoC Transfer Form" and proof of it's filing can be included. Normally the required post transfer notification must be submitted within 15 day (per 375-1.11(d)(3)(ii)) when no CoC is involved.

Name	Current property owner must sign and date the form on the designated lines. Print owner's name on the line provided.
Address1	Current owner's street address.
Address2	Current owner's city, state and zip code.

Xerox Webster Change of Use
Attachment A – Project Description

I. INTRODUCTION AND PROPERTY DESCRIPTION

TM Montante Solar Development LLC (Montante) is proposing development of a commercial-scale photovoltaic (PV) generation facility (Project) within portions of tax parcel (065.02-1-40.11) owned by Xerox Corporation (Xerox) (See Figure 1). The Project would be located within northern unoccupied portions of Xerox Corporation's Webster campus, at 139 Caracas Drive (Tax Parcel Address: 750 Phillips Road), Town of Webster, Monroe County, New York. The Project is located north of Caracas Drive and would encompass an area of approximately 20 acres and includes currently undeveloped forest and herbaceous land. The Project area also includes two separate former landfill areas including an asphalt-capped inactive landfill and associated monitoring and remedial infrastructure within the southern portion of the Project and a separate soil-capped construction and demolition (C&D) landfill located further north. The site will be leased by Montante for the purposes of the construction and operation of the Project. A lease agreement has been established between Montante and Xerox (see Appendix M). The Project construction start date is planned for Spring 2026 (pending all applicable approvals) with site restoration and stabilization to follow during the same construction season.

The Project would be located within the larger Xerox Webster campus, which is a NYSDEC RCRA Facility (Site Code 828178) for identified groundwater and soil contamination currently managed under a RCRA Part 373 Hazardous Waste Management Permit (No. 8-2654-00064/00040). The Project is designed to minimize potential ground disturbances to the extent feasible.

The proposed development areas are currently comprised of vacant forested land and the capped C&D landfill in the north-central portion of the site, vacant forested lands in the west and southeastern portions of the Project site, and the asphalt-capped inactive landfill in the south-central portion of the site. The proposed development site is generally flat, with the exception of slightly higher-elevation topography associated with the C&D landfill which has localized grades between 5 and 10%.

II. PROPOSED PHOTOVOLTAIC SYSTEM

The proposed PV system is an approximate 6.6 megawatt (MW) direct current (DC), geoballast mounted, fixed-tilt system. The most up-to-date iteration of the civil plan set is attached (see Appendix A) which depicts the planned solar PV system, including proposed ballasted solar arrays, perimeter fence, utilities, and access. We understand that final site plans and as-built drawings will be required to be submitted, and these will be submitted to NYSDEC once available.

The proposed solar arrays will be installed on geoballast foundations that will sit flush on the ground surface (see attached specification sheet – Appendix B). No mass grading is proposed, and only minimal grading is needed for the planned permanent site access in the southeastern corner. Existing trees within the site will be hand-felled, cut flush to the ground with stumps left in place to minimize ground disturbance. Installation of the arrays on the geoballast foundations will limit disturbances to the ground and avoid penetration of the existing landfills and their respective cover systems (asphalt/soil cover) from foundation elements. The geoballast system can be adjusted to accommodate slopes up to 15%, while Project slopes are much less (localized grades up to 5-10% maximum). To the extent that the civil

plans depict the ballasts as being located within on-site drainage ditches or overlying bedrock recovery trenches, the ballasts will be field adjusted as necessary during construction to avoid direct placement within drainage ditches or overlying the bedrock recovery trenches.

The electrical/utility lines running from the arrays will be located within encased aboveground conduits which will not require ground disturbances. Grounding for the Project will be accomplished by keeping the entire grounding system above ground until it reaches the equipment pad in the southeastern corner of the site. Once at the equipment pad, the PV system will be connected to earth by a series of ground plates or ground rods which will be buried or driven into the ground. All grounding plates for the Project will be located at the equipment pad area and outside of the landfill limits. The point of interconnection is located at the southeast portion of the planned development where the facility will tie into an existing Rochester Gas and Electric (RG&E) overhead utility line. Project-specific draft electrical three-line drawings and an example/representative set of electrical drawings illustrating the intended installation methods and impact minimization measures are included as Appendix C. Final electrical drawings will be available following local approvals and will be provided once available. The Project will include a permanent 20-feet wide access driveway (with required fire equipment turnaround) in the southeastern corner of the site for access from Caracas Drive.

During construction, up to two temporary stabilized entrances may be established: one at the location of an existing driveway west of the asphalt-capped landfill, and another adjacent to the proposed permanent access driveway. These temporary entrances are not expected to require grading or excavation and would be installed as per the stabilized construction entrance detail outlined in the New York State Standards and Specifications for Erosion and Sediment Control.

The Project will include a 7-feet high chain link security fence installed around its perimeter. The northern, eastern and western portions of the existing fence along the perimeter of the asphalt-capped landfill would be removed, and fence poles would be cut flush to allow for installation of the system. If these fence poles are found to be hollow and containing soils, soils will be extracted to a minimum depth of 4-inches below grade, and the fence poles will be backfilled and sealed with asphalt or grout. Extracted soil (if any) will be characterized and managed as per conditions required in the Xerox Soil Management Plan. The southern portion of this existing asphalt-capped landfill fence will be tied into the solar development security fence. The replacement fence and remaining existing landfill fence will surround the Project and will continue to serve to restrict access from outsiders to the closed landfill areas. The majority of the perimeter security fence will be installed on ballasted poles in order to minimize ground penetrations. Two limited sections of the perimeter fence along the western edge of the C&D landfill are proposed to have driven fence posts in order to better secure the fence to the sloping contours in these locations. Prior to construction, existing utilities and site infrastructure throughout the site including existing gas lines, electrical lines, and monitoring wells will be marked in the field.

Sample wind and snow load calculations have been prepared and are included as Appendix D. The final snow and wind load calculation and civil and electrical plan sets will be submitted prior to the start of construction.

Stormwater Conditions

The Project site currently drains via sheet flow to unnamed tributaries of Fourmile Creek, with existing cover consisting of forested areas, gravel and dirt access roads, and about 3.5 acres of impervious surface (mostly associated with the existing asphalt-capped landfill). Following development, impervious

cover will increase to approximately 8.3 acres with solar panels, an equipment pad, and access infrastructure. Although minor increases in runoff may occur due to tree clearing and impervious area expansion, post-construction peak discharge rates remain within the NYSDEC 5% threshold. Stormwater will be managed using non-rooftop disconnection, a gravel diaphragm located north of the C&D landfill (which has localized grades up to approximately 8% slopes) to restore sheet flow, and vegetated filter strips, ensuring compliance with water quality and runoff reduction requirements while maintaining pre-development hydrology. Temporary construction Best Management Practices (BMPs) will include the use of filter socks, which are temporary sediment barriers designed to stop the flow of sediments and prevent potential for deposition off-site. Pre-and post-development stormwater runoff calculations are provided in Appendix E.

III. ENVIRONMENTAL CONSIDERATIONS

Except for portions of the existing asphalt-capped landfill security fence, existing remediation infrastructure within the development areas, including the two landfill caps, monitoring wells, groundwater pumping wells, and associated infrastructure, will remain in place and access maintained for operation and maintenance activities required by the Part 373 Permit. Planned ground disturbance is limited to areas outside the landfill limits (see additional details below). Details on the remedial elements and information to address potential environmental considerations associated with the Project development are provided herein.

Planned Ground Disturbance

The proposed PV system will require limited ground disturbance within the southeastern corner of the site associated with the six utility poles for interconnection (anticipated to be augered installations), the 20-feet wide site access driveway, underground electric lines, system grounding infrastructure, and utility pad. Depths of planned ground disturbance in these locations will be approximately 1 to 2 feet deep for the access road and utility pad and up to 6 feet deep for the underground utilities, grounding, and utility poles. Minor amounts of imported soil material may be brought into these areas to backfill excavations, fill augered holes, etc. No ground disturbance to either of the landfill caps is planned as part of the Project's construction activities.

Minor ground disturbance associated with the Project will also include installation of a gravel diaphragm located to the north and outside the limits of the C&D landfill area, to serve as a permanent stormwater management feature for the site. The gravel diaphragm will be installed in accordance with NYSDEC stormwater management guidance and will extend to an approximate depth of one foot below existing ground surface. In addition, temporary stormwater controls, including filter socks, will be installed (outside the C&D landfill limits) during active construction within the site. These filter socks will be secured with stakes to prevent surface water bypass. For this Project, staking depths will be limited to six inches or less to minimize soil disturbance. Temporary controls will be removed during the restoration phases of the Project and if needed, any minor disturbance to the existing ground cover will be supplemented with clean imported soil.

As needed, imported soil material will meet the backfill and cover soil testing and quality standards presented in DER-10 and NYSDEC Sampling, Analysis, and Assessment of Per- and Polyfluoroalkyl (PFAS) document. The material will need to meet the unrestricted use SCO in accordance with Table 375-6.8(a) in 6 NYCRR 375. Furthermore, we understand that imported materials (soils, gravel, etc.) will require a soil import request with either chemical testing (soils) and/or sieve analysis (non-soil materials including gravel) to be submitted to the NYSDEC for approval prior to use for fill on-site.

As part of the planned Project development, soil sampling will be completed in the limited areas of ground disturbance to characterize soils for proper management and screen for Xerox property contaminants of concern (COCs) as per the Xerox Soil Management Plan.

Groundwater in portions of the site subject to ground disturbance is estimated to be approximately 6 to 10 feet below ground surface, based on available historical groundwater elevation data. Given the anticipated depths of ground disturbance, it is unlikely that groundwater will be encountered. In the event it is encountered, groundwater will be managed in accordance with the Groundwater Management Plan for the facility.

Closed C&D Landfill

The closed C&D landfill encompasses an approximate 1.5-acre area within the north-central portion of the proposed solar site. The C&D landfill was reportedly operated for approximately 15 years and received mostly grass cuttings, leaves and brush debris. This landfill was reportedly closed under direction of the NYSDEC in 1988 and is designated Solid Waste Management Unit (SWMU) #88 – Former Disposal Area North of W200 under the Xerox program; this SWMU is classified as No Further Action Required. Landfill closure plans/details were prepared by Bergmann Associates in February 1988 and are attached for reference (Appendix F). These plans indicate that the C&D landfill is comprised of a variable thickness layer of landfill debris, which as part of its closure was capped with a minimum of 18-inches of low permeability clay cover and further topped with a minimum of 6-inches of topsoil. The landfill side slopes were graded to a maximum of 2/1 with a 3% minimum grade at the landfill center which was likely intended to minimize infiltration of surface water and direct surface water off the cap. The C&D landfill construction also included two 6-inch PVC landfill gas vents, though only the western vent remains functional as the eastern vent has since broken and filled in with leaf litter and soil. Xerox will repair the broken landfill gas vent prior to solar Project construction.

An investigation of the C&D landfill cover was completed in Fall 2025 to confirm the 2-foot depth of cover. The investigation (as detailed in the attached report included as Appendix G) identified the depth of the site soil cover to be consistent with the design documents. Both gas vents and existing groundwater wells (B-20 well cluster) within the western landfill limits will be protected in place (i.e. temporarily barricaded or marked with fencing, high-visibility flagging, or similar barriers) during construction activities. A 30-foot diameter open space will be maintained around the landfill vents and monitoring wells to allow for future access or maintenance.

Recent field observations of the landfill indicate that the vegetative cover has become well established with thick grass ground cover and sporadic shrubs and small tree saplings present throughout the capped landfill area. The grass cover will be maintained and supplemented as necessary throughout and following active construction to maintain the soil cap integrity. Woody-stem vegetation will be cut flush with the ground surface. Conditions will be monitored during inspection(s), and should stumps decay or subside over time, clean imported soil meeting the cap soil requirements and Site Management Plan will be placed as indicated in the Project Operations and Maintenance Manual (Appendix H).

The Project contractor will be responsible for inspecting and maintaining the vegetative integrity of the solar facility at the time of construction. The construction will be timed such that active construction will be planned for dry season (summertime) construction to the extent feasible. Throughout and following construction, weather forecasts will be consulted, and field inspections will be conducted to monitor for and plan for avoiding rutting, erosion, and soil compaction. Future mowing and vegetative maintenance

activities will similarly be planned for when the site is able to withstand these types of activities. Additional requirements for the ongoing maintenance and O&M of the landfill are detailed in the Project-Specific Operations and Maintenance plan (Appendix H).

There is no planned ground intrusive activity or regrading of surface soils within the footprint of the C&D landfill, and existing surface drainage patterns will be maintained. The C&D landfill cap was graded in such a way that rainfall and surface water appear to naturally flow to the west and northwest towards low-laying areas off-site. Project-specific erosion and sediment control plans are being developed to manage stormwater within the Project area. Low-ground pressure equipment will be utilized to reduce impacts to soil throughout the site, and the geoballast foundations and above ground electrical conduits will eliminate the need for ground disturbances in the landfill areas. The geoballast foundations can be carried into the site and filled with quarry rock once in place. The geoballasts would each measure approximately 2 feet wide by 7 feet in length by 2 feet in height. Typical weights of the geoballast foundations are 2,250 pounds per basket once filled, with typical spacing between the geoballasts of 11.25 feet.

Geotechnical evaluations were completed in Fall 2025 in the landfill areas on site (see reports included as Appendix I and Appendix J) to assess potential for settlement and confirm the suitability of the proposed foundation designs. Based on the results of the geotechnical evaluations, both landfills-the C&D Landfill and the Asphalt-Capped Inactive Landfill, as further described below, exhibit adequate cover design and/or surface bearing capacity to avoid or minimize the potential for settlement, slippage, or slope instability. Please refer to the attached geotechnical reports for a detailed summary of the analyses.

Asphalt-Capped Inactive Landfill and Associated Remedial Infrastructure

The asphalt-capped inactive landfill is a 2.8-acre closed landfill that operated between 1960 and 1971 and is designated Solid Waste Management Unit (SWMU) #62 – Inactive Landfill (Closed Solid Waste Management Facility). Xerox progressed through the regulatory phases for the landfill including Remedial Investigation, Feasibility Study, Remedial Design and Remedial Action implementation under Consent Order No. B800528410 (terminated in 2000), Site Code 828013, and is currently classified as Class 4. The site is now being managed under the corrective action portion of the Part 373 Permit for the Xerox Webster campus.

The remedial action plan for this landfill, which was implemented in 1990, involved the installation of a 4-inch asphalt cap (2.5 inches of NYSDOT Type 3 mixture compacted to 90% of maximum obtainable density and 1.5 inches of NYSDOT Type B Bituminous Pavement Mixture). According to the as-built drawings for this landfill (included as Appendix K), the asphalt is also reportedly underlain by a minimum of 6 to 8 inches of NYSDOT Type 1 subbase material and a further underlying geotextile fabric (Trevira 1145) separating the subbase from the underlying compacted earthen fill. The cap was placed over the entirety of the landfill to control infiltration and percolation. Based on available historical information, the underlying landfill is comprised of five parallel trenches excavated 4 feet wide by 4 feet deep spaced on 50-feet centers. The trenches were reported to be approximately 300 feet long and oriented in an east-west direction. Wastes disposed of in the landfill were categorized as “general” or “selenium”. The wastes were residues from former manufacturing operations and included various solid wastes and limited waste solvents. “General” wastes were reportedly typically placed in 12-inch-thick lifts, with each lift consisting of 9-inch layers of wastes covered within 3 inches of soil cover up to the top grade of the trench. The trenches were then reportedly capped with 2 feet of soil cover. Waste classified as

"selenium" were placed in 3-inch-thick layers which were lined on the bottom and sides with 6-inches of clay. Reportedly each 3-inch layer of waste was encapsulated with 6-inches of clay. The trenches were then capped with 6-inches of clay and approximately 2 feet of compacted soil cover. An approximate representation of a cross section of the landfill cover and across the landfill site is attached for reference (Figure 3).

The asphalt cap condition is monitored annually by Xerox as part of its Part 373 permit obligations and has been periodically re-sealed with surface cracks repaired, as needed. Given the reported design of the asphalt capped landfill (as described above) settlement from lightly loaded structures would not be anticipated, however, settlement could not be entirely excluded. Intrusive geotechnical data collection (e.g. geotechnical test borings, test pits, etc.) was not desired, therefore non-intrusive geotechnical testing was also completed for this landfill (see report included as Appendix J) to evaluate conditions relative to potential for settlement. As noted above, the results of the analysis indicated sufficient bearing capacity of the existing cap to limit settlement. Slippage and slope instability are not anticipated to be of concern for this landfill given the landfill grades and design. The Town of Webster, as part of preliminary site plan approvals, has also requested that the existing asphalt surface be supplemented with an additional 1.5 inches of asphalt prior to construction of the Project. This additional asphalt (laid on top of existing asphalt cover) is intended to minimize the need for future cap maintenance following installation of the Project. The installation of the additional asphalt cap would be completed in late Spring or early Summer 2026 ahead of construction of the solar Project on the asphalt cap areas. Annual monitoring and maintenance of the landfill cap will continue post-construction, and specific attention will be given to conditions surrounding the geoballasts (or alternative ballast, if implemented) to monitor for settlement and asphalt cracking. Details for monitoring and maintenance are provided in Appendix L.

The contamination resulting from this landfill was primarily low-level VOC concentrations in shallow and/or bedrock groundwater, which flows north at the site. A groundwater migration control system was installed as part of the remedy to the north (downgradient) of the landfill which consists of a 20-feet-deep blast-enhanced bedrock trench with two groundwater recovery wells. The recovered groundwater is discharged to the Town of Webster's publicly owned treatment works (POTW) under Xerox's Industrial Sewer Use Permit. Surface drainage patterns were also modified by installing drainage swales along the landfill perimeter to help control the flow of surface drainage. The swales within the Project site along the western and northern perimeter of this landfill (which have since developed wetland characteristics) will be maintained in pre-construction condition and will not be graded or in-filled as part of the Project development.

In addition to the active groundwater pumping wells and surface water control, several monitoring wells were installed to the north and east of the landfill. The monitoring wells and recovery wells on-site are routinely monitored as part of Xerox's campus-wide ongoing Part 373 Permit requirements. The Project has been designed so that a 30-feet diameter open space will remain around existing monitoring wells throughout the Project development areas. The arrays have been sited to allow for additional open space between the two recovery wells to facilitate access for future well or pump maintenance if necessary. A minimum of 12-foot spacing between Project array rows will be maintained to allow for future vehicle access to existing remedial systems and monitoring wells within the Project footprint.

LANDFILL GAS MANAGEMENT

Landfill gas management was not a requirement for the closure of the asphalt-capped hazardous waste landfill given the nature of the waste materials. As part of the C&D landfill closure, two PVC stick-up

vents were installed as a preemptive measure to allow for passive gas venting. As noted previously, one damaged vent will be repaired or replaced by Xerox prior to Project construction to maintain continued compliance. Project equipment and the majority of fencing will be ballast-supported, and no buried utilities are included within or near the landfill limits. Below-grade utilities for the Project are limited to the grounding plates and a short conduit segment beneath the permanent access driveway, which will be located well outside (approximately 575-feet away) from the C&D landfill footprint, effectively mitigating potential for landfill gas interaction or migration. Additionally, a 30-foot diameter open space will be maintained between the solar Project components and the gas vents, thereby minimizing the potential for interaction with vented landfill gas, if present. Given the separation between intrusive Project activities and the landfill boundaries, as well as the distance between Project components and landfill gas vents, landfill gas migration and receptor exposure are not anticipated.

The Project team will remain responsive to evolving site conditions and regulatory input. Any future intrusive work (only if needed) within the C&D landfill will follow Xerox Site Management procedures and be documented and reported to NYSDEC in accordance with applicable regulatory requirements. If warranted by a changed condition or agency feedback, additional landfill gas management measures, including monitoring or controls, can be evaluated and implemented in consultation with NYSDEC. The response procedures are further outlined in the Contingency Plan memorandum (see Appendix L).

SWMUs Located Outside and Adjacent to the Solar Development Footprint

Building 343 is located in the southern portion of the Project study area and was the location of SWMU #67, a former hazardous waste storage location which had closure activities implemented in 2022. NYSDEC has determined that no further closure requirements are required. The building and associated parking areas east of the building are located outside of the Project development areas (see Figure 2).

SWMU #79, also known as the Wailing Wall, is located to the northeast of the planned Project and approximately 50 feet outside of the development footprint (see Figure 2). The SWMU consists of a reinforced concrete wall within a 25-feet by 25-feet chain link fenced area with restricted access. The area was previously sampled and assessed and was classified as requiring no further remedial action by NYSDEC. Xerox has an approved work plan in place under the facility's Part 373 Permit to remove the concrete and fence and conduct soil sampling to formalize closure of the SWMU. Access to this area will be maintained for future implementation of the closure activities.

IV. OPERATION AND MAINTENANCE ROLES AND RESPONSIBILITIES

Montante will be responsible for the operations of the solar Project, vegetative maintenance within the Project, and maintenance of the perimeter security fence throughout the life of the Project. A Project-Specific Operation & Maintenance (O&M) Plan has been prepared and is attached as Appendix H. This plan outlines the specific roles and responsibilities of Xerox and Montante during post-construction operations, including inspection protocols, maintenance responsibilities, and reporting commitments. All existing environmental liabilities within the Project area, including landfill cap inspection and maintenance, groundwater monitoring and migration control, SWMU management, and soil and groundwater management, will be retained by Xerox. Xerox's landfill cap monitoring and maintenance requirements are included as Appendix L.

V. CONTINGENCY PLANS

The monitoring, contingency notification, and response procedures applicable to these landfills have been, or will be, incorporated into the Xerox Site Management Plan (SMP). The Xerox SMP is currently being updated for submittal to NYSDEC for review. To avoid duplication of requirements, a memorandum has been prepared that summarizes the applicable response and notification procedures for incorporation into the SMP. The specific contingency measures summarized from the SMP remain subject to revision and future approval by NYSDEC. Additional details are provided in Appendix L.

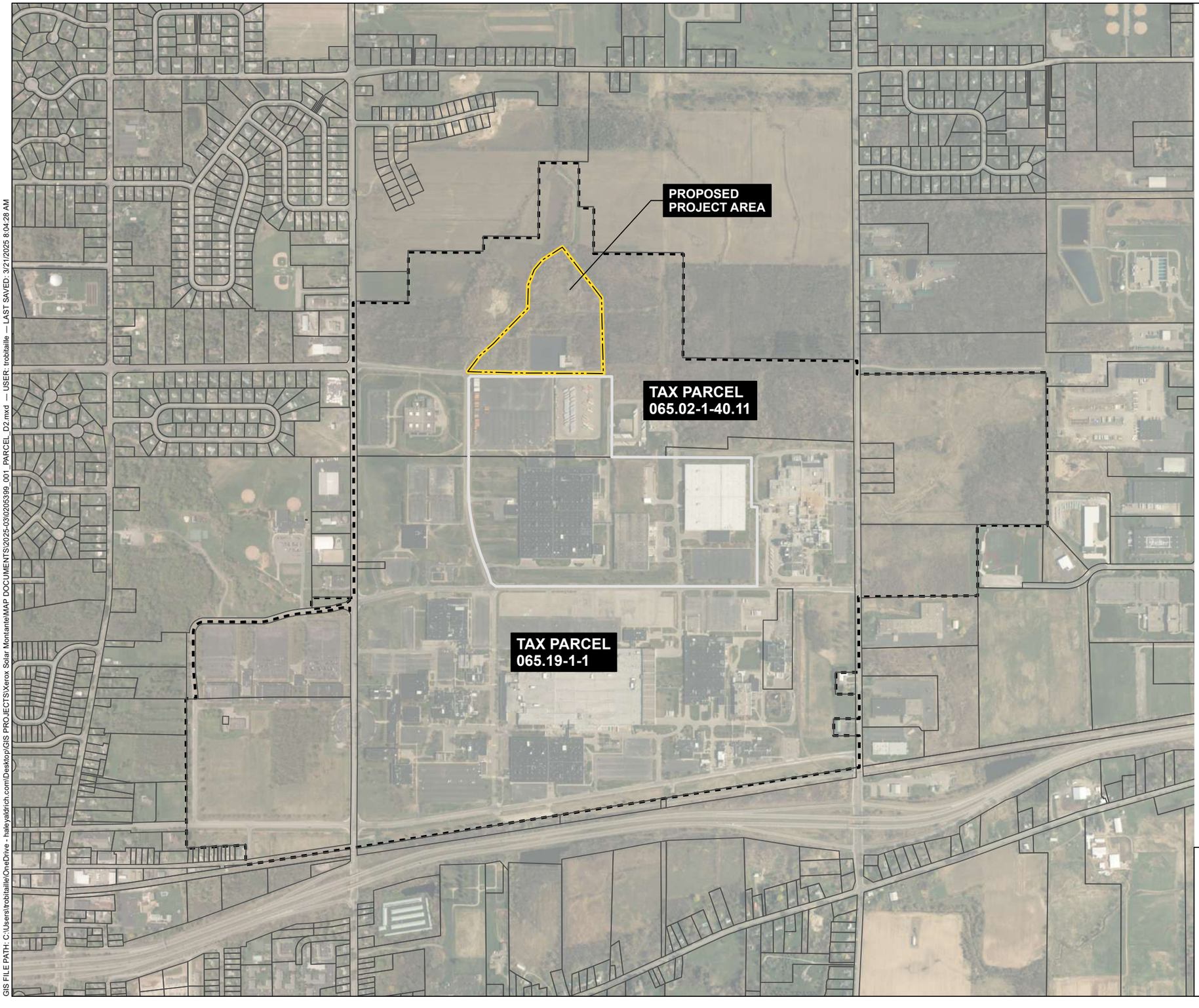
VI. PROJECT DECOMMISSIONING

Montante is currently developing a Project decommissioning plan, which will be a component of local site approvals and can be provided to the NYSDEC at a future date, as needed.

ATTACHMENTS

PROJECT PARCEL MAPPING – FIGURE 1
PRELIMINARY SITE PLAN – FIGURE 2 (SEE APPENDIX A FOR MOST UP-TO-DATE LAYOUT)
LANDFILL SKETCH/CROSS SECTION – FIGURE 3
APPENDIX A – CIVIL DESIGN PLANS (FEBRUARY 2026)
APPENDIX B - GEOBALLAST FOUNDATION SPECIFICATION SHEET
APPENDIX C – PROJECT SPECIFIC DRAFT THREE-LINE ELECTRICAL DRAWINGS AND EXAMPLE/REPRESENTATIVE ELECTRICAL DETAILS
APPENDIX D – SAMPLE SNOW/WIND LOAD CALCULATIONS
APPENDIX E – STORMWATER RUNOFF CALCULATIONS (PRE AND POST DEV)
APPENDIX F – C&D LANDFILL CLOSURE PLAN DRAWING
APPENDIX G – C&D LANDFILL COVER INVESTIGATION REPORT
APPENDIX H – PROJECT SPECIFIC O&M PLAN
APPENDIX I – C&D LANDFILL GEOTECHNICAL REPORT (REVISED FEBRUARY 2026)
APPENDIX J - ASPHALT LANDFILL GEOTECHNICAL REPORT (REVISED FEBRUARY 2026)
APPENDIX K – ASPHALT LANDFILL AS-BUILT DRAWINGS
APPENDIX L – LANDFILL CAP MONITORING, MAINTENANCE, AND CONTINGENCY PLAN REQUIREMENTS
APPENDIX M – LEASE AGREEMENT WITH AMENDMENT

FIGURES



LEGEND

- SOLAR PROJECT STUDY AREA
- TAX PARCELS
- PROPERTY NOT OWNED BY XEROX
- XEROX PROPERTY BOUNDARY

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY

N

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SCALE IN FEET

HALEY ALDRICH

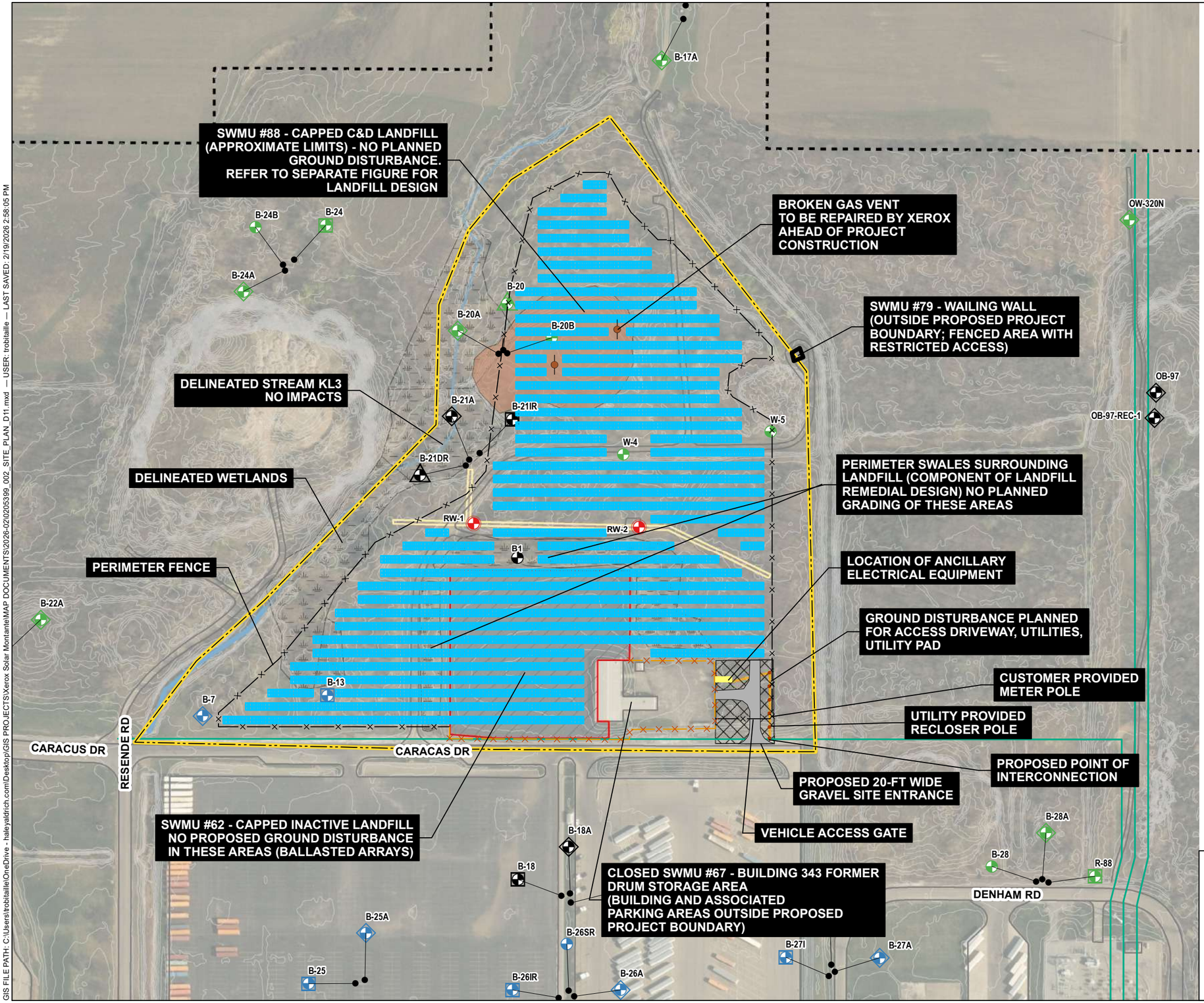
TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

PROJECT
PARCEL MAPPING

SCALE: AS SHOWN
MARCH 2025

FIGURE 1

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2025-0302050399_001_PARCEL_D2.mxd — USER: trobitaille — LAST SAVED: 3/21/2025 8:04:28 AM



LEGEND

- SOLAR PROJECT STUDY AREA
- PROPOSED SOLAR ARRAY
- XEROX PROPERTY BOUNDARY
- 1 FT TOPOGRAPHIC CONTOURS
- APPROXIMATE AREAS POTENTIAL SOIL DISTURBANCE
- PROPSOED FENCELINE (NEW)
- EXISTING FENCELINE TO REMAIN
- UTILITY EQUIPMENT PAD
- UTILITY POLES
- APPROX EXTENTS OF C&D LANDFILL
- CAPPED INACTIVE LANDFILL
- EXISTING TRANSMISSION LINES
- PROPOSED DRIVEWAY

MONITORING OR RECOVERY WELL INTERVAL

- DEEP
- INTERMEDIATE
- OVERBURDEN
- SHALLOW

OTHER SITE FEATURES

- GROUNDWATER PUMPING WELL
- GROUNDWATER QUALITY MONITORING (GQM) WELL
- TRIGGER WELL
- LINE OF COMPLIANCE WELL
- WELL LEADER LINES
- BLASTED TRENCH IN BEDROCK
- LANDFILL GAS VENTS
- DELINEATED WETLAND
- DELINEATED STREAM

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY
3. WELL LOCATION DATA SOURCE: XEROX CORPORATION
4. TOPOGRAPHIC DATA SOURCE: MONROE COUNTY LIDAR DOWNLOAD SITE
5. WETLAND/STREAM MAPPING SOURCE: DELINEATIONS COMPLETED BY H & A OF NEW YORK ENGINEERING AND GEOLOGY, LLP ON 22 OCTOBER 2024.

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SCALE IN FEET

HALEY ALDRICH

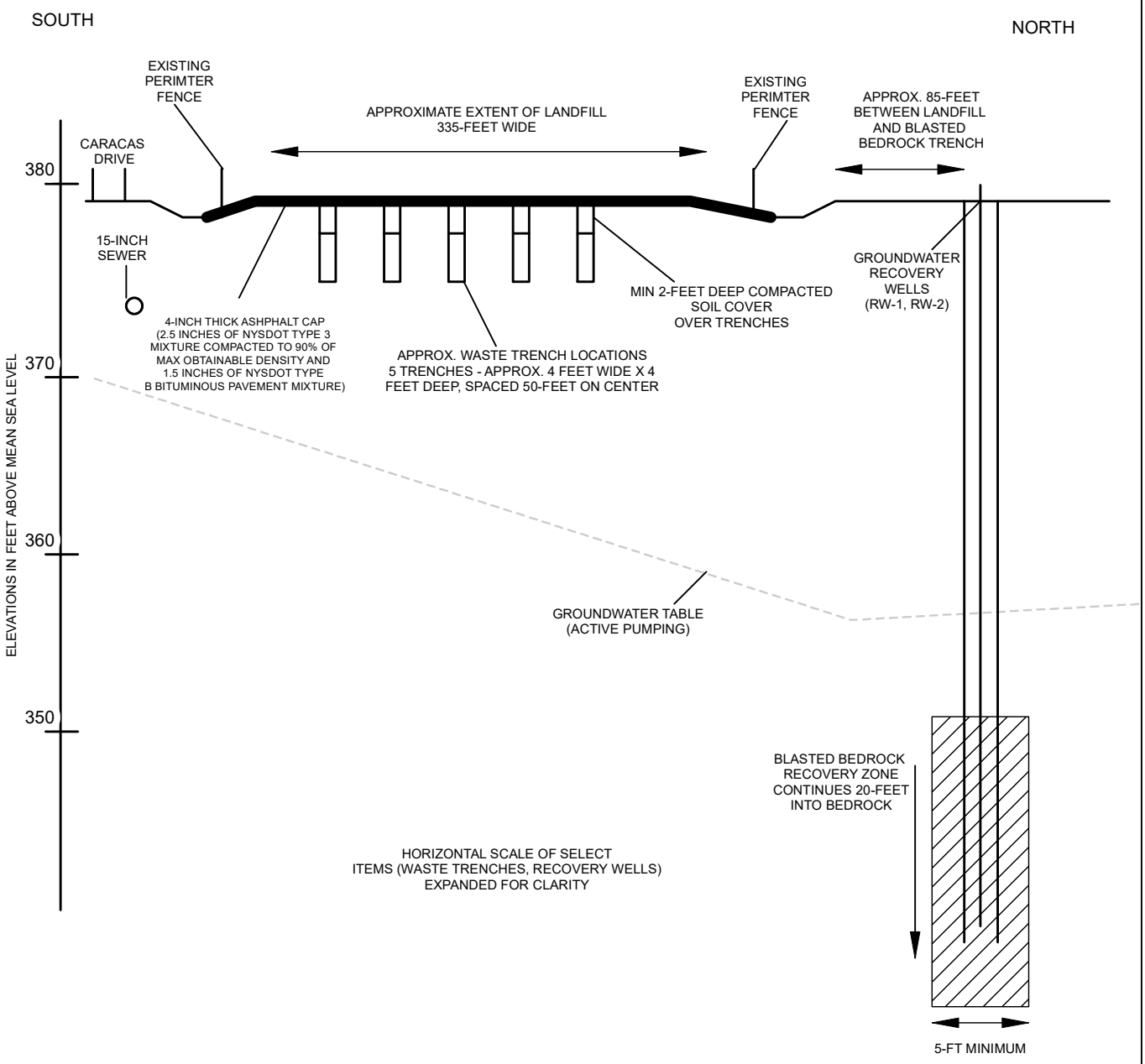
TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

PRELIMINARY SITE PLAN (SEE APPENDIX A FOR MOST UP-TO-DATE LAYOUT)

SCALE: AS SHOWN
LAST UPDATED: FEBRUARY 2026

FIGURE 2

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2026-02\20260205399_002_SITE_PLAN_D11.mxd — USER: trobitaille — LAST SAVED: 2/19/2026 2:58:05 PM



NOTES:

1) DIMENSIONS AND CROSS SECTION DETAILS ARE BASED ON THE RECORD OF DECISION DOCUMENT FOR INACTIVE LANDFILL SITE, XEROX WEBSTER CORPORATION, AND NORTH SOUTH CROSS SECTION A - A', ALTERNATIVE 7, PREPARED BY WOODWARD-CLYDE CONSULTANTS, DATED 22 FEBRUARY 1988.

2. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE AND ARE SHOWN FOR ILLUSTRATION PURPOSES ONLY.

HALEY ALDRICH

TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

LANDFILL SKETCH/CROSS-SECTION

PROFILE DRAWING NOT TO SCALE
MARCH 2025

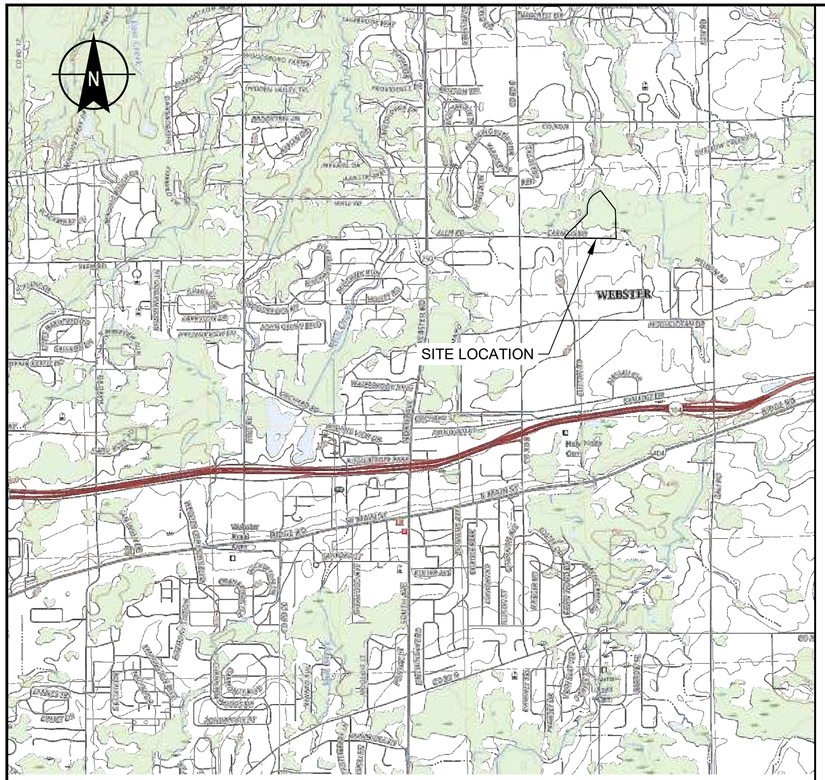
FIGURE 3

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2025-03\0205399_002_CROSSSECTION_D2.mxd — USER: trobitaille — LAST SAVED: 3/21/2025 3:01:22 PM

APPENDICES

APPENDIX A
CIVIL DESIGN PLANS (FEBRUARY 2026)

WORK IN PROGRESS - V.1



SITE LOCUS
TOPO SOURCE: USGS TOPOGRAPHIC MAP, WEBSTER, NEW YORK QUADRANGLE (2023).

0 2000 4000
SCALE IN FEET



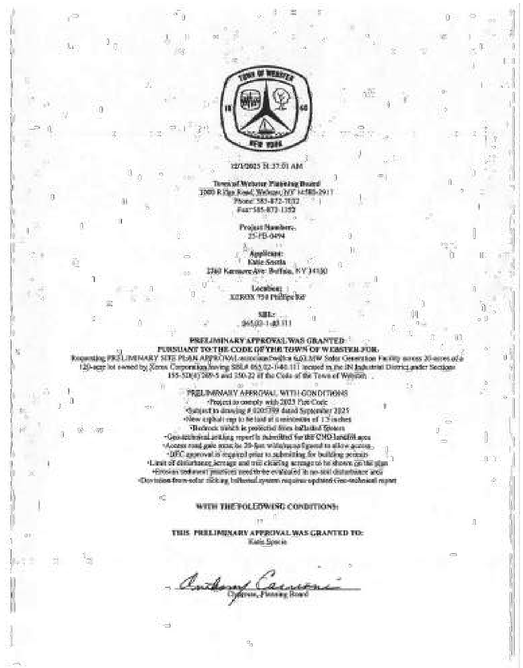
SITE AERIAL
MAP SOURCE: 2023 MICROSOFT CORPORATION © 2023 MAXAR © CNES (2023) DISTRIBUTION AIRBUS DS

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SCALE IN FEET

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MONTANTE XEROX SOLAR DEVELOPMENT

139 CARACAS DRIVE WEBSTER, NY



12/1/2025 11:37:01 AM
Town of Webster Planning Board
1000 Ridge Road, Webster, NY 14580-2917
Phone: 585-872-7033
Fax: 585-872-1352

Project Number:
25-PB-0494

Applicant:
Katie Sorola
2760 Kenmore Ave Buffalo, NY 14150

Location:
65 West XEROX 750 Phillips Rd

SBL:
965.00-1-40.111

PRELIMINARY APPROVAL WAS GRANTED
PURSUANT TO THE CODE OF THE TOWN OF WEBSTER FOR:
Requesting PRELIMINARY SITE PLAN APPROVAL associated with a 6.03 MW Solar Generation Facility across 20 acres of a 120-acre lot owned by Xerox Corporation having SBL# 065.02-1-40.111 located in the IN Industrial District under Sections 185-52(A)(3)(b)9-5 and 350-22 of the Code of the Town of Webster.

PRELIMINARY APPROVAL WITH CONDITIONS
- Project to comply with 2023 Fire Code
- Subject to drawing # 0205399 dated September 2025
- New asphalt ramp to be laid at a minimum of 12 inches
- Bollards which is provided from behind Shown
- Geo-technical testing report is submitted for the CMO landfill area
- Access road gate must be 20-feet wide/must expand to allow passing
- DEC approval is required prior to submitting for building permits
- Limit of disturbance acreage and tree clearing acreage to be shown on the plan
- Erosion control practices must be evaluated if in-situ disturbance area
- Deviation from solar sticking technical system requires updated Geo-technical report

WITH THE FOLLOWING CONDITIONS:

THIS PRELIMINARY APPROVAL WAS GRANTED TO:
Katie Sorola

Chairman, Planning Board



1/7/2026 2:59:36 PM
Town of Webster Planning Board
1000 Ridge Road, Webster, NY 14580-2917
Phone: 585-872-7033
Fax: 585-872-1352

Project Number:
25-PB-0509

Applicant:
Katie Sorola
2760 Kenmore Ave Buffalo, NY 14150
Location:
65 West XEROX 750 Phillips Rd
SBL:
965.00-1-40.111

PURSUANT TO THE CODE OF THE TOWN OF WEBSTER
FINAL APPROVAL CHECKLIST
1. Subject to PSC comments.
2. Subject to Parks and Recreation fees (if applicable)
3. Subject to Preliminary Approval Conditions.
4. Subject to all applicable governmental fees.
5. Subject to Department of Public Works approval
6. Significant construction shall occur within one year, as deemed by the Planning Board, to expire on: 1.6.27.
7. The conditions of Preliminary and Final approval are depicted on the cover page of the final designed plans.
8. The Engineer for the proposed project shall provide a Letter of Certification that all proposed work was completed, as per Planning Board resolution of final approval before a Certificate of Occupancy will be issued.
9. A Letter of Credit to the Town for this project shall include the fee for the Engineer's final certification inspection of the site.
10. Subject to resolution of the final approved minutes.
11. Must comply with all preliminary approval conditions
12. Vegetation and tree netting fences shall be added to the plans; removal shall be limited to surface level only (no soil disturbance).
13. Timber mats to be used in wetland delineated areas during construction
14. Drawing notes to call out and note to protect the bedrock trench system, fencing, and bollard array installation.
15. The applicant shall enter into a Host Community Agreement (HCA) with the Town of Webster to the Town Board's satisfaction.
16. The site plan should clearly show both the landfill owners and include the associated landfill, hazardous waste site and BCDA site numbers on the final drawing.
17. Decommissioning Bond must be Grade 'A' or higher.
18. DEC approval is required prior to submitting for building permits.

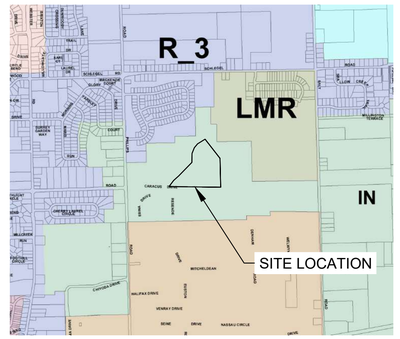
Recommendation for issuance of an Industrial Use Permit provided to Town Board

THIS APPROVAL WAS GRANTED TO:

Katie Sorola

Chairman

R-1	Single Family Residential District
R-2	Single Family Residential District
R-3	Single Family Residential District
LL	Large Lot Single Family Residential District
WD	Waterfront Development District
MHR	Medium High Residential District
LMR	Low Medium Residential District
LC-1	Class 1 Neighborhood Commercial District
LC-2	Class 2 Low Intensity Commercial District
MC	Medium Intensity Commercial District
HC	High Intensity Commercial District
CO	Commercial Outdoor Storage District
IN	Industrial District
OP	Office Park



ZONING MAP
NOT TO SCALE

NOTE: SEQRA DETERMINATION (TYPE II)

DRAWING INDEX

01	T-100	TITLE SHEET
02	G-100	NOTES
03	C-100	EXISTING CONDITIONS
04	C-200	PROPOSED CONDITIONS
05	C-201	EROSION AND SEDIMENT CONTROLS PLAN
06	C-202	PROPOSED ACCESS ROAD GRADING PLAN
07	C-203	PHASING PLAN
08	C-300	DETAILS
09	C-301	DETAILS

	By:
Department of Community Development	Title:
	Date:
Town Assessor	By:
	Title:
	Date:
Planning Board Chairman	By:
	Title:
	Date:
Monroe County Water Authority	By:
	Title:
	Date:

Entity	Approval	
Town Engineer	By:	
	Title:	
	Date:	
Fire Marshal	By:	
	Title:	
	Date:	
Webster Sewer District	By:	
	Title:	
	Date:	
Monroe County Department of Health	By:	
	Title:	
	Date:	

HALEY ALDRICH

H & A OF NEW YORK
ENGINEERING AND GEOLOGY, LLP
260 E. Main Street, Suite 2100
Rochester, NY 14604
Tel: 585.359.9000
www.haleyaldrich.com

PREPARED FOR:

TM MONTANTE SOLAR DEVELOPMENT, LLC

2760 KENMORE AVE #101
TONAWANDA, NY14150

THIS IS A COLOR DRAWING. INFORMATION WILL BE LOST IF REPRODUCED IN BLACK AND WHITE.

DRAFT

Project No.:	0205399
Scale:	SHOWN
Date:	FEBRUARY 2026
Drawn By:	SRG, JC
Designed By:	SRG, JC
Checked By:	SRG, ND
Approved By:	ND
Stamp:	

Rev.	Description	By	Date
H	FOR REVIEW	H&A	02/24/26
G	FOR REVIEW	H&A	02/19/26
F	FOR REVIEW	H&A	01/16/26
E	FOR REVIEW	H&A	12/18/25
D	FOR REVIEW	H&A	10/02/25
C	FOR REVIEW	H&A	09/17/25
B	FOR REVIEW	H&A	08/27/25
A	FOR REVIEW	H&A	08/11/25

MONTANTE XEROX SOLAR DEVELOPMENT

139 CARACAS DRIVE
WEBSTER, NY

TITLE SHEET

T-100

Sheet: 01 of 11

WORK IN PROGRESS - V.1

GENERAL NOTES

1. THESE DRAWINGS HAVE BEEN PREPARED FOR CONSTRUCTION OF A SOLAR ARRAY WITHIN PORTIONS OF THE XEROX CORPORATION CAMPUS IN WEBSTER, NEW YORK.
2. THE PROJECT IS LOCATED WITHIN THE LARGER XEROX WEBSTER CAMPUS, WHICH IS A NYSDEC RCRA FACILITY (SITE CODE 828178) FOR IDENTIFIED GROUNDWATER AND SOIL CONTAMINATION CURRENTLY MANAGED UNDER A RCRA PART 373 HAZARDOUS WASTE MANAGEMENT PERMIT (NO. 8-2654-00064/00040).
3. WATERCOURSES AND WETLANDS FIELD DELINEATED BY HALEY & ALDRICH OF NEW YORK ENGINEERING AND GEOLOGY, LLP (HALEY & ALDRICH OF NEW YORK) MAY 10, 2024.
4. EXISTING TOPOGRAPHIC CONTOURS FROM SURVEY REPORT BY TERRATEK GPS MODELING WHICH WAS PROVIDED TO HALEY & ALDRICH BY MONTANTE SOLAR DEVELOPMENT, LLC JULY 21, 2025.
5. TAX PARCEL BOUNDARIES FROM MONROE COUNTY REAL PROPERTY DATABASE, COURTESY OF MONROE COUNTY OFFICE OF REAL PROPERTY.
6. EXISTING CONDITIONS ARE A COMBINATION OF CAD DRAWINGS PROVIDED BY XEROX, GIS FILES FROM H&A STREAM AND WETLAND DELINEATIONS, AND AERIAL IMAGERY.
7. EXISTING CONDITIONS ARE APPROXIMATE.
8. PROPOSED CONDITIONS TAKEN FROM CAD DRAWINGS PROVIDED BY MONTANTE SOLAR DEVELOPMENT, LLC JULY 21, 2025.
9. PROJECT TO CONFORM TO 2025 NYS FIRE CODE.

CONSTRUCTION SEQUENCE FOR GRADING AND EROSION CONTROL

1. VERIFY THAT ALL APPLICABLE PERMITS AND PERMISSIONS HAVE BEEN OBTAINED.
2. OWNER TO HOLD A PRE-CONSTRUCTION MEETING PRIOR TO START OF CONSTRUCTION.
3. LOCATE AND DELINEATE EXISTING UTILITIES.
4. CONSTRUCT SITE ACCESS AND INSTALL ASSOCIATED EROSION AND SEDIMENT CONTROLS.
5. IF APPLICABLE, INSTALL CONSTRUCTION SAFETY FENCING AROUND THE STAGING AREA(S) AND AS DIRECTED BY THE OWNER.
6. INSTALL STABILIZED CONSTRUCTION ENTRANCE AS DIRECTED BY THE OWNER. EXISTING ENTRANCES WHICH MEET THE REQUIREMENTS OF A STABILIZED CONSTRUCTION ENTRANCE MAY MAY BE UTILIZED AS AVAILABLE. EXISTING ENTRANCES TO BE UPGRADED WITH ADDITIONAL GRAVEL AS NEEDED. UTILIZE ENTRANCE FOR ALL CONSTRUCTION VEHICLES ENTERING/EXITING SITE FOR THE DURATION OF THE CONSTRUCTION.
7. INSTALL PERIMETER EROSION AND SEDIMENT CONTROLS.
8. ALL EROSION CONTROLS WITHIN THE CURRENT ACTIVE CONSTRUCTION AREAS ARE TO BE INSPECTED FOR DEFICIENCIES BY THE CONTRACTOR ON A DAILY BASIS FROM INSTALLATION THROUGH THE DURATION OF THE PROJECT. THE EROSION CONTROLS ARE TO BE FUNCTIONING AS INTENDED OR REPAIRED/REPLACED AS NECESSARY. ADDITIONAL REQUESTS FOR REPAIR/REPLACEMENT MAY COME FROM THE ENVIRONMENTAL INSPECTOR OR THE OWNER.
9. PROTECT TREES AND VEGETATION TO REMAIN.
10. CONDUCT TREE CLEARING WITHIN THE LIMITS OF DISTURBANCE AND AS DIRECTED BY THE OWNER. FELLED TREES ARE TO BE REMOVED FROM THE SITE AS DIRECTED BY THE OWNER.
11. TREES ARE TO BE CUT TO GRADE WITH HAND FELLING. TREE STUMPS TO REMAIN IN PLACE. STUMPS LOCATED WITHIN C&D LANDFILL AREA MAY BE REMOVED OR GROUND BELOW GRADE IF REQUIRED BY NYSDEC IN ACCORDANCE WITH SEPARATE NYSDEC PROJECT APPROVALS.
12. CONTINUE TO MAINTAIN AND MONITOR ALL SITE EROSION AND SEDIMENT CONTROLS DAILY.
13. CONDUCT EARTHWORK ACTIVITIES FOR INSTALLATION OF EQUIPMENT PAD AND POWER POLES. EARTHWORK TO REMAIN WITHIN THE APPROVED BOUNDARIES OF THE PROJECT (LIMITS OF DISTURBANCE).
14. SOILS FROM STABILIZED ENTRANCE CONSTRUCTION, EQUIPMENT PAD INSTALLATION, AND POWER POLE INSTALLATION ARE TO BE STOCKPILED, COVERED, SAMPLED, AND STABILIZED ACCORDING TO XEROX SITE MANAGEMENT PLAN REQUIREMENTS AND THE OWNER'S DIRECTION.
15. COMPLETE FINAL GRADING, PERMANENT SEEDING, TOPSOILING AND MULCHING OF ALL DISTURBED AREAS.
16. UPON COMPLETE STABILIZATION OF DISTURBED SITE (VEGETATIVE COVER = 80%), ALL TEMPORARY EROSION AND SEDIMENT CONTROLS ARE TO BE REMOVED AND DISTURBANCE RESTORED.

SOIL RESTORATION

1. SOIL RESTORATION IS TO OCCUR IN LAYDOWN AREAS AND AREAS RECEIVING HIGH VOLUMES OF CONSTRUCTION TRAFFIC.
2. SOIL RESTORATION IS TO BE CONDUCTED DURING FINAL SITE STABILIZATION.
3. DURING PERIODS OF LOW TO MODERATE SUBSOIL MOISTURE, THE DISTURBED SUBSOILS ARE RETURNED TO ROUGH GRADE AND THE FOLLOWING SOIL RESTORATION STEPS APPLIED:

3.1. APPLY 3 INCHES OF COMPOST OVER SUBSOIL. COMPOST SHALL BE WELL DECOMPOSED (MATURED AT LEAST 3 MONTHS), WEED-FREE, ORGANIC MATTER. IT SHALL BE AEROBICALLY COMPOSTED, POSSESS NO OBJECTIONABLE ODORS, AND CONTAIN LESS THAN 1%, BY DRY WEIGHT, OF MAN-MADE, FOREIGN MATTER. THEY PHYSICAL PARAMETERS OF THE COMPOST SHALL MEET THE STANDARDS SET FORTH IN THE NOVEMBER 2016 NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL.

3.2. TILL COMPOST INTO SUBSOIL TO A DEPTH OF AT LEAST 12 INCHES USING A CAT-MOUNTED RIPPER, TRACTOR MOUNTED DISC, OR TILLER, TO MIX AND CIRCULATE AIR AND COMPOST INTO THE SUBSOIL.

3.3. ROCK-PICK UNTIL UPLIFTED STONE/ROCK MATERIALS OF FOUR INCHES OR LARGER SIZE ARE CLEANED OFF THE RESTORATION AREAS.

3.4. APPLY TOPSOIL TO A DEPTH OF 6 INCHES.

3.5. VEGETATE AS REQUIRED BY THE SEEDING PLAN.

3.6. TOPSOIL MAY BE MANUFACTURED AS A MIXTURE OR A MINERAL COMPONENT AND ORGANIC MATERIAL SUCH AS COMPOST. AT THE END OF THE PROJECT AN INSPECTOR SHOULD BE ABLE TO PUSH A $\frac{5}{8}$ INCH METAL BAR 12 INCHES INTO THE SOIL JUST WITH BODY WEIGHT. THIS SHOULD NOT BE PERFORMED WITHIN THE DRIP LINE OF ANY EXISTING TREES OR OVER UTILITY INSTALLATIONS THAT ARE WITHIN 24 INCHES OF THE SURFACE.

SITE STABILIZATION

1. INITIATE SITE STABILIZATION IMMEDIATELY (NO LATER THAN 1 BUSINESS DAY) IN ANY AREAS OF EXPOSED SOIL WHERE CONSTRUCTION ACTIVITIES HAVE PERMANENTLY CEASED OR WILL BE TEMPORARILY INACTIVE FOR MORE THAN 14 CALENDAR DAYS.
2. COMPLETE SITE STABILIZATION. FINALIZE THE INSTALLATION OF STABILIZATION MEASURES AS SOON AS PRACTICABLE, BUT NO LATER THAN 14 DAYS AFTER CONSTRUCTION ACTIVITIES HAVE PERMANENTLY CEASED (STABILIZATION INITIATED) (SEE ABOVE), REGARDLESS OF AREA DISTURBED.

MULCHING

1. IMMEDIATELY FOLLOWING TEMPORARY/PERMANENT SEED PLACEMENT, STRAW MULCH IS TO BE APPLIED AT A RATE OF 3 TONS PER ACRE OVER DISTURBED AREAS. STRAW MULCH COVERAGE RATES ARE EXPECTED TO PROVIDE A THICKNESS OF 1/4" - 1/2". EXCESSIVE MULCH DEPTH WILL IMPEDE VEGETATIVE GROWTH AND MAY REQUIRED THE REMOVAL OF EXCESS MULCH AS DETERMINED BY THE OWNER AND ENVIRONMENTAL INSPECTOR. MULCHING IS TO BE USED WITH SEEDING AS A COMPONENT OF SITE STABILIZATION. ADDITIONAL MULCH WILL BE REQUIRED IF PROPER COVERAGE (BARE SPOTS) HAS NOT BEEN ACHIEVED.
2. MULCH APPLIED FOR WINTER SEEDING (OCTOBER 15 THROUGH MARCH 31) SHALL BE INCREASED TO 4 TONS PER ACRE.
3. AS AN ALTERNATIVE TO STRAW MULCHING, WOOD FIBER HYDROMULCH MAY BE APPLIED AS SPECIFIED BY MANUFACTURER'S RECOMMENDATIONS OR AT A MINIMUM RATE OF 2000 LBS. PER ACRE (45 LBS. PER 1,000 S.F.) WITH A GUAR TACKIFIER (OR EQUIVALENT) AT 60 LBS. ACRE. SOIL AMENDMENTS MAY BE APPLIED WITH MULCH AT THE SPECIFIED APPLICATION RATES. WATER VOLUME IS TO BE DETERMINED BY THE MANUFACTURER. HYDROMULCH IS NOT TO BE APPLIED DURING RAIN. HYDROMULCH APPLICATION IS NOT TO BE APPLIED IF RAIN IS FORECAST WITHIN THE NEXT 2 DAYS. A CURING PERIOD OF 24 HOURS AND A MINIMUM SOIL TEMPERATURE OF 45 DEGREES ARE REQUIRED.

SPECIFICATION NOTES FOR TOPSOILING:

SITE PREPARATION

1. COMPLETE GRADING TO SUBBASE AND ALLOW FOR FULL DEPTH OF TOPSOIL TO BE ADDED TO ACHIEVE FINAL GRADE.
2. SCARIFY ALL COMPACTED SUBSOIL AREAS.

APPLICATION AND GRADING

1. TOPSOIL SHALL BE DISTRIBUTED EVENLY TO THE UNIFORM DEPTH THAT WAS REMOVED FOR STOCKPILING OR 6" MINIMUM, OVER THE RESTORATION AREA. THE TOPSOIL SHALL NOT BE PLACED WHEN IT IS PARTLY FROZEN, MUDDY, OR ON FROZEN SLOPES OR OVER ICE, SNOW, OR STANDING WATER PUDDLES.
2. TOPSOIL PLACED AND GRADED ON SLOPES STEEPER THAN 3H:1V SHALL BE STABILIZED IMMEDIATELY: FERTILIZED, SEEDED, MULCHED AND TRACKED WITH METAL CLEATED EQUIPMENT TO KEY IN MULCH AND ENCOURAGE COLLECTING SMALL POCKETS OF WATER ALONG THE SLOPE FOR SEED GROWTH. TRACKING SHALL BE PARALLEL TO GRADE (UP & DOWN SLOPES). SLOPES FLATTER THAN 3H:1V SHALL BE STABILIZED AS SOON AS POSSIBLE BUT NO LATER THAN 14 DAYS.

EROSION AND SEDIMENT CONTROL NOTES

1. ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IN ACCORDANCE WITH THE APPROVED PERMIT DOCUMENTS.
2. ALL SITE ACTIVITIES SHALL TAKE PLACE WITHIN THE LIMITS OF DISTURBANCE AS SHOWN ON THE DESIGN PLANS.
3. THE CONTRACTOR SHALL INSPECT, MAINTAIN AND REPAIR/REPLACE EROSION AND SEDIMENT CONTROLS DAILY AND CONTINUOUSLY THROUGHOUT THE DURATION OF THIS PROJECT.
4. THE PROPOSED EROSION AND SEDIMENT CONTROLS AS INDICATED ON THE DESIGN PLANS SHALL BE CONSTRUED AS THE MINIMUM REQUIRED STANDARDS. ADDITIONAL CONTROLS ARE TO BE PROVIDED AS NECESSARY TO PREVENT SEDIMENT LADEN DISCHARGES FROM LEAVING THE SITE.
5. THE CONTRACTOR SHALL CONTINUOUSLY MONITOR THE INSTALLED CONTROLS FOR FUNCTIONALITY AND PROVIDE ADDITIONAL MEASURES AS DIRECTED BY THE OWNER OR ENVIRONMENTAL INSPECTOR. THE CONTRACTOR SHALL MAINTAIN AND REPAIR/REPLACE ALL EROSION AND SEDIMENT CONTROLS UPON DAMAGE OR FAILURE TO FUNCTION AS INTENDED AND UNTIL SITE CONSTRUCTION HAS BEEN COMPLETED.
6. ALL STOCKPILED SOILS ON SITE SHALL RECEIVE SEDIMENT FILTER SOCK ALONG THE DOWNGRADE AREAS.
7. ALL SOIL STOCKPILES SHALL BE STABILIZED OR REMOVED FROM THE SITE AS DIRECTED BY THE OWNER.
8. ALL EXISTING INLETS ADJACENT TO OR ON SITE SHALL BE PROTECTED FROM SEDIMENT WITH INLET PROTECTION MEASURES.
9. THE CONTRACTOR SHALL ENSURE THAT STABILIZED CONSTRUCTION ENTRANCE IS BEING UTILIZED. THE CONTRACTOR SHALL BE RESPONSIBLE TO PREVENT SEDIMENT AND TRACKED MUD/DIRT ONTO THE PUBLIC RIGHT OF WAY, ROAD OR ACROSS PRIVATE DRIVEWAYS.
10. CONTRACTOR SHALL INSPECT DAILY, SWEEP AND CLEAN STREETS / DRIVES AS NECESSARY TO MAINTAIN CLEAN AND CLEAR SURFACES. LARGE STONES, ACCUMULATED GRAVEL AND/OR MUD THAT MAY IMPACT PASSING VEHICLES SHALL BE CLEANED IMMEDIATELY.
11. ALL ACCUMULATED SEDIMENT SHALL BE REMOVED FROM SEDIMENT FILTER SOCKS AND EROSION CONTROLS WHEN 50% OF AVAILABLE STORAGE AREA HAS BEEN ACHIEVED OR WHEN THE THE DEVICE NO LONGER FUNCTIONS AS INTENDED OR SEDIMENT LADEN WATER IS PASSING THROUGH OR AROUND THE CONTROL. SILT FENCE THAT IS TORN, HAS BROKEN STAKES OR IS BOWING OUT DUE TO ACCUMULATED SOILS SHALL BE REPAIRED, REPLACED OR CLEANED AND RESET.
12. ALL EROSION CONTROLS SHALL BE INSPECTED AND CLEANED OF SEDIMENT AS NECESSARY TO ENSURE NO DISCHARGE OF SEDIMENT OFF THE PROPERTY OR OUTSIDE OF LIMITS OF DISTURBANCE IS TAKING PLACE.
13. DUST CONTROL SHALL BE EMPLOYED THROUGH WATER SPRAYING AS NECESSARY TO MINIMIZE AIRBORNE DUST AND WIND EROSION.
14. ALL EXCESS EXCAVATED MATERIAL, BRUSH / TIMBER, AND CONSTRUCTION DEBRIS SHALL BE DISPOSED OF AS PER THE OWNER'S PREFERENCE. CONSTRUCTION DEBRIS SHALL BE ADDRESSED AS PER THE OWNER'S DIRECTION OR HAULED OFFSITE TO A LICENSED LANDFILL, AS NECESSARY.
15. SOIL STOCKPILES TO BE LOCATED ON SITE WITHIN THE LIMITS OF DISTURBANCE UNLESS DIRECTED OTHERWISE BY THE OWNER OR NOTED ELSEWHERE ON THE CONSTRUCTION DRAWINGS.
16. UNDER NO CIRCUMSTANCES SHALL SILT LADEN WATER BE DISCHARGED FROM TRENCHES / EXCAVATIONS OR RELEASED FROM HYDROSTATIC TESTING DIRECTLY TO ANY WETLAND OR WATER BODY. ANY GROUNDWATER THAT IS ENCOUNTERED WILL NEED TO BE HANDLED AS PER XEROX SITE MANAGEMENT PLAN REQUIREMENTS.

CONSTRUCTION NOTES

1. MINIMIZE CUT/FILL FOR ACCESS TO SITE TO THE GREATEST EXTENT PRACTICABLE.
2. IN ACCORDANCE WITH UNITED STATES ARMY CORPS OF ENGINEERS WETLAND PERMIT CONDITIONS, HEAVY EQUIPMENT WORKING IN WETLANDS MUST BE PLACED ON MATS OR OTHER MEASUREMENTS MUST BE TAKEN TO MINIMIZE SOIL DISTURBANCE.
3. WITH THE EXCEPTION OF MINIMAL GROUND DISTURBANCE ASSOCIATED WITH INSTALLING TEMPORARY EROSION AND SEDIMENT CONTROLS (I.E., STAKING FILTER SOCKS TO A MAXIMUM DEPTH OF 6 INCHES) WITHIN THE C&D LANDFILL, NO SOIL EXCAVATION, GRADING, OR GROUND PENETRATIONS ARE PERMISSIBLE IN THE TWO LANDFILL AREAS DENOTED ON THE SITE PLANS. THE PERIMETER OF THE C&D LANDFILL IS APPROXIMATE AND NEARBY/ADJACENT AREAS SHALL BE TREATED THE SAME.
4. PORTIONS OF THE EXISTING FENCE ALONG THE PERIMETER OF THE ASPHALT-CAPPED LANDFILL WILL BE REMOVED, AND FENCE POLES WILL BE CUT FLUSH TO ALLOW FOR INSTALLATION OF THE SYSTEM.
5. EXCEPT FOR PORTIONS OF THE EXISTING ASPHALT-CAPPED LANDFILL SECURITY FENCE, EXISTING REMEDIATION INFRASTRUCTURE WITHIN THE DEVELOPMENT AREAS, INCLUDING THE TWO LANDFILL CAPS, MONITORING WELLS,GROUNDWATER PUMPING WELLS, AND ASSOCIATED INFRASTRUCTURE AND UTILITIES, WILL REMAIN IN PLACE AND PROTECTED FROM CONSTRUCTION ACTIVITIES. ACCESS WILL BE MAINTAINED AT THESE AREAS FOR OPERATION AND MAINTENANCE ACTIVITIES REQUIRED BY THE PART 373 PERMIT.
6. THE MANAGEMENT OF EXCAVATED SOIL WILL BE DETERMINED BY THE PROCESSES IDENTIFIED IN XEROX'S SOIL MANAGEMENT PLAN, UNDER ITS SITE MANAGEMENT PLAN AND AS PART OF THE PART 373 RCRA PERMIT.
7. THE XEROX SOIL MANAGEMENT PLAN IS CURRENTLY UNDER REVIEW AND REVISION. ANY MODIFICATIONS TO SOIL MANAGEMENT REQUIREMENTS REQUESTED BY NYSDEC WILL BE COORDINATED WITH XEROX AND INTEGRATED INTO THE CONSTRUCTION ACTIVITIES.
8. IN THE EVENT GROUNDWATER IS ENCOUNTERED IN ANY EXCAVATIONS, IT WILL BE MANAGED IN ACCORDANCE WITH THE GROUNDWATER MANAGEMENT PLAN FOR THE XEROX SITE.

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NOTES

G-100

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9. EXCAVATED SOIL WILL BE STOCKPILED IN AN ADJACENT AREA AND WILL BE SAMPLED PER PROCEDURES IN XEROX'S SITE MANAGEMENT PLAN. PENDING THE RESULTS FROM SOIL ANALYTICAL TESTING, THE APPROPRIATE SOIL CLASSIFICATION, MANAGEMENT, AND DISPOSITION REQUIREMENTS WILL BE DETERMINED PER THE XEROX SOIL MANAGEMENT PLAN. STOCKPILES SHALL BE COVERED WITH AN IMPERVIOUS TARP OR BARRIER AND PLACED ON IMPERVIOUS PLASTIC, PENDING THE RESULTS OF THE ANALYSIS.

10. IN THE UNLIKELY EVENT OF A DISCOVERY OF AN UNKNOWN CONDITION DURING EXCAVATION IN ANY AREA OF THE SITE, REGARDLESS OF CLASSIFICATION NOTED IN THE XEROX SITE MANAGEMENT PLAN, INCLUDING THE DISCOVERY OF AN UNDERGROUND STORAGE TANK (UST), DRUMS, UNIDENTIFIED CONTAINMENT SOURCE, PRODUCT, ETC. INVASIVE WORK SHALL BE SUSPENDED AND THE XEROX CORPORATION ENVIRONMENTAL OPERATIONS ENGINEER SHALL BE CONTACTED IMMEDIATELY. NYSDEC SPILLS AND NYSDEC DMM PROJECT MANAGER SHALL BE NOTIFIED AS NECESSARY.

11. IDENTIFICATION OF UNKNOWN OR UNEXPECTED CONTAMINATED MEDIA IDENTIFIED BY SCREENING DURING INVASIVE SITE WORK WILL BE PROMPTLY COMMUNICATED BY PHONE TO THE XEROX CORPORATION ENVIRONMENTAL OPERATIONS ENGINEER FOR REPORTING TO NYSDEC'S PROJECT MANAGER. REPORTABLE QUANTITIES OF PETROLEUM PRODUCT WILL ALSO BE REPORTED TO THE NYSDEC SPILLS HOTLINE WITHIN 2 HOURS OF DISCOVERY.

12. COVER MATERIAL (GRASS, TOPSOIL, ASPHALT, CONCRETE OR GRAVEL) WILL BE SEGREGATED FROM THE UNDERLYING SOIL. THE COVER MATERIAL CAN BE DISPOSED OF OR REUSED ON SITE AS DETERMINED BY THE FACILITY AND IS NOT SUBJECT TO THE MANAGEMENT PRACTICES DESCRIBED IN THE XEROX SITE MANAGEMENT PLAN, UNLESS THE EXCAVATION AREA IS ASSOCIATED WITH A SURFACE SPILL.

13. EXCAVATION EQUIPMENT AND TOOLS THAT CONTACT CONTAMINATED SOIL WILL BE DECONTAMINATED UPON COMPLETION OF EXCAVATION ACTIVITIES. DECONTAMINATION WILL BE ACCOMPLISHED WITH A PRESSURE WASHER, HOSE, HAND-WASHING WITH SOAP AND WATER, OR OTHER APPROPRIATE METHOD. DECONTAMINATION WATER/RINSATE WILL BE CONTAINERIZED AND DISCHARGED IN COMPLIANCE WITH THE EXISTING PROCEDURES, PERMITS AND APPROVALS.

14. EXCAVATED SOIL THAT REQUIRES OFF-SITE DISPOSAL WILL BE CHARACTERIZED FOR MANAGEMENT PURPOSES THROUGH THE COLLECTION OF REPRESENTATIVE SAMPLES FOR LABORATORY ANALYSIS OF SITE CONTAMINANTS DEPENDING ON THE NATURE OF THE CONTAMINANTS LIKELY TO BE PRESENT AND LANDFILL REQUIREMENTS.

15. SOILS STAGED AFTER WORKING HOURS MUST BE SECURED AGAINST UNINTENDED INTRUSION AND MUST BE KEPT UNDER A COVER IN ORDER TO MINIMIZE SOIL RUN-OFF DURING PRECIPITATION EVENTS. EXCAVATED SOIL WILL ALSO BE STAGED ON IMPERMEABLE MATERIAL, SUCH AS PLASTIC SHEETING OR TARPS TO MINIMIZE CONTAMINATION OF THE STAGING AREA. SOILS WILL REMAIN STAGED PENDING THE RECEIPT OF LABORATORY ANALYTICAL RESULTS CHARACTERIZING THE SOIL. SOIL MUST BE CHARACTERIZED PRIOR TO BACKFILLING THE EXCAVATION. IF THE SOIL IS CHARACTERIZED AS A HAZARDOUS WASTE, THEN THE SOIL MUST BE CONTAINERIZED (DRUMS, ROLLBACKS, OR SIMILAR) AND CANNOT REMAIN ON THE TARP. THE SOIL CAN BE SHIPPED OFF AS HAZARDOUS WASTE OR A REQUEST FOR A CONTAINED-IN DETERMINATION MAY BE FILED WITH DEC FOR POTENTIAL REUSE. NON-HAZARDOUS SOILS (ONLY IF CHARACTERIZED AS SUCH) CAN BE PLACED BACK INTO THE EXCAVATION.

16. ALL MATERIALS PROPOSED FOR IMPORT ONTO THE SITE WILL BE TESTED AND APPROVED BY XEROX AND WILL BE FOLLOWING PROVISIONS IN THIS SOIL MANAGEMENT PLAN PRIOR TO RECEIPT AT THE SITE.

17. IMPORTED SOIL MATERIAL WILL MEET THE BACKFILL AND COVER SOIL TESTING AND QUALITY STANDARDS PRESENTED IN DER-10 (UNRESTRICTED SCOS IN ACCORDANCE WITH TABLE 375-6.8(A) IN 6 NYCRR 375) AND THE NYSDEC SAMPLING, ANALYSIS, AND ASSESSMENT OF PER- AND POLYFLUOROALKYL (PFAS) DOCUMENT.

18. TRUCKS ENTERING THE SITE WITH IMPORTED SOILS WILL BE SECURELY COVERED WITH TIGHT FITTING COVERS. IMPORTED SOILS WILL BE STOCKPILED SEPARATELY FROM EXCAVATED MATERIALS AND COVERED TO PREVENT DUST RELEASES.

19. SOIL IMPORT REQUEST WILL BE REQUIRED TO BE FILED WITH NYSDEC PROJECT MANAGER FOR ANY SOIL MATERIALS BROUGHT ON-SITE FOR USE IN SOIL COVER/CAP OR FOR BACKFILL. ONE IMPORT REQUEST PER MATERIAL TYPE IS REQUIRED WITH TESTING IN ACCORDANCE WITH REQUIREMENTS OF DER-10. ANY GRAVEL/NON-SOIL MATERIALS WILL REQUIRE A SIEVE ANALYSIS AS OPPOSED TO CHEMICAL TESTING. FOR SUBSEQUENT IMPORTS, NYSDEC SHOULD BE INFORMED BUT SAMPLYING MAY NOT BE REQUIRED IF THE SOURCE IS THE SAME, AND WAS PREVIOUSLY SAMPLED AND ACCEPTED. CONSULTATION WITH THE NYSDEC PROJECT MANAGER IS REQUIRED PRIOR TO SECONDARY IMPORTS.

20. ALL SOIL OR SAND IMPORTED FROM A VIRGIN MINE/PIT FOR USE AS A SOIL COVER/CAP OR FOR BACKFILLING AN EXCAVATION REQUIRES AT LEAST ONE ROUND OF CHARACTERIZATION SAMPLES FOR THE INITIAL 100 CUBIC YARDS OF MATERIAL. SOIL SOURCES OTHER THAN THE VIRGIN MINE/PIT MUST FOLLOW TABLE 5.4(E)(10) IN DER-10. MATERIAL OTHER THAN SOIL (I.E. GRAVEL, STONE, ROCK, ETC.) WILL REQUIRE A SIEVE ANALYSIS AS NOTED ABOVE. SOILS WILL NEED TO MEET UNRESTRICTED SCOS. ALL IMPORTED SOILS THAT REQUIRE TESTING WILL MEET THE BACKFILL SOIL QUALITY STANDARDS ESTABLISHED IN 6NYCRR 375-6.7(D).

21. SOLID WASTE OR IMPORTED MATERIALS FROM OTHER INDUSTRIAL SITES, SPILL SITES, OR OTHER REMEDIATION OR CONTAMINATED SITES IS PROHIBITED FROM BEING USED ON-SITE.

SPECIFICATION NOTES FOR PERMANENT SEEDING

SOIL AMENDMENTS

1. SOIL AMENDMENTS SHOULD BE INCORPORATED INTO THE UPPER 2-4 INCHES OF SOIL WHEN FEASIBLE.

2. CONTRACTOR SHALL CONDUCT SOIL TESTING FOR pH AND CONSULT WITH WITH ENTITIES SUCH AS THE CORNELL COOPERATIVE EXTENSION, OR OTHERS, FOR RECOMMENDATIONS FOR SOIL AMENDMENTS.

TIME OF PLANTING

1. PERMANENT SEEDING TO BE DONE BETWEEN APRIL 1 AND OCTOBER 14. ANY SEEDING FROM OCTOBER 15 TO MARCH 31 WILL BE TEMPORARY SEEDING (AROOSTOOK-WINTER RYE) AND WILL REQUIRE PERMANENT SEEDING TO TAKE PLACE THE FOLLOWING SEASON. FALL SEEDING IS THE PREFERRED SEEDING TIME.

SITE RESTORATION

1. PROVIDE EROSION CONTROL MEASURES, BRING THE TOPSOILED AREA TO DESIRED GRADE AND PREPARE FOR SEEDING. RESTORE DISTURBED SOIL AREAS TO EXISTING GRADE AND PROVIDE TOPSOIL, SEED, MULCH, AND ADD SOIL AMENDMENTS AS NECESSARY.

2. PREPARE SEEDBED, LOOSENING SOIL TO A DEPTH OF 3-4 INCHES.

3. REMOVE ALL STONES OVER THREE INCH (3") IN DIAMETER, STICKS AND FOREIGN MATTER FROM THE SURFACE.

4. APPLY FERTILIZER PER RECOMMENDATIONS RESULTING FROM SOIL pH TESTING.

5. SEED PREPARED AREAS WITH SEED MIX AT APPLICATION RATE SPECIFIED BY THE MANUFACTURER.

6. SMOOTH AND FIRM SEEDBED.

7. APPLY MULCH AT SPECIFIED APPLICATION RATE (MULCHING SECTION OF THESE NOTES).

PERMANENT SEEDING MIXTURES

1. PERMANENT GRASS SEED TYPE TO BE AS DIRECTED BY MONTANTE.

2. FOR AREAS DESIGNATED AS POLLINATOR PLANTING IN THESE PLANS THE CONTRACTOR SHALL FURNISH POLLINATOR FRIENDLY, NATIVE PLANT SEED MIX TO THE OWNER FOR REVIEW AND APPROVAL. THE POLLINATOR SEED MIX SHALL CONTAIN ONE OR SEVERAL OF THE FOLLOWING:

   COMMON MILKWEED (ASCLEPIEAS SYRIACA) - UPLAND AREAS
   BUTTERFLY MILKWEED (ASCLEPIEAS TUBEROSA, PA ECOTYPE - UPLAND AREAS
   SWAMP MILKWEED (A. INCARNATA) - WETLAND AREAS

3. FOR DISTURBED AREAS NOT DESIGNATED AS POLLINATOR PLANTING IN THESE PLANS THE CONTRACTOR SHALL FURNISH A NORTHEAST ROADSIDE MIX, SUCH AS PARTIALLY SHADED AREA ROADSIDE MIX (ERNMX-140), TO THE OWNER FOR REVIEW AND APPROVAL.

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TONAWANDA, NY14150

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MONTANTE XEROX SOLAR DEVELOPMENT
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NOTES

G-101

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NOW OR FORMALLY
THE MEADOWS OF WEBSTER
5B 360/68-70
16.11 ACRES

NOW OR FORMALLY
REMAINING LAND
LABELLA TERRA
5B 360/68-70
151.67 ACRES

NOW OR FORMALLY
XEROX-ALDER SUB
365/34
165.72 ACRES

B-17A
B-24B
B-24
B-24A
B-20
B-20A
B-20B
B-21A
B-21IR
B-21DR
W-5
W-4
RW-1
RW-2
B-1
B-13
B-22A
B-18A
B-37-1

APPROXIMATE LOCATION OF EXISTING BROKEN VENT PIPE TO BE REPAIRED BY XEROX PRIOR TO CONSTRUCTION

CLOSED C&D LANDFILL WITH EXISTING SOIL COVER. SOLID WASTE MANAGEMENT UNIT (SWMU) #88

EXISTING LANDFILL VENTS TO BE PROTECTED

PRIOR TO CONSTRUCTION, XEROX WILL REPAVE THE LANDFILL IN ACCORDANCE WITH THE TOWN'S REQUEST, PENDING NYSDEC APPROVAL

EXISTING DRAINAGE DITCH

EXISTING BEDROCK TRENCH

EXISTING UNPAVED DRIVE

EXISTING DRAINAGE DITCH

EXISTING UNPAVED SITE ACCESS DRIVE

EXISTING UNPAVED DRIVE

GRAVEL

CARACAS DRIVE

RESENDE ROAD

INACTIVE, ASPHALT-CAPPED HAZARDOUS WASTE LANDFILL. NYSDEC SITE CODE 828013, SOLID WASTE MANAGEMENT UNIT (SWMU) #62

LEGEND

- SOLAR STUDY AREA
- PARCEL BOUNDARY
- EXISTING BUILDING
- EXISTING TREELINE
- DELINEATED WETLAND
- EXISTING DELINEATED STREAM
- EXISTING DRAINAGE DITCH
- EXISTING CULVERT
- EXISTING BEDROCK TRENCH
- EXISTING CHAIN LINK FENCE
- EXISTING OVERHEAD ELECTRIC LINE
- APPROX. EXTENTS OF C&D LANDFILL
- APPROX. EXTENTS OF ASPHALT CAPPED HAZ. WASTE LANDFILL
- EXISTING C&D LANDFILL VENT
- B-21A EXISTING MONITORING WELL

NOTES

1. LANDFILL VENTS ARE TO BE PROTECTED DURING CONSTRUCTION.
2. EXISTING TREELINE SHOWN IS DRAWN USING AERIAL IMAGERY AND SHOULD BE CONSIDERED APPROXIMATE.
3. THE PROJECT IS LOCATED WITHIN THE LARGER XEROX WEBSTER CAMPUS, WHICH IS A NYSDEC RCRA FACILITY (NYSDEC SITE CODE 828178) AND IS CURRENTLY MANAGED UNDER RCRA PART 373 HAZARDOUS WASTE MANAGEMENT PERMIT (NO. 8-2654-00064/00040).

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EXISTING CONDITIONS

C-100

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NOW OR FORMALLY
REMAINING LAND
LABELLA TERRA
5B 360/68-70
151.67 ACRES

NOW OR FORMALLY
XEROX-ALDER SUB
365/34
165.72 ACRES

2 C-302 PROPOSED 4' WIDE GRAVEL DIAPHRAGM SECTION

1 C-301 2A C-301 2B C-301 PERIMETER FENCE

2 C-302 PROPOSED 2' WIDE GRAVEL DIAPHRAGM SECTION

LIMITS OF DISTURBANCE (17.5 ACRES)

PROPOSED BALLASTED SOLAR ARRAYS 3 C-300

PROPOSED ACCESS GATE 3 C-301

PROPOSED 1' WIDE GRAVEL DIAPHRAGM SECTION 2 C-302

OW-320N

B-24B

B-24

B-24A

B-20

ANY MINIMAL DISTURBANCE WITHIN THE C&D LANDFILL IS SUBJECT TO APPROVAL BY NYSDEC PROJECT MANAGER (I.E., STAKING OF TEMPORARY E&S CONTROLS). SEE NOTE 5.

APPROXIMATE LOCATION OF EXISTING BROKEN VENT PIPE TO BE REPAIRED BY XEROX PRIOR TO CONSTRUCTION

B-20A

B-20B

AREA TO RECEIVE POLLINATOR SEED MIX (TYP). SEE SHEET G-101 FOR SEEDING NOTES.

PANEL BALLASTS WILL BE FIELD ADJUSTED, AS NECESSARY, TO AVOID DIRECT PLACEMENT WITHIN ON-SITE DRAINAGE CHANNELS.

SEE INSET ON SHEET C-301 FOR CONFIGURATION OF PERIMETER FENCE ALONG WESTERN EDGE OF C&D LANDFILL

B-21A

B-21IR

W-5

W-4

B-21DR

ALL PROJECT ELECTRICAL GROUNDING PLATES TO BE LIMITED TO LOCATIONS AROUND PERIMETER OF EQUIPMENT PAD

FENCE BALLASTS FOUNDATIONS WILL BE FIELD ADJUSTED TO AVOID APPROXIMATE BEDROCK TRENCH LOCATIONS SHOWN

GEOBALLAST FOUNDATIONS WILL BE FIELD-ADJUSTED TO AVOID THE APPROXIMATE BEDROCK TRENCH LOCATION SHOWN

RW-1

RW-2

B-1

REMOVE SECTION OF EXISTING CHAIN LINK FENCE (SEE NOTE 4)

B-7

GRAVEL

CARACAS DRIVE

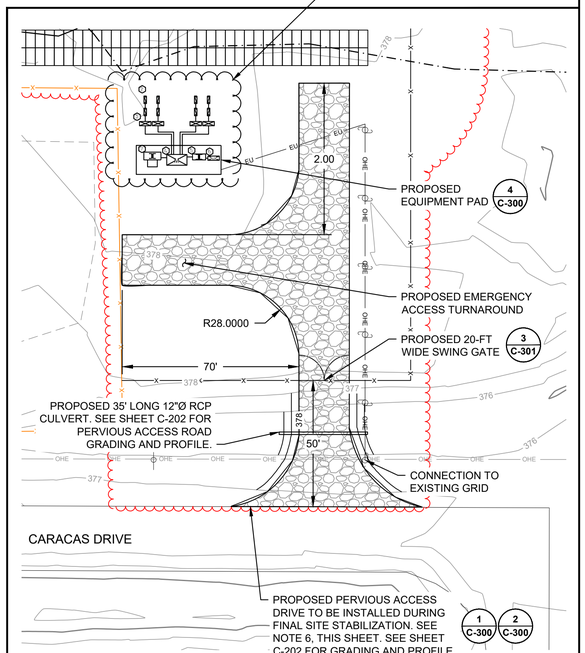
NO DISTURBANCE ALLOWED WITHIN FOOTPRINT OF ASPHALT CAPPED HAZ. WASTE LANDFILL (SEE NOTE 3)

SEE PROPOSED SITE ENTRANCE INSET

B-18A

B-18

B-18SR



PROPOSED SITE ENTRANCE INSET

SCALE: 1"=30'

LEGEND

- SOLAR STUDY AREA
- PARCEL BOUNDARY
- EXISTING BUILDING
- DELINEATED WETLAND
- EXISTING DELINEATED STREAM
- EXISTING BEDROCK TRENCH
- EXISTING CHAIN LINK FENCE
- PROPOSED CHAIN LINK FENCE
- B-21A EXISTING MONITORING WELL
- APPROX. EXTENTS OF C&D LANDFILL
- APPROX. EXTENTS OF ASPHALT CAPPED HAZ. WASTE LANDFILL
- EXISTING C&D LANDFILL VENT
- PROPOSED LIMITS OF DISTURBANCE
- OHE PROPOSED OVERHEAD ELECTRIC
- EU PROPOSED UNDERGROUND ELECTRIC
- PROPOSED UTILITY POLE
- PROPOSED SITE ACCESS DRIVE
- PROPOSED LIMITS OF TREE CLEARING
- EXISTING FENCE TO BE REMOVED
- PROPOSED AREAS TO RECEIVE POLLINATOR SEED MIX
- AREAS WITH SOLAR PANELS INSTALLED ON CONTINUOUS SLOPES >/= 8%

NOTES

1. **LIMITS OF DISTURBANCE = 17.5 ACRES**
   LIMITS OF TREE CLEARING = 15.1 ACRES
2. LANDFILL VENTS ARE TO BE PROTECTED DURING CONSTRUCTION.
3. A MINIMUM SETBACK OF 30 FEET TO BE ESTABLISHED AROUND ALL MONITORING WELLS WITHIN THE PROPOSED WORK AREA.
4. PROPOSED PERVIOUS ACCESS DRIVE TO BE PROTECTED DURING CONSTRUCTION. CONSTRUCTION TRAFFIC TO USE TEMPORARY CONSTRUCTION ACCESSES SHOWN ON SHEET C-201.
5. REFER TO NOTES ON G-100 REGARDING CONSTRUCTION SEQUENCE FOR GRADING AND EROSION AND SEDIMENT CONTROLS.
6. ALL SITE TRAFFIC AND EQUIPMENT IS NOT PERMITTED TO ENTER THE ASPHALT CAPPED HAZ. WASTE LANDFILL AREA OR USE FOR SITE ACCESS IN ANY CAPACITY.
7. DECOMPACTION OF ALL TEMPORARY CONSTRUCTION ACCESS DRIVES MAY BE REQUIRED IN ACCORDANCE WITH 2024 NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION'S (NYSDEC) CONSTRUCTION GENERAL PERMIT (CGP) REQUIREMENTS.

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PREPARED FOR:

TM MONTANTE SOLAR DEVELOPMENT, LLC

2760 KENMORE AVE #101
TONAWANDA, NY14150

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MONTANTE XEROX SOLAR DEVELOPMENT
DESIGN SUBMITTAL

139 CARACAS DRIVE
WEBSTER, NY

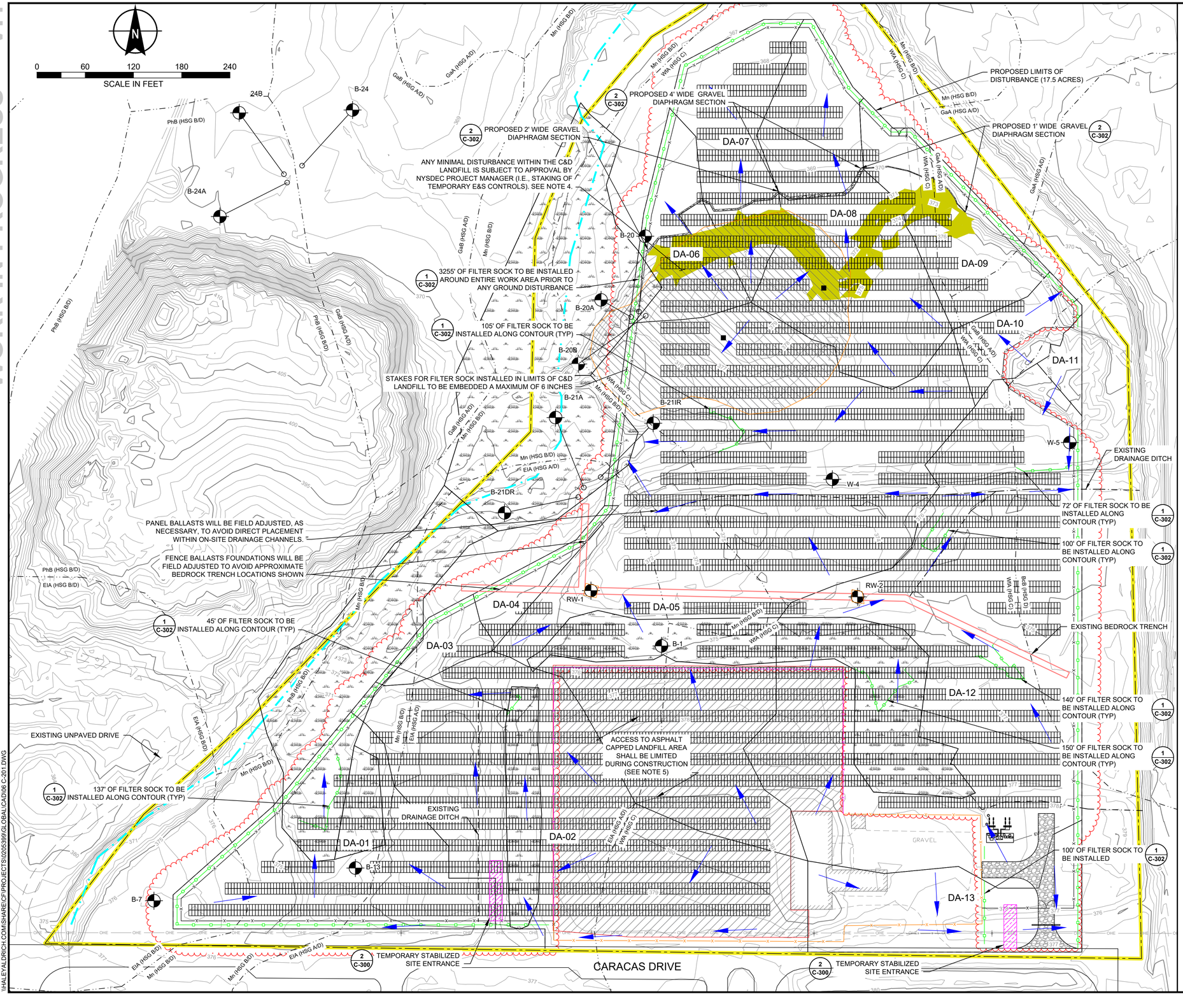
PROPOSED CONDITIONS

C-200

Sheet: 05 of 11

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WORK IN PROGRESS - V.1



LEGEND

- SOLAR STUDY AREA
- EXISTING BUILDING
- DELINEATED WETLAND
- DELINEATED STREAM
- EXISTING BEDROCK TRENCH
- EXISTING DRAINAGE DITCH
- EXISTING CULVERT
- EXISTING CHAIN LINK FENCE
- PROPOSED CHAIN LINK FENCE
- EXISTING OVERHEAD ELECTRIC LINE
- EXISTING DELINEATED STREAM
- APPROX. EXTENTS OF C&D LANDFILL
- APPROX. EXTENTS OF ASPHALT CAPPED HAZ. WASTE LANDFILL
- EXISTING C&D LANDFILL VENT
- B-21A EXISTING MONITORING WELL
- PROPOSED LIMITS OF DISTURBANCE
- PROPOSED LIMITS OF FILTER SOCK
- PROPOSED LIMITS OF TREE CLEARING
- SOIL BOUNDARY
- DELINEATED DRAINAGE AREAS
- FLOW ARROW
- EXISTING FENCE TO BE REMOVED
- AREAS WITH SOLAR PANELS INSTALLED ON CONTINUOUS SLOPES >/= 8%

NOTES

1. **LIMITS OF DISTURBANCE = 17.5 ACRES**
   LIMITS OF TREE CLEARING = 15.1 ACRES
2. LANDFILL VENTS ARE TO BE PROTECTED DURING CONSTRUCTION.
3. A MINIMUM SETBACK OF 30 FEET TO BE ESTABLISHED AROUND ALL MONITORING WELLS WITHIN THE PROPOSED WORK AREA.
4. PROPOSED PERVIOUS ACCESS DRIVE TO BE PROTECTED DURING CONSTRUCTION. CONSTRUCTION TRAFFIC TO USE TEMPORARY CONSTRUCTION ACCESSES SHOWN ON SHEET C-201.
5. REFER TO NOTES ON G-100 REGARDING CONSTRUCTION SEQUENCE FOR GRADING AND EROSION AND SEDIMENT CONTROLS.
6. ALL SITE TRAFFIC AND EQUIPMENT IS NOT PERMITTED TO ENTER THE ASPHALT CAPPED HAZ. WASTE LANDFILL AREA OR C&D LANDFILL. LOADS SHALL BE MAINTAINED TO LESS THAN 10 PSI. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE INTEGRITY OF THE EXISTING LANDFILL COVERS.
7. DECOMPACTION OF ALL TEMPORARY CONSTRUCTION ACCESS DRIVES MAY BE REQUIRED IN ACCORDANCE WITH 2024 NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION'S (NYSDEC) CONSTRUCTION GENERAL PERMIT (CGP) REQUIREMENTS.
8. FILTER SOCKS INTERIOR TO THE LOD ARE TO BE INSTALLED DOWNSTREAM OF ACTIVE TREE CLEARING AND MAINTAINED UNTIL FINAL STABILIZATION. FILTER SOCKS INTERIOR TO THE LOD MAY BE MOVED TO LOCATIONS APPROXIMATE TO THOSE SHOWN ON THE PLANS TO ALLOW FOR SOLAR ARRAY INSTALLATION.
7. STORMWATER CONTROLS ARE SUBJECT TO REVIEW AND APPROVAL BY THE TOWN ENGINEER/MS4 REVIEWER AND BY NYSDEC.

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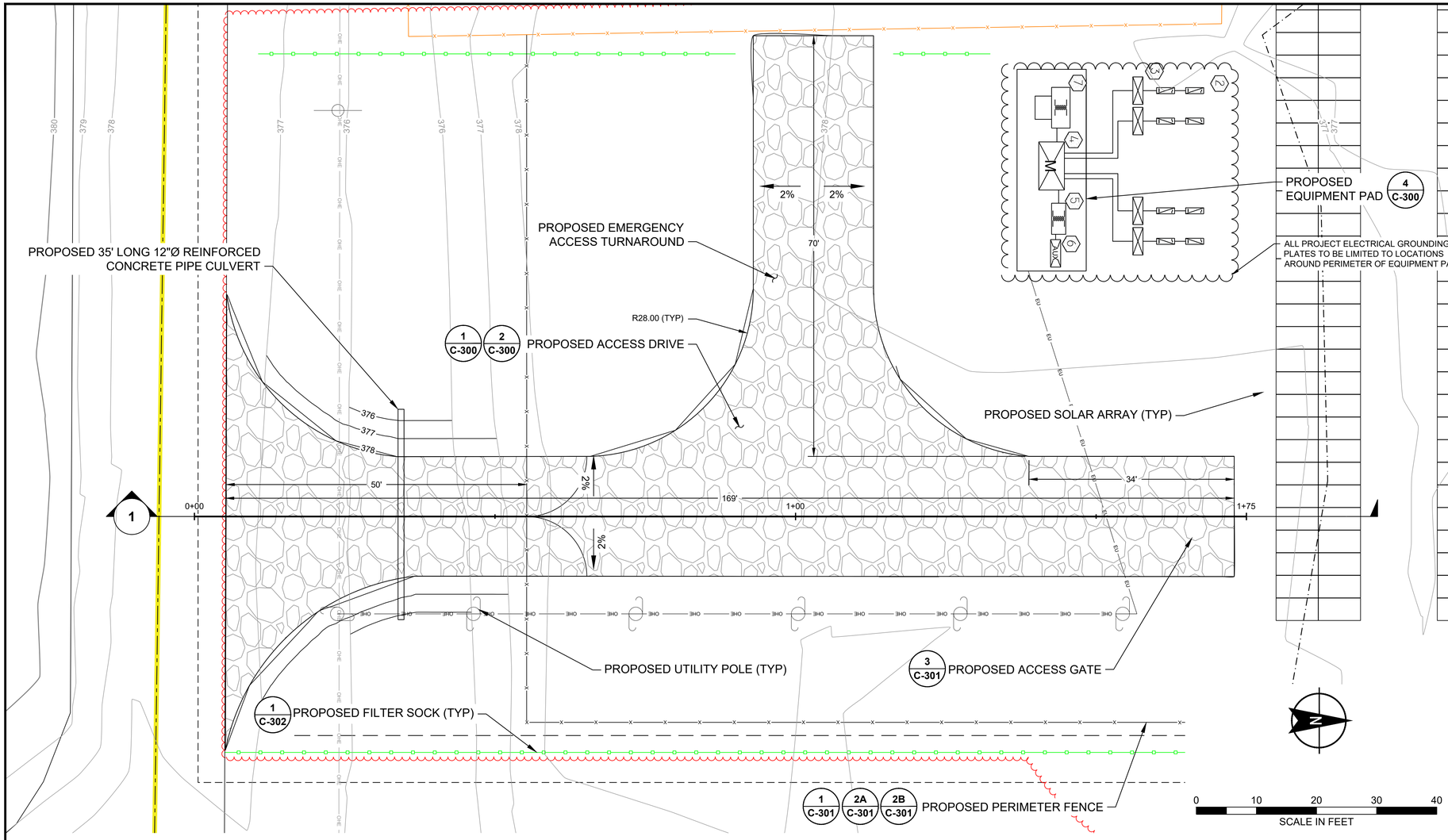
EROSION AND SEDIMENT CONTROLS PLAN

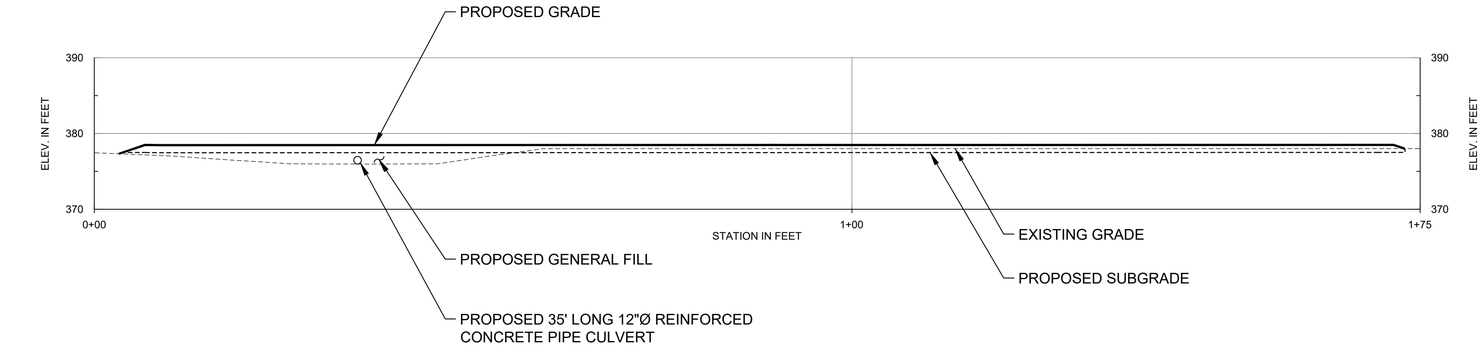
C-201

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WORK IN PROGRESS - V.1





1 **PROPOSED ACCESS ROAD PROFILE**
SCALE: AS SHOWN

LEGEND

- SOLAR STUDY AREA
- PARCEL BOUNDARY
- 100 FT STREAM SETBACK
- EXISTING OVERHEAD ELECTRIC
- EXISTING CHAIN LINK FENCE
- PROPOSED CHAIN LINK FENCE
- PROPOSED LIMITS OF DISTURBANCE
- PROPOSED FILTER SOCK
- PROPOSED OVERHEAD ELECTRIC
- PROPOSED UNDERGROUND ELECTRIC
- PROPOSED LIMITS OF TREE CLEARING

NOTES

1. PROPOSED PERVIOUS ACCESS DRIVE TO BE PROTECTED DURING CONSTRUCTION. CONSTRUCTION TRAFFIC TO USE TEMPORARY CONSTRUCTION ACCESSES SHOWN ON SHEET C-201.
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PROPOSED ACCESS ROAD GRADING PLAN

C-202

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LLY
EBSTER

NOW OR FORMALLY
REMAINING LAND
LABELLA TERRA
5B 360/68-70
151.67 ACRES

NOW OR FORMALLY
XEROX-ALDER SUB
365/34
165.72 ACRES

OW-320N

2 C-302 PROPOSED 4' WIDE GRAVEL DIAPHRAGM SECTION

PROPOSED BALLASTED SOLAR ARRAYS 3 C-300

LIMITS OF DISTURBANCE (17.5 ACRES)

PROPOSED ACCESS GATE 3 C-301

1 C-301 2A C-301 2B C-301 PROPOSED PERIMETER FENCE

2 C-302 PROPOSED 2' WIDE GRAVEL DIAPHRAGM SECTION

PROPOSED 1' WIDE GRAVEL DIAPHRAGM SECTION 2 C-302

B-24B B-24 B-24A

ANY MINIMAL DISTURBANCE WITHIN THE C&D LANDFILL IS SUBJECT TO APPROVAL BY NYSDEC PROJECT MANAGER (I.E., STAKING OF TEMPORARY E&S CONTROLS). SEE NOTE 5.

B-20 B-20A B-20B B-21A B-21IR B-21DR

SEE INSET ON SHEET C-301 FOR CONFIGURATION OF PERIMETER FENCE ALONG WESTERN EDGE OF C&D LANDFILL

1 C-302 FILTER SOCK TO BE INSTALLED AROUND ENTIRE WORK AREA PRIOR TO ANY GROUND DISTURBANCE

W-5 W-4 RW-1 RW-2 B-1

GEOBALLAST FOUNDATIONS WILL BE FIELD-ADJUSTED TO AVOID THE APPROXIMATE BEDROCK TRENCH LOCATION SHOWN

PROPOSED LAYDOWN AREA. SHEET G-100 FOR SOIL RESTORATION NOTES.

PROPOSED LAYDOWN AREA. SHEET G-100 FOR SOIL RESTORATION NOTES.

REMOVE SECTION OF EXISTING CHAIN LINK FENCE (SEE NOTE 4)

GRAVEL

B-13 B-7

CARACAS DRIVE

2 C-300 TEMPORARY STABILIZED SITE ENTRANCE

NO DISTURBANCE ALLOWED WITHIN FOOTPRINT OF ASPHALT CAPPED HAZ. WASTE LANDFILL (SEE NOTE 3)

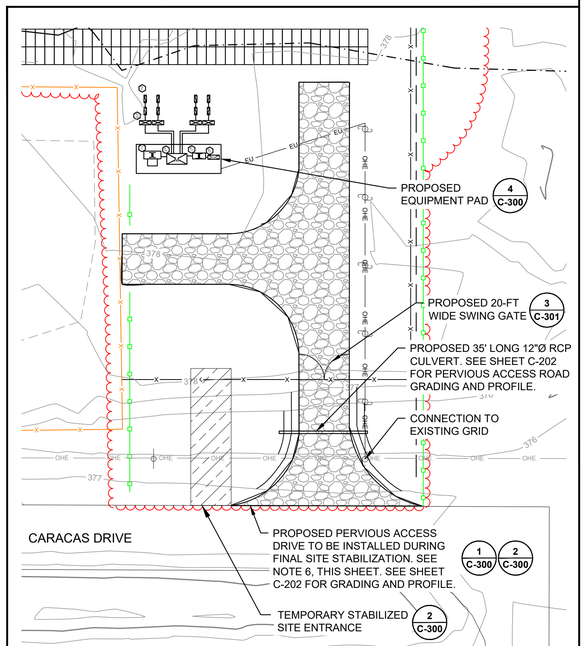
PRIOR TO CONSTRUCTION, XEROX WILL REPAVE THE LANDFILL IN ACCORDANCE WITH THE TOWN'S REQUEST, PENDING NYSDEC APPROVAL

2 C-300 TEMPORARY STABILIZED SITE ENTRANCE

SEE PROPOSED SITE ENTRANCE INSET

B-18A B-18 B-18SR

CONSTRUCTION PHASING		
PRE-CONSTRUCTION	XEROX TO RE-PAVE THE ASPHALT LANDFILL	Schedule TBD
INSTALL EROSION AND SEDIMENT CONTORLS	INSTALL PERIMETER EROSION AND SEDIMENT CONTROLS	Schedule TBD
	INSTALL STABILIZED CONSTRUCTION ENTRANCES	Schedule TBD
TREE CLEARING	CONDUCT TREE CLEARING	Schedule TBD
GRADING AND UTILITY INSTALLATION	INSTALL GRAVEL DIAPHRAGM	Schedule TBD
	INSTALL EQUIPMENT PAD	Schedule TBD
	INSTALL UTILITY POLES	Schedule TBD
	INSTALL ELECTRIC LINES	Schedule TBD
SOLAR ARRAY INSTALLATION	INSTALL SOLAR ARRAYS	Schedule TBD
FINAL SITE STABILIZATION	INSTALL PERVIOUS SITE ENTRANCE	Schedule TBD
	CONDUCT FINAL STABILIZATION	Schedule TBD
	CONDUCT FINAL SITE INSPECTION	Schedule TBD
	REMOVE EROSION AND SEDIMENT CONTROLS	Schedule TBD



PROPOSED SITE ENTRANCE INSET

SCALE: 1"=30'

LEGEND

- SOLAR STUDY AREA
- PARCEL BOUNDARY
- EXISTING BUILDING
- DELINEATED WETLAND
- DELINEATED STREAM
- EXISTING BEDROCK TRENCH
- EXISTING CHAIN LINK FENCE
- PROPOSED CHAIN LINK FENCE
- B-21A EXISTING MONITORING WELL
- APPROX. EXTENTS OF C&D LANDFILL
- APPROX. EXTENTS OF ASPHALT CAPPED HAZ. WASTE LANDFILL
- EXISTING C&D LANDFILL VENT
- PROPOSED LIMITS OF DISTURBANCE
- PROPOSED OVERHEAD ELECTRIC
- PROPOSED UNDERGROUND ELECTRIC
- PROPOSED UTILITY POLE
- PROPOSED SITE ACCESS DRIVE
- PROPOSED LIMITS OF TREE CLEARING
- PROPOSED LIMITS OF FILTER SOCK
- EXISTING FENCE TO BE REMOVED
- PROPOSED STAGING AREAS
- AREAS WITH SOLAR PANELS INSTALLED ON CONTINUOUS SLOPES >/= 8%

NOTES

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MONTANTE XEROX SOLAR DEVELOPMENT
DESIGN SUBMITTAL

139 CARACAS DRIVE
WEBSTER, NY

PHASING PLAN

C-203

Sheet: 08 of 11

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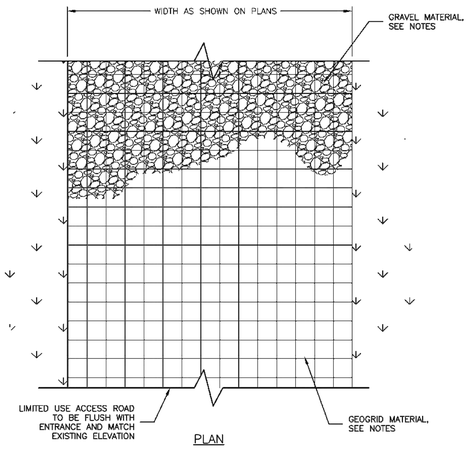
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GENERAL NOTES:

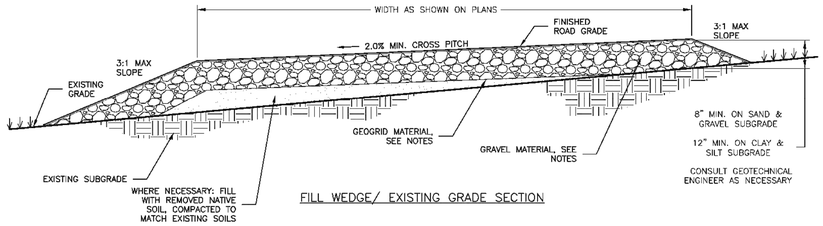
1. USE OF THIS DETAIL/CRITERION IS LIMITED TO ACCESS ROAD USED ON AN OCCASIONAL BASIS ONLY (I.E. PROVIDE ACCESS FOR MOWING, EQUIPMENT REPAIR OR MAINTENANCE, ETC.).
2. LIMITED USE PERVIOUS ACCESS ROAD IS LIMITED TO LOW IMPACT IRREGULAR MAINTENANCE ACCESS ASSOCIATED WITH RENEWABLE ENERGY PROJECTS IN NEW YORK STATE
3. REMOVE STUMPS, ROCKS AND DEBRIS AS NECESSARY. FILL VOIDS TO MATCH EXISTING NATIVE SOILS AND COMPACTION LEVEL.
4. THE LIMITED USE PERVIOUS ACCESS ROAD CROSS SLOPE SHALL BE 2% IN MOST CASES AND SHOULD NOT EXCEED 6%. THE LONGITUDINAL SLOPE OF THE ACCESS DRIVE SHOULD NOT EXCEED 15%
5. TO ENSURE THAT SOIL IS NOT TRACKED ONTO THE LIMITED USE PERVIOUS ACCESS ROAD, IT SHALL NOT BE USED BY CONSTRUCTION VEHICLES TRANSPORTING SOIL, FILL MATERIAL, ETC. IF THE LIMITED USE PERVIOUS ACCESS IS COMPLETED DURING THE INITIAL PHASES OF CONSTRUCTION, A STANDARD NEW YORK STATE STABILIZED CONSTRUCTION ACCESS SHALL BE CONSTRUCTED AND UTILIZED TO REMOVE SEDIMENT FROM CONSTRUCTION VEHICLES AND EQUIPMENT PRIOR TO ENTERING THE LIMITED USE PERVIOUS ACCESS ROAD FROM ANY LOCATION ON, OR OFF SITE. MAINTENANCE OF THE PERVIOUS ACCESS ROAD WILL BE REQUIRED IF SEDIMENT IS OBSERVED WITHIN THE CLEAN STONE.
6. THE LIMITED USE PERVIOUS ACCESS ROAD SHALL NOT BE CONSTRUCTED OR USED UNTIL ALL AREAS SUBJECT TO RUNOFF ONTO THE PERVIOUS ACCESS HAVE ACHIEVED FINAL STABILIZATION.

GEOGRID MATERIAL NOTES:

1. GRAVEL FILL MATERIAL SHALL CONSIST OF 1–4" CLEAN, DURABLE, SHARP–ANGLED CRUSHED STONE OF UNIFORM QUALITY, MEETING THE SPECIFICATIONS OF NYSDOT ITEM 703–02, SIZE DESIGNATION 3–5 OF TABLE 703–4. STONE MAY BE PLACED IN FRONT OF, AND SPREAD WITH, A TRACKED VEHICLE. GRAVEL SHALL NOT BE COMPACTED.
2. GEOGRID SHALL BE MIRAFI BXG110 OR APPROVED EQUAL. GEOGRID SHALL BE DESIGNED BASED ON EXISTING SOIL CONDITIONS AND PROPOSED HAUL ROAD SLOPES.
3. IF MORE THAN ONE ROLL WIDTH IS REQUIRED, ROLLS SHOULD OVERLAP A MINIMUM OF SIX INCHES.
4. REFER TO MANUFACTURER'S SPECIFICATION FOR PROPER TYING AND CONNECTIONS
5. LIMITED USE PERVIOUS ACCESS ROAD SHALL BE TOP DRESSED AS REQUIRED WITH ONLY 1–4" CRUSHED STONE MEETING NYSDOT ITEM 703–02 SPECIFICATIONS.
6. STANDARD PENETROMETER TESTING IS REQUIRED EVERY 100 FEET ALONG PROPOSED ROADWAY PRIOR TO INSTALLATION OF TEMPORARY ROAD.



PLAN



FILL WEDGE/ EXISTING GRADE SECTION

NOTE

1. PERVIOUS ACCESS ROAD DETAIL PROVIDED BY MONTANTE SOLAR DEVELOPMENT, LLC.

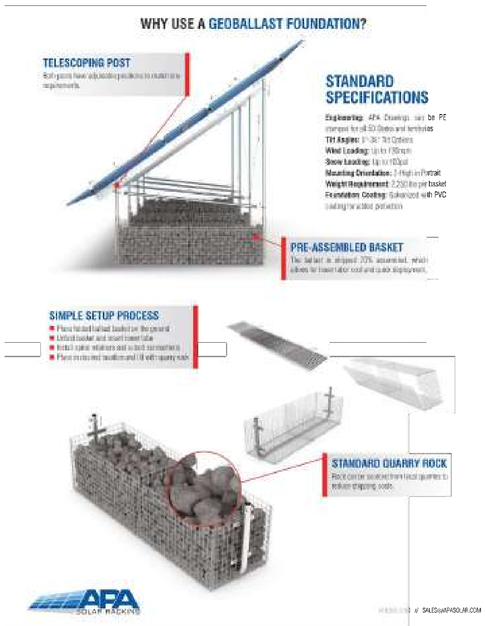
1 PERVIOUS ACCESS ROAD

SCALE: NOT TO SCALE



3 GEOBALLAST DATA SHEET

SCALE: NOT TO SCALE

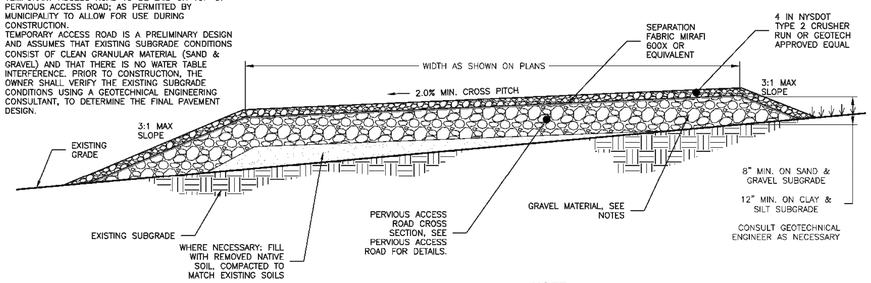


NOTE

1. GEOBALLAST DATA SHEET PROVIDED BY MONTANTE SOLAR DEVELOPMENT, LLC.

NOTES:

1. TEMPORARY ACCESS ROAD TO BE LAID ON TOP OF PERVIOUS ACCESS ROAD; AS PERMITTED BY MUNICIPALITY TO ALLOW FOR USE DURING CONSTRUCTION.
2. TEMPORARY ACCESS ROAD IS A PRELIMINARY DESIGN AND ASSUMES THAT EXISTING SUBGRADE CONDITIONS CONSIST OF CLEAN GRANULAR MATERIAL (SAND & GRAVEL) AND THAT THERE IS NO WATER TABLE INTERFERENCE. PRIOR TO CONSTRUCTION, THE OWNER SHALL VERIFY THE EXISTING SUBGRADE CONDITIONS USING A GEOTECHNICAL ENGINEERING CONSULTANT, TO DETERMINE THE FINAL PAVEMENT DESIGN.

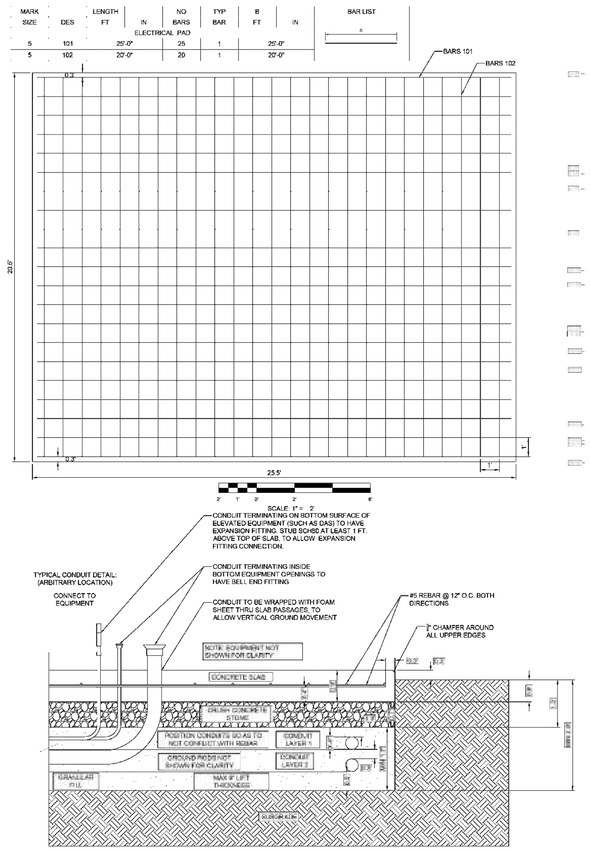


NOTE

1. TEMPORARY ACCESS ROAD DETAIL PROVIDED BY MONTANTE SOLAR DEVELOPMENT, LLC.

2 TEMPORARY ACCESS ROAD DETAIL

SCALE: NOT TO SCALE



NOTE

1. EQUIPMENT PAD DETAIL PROVIDED BY MONTANTE SOLAR DEVELOPMENT, LLC.

4 EQIPMENT PAD DETAIL

SCALE: NOT TO SCALE

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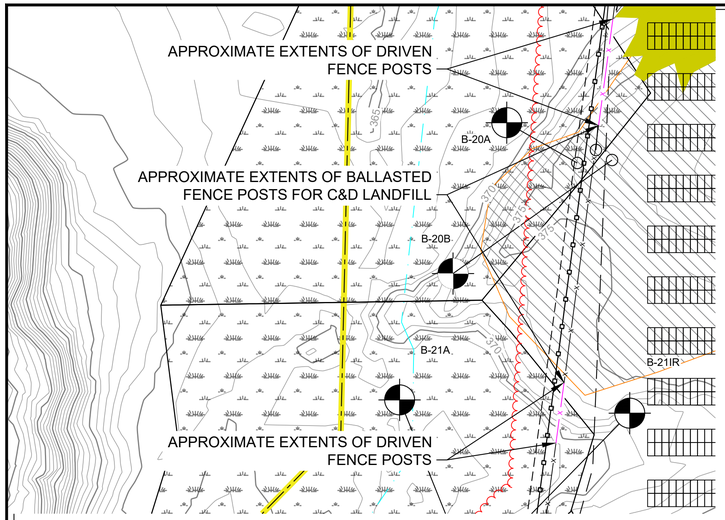
DETAILS

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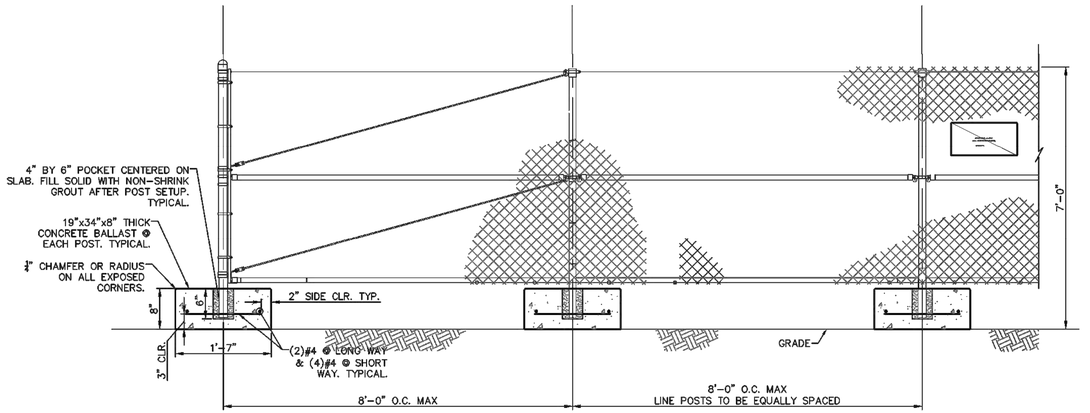


C&D LANDFILL DRIVEN POST EXTENTS INSET

SCALE: 1"=40'

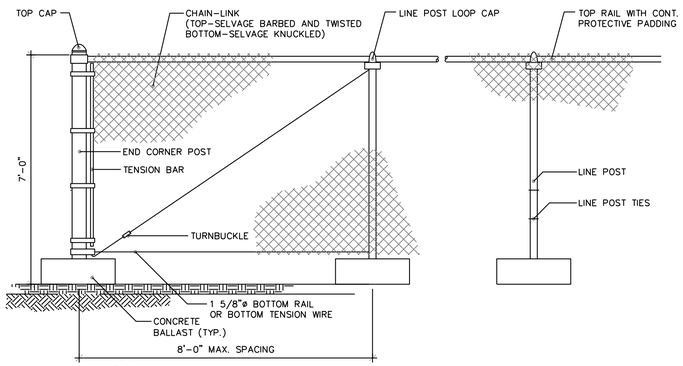
NOTE

1. DRIVEN POST FENCING TO BE UTILIZED TO PROPERLY ANCHOR FENCING PROPOSED ON SLOPES WHICH ARE IN OR ADJACENT C&D LANDFILL EXTENTS.
2. WHERE FEASIBLE, BALLASTED POST FENCING IS TO BE INSTALLED WITHIN LIMITS OF C&D LANDFILL.



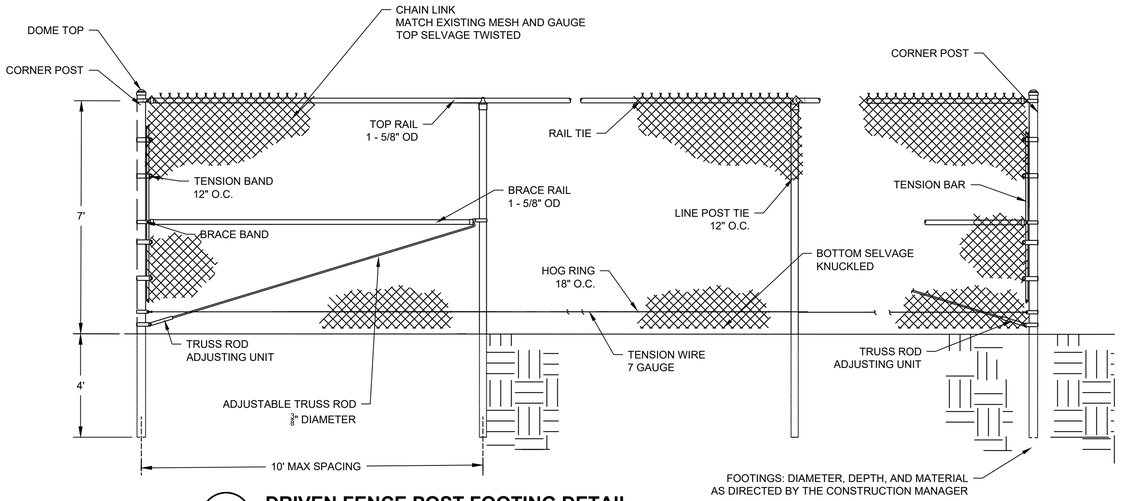
2A BALLASTED FENCE POST FOOTING DETAIL

SCALE: NOT TO SCALE



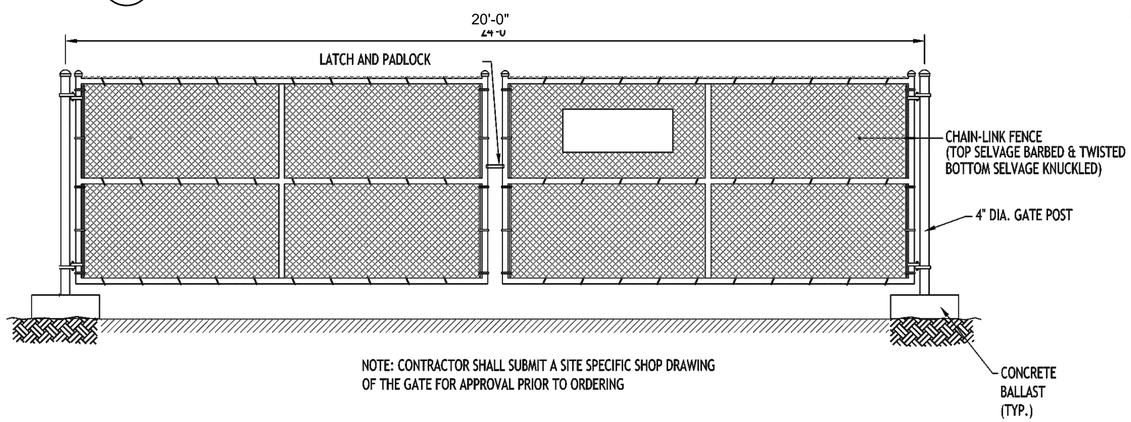
1 7' BALLASTED CHAIN-LINK FENCE DETAIL

SCALE: NOT TO SCALE



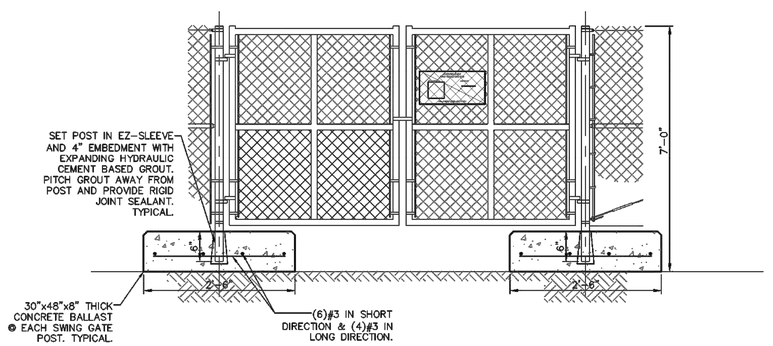
2B DRIVEN FENCE POST FOOTING DETAIL

SCALE: NOT TO SCALE



3 20' WIDE BALLASTED DUAL SWING GATE DETAIL

SCALE: NOT TO SCALE



4 20' WIDE BALLASTED DOUBLE SWING GATE POST FOOTING DETAIL

SCALE: NOT TO SCALE

HALEY ALDRICH

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www.haleyaldrich.com

PREPARED FOR:

TM MONTANTE SOLAR DEVELOPMENT, LLC

2760 KENMORE AVE #101
TONAWANDA, NY14150

THIS IS A COLOR DRAWING. INFORMATION WILL BE LOST IF REPRODUCED IN BLACK AND WHITE.

DRAFT

Project No.:	0205399
Scale:	SHOWN
Date:	FEBRUARY 2026
Drawn By:	SRG, JC
Designed By:	SRG, JC
Checked By:	SRG, ND
Approved By:	ND
Stamp:	

Rev.	Description	By	Date
H	FOR REVIEW	H&A	02/24/26
G	FOR REVIEW	H&A	02/19/26
F	FOR REVIEW	H&A	01/16/26
E	FOR REVIEW	H&A	12/18/25
D	FOR REVIEW	H&A	10/02/25
C	FOR REVIEW	H&A	09/17/25
B	FOR REVIEW	H&A	08/27/25
A	FOR REVIEW	H&A	08/11/25

MONTANTE XEROX SOLAR DEVELOPMENT
DESIGN SUBMITTAL

139 CARACAS DRIVE
WEBSTER, NY

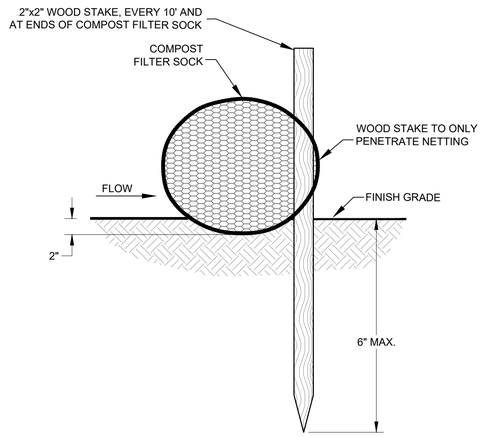
DETAILS

C-301

Sheet: 10 of 11

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\\HALEYALDRICH.COM\SHARE\CF\PROJECTS\0205399\GLOBAL\CAD\10 C-301.DWG

WORK IN PROGRESS - V.1



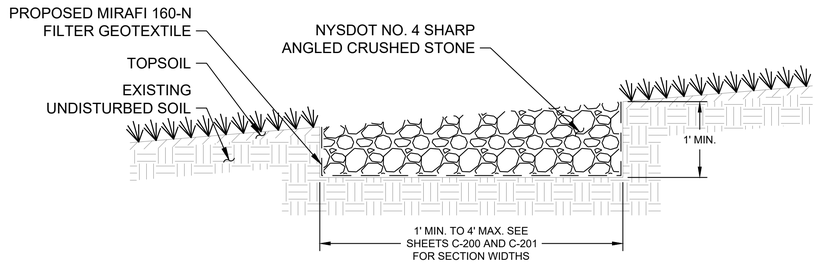
NOTES:

1. COMPOST FILTER SOCKS SHALL CONFORM TO THE NEW YORK STATES STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROLS, 2016.
2. COMPOST FILTER SOCKS SHALL BE A MINIMUM 12" DIAMETER.
3. COMPOST FILTER SOCKS SHALL BE INSTALLED ALONG A LEVEL GRADE WITH ENDS OVERLAPPING.
4. COMPOST FILTER SOCKS TO BE WESTERN GREEN STRAW WATTLE OR APPROVED EQUAL.
5. COMPOST FILTER SOCKS ARE NOT TO BE USED IN CONCENTRATED FLOW SITUATIONS.
6. ROUTINELY INSPECT COMPOST FILTER SOCKS AFTER EACH SIGNIFICANT RAIN, MAINTAINING WATTLES IN A FUNCTIONAL CONDITION AT ALL TIMES.
7. REMOVE SEDIMENTS COLLECTED AT THE BASE OF THE COMPOST FILTER SOCKS WHEN THEY REACH 1/3 OF THE EXPOSED HEIGHT OF THE PRACTICE.
8. WHERE THE COMPOST FILTER SOCK DETERIORATES OR FAILS, IT WILL BE REPAIRED OR REPLACED WITH A MORE EFFECTIVE ALTERNATIVE.
9. TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
10. REMOVAL - COMPOST FILTER SOCKS WILL BE DISPERSED ON SITE WHEN NO LONGER REQUIRED IN SUCH AS WAY AS TO FACILITATE AND NOT OBSTRUCT SEEDINGS.
11. FILTER SOCK STAKES TO BE EMBEDDED A MAXIMUM OF 6" PER SPECIFIC REQUESTS FROM THE NYSDEC DIVISION OF ENVIRONMENTAL REMEDIATION.
12. UPON ACHIEVING FINAL STABILIZATION AND DURING FILTER SOCK REMOVAL, STAKES SHALL BE PULLED AND ANY RESULTING VOIDS SHALL BE BACKFILLED WITH CLEAN SOIL, AS NEEDED.

1 COMPOST FILTER SOCK
SCALE: NOT TO SCALE

NOTES:

1. GRAVEL FILL MATERIAL SHALL CONSIST OF NYSDOT NO. 4 CLEAN, DURABLE, SHARP ANGLED CRUSHED STONE OF UNIFORM QUALITY, MEETING THE SPECIFICATIONS OF NYSDOT ITEM 703-02 SIZE DESIGNATION 4 OF TABLE 703-4.
2. TO MINIMIZE SEDIMENT ACCUMULATION IN THE GRAVEL DIAPHRAGM, THE GEOTEXTILE FABRIC SHALL BE WRAPPED OVER THE TOP UNTIL FINAL STABILIZATION IS ACHIEVED. THE FABRIC FOLD SHOULD ORIGINATE FROM THE UPGRADIENT SIDE.
3. THE GRAVEL DIAPHRAGM SHALL NOT BE DRIVEN UPON DURING SOLAR PANEL CONSTRUCTION ACTIVITIES UNLESS THE GRAVEL IS PROTECTED WITH A RIGID BARRIER SUCH AS PLYWOOD OR CONSTRUCTION EQUIPMENT IS CONSIDERED TO BE LOW GROUND PRESSURE EQUIPMENT TO MINIMIZE COMPACTION.
4. GRAVEL DIAPHRAGM IS TO BE INSTALLED ALONG THE CONTOUR AS SHOWN ON THESE DESIGN PLANS.
5. THE GRAVEL DIAPHRAGM SHALL NOT BE INSTALLED WITHIN THE LIMITS OF THE EXISTING CONSTRUCTION AND DEMOLITION (C&D) LANDFILL. THE CONTRACTOR IS TO COORDINATE WITH THE OWNER TO VERIFY THE LIMITS OF THE C&D LANDFILL BEFORE GRAVEL DIAPHRAGM CONSTRUCTION. THE GRAVEL DIAPHRAGM SHALL BE OFFSET FROM THE C&D LANDFILL AS NECESSARY.



2 GRAVEL DIAPHRAGM DETAIL
SCALE: NOT TO SCALE

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MONTANTE XEROX SOLAR DEVELOPMENT
DESIGN SUBMITTAL

139 CARACAS DRIVE
WEBSTER, NY

DETAILS

C-302

Sheet: 11 of 11

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APPENDIX B
GEOBALLAST FOUNDATION SPECIFICATION SHEET

APA SOLAR RACKING

APASOLAR.COM
WHERE INNOVATION MEETS AUTOMATION



NO MORE CONCRETE

By utilizing locally sourced quarry rock, simply drop the weight in and you're done. No more waiting on concrete trucks, renting concrete pumps, or washing out trucks onsite. No more labor hours for setting up temporary concrete molds. No more waiting 24 hours for concrete to cure. The flow and speed of your job is 100% in your control.

RAPID SETUP

The galvanized steel wire box is delivered to the site over 70% pre-assembled. Simply unfold the box, install the spiral wires and connect the anchor tubes. The Geoballast foundation is then fully assembled and can be moved to the proper position in the row and filled with quarry rock. It's easy to assemble, stage, and stringline.

GEOBALLAST FOUNDATION

The **Geoballast Foundation** was developed after years of installing ballasted solar projects. Concrete, whether pre-cast or pour in place, proved to be an expensive and time-consuming method. Our innovative engineering and R&D teams developed a revolutionary process for ballasted projects. The goal was to remove all concrete and take the idea of a standard gabion basket and engineer it to excel as a ballast solution. Our highly engineered Geoballast box has the fastest installation time available, and is one of the most cost effective products on the market.

In business since 2008, APA offers a versatile line of racking and foundation solutions for projects in even the most challenging environments. With projects nationwide, APA is a trusted racking partner.

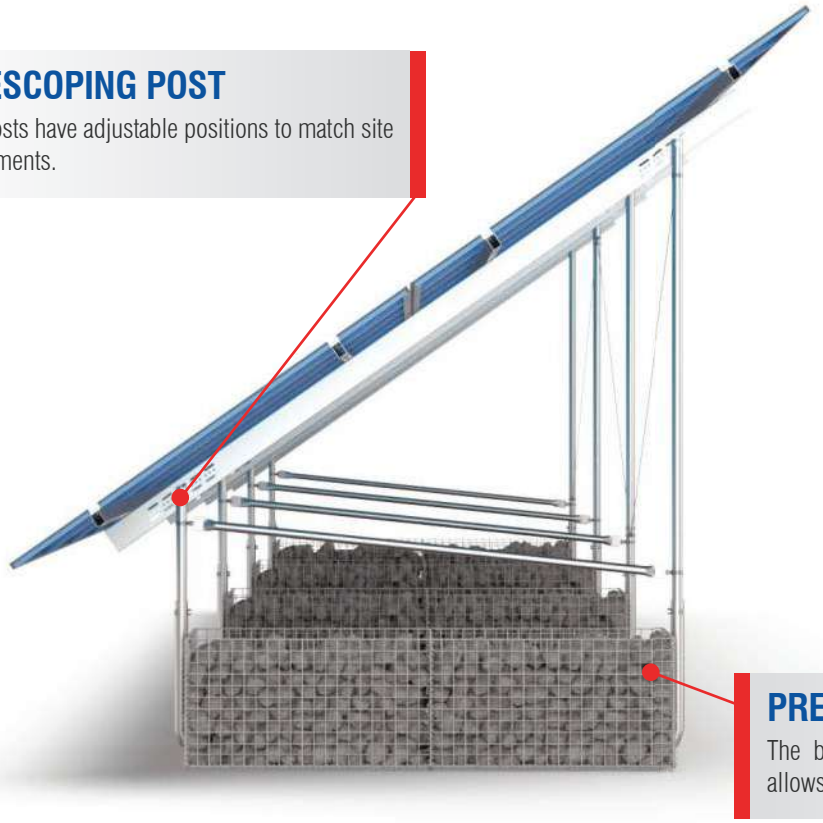
20-345 COUNTY ROAD X \\ PO. BOX 224 \\ RIDGEVILLE CORNERS, OH 43555

419.267.5280 // SALES@APASOLAR.COM

WHY USE A GEOBALLAST FOUNDATION?

TELESCOPING POST

Both posts have adjustable positions to match site requirements.



STANDARD SPECIFICATIONS

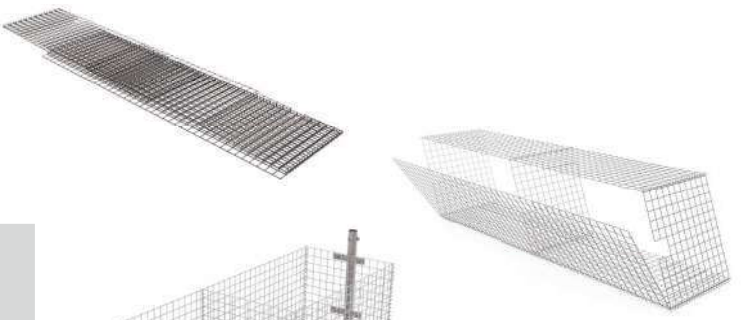
Engineering: APA Drawings can be PE stamped for all 50 States and territories
Tilt Angles: 5°-35° Tilt Options
Wind Loading: Up to 130mph
Snow Loading: Up to 100psf
Mounting Orientation: 2-High in Portrait
Weight Requirement: 2,250 lbs per basket
Foundation Coating: Galvanized with PVC coating for added protection

PRE-ASSEMBLED BASKET

The ballast is shipped 70% assembled, which allows for lower labor cost and quick deployment.

SIMPLE SETUP PROCESS

- Place folded ballast basket on the ground
- Unfold basket and insert lower tube
- Install spiral retainers and u-bolt connections
- Place in desired location and fill with quarry rock





STANDARD QUARRY ROCK

Rock can be sourced from local quarries to reduce shipping costs.

APA SOLAR RACKING

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READYRACK™
GEOBALLAST FOUNDATION



APA SOLAR RACKING

INSTALLATION MANUAL

Updated: February 2023

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3	PARTS LIST
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5	REVIEW & INSTALLATION PREPARATION, UNEVEN GROUND, MEASURING
6	BASKET ASSEMBLY
7	BALLAST STAGING & FILL WITH AGGREGATE, POST INSTALLATION
8	POST INSTALLATION (CONTINUED)
9	TUBE CLAMP STAGING, CROSS BRACE TUBE INSTALLATION
10	N/S CHORD-TO-POST, FINE HEIGHT ADJUSTMENT, E/W STRUT PURLIN-TO-N/S CHORD, E/W STRUT PURLIN SPLICE
12	PANEL INSTALLATION
13	CABLE BRACE-TO-POST
14	PRO TIPS
14	O&M REQUIREMENTS

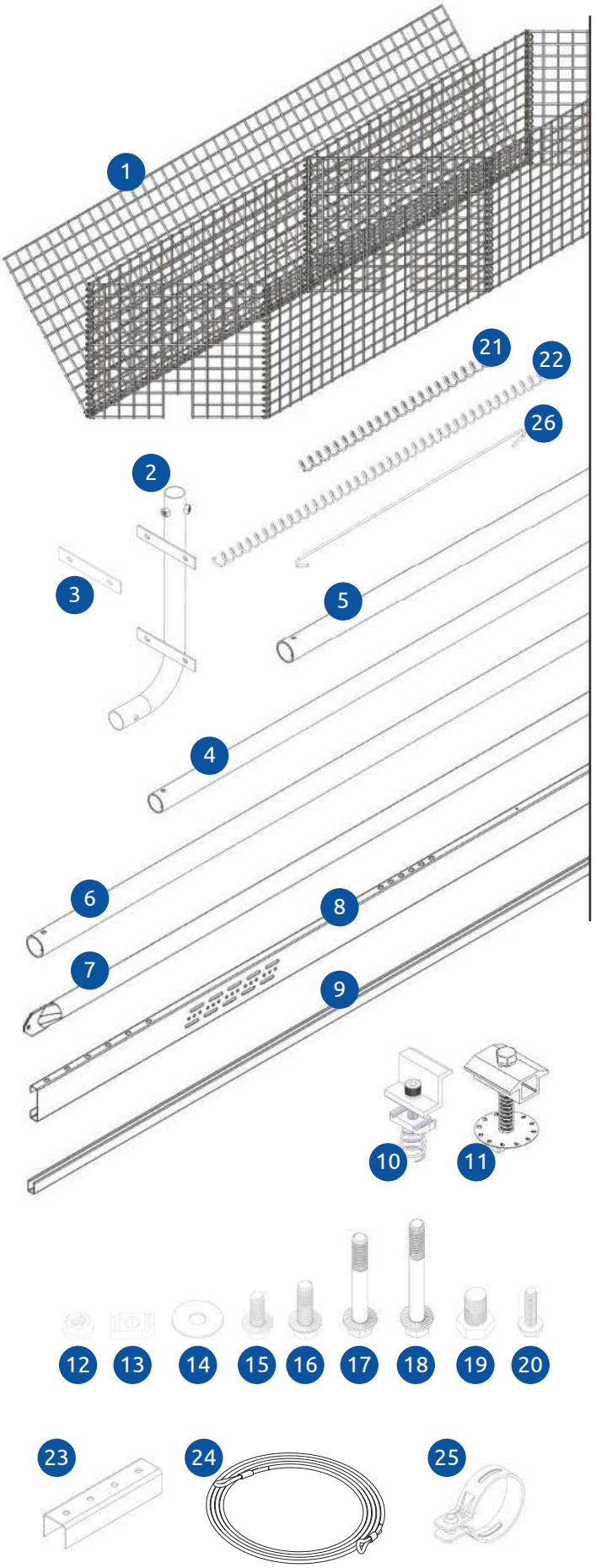
PARTS LIST

STRUCTURAL MEMBERS

[1] WIRE BASKET ASSEMBLY
FOLDED WIRE BASKET
SHORT SPIRAL RING [21]
LONG SPIRAL RING [22]
[2] ELBOW TUBE
[3] ELBOW CLAMP PLATE
[4] BASE TUBE
[5] FRONT (SOUTH) POST
[6] REAR (NORTH) POST
[7] CROSS BRACE TUBE
[8] N/S CHORD
[9] E/W STRUT PURLIN

HARDWARE

[10] END CLAMP ASSEMBLY
[11] MID CLAMP ASSEMBLY
[12] 3/8-16 SERRATED FLANGE NUT
[13] 3/8-16 CHANNEL NUT
[14] 3/8 FENDER WASHER
[15] 3/8-16X1.00” SERRATED FLANGE BOLT
[16] 3/8-16X1.25” SERRATED FLANGE BOLT
[17] 3/8-16X2.75” SERRATED FLANGE BOLT
[18] 3/8-16X3.00" SERRATED FLANGE BOLT
[19] M16-2.0X25 HEX SET BOLT
[20] TUBE CLAMP BOLT
[21] SHORT SPIRAL RING
[22] LONG SPIRAL RING
[23] STRUT SPLICE
[24] CABLE BRACING
[25] TUBE CLAMP
[26] BASKET STABILIZER



CONSTRUCTION 101

All structures, regardless of how complex, are built one step at a time. Each new step builds on the previous. In order for the final structure to be complete, it is best to ensure that each step is done correctly. It is far easier, faster, and cheaper to take your time to ensure each step is done correctly before moving on. There is no remedy for an error other than a correction. Time and effort will be put forth eventually to redo steps, reconstruct, and make adjustments, but far less effort is required to correct mistakes as they appear. If errors are fixed immediately, each subsequent step will begin correct and projects will run more efficiently.

It is recommended to completely install a test row from beginning-to-end of the row, and from beginning-to-end of all steps (from layout, to panel installation, and adjustments). This will help you understand not only how to install the product, but understand how tolerances stack up, how mistakes in one step cause errors in other steps, and how to make the adjustments required to keep everything in tolerance and looking good.

SITE PREPARATION

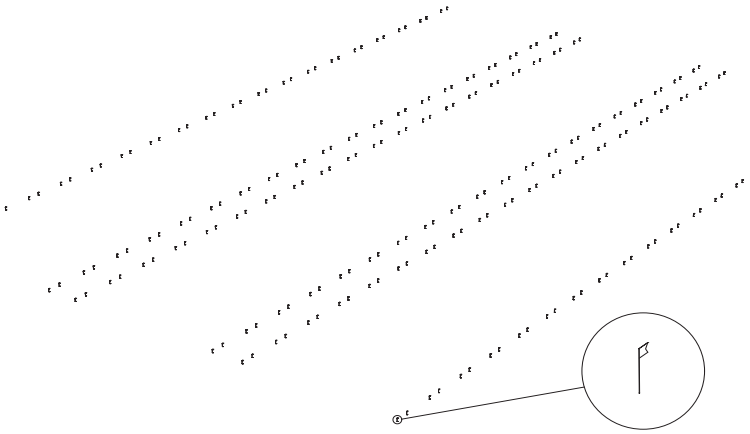
Site preparation should be conducted prior to construction. This will typically involve grubbing, rock and debris removal, and any other preparation that will facilitate swift and unhindered installation. For Geoballast systems, this means the preperation of level stone pads.

UTILITIES

All utilities should be marked before any construction begins.

SURVEYING

To ensure that the solar array is installed according to customer plans, the site should be surveyed according to the scope of work. This includes the North and South elbow tube locations for the Geoballast system. For projects with significant topography, survey points should be checked with a tape measure.



NOTE

Foundation locations should be surveyed by a professional surveyor, who can stake out row locations to the highest precision.

REVIEW & INSTALLATION PREPARATION

After site preparation has been completed and the site surveying is done, the foundations are ready to be installed. It is the responsibility of the site lead to review and understand the site foundation layout, surroundings, installation procedures, and to manage the installation to completion. Each project has different requirements.

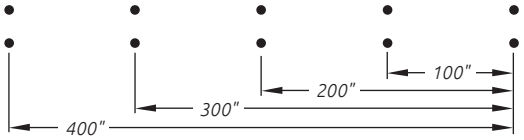
Dimensions in the East-West direction will change depending on row length, row location, and location within the row. Closely follow the construction plans and if there are any questions or discrepancies, bring them up to the site lead or project manager before installing any foundations.

UNEVEN GROUND

For sites with ungraded or rolling topography, it is important to ensure that the foundations are in the correct location. In cases where post locations can not be marked on the ground (uneven terrain), it is advised to check the survey points with a steel tape measure.

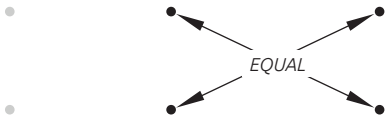
MEASURING

All measurements in the East-West direction must be made with a steel tape measure, and made from the start of the row, not foundation-to-foundation. For example, if the plans were to call for the first 4 spans to be 100" each, the tape should be staked to the ground, and the ground should be marked at 0", 100", 200", 300", 400".



Measuring from foundation-to-foundation allows tolerances to stack up. In this case, a 2" tolerance would have stacked up to be 8" off by only the fifth foundation, and will cause major installation issues further down the road. If necessary, tape measures should be marked prior to use or prints should be marked up to indicate additive measurements (ie. 0", 8'4", 16'8", 25'0", 33'4").

For Geoballast systems, the North and South points must remain in line with each other (one should not "chase" the other). Ensure the points are square every few sets by measuring diagonals. If marks are found to be out of square, make adjustments to correct.

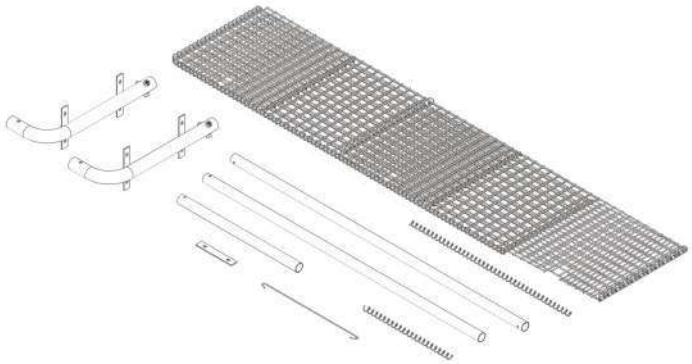


Measuring diagonals

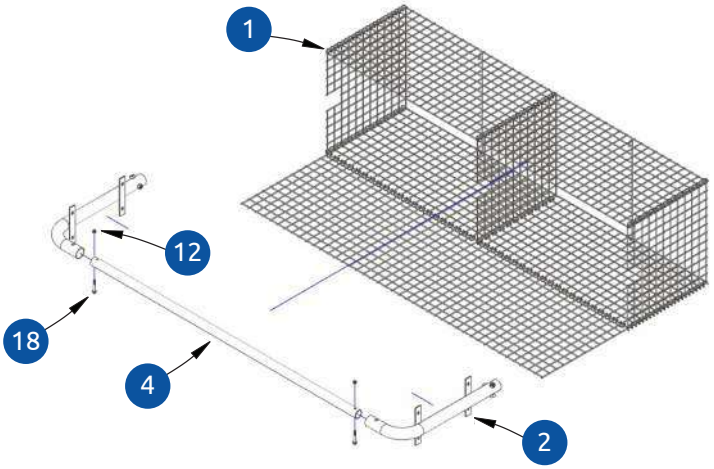
PRO TIP

BASKET ASSEMBLY

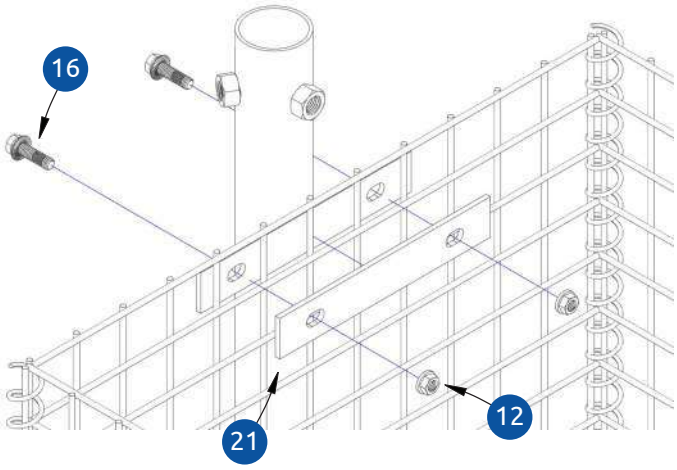
1. Unfold the semi-assembled wire basket assembly.

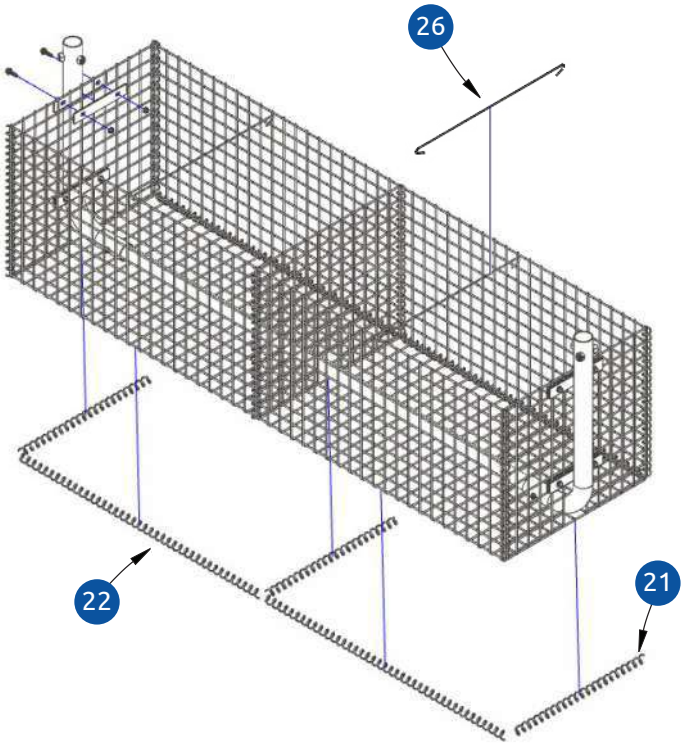


[1] WIRE BASKET ASSEMBLY
[2] ELBOW TUBE
[4] BASE TUBE
[12] 3/8-16 SERRATED FLANGE NUT
[16] 3/8-16X1.25" SERRATED FLANGE BOLT
[18] 3/8-16X3.00" SERRATED FLANGE BOLT
[21] SHORT SPIRAL RING
[22] LONG SPIRAL RING
[26] BASKET STABILIZER



2. Bolt the base tube to the elbow tubes.
3. Place the wire basket assembly on its side and position the elbow and base tube assembly into the bottom of the wire basket assembly so that it is positioned within the cutouts of the mid and end panels of the basket.
4. Fold the bottom panel of the basket back into place and insert the short spirals. Then proceed by inserting the long spirals.
5. Stand the assembly up and check elbow tube for plumbness.
6. Secure the elbow tubes to the basket assembly using the elbow clamp plate by placing it inside the basket and pinching the wire mesh of the basket between the two surfaces.
7. Install stabilizer to ensure basket integrity.



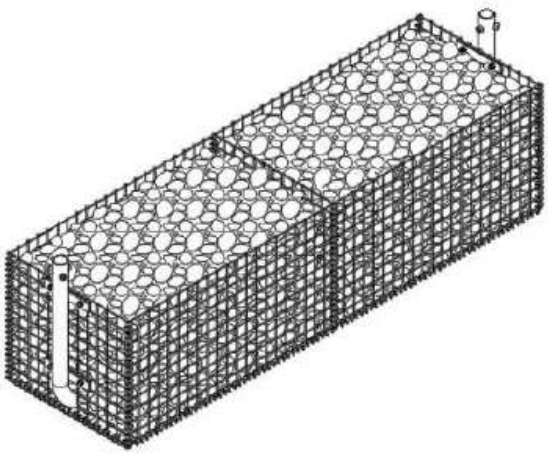


BALLAST STAGING & FILL WITH AGGREGATE

1. An equipment operator should carry the basket and tube assembly to each foundation location.

2. Positing the basket and tube assembly so that the North and South survey points align with the center of the elbow tube. Stage all ballast assemblies in the row.

3. Once the location has been confirmed, fill the ballast assemblies with aggregate as specified in the construction documents.



POST INSTALLATION

1. Insert the front (south) post at the first point on the south set of elbow tubes.

2. Measuring from the ground to the top of the post, ensure the post is at the correct height according to the construction plans. Also ensure the upper bolt holes are rotated to align in the East-West direction. Temporarily secure the post in place with a quick clamp or similar tool.

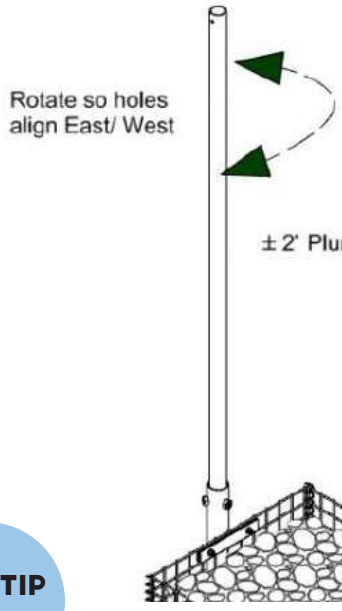
3. Repeat further down the front (south) side of the row with another foundation to create the next attachment point for the string line, once again measuring and ensuring plumbness. When topography is present, additional transition posts must be installed at transition points so that the string line can flow through hills and valleys at a moderate rate. Although the racking will follow terrain, it has limitations. Therefore, ensuring smooth flow will greatly aid the installation process.

4. Continue to the end of the row as required to set a string line.

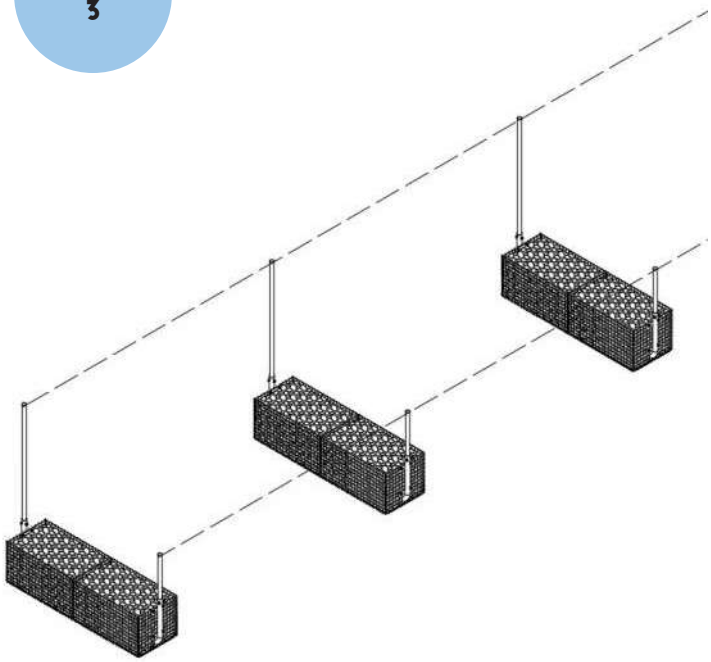
5. Run a string line between all foundations at the same height. For consistency, string line should run along the same side of the foundation for the entirety of the site. For example, always wrap counterclockwise, then install new posts on the south side of the string. This will help prevent zigzagging.

6. Ensure the heights of all posts are correct and flow with the terrain in the method indicated in the construction plans.

PRO TIP 2



PRO TIP 3



POST INSTALLATION (CONTINUED)

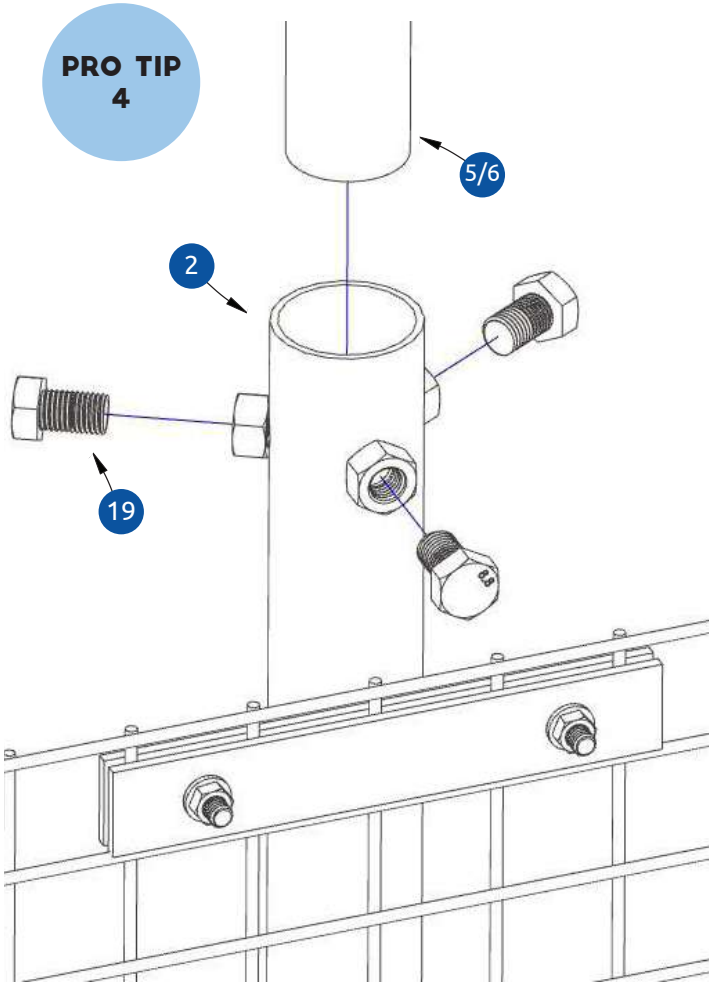
7. Secure the transition posts at the foundations with three (3) bolts and remove the temporary clamps.

[5/6] FRONT (SOUTH)/REAR (NORTH) POST
[2] ELBOW TUBE
[19] M16X2.0 25MM HEX BOLT

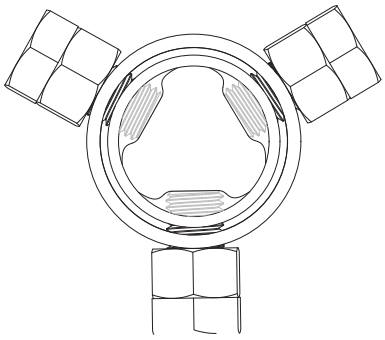
8. Once heights are verified correct and rotation is correct, bolts can be tightened. Every post should be checked to ensure the bolts are fully seated.

9. Once the front (south) posts are installed, set the rear (north) side posts by adjusting the height to the designed tilt angle per foundation set (check the build plans for the correct tilt angle). Set a steel guide on top of the front (south) and rear (north) posts and use an angle finder to determine the tilt. Repeat the string method for the rear (north) side posts.

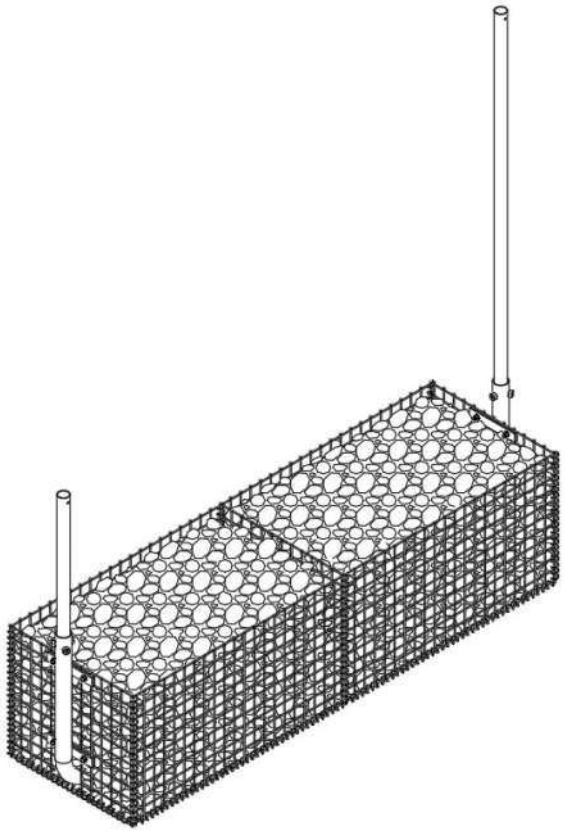
PRO TIP 4



PRO TIP 5



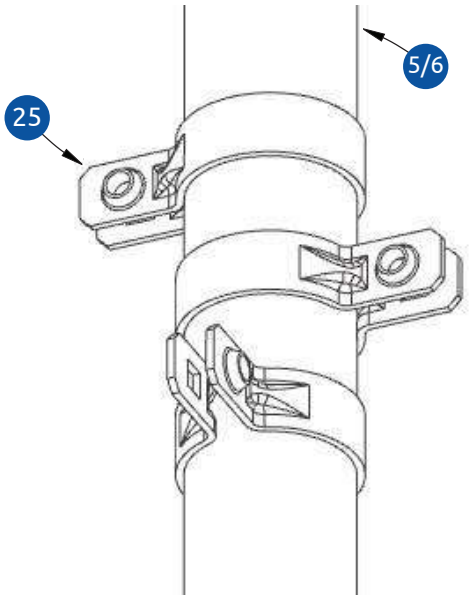
Top view of post and fully tightened bolts



TUBE CLAMP STAGING

1. Slide the tube clamp over the rear (north) post prior to attaching the N/S chord or stretch the clamp apart, fit it around the post, and squeeze it back to its original shape. Refer to the construction prints for the bracing requirements and locations.

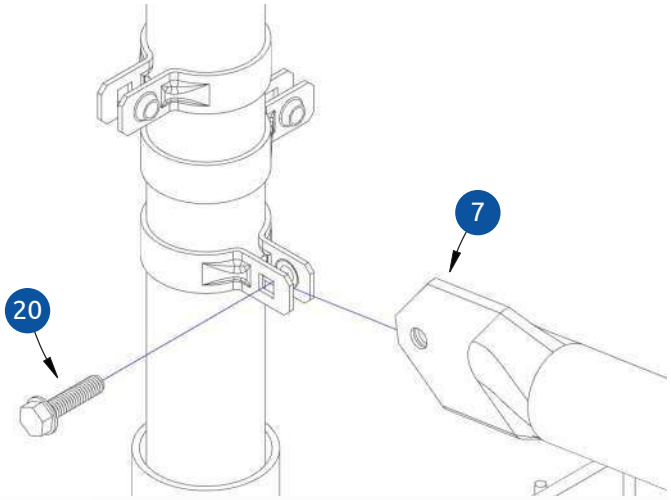
[25] TUBE CLAMP
[5/6] FRONT (SOUTH)/REAR (NORTH) POST

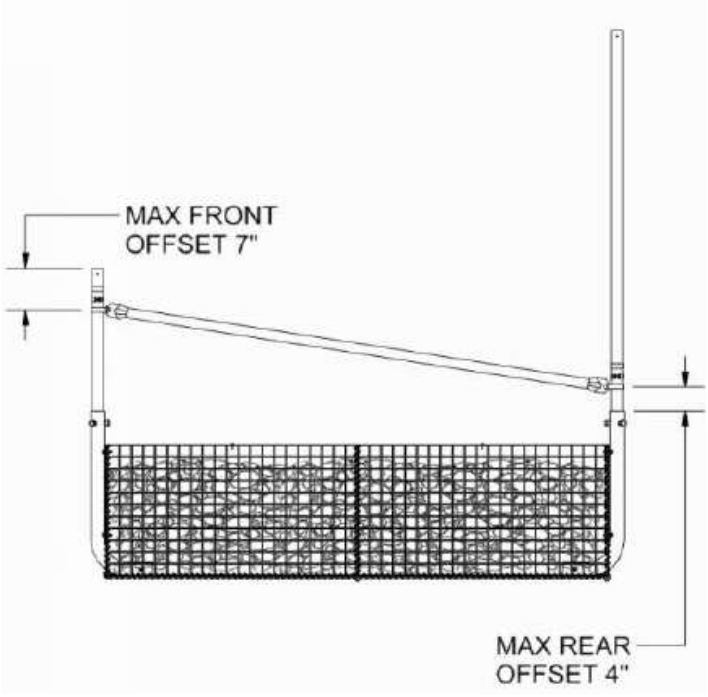


CROSS BRACE TUBE INSTALLATION

1. Install the cross brace tube at the lower location on the rear (north) post and at the upper location on the front (south) post.

[7] CROSS BRACE TUBE
[20] TUBE CLAMP BOLT

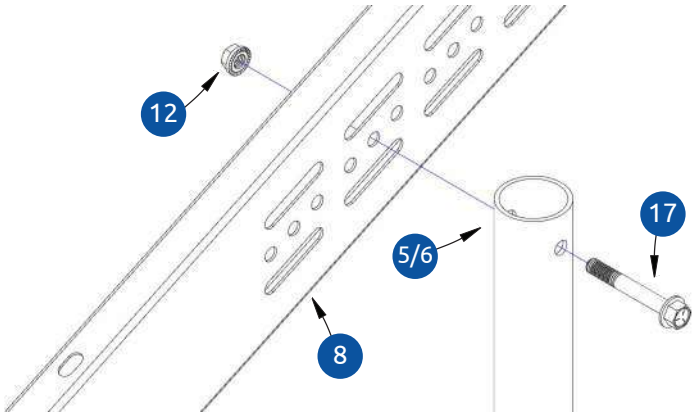




N/S CHORD-TO-POST

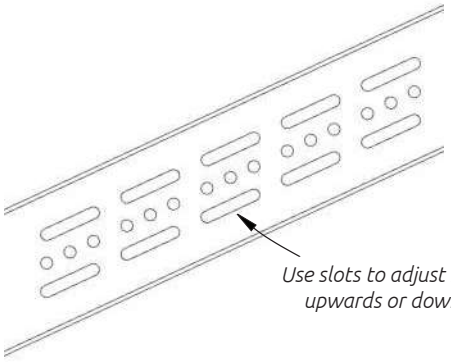
1. Attach the N/S chord to the West side of the posts. Refer to the construction documents for the correct hole locations.

[8] N/S CHORD
[5/6] FRONT (SOUTH)/REAR (NORTH) POST
[17] 3/8-16X2.75" SERRATED FLANGE BOLT
[12] 3/8-16 SERRATED FLANGE NUT



FINE HEIGHT ADJUSTMENT

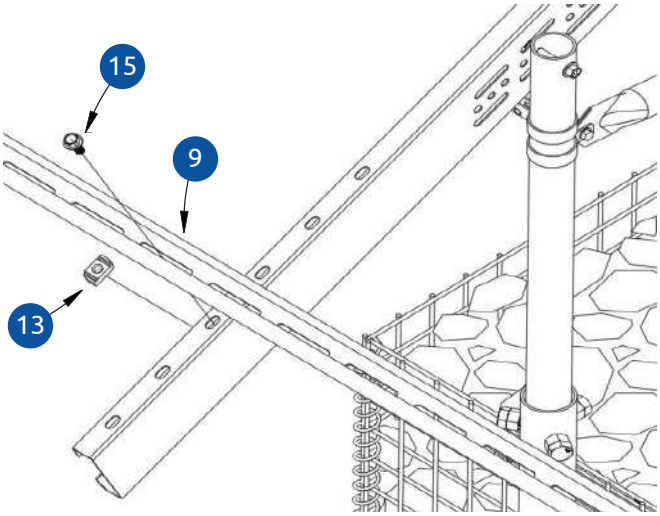
1. Sighting down a row, looking from East or West, look for inconsistencies in the heights of the racking. Adjust the height by removing the N/S chord hardware and adjusting the height using the adjustment holes before re-attaching the hardware.



E/W STRUT PURLIN-TO-N/S CHORD

1. Bolt the E/W strut purlin to the N/S chord using the correct hole location per the construction documents.

[9] E/W STRUT PURLIN
[15] 3/8-16X1.00" SERRATED FLANGE BOLT
[13] 3/8-16 CHANNEL NUT

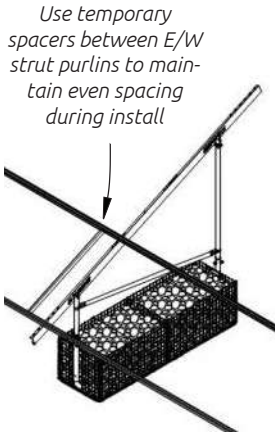


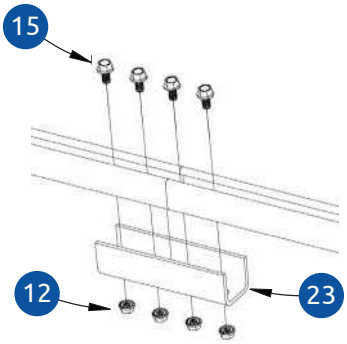
E/W STRUT PURLIN SPLICE

1. Connect two E/W strut purlins by using the splice to make a continuous rail. Stagger the location of the splices between foundations to reduce sagging.

[23] STRUT SPLICE
[12] 3/8-16 SERRATED FLANGE NUT
[15] 3/8-16X1.00" SERRATED FLANGE BOLT

2. Ensure the bottom or southern most E/W strut purlin is straight by adjusting it in the slot or by moving the N/S chord. Once visually straight, fasten down. Use spacers (scrap wood, scrap metal, etc.) to set the remaining E/W strut purlins to keep even spacing per the construction documents.





PANEL INSTALLATION

1. Beginning at one end, start setting the panels by squaring the first panel to the bottom E/W strut purlin. Refer to the construction documentation to determine the correct orientation of the panel.

2. Place the top panel after squaring the bottom panel. Make sure the spacing between the top and bottom panel is correct. Note: Use a fixture to create the proper gap, such as a piece of wood and spare clamp from the site.

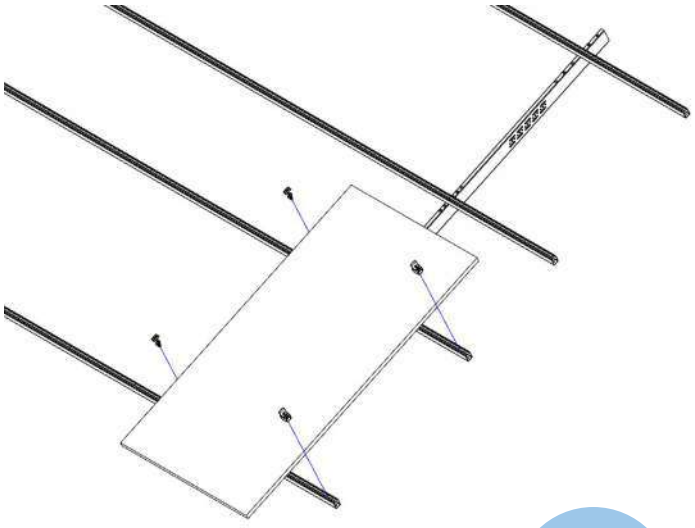
3. Place transition panels periodically throughout the row, squaring them to the rack as you go. Once these transition panels have been placed, run a string across them to create your leading edge for the row. This is done to provide the best aesthetics while keeping the panel placement within specification.

4. Stage panels throughout the row in preparation to be set to the string line.

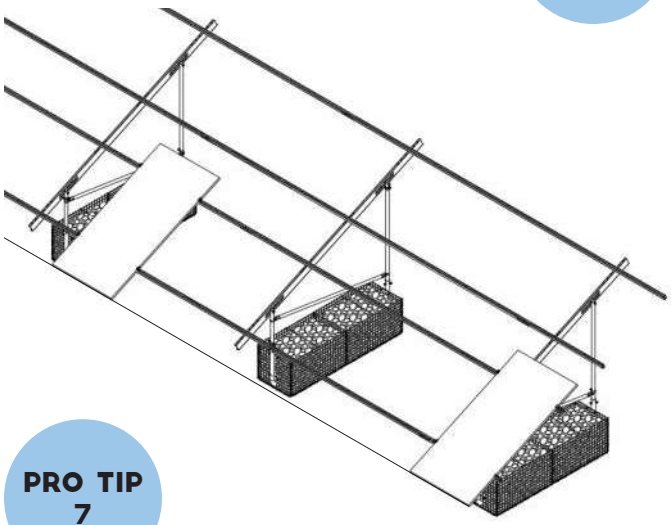
5. When starting panels, mid clamps can be placed in any area of the E/W strut purlin, except for splice locations.

6. Tighten the clamps square to the panels and ensure the grounding teeth are coming into contact with the panel frame.

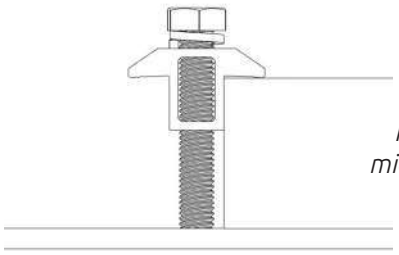
7. Repeat steps down the row, moving transition panels as you reach them if necessary.



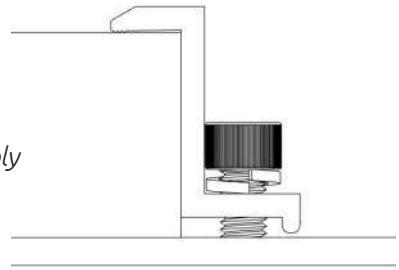
PRO TIP 6



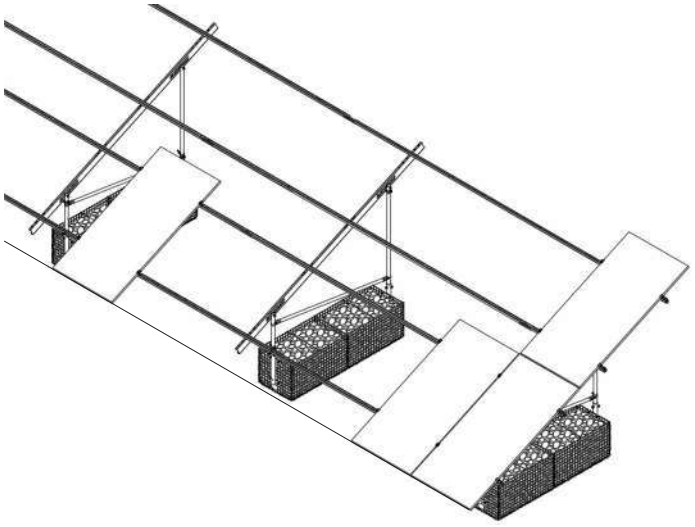
PRO TIP 7



Properly seated mid clamp assembly

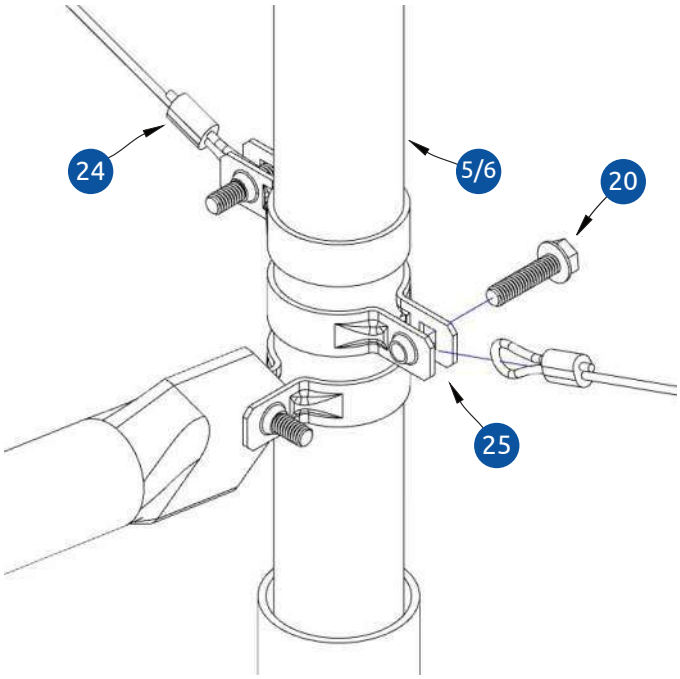


Properly seated end clamp assembly



CABLE BRACE-TO-POST

Depending on the array and project parameters, a set of posts may get multiple sets of cable braces to limit movement and reduce fatigue. Refer to the construction documentation for locations and frequency of cable bracing.

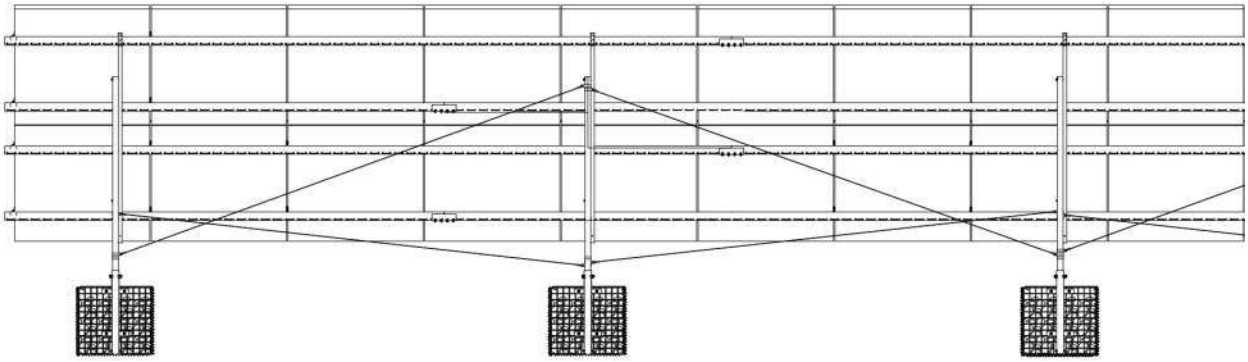


1. If clamps were not installed prior to installing the N/S chord (highly recommended), spread the clamp apart, slide around the post, and re-compress it. Install clamps on the North and South posts as required.
2. Once clamps are installed, string a cable between the two clamps. Note: There may be multiple lengths of cables on one job; refer to construction documentation to use the correct one.
3. Secure cable with bolts, loosely. Ensure that the bolts are pointing to the interior of the rack.
4. Install all cables as indicated in the construction documents. Cables should be taut and have no noticeable slack or sag.

[5/6] FRONT (SOUTH)/REAR (NORTH) POST
[20] TUBE CLAMP BOLT
[24] CABLE BRACING
[25] TUBE CLAMP

Note: In areas involving terrain, custom bracing may be needed. This is achieved by cutting a piece to the correct length off of a spool. It will then be crimped by a swagging tool.

NOTE
Cable braces are not used to induce static tension, like trusses of a bridge, but instead only to keep the posts from spreading or shifting.



PRO TIPS

PRO TIPS are pieces of advice from installation experts. They are intended to provide practical solutions to real-world problems that you may encounter. But before implementing any of the below tips, make sure to request approval from APA's Engineering team, as not all solutions are allowed on every site.

[1] Use a STEEL tape rather than a fiberglass tape to measure distances over the length of the row. Fiberglass can stretch over time and affect layout and installation lengths.

[2] When dealing with extreme topography, additional transitions will have to be added to make the racking flow with the terrain.

[3] With extreme topography, every foundation must be marked and the string line must be attached to every post, in order to ensure that the racking will flow at every high and low point.

[4] While it is very difficult to make every post perfectly plumb and at the correct height, care should be taken to keep them within the tolerances specified in the plans in order to ensure that the system fits and functions as intended.

[5] Use grip clamps when setting post transitions . This is the best technique to use so that no improper dimpling is done to the posts if adjustments are needed. Once desired flow is reached, then tighten the set bolts.

[6] When staging panel clamps, tighten the bolt until the spring is slightly compressed. This will help the panel installer free up one hand. If the spring is not compressed prior to staging, the clamp will spin and another installer will have to hold the panel.

[7] To square the panel, measure the distance from the edge of the panel to the flat part of the E/W strut purlin on both sides, ensuring equal distance.

OPERATION & MAINTENANCE

Regular inspections and proper maintenance are essential to maintain the design life and warranty of the Geoballast solar racking system. Solar racking is exposed to many elements from initial construction to seasonal changes and can even be susceptible to severe weather conditions. The following Operation and Maintenance procedures should be followed and performed on an annual basis to ensure the warranty for the racking system remains active. The procedures below highlight the critical points to be examined and maintained for a properly functioning solar racking system.

BOLTED CONNECTIONS

Annually, bolted connections need to be visually inspected to ensure the components are properly joined together. At the beginning, middle, and end of each row, there are torque marked nuts/bolts per the APA QA/QC process. These marked locations should be visually inspected to confirm no movement of the bolted connections has occurred. If loose hardware is found, re-tighten to the specifications noted in the plan set for the racking system.

SOLAR PANEL CLAMPS

Clamps need to be visually inspected annually to ensure modules are properly secured. If loose clamps are found, retighten to the specifications noted in the plan set for the racking system.

GALVANIZE COATING

The galvanized coating ensures that the steel components are protected from corrosion and that the solar racking system will last for the entire project life. Annually, the solar racking system should be inspected for any areas of corrosion or rust. If areas are found, they should be repaired in a timely manner by cleaning the area and then generously applying cold galvanizing compound.

The cut, slit and sheared edges of the racking system are exposed steel that are expected to show corrosion or rust. This is normal and should be expected, but does not affect the structural integrity of the system.

Copper should be isolated and cannot touch the galvanized coating, as it will expedite corrosion.

CROSS BRACING CABLE WIRE

If applicable to your project, the bracing needs to be visually inspected on an annual basis to ensure that it has not become loose. Cables are required to be taut for motion limiting, but do not need to be tensioned.

SYSTEM DAMAGE

The solar racking system should be visually inspected for other types of damage. Examples include damage from mowing equipment, maintenance equipment, falling trees or branches, storm damage, system lean, soil erosion, etc. Any components that are damaged or deformed should be replaced as they may be weaker or corrode faster than intended. Some components may be able to be repaired onsite if replacement is not practical. For assistance with replacement parts or instructions on repairing a component, contact APA.

GENERAL OPERATION

It is important to maintain the solar racking system to ensure the design lifespan.

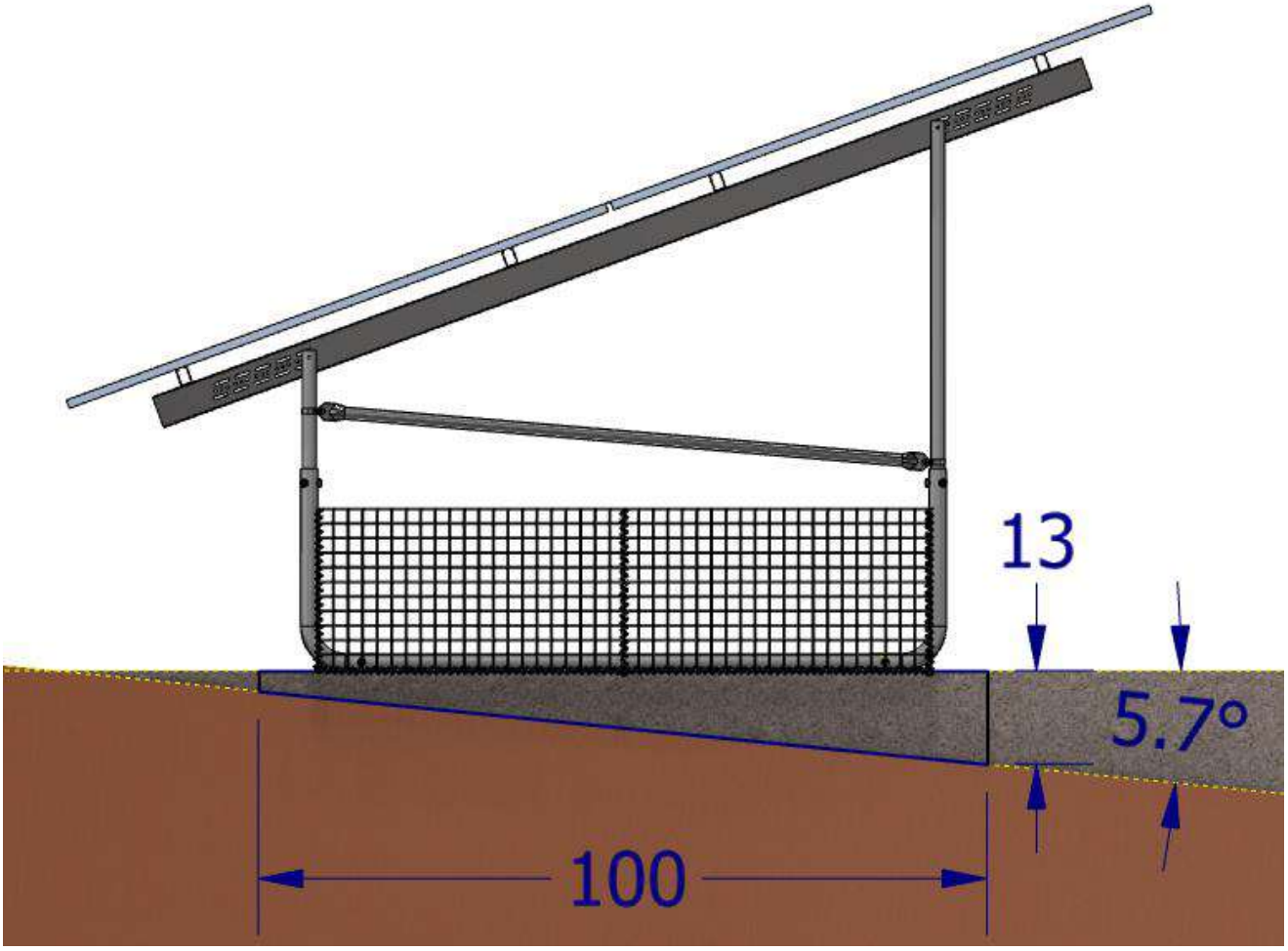
Keep all vegetation managed and off of the racking system and solar modules.

Check that module cleaner solution, weed killer, or other chemicals used on or around the solar racking system are not corrosive to galvanized or stainless steels.

Check electrical wiring to ensure it is managed properly and protected.

Take note and log when inspections were performed and if any corrective actions were taken or need to be taken in the future.

READY RACK GEOBALLAST GRAVEL PAD ANALYSIS



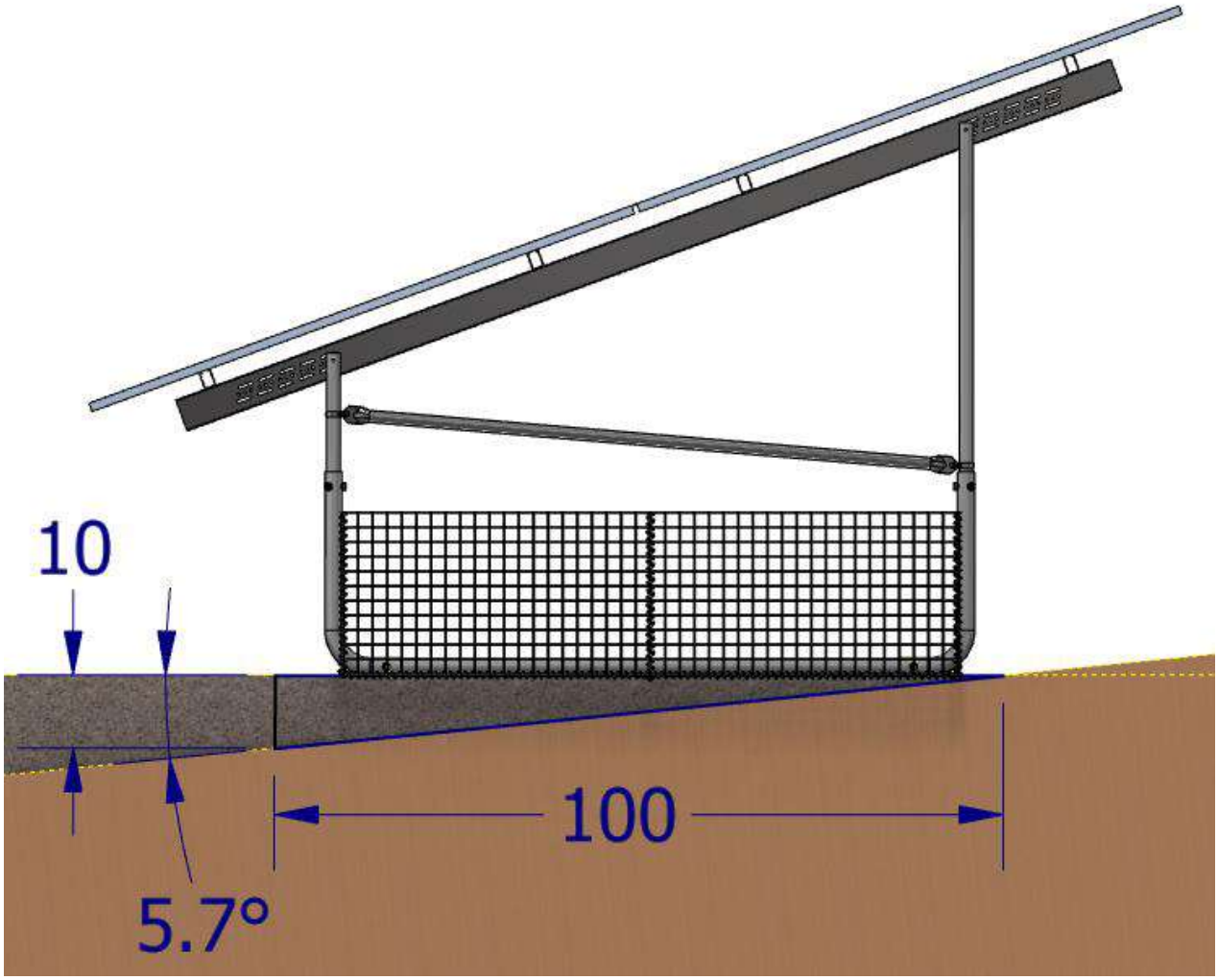
FOR NORTH-FACING SLOPES OF 10% (5.7°):

$$V = (\frac{1}{2}) \text{ X b X h X w}$$

$$V = (\frac{1}{2}) \text{ X 100 X 13 X 30}$$

$$V = 19500 \text{ in}^3 = 11.3 \text{ ft}^3 = 0.42 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



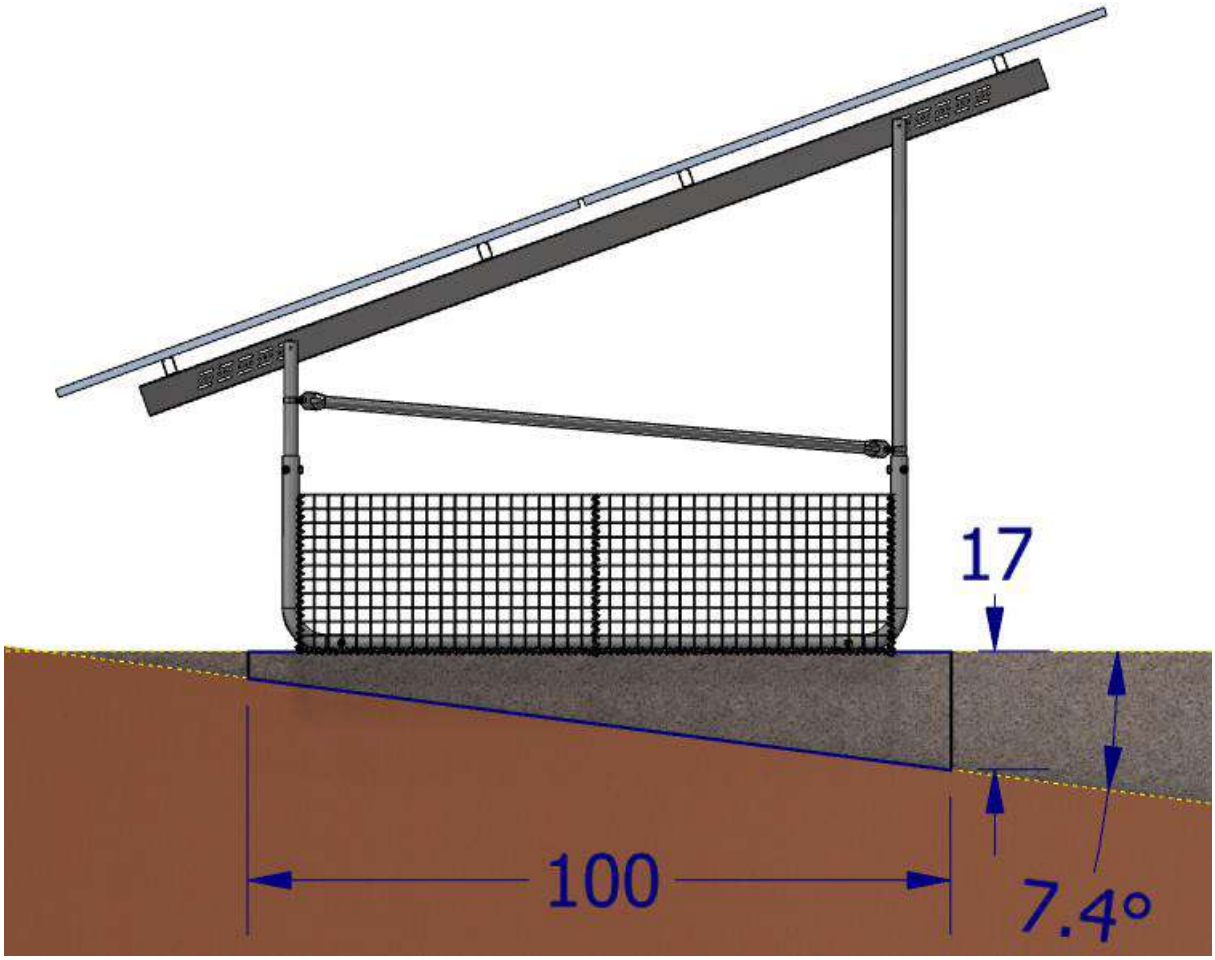
FOR SOUTH-FACING SLOPES OF 10% (5.7°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 100 \text{ X } 10 \text{ X } 30$$

$$V = 15000 \text{ in}^3 = 8.7 \text{ ft}^3 = 0.32 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



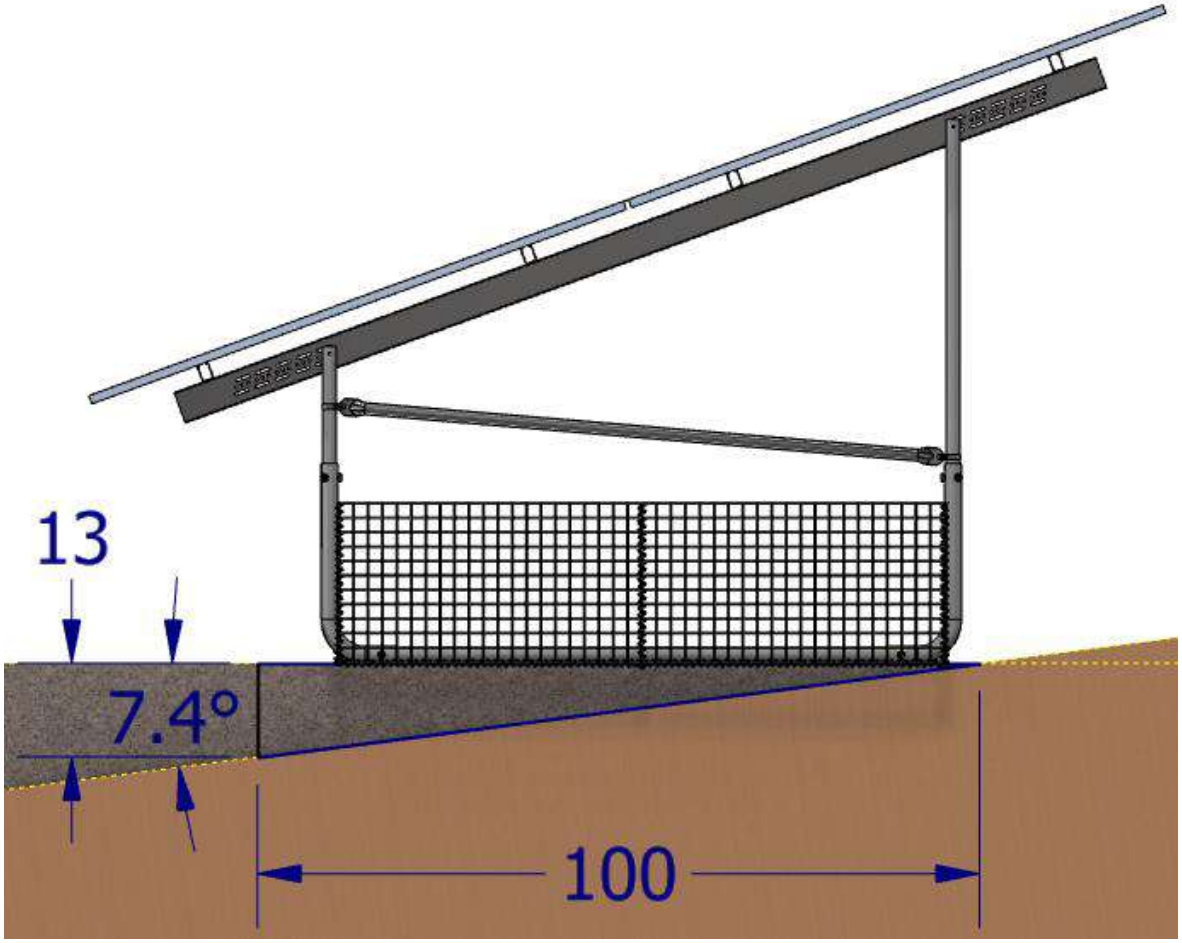
FOR NORTH-FACING SLOPES OF 13% (7.4°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 100 \text{ X } 17 \text{ X } 30$$

$$V = 25500 \text{ in}^3 = 14.8 \text{ ft}^3 = 0.55 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



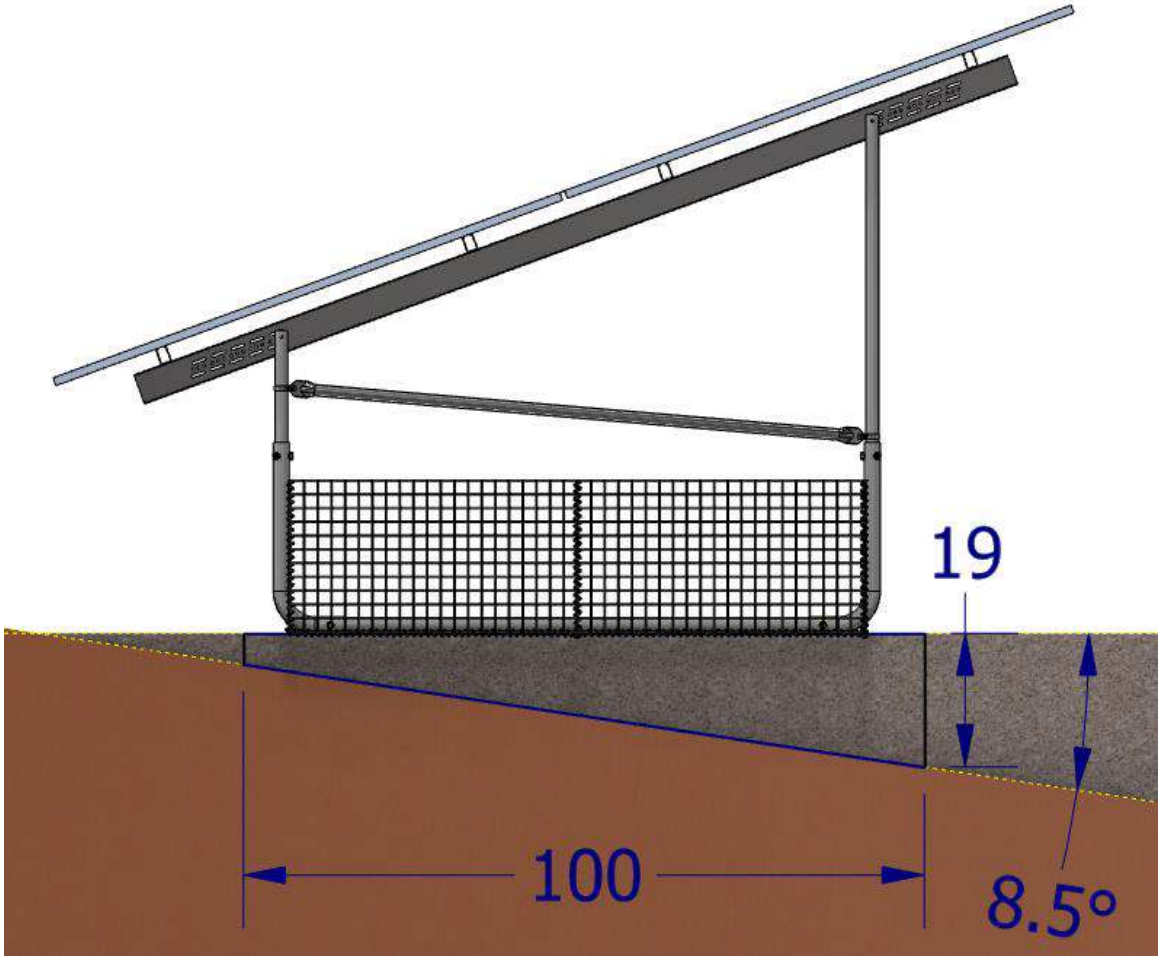
FOR SOUTH-FACING SLOPES OF 13% (7.4°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 100 \text{ X } 13 \text{ X } 30$$

$$V = 19500 \text{ in}^3 = 11.3 \text{ ft}^3 = 0.42 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



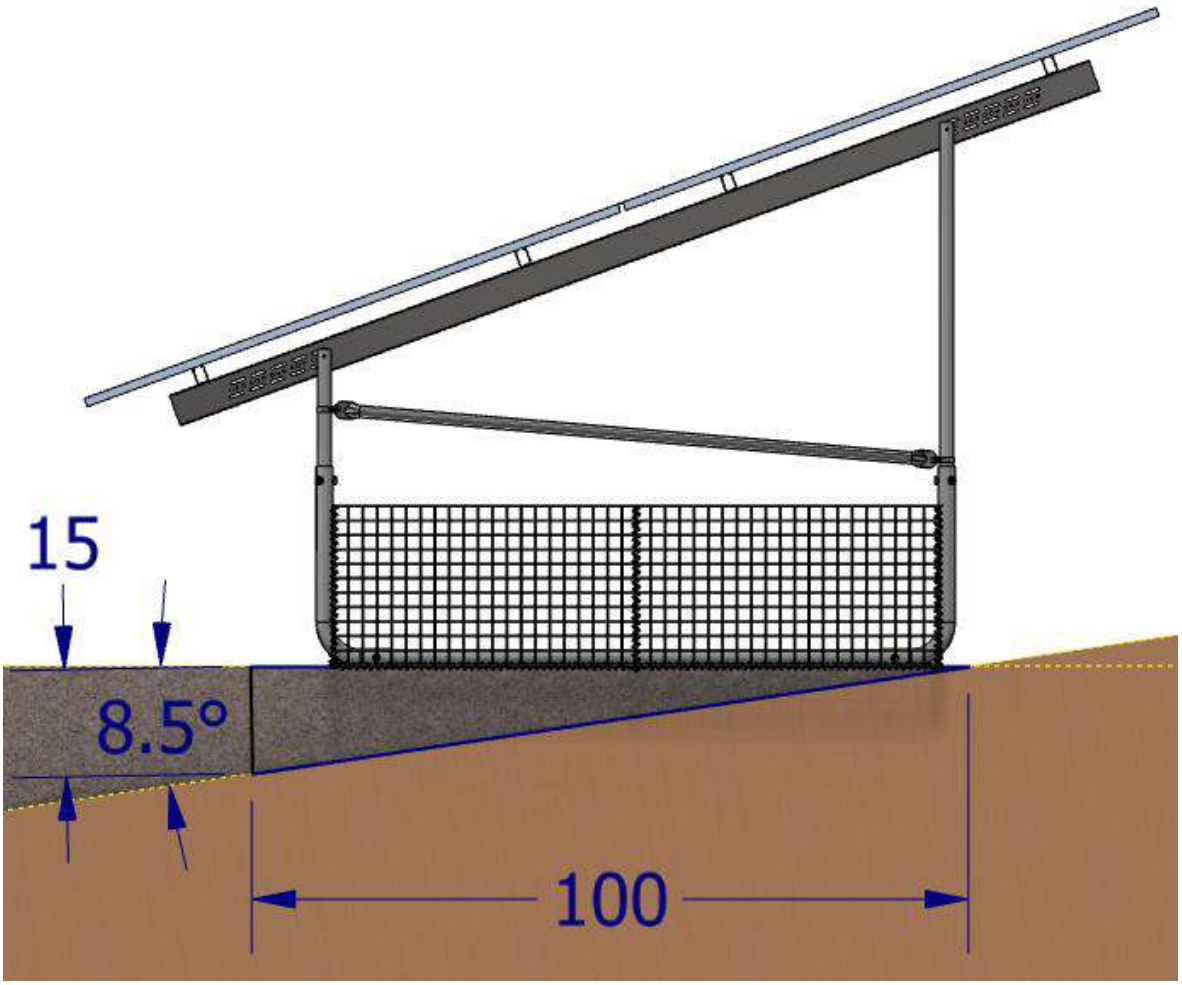
FOR NORTH-FACING SLOPES OF 15% (8.5°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 100 \text{ X } 19 \text{ X } 30$$

$$V = 28500 \text{ in}^3 = 16.5 \text{ ft}^3 = 0.61 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



FOR SOUTH-FACING SLOPES OF 15% (8.5°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

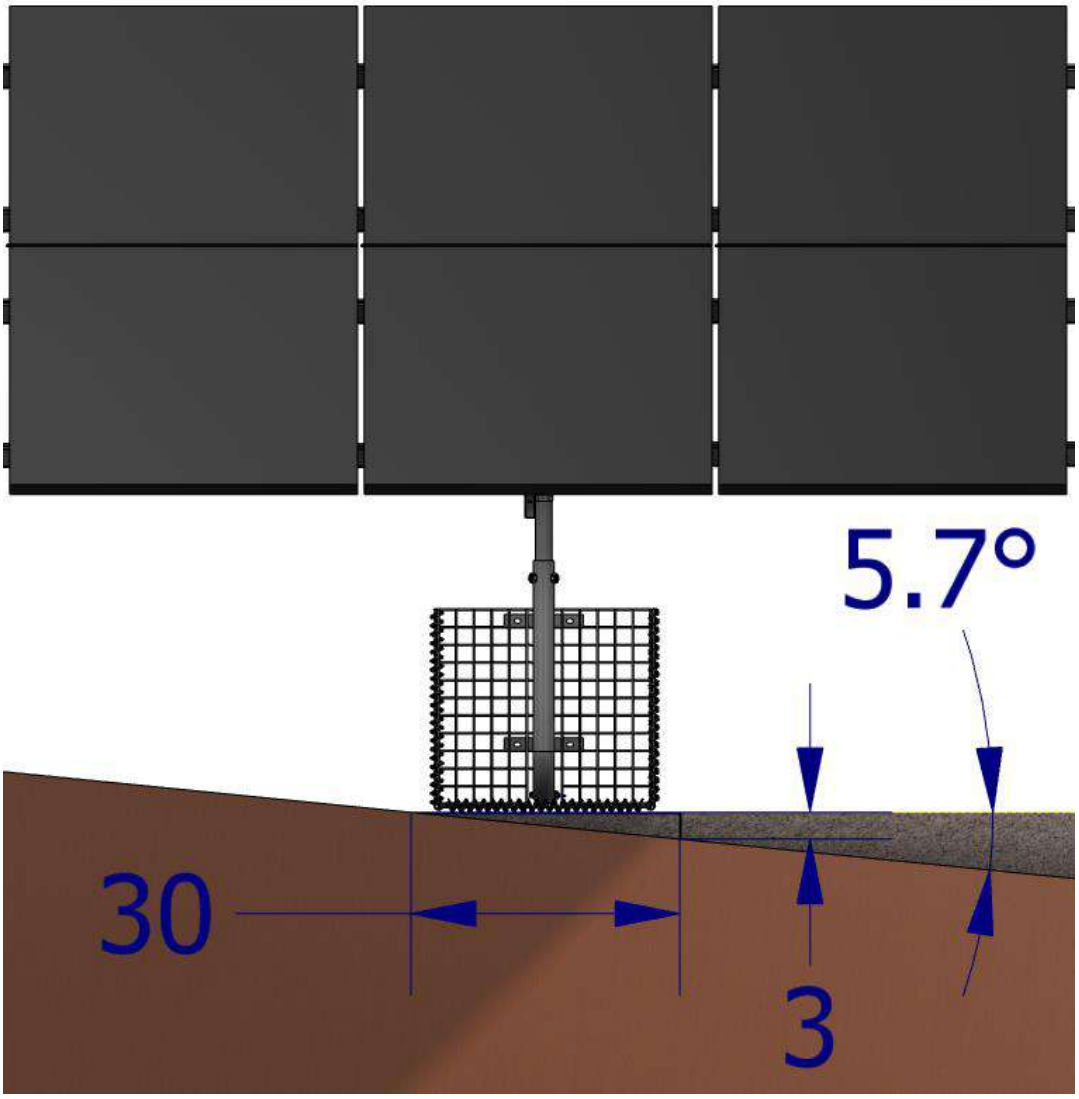
$$V = (\frac{1}{2}) \text{ X } 100 \text{ X } 15 \text{ X } 30$$

$$V = 22500 \text{ in}^3 = 13.0 \text{ ft}^3 = 0.48 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.

APA SOLAR RACKING

READYRACK



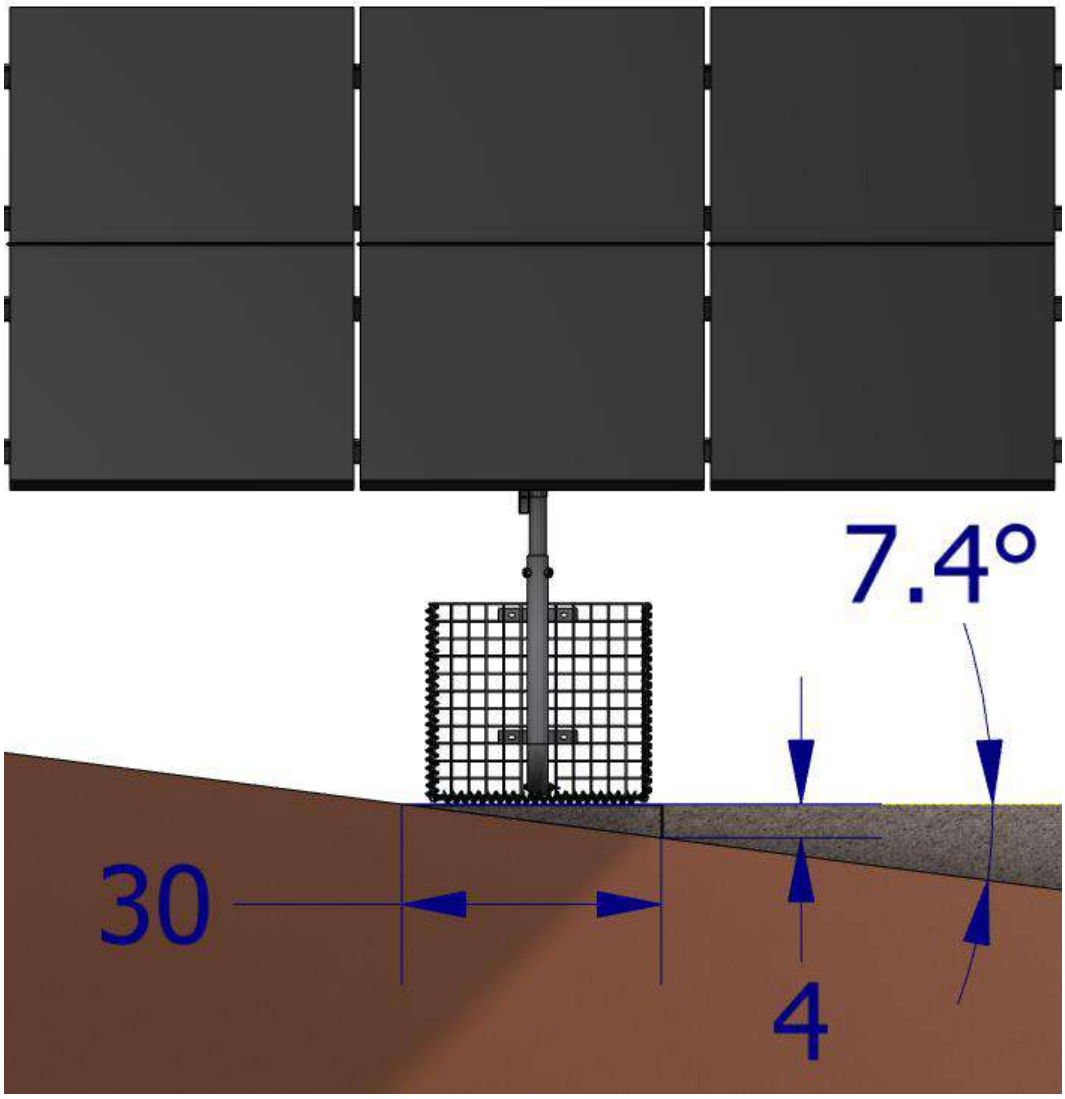
FOR EAST-FACING OR WEST-FACING SLOPES OF 10% (5.7°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 30 \text{ X } 3 \text{ X } 100$$

$$V = 4500 \text{ in}^3 = 2.6 \text{ ft}^3 = 0.10 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



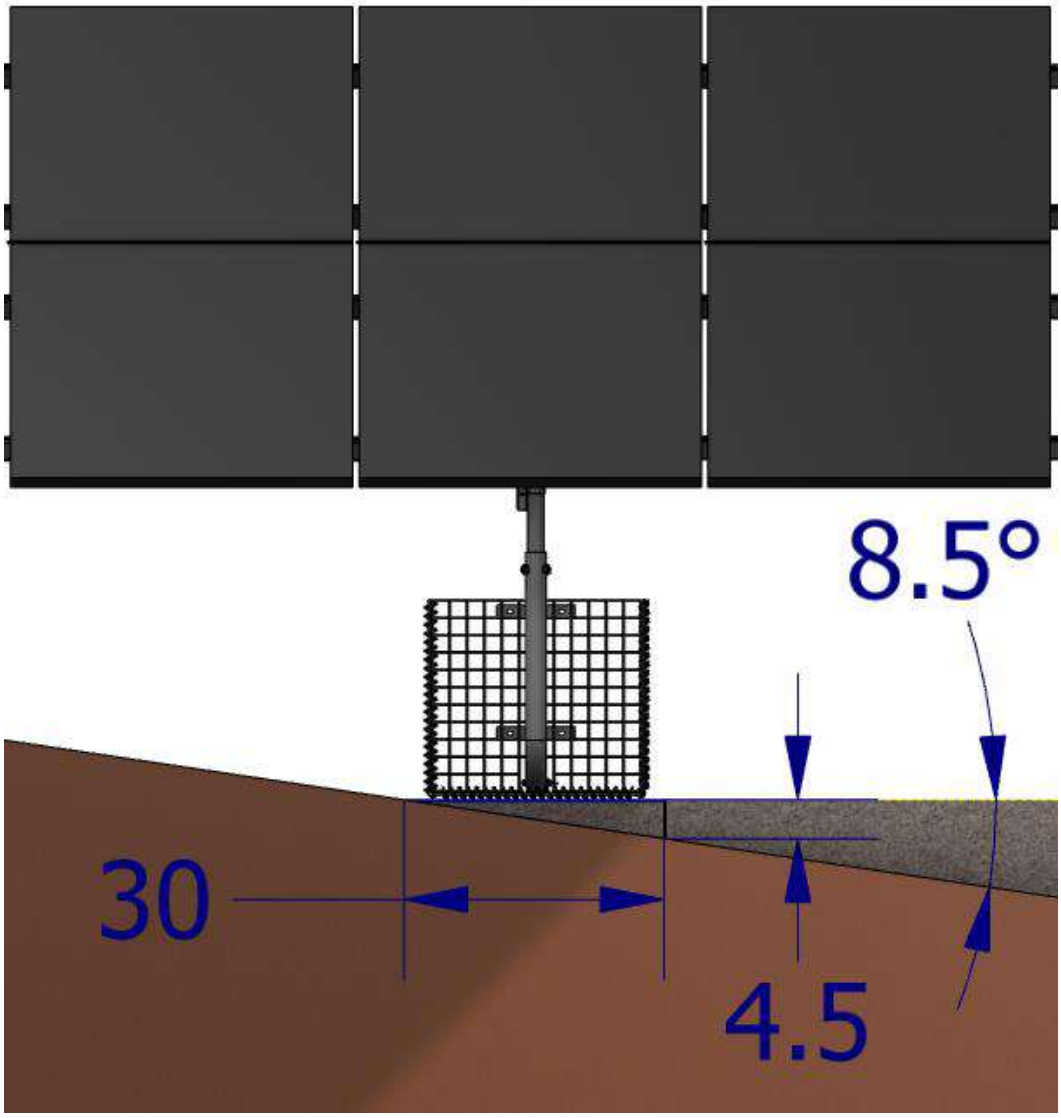
FOR EAST-FACING OR WEST-FACING SLOPES OF 13% (7.4°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 30 \text{ X } 4 \text{ X } 100$$

$$V = 6000 \text{ in}^3 = 3.5 \text{ ft}^3 = 0.13 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.



FOR EAST-FACING OR WEST-FACING SLOPES OF 15% (8.5°):

$$V = (\frac{1}{2}) \text{ X } b \text{ X } h \text{ X } w$$

$$V = (\frac{1}{2}) \text{ X } 30 \text{ X } 4.5 \text{ X } 100$$

$$V = 6750 \text{ in}^3 = 3.9 \text{ ft}^3 = 0.15 \text{ yd}^3$$

NOTE: ALL DIMENSIONS ARE FOR ESTIMATING PURPOSES ONLY. SITE CONDITIONS WILL DETERMINE ACTUAL VOLUME.

APPENDIX C
PROJECT SPECIFIC DRAFT THREE-LINE ELECTRICAL DRAWINGS AND EXAMPLE/REPRESENTAITVE ELECTRICAL DETAILS

Overview:

6,778.32 kW-DC (STC), 4,950 kW-AC PV system containing (18) Chint Power Systems 275 kW Inverters with (11,112) VSUN 610W Modules.

General Notes:

A. All designs were created in accordance with 2017 NEC (NFPA 70) and 2020 NYS Fire Code

B. All equipment to be labeled per NEC article 690 and 705 requirements

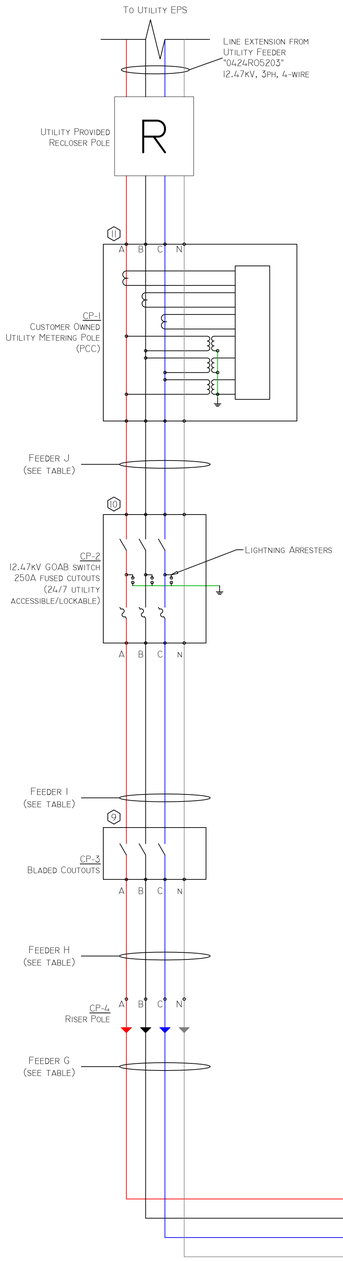
C. All 800V wiring to utilize 1000V rated XHHW-2 conductor insulation

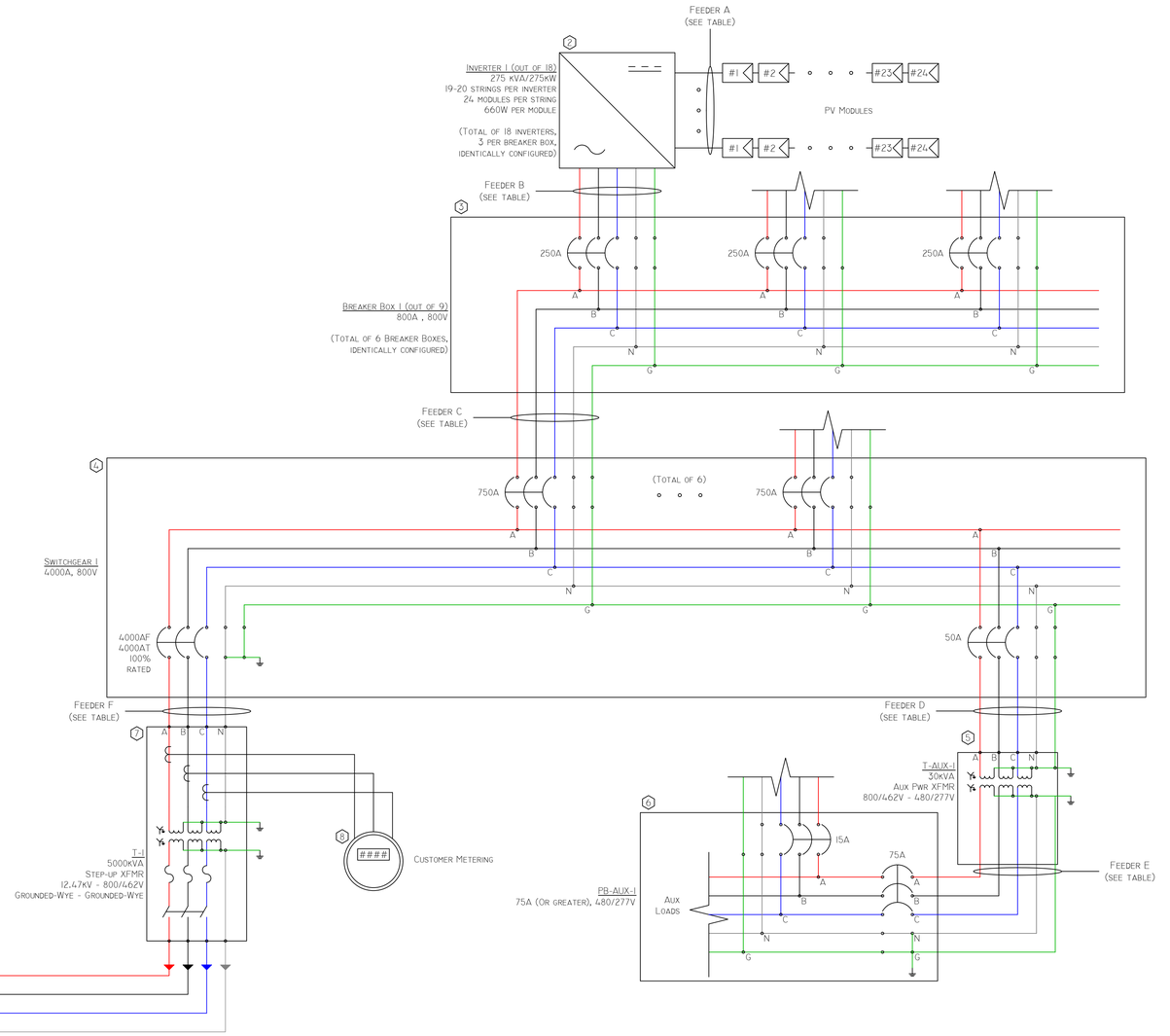
(#) Notes:

1. (11,112) 610W VSUN 72-cell solar module "610-144BMH-DG"
   - Rated to 1500VDC
2. (18) Chint Power Systems "CPS SCH275KTL-DO/US-800" 275kW grid-tied inverter
   - 800VAC, 198.5A Max output
3. AC Breaker Box
   - 3Ph, 800V rated, 800A
   - NEMA 3R
4. Dedicated PV system Main Switchgear - "PV1"
   - 3Ph, 800V rated, 4000A busbar (or greater)
   - 4000A MCB with aux contacts - 100% rated
   - NEMA 3R
5. Aux transformer - "T-AUX-1"
   - Rated at 30kVA
   - Primary: 800V Three Pahse
   - Secondary: 480/277V three Phase
   - NEMA 3R
6. Aux panelboard - "PB-AUX-1"
   - 3Ph, 480/277V rated, 75A busbar (or greater)
   - 75A MCB with aux contacts
   - NEMA 3R
7. Chint Medium Voltage Transformer - "T-1"
   - Rated at 5000kVA
   - Primary: 12.47kV Three Pahse
   - Secondary: 800V three Phase
   - NEMA 3R
8. Customer Metering
   - Rated for 800V
9. Bladed Coutouts - "CP-3"
   - Rated for 15kV
10. GOAB Switch with Lightning Arresters - "CP-2"
    - Rated for 15kV
    - SM-4 100A E Fuses
11. Utility Metering Pole - "CP-1"
    - Rated for 15kV

Wire and Conduit Sizing **See E3 for Feeder Locations**

Feeder	Connection	Load Wire Size	Neutral Size	Ground Size	Conduit Size	Location
A	1 Phase	48-52x 10 AWG Cu PV Wire rated to 1500V	N/A	#6 AWG Cu EGC	2" PVC SCH 80 Raceway	Modules to Inverter
B	3 Phase	3x 350MCM XHHW-2 90°C Al	350MCM XHHW-2 90°C Al	#2 AWG Al EGC	3" PVC SCH 80 Raceway	Inverter to Breaker Box
C	3 Phase	3 sets of: 3x 350MCM XHHW-2 90°C Al	3 sets of: 350MCM XHHW-2 90°C Al	#4/0 AWG Al EGC	2 Sets of 4" PVC SCH 80 Raceway	Breaker Box to Switchgear
D	3 Phase	3x #6 AWG XHHW-2 90°C Al	#6 AWG XHHW-2 90°C Al	#8 AWG Al EGC	1" PVC SCH 80 Raceway	Switchgear to Aux Transformer
E	3 Phase	3x #3 AWG XHHW-2 90°C Al	#3 AWG XHHW-2 90°C Al	#6 AWG Al EGC	1.5" PVC SCH 80 Raceway	Aux Transformer to Aux Panelboard
F	3 Phase	13 sets of: 3x 500MCM XHHW-2 90°C Al	13 sets of: 500MCM XHHW-2 90°C Al	N/A	13 Sets of 4" PVC SCH 80 Raceway	Switchgear to Transformer (T-1)
G	3 Phase	3x #350MCM XLPE	#350MCM XLPE	N/A	4" PVC SCH 80 Raceway	Transformer (T-1) to CP-5
H	3 Phase	3x 4/0 AAC Conductors	4/0 AAC Conductor	N/A	N/A	CP-4 to CP-3
I	3 Phase	3x 4/0 AAC Conductors	4/0 AAC Conductor	N/A	N/A	CP-3 to CP-2
J	3 Phase	3x 4/0 AAC Conductors	4/0 AAC Conductor	N/A	N/A	CP-2 to CP-1





Montante Solar
2760 Kenmore Avenue
Tonawanda, NY 14150

Engineering Stamp No. _____



Unauthorized alteration or addition to this engineering drawing is a violation of Section 7209, Provision 2 of New York State Education Law

Xerox Corporation Solar Array
6.78 MWDC / 4.95 MWAC Photovoltaic System
139 Caracas Dr, Webster, NY 14580

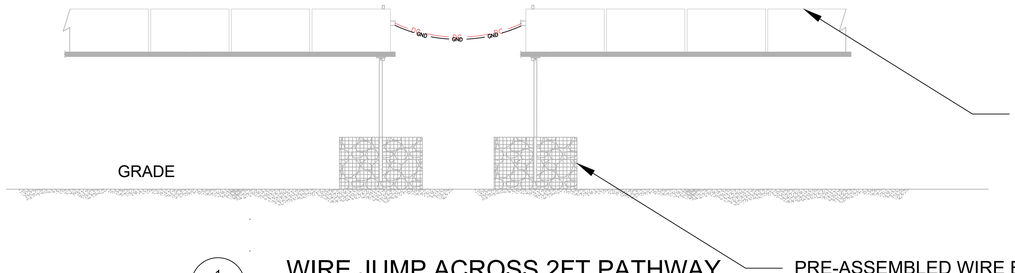
Revisions
Change Log

#		
1.0	2024-08-22	Initial design of project
1.1	2024-08-29	Pole line-up modified per utility comments
1.2	2025-01-08	Layout modified following environmental survey
1.3	2025-02-24	Added fence and removed modules above blasted trench
1.4	2025-03-13	Updated module spec, added scale markers and distance to property boundaries

Scale: No Scale
Drawn by: JPG
Date: 2024-08-22

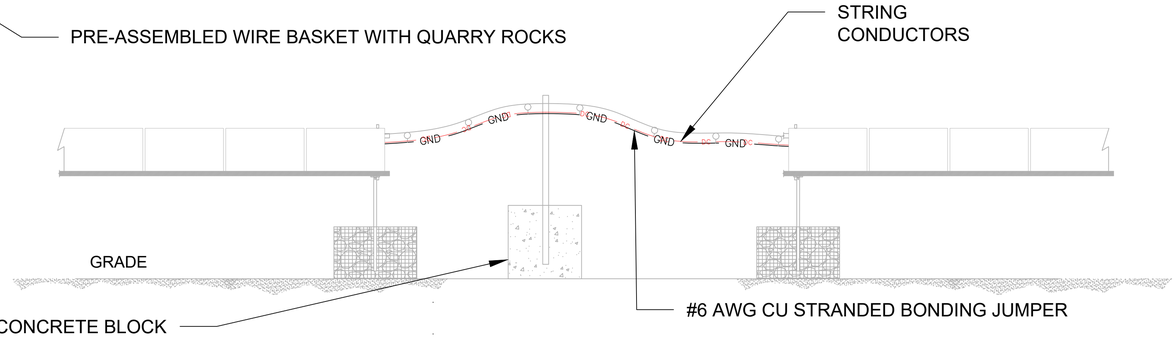
Sheet # E3

Electrical Three-line



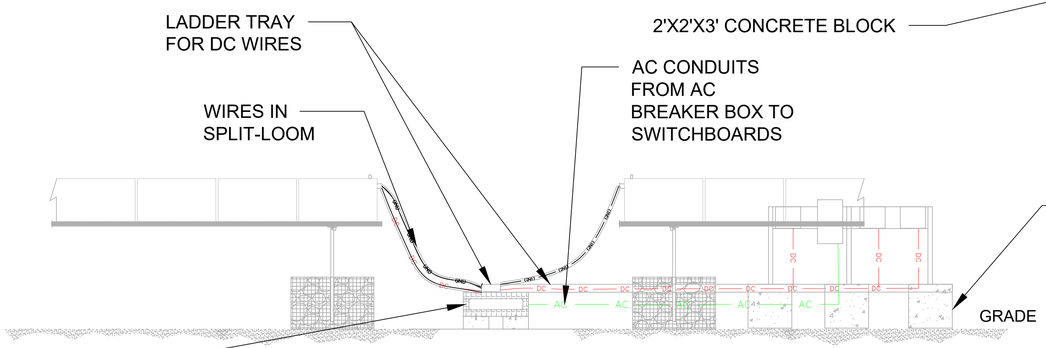
1 / E-450

WIRE JUMP ACROSS 2FT PATHWAY
SCALE:NTS



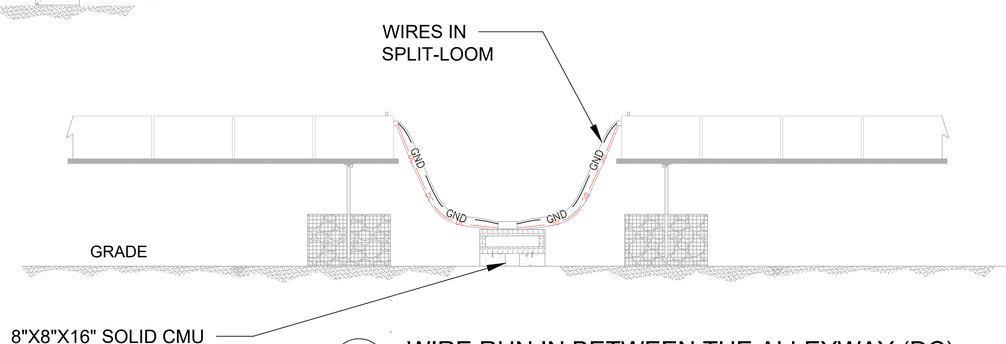
2 / E-450

WIRE JUMP ACROSS VENT DETAILS AND INTER-ROW JUMPS
SCALE:NTS



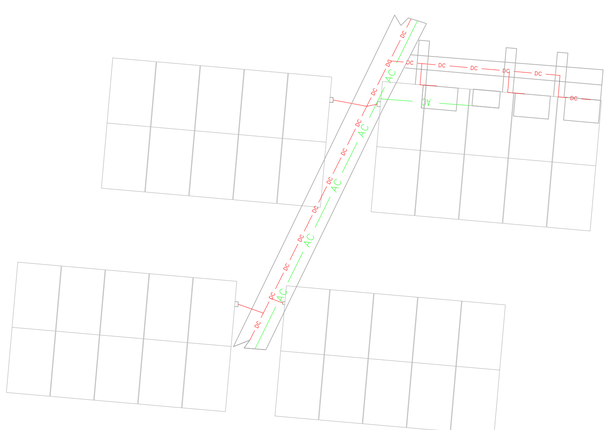
3 / E-450

WIRE RUN IN BETWEEN THE ALLEYWAY (AC AND DC)
SCALE:NTS



4 / E-450

WIRE RUN IN BETWEEN THE ALLEYWAY (DC)
SCALE:NTS



5 / E-450

WIRE CROSSING ACROSS ALLEYWAY-TOP VIEW
SCALE:NTS

NOTES:

1. MODULES ARE BONDED TO THE RACKING SYSTEM PER UL 2703 CERTIFICATION FOR FIXED-TILT RACKING .A CONTINUOUS BONDING PATH NEED TO BE ESTABLISHED FROM EACH NON-CONTINUOUS RACK TO THE STRING INVERTER VIA SHORT RACK-TO-RACK JUMPERS OR #6 EQUIPMENT GROUNDS AS REQUIRED TO ENSURE THAT ALL METALLIC ARRAY ELEMENTS ARE INTERCONNECTED.
   - USE WEEB-BNDJMP OR EQUIVALENT APPROVED BY THE ENGINEER FOR AREAS WHERE THERE IS SPACE BETWEEN ADJACENT RACKS.
   - BE SURE TO FOLLOW THE MODULE AND RACKING MANUFACTURER'S INSTALLATION MANUAL.
   - SLACK TO BE INSTALLED ON JUMPERS.
   - GROUND LUGS WITH #6 CU WIRE IS ALSO ACCEPTABLE

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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DESCRIPTION	DATE	DESIGN	CHECK	EOR	REV
RECDWG	04/18/2025	DG	MP	RC	9
RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A NORTH ARRAY
5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
ABOVE GROUND CONDUIT RUN DETAILS-SHEET 1

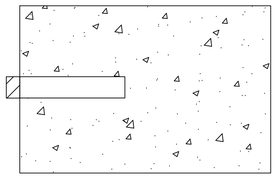
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ARCH D
24" X 36"

SHEET NUMBER
E-450

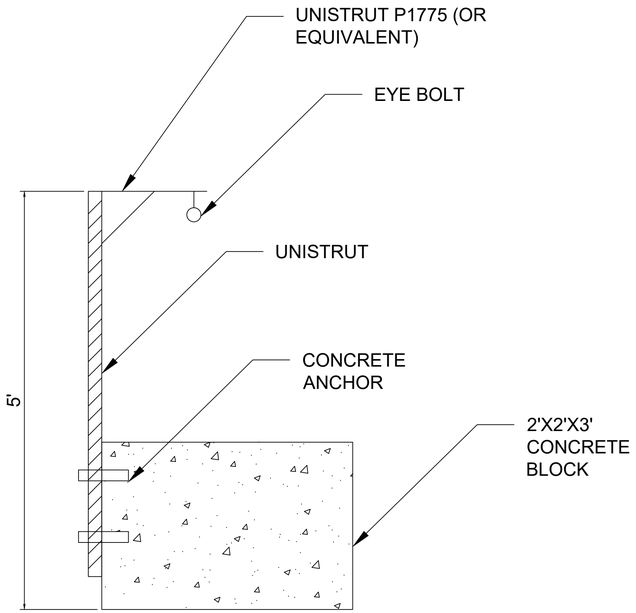
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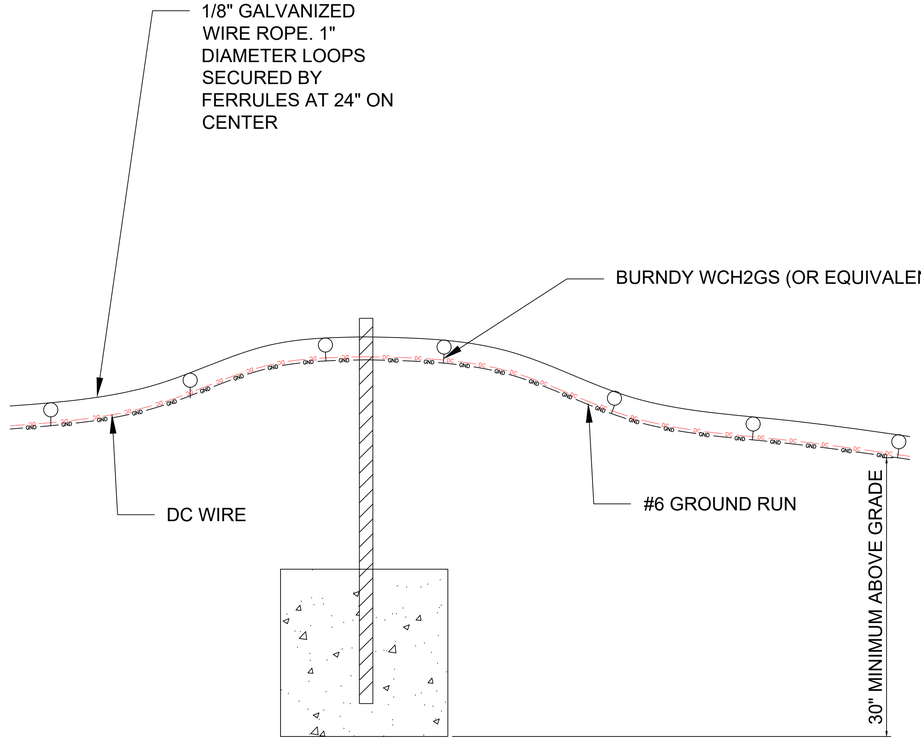
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TOP VIEW



SIDE VIEW



FRONT VIEW

1
E-460

WIRE CROSSING ACROSS VENT DETAILS
SCALE:NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A
NORTH ARRAY
5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
ABOVE GROUND CONDUIT RUN DETAILS-SHEET 2

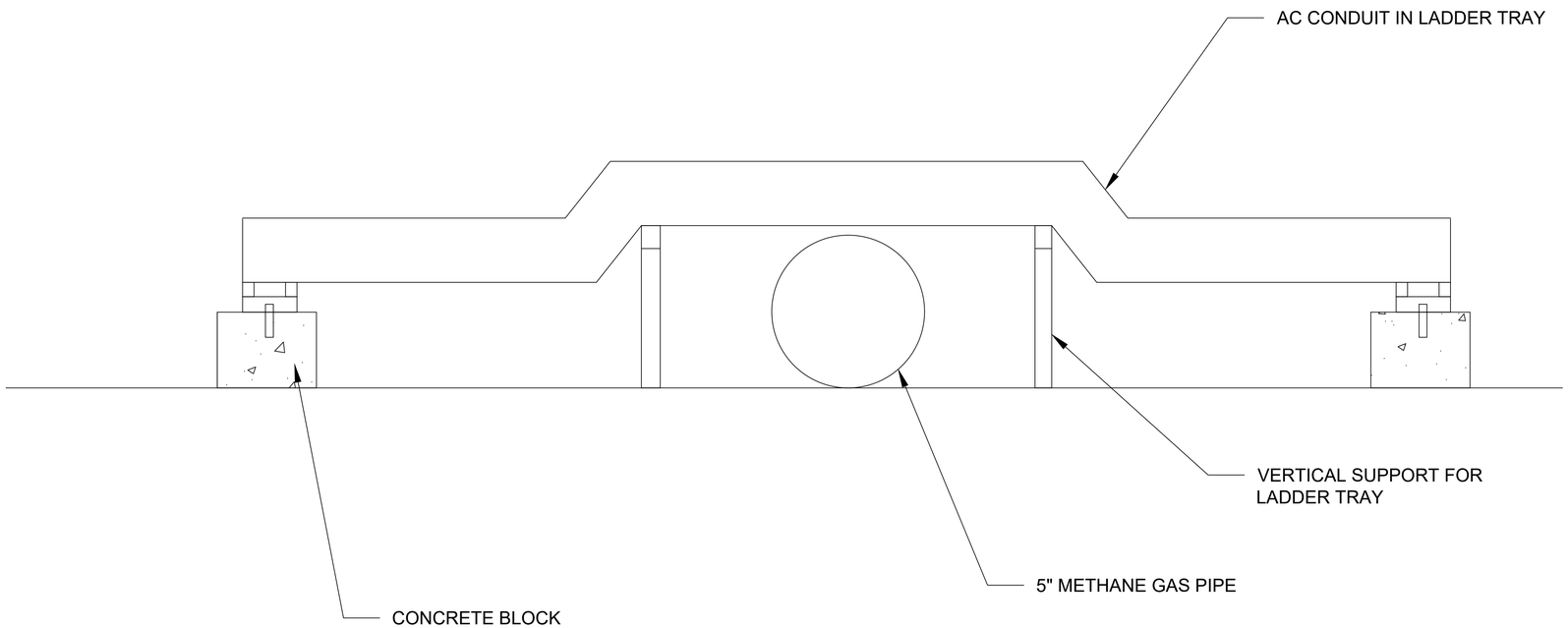
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ARCH D
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SHEET NUMBER
E-460

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1 / E-470 METHANE PIPELINE CROSSING
SCALE:NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A
NORTH ARRAY
5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
ABOVE GROUND CONDUIT RUN DETAILS-SHEET 3

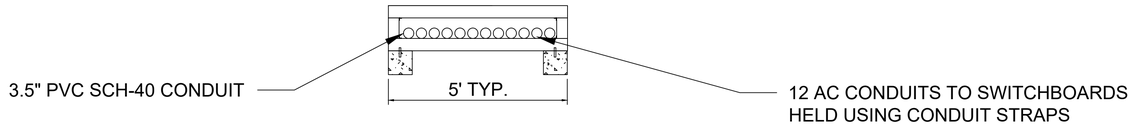
SHEET SIZE
ARCH D
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SHEET NUMBER
E-470

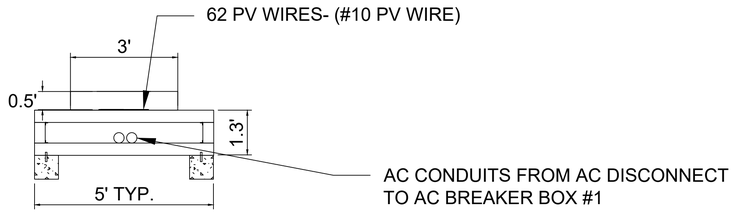
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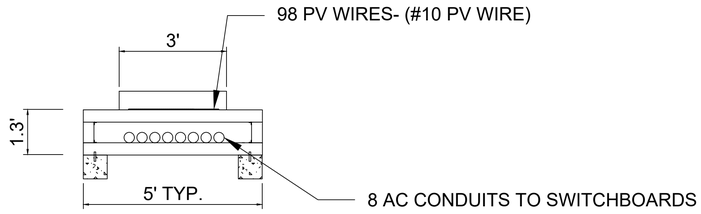
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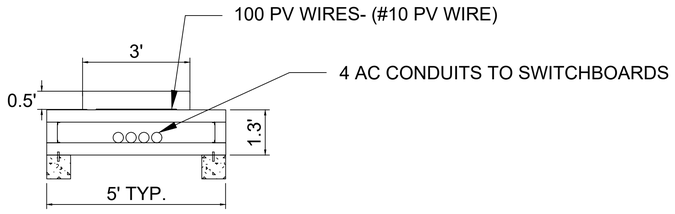


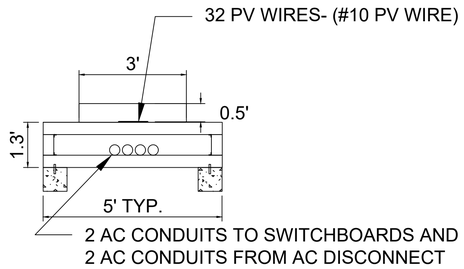
1 E-480 TYPICAL AC CONDUIT DETAIL- 12 AC CONDUITS
SCALE: NTS



2 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #14 AND #15
SCALE: NTS



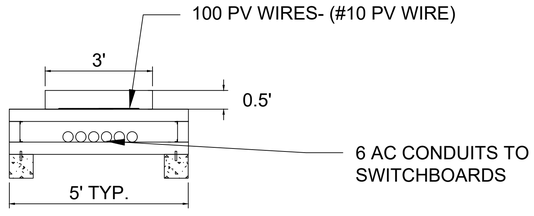


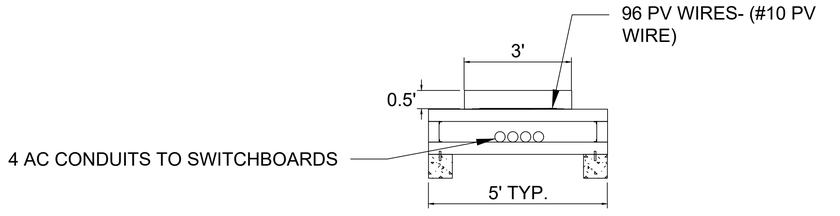


5 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #10, #11 AND #12
SCALE: NTS

4 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #16, #17 AND #18
SCALE: NTS

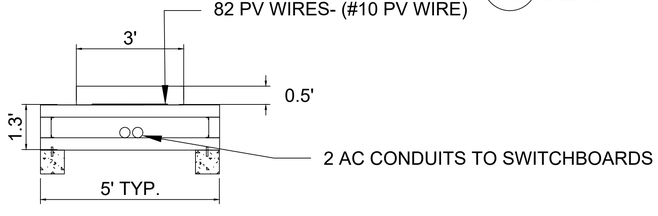
3 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #13
SCALE: NTS





6 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #7, #8 AND #9
SCALE: NTS

7 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #4, #5 AND #6
SCALE: NTS



8 E-480 TYPICAL DC AND AC RUN DETAIL- INVERTER #1, #2 AND #3
SCALE: NTS

NOTES:

1. THE DISTANCE BETWEEN THE CONCRETE BLOCKS HOLDING THE CONDUIT SUPPORTS MAY VARY BASED ON THE NUMBER OF CONDUITS..

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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1060 MAITLAND COMMONS, SUITE 270,
MAITLAND, FL 32751
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DESCRIPTION	DATE	DESIGN	CHECK	EOR	REV
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RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY
EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A
NORTH ARRAY
5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
ABOVE GROUND CONDUIT RUN DETAILS-SHEET 4

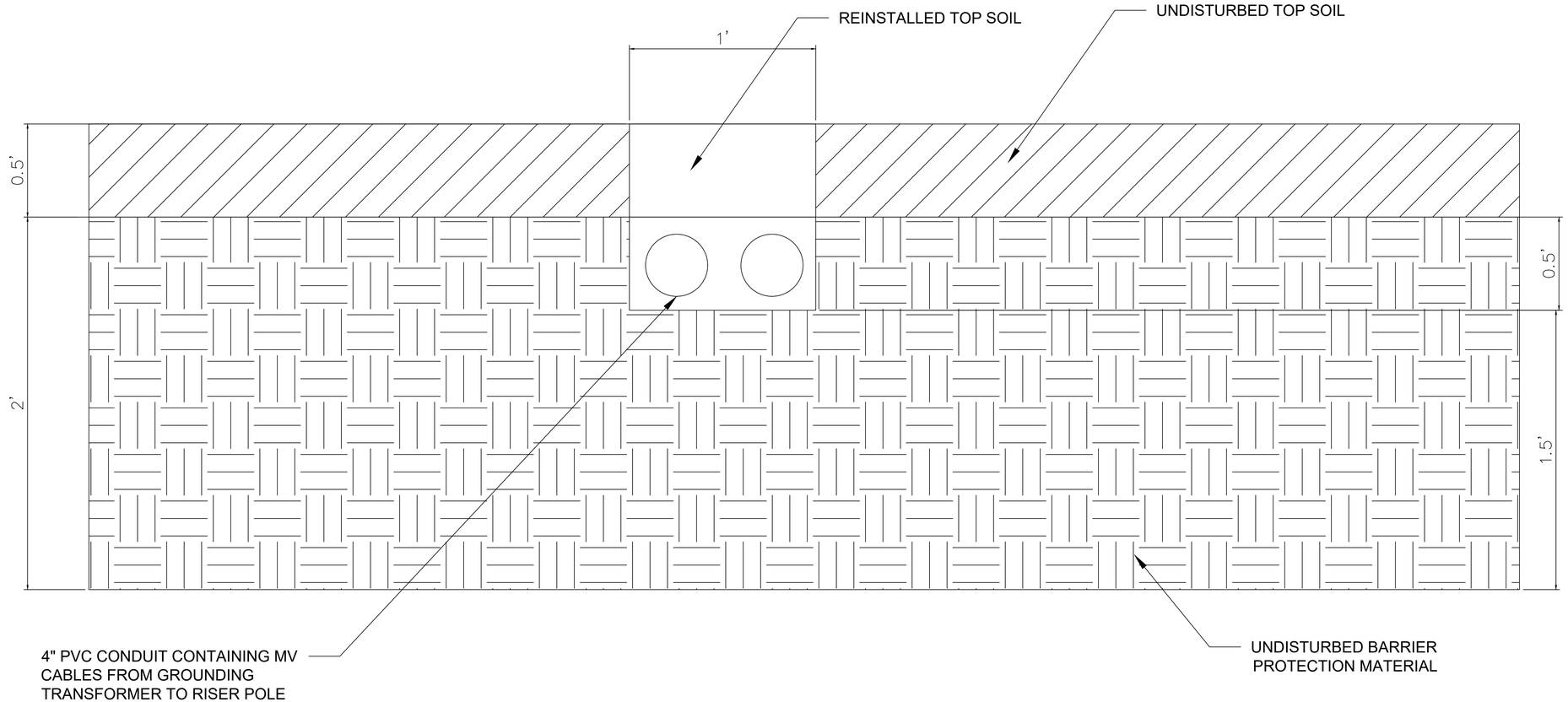
SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-480

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1 / E-490 MV CONDUIT RUN TO RISER POLE

SCALE: NTS

NOTES:

1. PVC SCH 40 FOR UNDERGROUND MV CABLE CONVEYANCE FROM THE EQUIPMENT PAD TO RISER IS SUBJECT TO AHJ APPROVAL.

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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DESCRIPTION	DATE	DESIGN	CHECK	EOR	REV
RECDWG	04/18/2025	DG	MP	RC	9
RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER

CALIBRANT ENERGY

EPC CONTRACTOR

Montante Solar

CASTILLO PROJECT ID.

2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A
NORTH ARRAY

5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME

UNDERGROUND MV CONDUIT RUN DETAIL

SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER

E-490

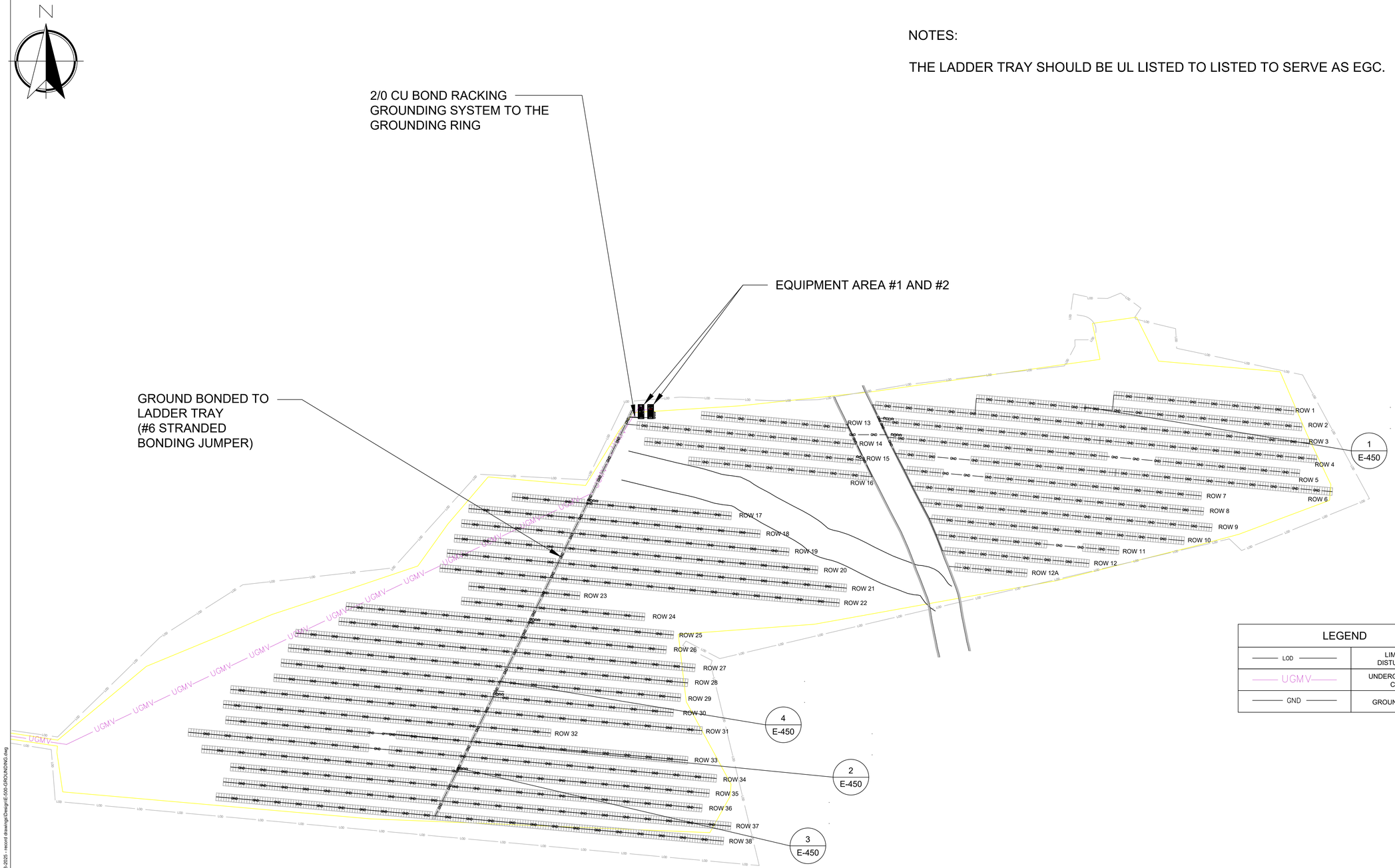
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drawings\Design\E-490_UNDERGROUND_AND_OVERHEAD_DETAIL.dwg
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NOTES:

THE LADDER TRAY SHOULD BE UL LISTED TO LISTED TO SERVE AS EGC.



LEGEND	
LOD	LIMITS OF DISTURBANCE
UGMV	UNDERGROUND MV CABLE
GND	GROUNDING LINE

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

80' 40' 0' 80' 240'

SCALE: 1" = 80'

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RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A
NORTH ARRAY

5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
OVERALL SITE GROUNDING PLAN

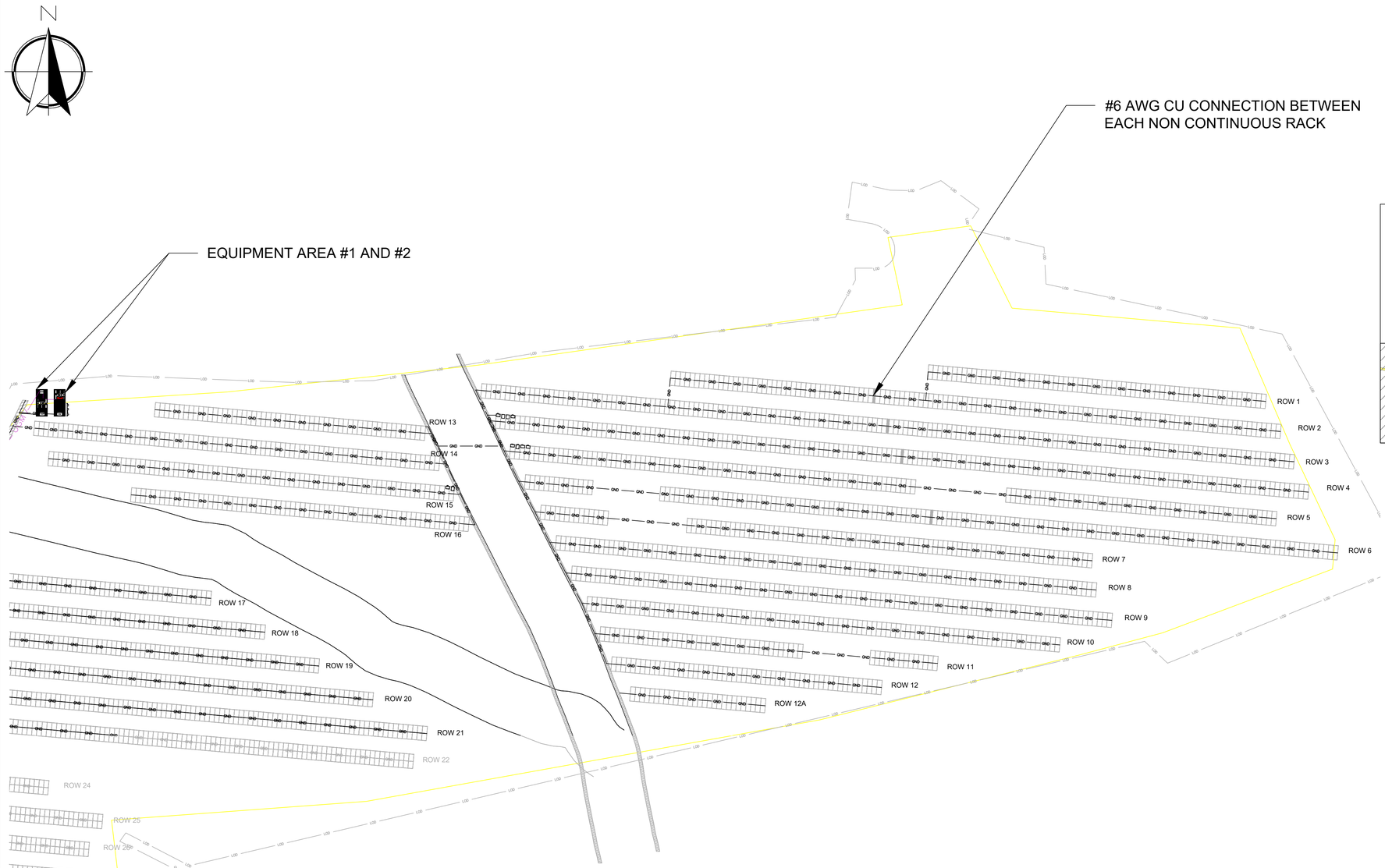
SHEET SIZE
ARCH D
24" X 36"

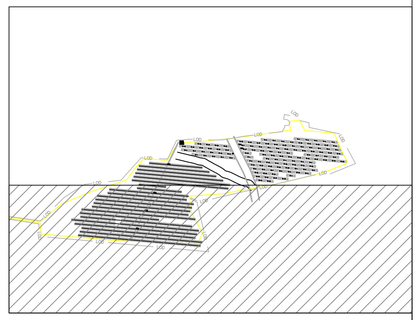
SHEET NUMBER
E-500

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KEY PLAN :NTS

LEGEND	
LOD	LIMITS OF DISTURBANCE
UGMV	UNDERGROUND MV CABLE
GND	GROUNDING LINE

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
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STRING QUANTITY	348
MODULE QUANTITY	9,744
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INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

50' 25' 0' 50' 150'

SCALE: 1" = 50'

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IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER

CALIBRANT ENERGY

EPC CONTRACTOR

Montante Solar

CASTILLO PROJECT ID.

2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A NORTH ARRAY

5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304 (43.092233, -78.999703)

SHEET NAME

GROUNDING PLAN-ZONE 1

SHEET SIZE

ARCH D

24" X 36"

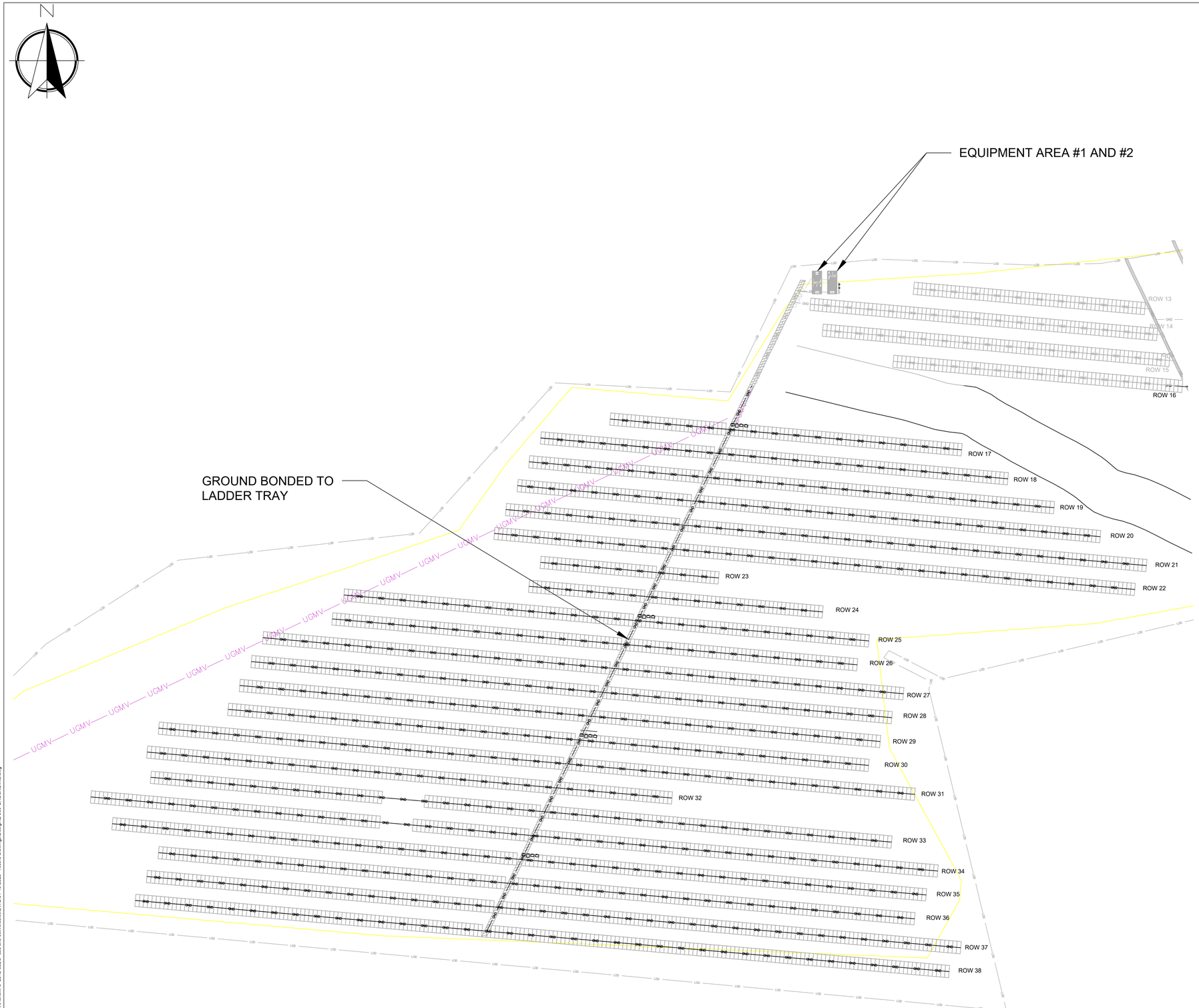
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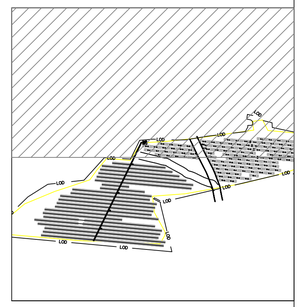
E-501

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KEY PLAN :NTS

LEGEND	
LOD	LIMITS OF DISTURBANCE
UGMV	UNDERGROUND MV CABLE
GND	GROUNDING LINE

SYSTEM INFORMATION	
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MODULE QUANTITY	9,744
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TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

50' 25' 0' 50' 150'

SCALE: 1" = 50'

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OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A NORTH ARRAY

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(43.092233, -78.999703)

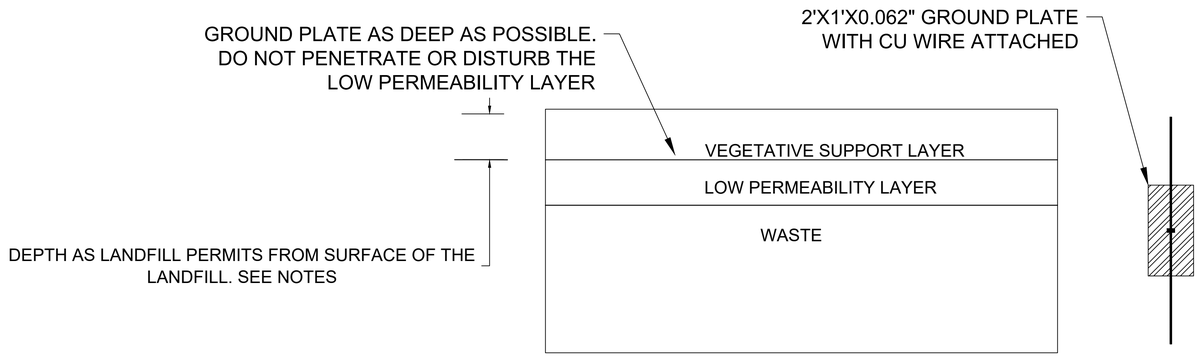
SHEET NAME
GROUNDING PLAN-ZONE 2

SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-502

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Shivam Joshi 4/28/2025 4:22:44 PM



1 E-503 GROUND PLATE INSTALLATION

SCALE: NTS

NOTES:

1. UNDER NO CIRCUMSTANCE IS THE LOW PERMEABILITY LAYER TO BE DISTURBED OR PENETRATED IN ANY FASHION.
2. IDEALLY GROUND RING/PLATE IS TO BE 30" BELOW GRADE BUT DUE TO SITE CONDITIONS, INSTALL GROUND RINGS/PLATES AS DEEP AS POSSIBLE.

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
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INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER

CALIBRANT ENERGY

EPC CONTRACTOR

Montante Solar

CASTILLO PROJECT ID.

2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A
NORTH ARRAY

5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME

GROUND PLATE INSTALLATION DETAIL

SHEET SIZE
ARCH D
24" X 36"

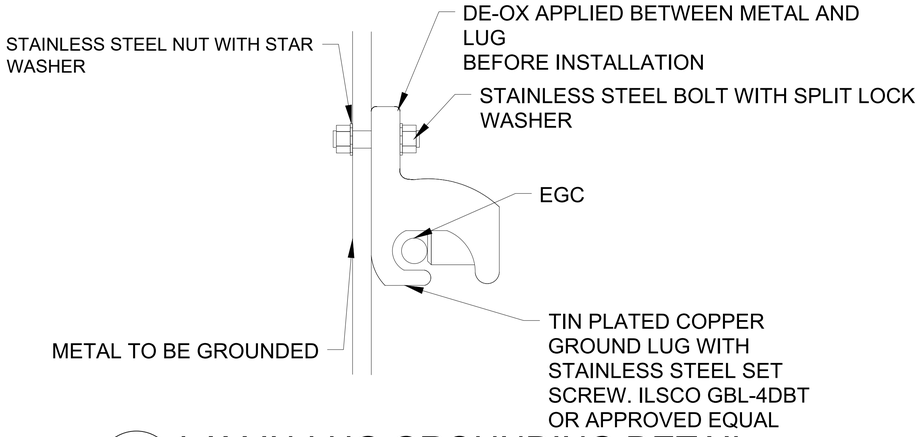
SHEET NUMBER

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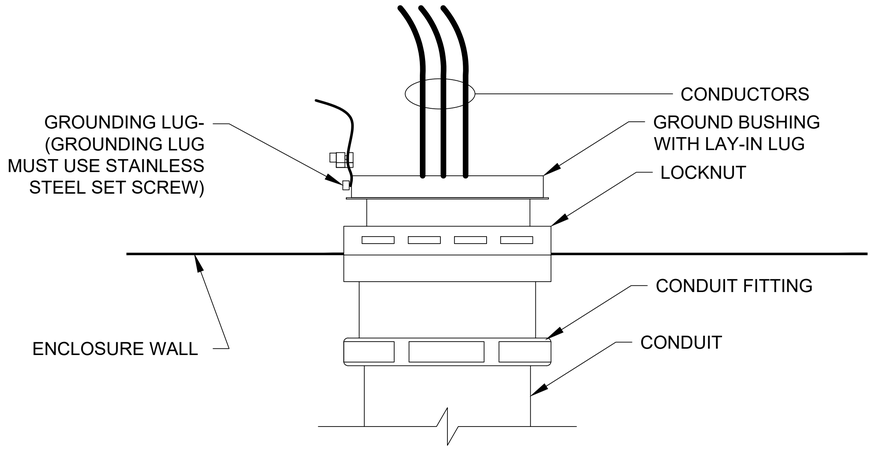
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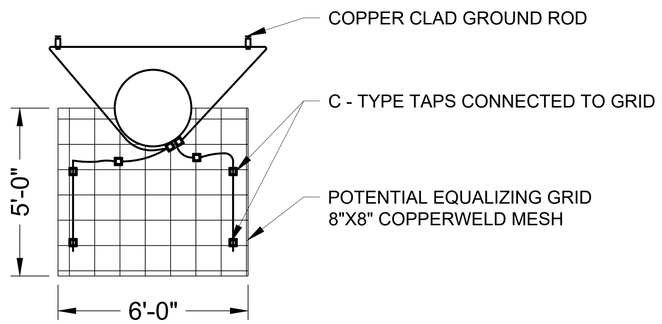
1 E-504 **LAY-IN-LUG GROUNDING DETAIL**

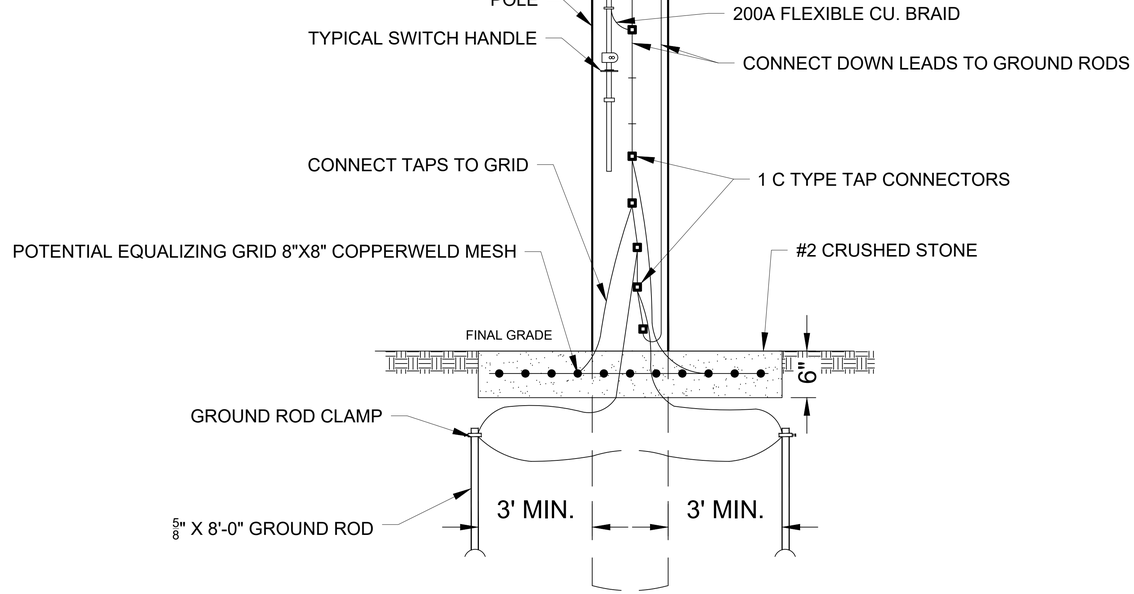
SCALE: NTS

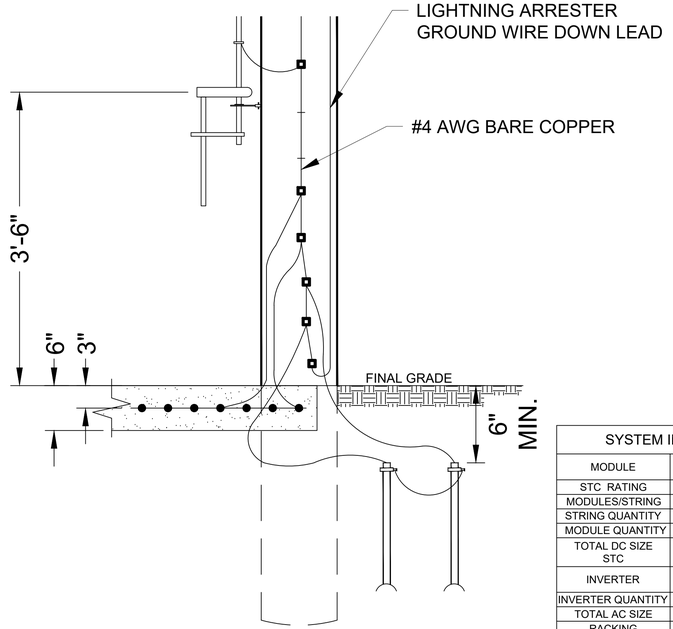


2 E-504 **METALLIC CONDUIT GROUNDING DETAIL**

SCALE: NTS







3 E-504 **GROUND GRID AND SWITCH HANDLE GROUNDING**

SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY
EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A
NORTH ARRAY
5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
GROUNDING DETAILS

SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-504

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ITEM	DESCRIPTION	SUPPLIED BY
1	WOODEN POLE MIN 45', MIN CLASS 3, PENTA TREATED	CONTRACTOR
2	3.5"x4.5"x10' FIBERGLASS DEADEND ARMS	CONTRACTOR
3	PHASE CONDUCTORS, 2/0 AAC(ASTER) CONDUCTORS	CONTRACTOR
4	NEUTRAL, 2/0 AAC(ASTER)	CONTRACTOR
5	15.5 KV, 900A, 40KA MOM., 110 KV BIL GOAB	CONTRACTOR
6	15 KV, 630A, 12.5KA, 150 KV BIL, RECLOSER	CONTRACTOR
7	RECLOSER CONTROL PANEL	CONTRACTOR
8	SURGE ARRESTER, POLE MOUNT, 8.4 KV MCOV, HD RISER POLE CLASS	CONTRACTOR
9	PHASE CONDUCTORS, 2/0 AAC(ASTER) CONDUCTORS	CONTRACTOR
10	CABLE TERMINATION, 15 KV CLASS	CONTRACTOR
11	POLYMER (SILICONE) INSULATOR, 15 KV	CONTRACTOR
12	15KV, 300A, 110 KV BIL, 40KA MOM., SOLID BLADE CUTOUTS	CONTRACTOR
13	5/8" X 8' COPPER CLAD GROUND ROD	CONTRACTOR
14	CONDUCTOR BARE COPPERCLAD #4	CONTRACTOR
15	5'X6' POTENTIAL EQUALIZING GRID OF 8" X 8" COPPERWELD MESH	CONTRACTOR
16	UTILITY METERING INSTRUMENT	UTILITY
17	UTILITY METER ENCLOSURE	UTILITY

POLE LAYOUT GENERAL NOTES:

1. ALL POLES ARE SHOWN AS CONCEPTUAL AND ARE FOR DIAGRAMMATIC PURPOSES ONLY. ALL UTILITY POLES SHALL CONFORM TO ANSI 05.1, EEI TD-103 LOCAL UTILITY REQUIREMENTS, AND ANY OTHER APPLICABLE CODES AND STANDARDS.
2. ALL OTHER MATERIAL NEEDED FOR A COMPLETE AND OPERABLE INSTALLATION, THOUGH NOT SPECIFICALLY CALLED OUT, SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR.
3. ALL POLE LINE HARDWARE SHALL BE RATED FOR 10,000 LBS MINIMUM.
4. COMMUNICATION SPACER˜CABLE SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL HELICAL ANCHORS AND DOWN GUY WIRES.
6. ACCORDING TO EEI TD-90, AT DEAD-END POLE MUST BE INSTALLED A DOUBLE-CROSSARM IF MADE OF WOOD
7. MAXIMUM OPERATION TEMPERATURE OF OVERHEAD CONDUCTOR 120°F.
8. POTENTIAL EQUALIZING GRID SHALL HAVE 6" (OPTIONAL) OF #2 CRUSHED STONE BENEATH THE GRADE AND SHALL BE LAID 3" BELOW GRADE, COVERED BY #2 CRUSHED STONE. PHOTOS OF BELOW GRADE GROUNDING INSTALLATION SHALL BE SUBMITTED TO NATIONAL GRID FOR REVIEW PRIOR TO BACKFILL.
9. IN ADDITION TO NUTS AND WASHERS, LOCKNUTS SHALL BE INSTALLED ON ALL THREADED MATERIAL AND HARDWARE. THE THREADS ON INSTALLED BOLTS SHALL PROTRUDE PAST THE LOCK WASHERS A MINIMUM OF ONE INCH BUT NOT MORE THAN TWO INCHES.
10. A 3 INCH BY 3 INCH (MINIMUM)SQUARE, CURVED WASHER SHALL BE USED ABUTTING THE POLE WHEN INSTALLED ON PRIMARY DEADEND, NEUTRAL DEADEND, AND GUY ASSEMBLIES DIRECTLY TO POLE.
11. CONDUCTORS SHALL BE SAGGED EVENLY AND IN ACCORDANCE WITH THE CONDUCTOR MANUFACTURER'S RECOMMENDATIONS.
12. EACH LIGHTNING ARRESTER TO BE BONDED SEPARATELY TO POLE GROUND.
13. ALL NON-CURRENT-CARRYING METALLIC PARTS SHALL BE BONDED TO THE EQUIPMENT GROUND WITH A STRANDED #4 AWG BARE COPPER CONDUCTOR.
14. ALL ARRESTORS SHALL HAVE A DEDICATED #4 AWG BARE COPPER CONDUCTOR TRAILED DOWN TO GROUNDING RODS/GRID SEPARATE FROM EQUIPMENT GROUND.
15. ALL DISCONNECTING MEANS SHALL BE UNIFORMLY PLACARDED WITH PERMANENTLY AFFIXED METAL OR PLASTIC LABELS AS REQUIRED BY THE NEC. NY SIR, AND ESB 750.

1 / E-600 INTERCONNECTION POLES BOM
SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
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MODULES/STRING	28
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DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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MAITLAND, FL 32751
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OWNER
CALIBRANT ENERGY
EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
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5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
INTERCONNECTION POLES BOM

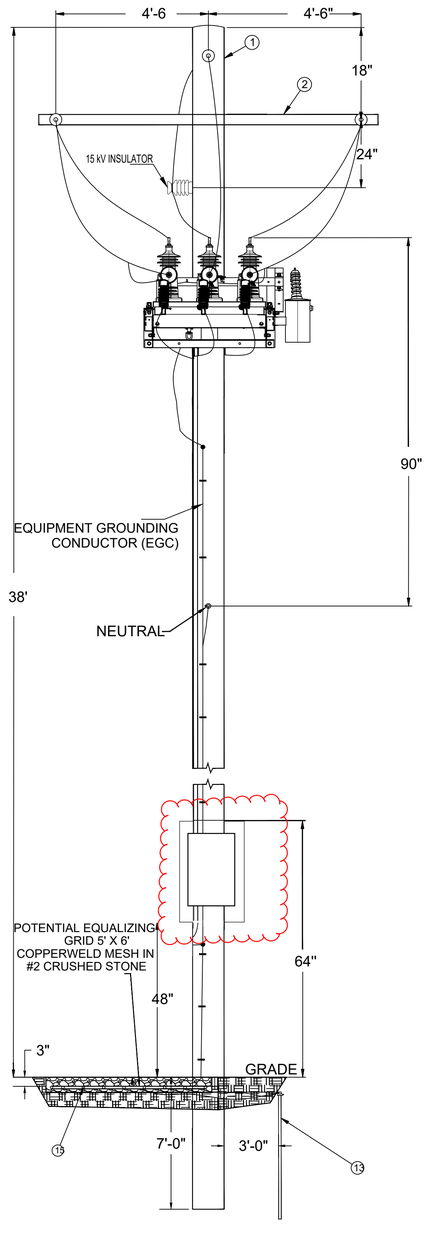
SHEET SIZE
ARCH D
24" X 36"

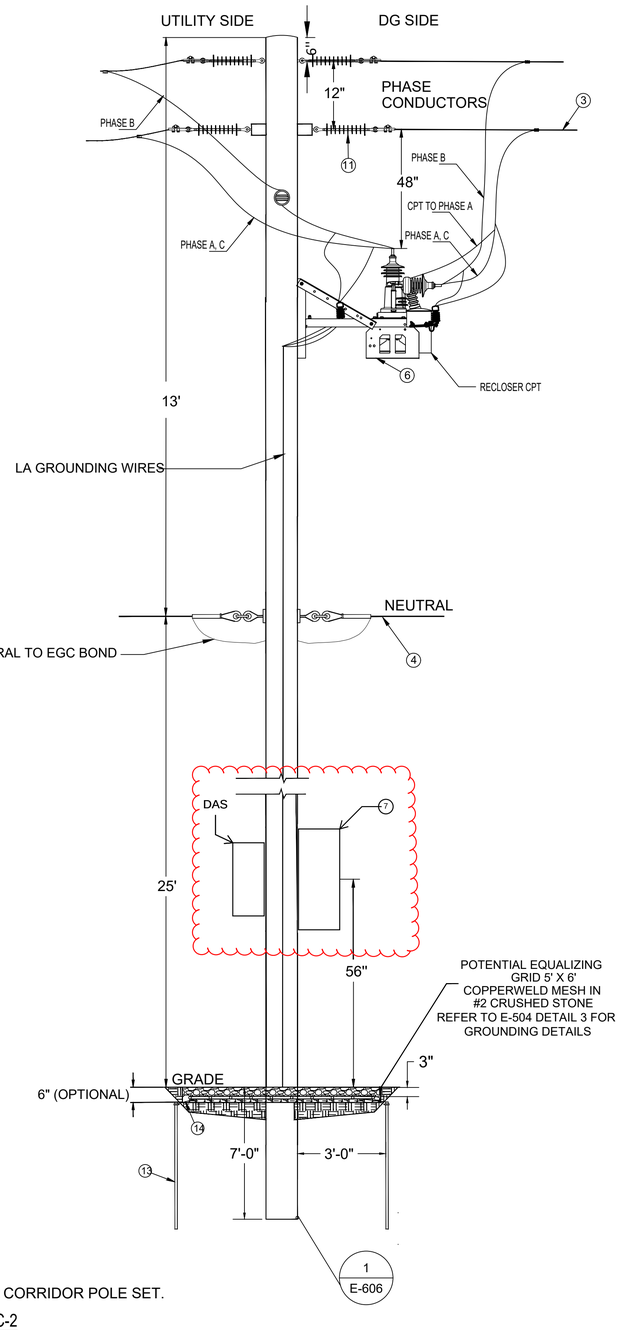
SHEET NUMBER
E-600

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NOTE:
-REFER TO SHEET E-606 DETAIL 1 FOR CORRIDOR POLE SET.

1 / E-601 CUSTOMER POLE #C-2 "CR512" CUSTOMER RECLOSER, BACK AND SIDE VIEW

SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
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EPC CONTRACTOR
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PINE AVENUE LANDFILL A NORTH ARRAY
5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
CUSTOMER RECLOSER POLE DETAIL

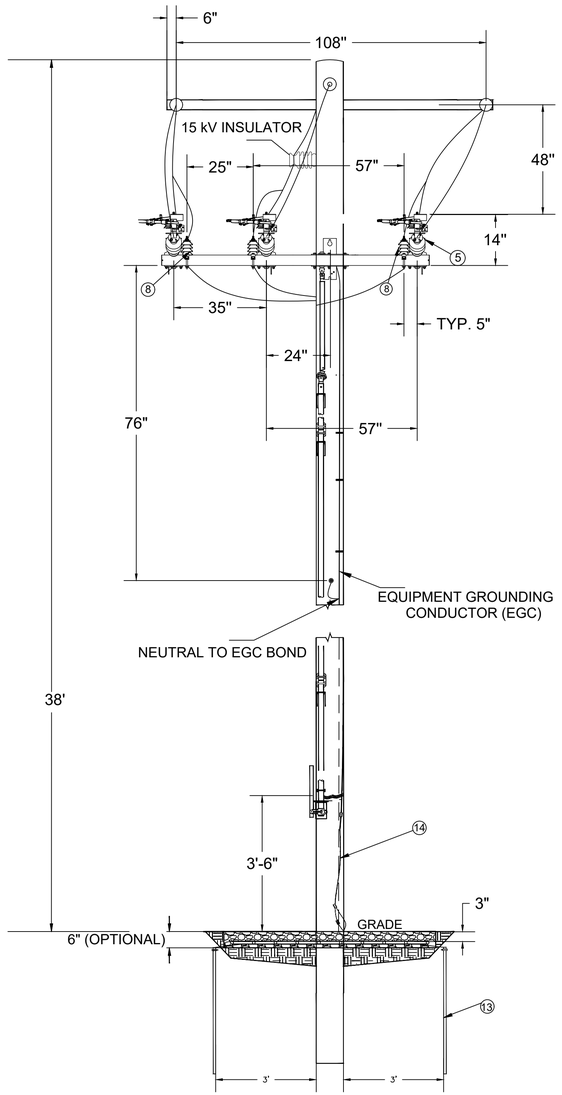
SHEET SIZE
ARCH D
24" X 36"

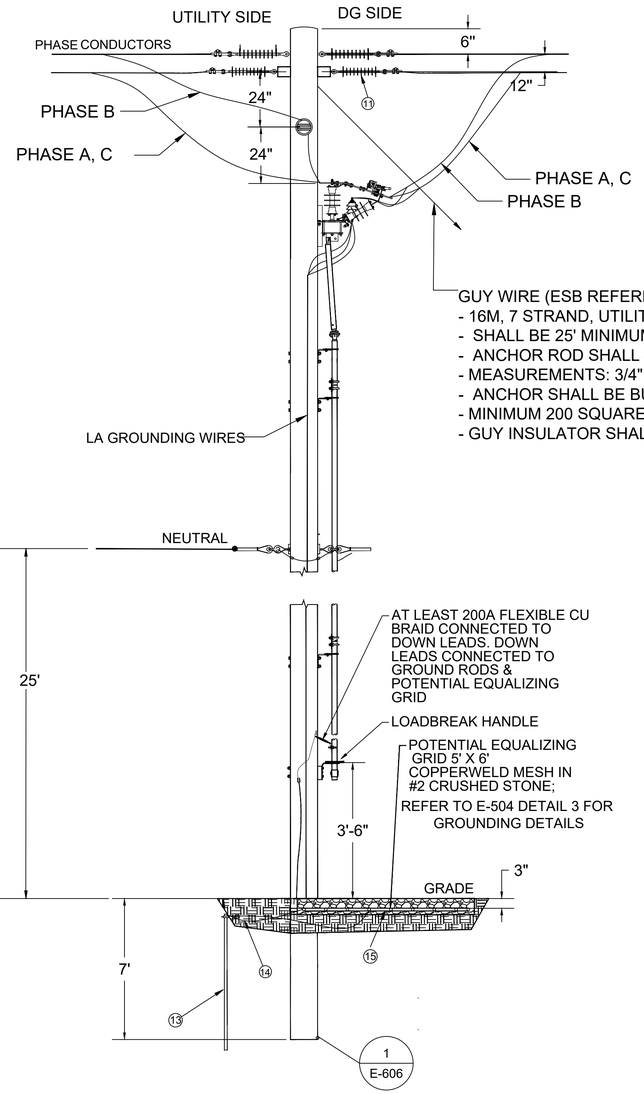
SHEET NUMBER
E-601

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GUY WIRE (ESB REFERENCE 753- SECTION V-(B)-(3)-1993 VERSION)
- 16M, 7 STRAND, UTILITIES GRADE, CONFORMING TO ANSI C7.46, ASTM A475.
- SHALL BE 25' MINIMUM LEAD WITH FIBERGLASS STRAIN INSULATOR.
- ANCHOR ROD SHALL CONFORM TO ANSI STAND C135.2.
- MEASUREMENTS: 3/4" DIAMETER BY 8'-0" MINIMUM.
- ANCHOR SHALL BE BURIED 6' MINIMUM IN GROUND.
- MINIMUM 200 SQUARE INCH EXPANDING TYPE ANCHOR.
- GUY INSULATOR SHALL CONFORM TO ANSI C29.4 CLASS 54-3.

NOTES:
- GOAB ORIENTATION "JAWS" TO BE ON UTILITY SIDE OF DISCONNECT WHILE "BLADES" TO BE ON GENERATOR SIDE.
- UTILITY SOURCE SHALL BE CONNECTED TO JAW SIDE OF SWITCH. HINGE SIDE OF SWITCH SHALL BE CONNECTED TOWARDS CUSTOMER DER.
- THE SWITCH SHALL BE CAPABLE OF OPENING UNDER 1/2" ICE LOADING CONDITIONS NESC "HEAVY".
- SWITCH BLADE SHALL BE ON DE-ENERGIZED SIDE OF SWITCH WHEN OPEN.
- DUAL LOCKING HANDLE ASSEMBLY TO BE INSTALLED.
-REFER TO SHEET E-606 DETAIL 1 FOR CORRIDOR POLE SET.

1 / E-602 CUSTOMER POLE #C-1: "C511" CUSTOMER GOAB BACK AND SIDE VIEW
SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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COA #28345
1060 MAITLAND COMMONS, SUITE 270,
MAITLAND, FL 32751
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REVISIONS

DESCRIPTION	DATE	DESIGN	CHECK	EOR	REV
RECDWG	04/18/2025	DG	MP	RC	9
RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY
EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A NORTH ARRAY
5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
CUSTOMER GOAB DETAIL

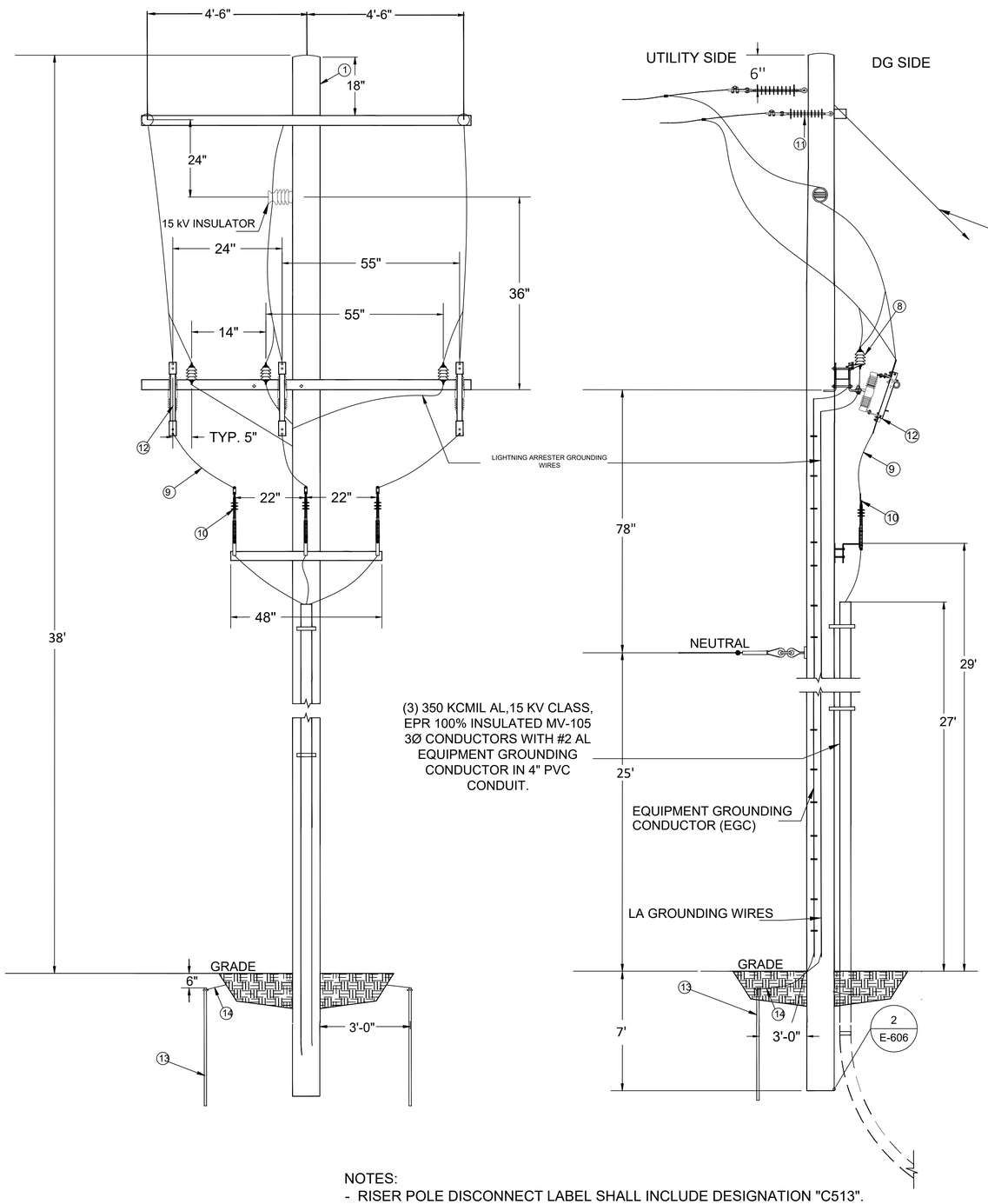
SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-602

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GUY WIRE
- SHALL BE 25' MINIMUM LEAD WITH FIBERGLASS STRAIN INSULATOR.
- INSULATING ROD IS 72".
- SECTION BELOW INSULATING ROD SHALL BE BONDED TO EGC.
- CLEARANCE FROM INSULATING ROD TO THE PRIMARY CONDUCTOR SHALL BE A MINIMUM OF 6" FOR DOWN GUYS, OR 11" MINIMUM FOR SPAN GUYS
- GUY WIRE TO BE ANCHORED TO A CONCRETE BLOCK THAT IS 4'x2'x2'.

Anchoring Features
- Embedded Hardware: Rod is 7/16" U-shaped with flat tabs on the ends, it is embedded to a depth of 1 foot into the concrete
- Lifting Inserts: Retaining wall lifting anchors
- Guy Wire Compatibility: Suitable for up to 5/8" diameter guy wires.

Installation Guidelines
- Ensure stable, level substrate before placing block.
- Position block with lifting anchors using proper rigging
- Attach guy wire to embedded anchor or wrap securely around the block (if anchor not installed)
- Bury partially or leave exposed depending on wind load and soil conditions

NOTES:
- RISER POLE DISCONNECT LABEL SHALL INCLUDE DESIGNATION "C513".
- RISER POLE SHALL INCLUDE DESIGNATION "C-3".
- DISCONNECT SHOULD OPEN ON UTILITY SIDE, HINGE ON THE PV SIDE

-REFER TO SHEET E-606 DETAIL 2 FOR CORRIDOR POLE SET.
-PVC SCH 40 FOR UNDERGROUND MV CABLE CONVEYANCE FROM THE EQUIPMENT PAD TO RISER IS SUBJECT TO AHJ APPROVAL.

1 / E-603 CUSTOMER POLE #C-4:
"C513" CUSTOMER SOLID BLADE CUTOUTS AND RISER BACK AND SIDE VIEW
SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A NORTH ARRAY
5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
CUSTOMER SOLID BLADE CUTOUTS AND RISER DETAIL

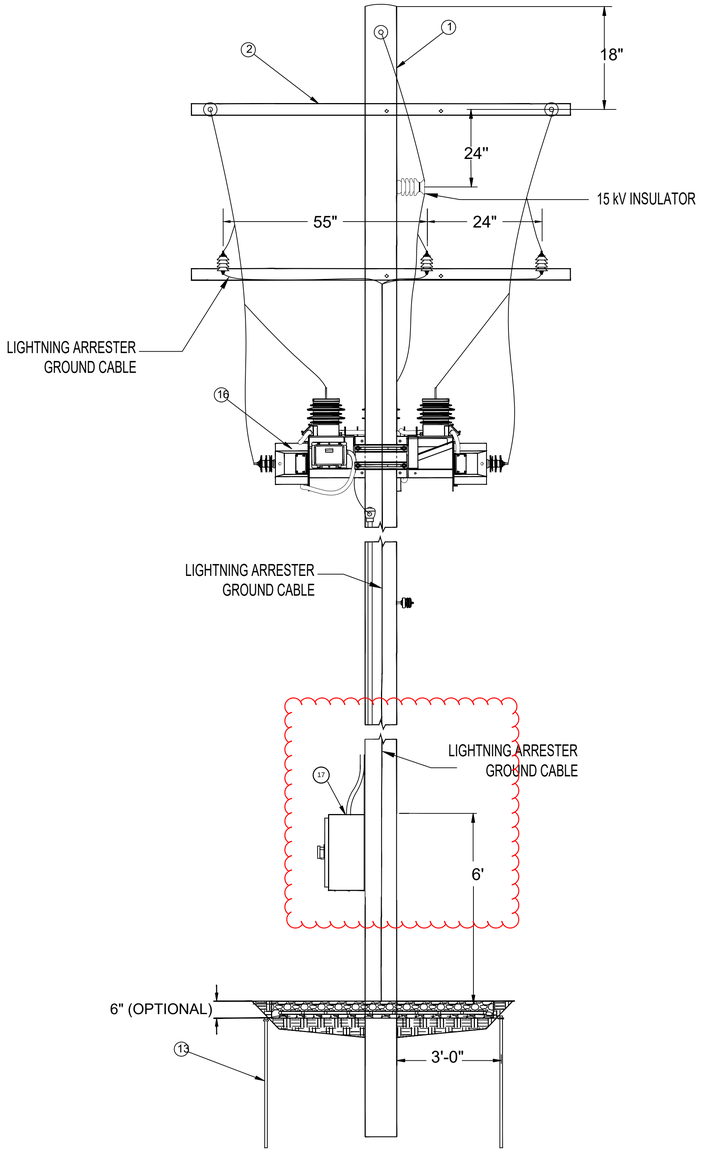
SHEET SIZE
ARCH D
24" X 36"

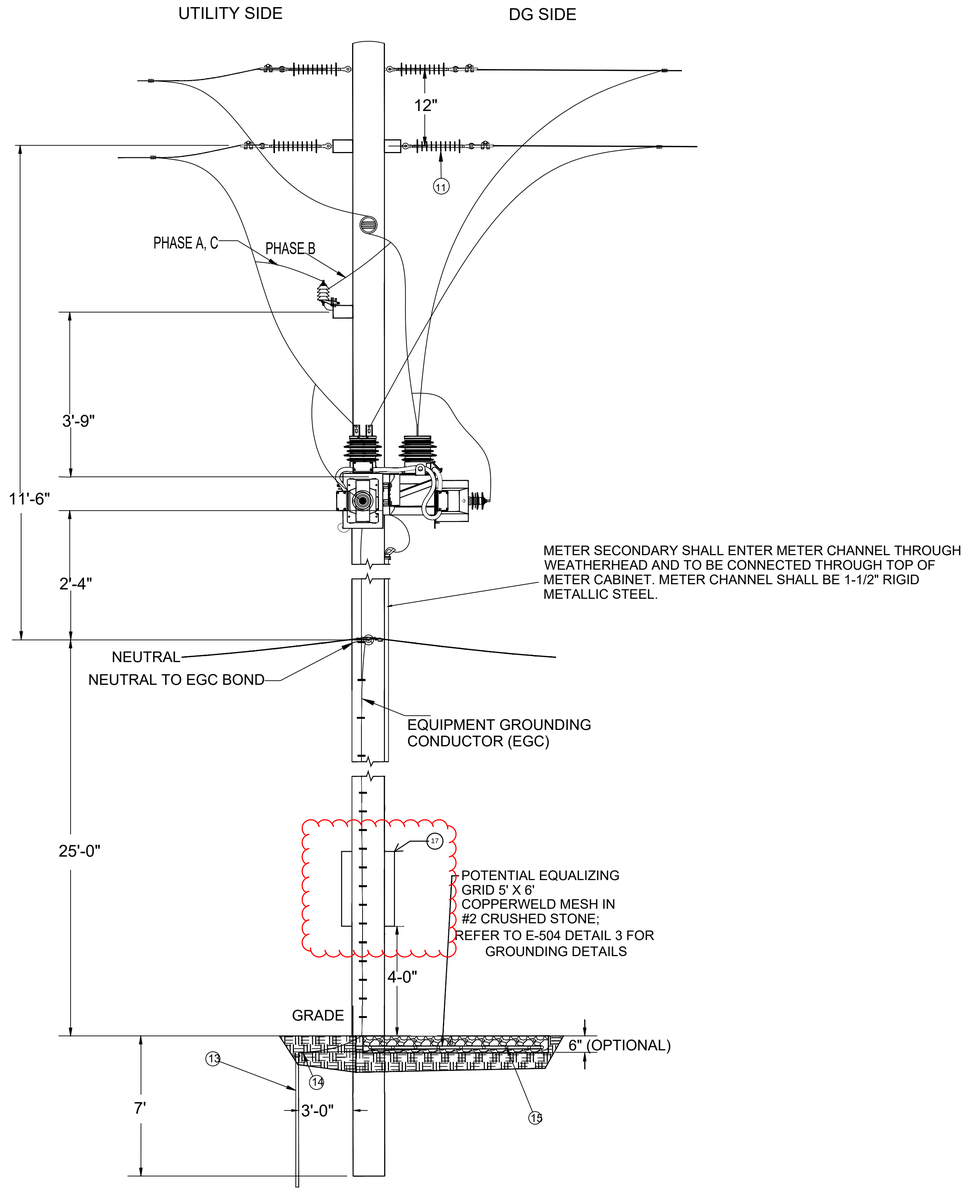
SHEET NUMBER
E-603

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NOTES:
- UTILITY METER- CUSTOMER POLE SHALL INCLUDE DESIGNATION "C-3".
- METER SHALL HAVE 1-1/2" RMC OR PVC SCH 80 CONDUIT.
- METERING EQUIPMENT IS SUPPLIED BY THE UTILITY AND INSTALLED BY THE CONTRACTOR.

CUSTOMER POLE #C-3:
UTILITY METER
BACK AND SIDE VIEW

1 / E-604 SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY

EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A
NORTH ARRAY
5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

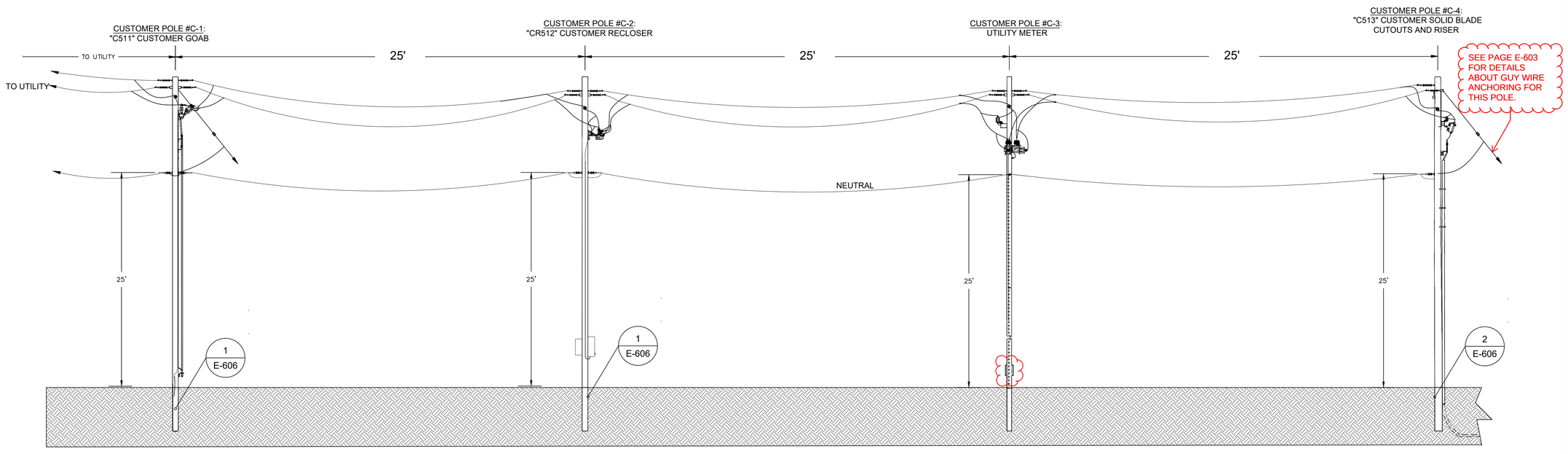
SHEET NAME
UTILITY METER POLE DETAIL

SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-604

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4/28/2025 4:22:07 PM



1 E-605 POLE LAYOUT

SCALE: NTS

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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RECDWG	04/01/2025	JI	MP	RC	8
RECDWG	03/04/2025	DG	MP	RC	7
IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER

CALIBRANT ENERGY

EPC CONTRACTOR

Montante Solar

CASTILLO PROJECT ID.

2312-352

SIGNATURE AND SEAL

PROJECT NAME

PINE AVENUE LANDFILL A
NORTH ARRAY

5600 NIAGARA FALLS BLVD,
NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME

POLE LAYOUT

SHEET SIZE
ARCH D
24" X 36"

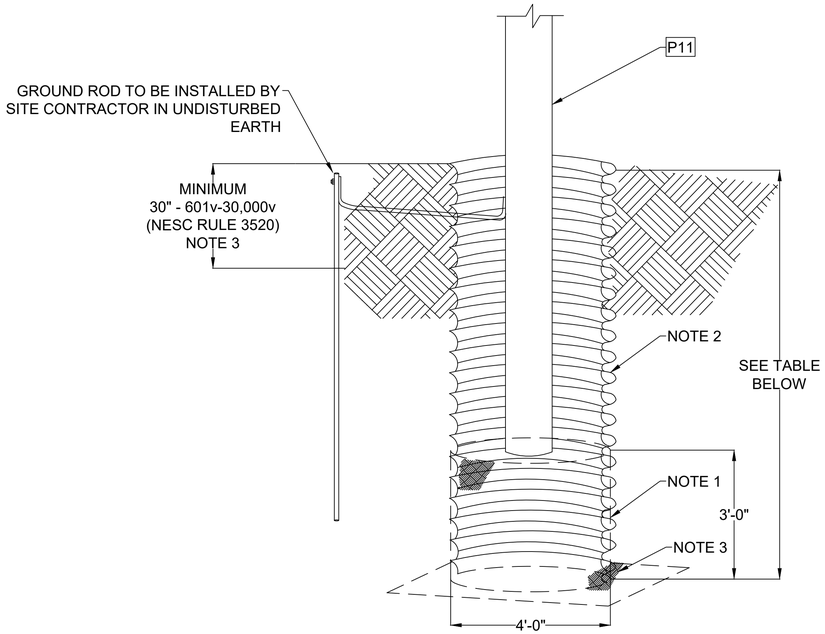
SHEET NUMBER

E-605

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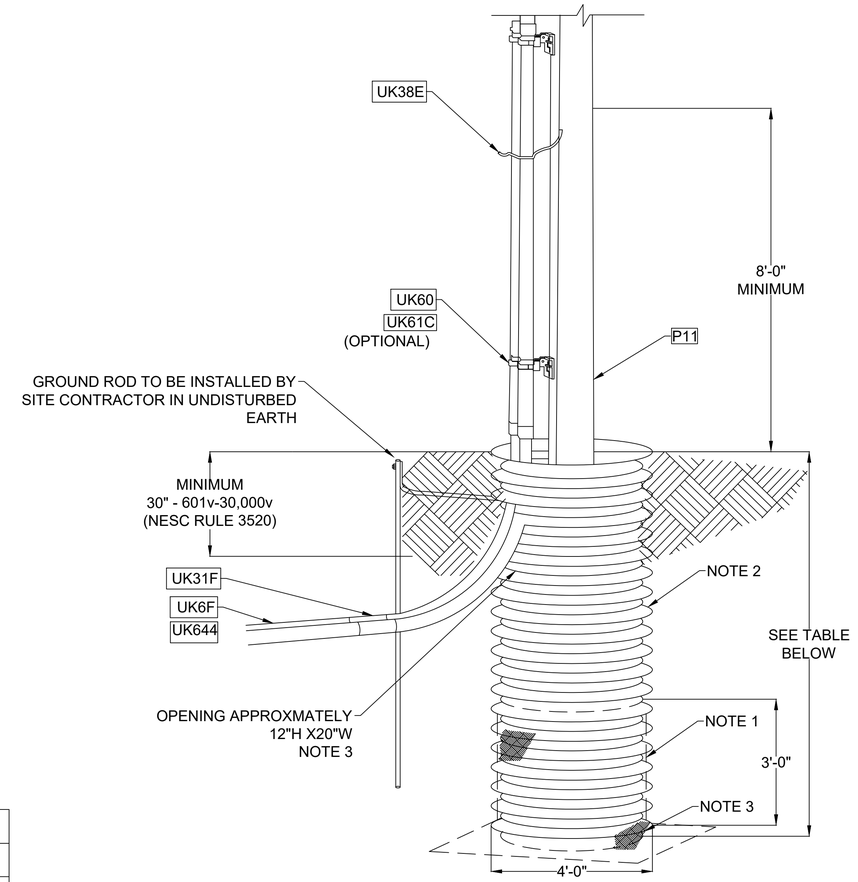
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1 E-606 CLEAN CORRIDOR POLE SET
SCALE: NTS

POLE HEIGHT	SET DEPTH
40'	9'-0"
45'	9'-6"
50'	10'-0"
55'	10'-6"



2 E-606 RISER POLE CORRIDOR SET
SCALE: NTS

NOTES:
1. CRUSHED STONE BACKFILL WITH AGGREGATE NOT EXCEEDING 3/4” IN DIAMETER SHALL BE USED TO BACK FILL POLES. CRUSHED STONE BACKFILL SHALL BE THOROUGHLY TAMPED USING MECHANICAL TAMPERS OR APPROPRIATE HAND TOOLS IN LAYERS NOT EXCEEDING 12” IN DEPTH.
2. CORRUGATED METAL PIPE WITH CORRUGATIONS 2-2/3” X 1/2", MINIMUM 16 GAUGE.
3. INSTALL GEOTEXTILE FABRIC AS A BARRIER ON ALL CORRUGATED METAL PIPE OPENINGS,
4. UTILITY CREWS TO INSTALL POLE AND CONNECT TO GROUNDING ALREADY INSTALLED WITH CORRUGATED PIPE, GEOTEXTILE BARRIER AND CRUSHED STONE BACKFILL.
5. CORRUGATED STEEL PIPE SHALL BE FREE OF ALL BURS AND JAGGED EDGES IN ORDER TO REDUCE THE RISK OF CUTTING INJURIES DURING HANDLING. CORRUGATED PIPE SHALL BE FITTED WITH EDGE TRIM SUCH AS NEOPRENE RUBBER TO COVER BURS AND JAGGED EDGES.

SYSTEM INFORMATION	
MODULE	CANADIAN SOLAR TOPBIHIKU7 CS7N-695TB-AG
STC RATING	695 W
MODULES/STRING	28
STRING QUANTITY	348
MODULE QUANTITY	9,744
TOTAL DC SIZE STC	6,772,080W
INVERTER	CPS SCH275KTL-DO/ US-800-36 V2
INVERTER QUANTITY	18
TOTAL AC SIZE	4,950,000 W
RACKING	TBD
DC/AC RATIO	1.36
GROUND COVERAGE RATIO	0.50
TILT	35°
AZIMUTH	185°

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RECDWG	04/01/2025	JI	MP	RC	8
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IFC	06/28/2024	RB	MP	RC	6
IFC	06/12/2024	RB	MP	RC	5

OWNER
CALIBRANT ENERGY
EPC CONTRACTOR
Montante Solar

CASTILLO PROJECT ID.
2312-352

SIGNATURE AND SEAL

PROJECT NAME
PINE AVENUE LANDFILL A NORTH ARRAY
5600 NIAGARA FALLS BLVD, NIAGARA FALLS, NY 14304
(43.092233, -78.999703)

SHEET NAME
POLE CORRIDOR SET DETAIL

SHEET SIZE
ARCH D
24" X 36"

SHEET NUMBER
E-606

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APPENDIX D
SAMPLE SNOW/WIND LOAD CALCULATIONS

Outlook

SAMPLE WIND/LOAD CALCULATIONS

FW: APA Info

From: Steve Erck <SErck@montante.com>
Sent: Wednesday, July 16, 2025 11:57 AM
To: Katie Soscia <ksoscia@montante.com>
Subject: APA Info

Below is the loading info from APA and attached are their sample drawing set and product data sheet. On our prior project, basket spacing ranged from 110-112 inches Hopefully this gives you what you need, but let me know if not.
Steve

Steve,

Please see below from our engineering team:

That psf load we provided incorporates the dead load (weight) and the calculated wind load and snow load (that is why there is a difference between the two tilts). Typically when we have been asked about the landfill cap's capacity we will supply the total loading psf, because it is very important to evaluate not only the dead weight sitting on top of the cap but also the potential forces it may see from nature. Below is a breakdown of the total load calculations. The 272 lb dead load being the weight of the racking without aggregate. Please let us know if there are any questions.

25 Degree Tilt Geoballast Model:
Max Down Load Per Reactions (Above Ballast Racking) = 3827 lbs.
Calculated Ballast Weight = 2220 lbs. (Aggregate) + 272lbs (Dead Load) = 2500 lbs. (rounded up from 2492lbs)
Total Down Force = Max Down Load Per Reactions + Calculated Ballast Weight = 3827 lbs. + 2500 lbs. = 6327 lbs.
Ballast Square Area = W x L = 24” x 84” = 2016 sq. in

Total Ballast Pressure = Force (lbs) / Area (sq.in) = 6327lbs / 2016 sq. in = 3.14 lbs/sq. in = ***452.16 lbs/sq. ft***

35 Degree Tilt Geoballast Model:
Max Down Load Per Reactions (Above Ballast Racking) = 3189 lbs.
Calculated Ballast Weight = 2220 lbs. (Aggregate) + 272lbs (Dead Load) = 2500 lbs. (rounded up from 2492lbs)
Total Down Force = Max Down Load Per Reactions + Calculated Ballast Weight = 3189 lbs. + 2500 lbs. = 5689 lbs.
Ballast Square Area = W x L = 24” x 84” = 2016 sq. in
Total Ballast Pressure = Force (lbs.) / Area (sq.in) = 5689 lbs./ 2016 sq. in = 2.82 lbs/sq. in = ***406.08lbs/sq. ft***

APPENDIX E
STORMWATER RUNOFF CALCULATIONS (PRE AND POST DEV)

DA-01

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 1.35 Acres
Increased Impervious Areas from Panels – 0.39 Acres (16982 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	1.35	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 29%
Precipitation (P) – 1”
Rv – 0.31”
Water Quality Volume – 0.03 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.02 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.97	1.96
Post-Development Peak Discharge Rate (cfs)	3	2.04
Percent Change	3	3.92

DA-02

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 1.24 Acres
Increased Impervious Areas from Panels – 0.55 Acres (24041 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	1.24	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 45%
Precipitation (P) – 1”
Rv – 0.45”
Water Quality Volume – 0.05 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.02 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	8.35	14.03
Post-Development Peak Discharge Rate (cfs)	8.35	14.03
Percent Change	0	0

DA-03

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.18 Acres
Increased Impervious Areas from Panels – 0.03 Acres (1427 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.18	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 18%
Precipitation (P) – 1”
Rv – 0.21”
Water Quality Volume – 0.003 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.001 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.19	0.39
Post-Development Peak Discharge Rate (cfs)	0.2	0.4
Percent Change	5	2.5

DA-04

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.16 Acres
Increased Impervious Areas from Panels – 0.03 Acres (1427 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.16	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 11%
Precipitation (P) – 1”
Rv – 0.15”
Water Quality Volume – 0.002 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.0008 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.14	0.29
Post-Development Peak Discharge Rate (cfs)	0.14	0.29
Percent Change	0	0

DA-05

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 3.65 Acres
Increased Impervious Areas from Panels – 1.38 Acres (60248 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	3.65	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 38%
Precipitation (P) – 1”
Rv – 0.39”
Water Quality Volume – 0.12 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.06 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	1.73	3.63
Post-Development Peak Discharge Rate (cfs)	1.8	3.7
Percent Change	3.89	1.89

DA-06

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.50 Acres
Increased Impervious Areas from Panels – 0.13 Acres (5868 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.5	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 38%
Precipitation (P) – 1”
Rv – 0.39”
Water Quality Volume – 0.12 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip and the use of a gravel diaphragm. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.006 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.47	0.97
Post-Development Peak Discharge Rate (cfs)	0.49	1.01
Percent Change	4.08	3.96

DA-06

GRAVEL DIAPHRAGM DESIGN

Water Quality Volume (WQv) for New Impervious Area Construction for Panels on Slopes >/= 8%

Total Area to Be Treated by Gravel Diaphragm – 0.2 Acres
Increased Impervious Areas from Panels – 0.04 Acres (1681 sft)
Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 19%
Precipitation (P) – 1”
Rv – 0.22”
Water Quality Volume – 0.004 Acre-feet (162 cft)

Gravel Diaphragm Dimensions – 2’(W) x 1’(D) x 133’(L)
Total Volume of Diaphragm – 266 cft

DA-07

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.68 Acres
Increased Impervious Areas from Panels – 0.27 Acres (11813 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.68	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 40%
Precipitation (P) – 1”
Rv – 0.41”
Water Quality Volume – 0.02 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip and the use of a gravel diaphragm. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.01 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.52	1.09
Post-Development Peak Discharge Rate (cfs)	0.54	1.11
Percent Change	3.70	1.8

DA-07

GRAVEL DIAPHRAGM DESIGN

Water Quality Volume (WQv) for New Impervious Area Construction for Panels on Slopes >/= 8%

Total Area to Be Treated by Gravel Diaphragm – 0.15 Acres
Increased Impervious Areas from Panels – 0.06 Acres (2429 sft)
Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 37%
Precipitation (P) – 1”
Rv – 0.39”
Water Quality Volume – 0.005 Acre-feet (209 cft)

Gravel Diaphragm Dimensions – 2’(W) x 1’(D) x 107’(L)
Total Volume of Diaphragm – 214 cft

DA-08

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 1.39 Acres
Increased Impervious Areas from Panels – 0.63 Acres (27304 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	1.39	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 45%
Precipitation (P) – 1”
Rv – 0.46”
Water Quality Volume – 0.05 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip and the use of a gravel diaphragm. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.03 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.42	0.88
Post-Development Peak Discharge Rate (cfs)	0.44	0.90
Percent Change	4.55	2.22

DA-08

GRAVEL DIAPHRAGM DESIGN

Water Quality Volume (WQv) for New Impervious Area Construction for Panels on Slopes >/= 8%

Total Area to Be Treated by Gravel Diaphragm – 0.78 Acres
Increased Impervious Areas from Panels – 0.09 Acres (4027 sft)
Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 12%
Precipitation (P) – 1”
Rv – 0.16”
Water Quality Volume – 0.01 Acre-feet (443 cft)

Gravel Diaphragm Dimensions – 4’(W) x 1’(D) x 124’(L)
Total Volume of Diaphragm – 496 cft

DA-09

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.63 Acres
Increased Impervious Areas from Panels – 0.13 Acres (27304 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.63	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 21%
Precipitation (P) – 1”
Rv – 0.24”
Water Quality Volume – 0.01 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip and the use of a gravel diaphragm. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.01 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.54	1.12
Post-Development Peak Discharge Rate (cfs)	0.56	1.14
Percent Change	3.57	1.75

DA-09

GRAVEL DIAPHRAGM DESIGN

Water Quality Volume (WQv) for New Impervious Area Construction for Panels on Slopes >/= 8%

Total Area to Be Treated by Gravel Diaphragm – 0.18 Acres
Increased Impervious Areas from Panels – 0.007 Acres (325 sft)
Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 4%
Precipitation (P) – 1”
Rv – 0.09”
Water Quality Volume – 0.001 Acre-feet (57 cft)

Gravel Diaphragm Dimensions – 1’(W) x 1’(D) x 151’(L)
Total Volume of Diaphragm – 151 cft

DA-10

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.29 Acres
Increased Impervious Areas from Panels – 0.05 Acres (2365 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0.06	19.8
D	77	0.29	80.2

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 76
% Impervious (I) – 19%
Precipitation (P) – 1”
Rv – 0.22”
Water Quality Volume – 0.01 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.002 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	0.11	0.23
Post-Development Peak Discharge Rate (cfs)	0.11	0.24
Percent Change	0	4.17

DA-11

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.08 Acres
Increased Impervious Areas from Panels – 0 Acres (0 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	0.08	100

Water Quality Volume (WQv) for New Impervious Area Construction

There is no increase in impervious area in this drainage area.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

There is no increase in impervious area in this drainage area.

Channel Protection Volume (CPv)

There is no increase in impervious area in this drainage area.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

There is no increase in impervious area in this drainage area.

DA-12

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 3.37 Acres
Increased Impervious Areas from Panels – 1.23 Acres (53459 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	3.37	100

Water Quality Volume (WQv) for New Impervious Area Construction

Composite Runoff Curve Number (RCN) – 77
% Impervious (I) – 36%
Precipitation (P) – 1”
Rv – 0.38”
Water Quality Volume – 0.11 Acre-feet

The water quality volume (WQv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025. WQv sizing for gravel diaphragm provided below.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

% Impervious (I) – 100% for calculating Rv
Rv – 0.95”
Runoff Reduction Factor (S) – 0.55 for HSG D
Runoff Reduction Volume (RRv) – 0.05 Acre-feet

The runoff reduction volume (RRv) is met through non-rooftop disconnected runoff through filter strip. Lengths of filter strips between solar panel tables is equal to or greater than the width of the proposed tables and meets the criteria set forth by the NYSDEC Solar Development Webinar July 28, 2025.

Channel Protection Volume (CPv)

There are no SMPs proposed for controlling CPv. CPv requirements are met through fulfilling the requirements of the RRv in accordance with NYSDEC Stormwater Manual 2024.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

The proposed development will not alter hydrology from pre-construction conditions to post-construction conditions according to the definition of “alter hydrology” in Appendix A of the General Permit. See HydroCAD outputs in this Appendix for SCS TR-20 calculations for peak discharge rates.

	10-year	100-Year
Pre-Development Peak Discharge Rate (cfs)	1.42	3.06
Post-Development Peak Discharge Rate (cfs)	1.49	3.14
Percent Change	4.70	2.55

DA-13

UNIFIED STORMWATER SIZING CRITERIA (NYSDEC STORMWATER MANUAL 2024)

Total Area – 0.81 Acres
Increased Impervious Areas from Construction Activities – 0 Acres (0 sft)

HSG	RCN	Area (Acres)	Percent
A	38	0	0
B	55	0	0
C	70	0	0
D	77	3.37	100

Water Quality Volume (WQv) for New Impervious Area Construction

There is no increase in impervious area in this drainage area.

Runoff Reduction Volume (RRv) for New Impervious Area Construction

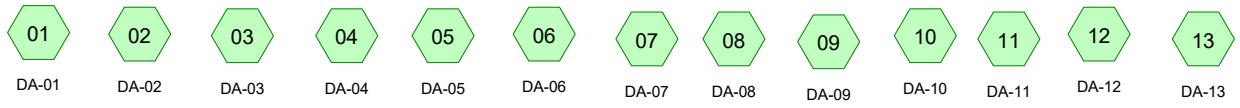
There is no increase in impervious area in this drainage area.

Channel Protection Volume (CPv)

There is no increase in impervious area in this drainage area.

Overbank Flood Protection (Qp) and Extreme Flood (Qf)

There is no increase in impervious area in this drainage area.



Subcat Reach Pond Link

Routing Diagram for Pre-Dev

Prepared by Haley & Aldrich, Inc, Printed 10/15/2025

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	Type II 24-hr		Default	24.00	1	1.87	2
2	2-yr	Type II 24-hr		Default	24.00	1	2.20	2
3	10-yr	Type II 24-hr		Default	24.00	1	3.16	2
4	100-yr	Type II 24-hr		Default	24.00	1	4.70	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.540	98	Paved parking, HSG D (02, 12, 13)
2.620	70	Woods, Good, HSG C (12)
11.398	77	Woods, Good, HSG D (01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12)
0.400	79	Woods/grass comb., Good, HSG D (13)
16.958	**79**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
2.620	HSG C	12
14.338	HSG D	01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12, 13
0.000	Other	
16.958		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2.540	0.000	2.540	Paved parking	02, 12, 13
0.000	0.000	2.620	11.398	0.000	14.018	Woods, Good	01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12
0.000	0.000	0.000	0.400	0.000	0.400	Woods/grass comb., Good	13
0.000	**0.000**	**2.620**	**14.338**	**0.000**	**16.958**	**TOTAL AREA**	

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	02	0.00	0.00	40.0	0.0100	0.011	0.0	18.0	0.0	

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>0.33"
Flow Length=235' Tc=46.6 min CN=77 Runoff=0.25 cfs 0.037 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>0.79"
Flow Length=916' Tc=7.6 min CN=88 Runoff=3.74 cfs 0.172 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=100' Slope=0.0700 '/' Tc=27.3 min CN=77 Runoff=0.05 cfs 0.005 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=100' Slope=0.0350 '/' Tc=36.0 min CN=77 Runoff=0.04 cfs 0.004 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>0.31"
Flow Length=494' Tc=113.0 min CN=77 Runoff=0.47 cfs 0.121 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=179' Tc=32.5 min CN=77 Runoff=0.12 cfs 0.014 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=330' Tc=41.0 min CN=77 Runoff=0.14 cfs 0.018 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>0.32"
Flow Length=417' Tc=58.0 min CN=77 Runoff=0.11 cfs 0.018 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=150' Slope=0.0740 '/' Tc=37.0 min CN=77 Runoff=0.14 cfs 0.017 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.31"
Flow Length=200' Tc=112.5 min CN=77 Runoff=0.03 cfs 0.008 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.33"
Flow Length=62' Slope=0.0215 '/' Tc=29.9 min CN=77 Runoff=0.08 cfs 0.008 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>0.28"
Flow Length=733' Tc=113.9 min CN=76 Runoff=0.36 cfs 0.097 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>0.85"
Tc=5.0 min CN=89 Runoff=1.36 cfs 0.058 af

Total Runoff Area = 16.958 ac Runoff Volume = 0.577 af Average Runoff Depth = 0.41"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

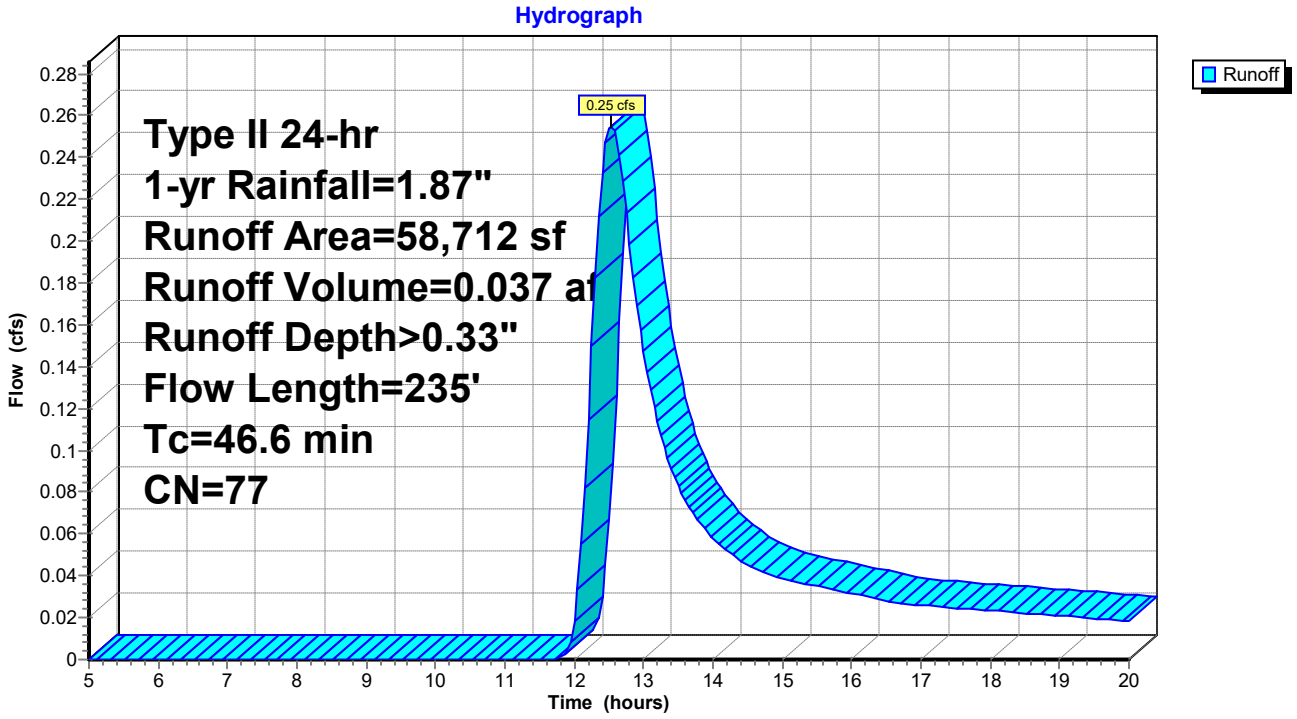
Runoff = 0.25 cfs @ 12.53 hrs, Volume= 0.037 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (sf)	CN	Description
58,712	77	Woods, Good, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.1	150	0.0450	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
46.6	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

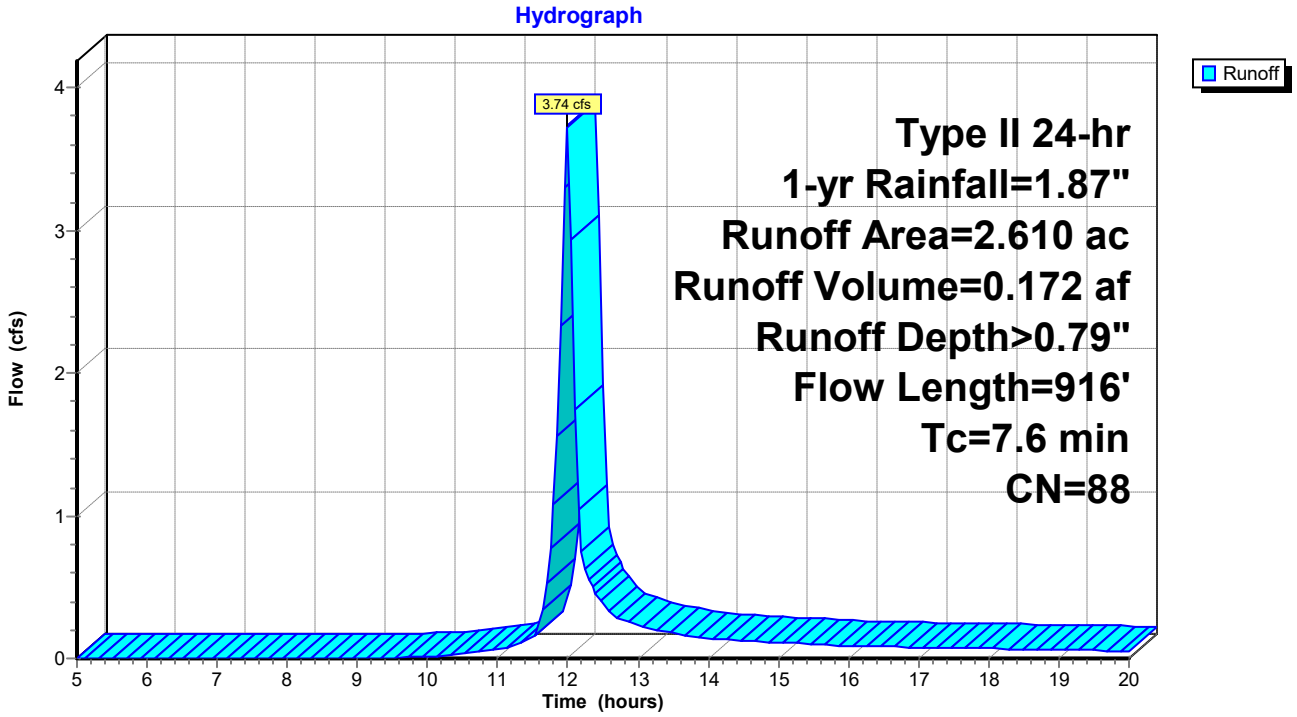
Runoff = 3.74 cfs @ 11.99 hrs, Volume= 0.172 af, Depth> 0.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
1.240	77	Woods, Good, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

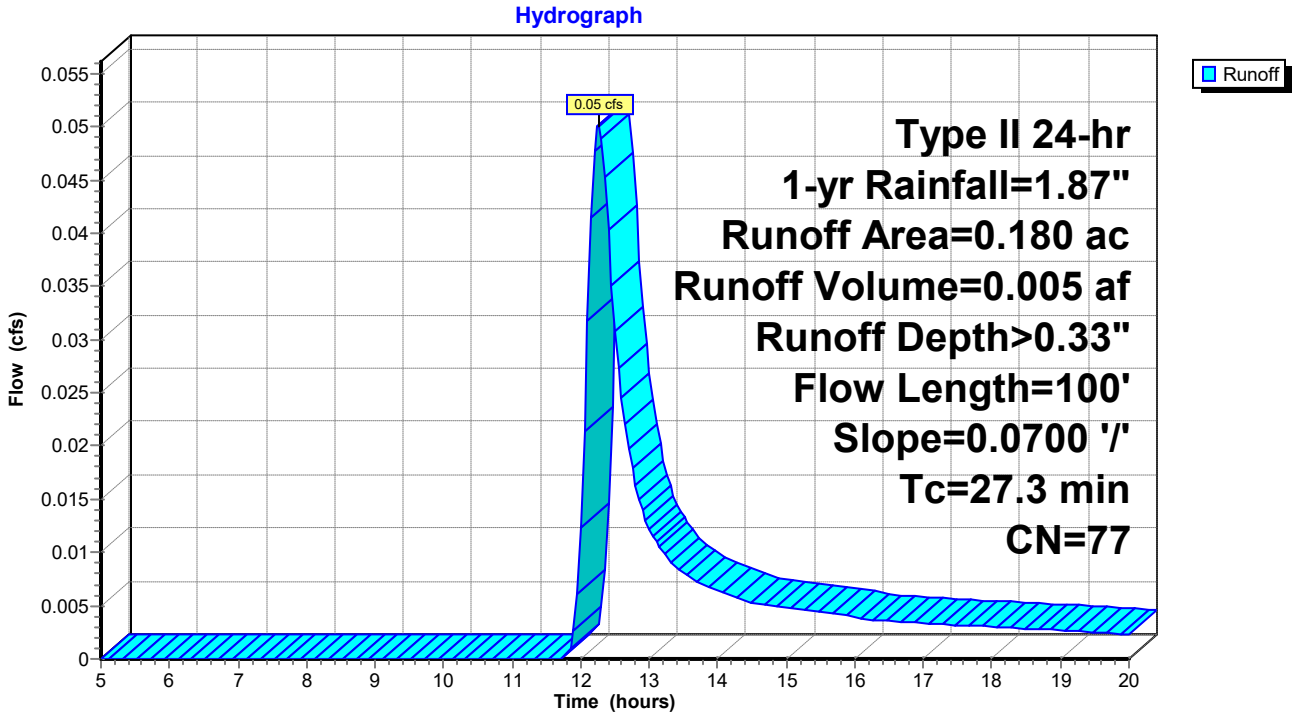
Runoff = 0.05 cfs @ 12.26 hrs, Volume= 0.005 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.180	77	Woods, Good, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.3	100	0.0700	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

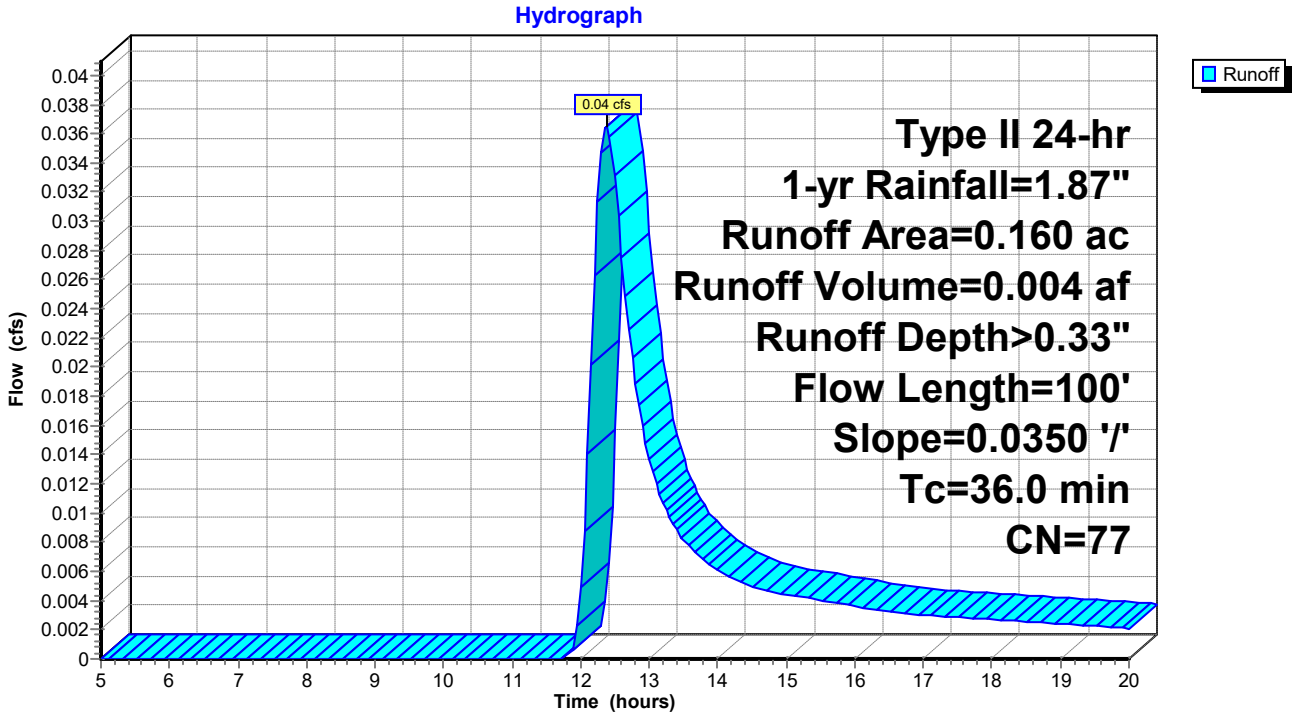
Runoff = 0.04 cfs @ 12.37 hrs, Volume= 0.004 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.160	77	Woods, Good, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.0	100	0.0350	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

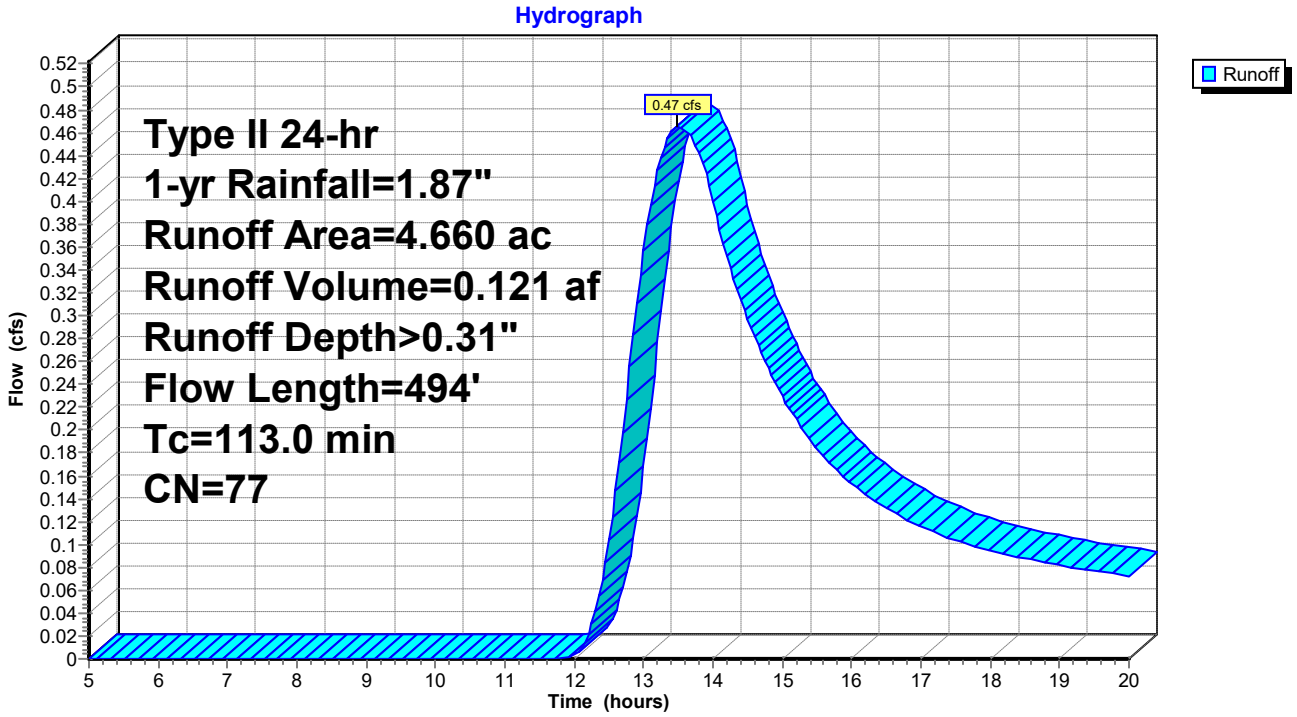
Runoff = 0.47 cfs @ 13.47 hrs, Volume= 0.121 af, Depth> 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
4.660	77	Woods, Good, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.3	150	0.0075	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
113.0	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

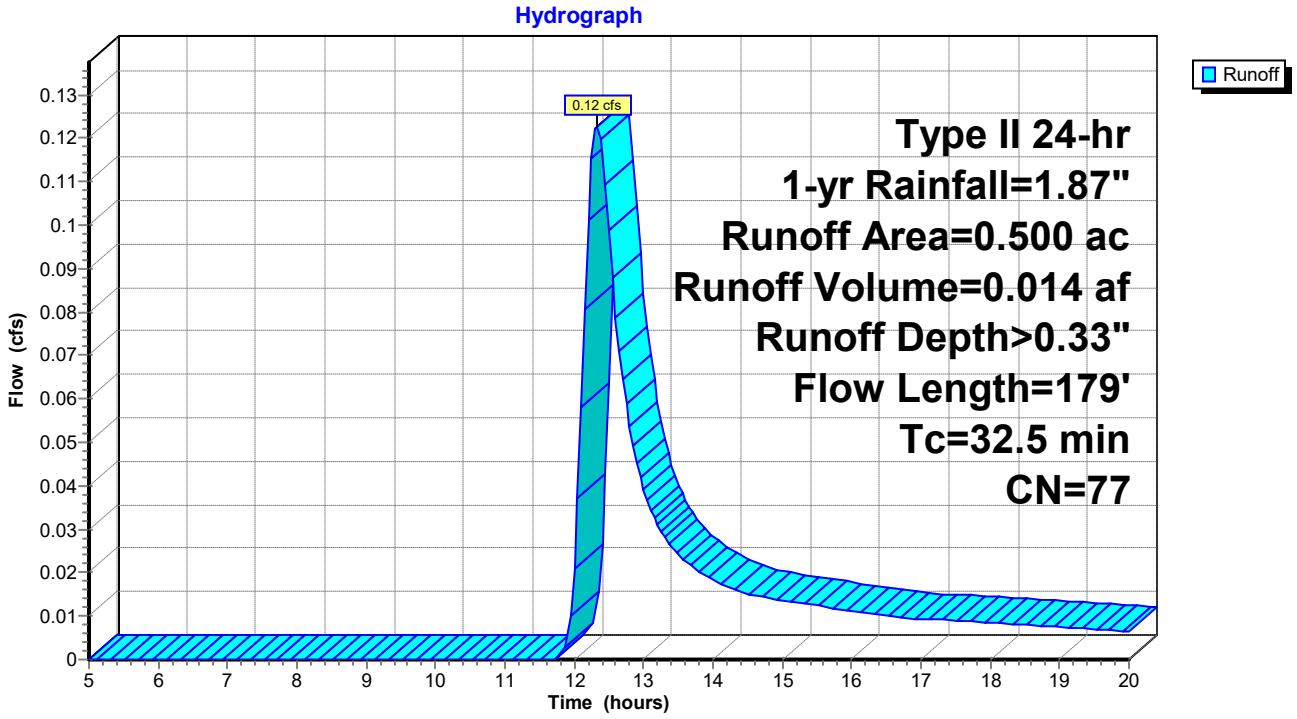
Runoff = 0.12 cfs @ 12.33 hrs, Volume= 0.014 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.500	77	Woods, Good, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

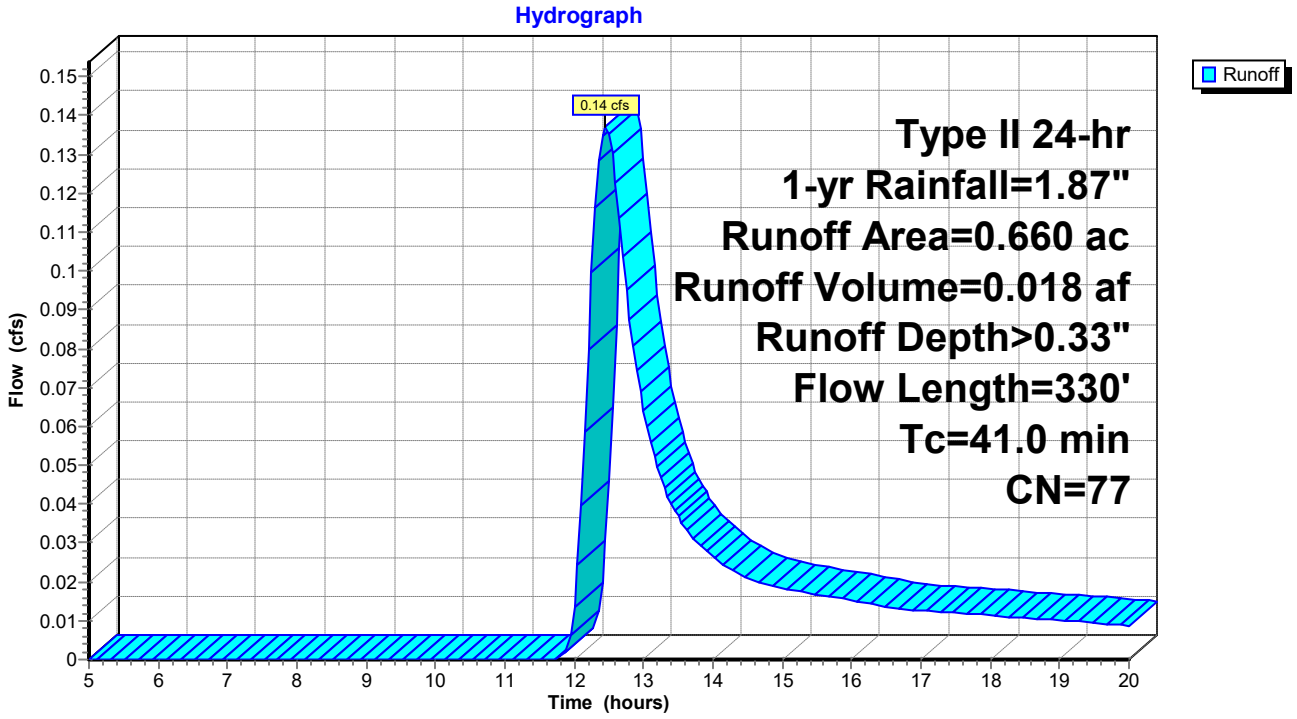
Runoff = 0.14 cfs @ 12.45 hrs, Volume= 0.018 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.660	77	Woods, Good, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.8	150	0.0750	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
41.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

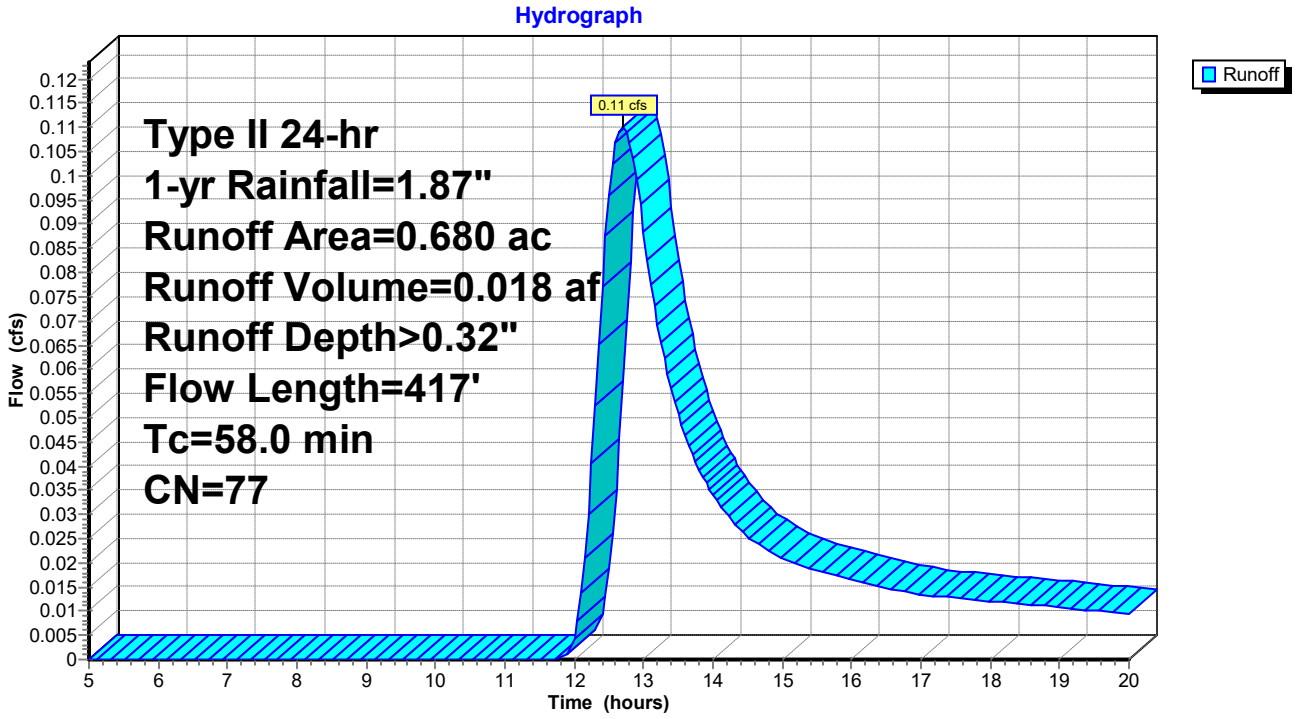
Runoff = 0.11 cfs @ 12.70 hrs, Volume= 0.018 af, Depth> 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.680	77	Woods, Good, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
51.7	150	0.0320	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
58.0	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

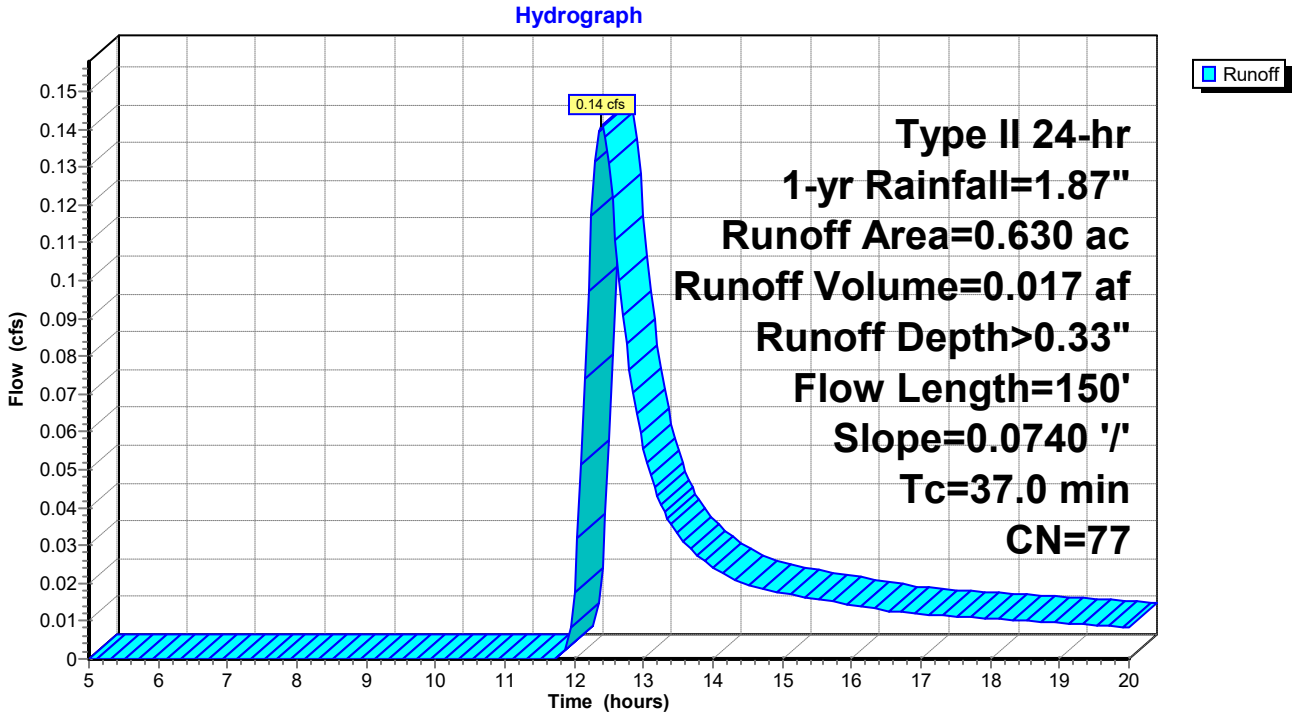
Runoff = 0.14 cfs @ 12.39 hrs, Volume= 0.017 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.630	77	Woods, Good, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.0	150	0.0740	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

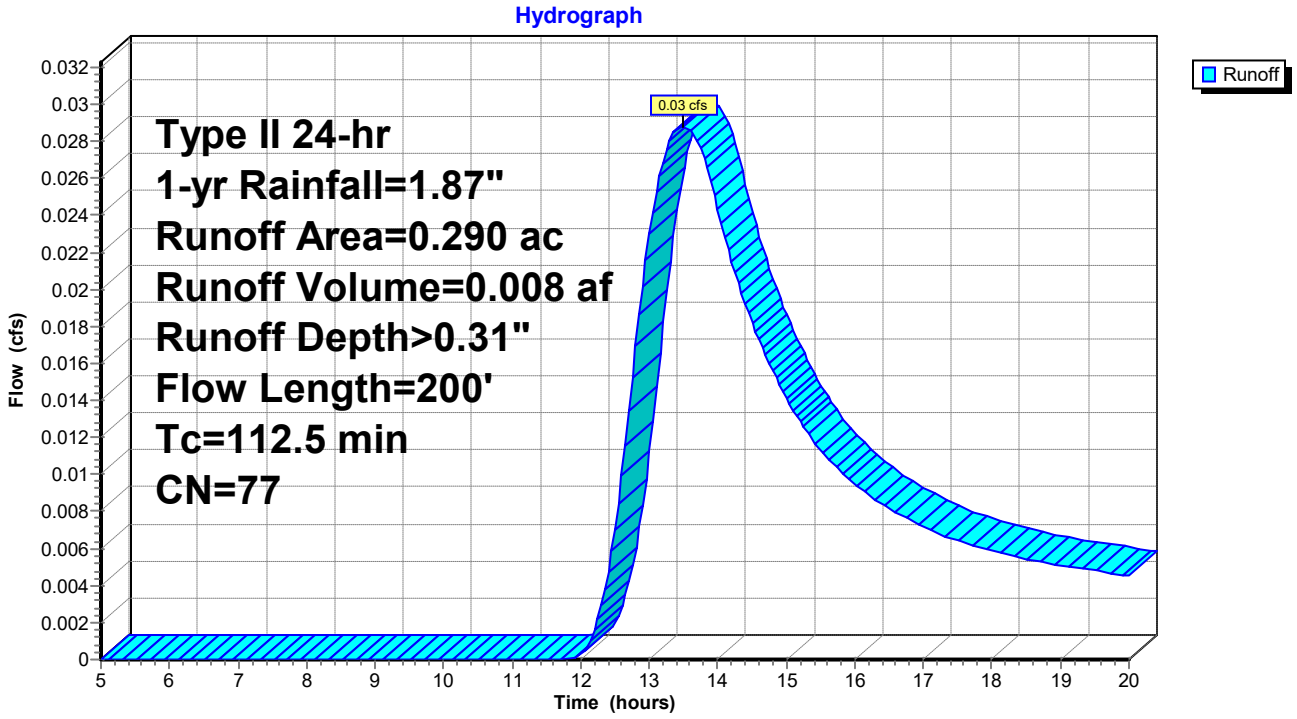
Runoff = 0.03 cfs @ 13.48 hrs, Volume= 0.008 af, Depth> 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

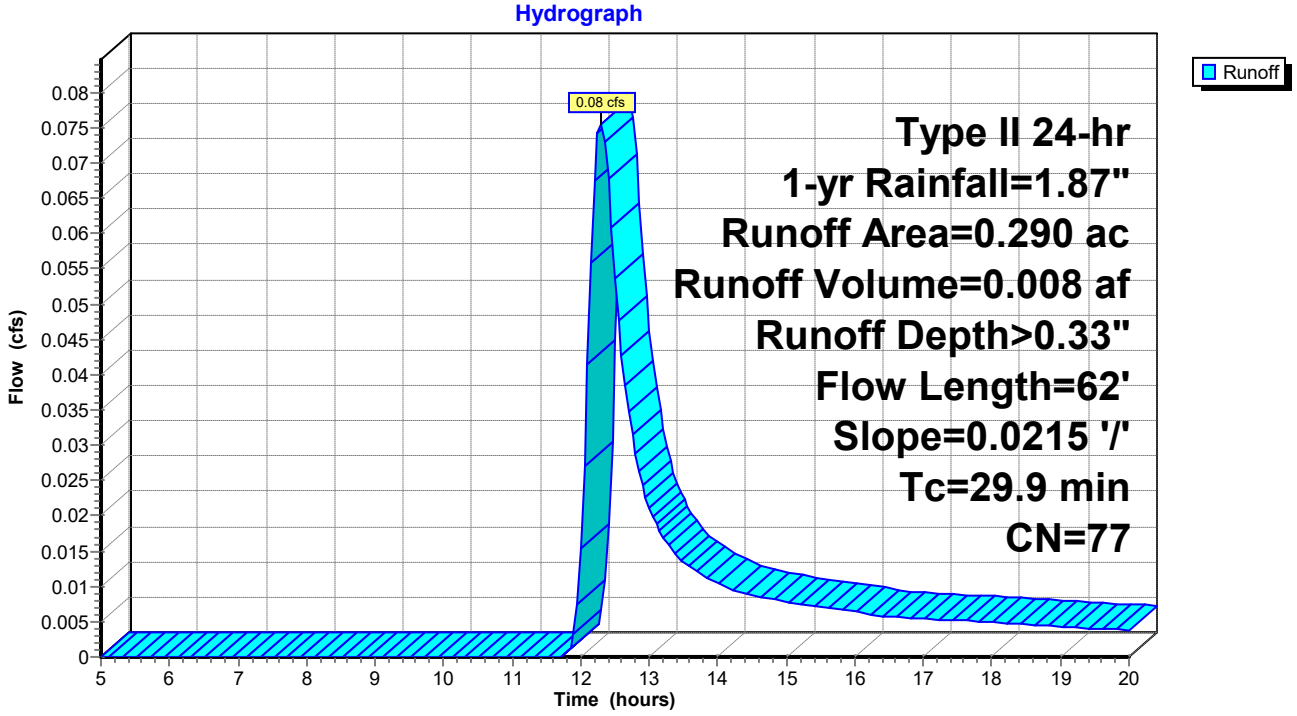
Runoff = 0.08 cfs @ 12.29 hrs, Volume= 0.008 af, Depth> 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.9	62	0.0215	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

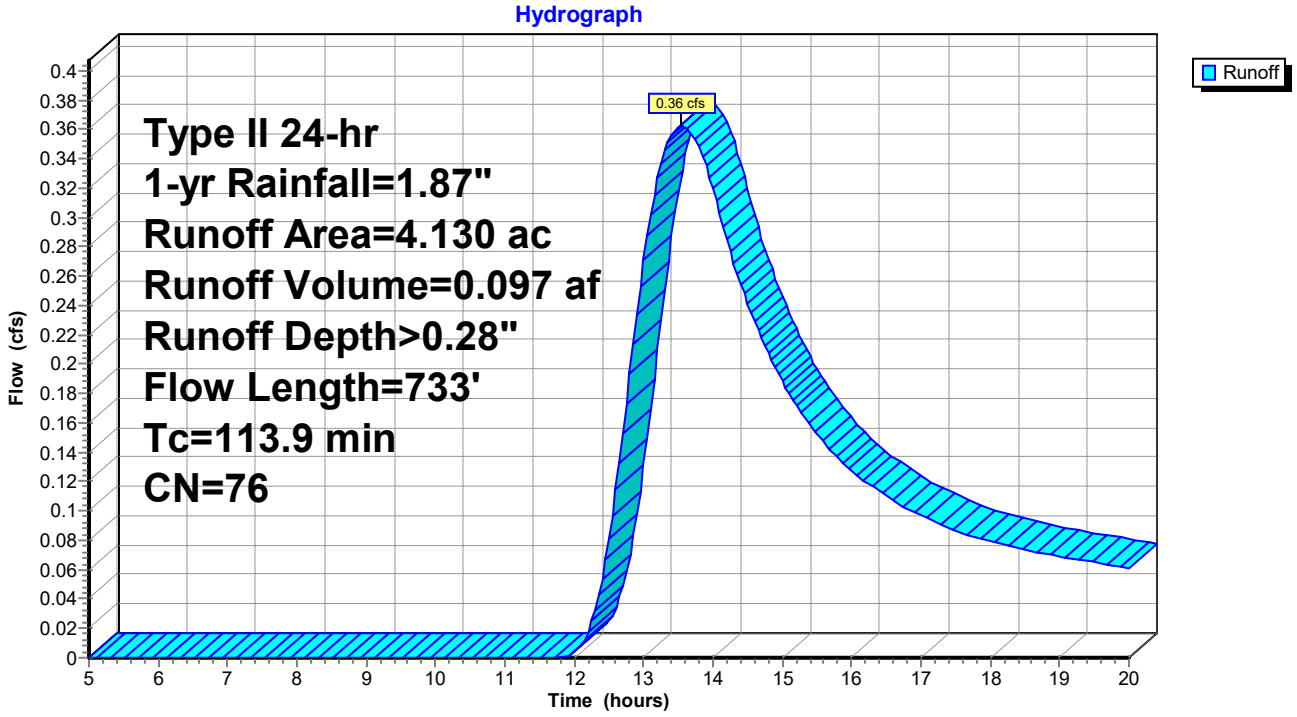
Runoff = 0.36 cfs @ 13.55 hrs, Volume= 0.097 af, Depth> 0.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	77	Woods, Good, HSG D
2.620	70	Woods, Good, HSG C
4.130	76	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.8	150	0.0074	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
113.9	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

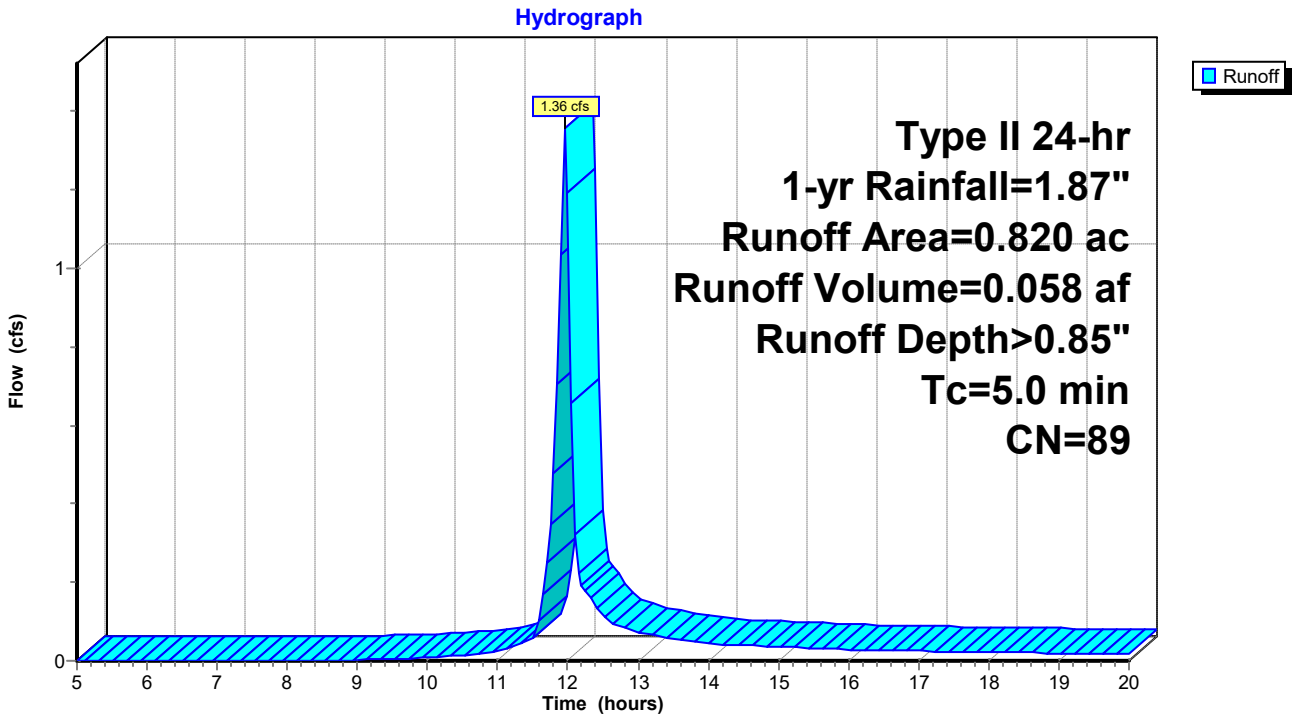
Runoff = 1.36 cfs @ 11.96 hrs, Volume= 0.058 af, Depth> 0.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>0.49"
Flow Length=235' Tc=46.6 min CN=77 Runoff=0.41 cfs 0.055 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>1.04"
Flow Length=916' Tc=7.6 min CN=88 Runoff=4.88 cfs 0.226 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=100' Slope=0.0700 '/' Tc=27.3 min CN=77 Runoff=0.08 cfs 0.007 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=100' Slope=0.0350 '/' Tc=36.0 min CN=77 Runoff=0.06 cfs 0.007 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>0.47"
Flow Length=494' Tc=113.0 min CN=77 Runoff=0.74 cfs 0.181 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=179' Tc=32.5 min CN=77 Runoff=0.20 cfs 0.020 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=330' Tc=41.0 min CN=77 Runoff=0.22 cfs 0.027 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>0.48"
Flow Length=417' Tc=58.0 min CN=77 Runoff=0.18 cfs 0.027 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=150' Slope=0.0740 '/' Tc=37.0 min CN=77 Runoff=0.23 cfs 0.026 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.47"
Flow Length=200' Tc=112.5 min CN=77 Runoff=0.05 cfs 0.011 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.49"
Flow Length=62' Slope=0.0215 '/' Tc=29.9 min CN=77 Runoff=0.12 cfs 0.012 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>0.43"
Flow Length=733' Tc=113.9 min CN=76 Runoff=0.59 cfs 0.149 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>1.11"
Tc=5.0 min CN=89 Runoff=1.76 cfs 0.076 af

Total Runoff Area = 16.958 ac Runoff Volume = 0.824 af Average Runoff Depth = 0.58"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

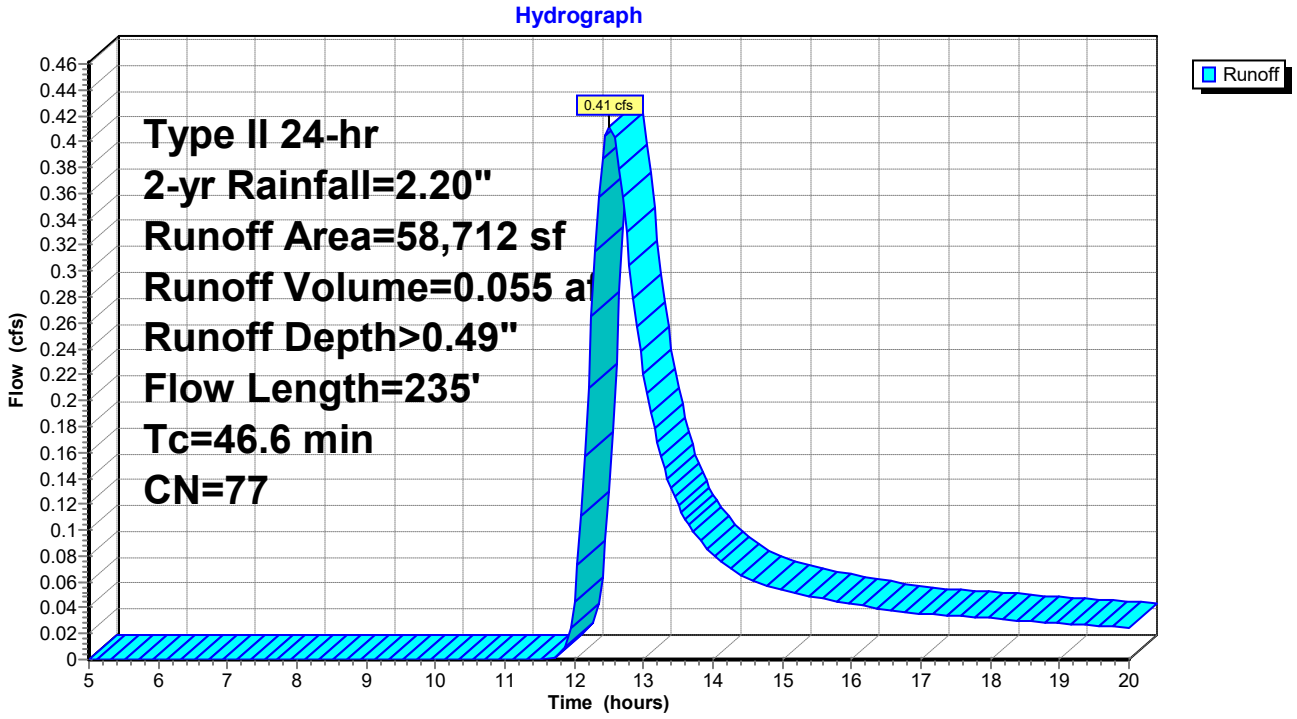
Runoff = 0.41 cfs @ 12.51 hrs, Volume= 0.055 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (sf)	CN	Description
58,712	77	Woods, Good, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.1	150	0.0450	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
46.6	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

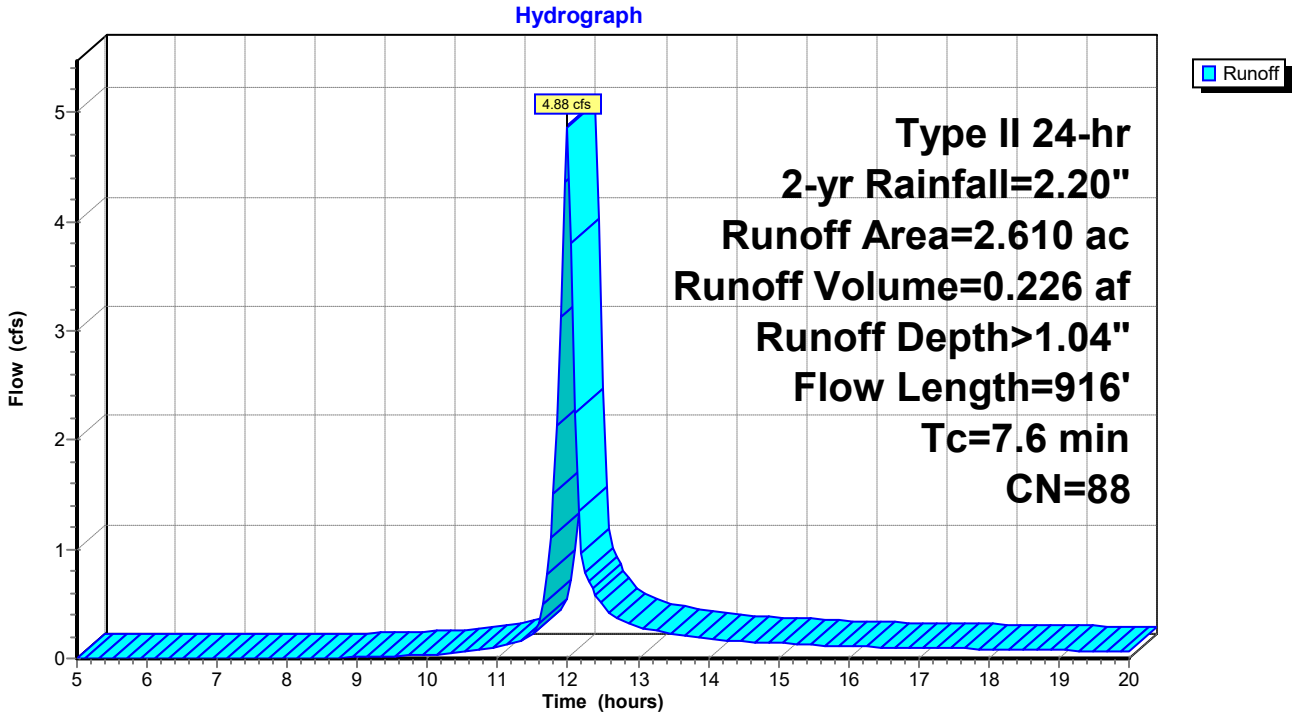
Runoff = 4.88 cfs @ 11.99 hrs, Volume= 0.226 af, Depth> 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
1.240	77	Woods, Good, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

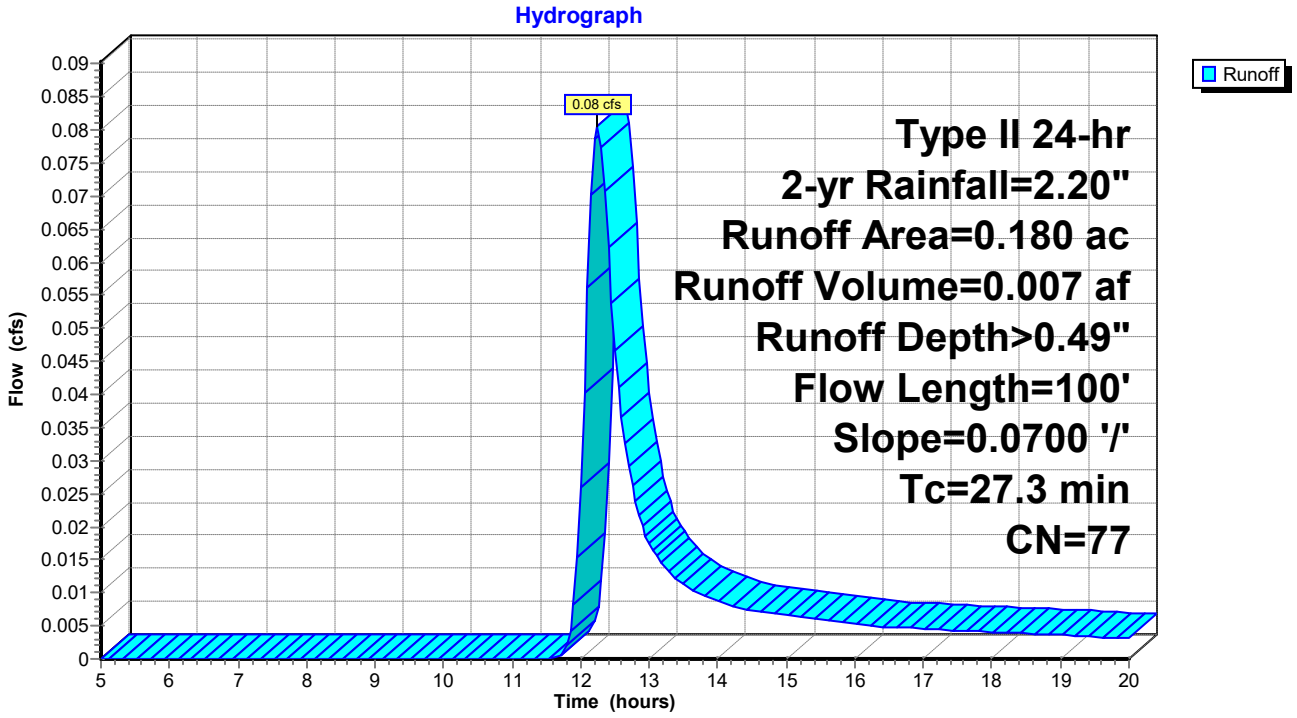
Runoff = 0.08 cfs @ 12.24 hrs, Volume= 0.007 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.180	77	Woods, Good, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.3	100	0.0700	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

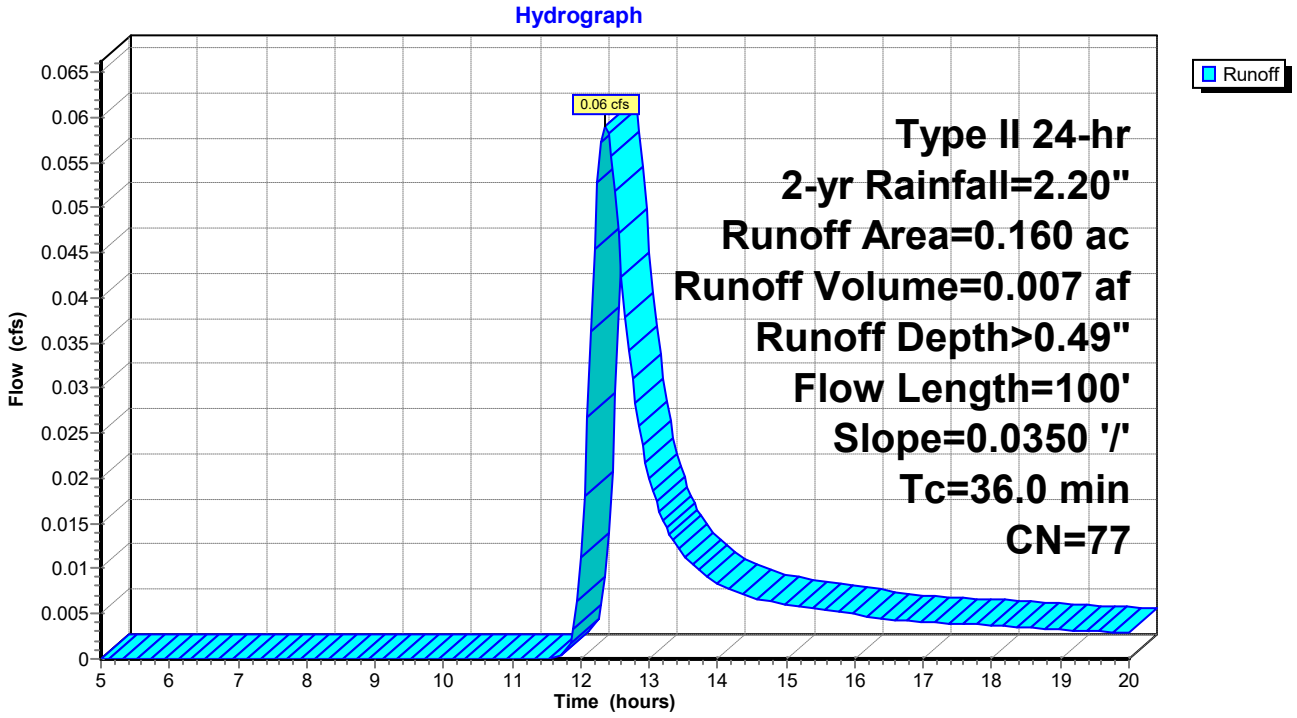
Runoff = 0.06 cfs @ 12.36 hrs, Volume= 0.007 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.160	77	Woods, Good, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.0	100	0.0350	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

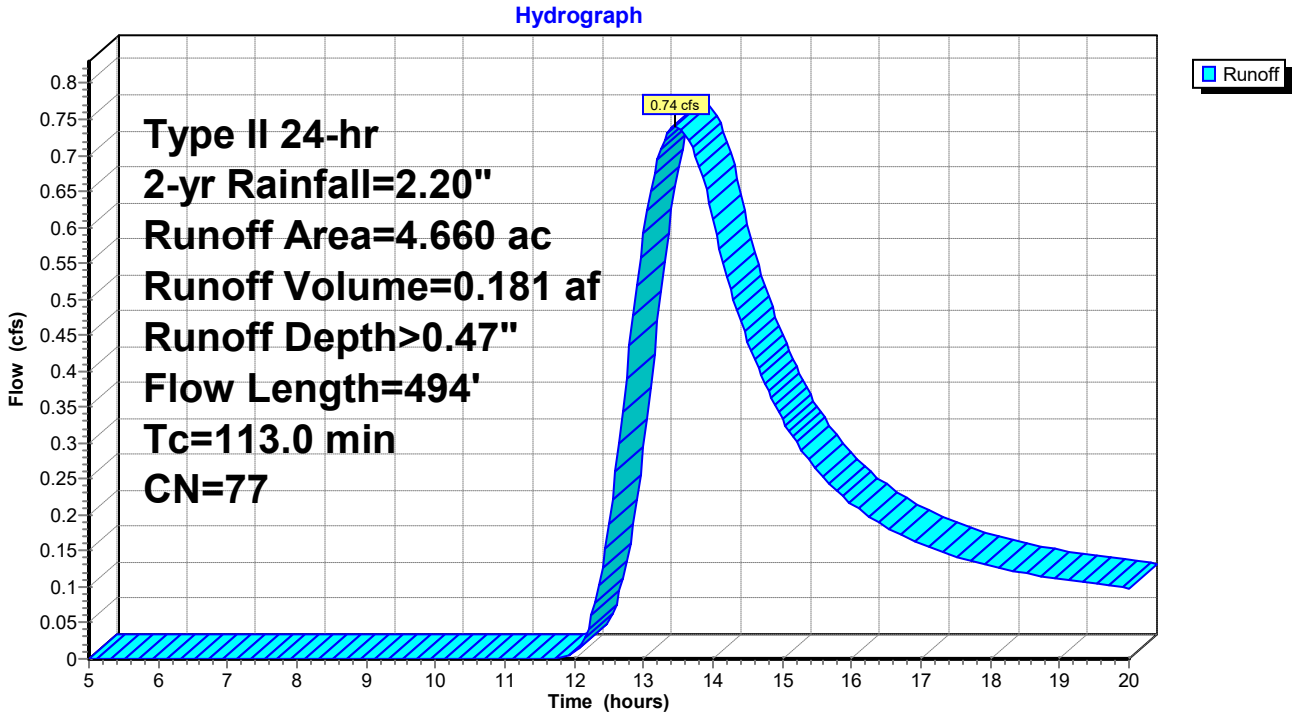
Runoff = 0.74 cfs @ 13.45 hrs, Volume= 0.181 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
4.660	77	Woods, Good, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.3	150	0.0075	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
113.0	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

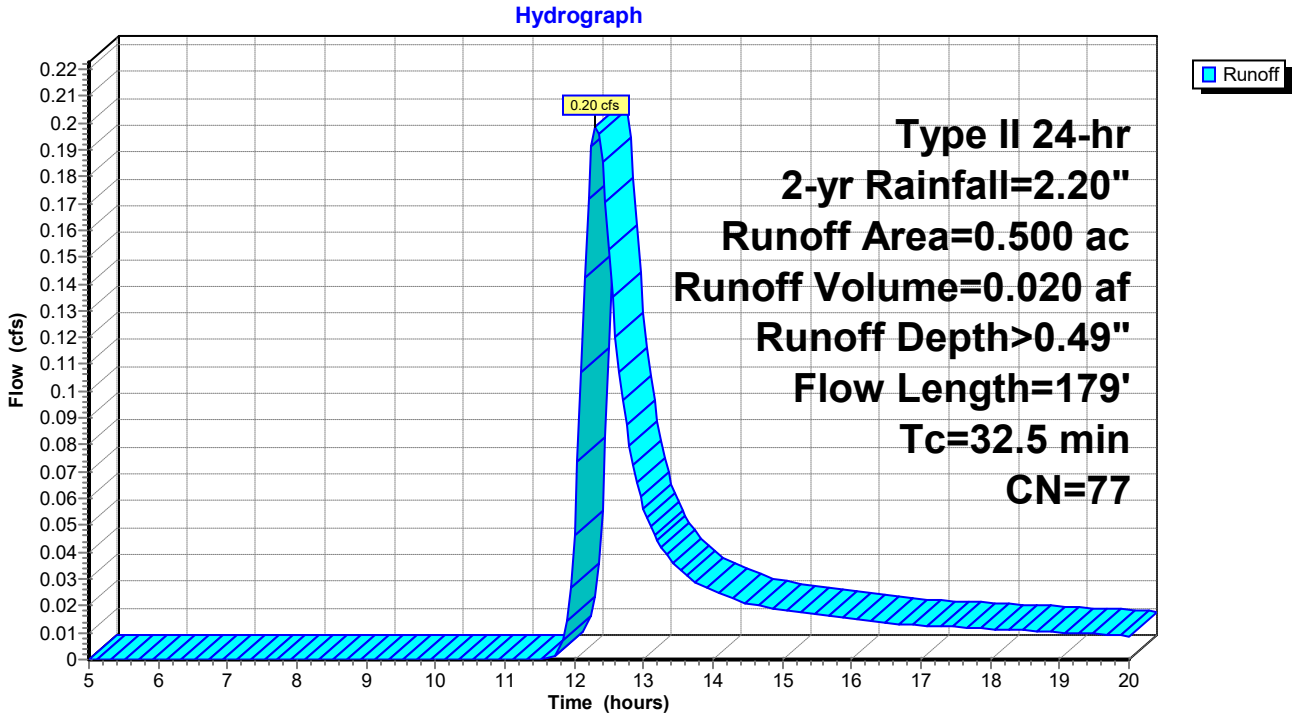
Runoff = 0.20 cfs @ 12.31 hrs, Volume= 0.020 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.500	77	Woods, Good, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

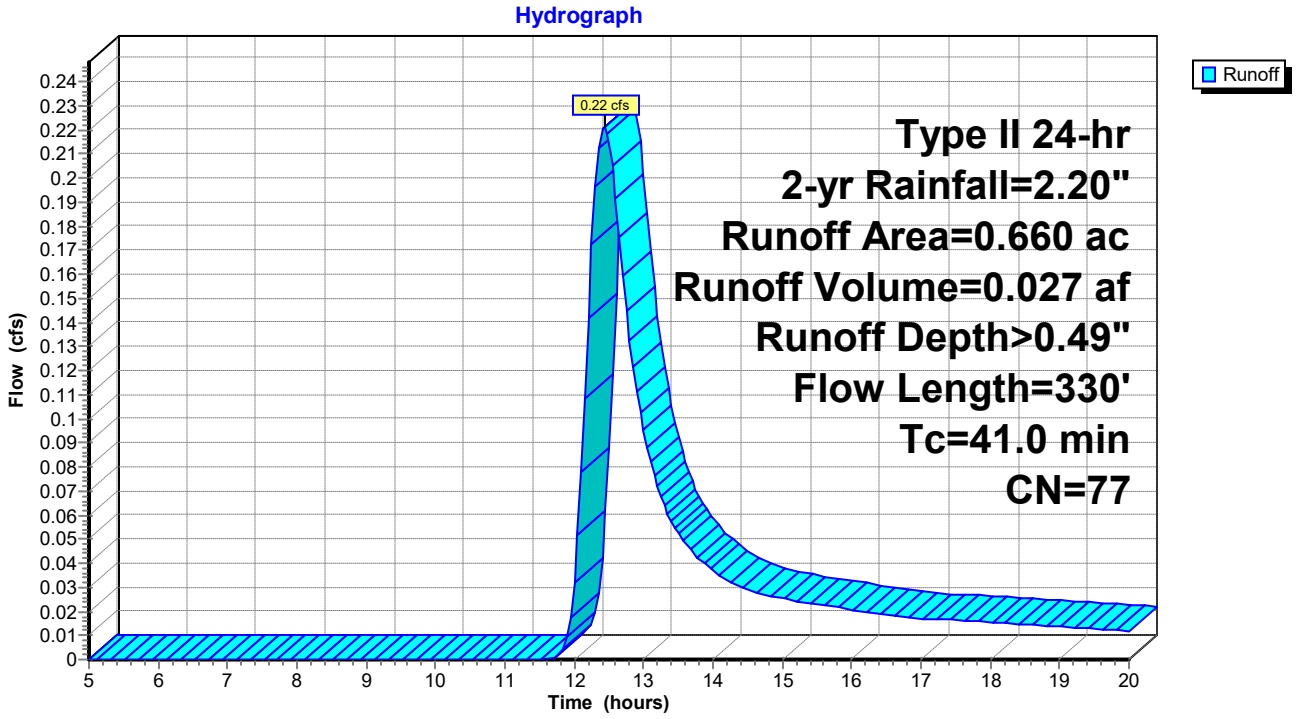
Runoff = 0.22 cfs @ 12.43 hrs, Volume= 0.027 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.660	77	Woods, Good, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.8	150	0.0750	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
41.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

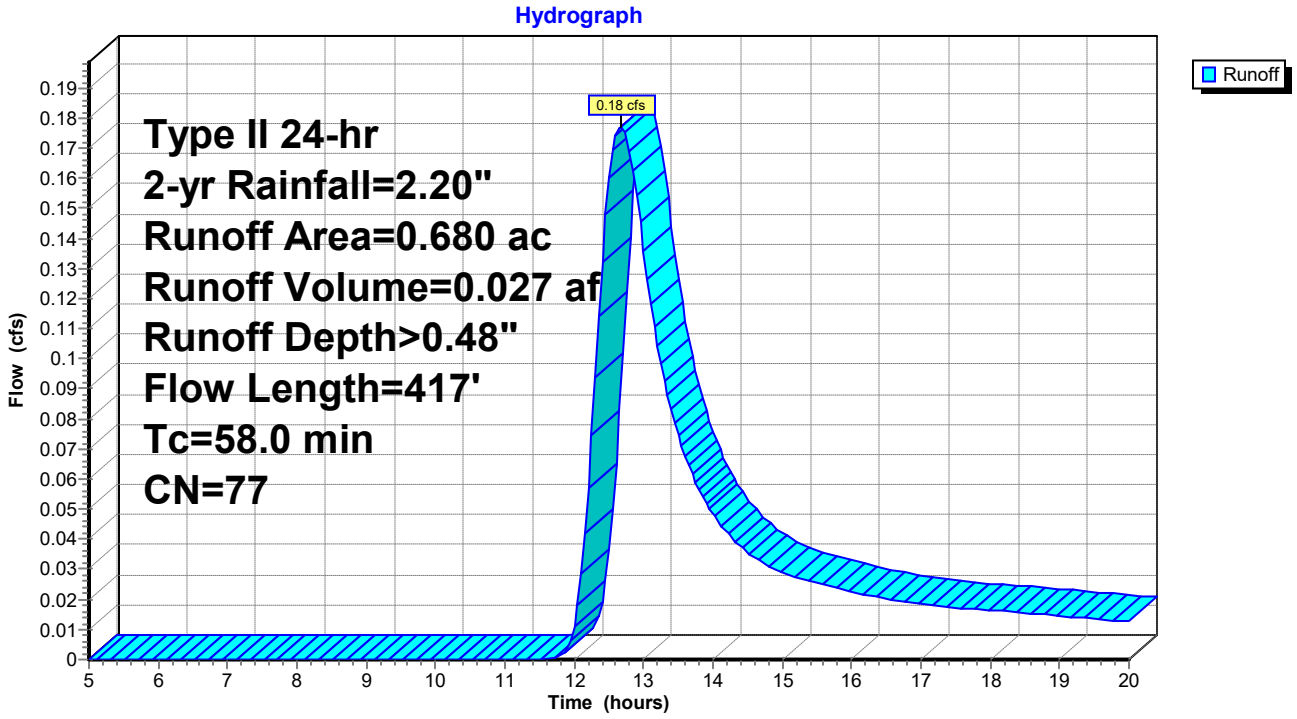
Runoff = 0.18 cfs @ 12.68 hrs, Volume= 0.027 af, Depth> 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.680	77	Woods, Good, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
51.7	150	0.0320	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
58.0	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

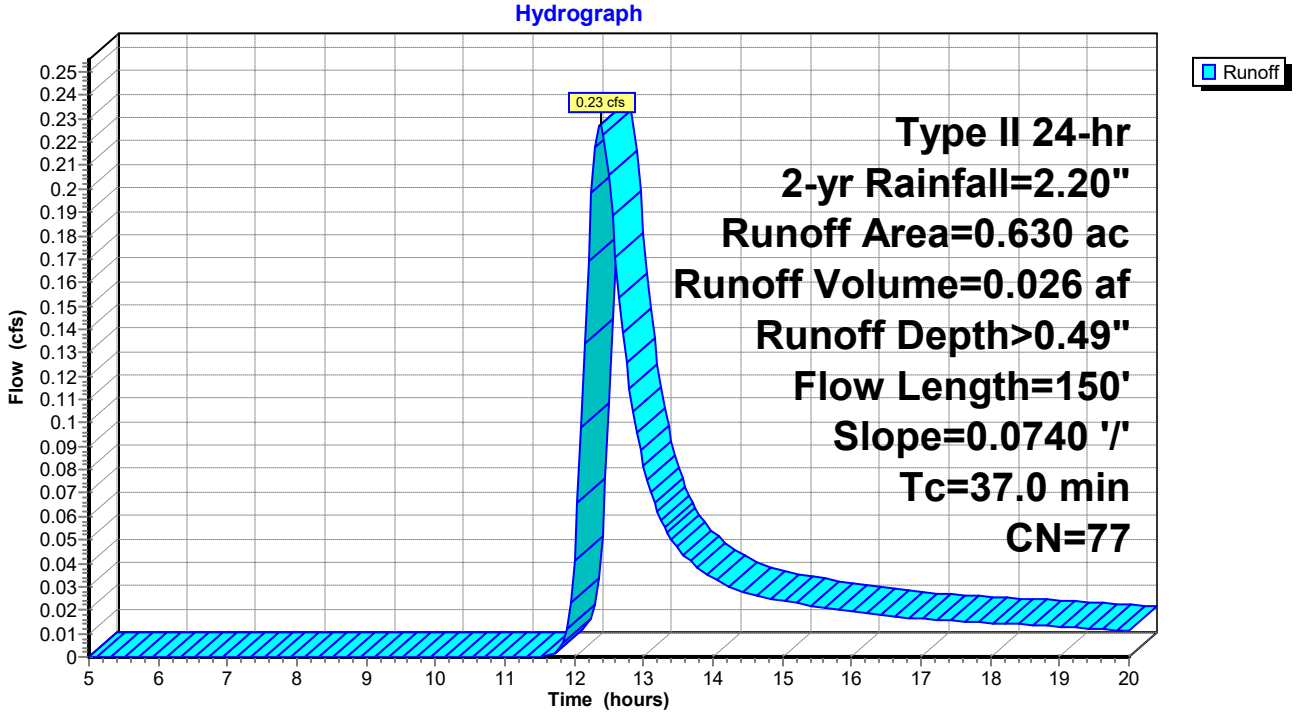
Runoff = 0.23 cfs @ 12.37 hrs, Volume= 0.026 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.630	77	Woods, Good, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.0	150	0.0740	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

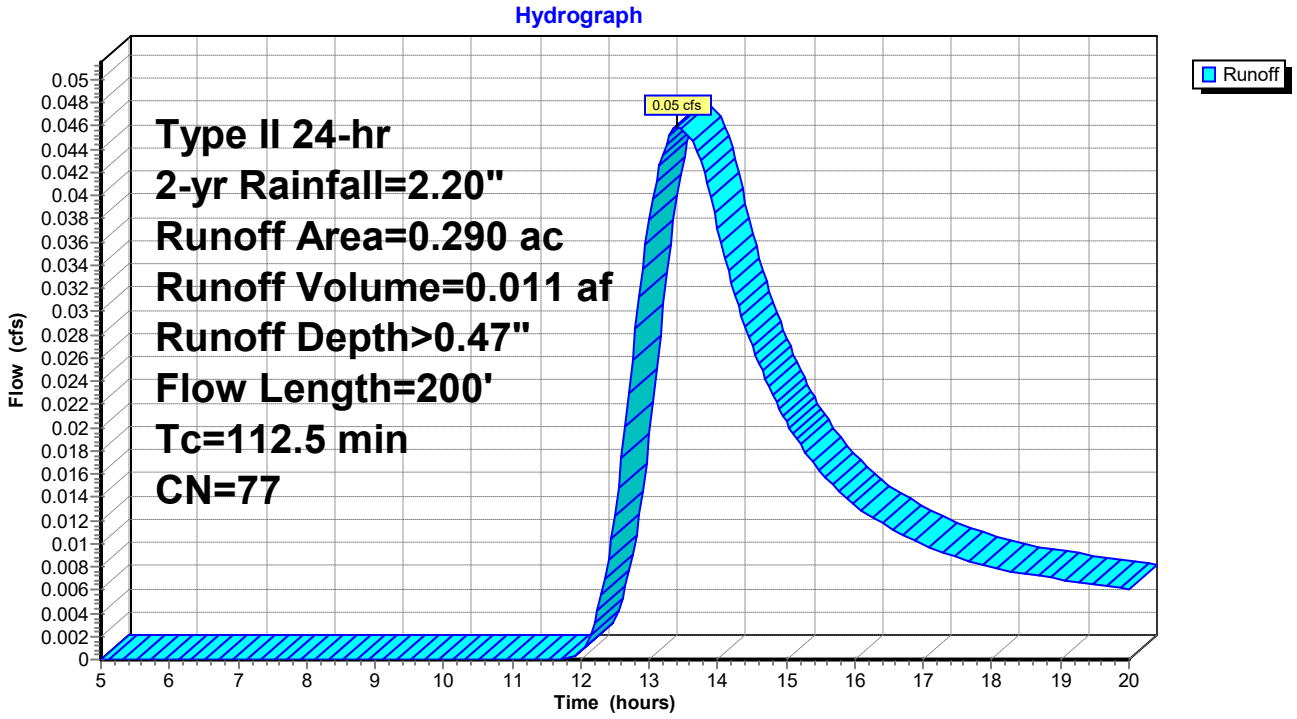
Runoff = 0.05 cfs @ 13.41 hrs, Volume= 0.011 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

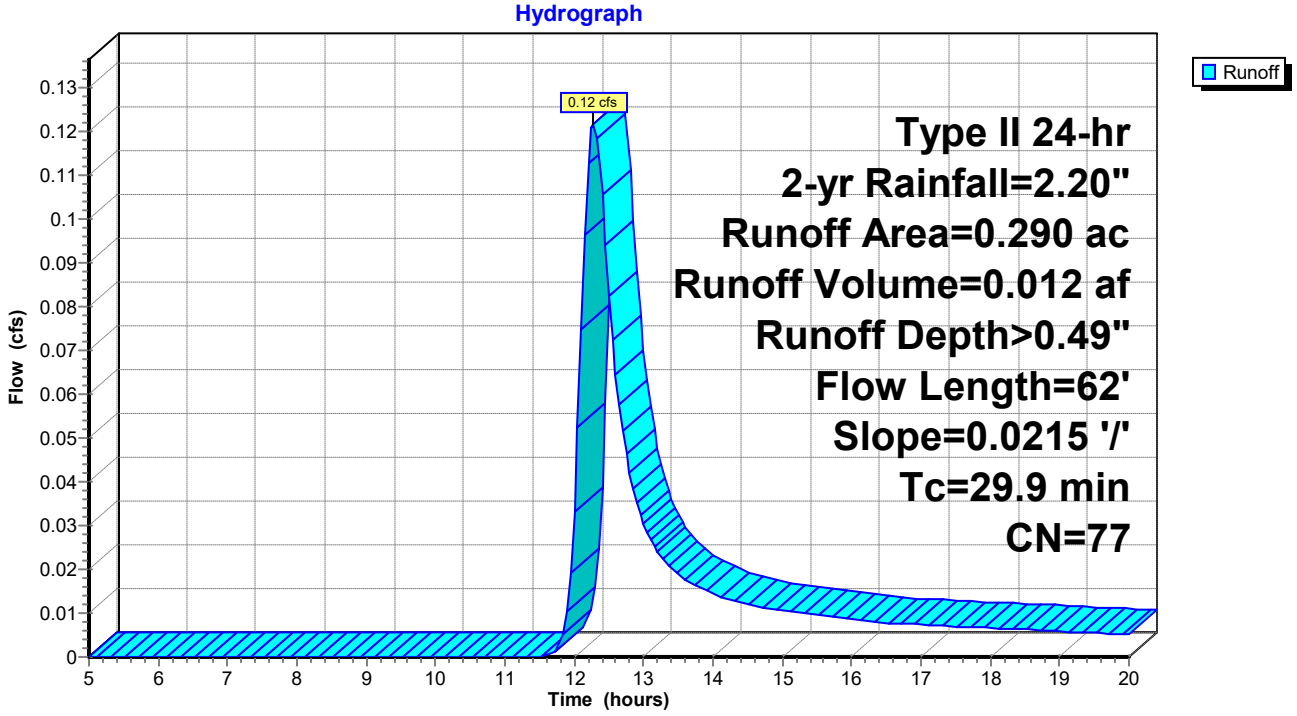
Runoff = 0.12 cfs @ 12.28 hrs, Volume= 0.012 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.9	62	0.0215	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

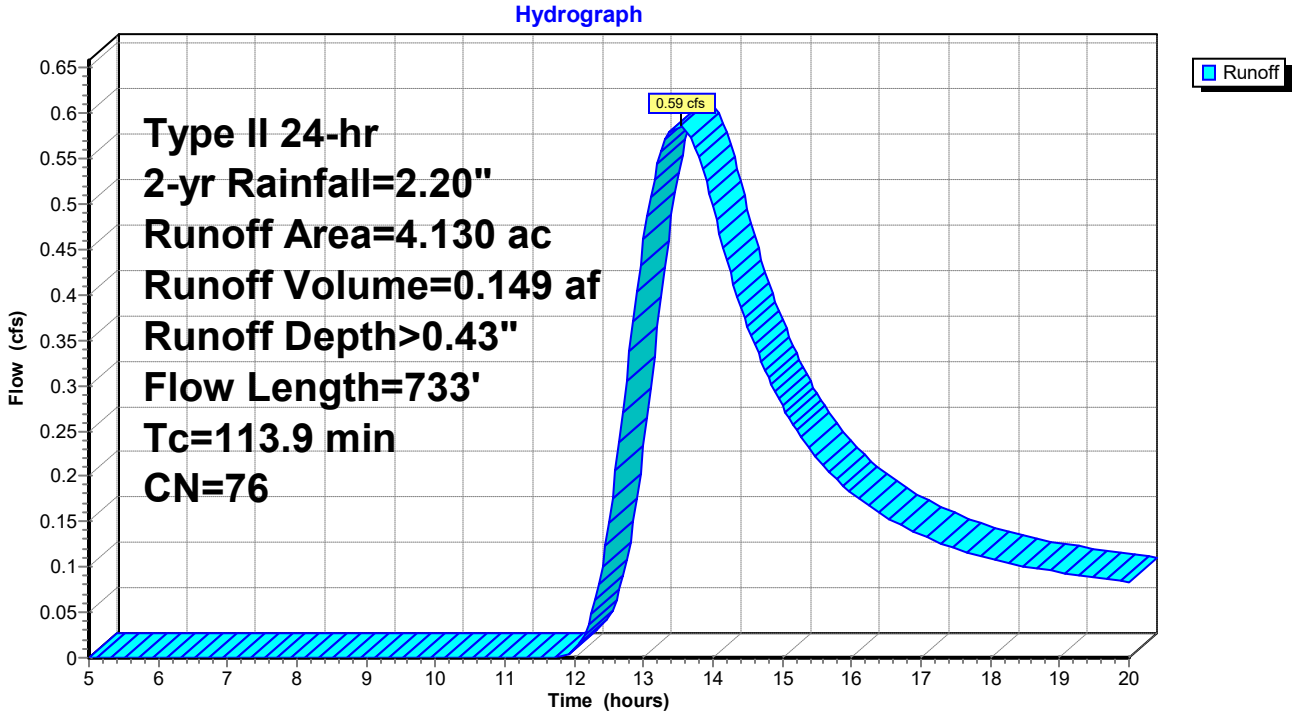
Runoff = 0.59 cfs @ 13.53 hrs, Volume= 0.149 af, Depth> 0.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	77	Woods, Good, HSG D
2.620	70	Woods, Good, HSG C
4.130	76	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.8	150	0.0074	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
113.9	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

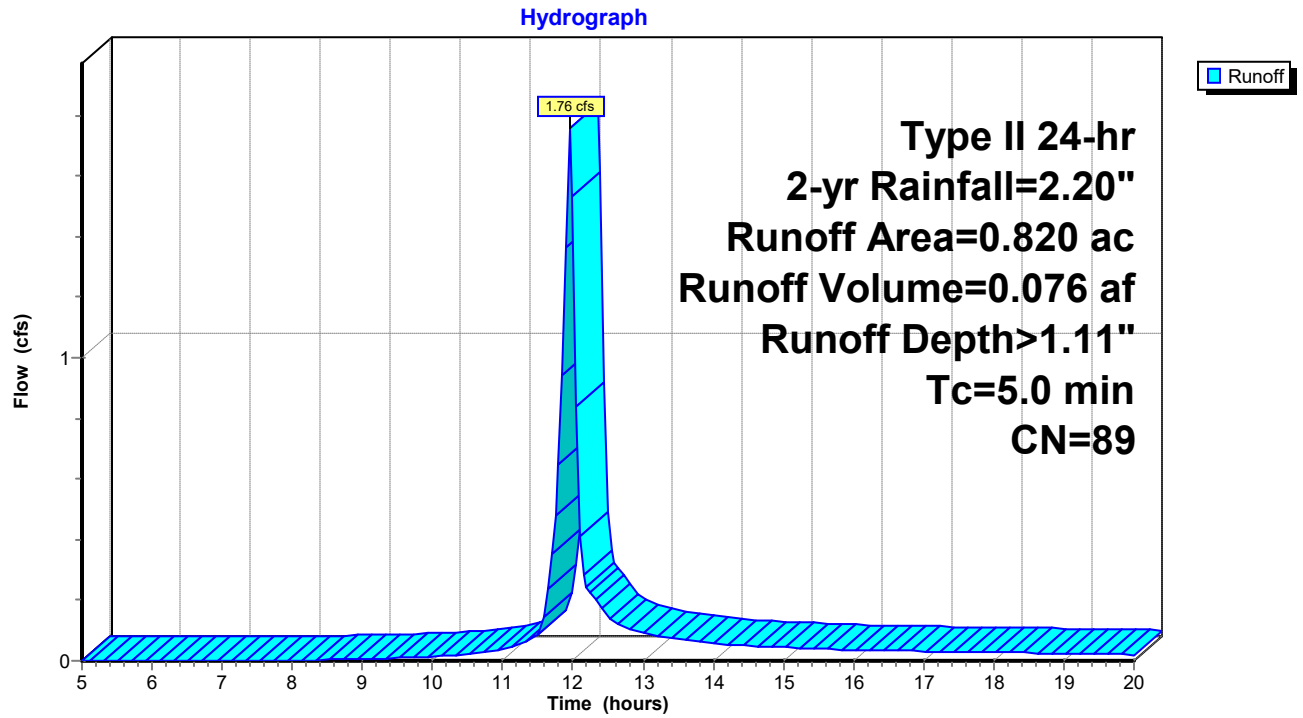
Runoff = 1.76 cfs @ 11.96 hrs, Volume= 0.076 af, Depth> 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>1.06"
Flow Length=235' Tc=46.6 min CN=77 Runoff=0.97 cfs 0.119 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>1.82"
Flow Length=916' Tc=7.6 min CN=88 Runoff=8.35 cfs 0.396 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=100' Slope=0.0700 '/' Tc=27.3 min CN=77 Runoff=0.19 cfs 0.016 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=100' Slope=0.0350 '/' Tc=36.0 min CN=77 Runoff=0.14 cfs 0.014 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>1.02"
Flow Length=494' Tc=113.0 min CN=77 Runoff=1.73 cfs 0.395 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=179' Tc=32.5 min CN=77 Runoff=0.47 cfs 0.044 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=330' Tc=41.0 min CN=77 Runoff=0.52 cfs 0.058 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>1.05"
Flow Length=417' Tc=58.0 min CN=77 Runoff=0.42 cfs 0.059 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=150' Slope=0.0740 '/' Tc=37.0 min CN=77 Runoff=0.54 cfs 0.056 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>1.02"
Flow Length=200' Tc=112.5 min CN=77 Runoff=0.11 cfs 0.025 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>1.06"
Flow Length=62' Slope=0.0215 '/' Tc=29.9 min CN=77 Runoff=0.29 cfs 0.026 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>0.96"
Flow Length=733' Tc=113.9 min CN=76 Runoff=1.42 cfs 0.331 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>1.91"
Tc=5.0 min CN=89 Runoff=2.94 cfs 0.130 af

Total Runoff Area = 16.958 ac Runoff Volume = 1.670 af Average Runoff Depth = 1.18"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

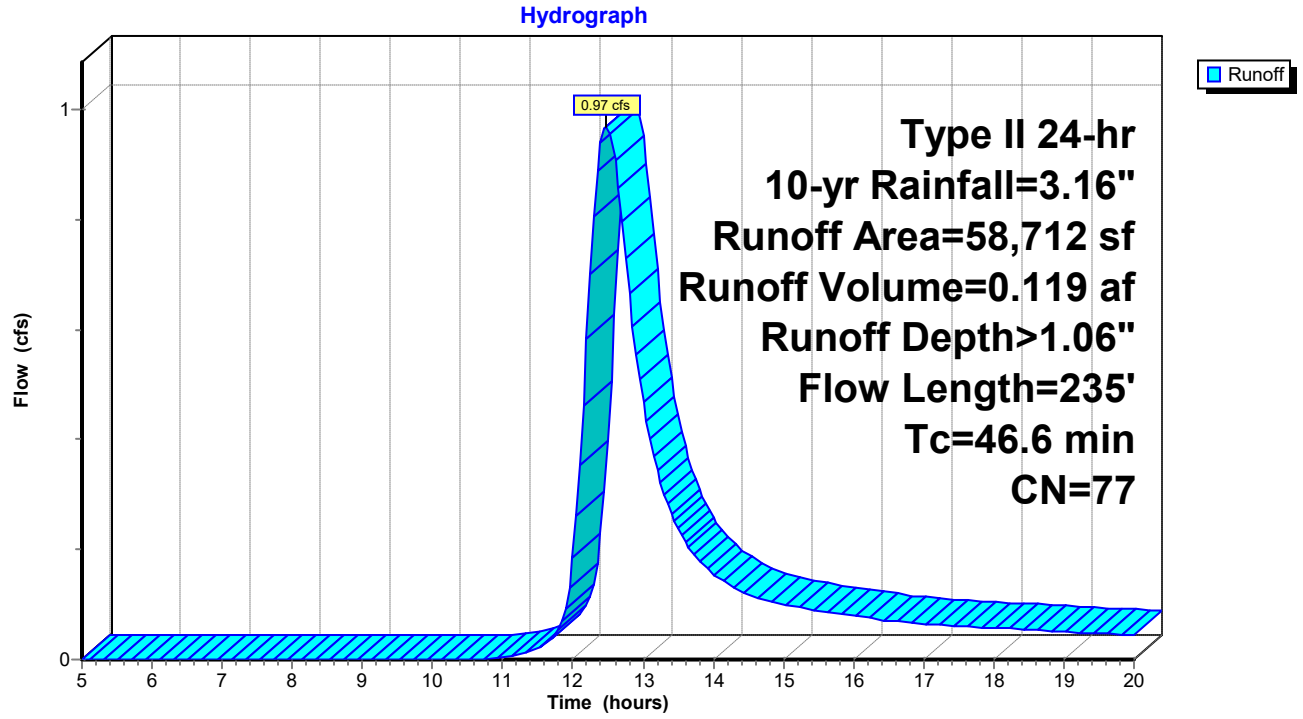
Runoff = 0.97 cfs @ 12.48 hrs, Volume= 0.119 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (sf)	CN	Description
58,712	77	Woods, Good, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.1	150	0.0450	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
46.6	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

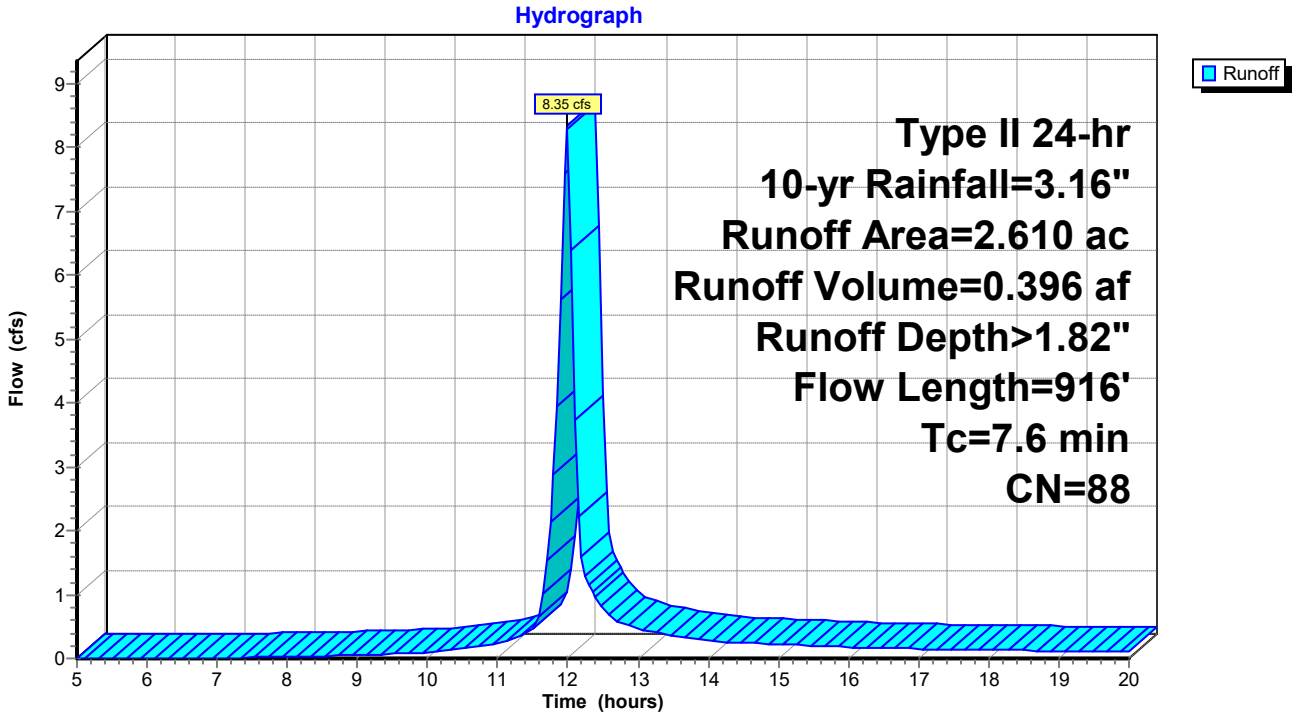
Runoff = 8.35 cfs @ 11.99 hrs, Volume= 0.396 af, Depth> 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
1.240	77	Woods, Good, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

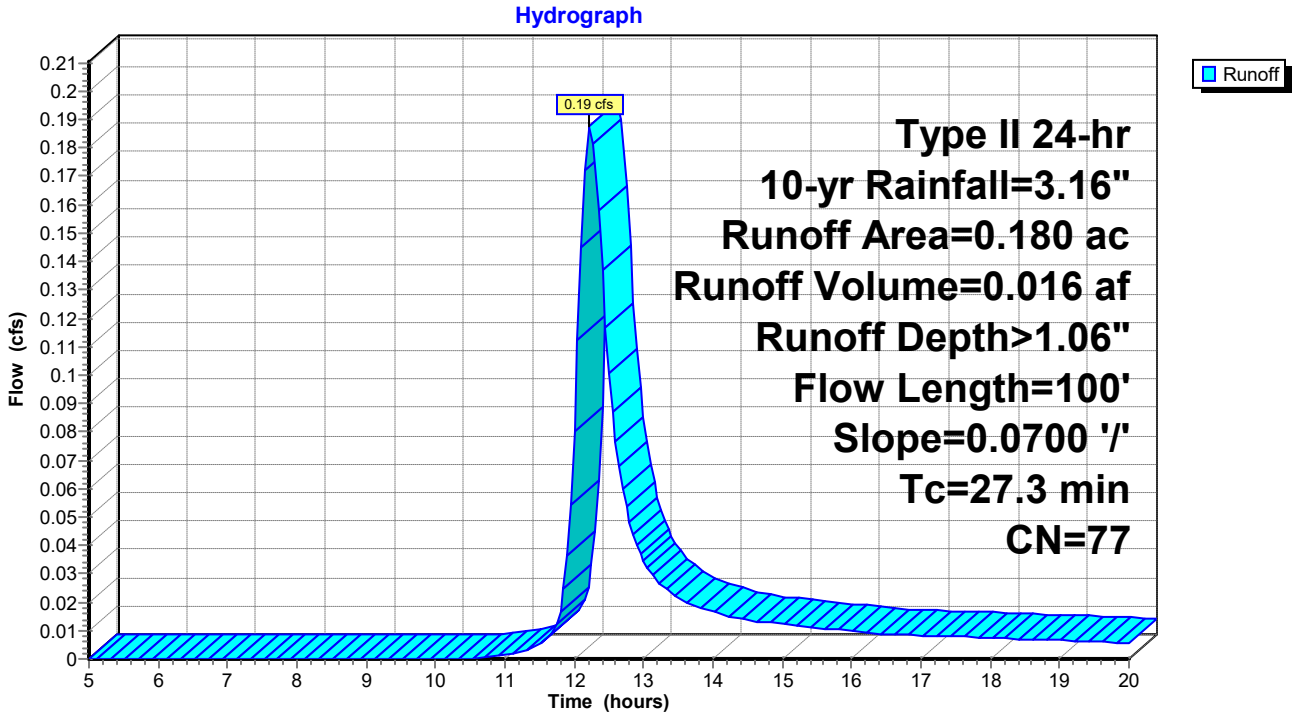
Runoff = 0.19 cfs @ 12.23 hrs, Volume= 0.016 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.180	77	Woods, Good, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.3	100	0.0700	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

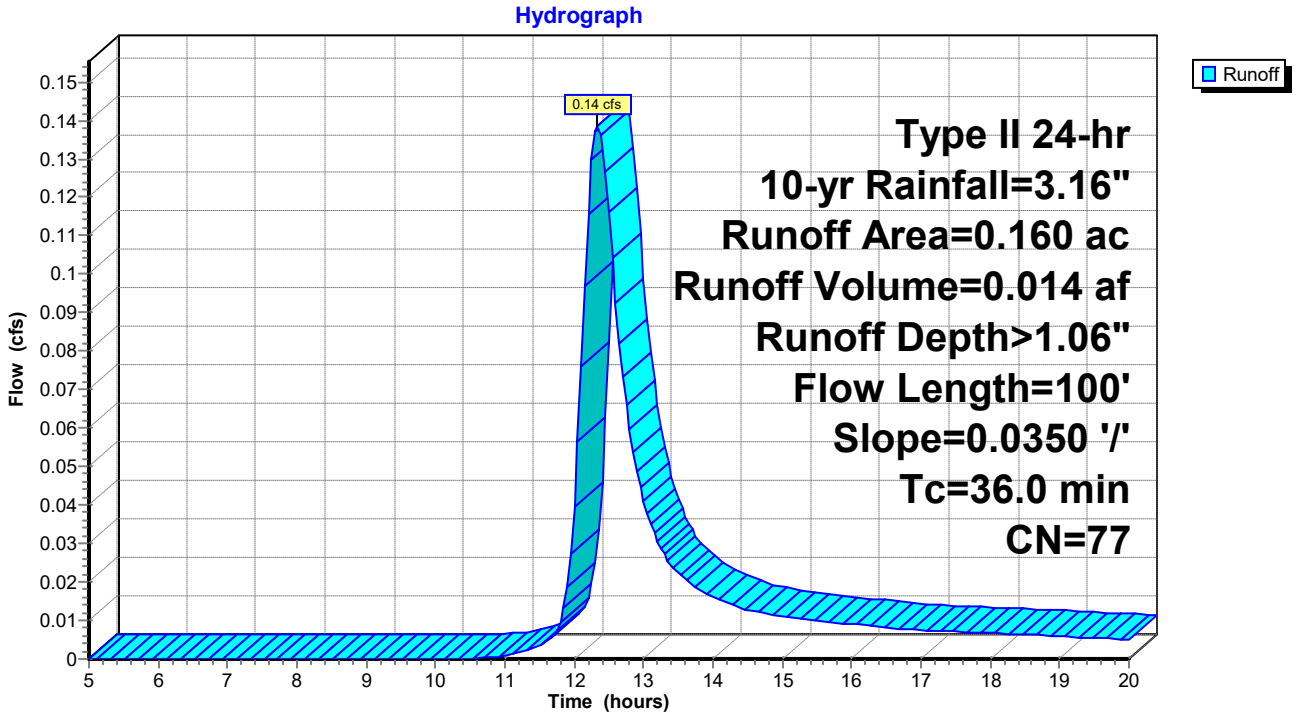
Runoff = 0.14 cfs @ 12.34 hrs, Volume= 0.014 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.160	77	Woods, Good, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.0	100	0.0350	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

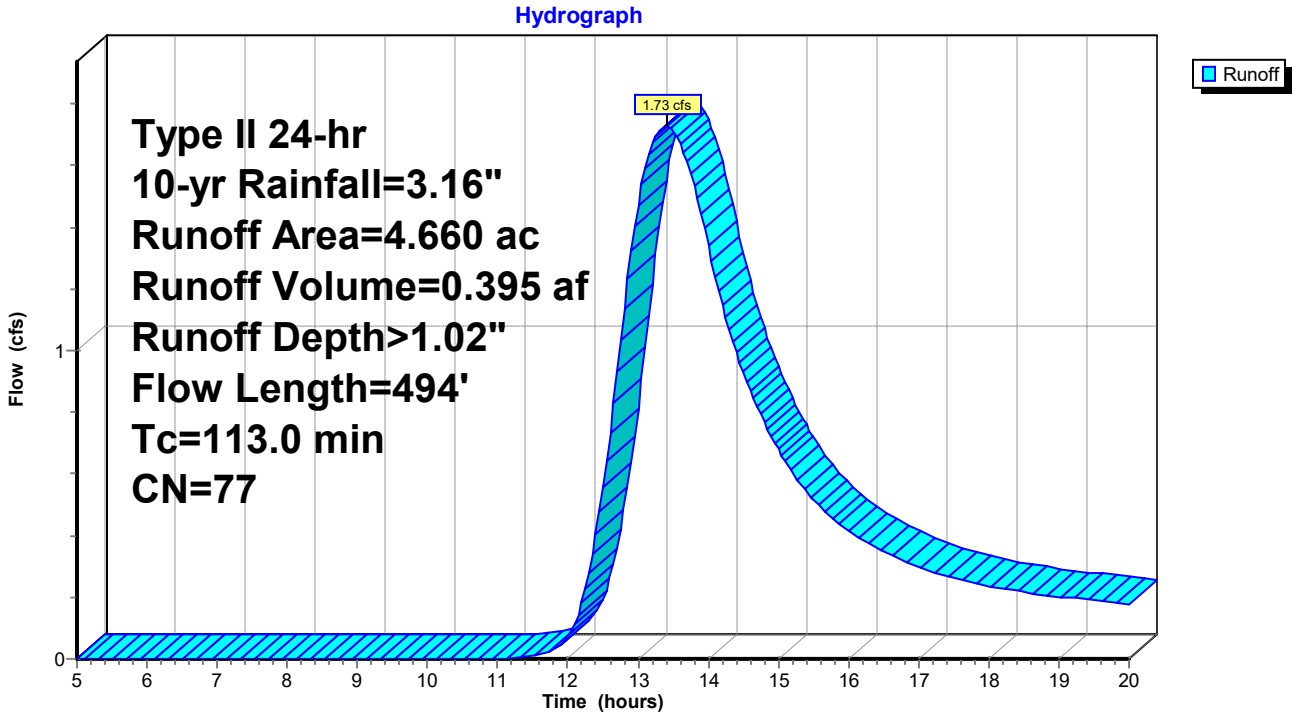
Runoff = 1.73 cfs @ 13.42 hrs, Volume= 0.395 af, Depth> 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
4.660	77	Woods, Good, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.3	150	0.0075	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
113.0	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

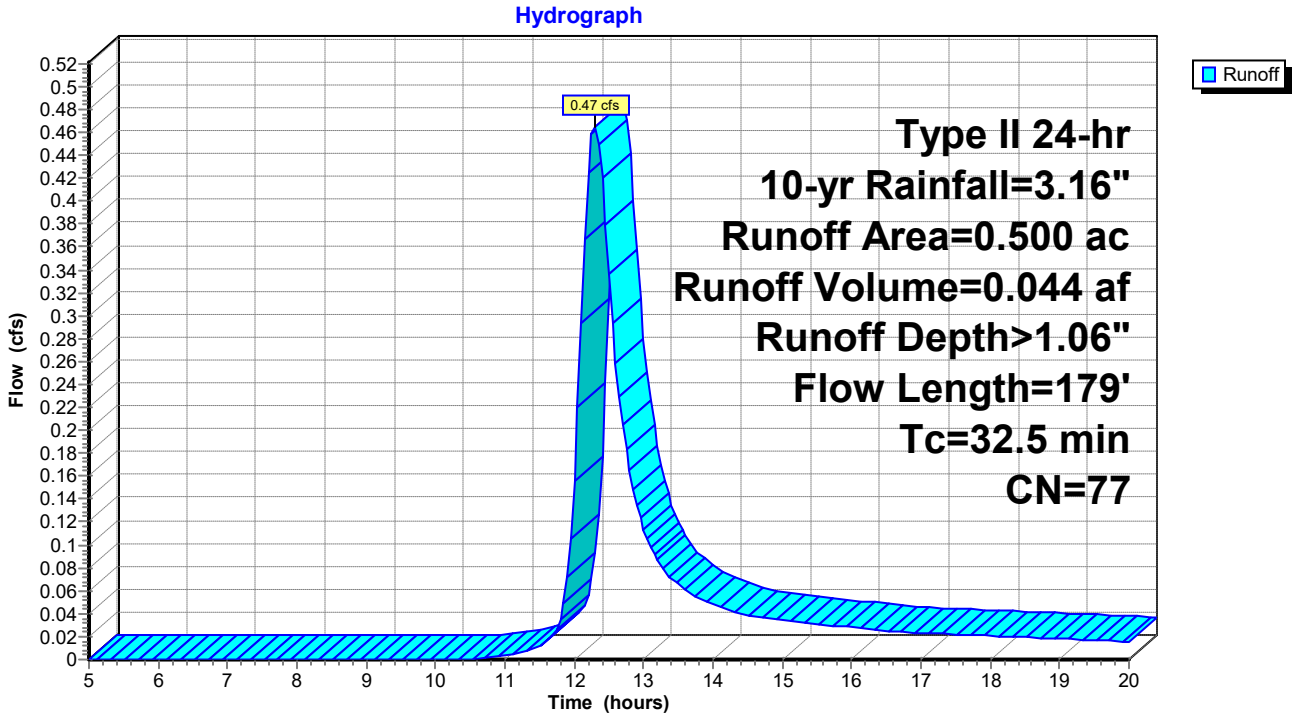
Runoff = 0.47 cfs @ 12.29 hrs, Volume= 0.044 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.500	77	Woods, Good, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

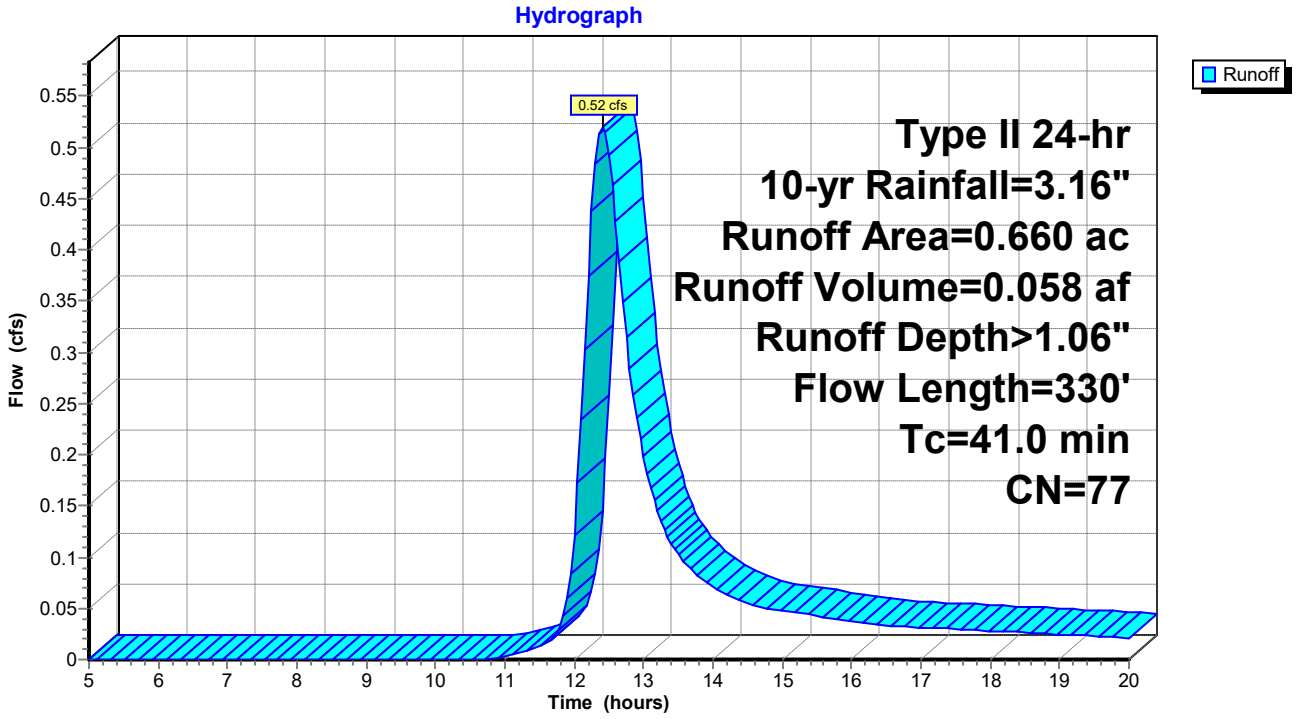
Runoff = 0.52 cfs @ 12.40 hrs, Volume= 0.058 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.660	77	Woods, Good, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.8	150	0.0750	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
41.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

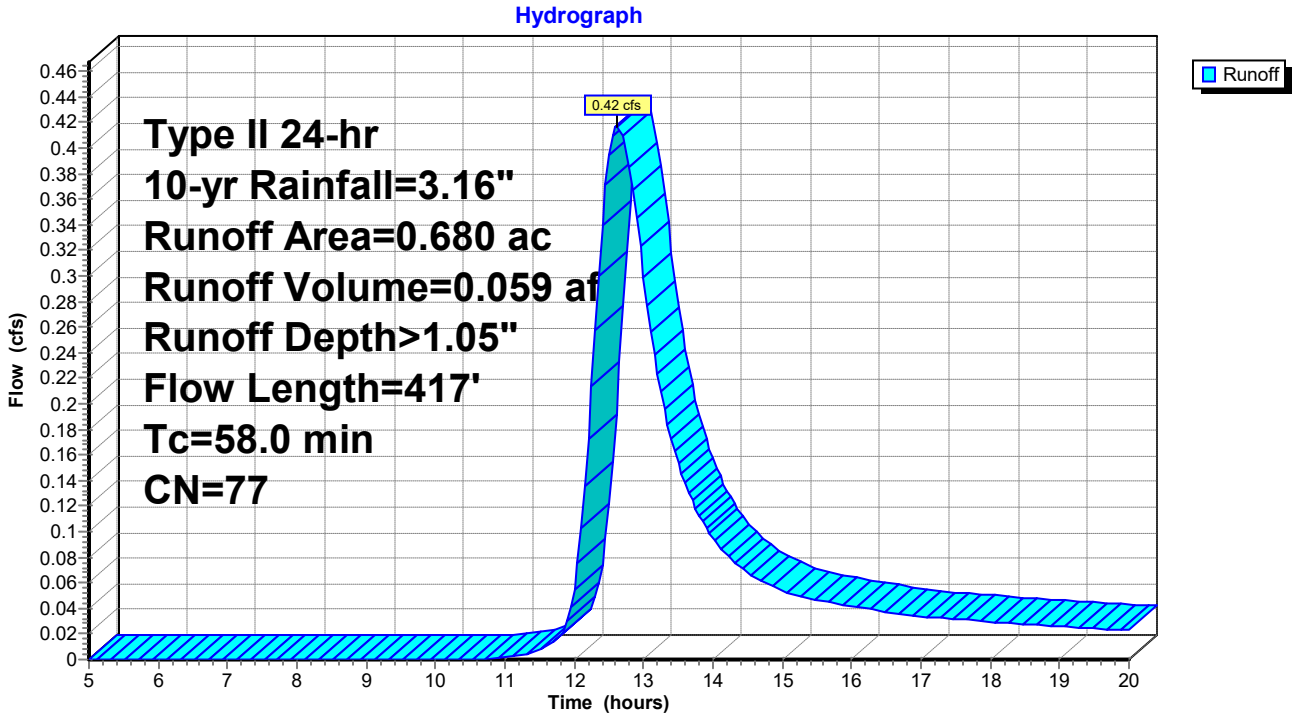
Runoff = 0.42 cfs @ 12.63 hrs, Volume= 0.059 af, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.680	77	Woods, Good, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
51.7	150	0.0320	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
58.0	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

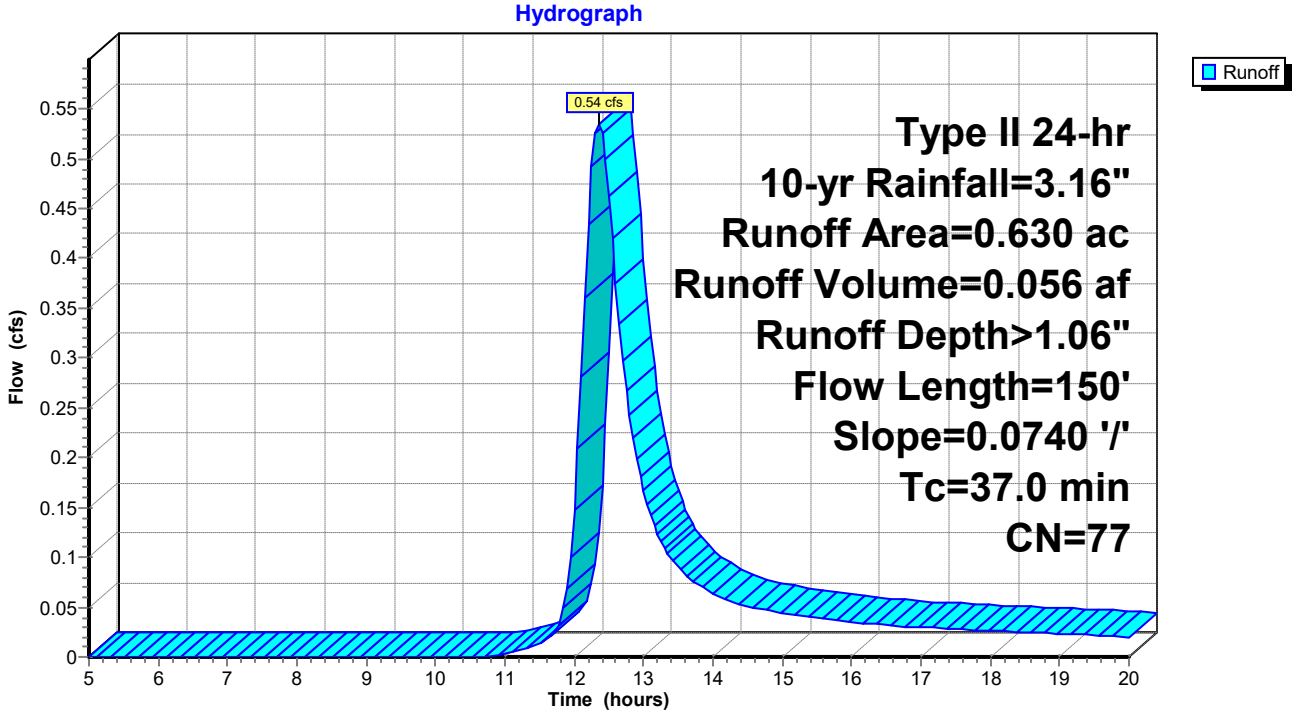
Runoff = 0.54 cfs @ 12.35 hrs, Volume= 0.056 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.630	77	Woods, Good, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.0	150	0.0740	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

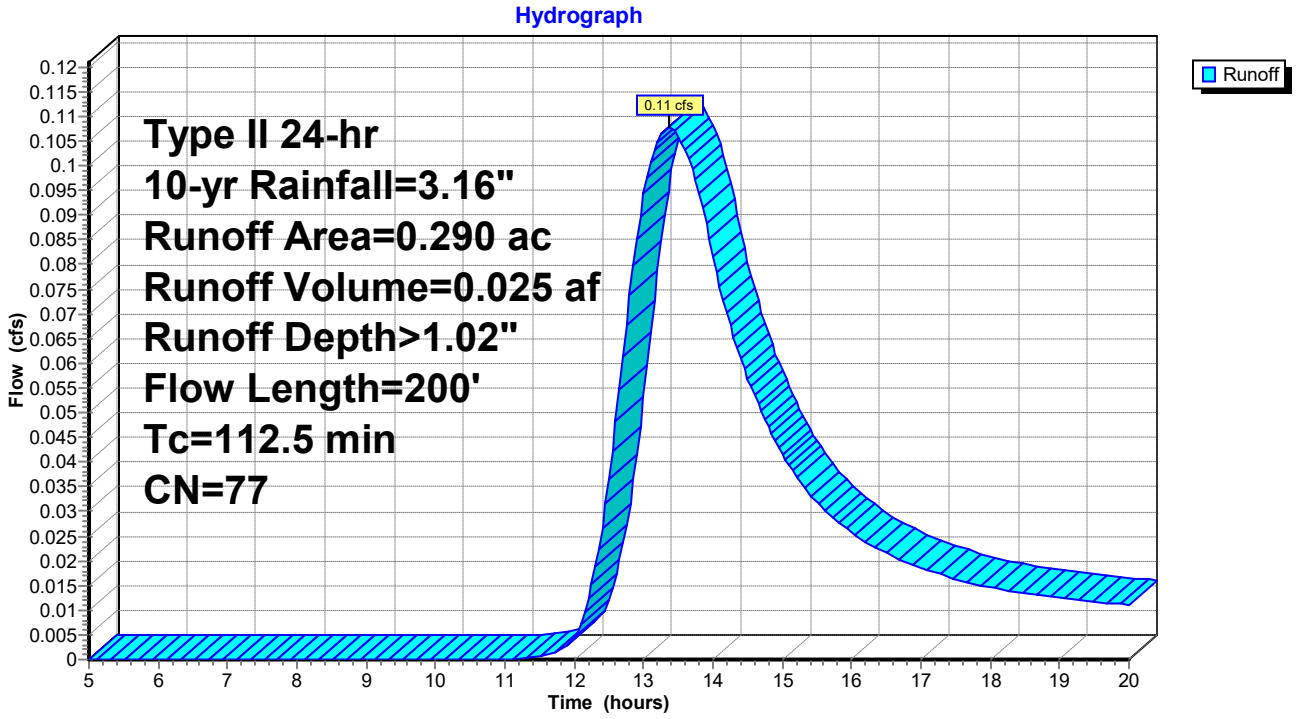
Runoff = 0.11 cfs @ 13.37 hrs, Volume= 0.025 af, Depth> 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

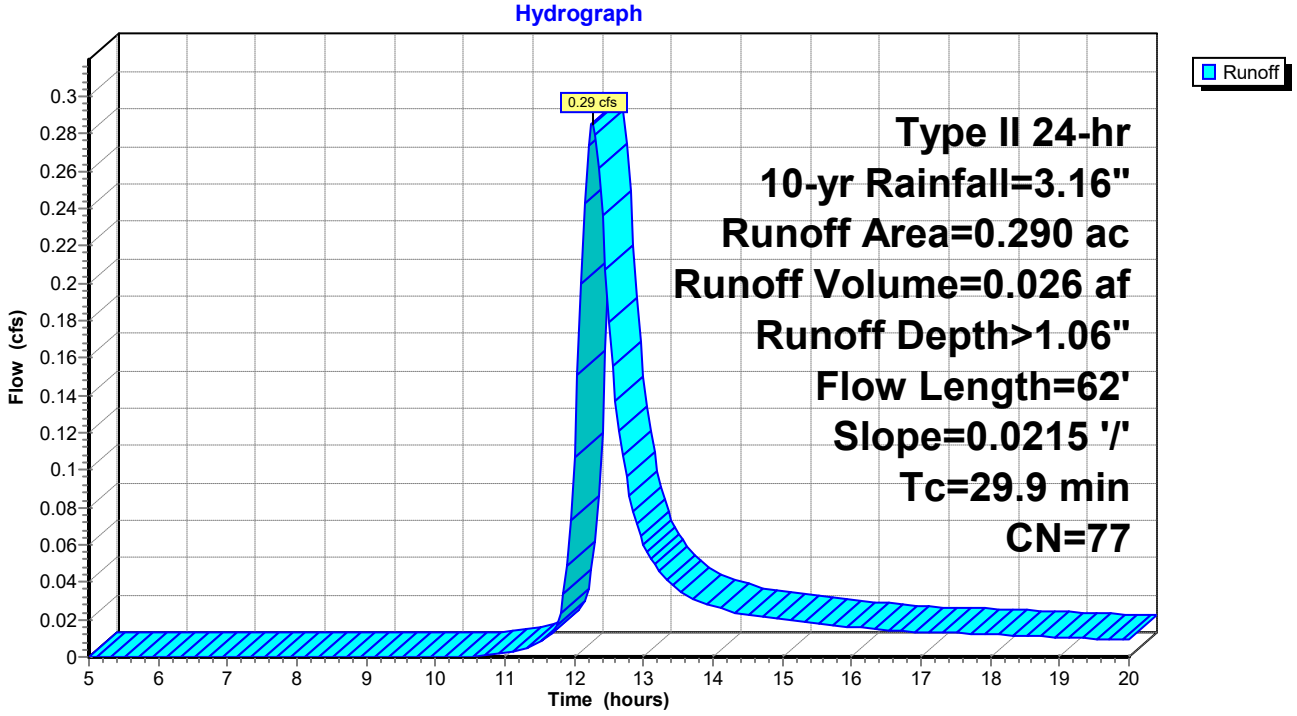
Runoff = 0.29 cfs @ 12.26 hrs, Volume= 0.026 af, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.9	62	0.0215	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

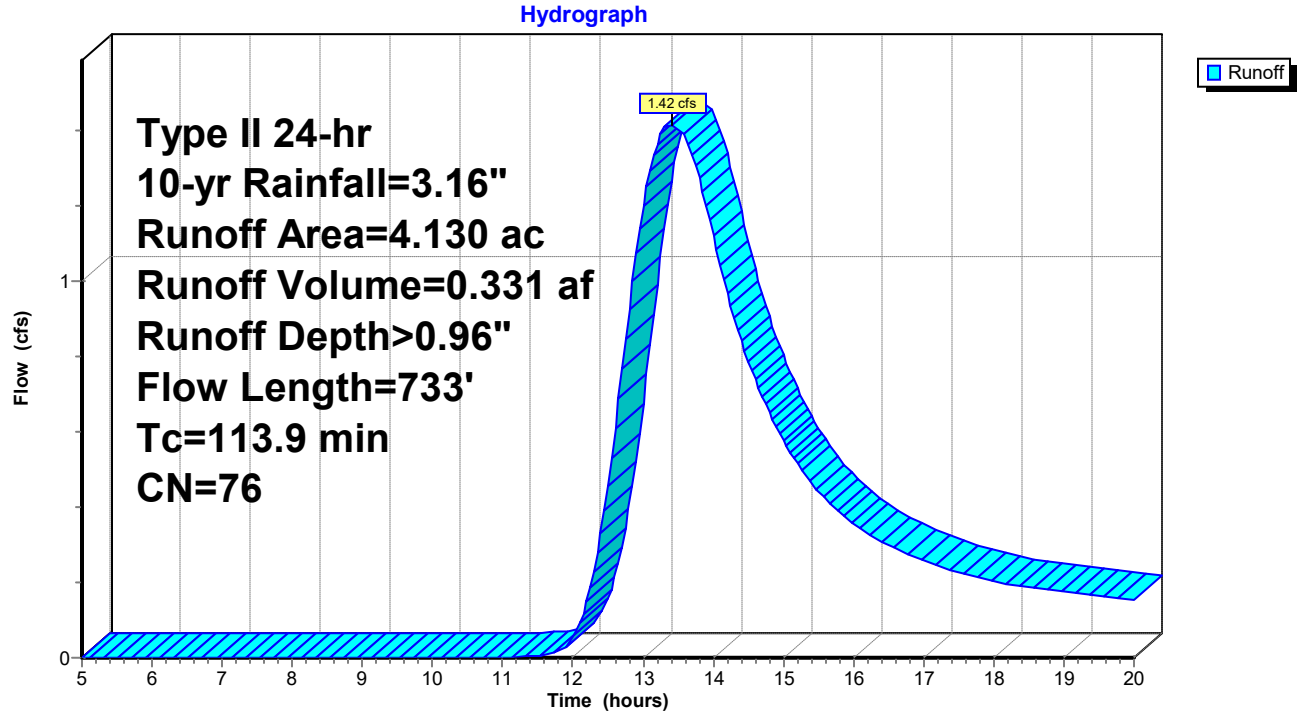
Runoff = 1.42 cfs @ 13.40 hrs, Volume= 0.331 af, Depth> 0.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	77	Woods, Good, HSG D
2.620	70	Woods, Good, HSG C
4.130	76	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.8	150	0.0074	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
113.9	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

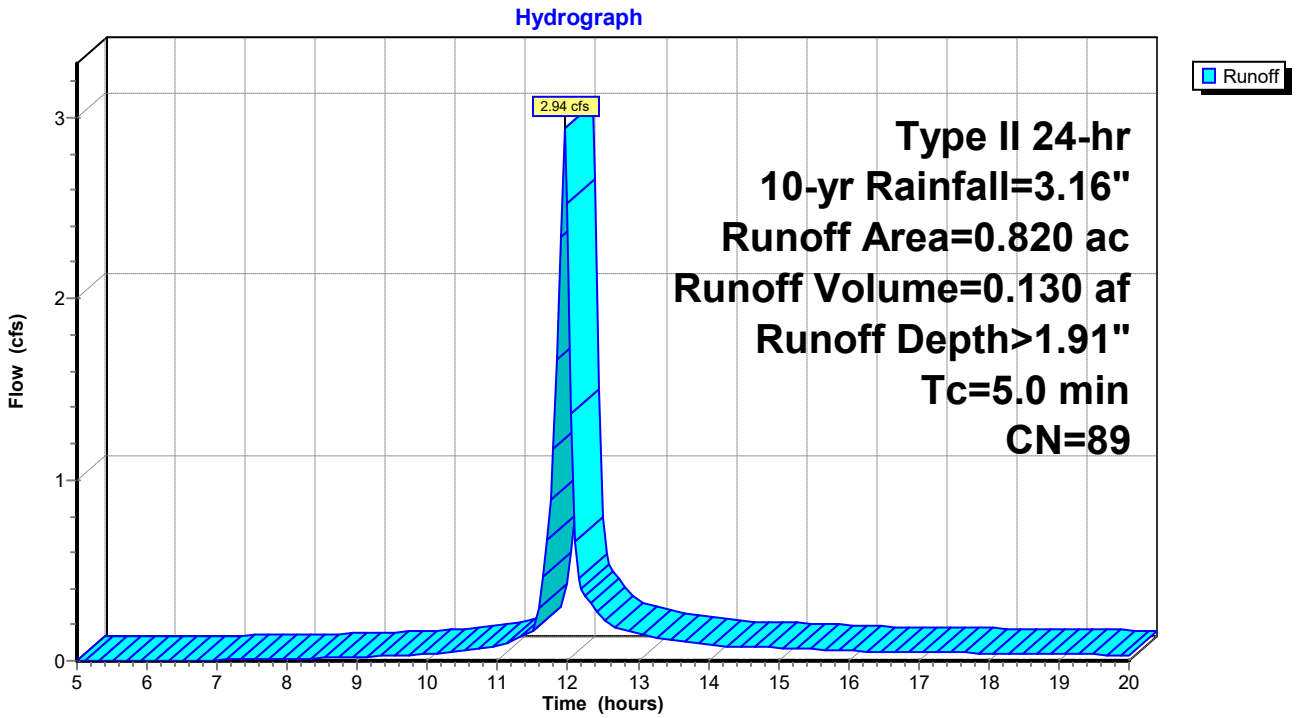
Runoff = 2.94 cfs @ 11.95 hrs, Volume= 0.130 af, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>2.15"
Flow Length=235' Tc=46.6 min CN=77 Runoff=2.03 cfs 0.242 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>3.17"
Flow Length=916' Tc=7.6 min CN=88 Runoff=14.03 cfs 0.689 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>2.17"
Flow Length=100' Slope=0.0700 '/' Tc=27.3 min CN=77 Runoff=0.39 cfs 0.033 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>2.16"
Flow Length=100' Slope=0.0350 '/' Tc=36.0 min CN=77 Runoff=0.29 cfs 0.029 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>2.09"
Flow Length=494' Tc=113.0 min CN=77 Runoff=3.63 cfs 0.811 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>2.16"
Flow Length=179' Tc=32.5 min CN=77 Runoff=0.97 cfs 0.090 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>2.16"
Flow Length=330' Tc=41.0 min CN=77 Runoff=1.09 cfs 0.119 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>2.14"
Flow Length=417' Tc=58.0 min CN=77 Runoff=0.88 cfs 0.121 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>2.16"
Flow Length=150' Slope=0.0740 '/' Tc=37.0 min CN=77 Runoff=1.12 cfs 0.113 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>2.09"
Flow Length=200' Tc=112.5 min CN=77 Runoff=0.23 cfs 0.050 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>2.17"
Flow Length=62' Slope=0.0215 '/' Tc=29.9 min CN=77 Runoff=0.59 cfs 0.052 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>2.01"
Flow Length=733' Tc=113.9 min CN=76 Runoff=3.06 cfs 0.691 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>3.27"
Tc=5.0 min CN=89 Runoff=4.87 cfs 0.223 af

Total Runoff Area = 16.958 ac Runoff Volume = 3.263 af Average Runoff Depth = 2.31"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

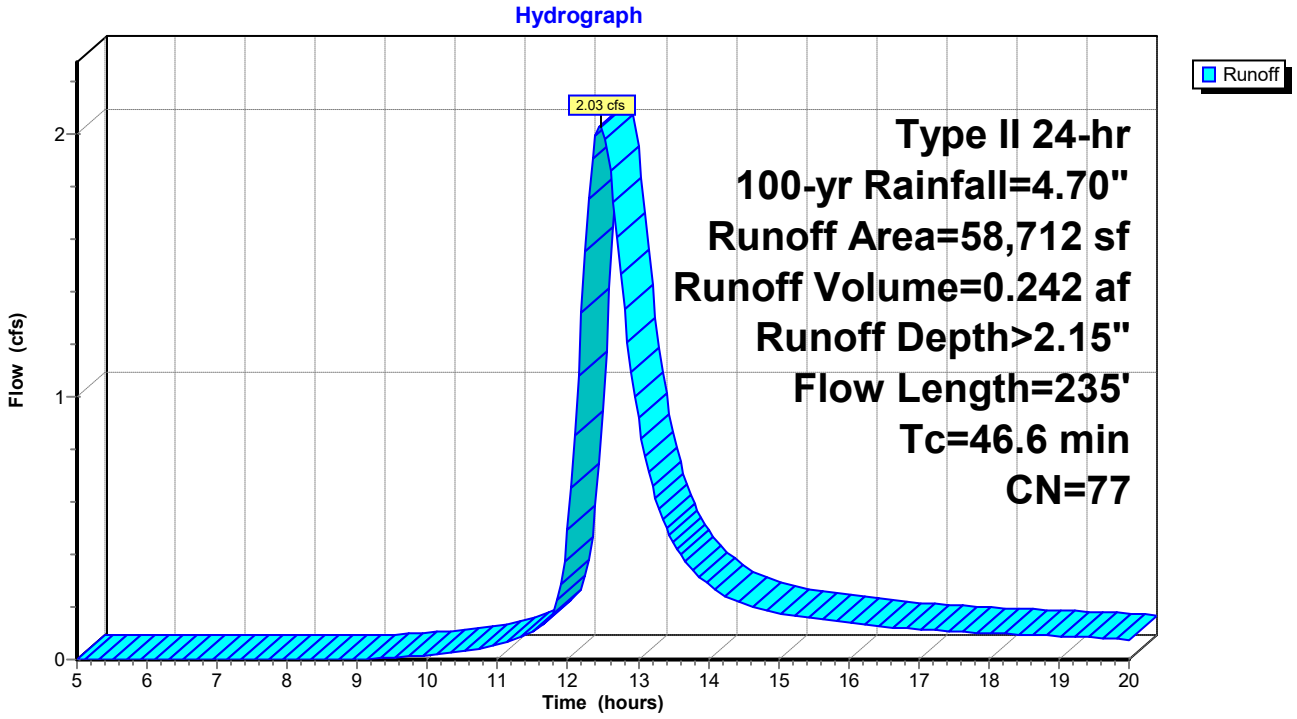
Runoff = 2.03 cfs @ 12.46 hrs, Volume= 0.242 af, Depth> 2.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (sf)	CN	Description
58,712	77	Woods, Good, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.1	150	0.0450	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
46.6	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

[47] Hint: Peak is 113% of capacity of segment #5

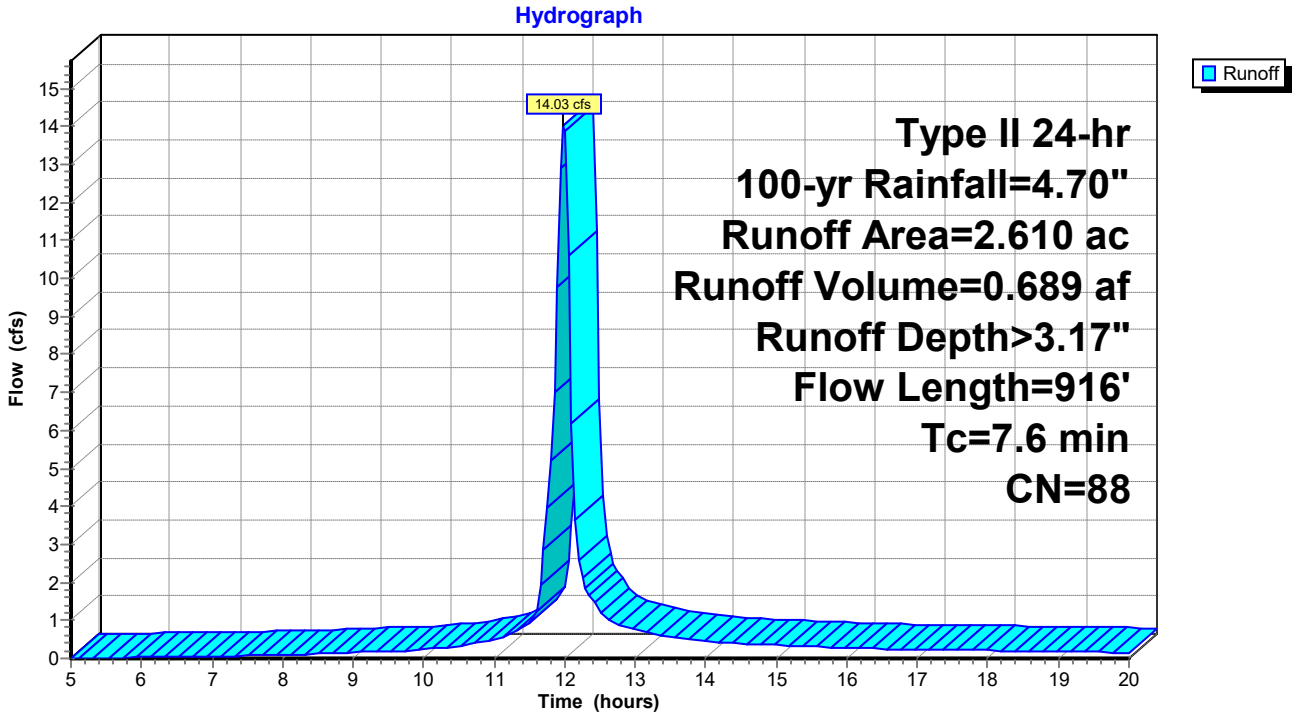
Runoff = 14.03 cfs @ 11.99 hrs, Volume= 0.689 af, Depth> 3.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
1.240	77	Woods, Good, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

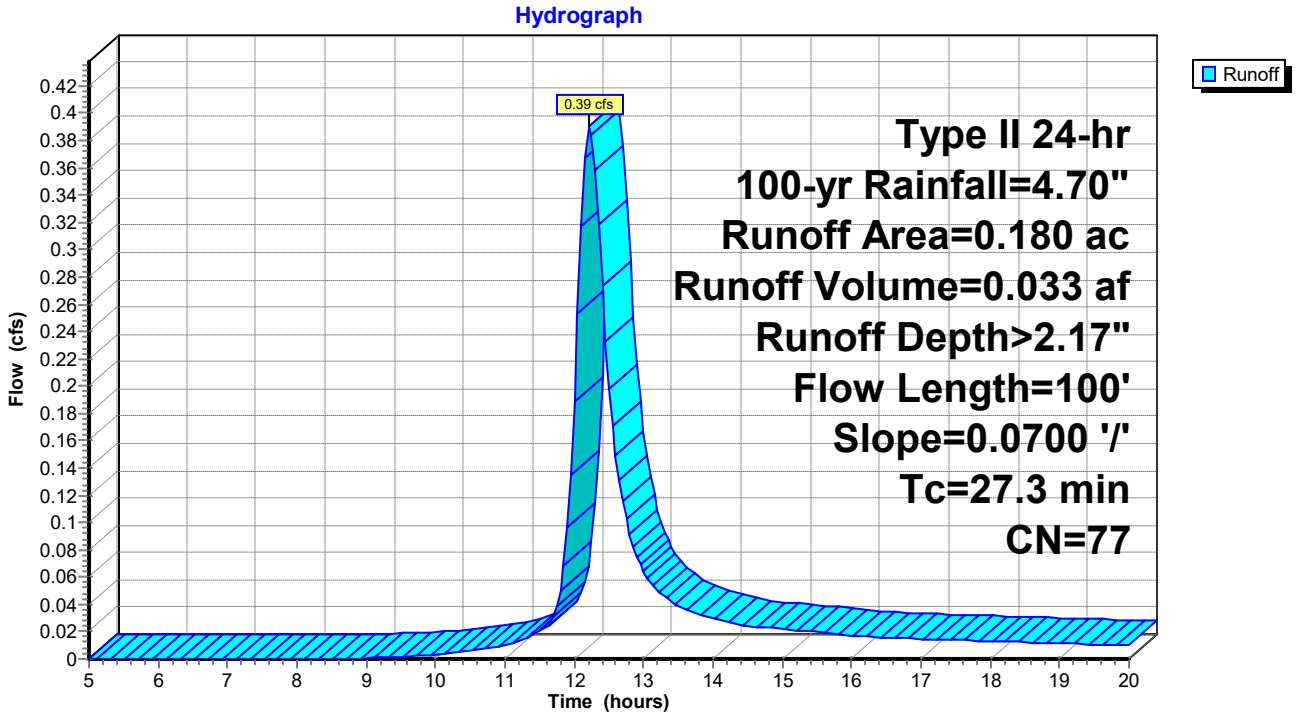
Runoff = 0.39 cfs @ 12.22 hrs, Volume= 0.033 af, Depth> 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.180	77	Woods, Good, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.3	100	0.0700	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

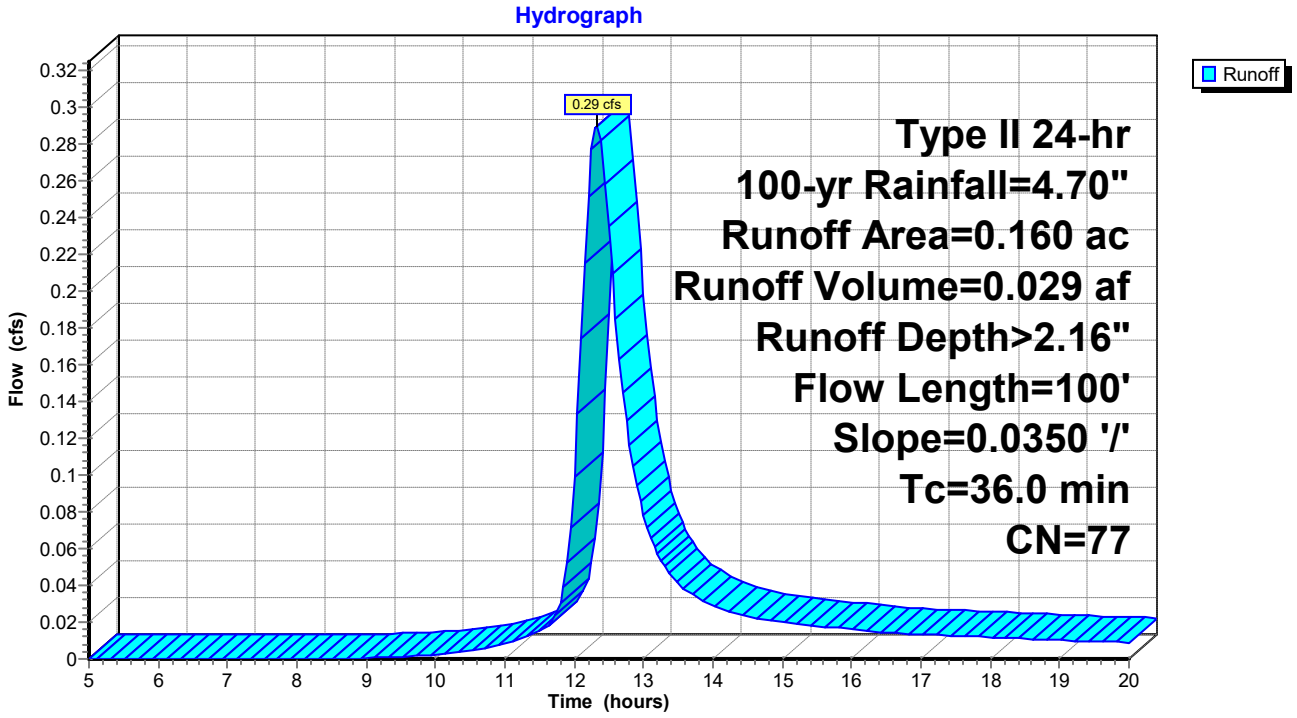
Runoff = 0.29 cfs @ 12.32 hrs, Volume= 0.029 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.160	77	Woods, Good, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.0	100	0.0350	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

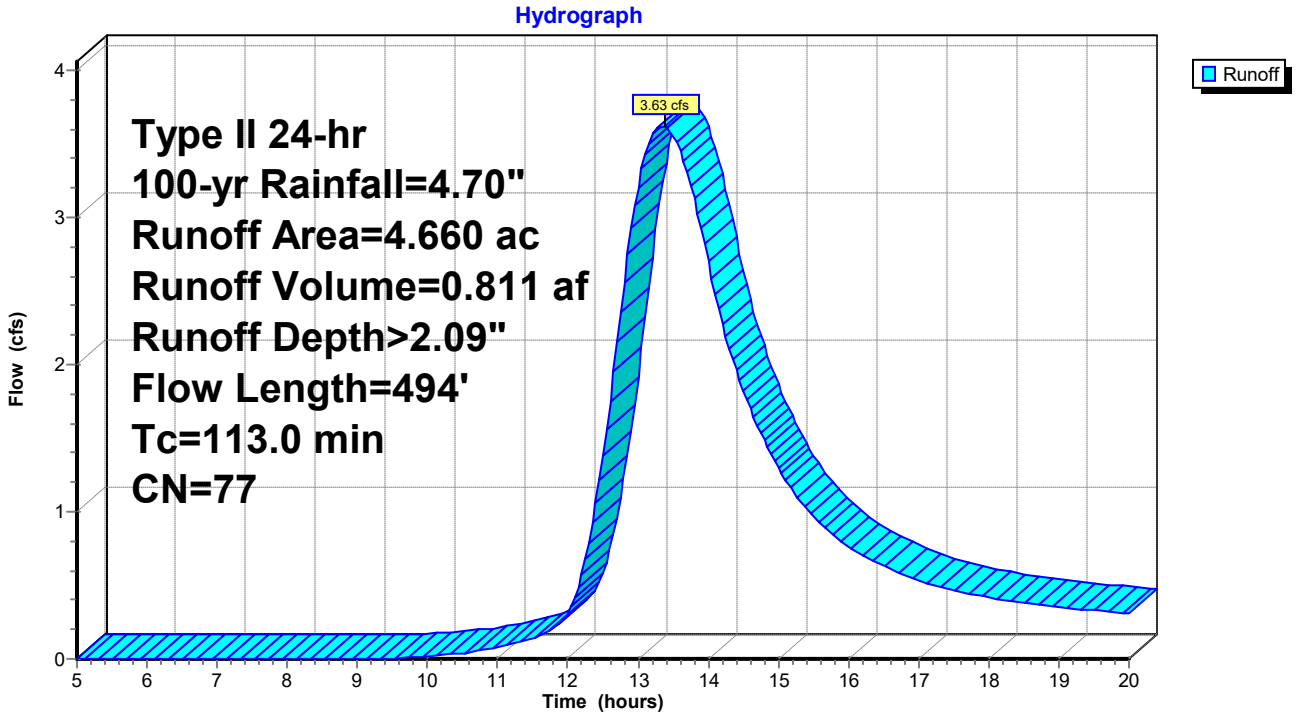
Runoff = 3.63 cfs @ 13.38 hrs, Volume= 0.811 af, Depth> 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
4.660	77	Woods, Good, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.3	150	0.0075	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
113.0	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

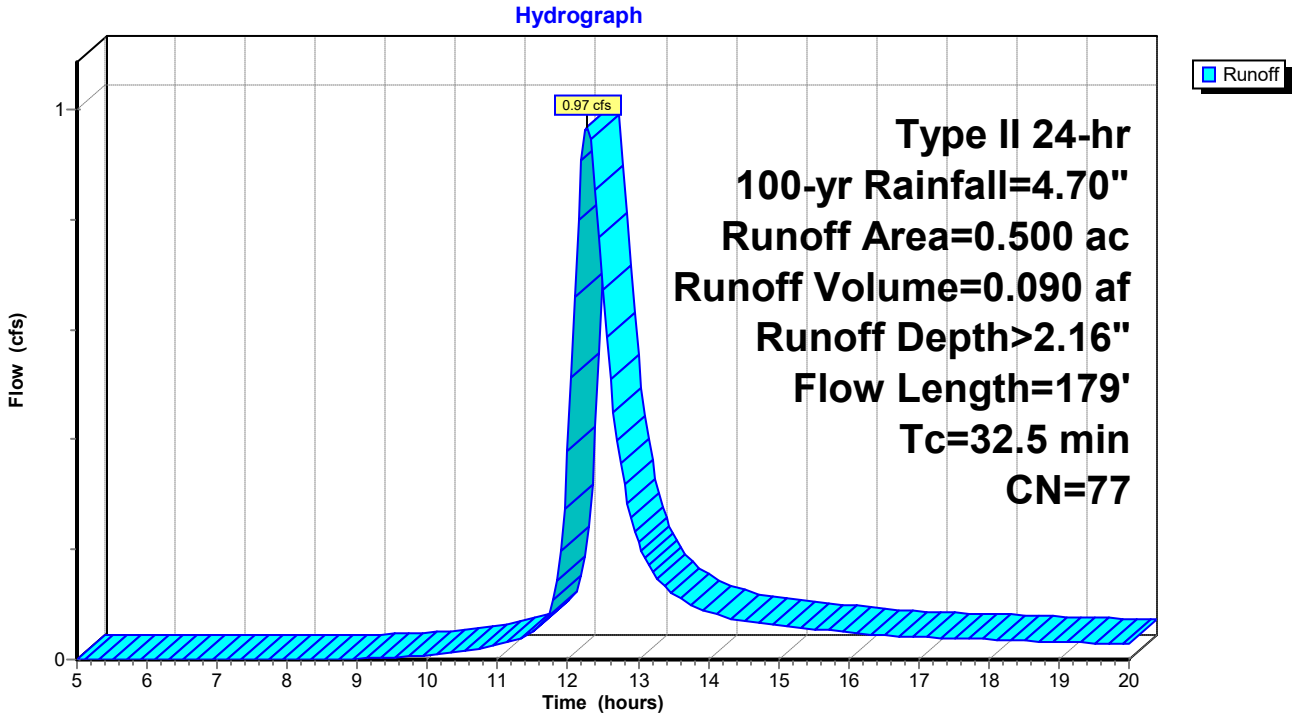
Runoff = 0.97 cfs @ 12.28 hrs, Volume= 0.090 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.500	77	Woods, Good, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

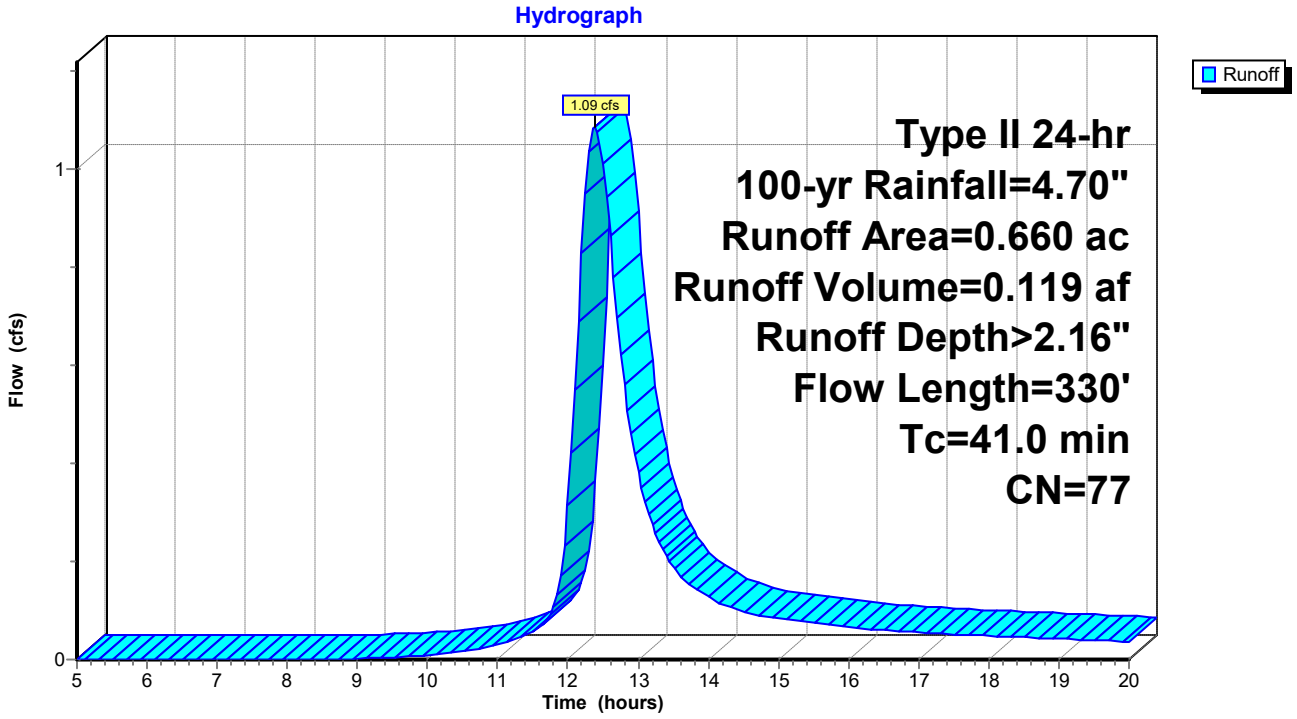
Runoff = 1.09 cfs @ 12.38 hrs, Volume= 0.119 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.660	77	Woods, Good, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.8	150	0.0750	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
41.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

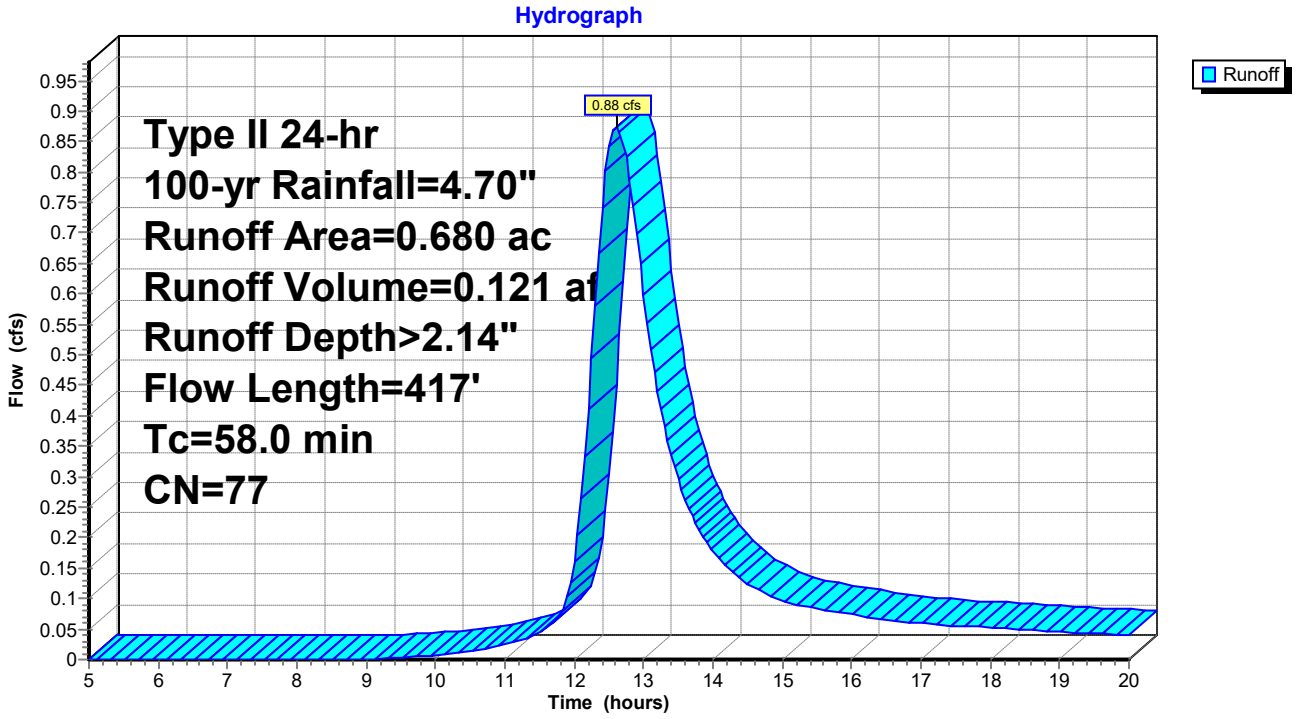
Runoff = 0.88 cfs @ 12.60 hrs, Volume= 0.121 af, Depth> 2.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.680	77	Woods, Good, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
51.7	150	0.0320	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
58.0	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

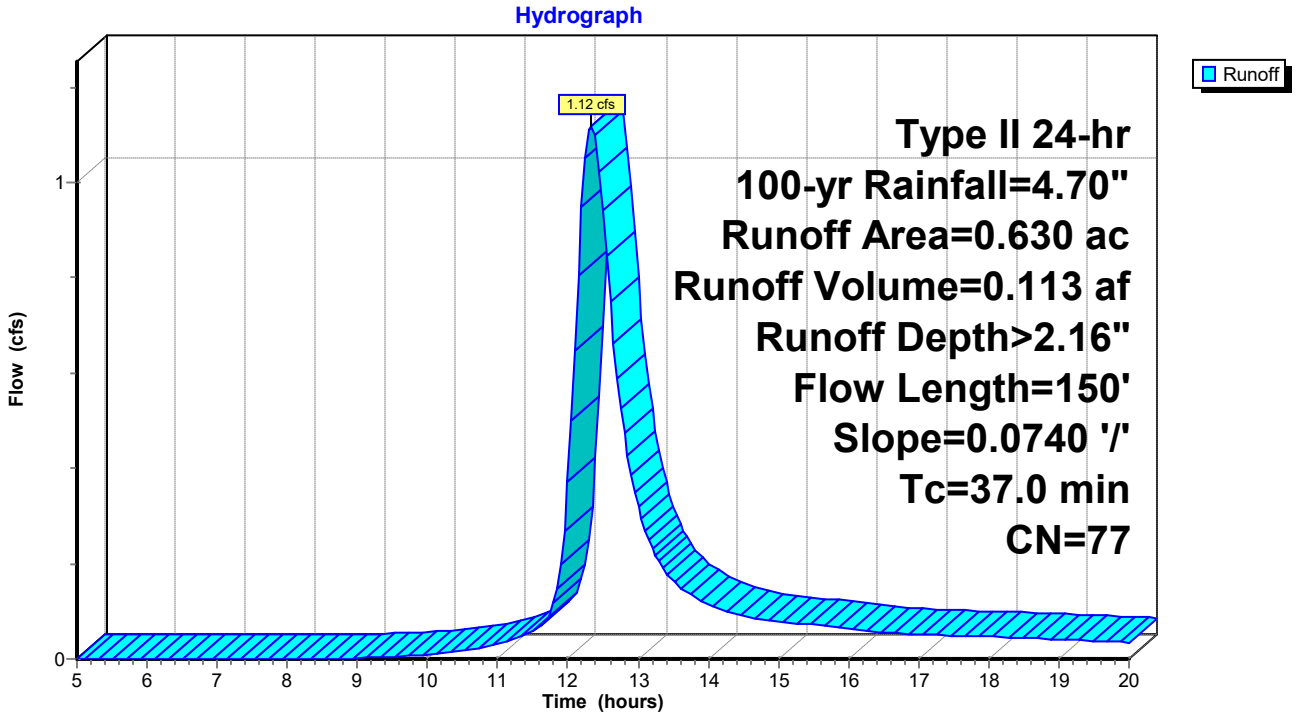
Runoff = 1.12 cfs @ 12.33 hrs, Volume= 0.113 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.630	77	Woods, Good, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.0	150	0.0740	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

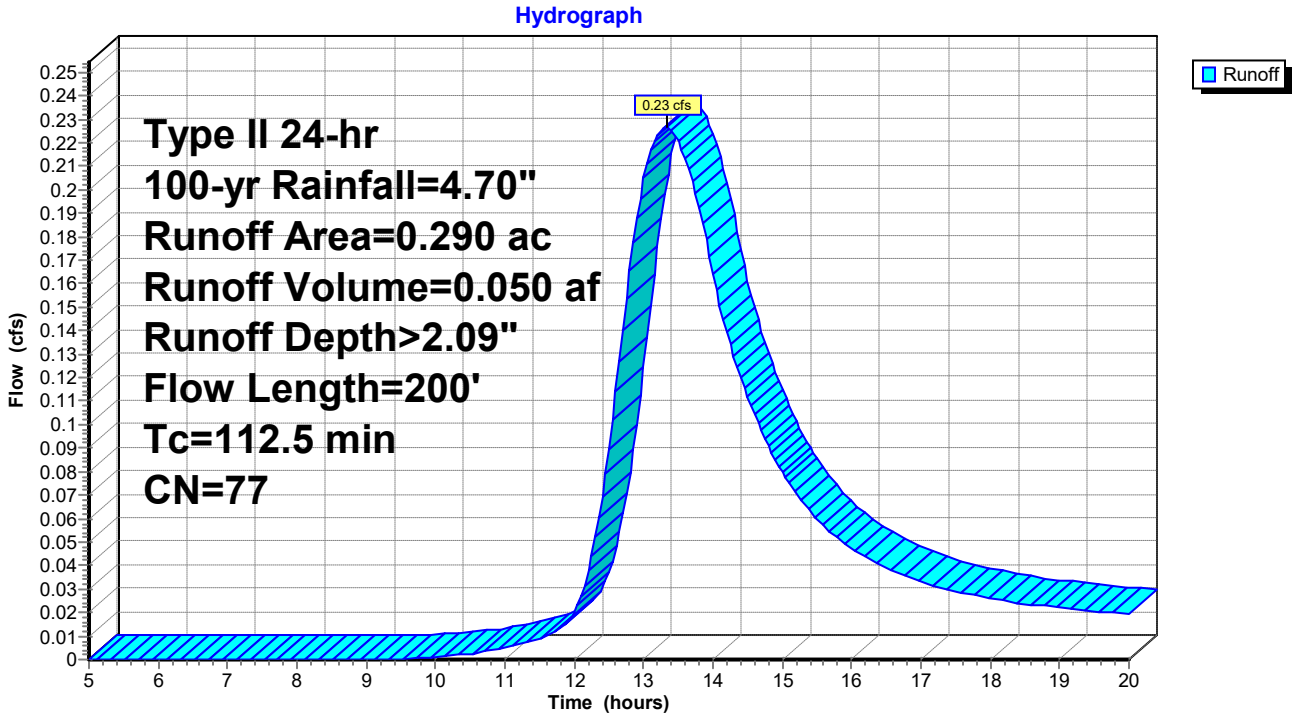
Runoff = 0.23 cfs @ 13.35 hrs, Volume= 0.050 af, Depth> 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

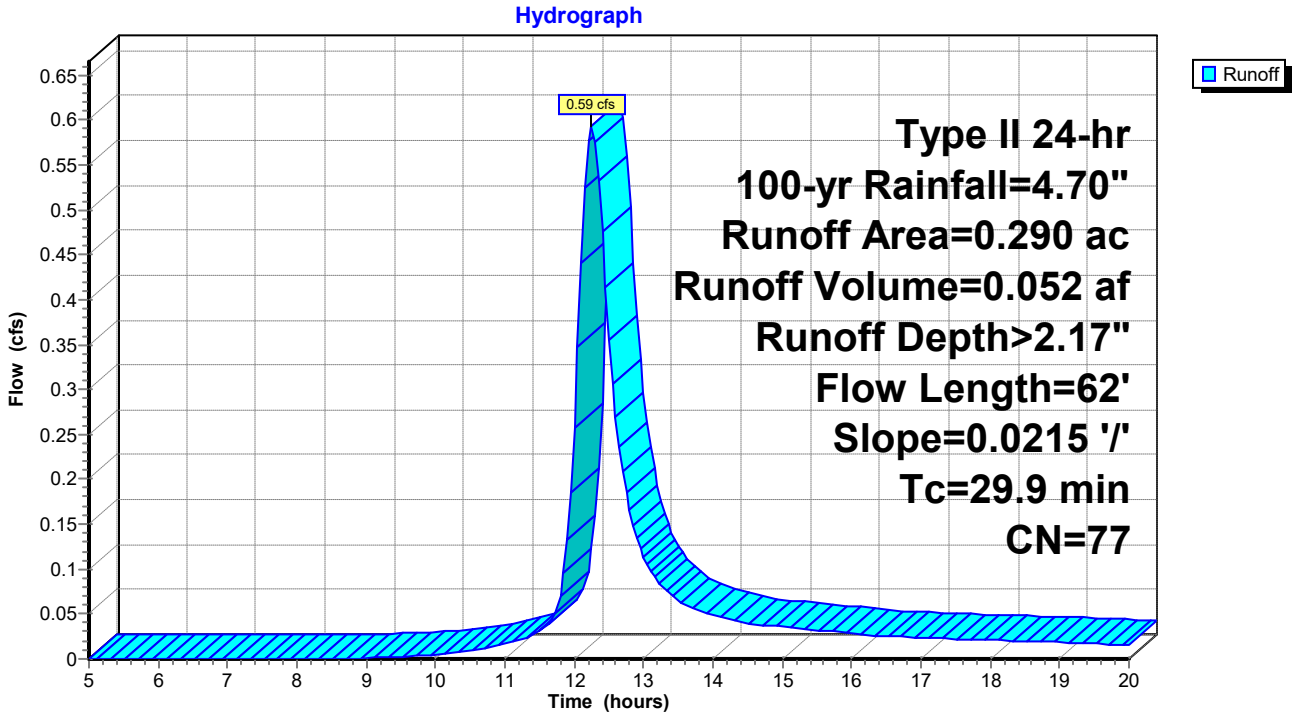
Runoff = 0.59 cfs @ 12.25 hrs, Volume= 0.052 af, Depth> 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.290	77	Woods, Good, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.9	62	0.0215	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

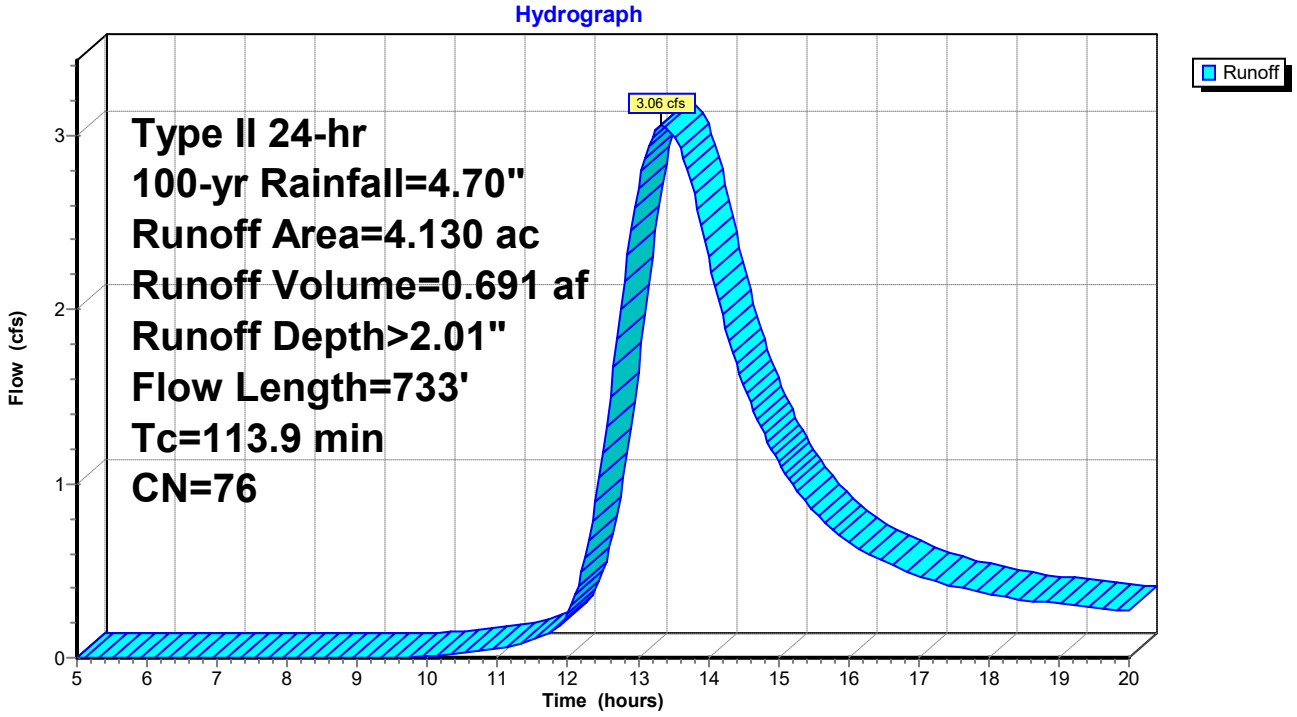
Runoff = 3.06 cfs @ 13.32 hrs, Volume= 0.691 af, Depth> 2.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	77	Woods, Good, HSG D
2.620	70	Woods, Good, HSG C
4.130	76	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
92.8	150	0.0074	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
113.9	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

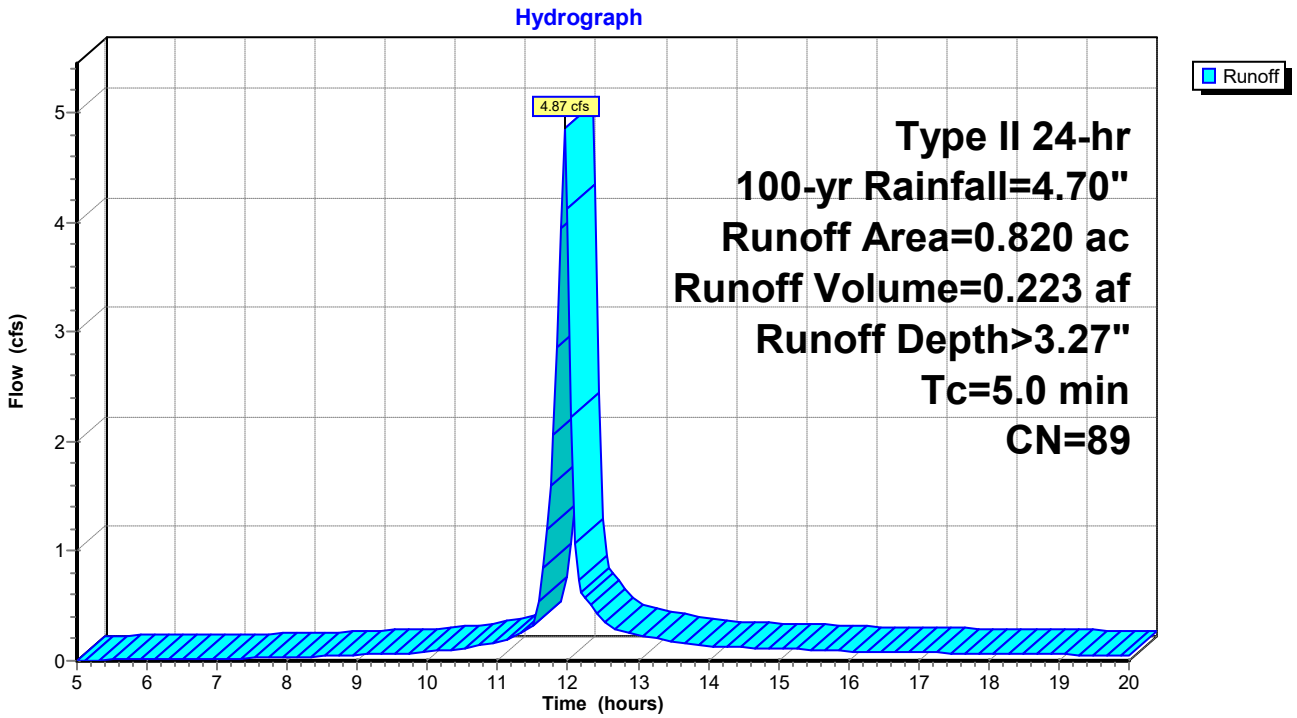
Runoff = 4.87 cfs @ 11.95 hrs, Volume= 0.223 af, Depth> 3.27"

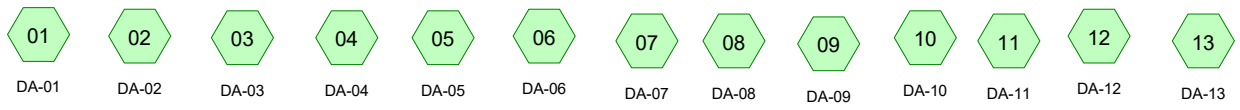
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13





Subcat Reach Pond Link

Routing Diagram for Post-Dev_01
Prepared by Haley & Aldrich, Inc, Printed 10/15/2025
HydroCAD® 10.20-6a s/n 03902 © 2024 HydroCAD Software Solutions LLC

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	Type II 24-hr		Default	24.00	1	1.87	2
2	2-yr	Type II 24-hr		Default	24.00	1	2.20	2
3	10-yr	Type II 24-hr		Default	24.00	1	3.16	2
4	100-yr	Type II 24-hr		Default	24.00	1	4.70	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.620	71	Meadow, non-grazed, HSG C (12)
11.398	78	Meadow, non-grazed, HSG D (01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12)
2.540	98	Paved parking, HSG D (02, 12, 13)
0.400	79	Woods/grass comb., Good, HSG D (13)
16.958	**80**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
2.620	HSG C	12
14.338	HSG D	01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12, 13
0.000	Other	
16.958		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	2.620	11.398	0.000	14.018	Meadow, non-grazed	01, 02, 03, 04, 05, 06, 07, 08, 09, 10, 11, 12
0.000	0.000	0.000	2.540	0.000	2.540	Paved parking	02, 12, 13
0.000	0.000	0.000	0.400	0.000	0.400	Woods/grass comb., Good	13
0.000	**0.000**	**2.620**	**14.338**	**0.000**	**16.958**	**TOTAL AREA**	

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	02	0.00	0.00	40.0	0.0100	0.011	0.0	18.0	0.0	

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>0.36"
Flow Length=235' Tc=48.8 min CN=78 Runoff=0.28 cfs 0.040 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>0.79"
Flow Length=916' Tc=7.6 min CN=88 Runoff=3.74 cfs 0.172 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=100' Slope=0.0650 '/' Tc=28.1 min CN=78 Runoff=0.06 cfs 0.005 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=100' Slope=0.0300 '/' Tc=38.3 min CN=78 Runoff=0.04 cfs 0.005 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>0.34"
Flow Length=494' Tc=115.6 min CN=78 Runoff=0.51 cfs 0.132 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=179' Tc=32.5 min CN=78 Runoff=0.14 cfs 0.015 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=330' Tc=42.0 min CN=78 Runoff=0.15 cfs 0.020 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>0.35"
Flow Length=417' Tc=59.3 min CN=78 Runoff=0.12 cfs 0.020 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=150' Slope=0.0700 '/' Tc=37.8 min CN=78 Runoff=0.16 cfs 0.019 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.34"
Flow Length=200' Tc=112.5 min CN=78 Runoff=0.03 cfs 0.008 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.36"
Flow Length=62' Slope=0.0200 '/' Tc=30.8 min CN=78 Runoff=0.08 cfs 0.009 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>0.31"
Flow Length=733' Tc=116.0 min CN=77 Runoff=0.41 cfs 0.107 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>0.85"
Tc=5.0 min CN=89 Runoff=1.36 cfs 0.058 af

Total Runoff Area = 16.958 ac Runoff Volume = 0.610 af Average Runoff Depth = 0.43"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

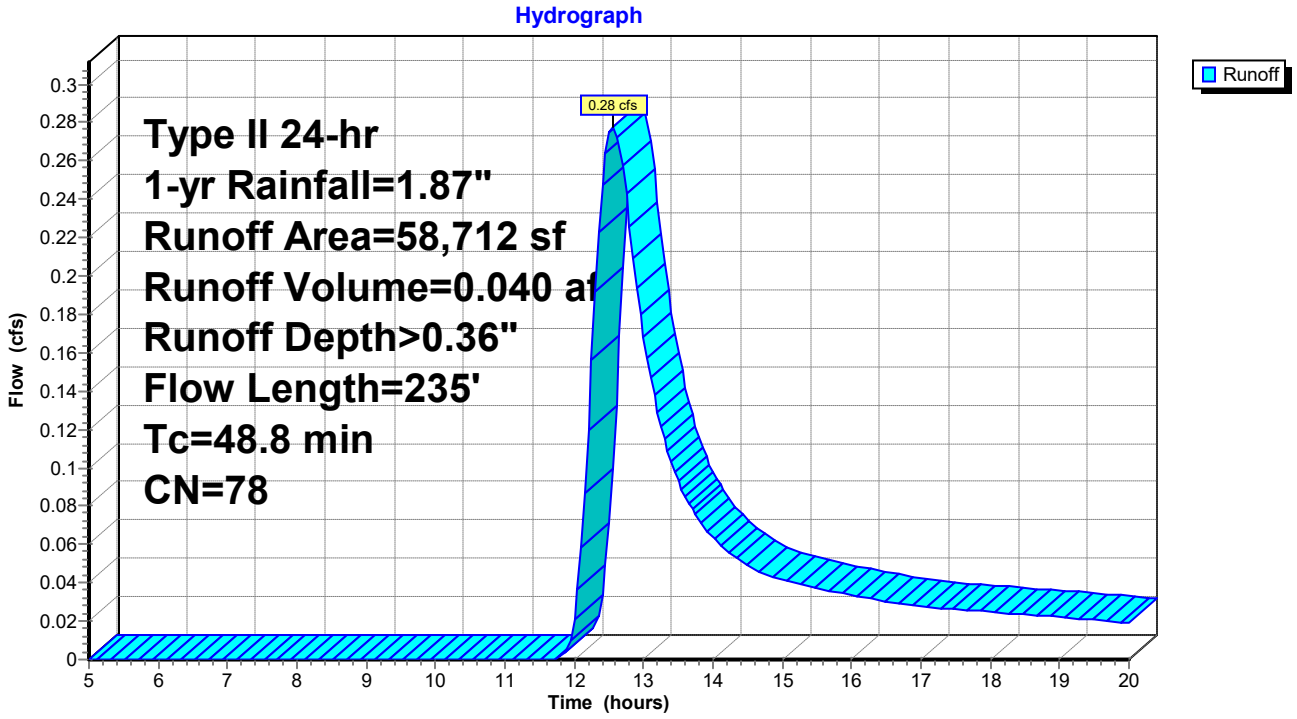
Runoff = 0.28 cfs @ 12.55 hrs, Volume= 0.040 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (sf)	CN	Description
58,712	78	Meadow, non-grazed, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.3	150	0.0400	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
48.8	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

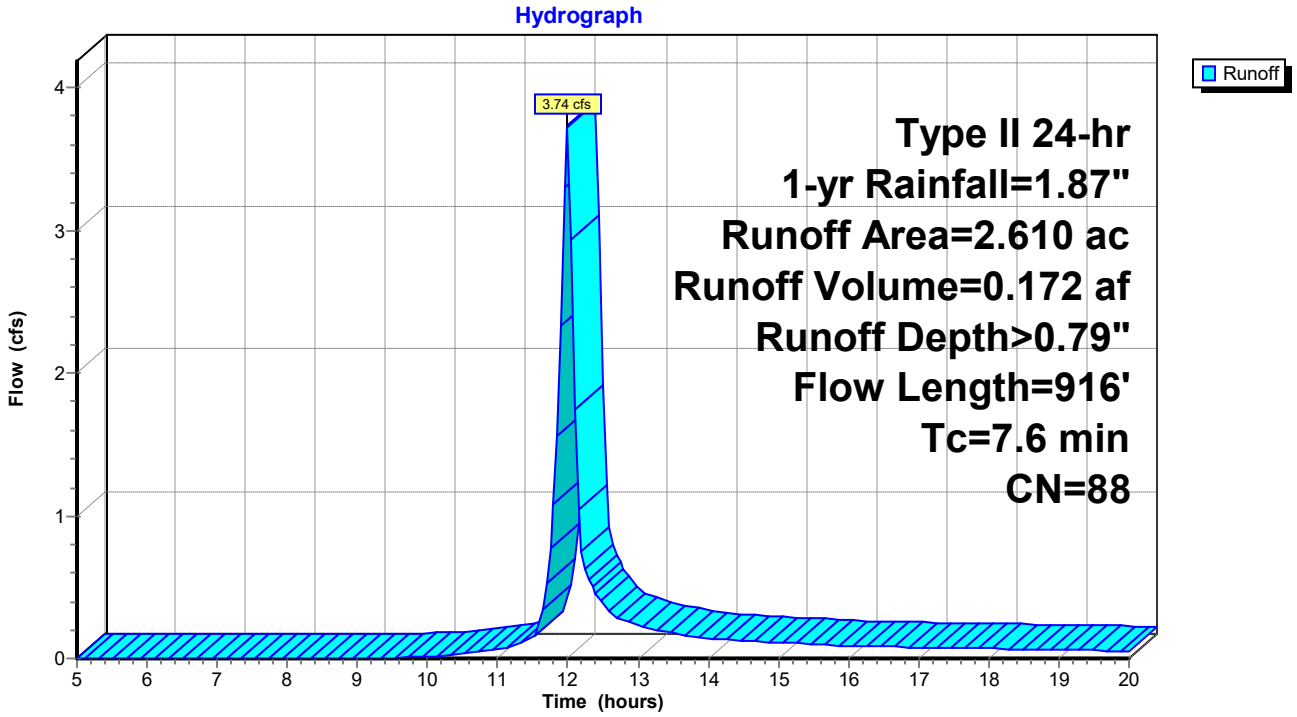
Runoff = 3.74 cfs @ 11.99 hrs, Volume= 0.172 af, Depth> 0.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
1.240	78	Meadow, non-grazed, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

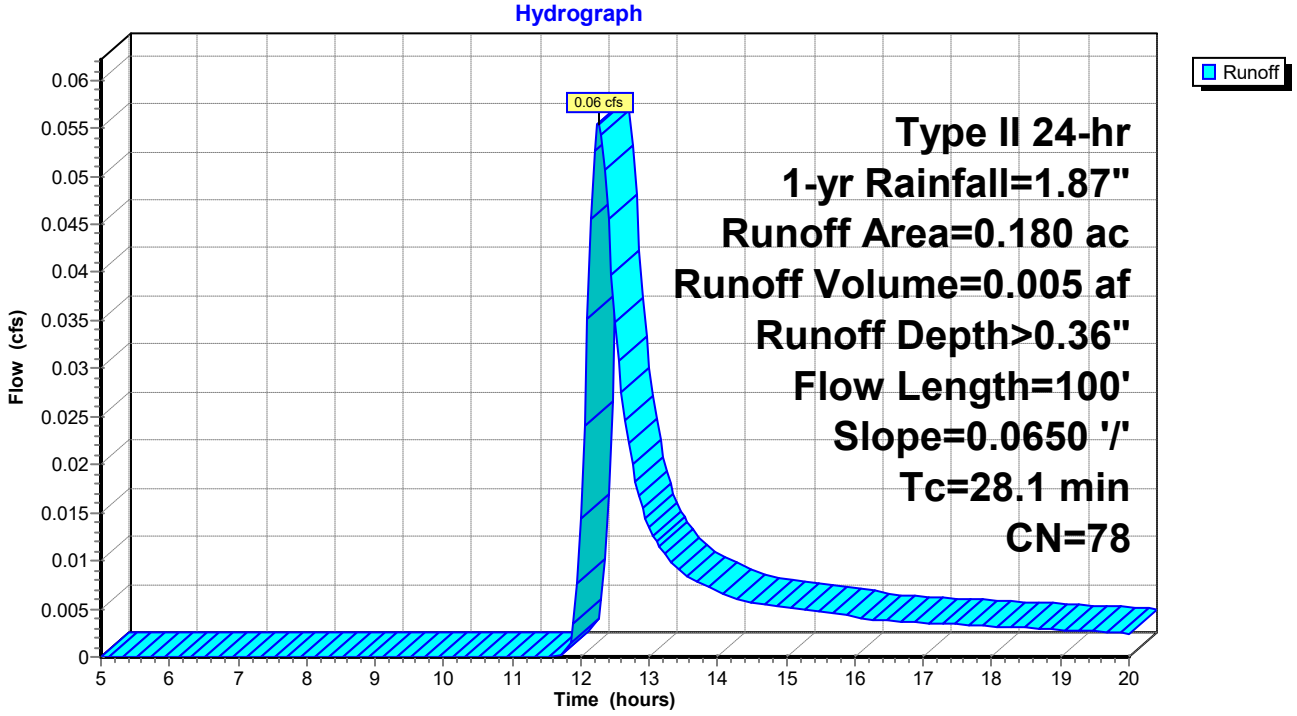
Runoff = 0.06 cfs @ 12.26 hrs, Volume= 0.005 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.180	78	Meadow, non-grazed, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.1	100	0.0650	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

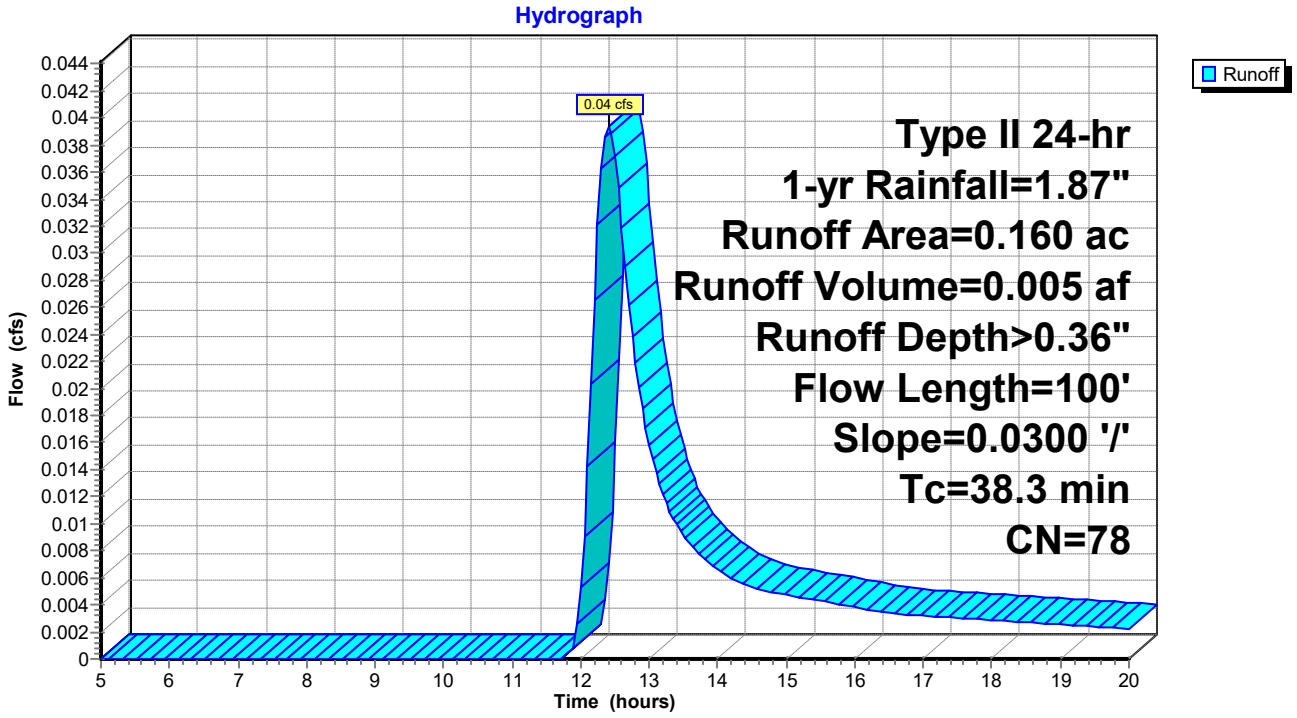
Runoff = 0.04 cfs @ 12.40 hrs, Volume= 0.005 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.160	78	Meadow, non-grazed, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.3	100	0.0300	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

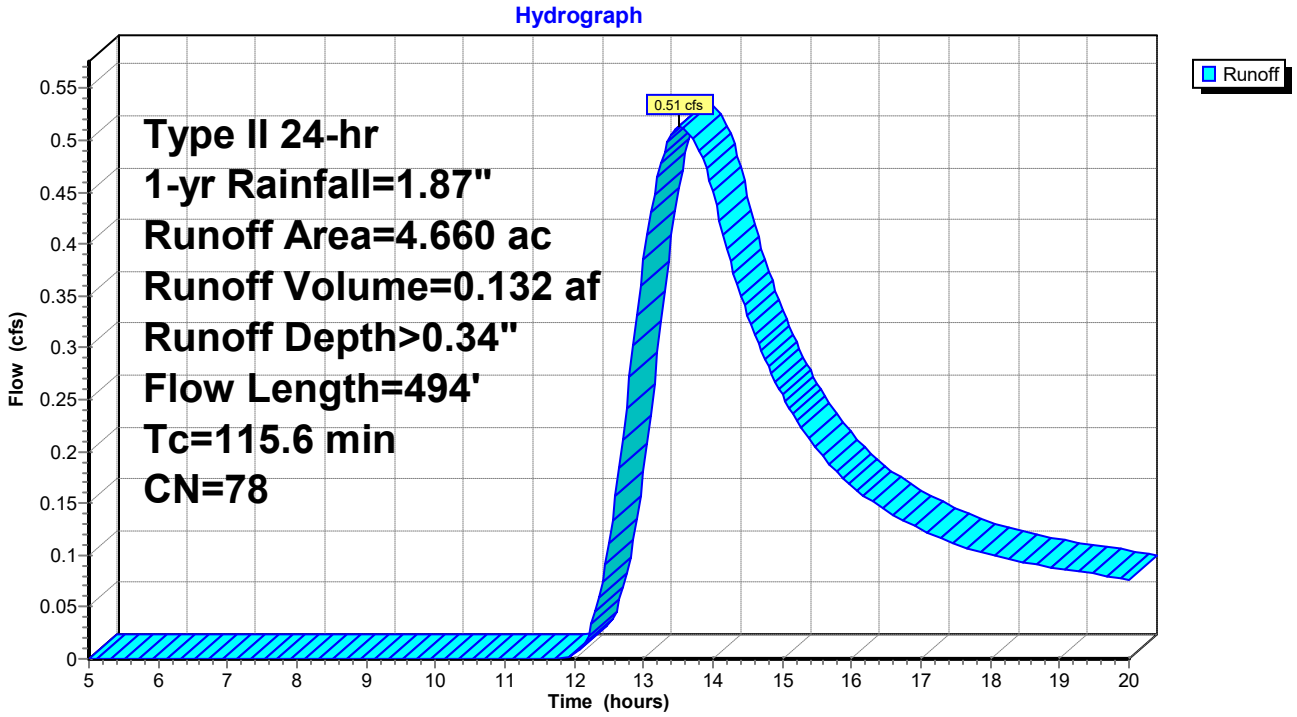
Runoff = 0.51 cfs @ 13.51 hrs, Volume= 0.132 af, Depth> 0.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
4.660	78	Meadow, non-grazed, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
115.6	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

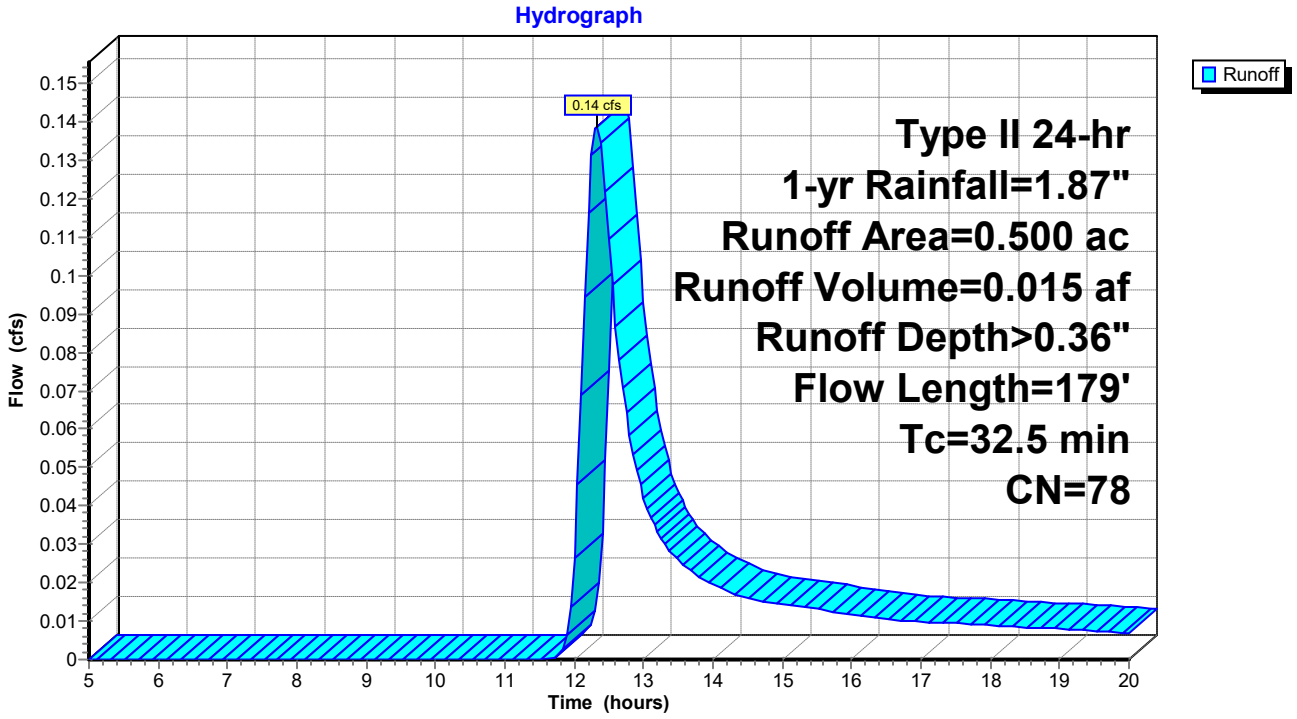
Runoff = 0.14 cfs @ 12.32 hrs, Volume= 0.015 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.500	78	Meadow, non-grazed, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

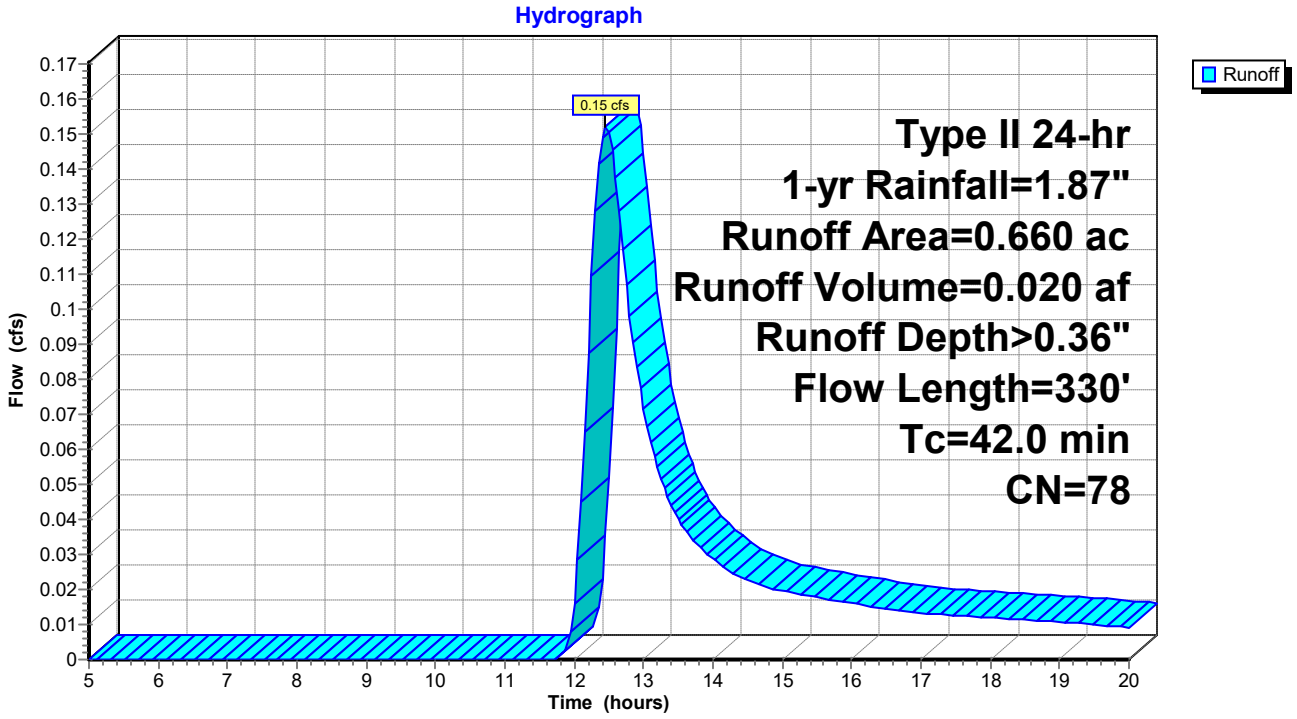
Runoff = 0.15 cfs @ 12.46 hrs, Volume= 0.020 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.660	78	Meadow, non-grazed, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
42.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

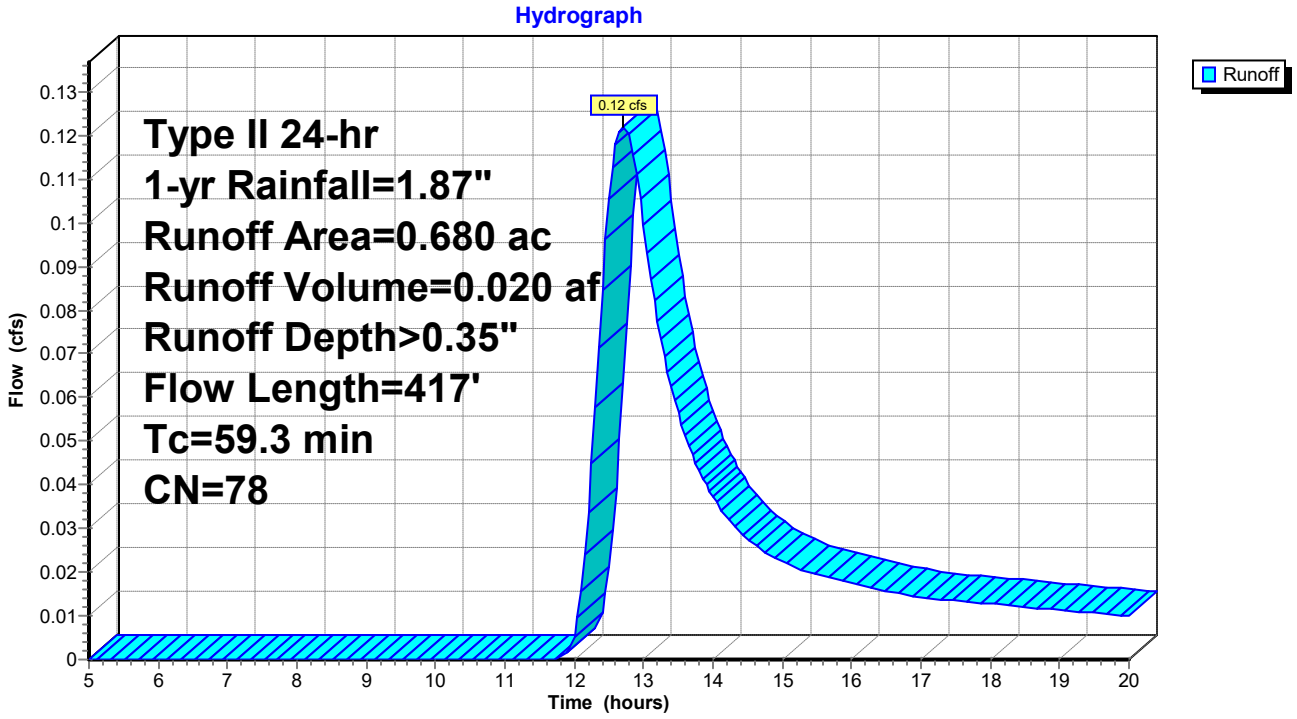
Runoff = 0.12 cfs @ 12.71 hrs, Volume= 0.020 af, Depth> 0.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.680	78	Meadow, non-grazed, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.0	150	0.0300	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
59.3	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

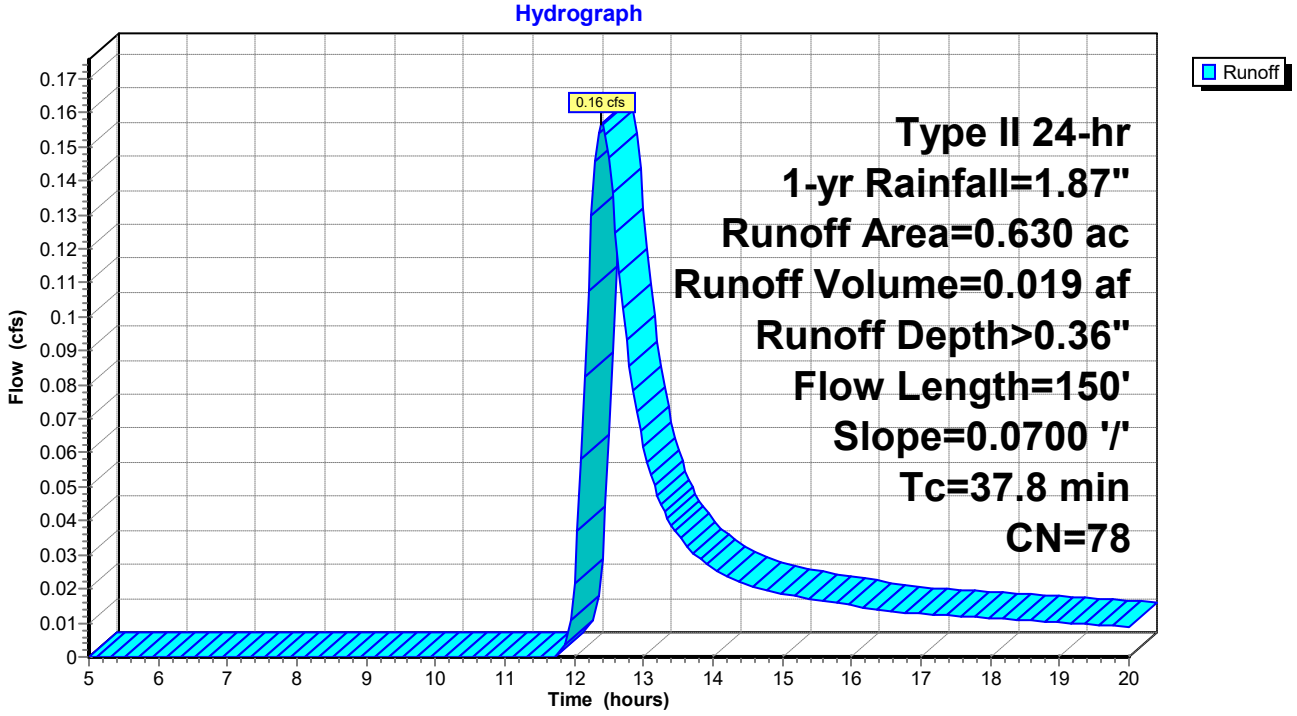
Runoff = 0.16 cfs @ 12.40 hrs, Volume= 0.019 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.630	78	Meadow, non-grazed, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

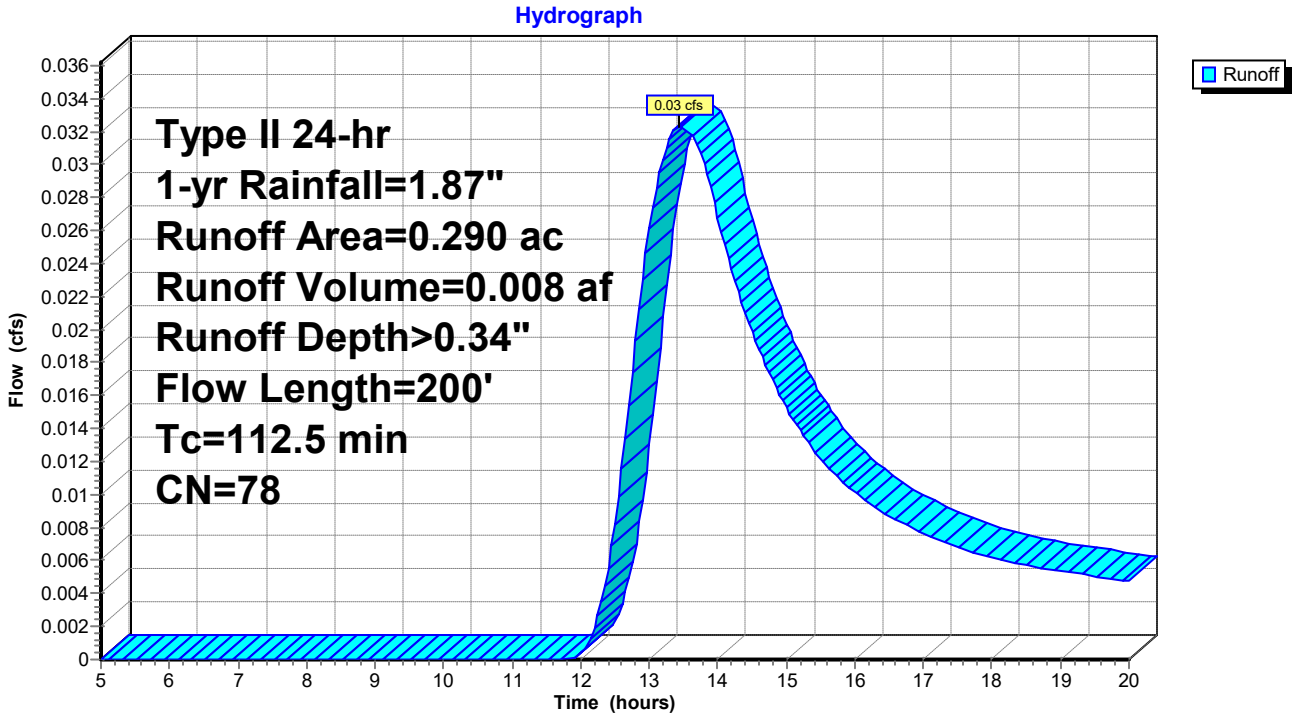
Runoff = 0.03 cfs @ 13.42 hrs, Volume= 0.008 af, Depth> 0.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

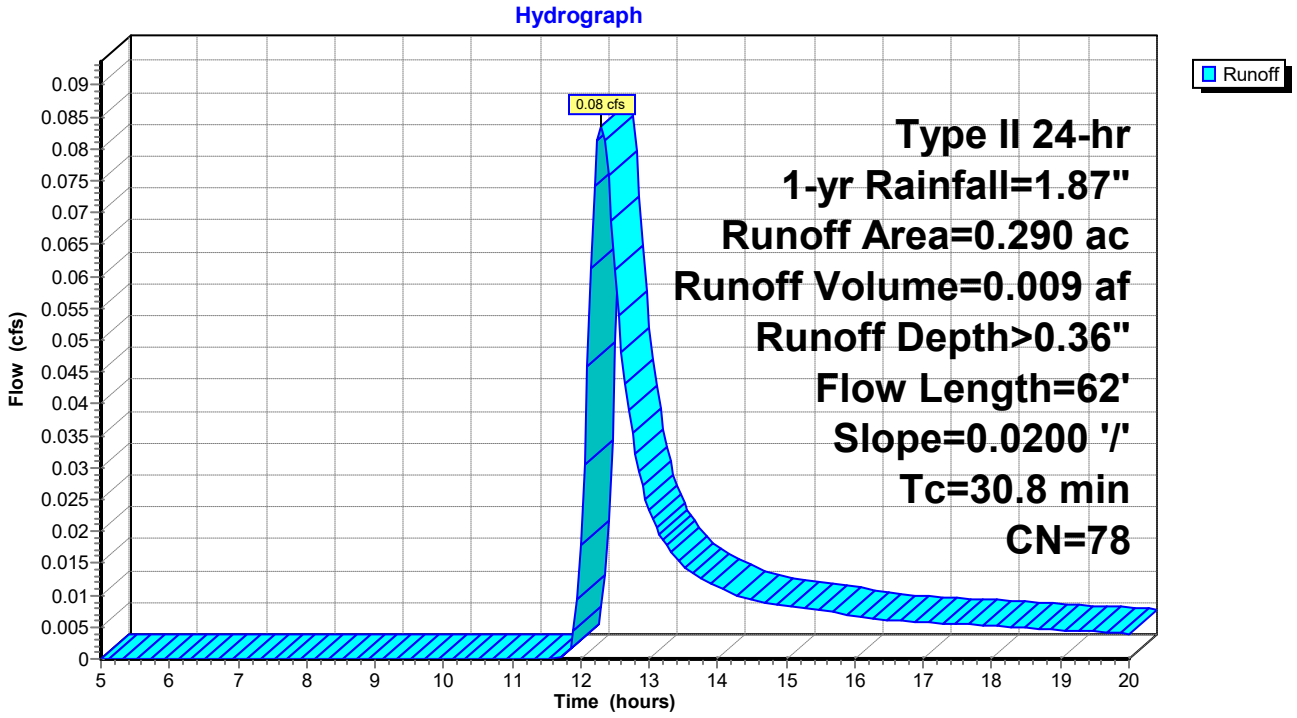
Runoff = 0.08 cfs @ 12.30 hrs, Volume= 0.009 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	62	0.0200	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

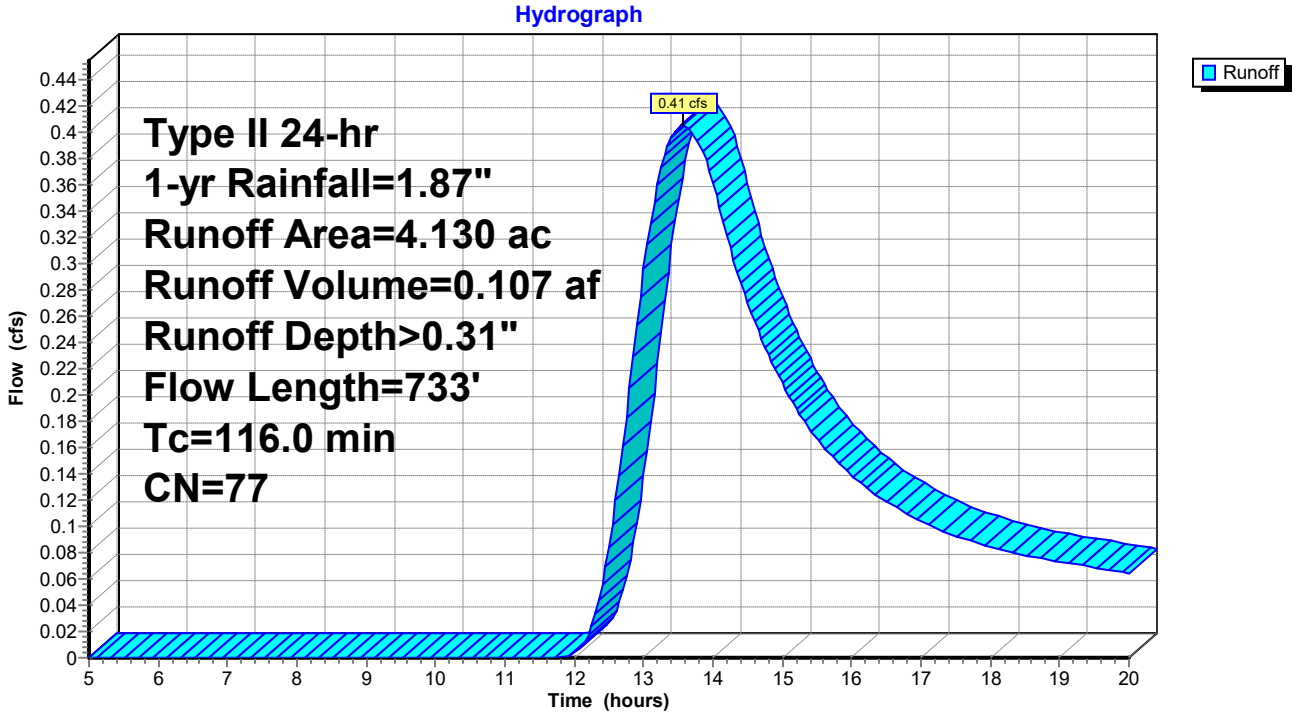
Runoff = 0.41 cfs @ 13.55 hrs, Volume= 0.107 af, Depth> 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	78	Meadow, non-grazed, HSG D
2.620	71	Meadow, non-grazed, HSG C
4.130	77	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
116.0	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

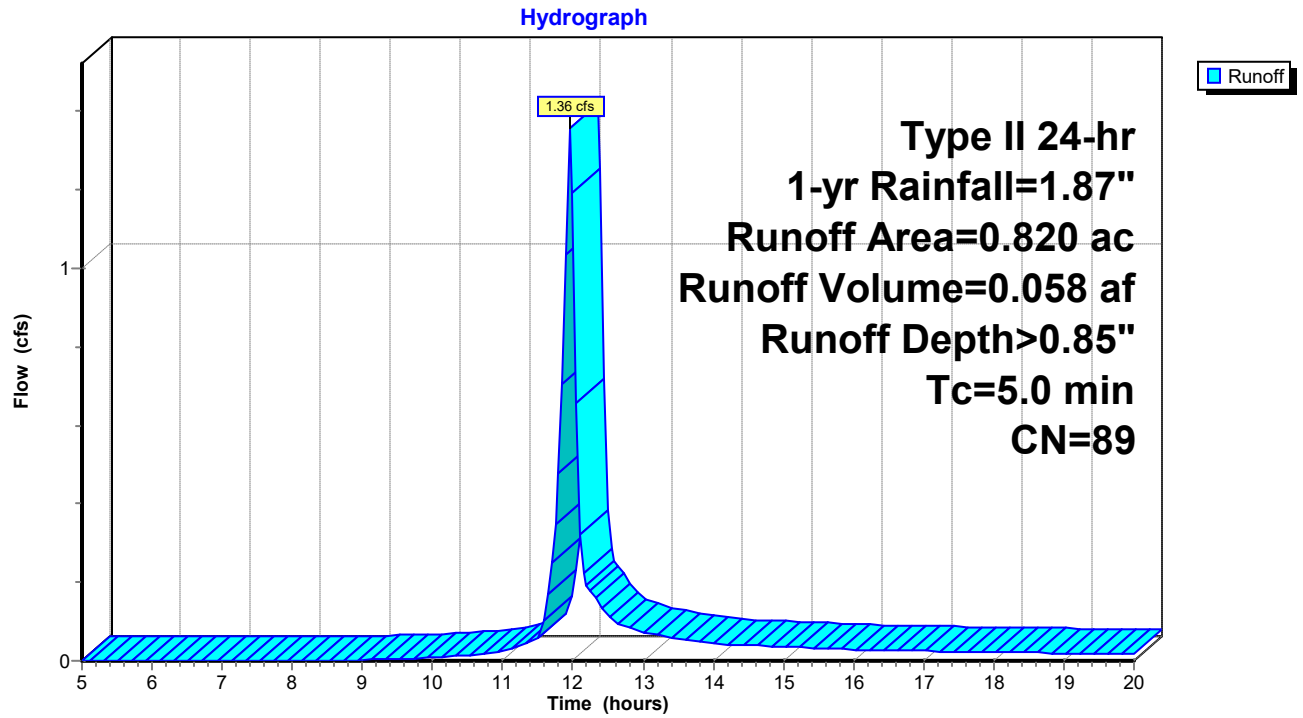
Runoff = 1.36 cfs @ 11.96 hrs, Volume= 0.058 af, Depth> 0.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-yr Rainfall=1.87"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>0.53"
Flow Length=235' Tc=48.8 min CN=78 Runoff=0.44 cfs 0.059 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>1.04"
Flow Length=916' Tc=7.6 min CN=88 Runoff=4.88 cfs 0.226 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=100' Slope=0.0650 '/' Tc=28.1 min CN=78 Runoff=0.09 cfs 0.008 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=100' Slope=0.0300 '/' Tc=38.3 min CN=78 Runoff=0.06 cfs 0.007 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>0.50"
Flow Length=494' Tc=115.6 min CN=78 Runoff=0.80 cfs 0.195 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=179' Tc=32.5 min CN=78 Runoff=0.22 cfs 0.022 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=330' Tc=42.0 min CN=78 Runoff=0.24 cfs 0.029 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>0.52"
Flow Length=417' Tc=59.3 min CN=78 Runoff=0.19 cfs 0.030 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=150' Slope=0.0700 '/' Tc=37.8 min CN=78 Runoff=0.25 cfs 0.028 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.50"
Flow Length=200' Tc=112.5 min CN=78 Runoff=0.05 cfs 0.012 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=62' Slope=0.0200 '/' Tc=30.8 min CN=78 Runoff=0.13 cfs 0.013 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>0.47"
Flow Length=733' Tc=116.0 min CN=77 Runoff=0.64 cfs 0.160 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>1.11"
Tc=5.0 min CN=89 Runoff=1.76 cfs 0.076 af

Total Runoff Area = 16.958 ac Runoff Volume = 0.865 af Average Runoff Depth = 0.61"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

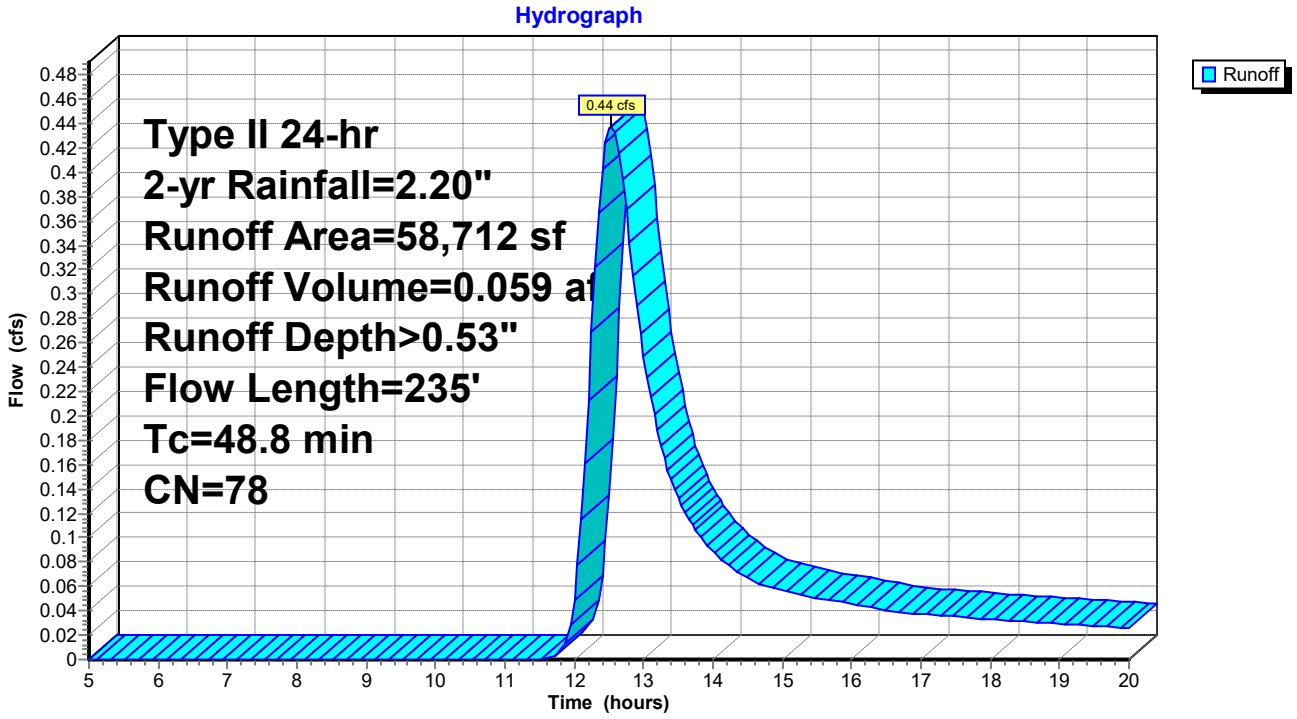
Runoff = 0.44 cfs @ 12.53 hrs, Volume= 0.059 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (sf)	CN	Description
58,712	78	Meadow, non-grazed, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.3	150	0.0400	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
48.8	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

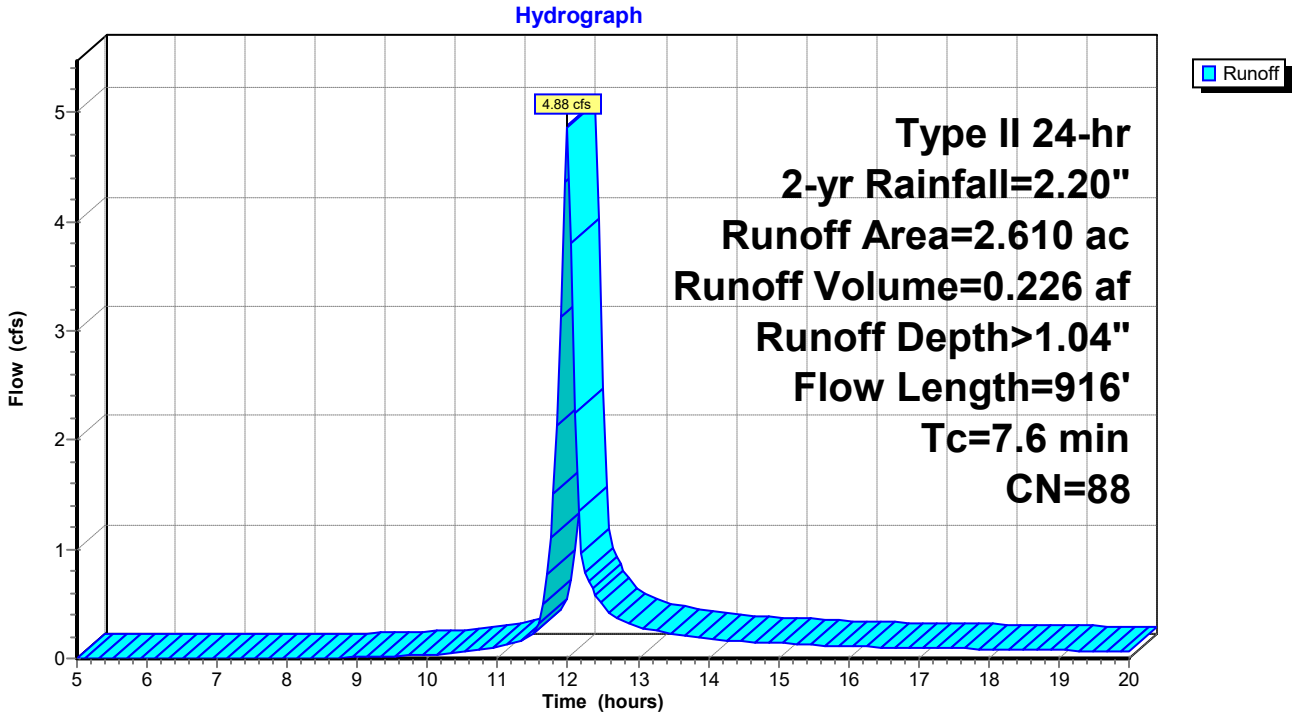
Runoff = 4.88 cfs @ 11.99 hrs, Volume= 0.226 af, Depth> 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
1.240	78	Meadow, non-grazed, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

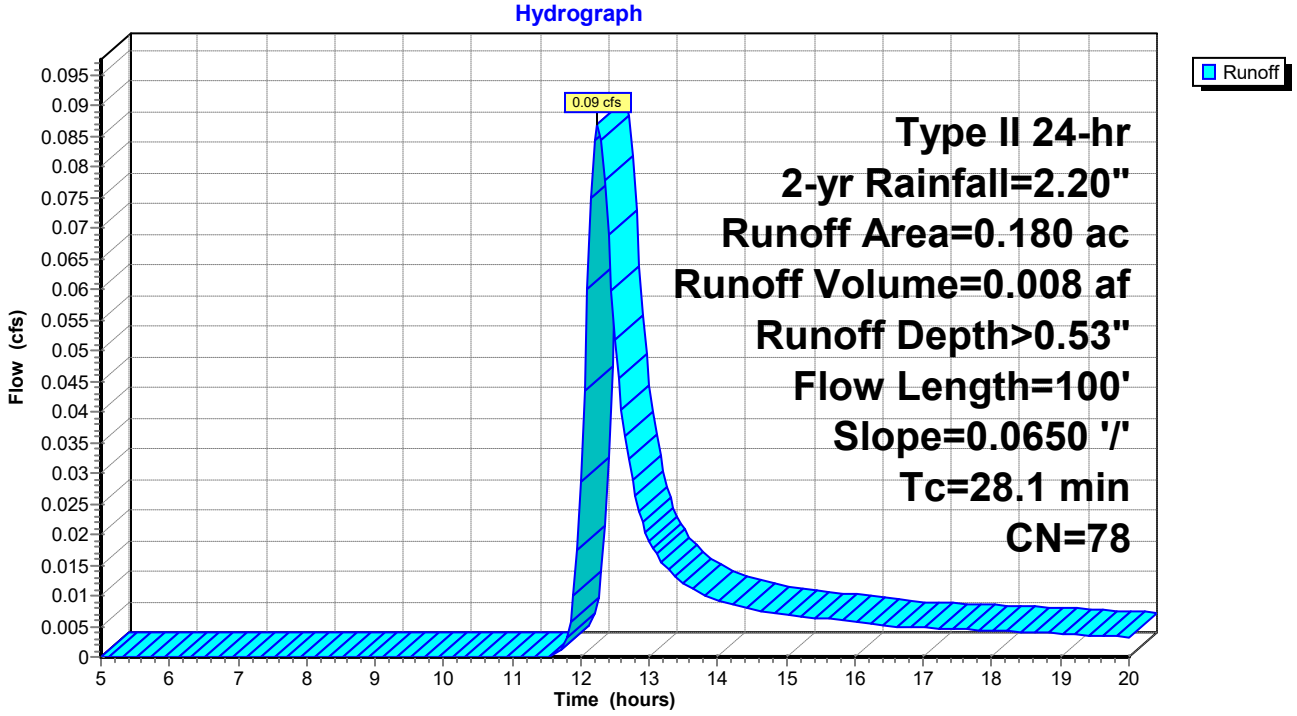
Runoff = 0.09 cfs @ 12.25 hrs, Volume= 0.008 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.180	78	Meadow, non-grazed, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.1	100	0.0650	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

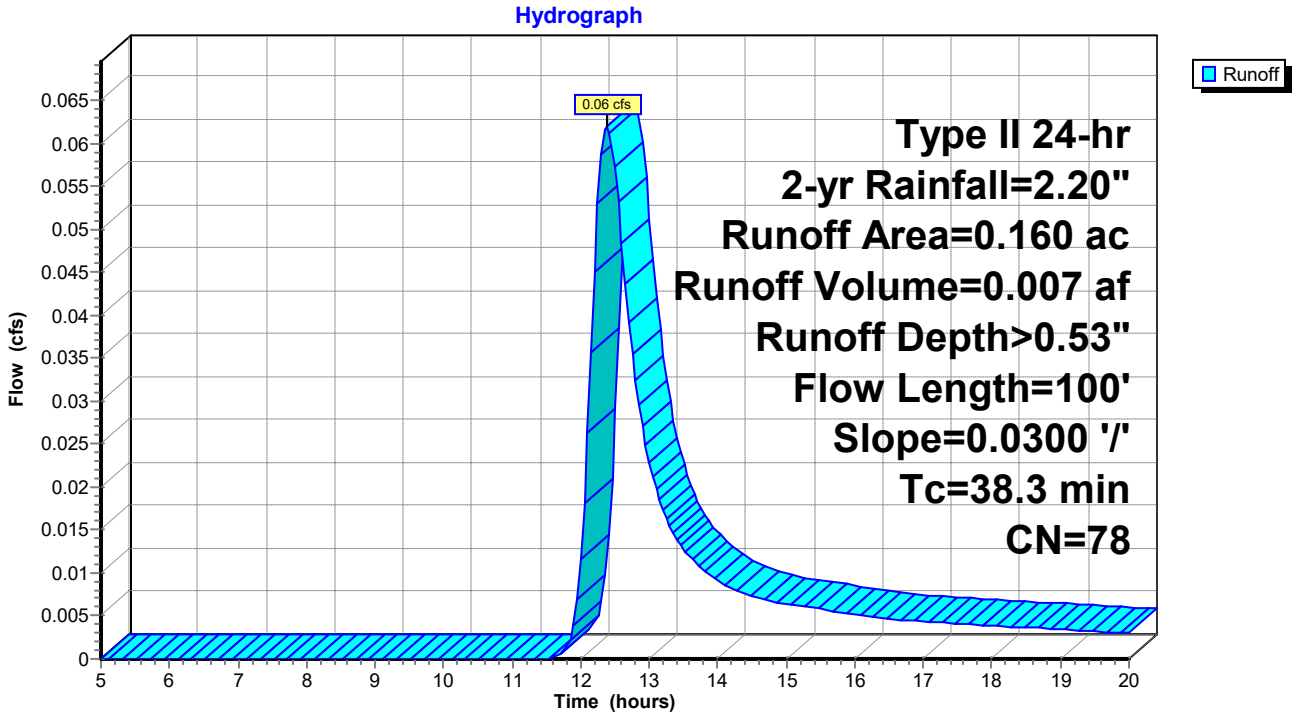
Runoff = 0.06 cfs @ 12.39 hrs, Volume= 0.007 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.160	78	Meadow, non-grazed, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.3	100	0.0300	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

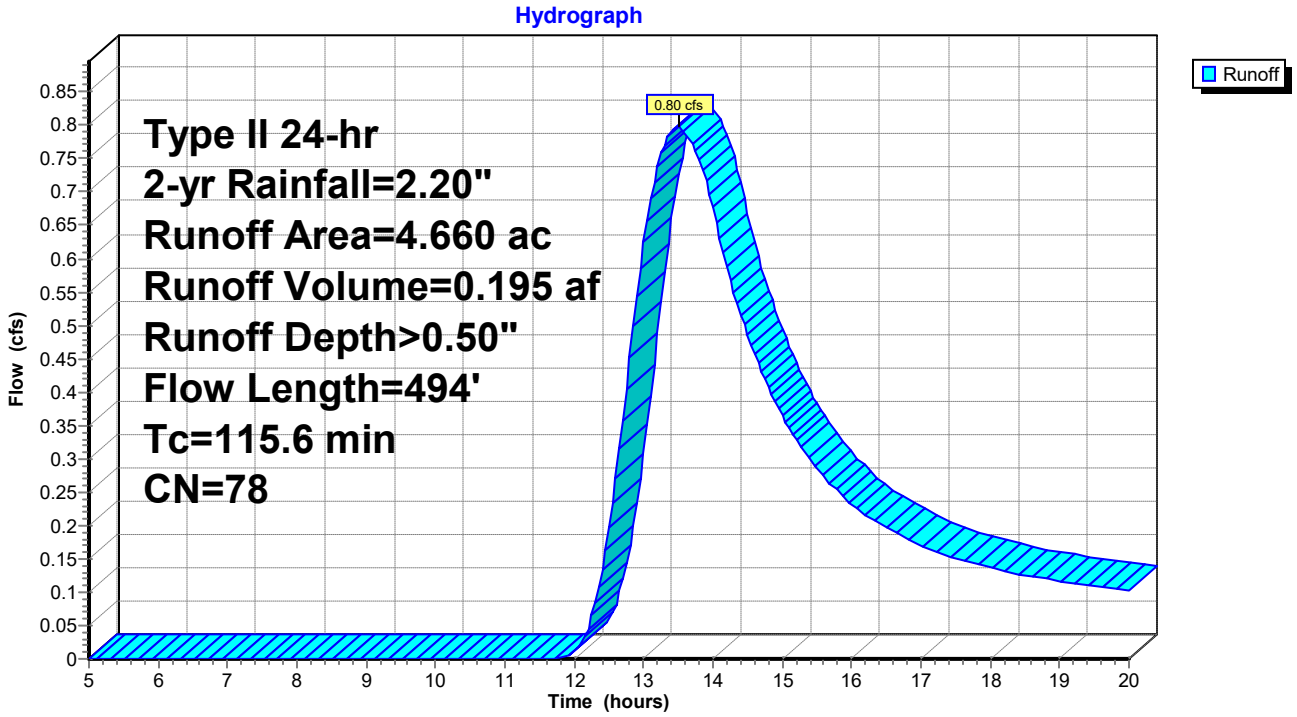
Runoff = 0.80 cfs @ 13.50 hrs, Volume= 0.195 af, Depth> 0.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
4.660	78	Meadow, non-grazed, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
115.6	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

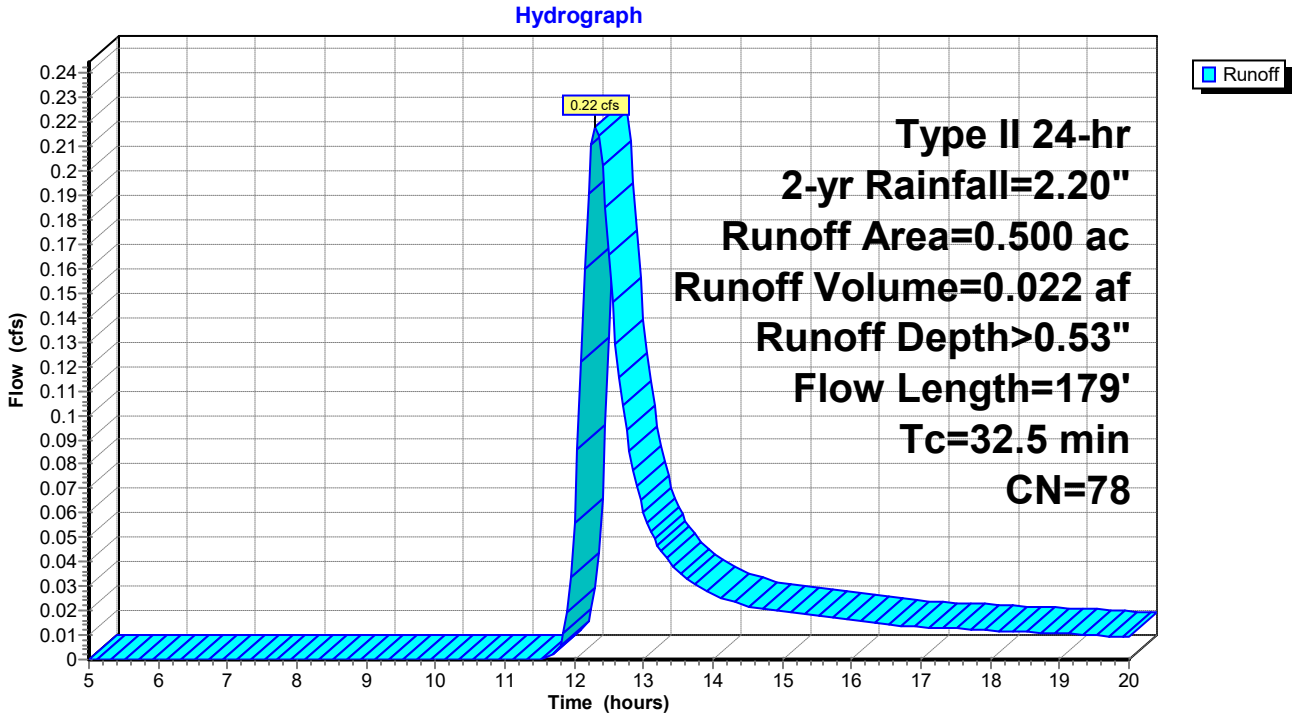
Runoff = 0.22 cfs @ 12.31 hrs, Volume= 0.022 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.500	78	Meadow, non-grazed, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

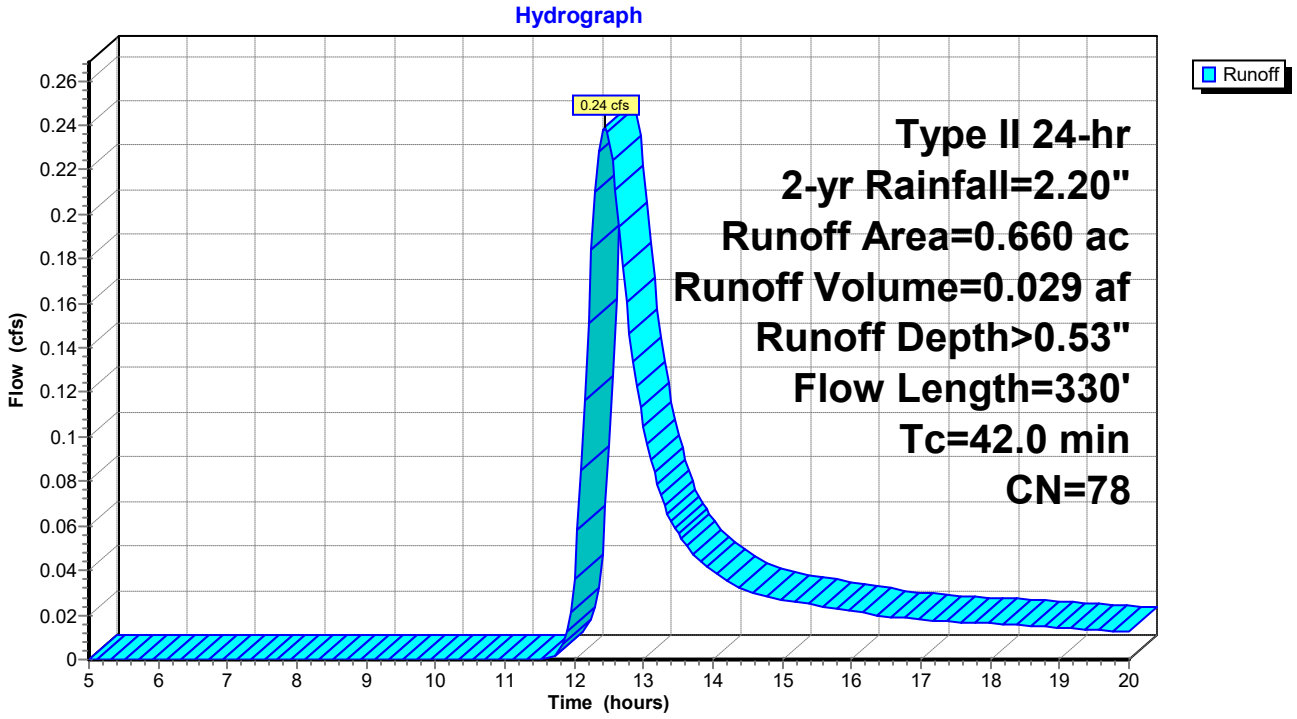
Runoff = 0.24 cfs @ 12.44 hrs, Volume= 0.029 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.660	78	Meadow, non-grazed, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
42.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

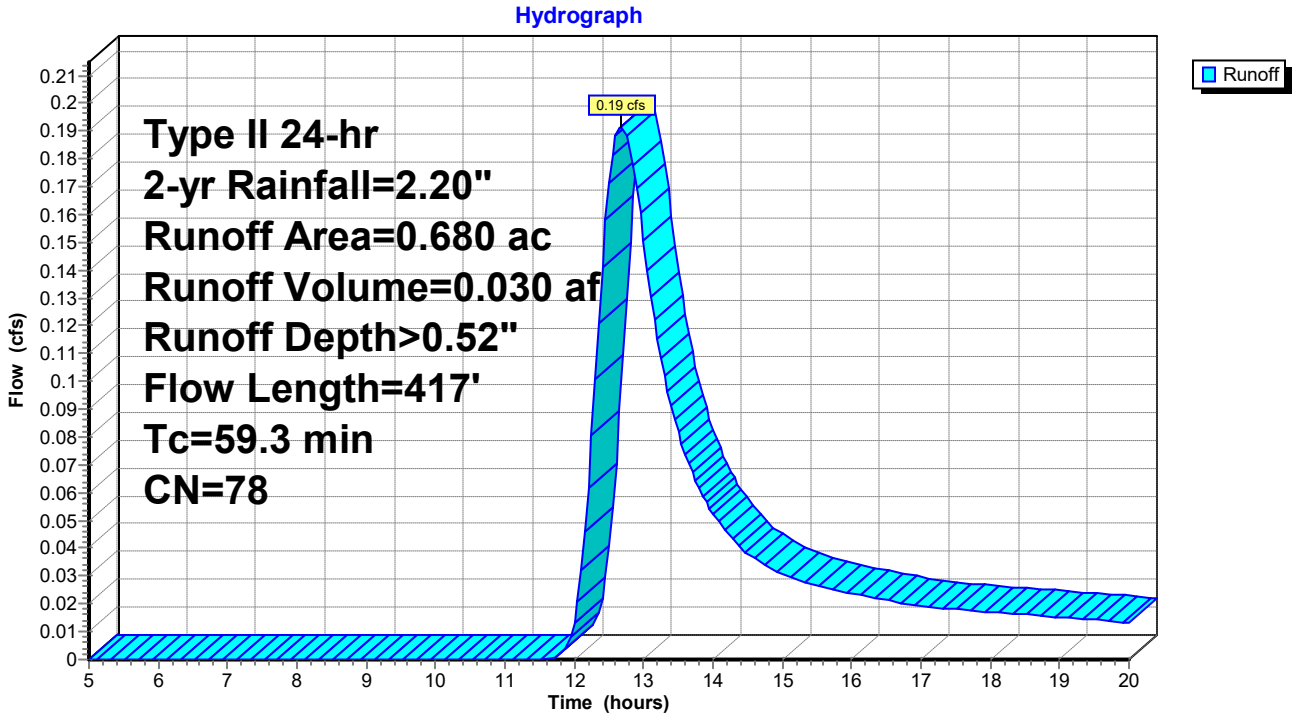
Runoff = 0.19 cfs @ 12.69 hrs, Volume= 0.030 af, Depth> 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.680	78	Meadow, non-grazed, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.0	150	0.0300	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
59.3	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

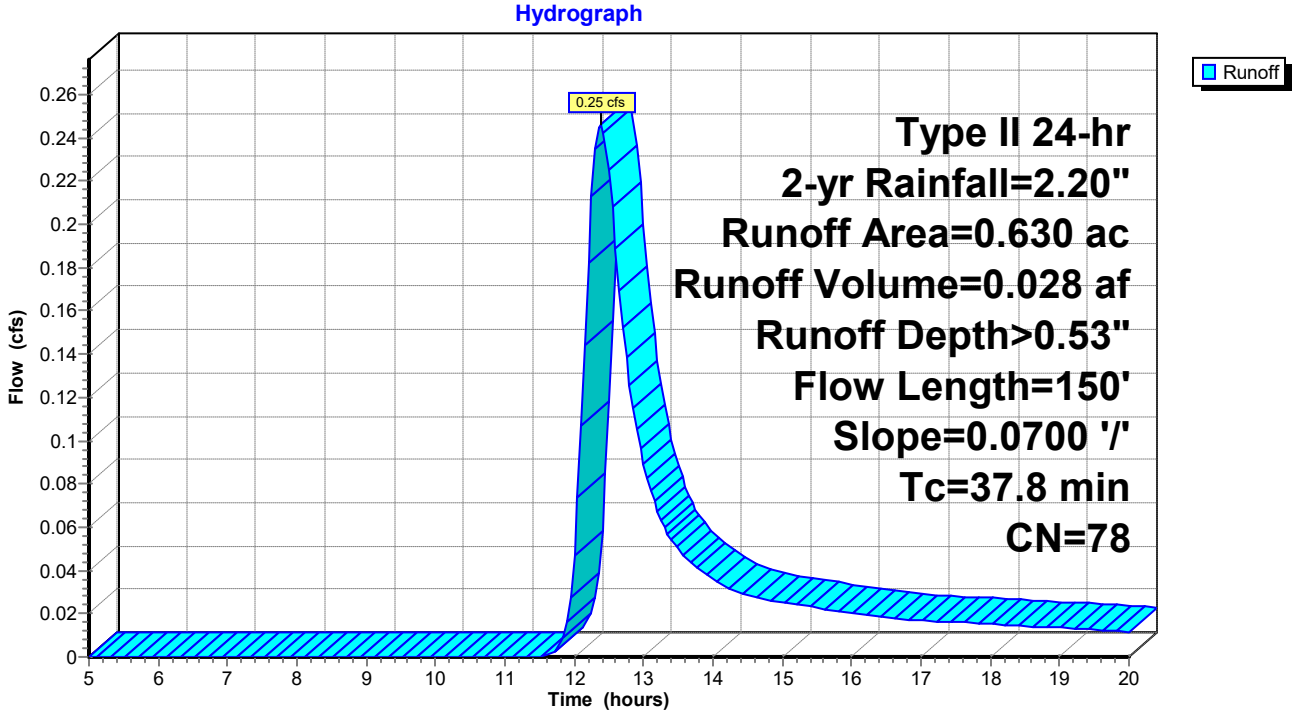
Runoff = 0.25 cfs @ 12.38 hrs, Volume= 0.028 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.630	78	Meadow, non-grazed, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

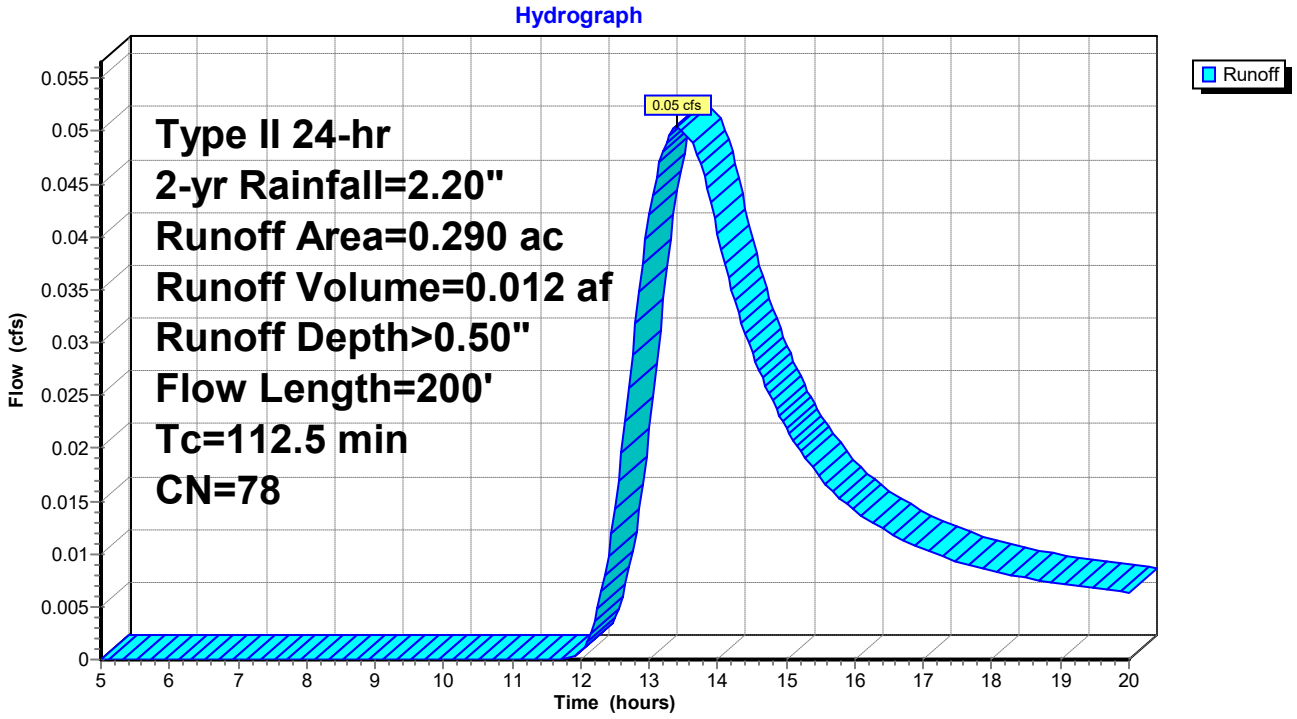
Runoff = 0.05 cfs @ 13.40 hrs, Volume= 0.012 af, Depth> 0.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

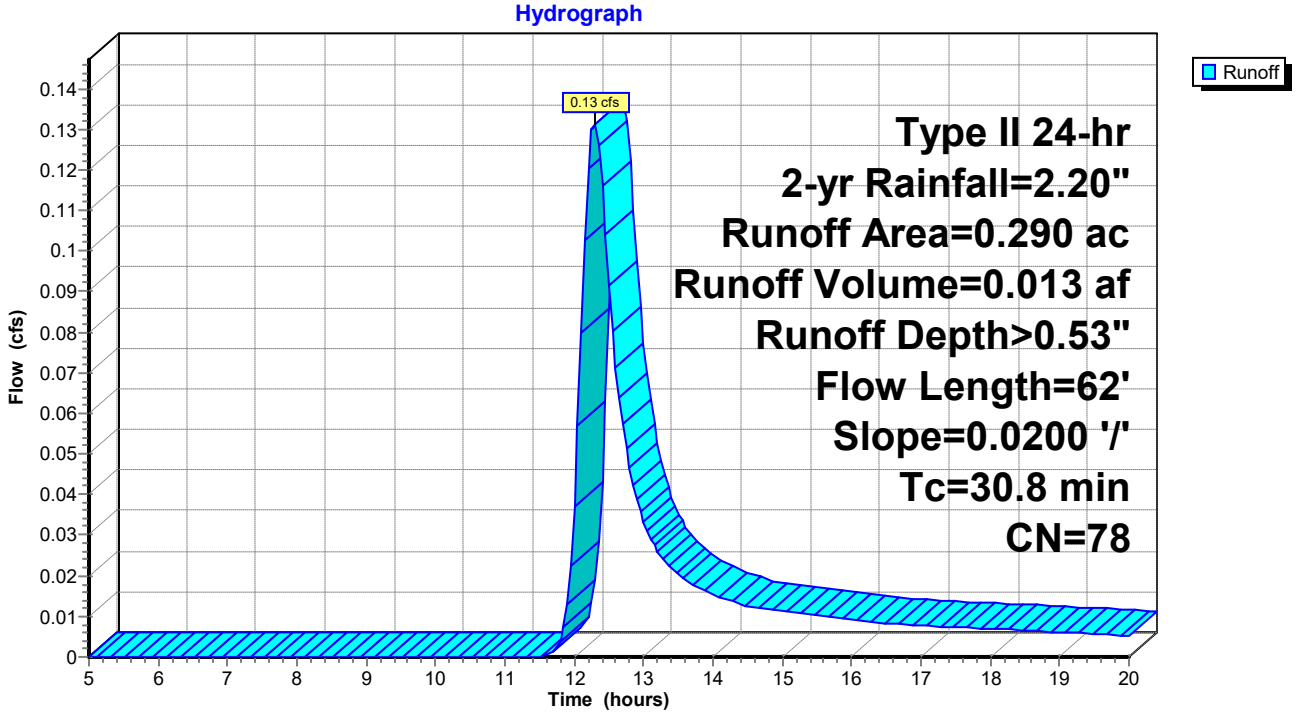
Runoff = 0.13 cfs @ 12.29 hrs, Volume= 0.013 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	62	0.0200	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

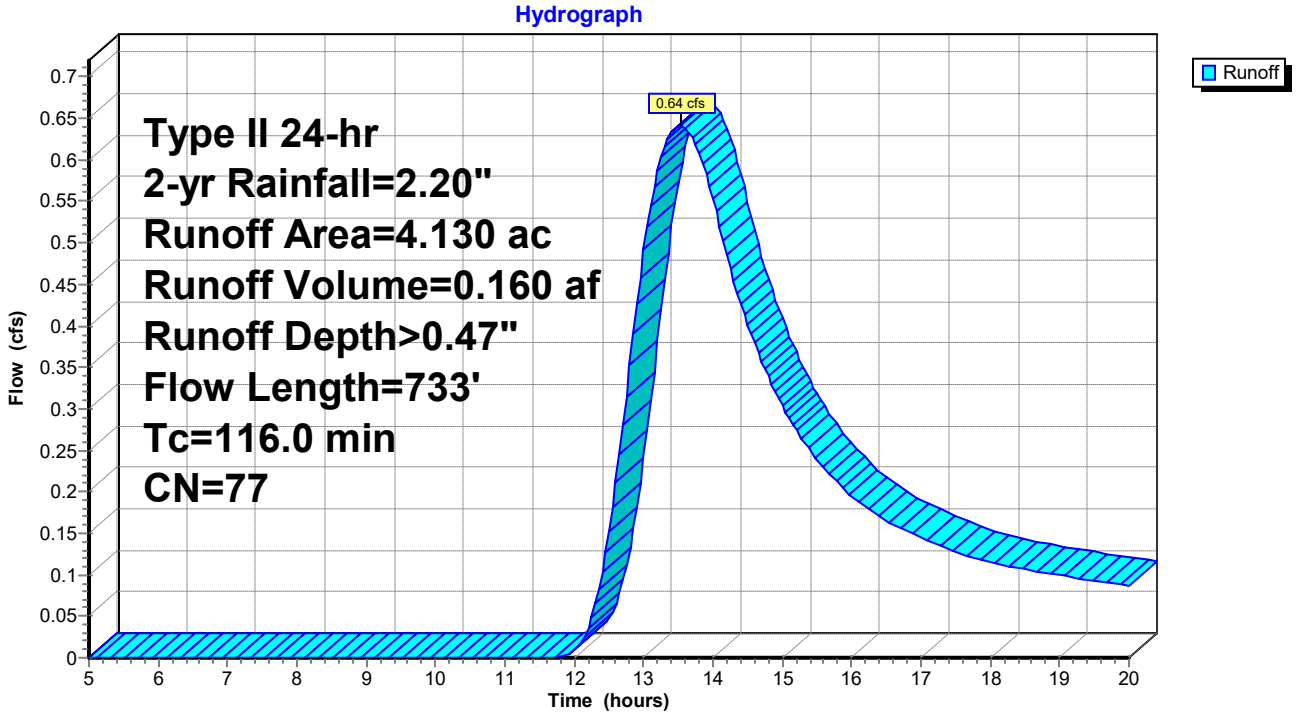
Runoff = 0.64 cfs @ 13.53 hrs, Volume= 0.160 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	78	Meadow, non-grazed, HSG D
2.620	71	Meadow, non-grazed, HSG C
4.130	77	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
116.0	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

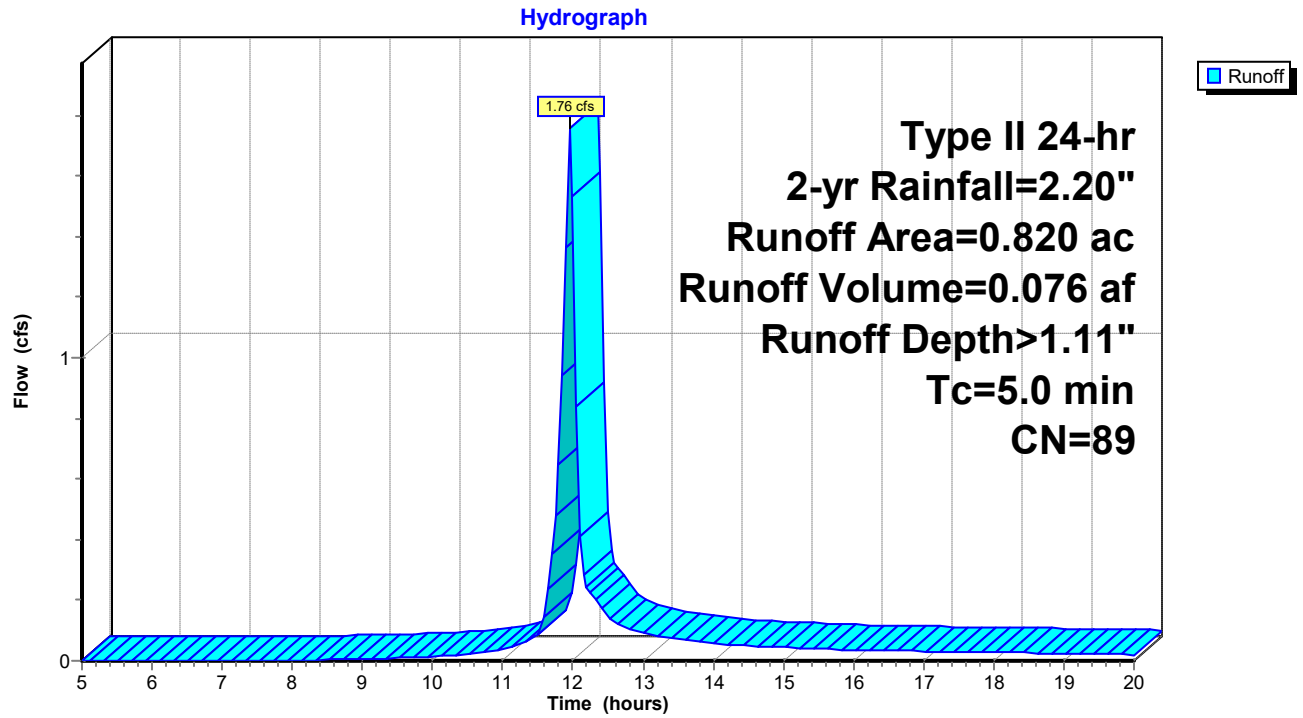
Runoff = 1.76 cfs @ 11.96 hrs, Volume= 0.076 af, Depth> 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 2-yr Rainfall=2.20"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>1.11"
Flow Length=235' Tc=48.8 min CN=78 Runoff=1.00 cfs 0.125 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>1.82"
Flow Length=916' Tc=7.6 min CN=88 Runoff=8.35 cfs 0.396 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=100' Slope=0.0650 '/' Tc=28.1 min CN=78 Runoff=0.20 cfs 0.017 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=100' Slope=0.0300 '/' Tc=38.3 min CN=78 Runoff=0.14 cfs 0.015 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>1.07"
Flow Length=494' Tc=115.6 min CN=78 Runoff=1.80 cfs 0.417 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=179' Tc=32.5 min CN=78 Runoff=0.49 cfs 0.047 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=330' Tc=42.0 min CN=78 Runoff=0.54 cfs 0.061 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>1.11"
Flow Length=417' Tc=59.3 min CN=78 Runoff=0.44 cfs 0.063 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=150' Slope=0.0700 '/' Tc=37.8 min CN=78 Runoff=0.56 cfs 0.059 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>1.07"
Flow Length=200' Tc=112.5 min CN=78 Runoff=0.11 cfs 0.026 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>1.12"
Flow Length=62' Slope=0.0200 '/' Tc=30.8 min CN=78 Runoff=0.30 cfs 0.027 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>1.02"
Flow Length=733' Tc=116.0 min CN=77 Runoff=1.49 cfs 0.350 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>1.91"
Tc=5.0 min CN=89 Runoff=2.94 cfs 0.130 af

Total Runoff Area = 16.958 ac Runoff Volume = 1.732 af Average Runoff Depth = 1.23"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

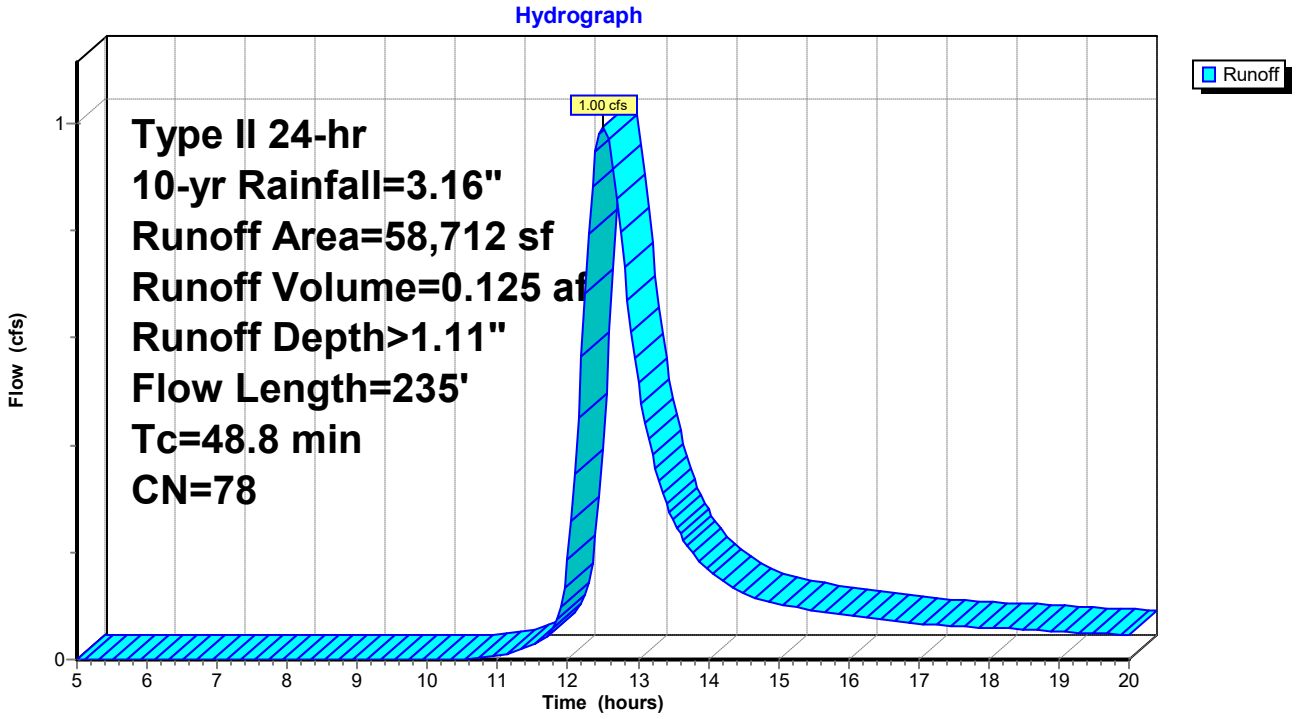
Runoff = 1.00 cfs @ 12.50 hrs, Volume= 0.125 af, Depth> 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (sf)	CN	Description
58,712	78	Meadow, non-grazed, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.3	150	0.0400	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
48.8	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

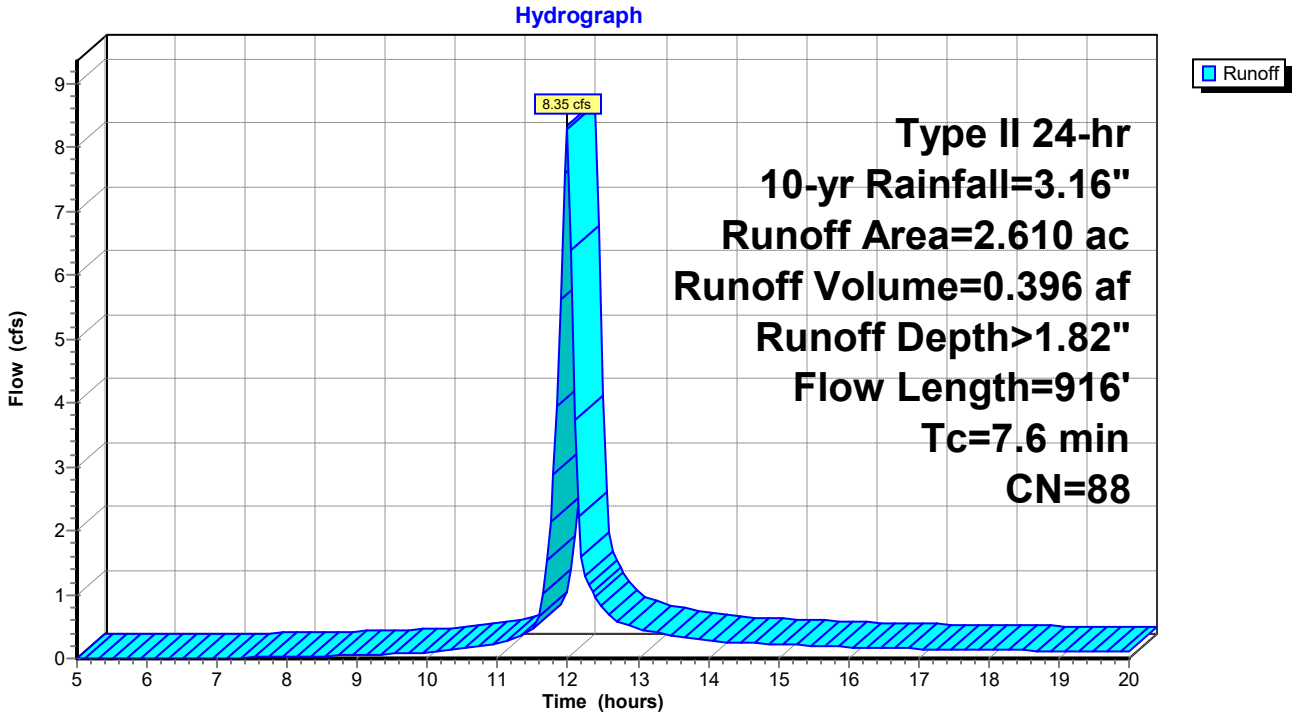
Runoff = 8.35 cfs @ 11.99 hrs, Volume= 0.396 af, Depth> 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
1.240	78	Meadow, non-grazed, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

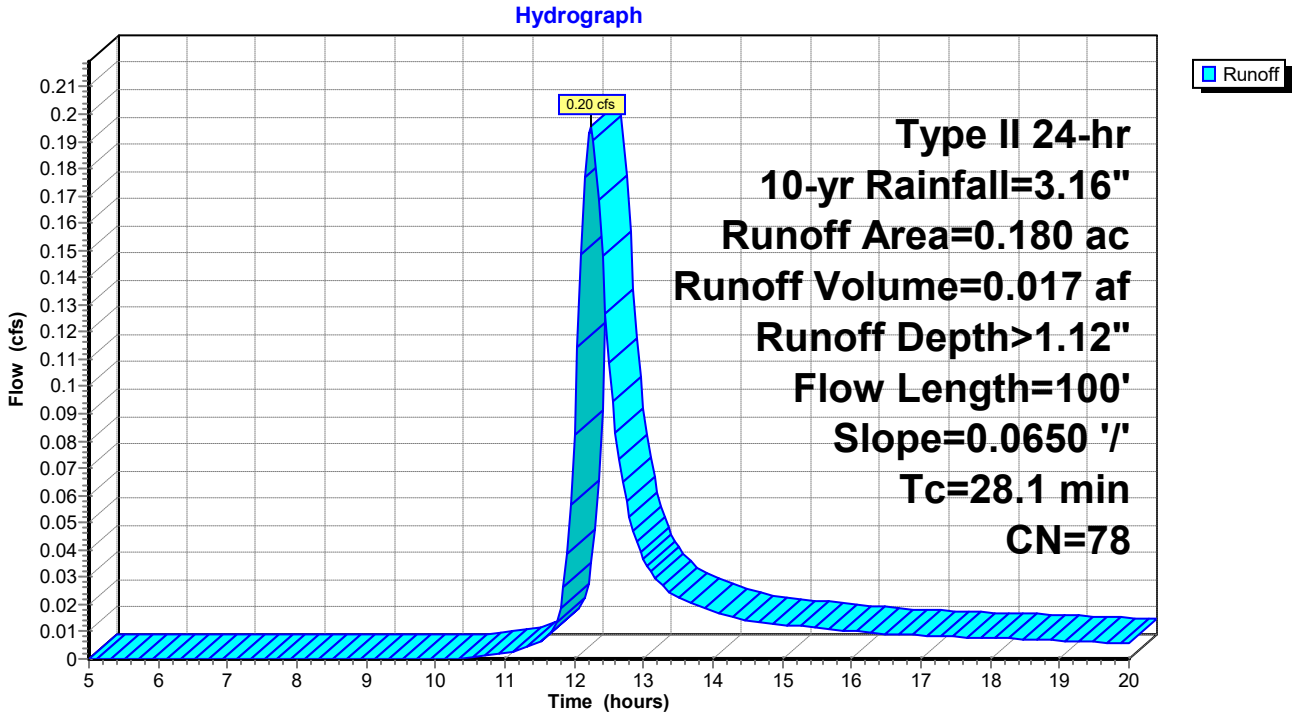
Runoff = 0.20 cfs @ 12.23 hrs, Volume= 0.017 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.180	78	Meadow, non-grazed, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.1	100	0.0650	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

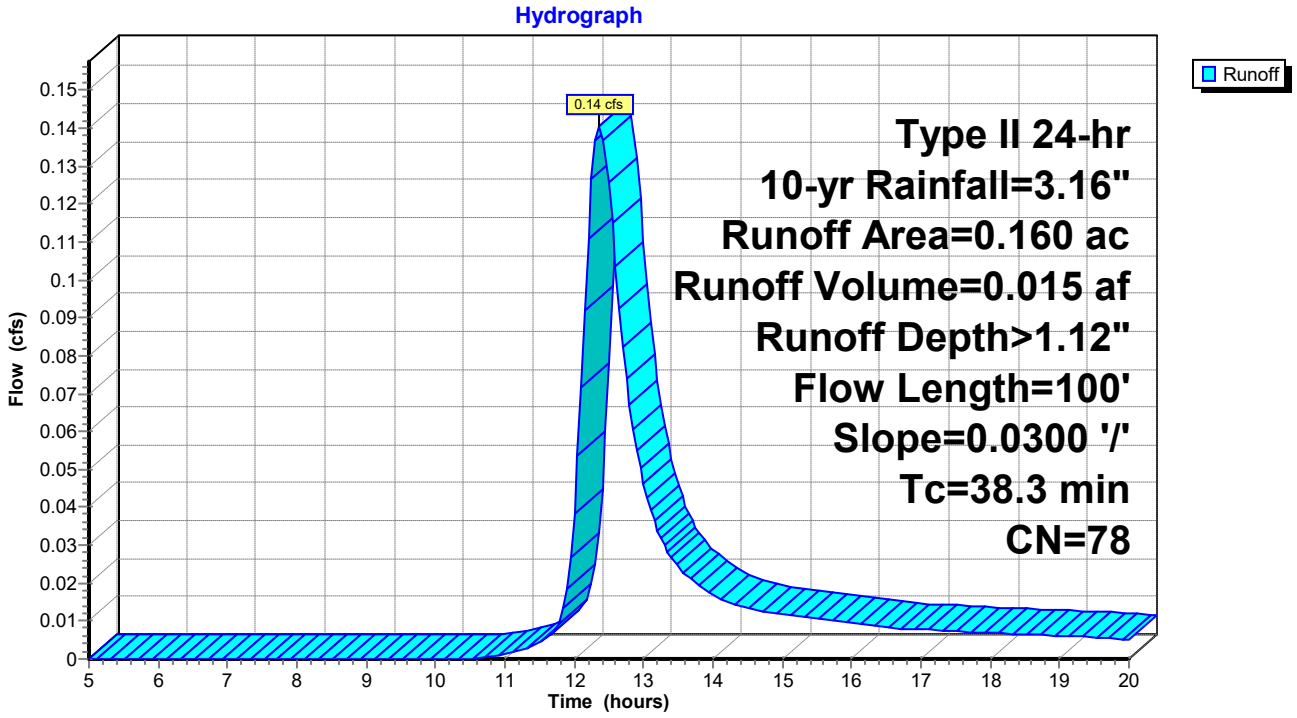
Runoff = 0.14 cfs @ 12.36 hrs, Volume= 0.015 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.160	78	Meadow, non-grazed, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.3	100	0.0300	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

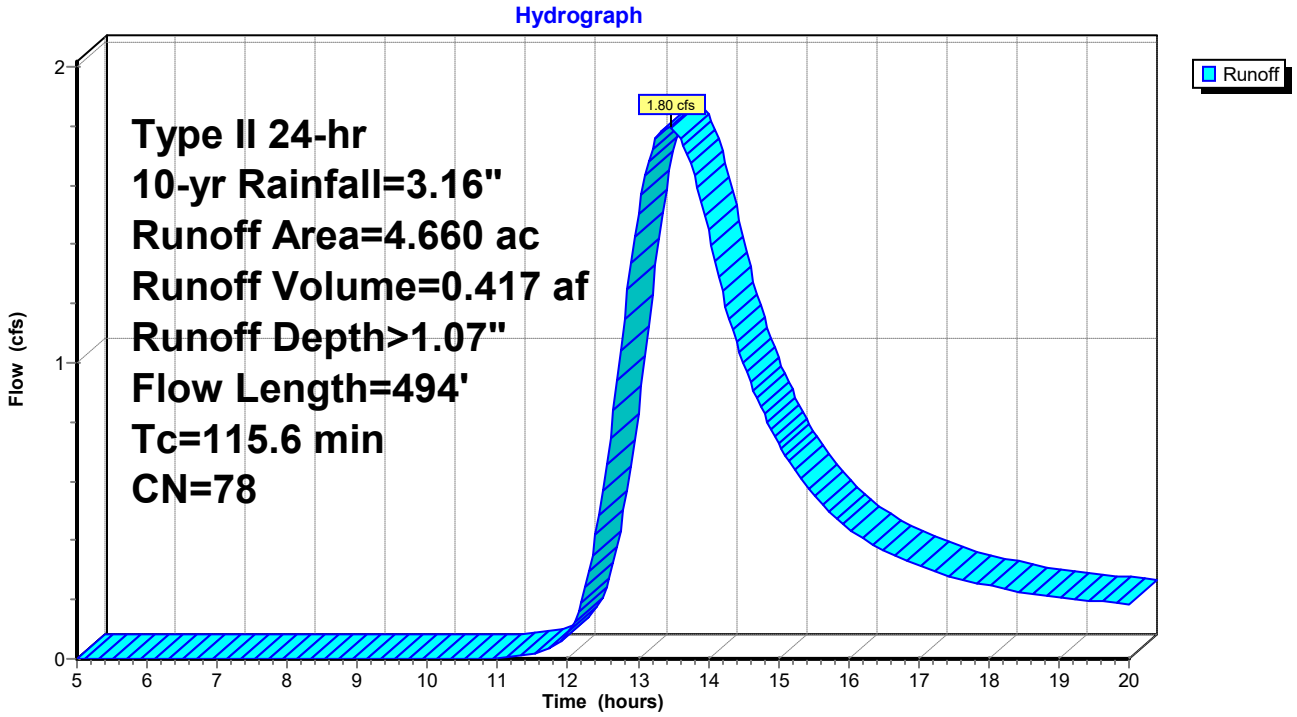
Runoff = 1.80 cfs @ 13.46 hrs, Volume= 0.417 af, Depth> 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
4.660	78	Meadow, non-grazed, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
115.6	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

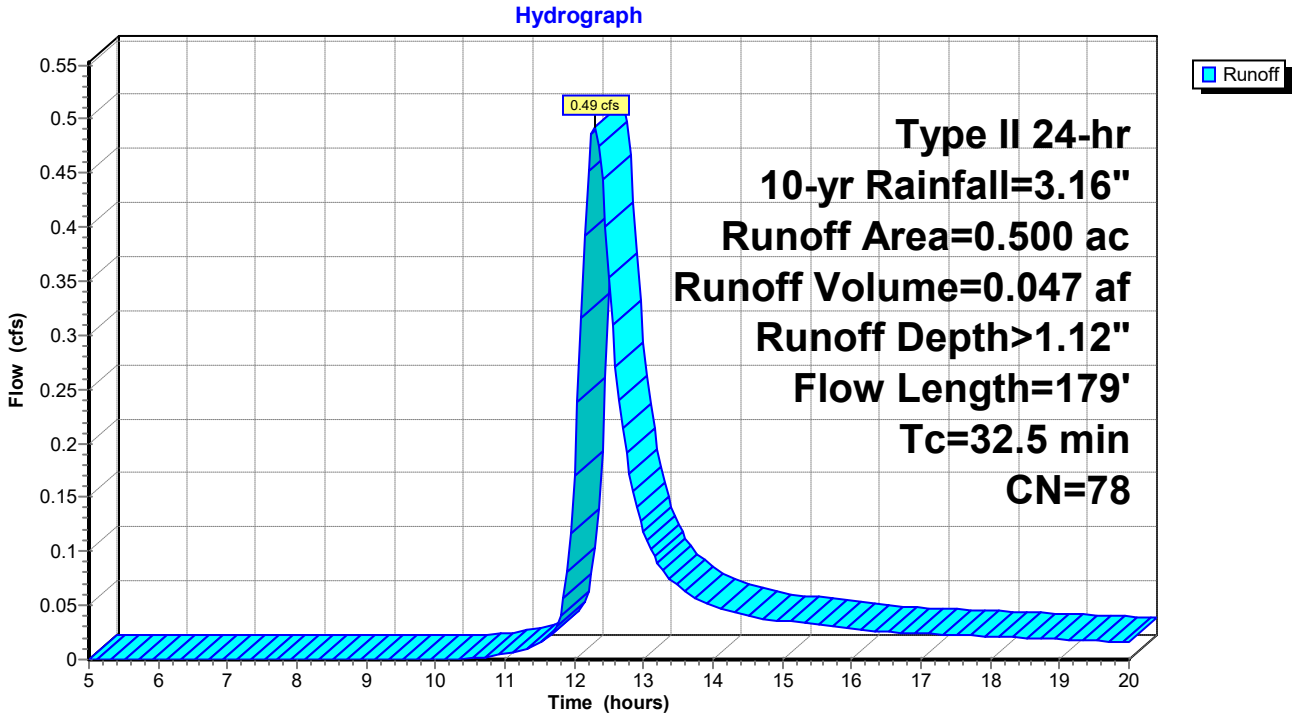
Runoff = 0.49 cfs @ 12.29 hrs, Volume= 0.047 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.500	78	Meadow, non-grazed, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

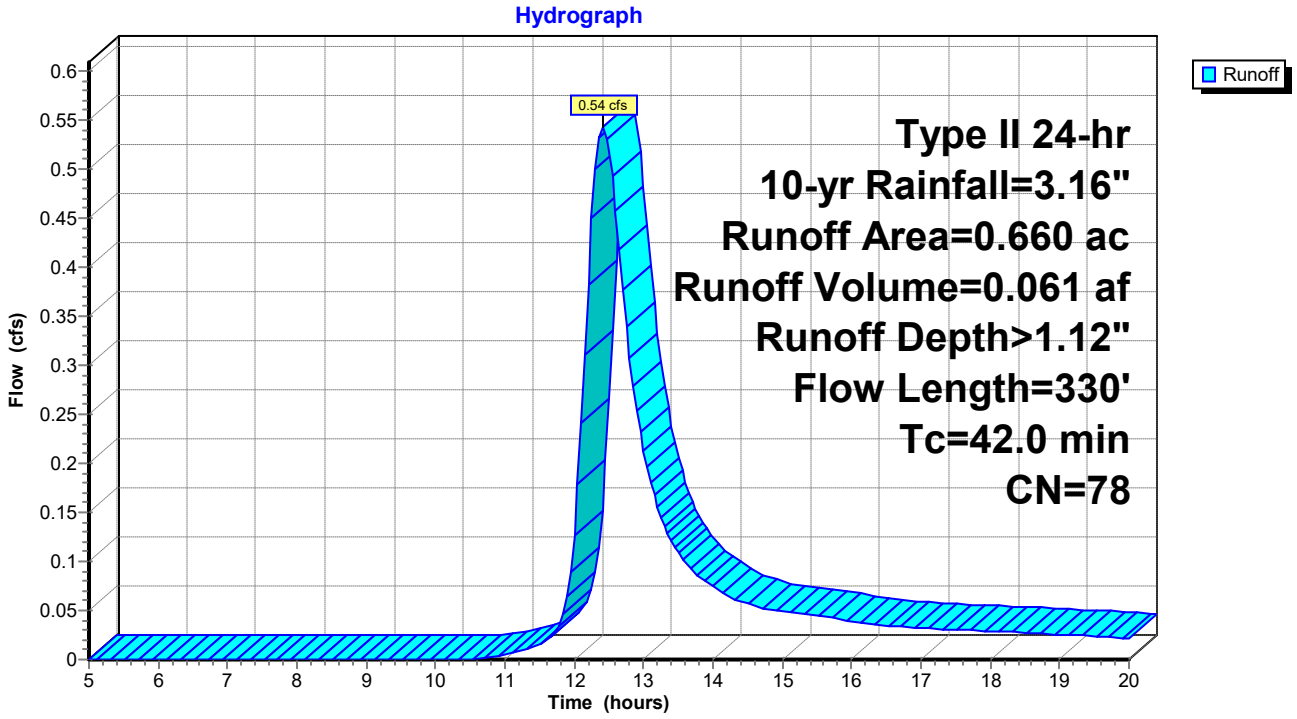
Runoff = 0.54 cfs @ 12.41 hrs, Volume= 0.061 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.660	78	Meadow, non-grazed, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
42.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

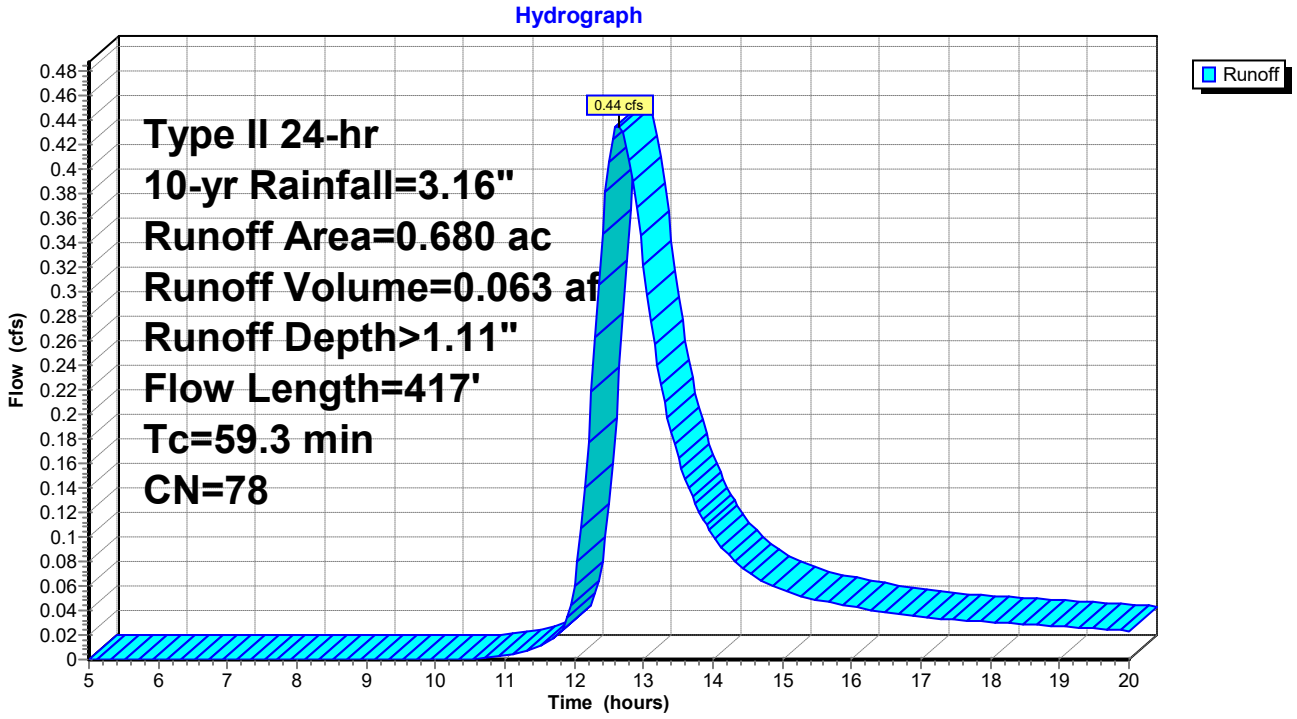
Runoff = 0.44 cfs @ 12.64 hrs, Volume= 0.063 af, Depth> 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.680	78	Meadow, non-grazed, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.0	150	0.0300	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
59.3	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

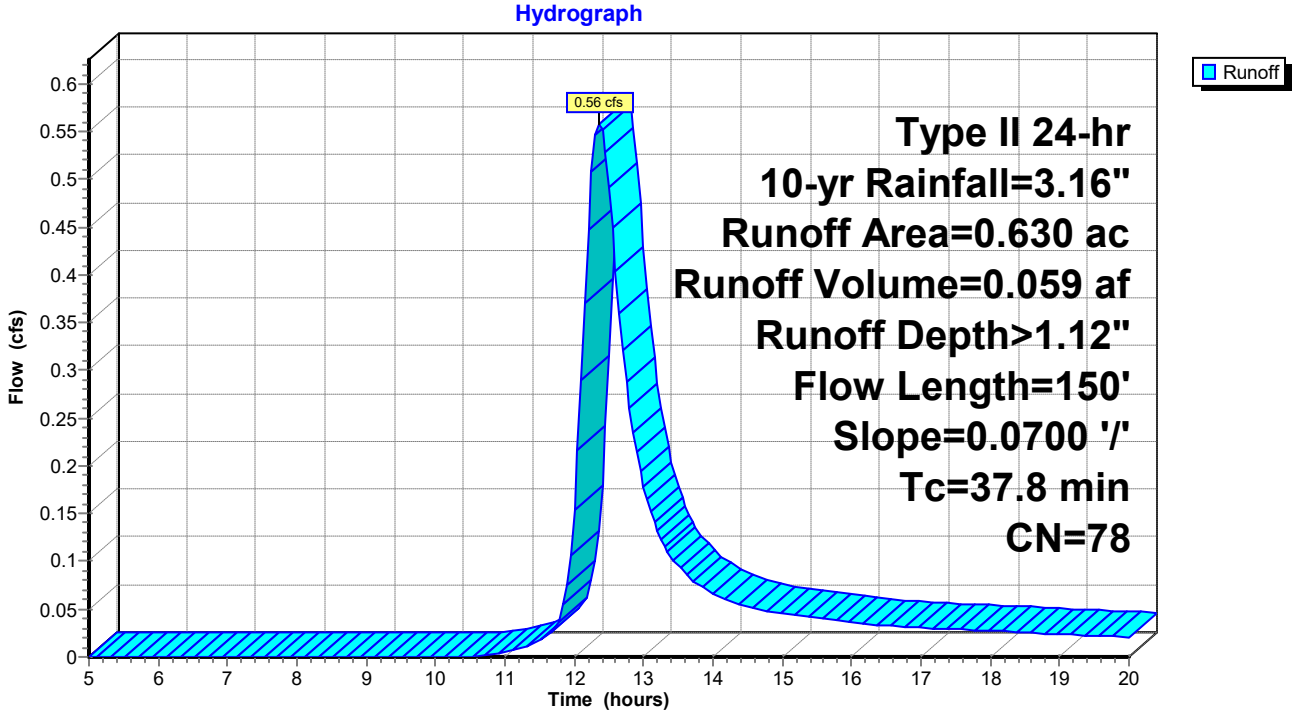
Runoff = 0.56 cfs @ 12.36 hrs, Volume= 0.059 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.630	78	Meadow, non-grazed, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

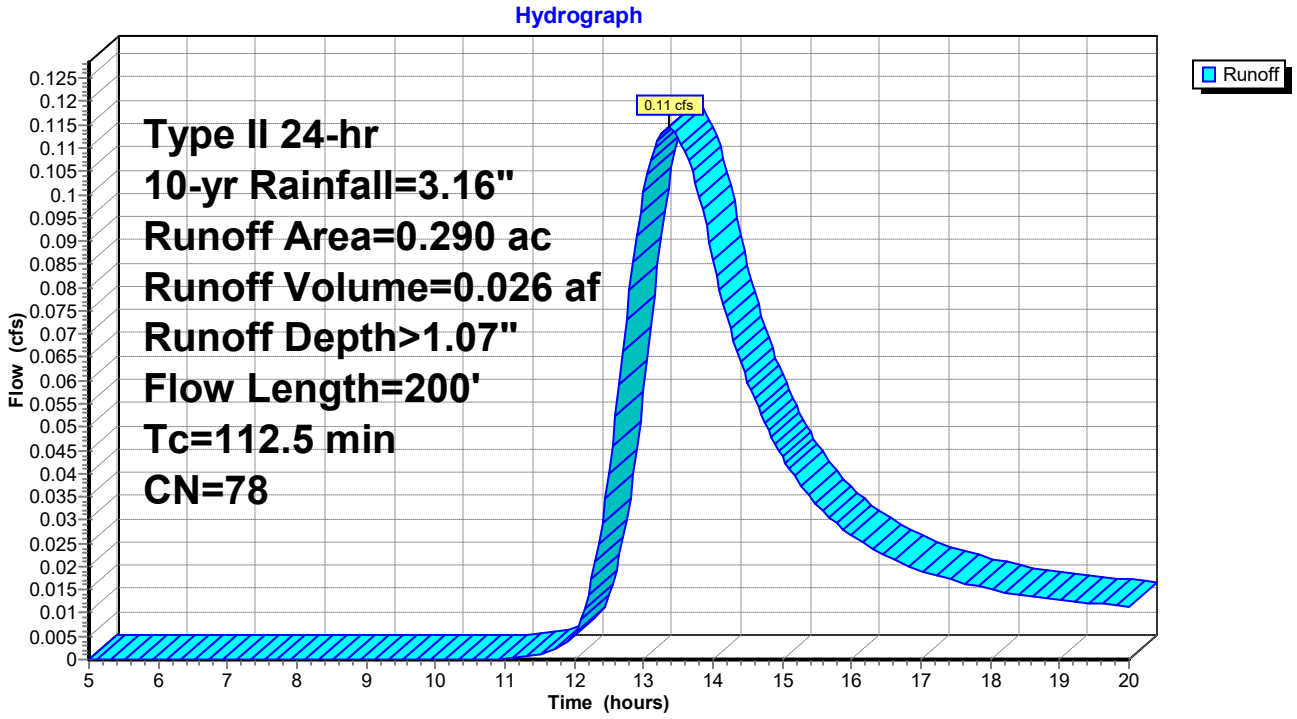
Runoff = 0.11 cfs @ 13.37 hrs, Volume= 0.026 af, Depth> 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

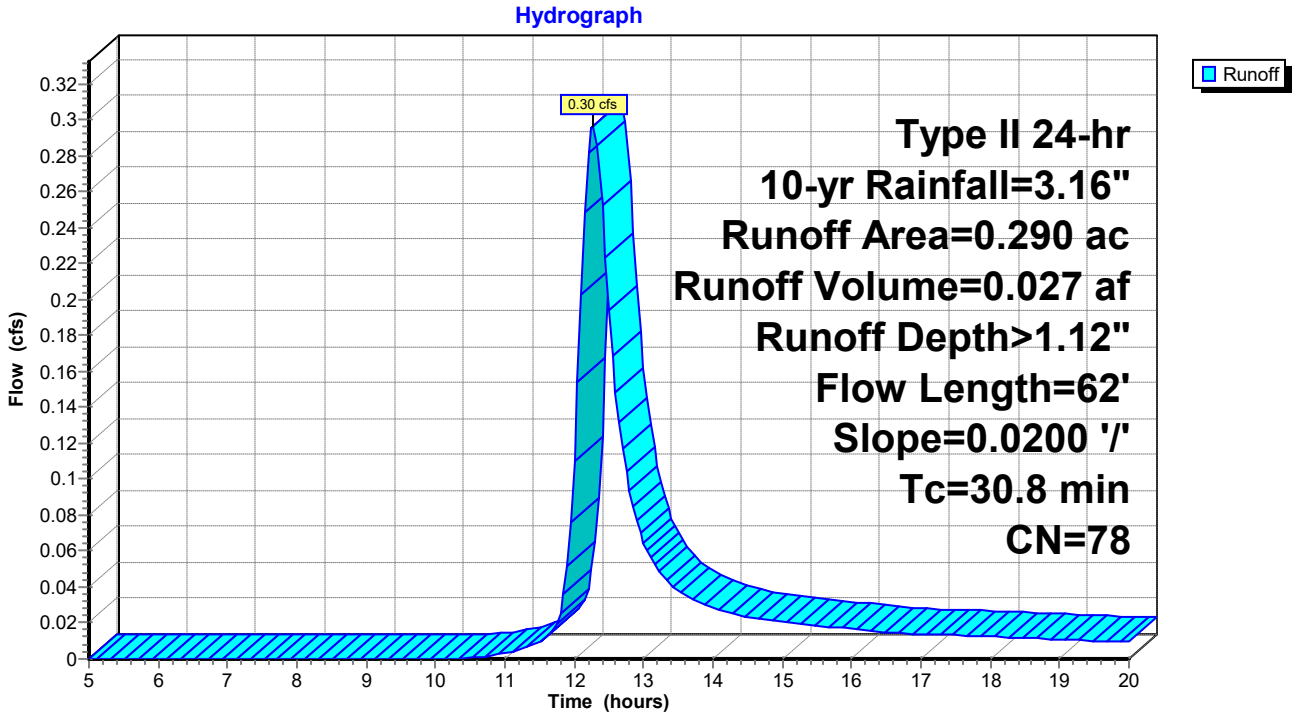
Runoff = 0.30 cfs @ 12.27 hrs, Volume= 0.027 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	62	0.0200	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

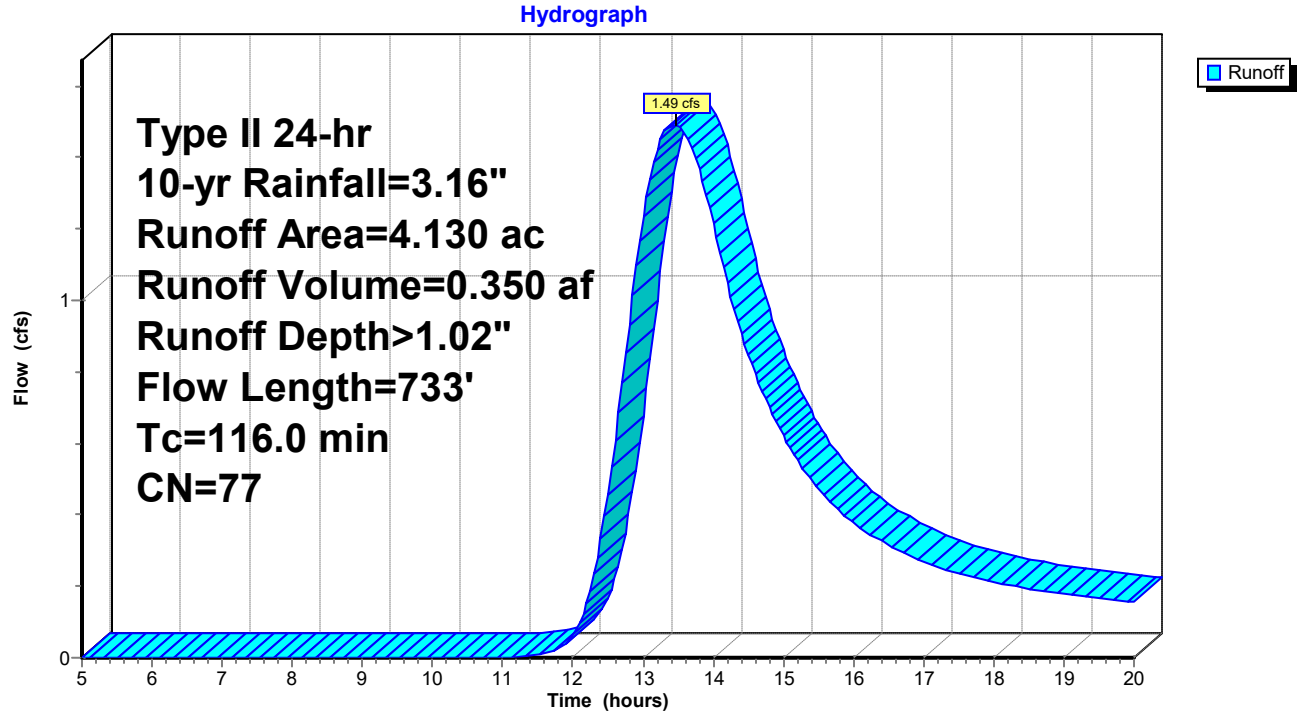
Runoff = 1.49 cfs @ 13.47 hrs, Volume= 0.350 af, Depth> 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	78	Meadow, non-grazed, HSG D
2.620	71	Meadow, non-grazed, HSG C
4.130	77	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
116.0	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

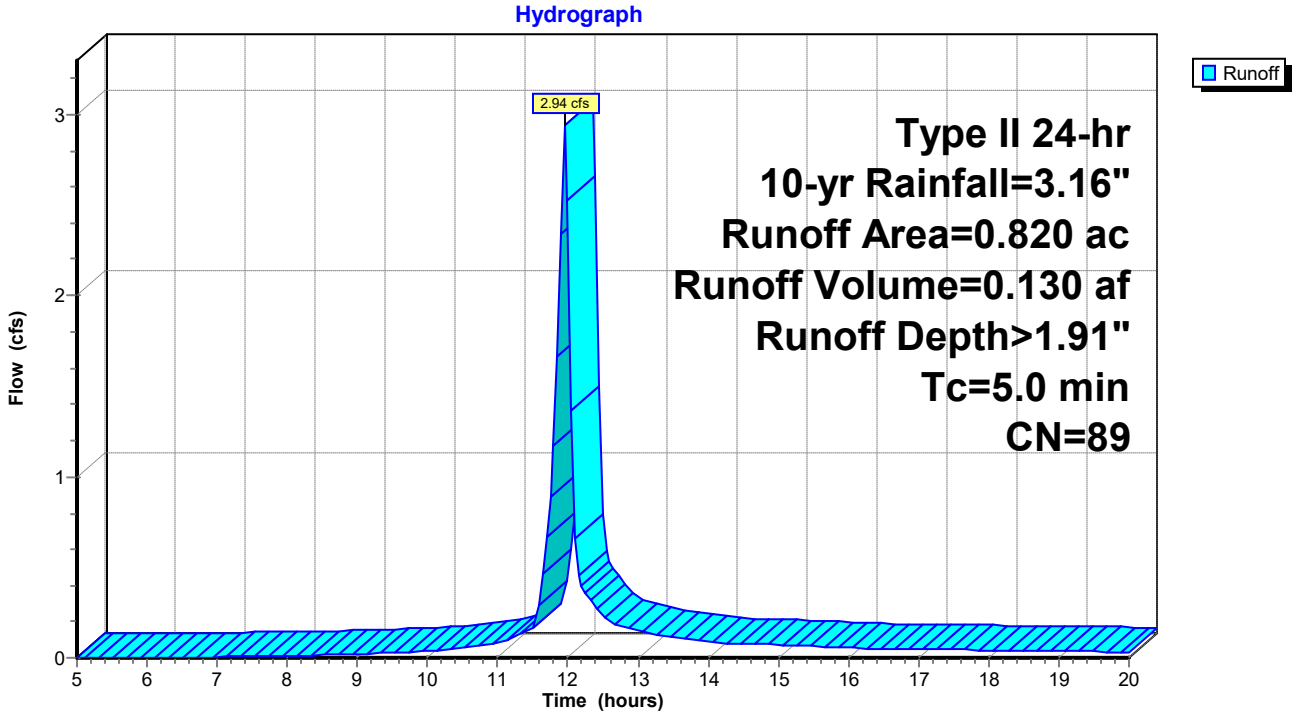
Runoff = 2.94 cfs @ 11.95 hrs, Volume= 0.130 af, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=3.16"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 01: DA-01 Runoff Area=58,712 sf 0.00% Impervious Runoff Depth>2.23"
Flow Length=235' Tc=48.8 min CN=78 Runoff=2.04 cfs 0.251 af

Subcatchment 02: DA-02 Runoff Area=2.610 ac 52.49% Impervious Runoff Depth>3.17"
Flow Length=916' Tc=7.6 min CN=88 Runoff=14.03 cfs 0.689 af

Subcatchment 03: DA-03 Runoff Area=0.180 ac 0.00% Impervious Runoff Depth>2.25"
Flow Length=100' Slope=0.0650 '/' Tc=28.1 min CN=78 Runoff=0.40 cfs 0.034 af

Subcatchment 04: DA-04 Runoff Area=0.160 ac 0.00% Impervious Runoff Depth>2.24"
Flow Length=100' Slope=0.0300 '/' Tc=38.3 min CN=78 Runoff=0.29 cfs 0.030 af

Subcatchment 05: DA-05 Runoff Area=4.660 ac 0.00% Impervious Runoff Depth>2.16"
Flow Length=494' Tc=115.6 min CN=78 Runoff=3.70 cfs 0.841 af

Subcatchment 06: DA-06 Runoff Area=0.500 ac 0.00% Impervious Runoff Depth>2.25"
Flow Length=179' Tc=32.5 min CN=78 Runoff=1.01 cfs 0.094 af

Subcatchment 07: DA-07 Runoff Area=0.660 ac 0.00% Impervious Runoff Depth>2.24"
Flow Length=330' Tc=42.0 min CN=78 Runoff=1.11 cfs 0.123 af

Subcatchment 08: DA-08 Runoff Area=0.680 ac 0.00% Impervious Runoff Depth>2.22"
Flow Length=417' Tc=59.3 min CN=78 Runoff=0.90 cfs 0.126 af

Subcatchment 09: DA-09 Runoff Area=0.630 ac 0.00% Impervious Runoff Depth>2.24"
Flow Length=150' Slope=0.0700 '/' Tc=37.8 min CN=78 Runoff=1.14 cfs 0.118 af

Subcatchment 10: DA-10 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>2.17"
Flow Length=200' Tc=112.5 min CN=78 Runoff=0.24 cfs 0.052 af

Subcatchment 11: DA-11 Runoff Area=0.290 ac 0.00% Impervious Runoff Depth>2.25"
Flow Length=62' Slope=0.0200 '/' Tc=30.8 min CN=78 Runoff=0.60 cfs 0.054 af

Subcatchment 12: DA-12 Runoff Area=4.130 ac 18.16% Impervious Runoff Depth>2.08"
Flow Length=733' Tc=116.0 min CN=77 Runoff=3.14 cfs 0.717 af

Subcatchment 13: DA-13 Runoff Area=0.820 ac 51.22% Impervious Runoff Depth>3.27"
Tc=5.0 min CN=89 Runoff=4.87 cfs 0.223 af

Total Runoff Area = 16.958 ac Runoff Volume = 3.351 af Average Runoff Depth = 2.37"
85.02% Pervious = 14.418 ac 14.98% Impervious = 2.540 ac

Summary for Subcatchment 01: DA-01

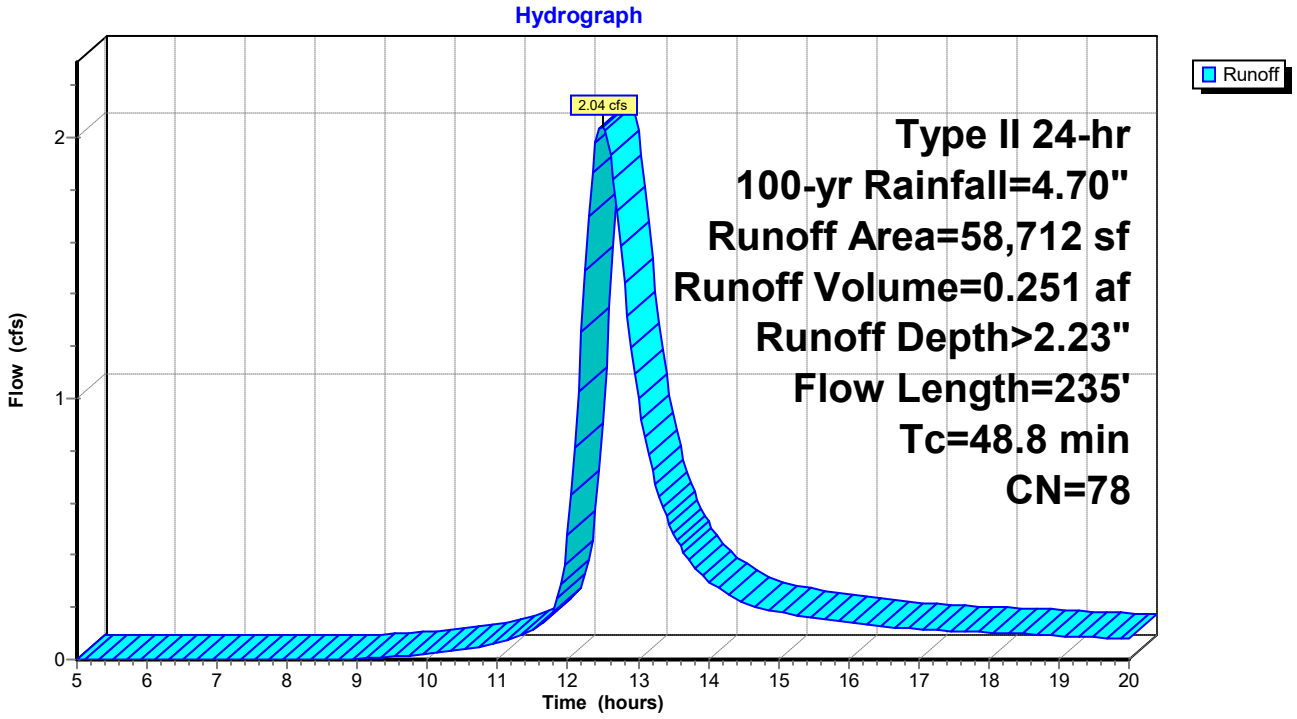
Runoff = 2.04 cfs @ 12.49 hrs, Volume= 0.251 af, Depth> 2.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (sf)	CN	Description
58,712	78	Meadow, non-grazed, HSG D
58,712		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.3	150	0.0400	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.5	85	0.0350	0.94		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
48.8	235	Total			

Subcatchment 01: DA-01



Summary for Subcatchment 02: DA-02

[47] Hint: Peak is 113% of capacity of segment #5

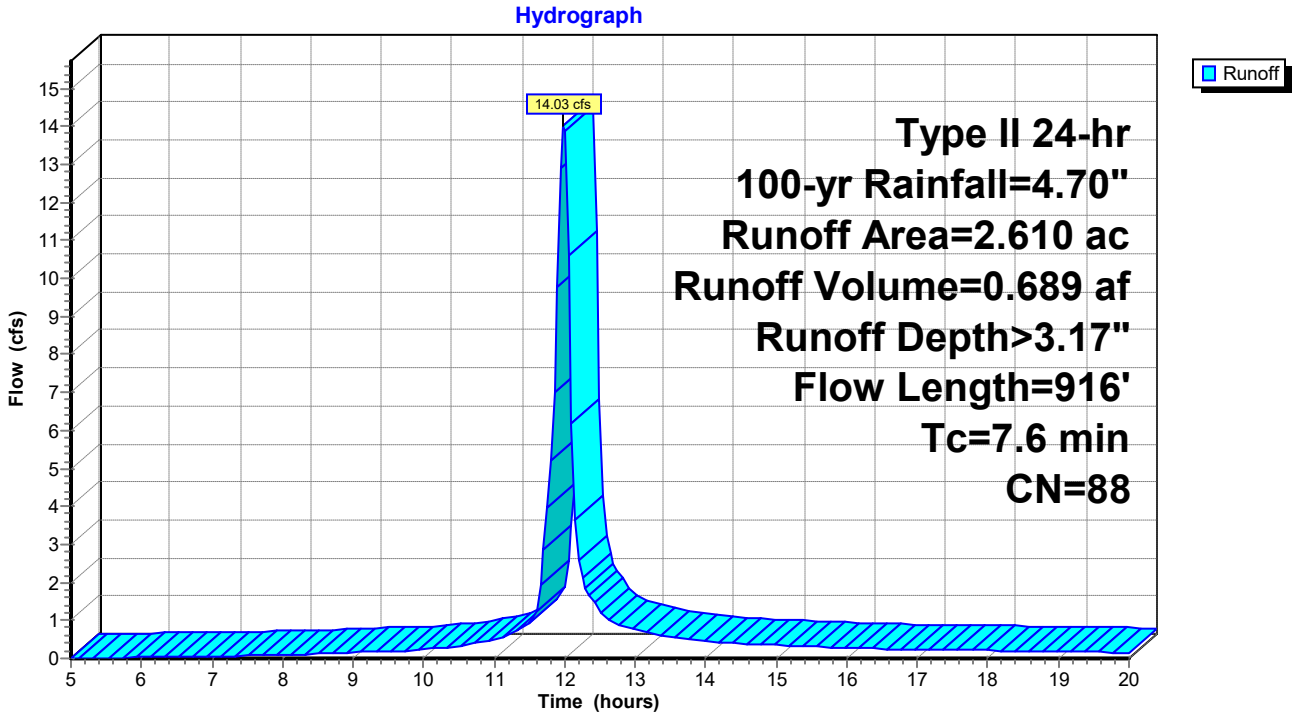
Runoff = 14.03 cfs @ 11.99 hrs, Volume= 0.689 af, Depth> 3.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
1.240	78	Meadow, non-grazed, HSG D
1.370	98	Paved parking, HSG D
2.610	88	Weighted Average
1.240		47.51% Pervious Area
1.370		52.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	65	0.0300	1.23		**Sheet Flow,** Smooth surfaces n= 0.011 P2= 2.20"
0.1	38	0.1000	6.42		**Shallow Concentrated Flow,** Paved Kv= 20.3 fps
1.6	326	0.0100	3.50	15.77	**Channel Flow,** Area= 4.5 sf Perim= 6.0' r= 0.75' n= 0.035 Earth, dense weeds
4.0	305	0.0070	1.27	60.94	**Channel Flow,** Area= 48.0 sf Perim= 65.0' r= 0.74' n= 0.080 Earth, dense brush, high stage
0.1	40	0.0100	7.03	12.41	**Pipe Channel,** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.011 Concrete pipe, finished
0.9	142	0.0200	2.61	65.15	**Channel Flow,** Area= 25.0 sf Perim= 25.3' r= 0.99' n= 0.080 Earth, dense brush, high stage
7.6	916	Total			

Subcatchment 02: DA-02



Summary for Subcatchment 03: DA-03

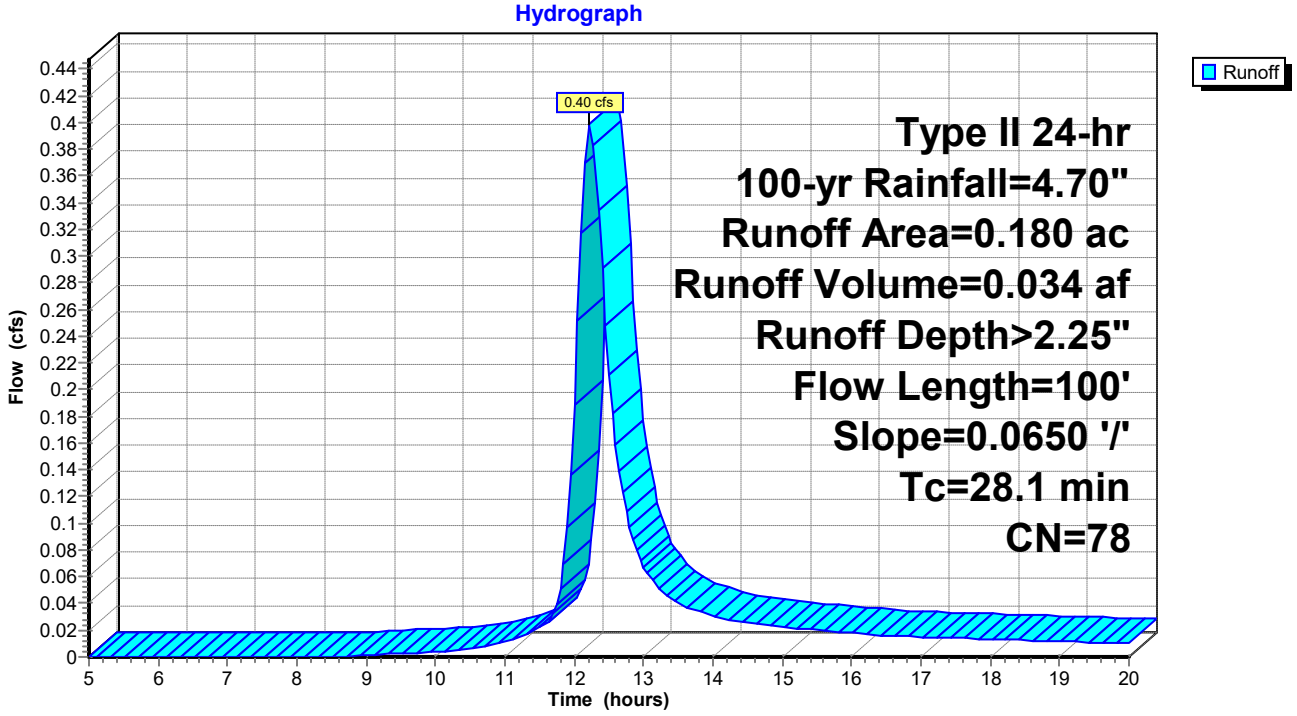
Runoff = 0.40 cfs @ 12.22 hrs, Volume= 0.034 af, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.180	78	Meadow, non-grazed, HSG D
0.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.1	100	0.0650	0.06		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 03: DA-03



Summary for Subcatchment 04: DA-04

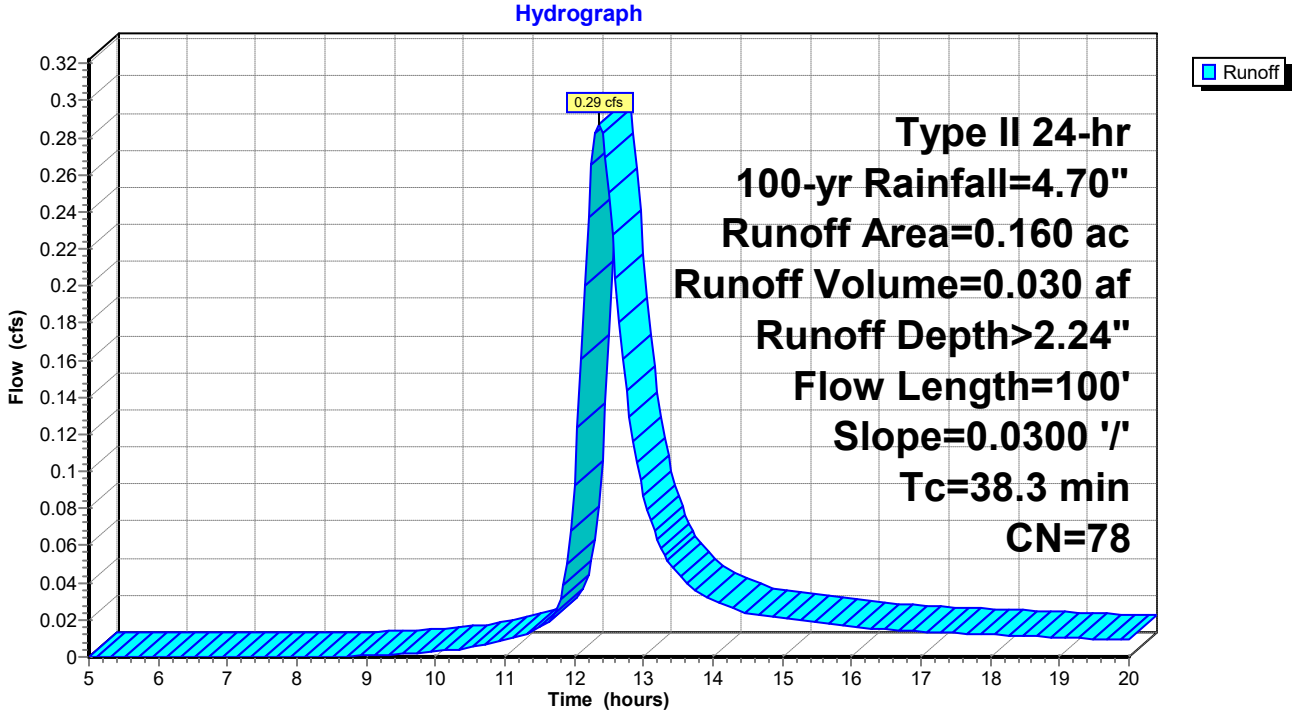
Runoff = 0.29 cfs @ 12.35 hrs, Volume= 0.030 af, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.160	78	Meadow, non-grazed, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.3	100	0.0300	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 04: DA-04



Summary for Subcatchment 05: DA-05

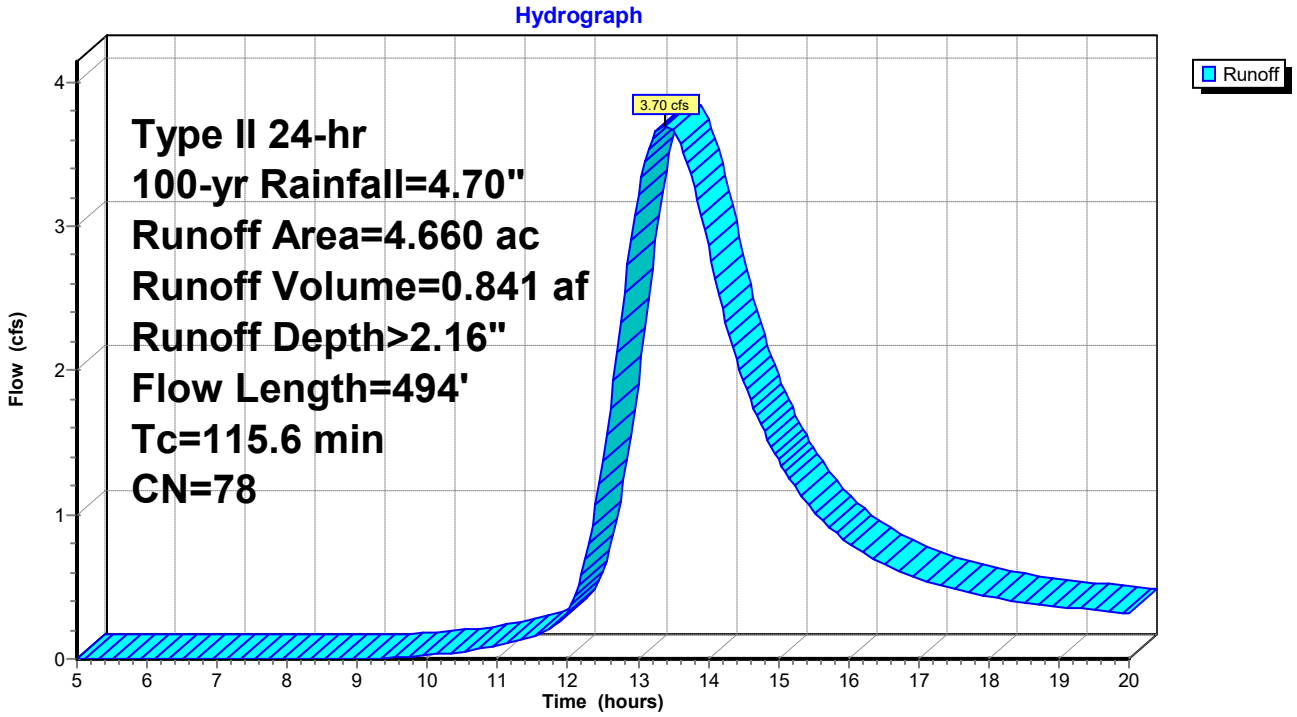
Runoff = 3.70 cfs @ 13.38 hrs, Volume= 0.841 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
4.660	78	Meadow, non-grazed, HSG D
4.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
12.9	194	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
7.8	150	0.0300	0.32	21.23	**Channel Flow,** Area= 66.0 sf Perim= 66.0' r= 1.00' n= 0.800 Sheet flow: Woods+dense brush
115.6	494	Total			

Subcatchment 05: DA-05



Summary for Subcatchment 06: DA-06

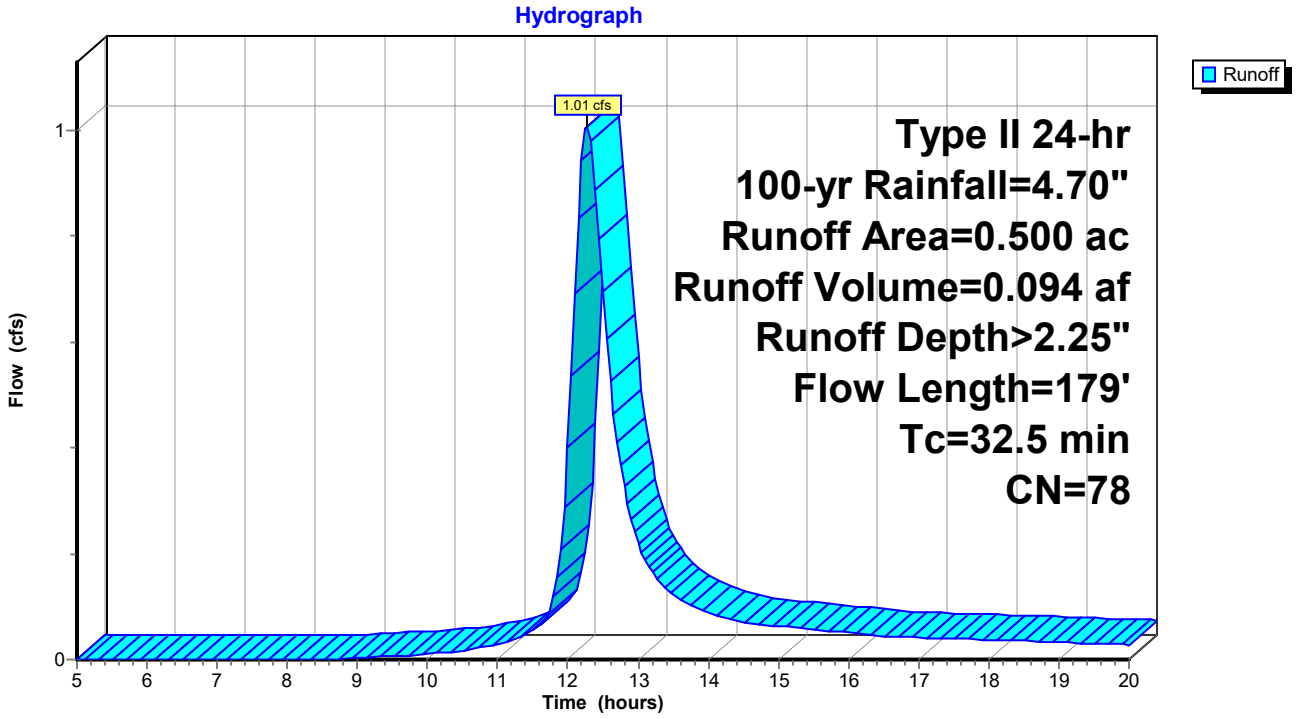
Runoff = 1.01 cfs @ 12.28 hrs, Volume= 0.094 af, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.500	78	Meadow, non-grazed, HSG D
0.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.6	120	0.0900	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
3.9	59	0.0100	0.25		**Shallow Concentrated Flow,** Forest w/Heavy Litter Kv= 2.5 fps
32.5	179	Total			

Subcatchment 06: DA-06



Summary for Subcatchment 07: DA-07

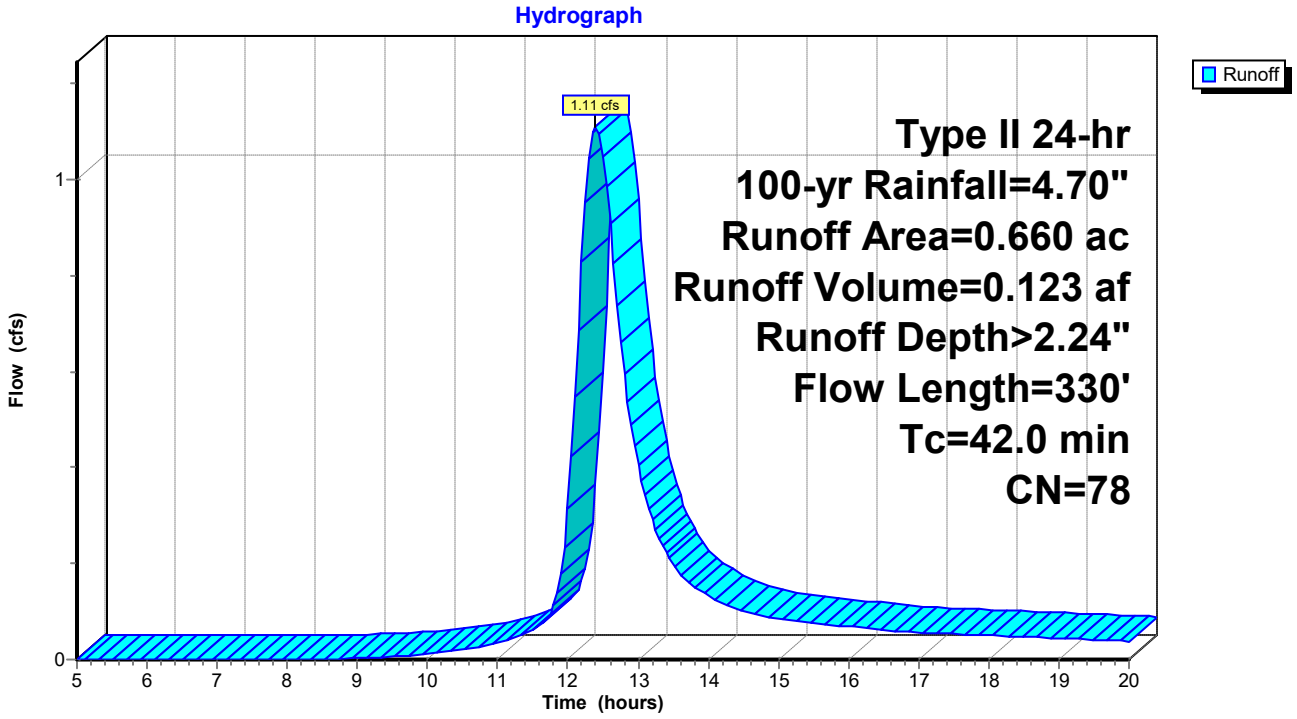
Runoff = 1.11 cfs @ 12.40 hrs, Volume= 0.123 af, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.660	78	Meadow, non-grazed, HSG D
0.660		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
4.2	180	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
42.0	330	Total			

Subcatchment 07: DA-07



Summary for Subcatchment 08: DA-08

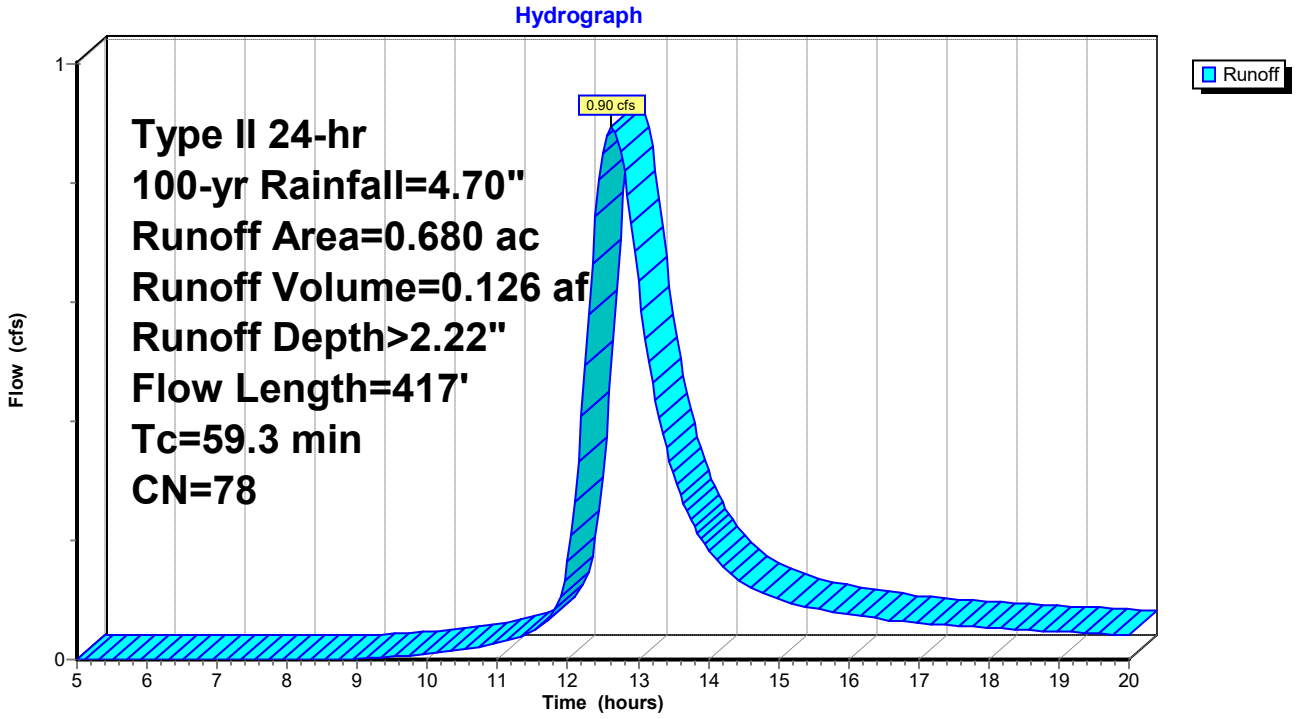
Runoff = 0.90 cfs @ 12.62 hrs, Volume= 0.126 af, Depth> 2.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.680	78	Meadow, non-grazed, HSG D
0.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.0	150	0.0300	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
6.3	267	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
59.3	417	Total			

Subcatchment 08: DA-08



Summary for Subcatchment 09: DA-09

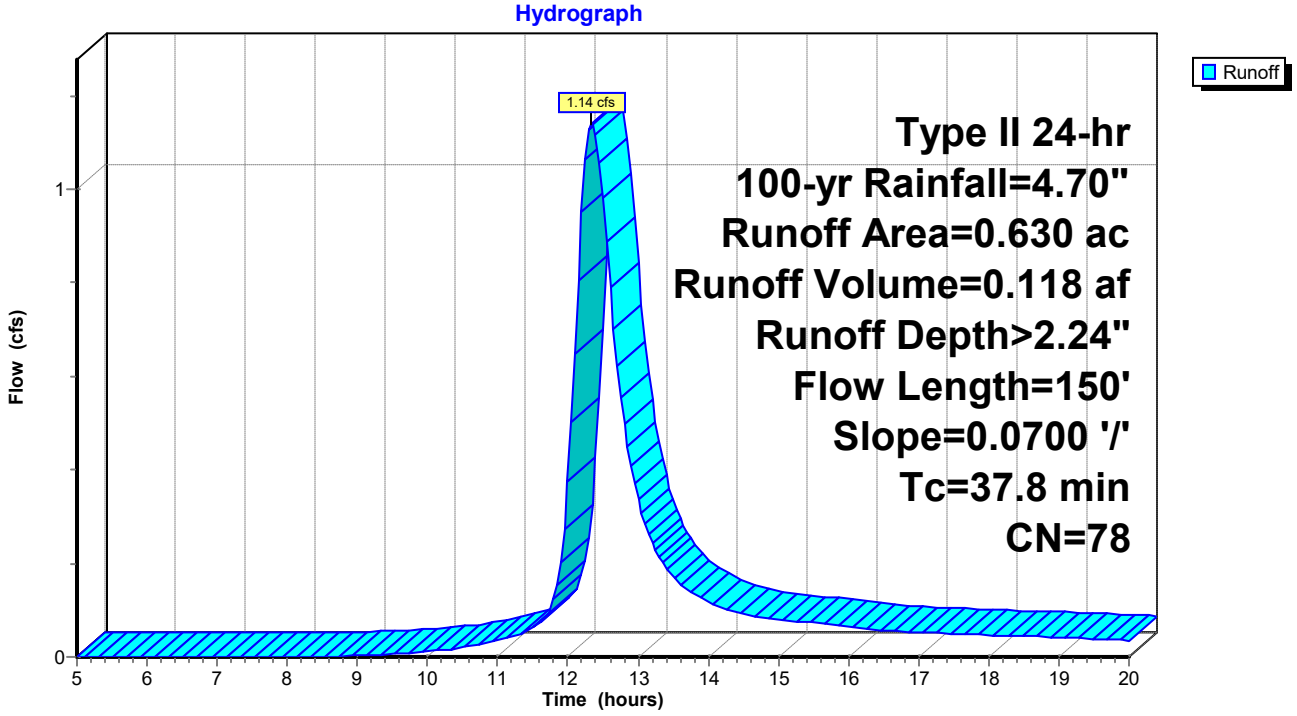
Runoff = 1.14 cfs @ 12.34 hrs, Volume= 0.118 af, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.630	78	Meadow, non-grazed, HSG D
0.630		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.8	150	0.0700	0.07		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 09: DA-09



Summary for Subcatchment 10: DA-10

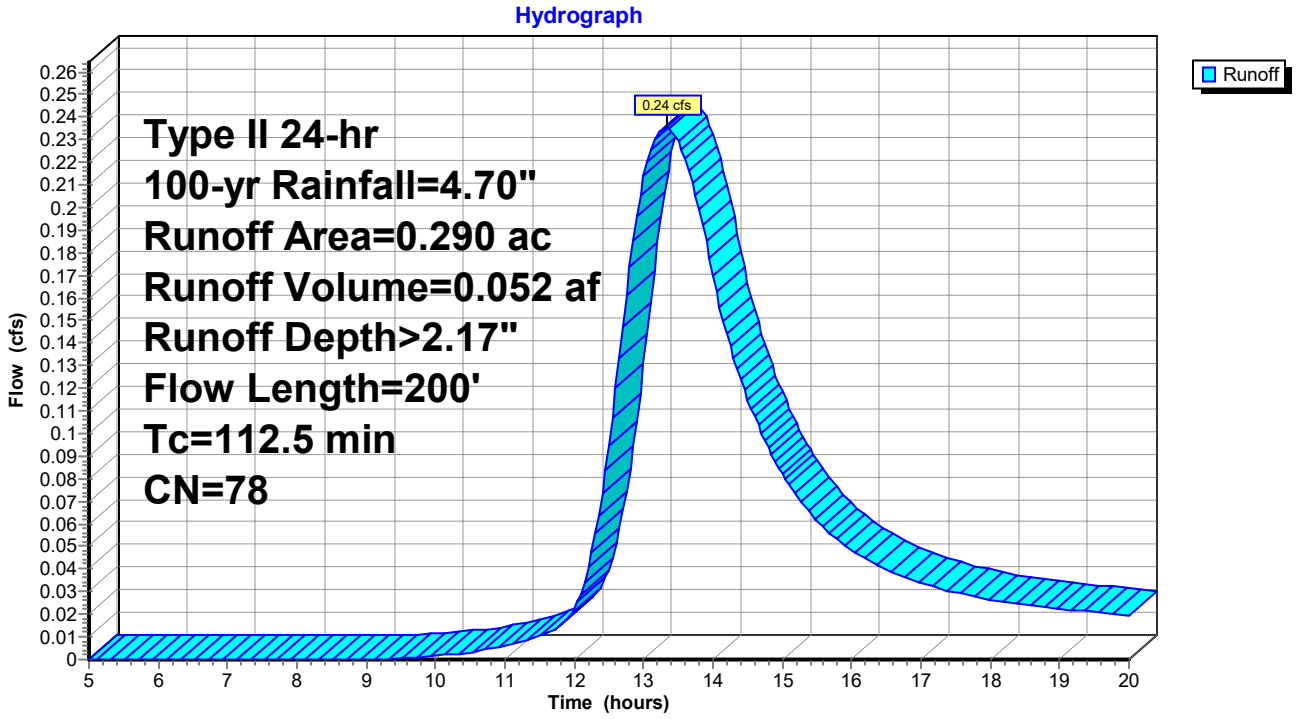
Runoff = 0.24 cfs @ 13.34 hrs, Volume= 0.052 af, Depth> 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
111.4	120	0.0030	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
1.1	80	0.0600	1.22		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
112.5	200	Total			

Subcatchment 10: DA-10



Summary for Subcatchment 11: DA-11

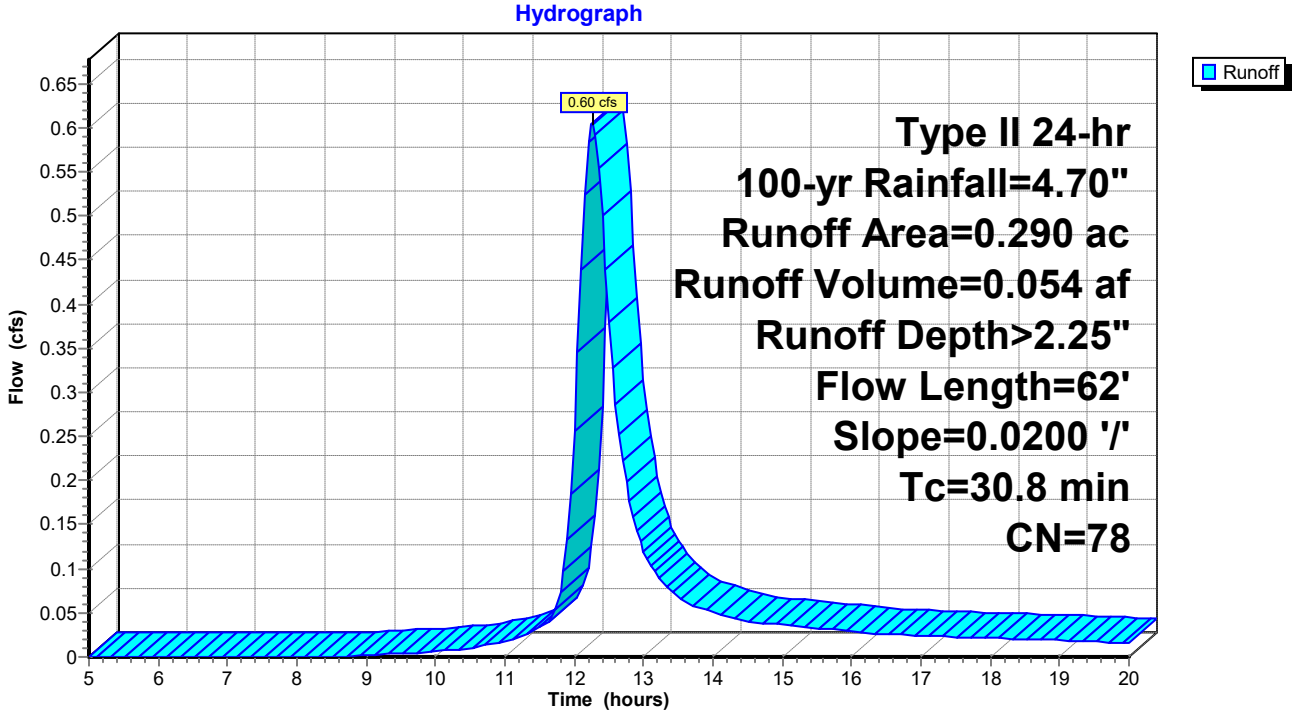
Runoff = 0.60 cfs @ 12.26 hrs, Volume= 0.054 af, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.290	78	Meadow, non-grazed, HSG D
0.290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.8	62	0.0200	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"

Subcatchment 11: DA-11



Summary for Subcatchment 12: DA-12

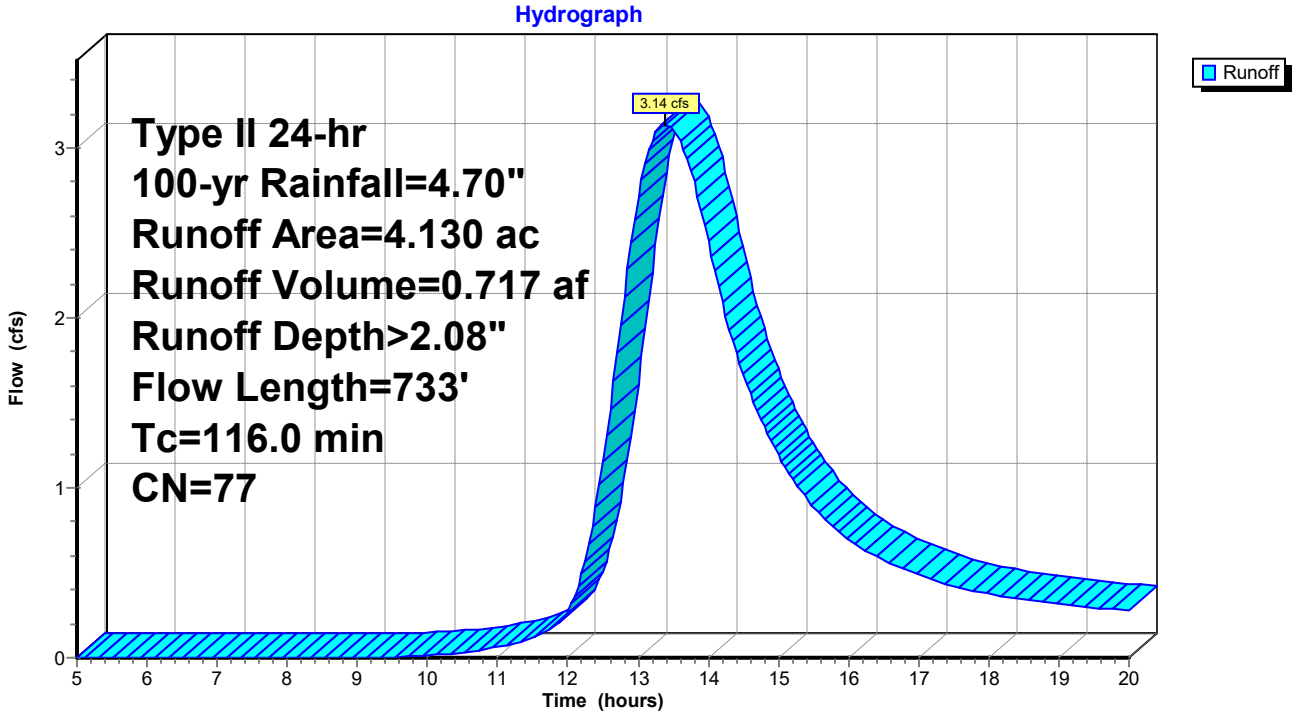
Runoff = 3.14 cfs @ 13.39 hrs, Volume= 0.717 af, Depth> 2.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.750	98	Paved parking, HSG D
0.760	78	Meadow, non-grazed, HSG D
2.620	71	Meadow, non-grazed, HSG C
4.130	77	Weighted Average
3.380		81.84% Pervious Area
0.750		18.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
94.9	150	0.0070	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.20"
9.9	297	0.0100	0.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
11.2	286	0.0030	0.43	7.24	**Channel Flow,** Area= 17.0 sf Perim= 34.2' r= 0.50' n= 0.120 Earth, long dense weeds
116.0	733	Total			

Subcatchment 12: DA-12



Summary for Subcatchment 13: DA-13

[49] Hint: Tc<2dt may require smaller dt

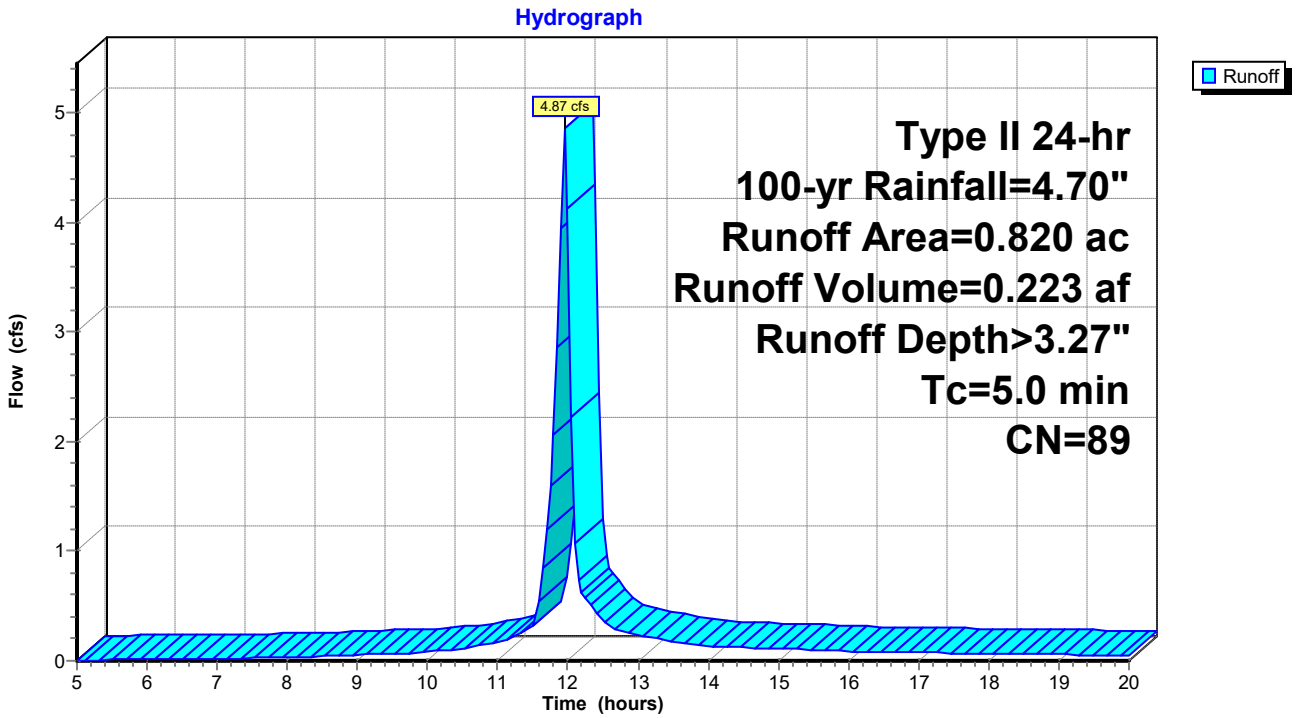
Runoff = 4.87 cfs @ 11.95 hrs, Volume= 0.223 af, Depth> 3.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=4.70"

Area (ac)	CN	Description
0.420	98	Paved parking, HSG D
0.400	79	Woods/grass comb., Good, HSG D
0.820	89	Weighted Average
0.400		48.78% Pervious Area
0.420		51.22% Impervious Area

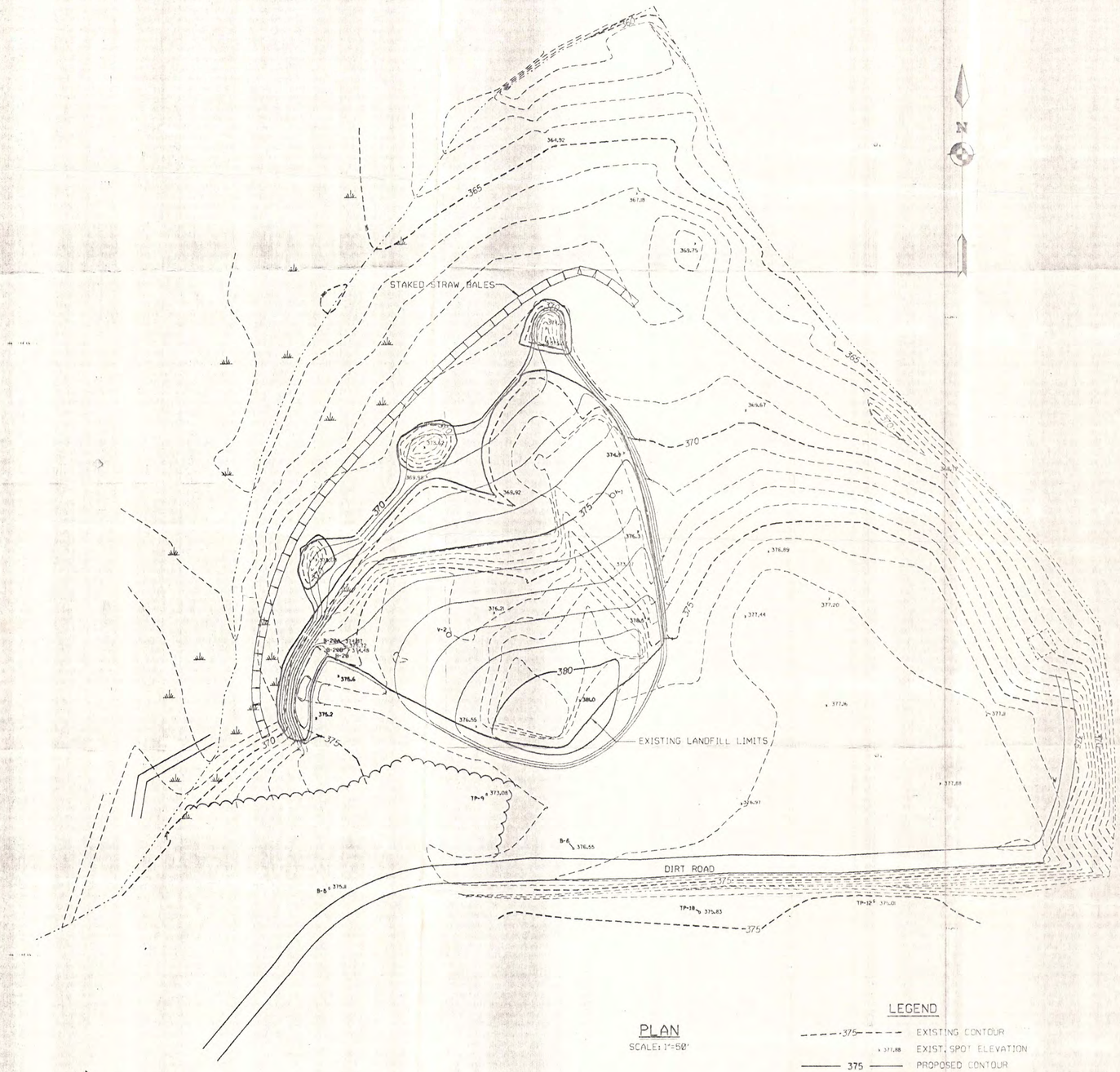
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13: DA-13



APPENDIX F
C&D LANDFILL CLOSURE PLAN DRAWING

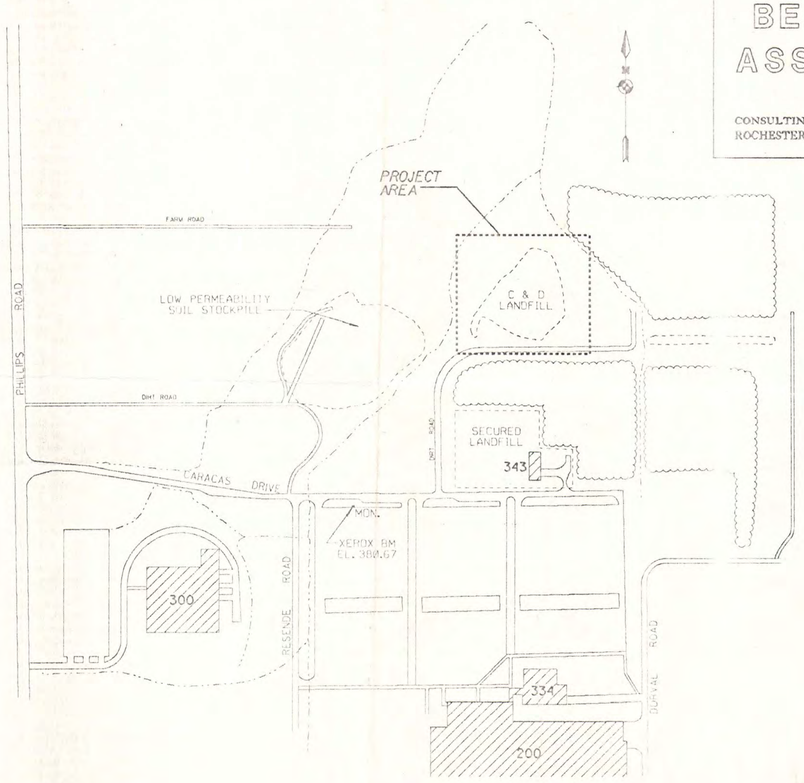
BERGMANN ASSOCIATES
CONSULTING ENGINEERS & PLANNERS
ROCHESTER NEW YORK



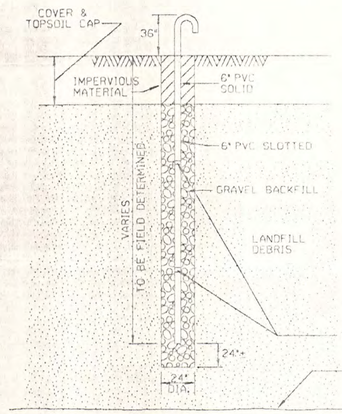
PLAN
SCALE: 1"=50'

LEGEND

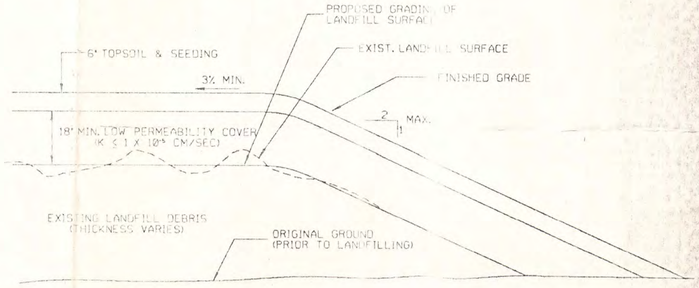
- EXISTING CONTOUR
- EXIST. SPOT ELEVATION
- PROPOSED CONTOUR
- PROPOSED SPOT ELEVATION
- EXIST. MONITORING WELL
- PROPOSED PVC VENT PIPE
- EXIST. LANDFILL LIMITS
- STAKED STRAW BALES



LOCATION PLAN
NO SCALE



LANDFILL VENT DETAIL
NO SCALE



TYPICAL SECTION
NO SCALE

STATE OF NEW YORK
KENNETH R. AVERY
No. 058243
LICENSED PROFESSIONAL ENGINEER
12/14/88

ISSUE	DATE	REVISIONS	BY

XEROX FACILITIES OPERATIONS
JOSEPH C. WILSON CENTER FOR TECHNOLOGY
ROCHESTER, NEW YORK 14644

LAND/SITE
C & D LANDFILL CLOSURE
PLAN & DETAILS

DRAWN BY	DATE	SHEET NO	PROJECT NO
DMB	12-5-88	1 OF 1	
CHK'D BY / DATE	CHK'D / DATE		
APPR'D BY / DATE	APPR'D / DATE	ENVIRONMENTAL APPR'D / DATE	DRAWING NO / ISSUE
SCALE	FILE	ENTRY NO	L-1 / A
AS NOTED			

ORIENTATION

APPENDIX G
C&D LANDFILL COVER INVESTIGATION REPORT

H & A OF NEW YORK ENGINEERING
AND GEOLOGY, LLP
260 E. Main Street
Suite 2100
Rochester, NY 14604
585.359.9000

Revised February 6, 2026
December 11, 2025
File No. 0134970-105

Xerox Corporation
800 Phillips Road
Webster, New York 14580

Attention: Marcus Lathrop
Manager, Assessment & Environmental Operations

SUBJECT: C&D Landfill Soil Cover Assessment
Xerox Corporation Facility – Webster, New York
NYSDEC Permit No.: 8-2654-00064/00040
DER Site ID No. 828178
US EPA ID No. NYD002211324

Dear Mr. Lathrop,

H & A of New York Engineering and Geology, LLP (Haley & Aldrich of New York) has prepared this letter report to summarize the landfill cover assessment of the capped and closed Construction and Demolition (C&D) Landfill at the Xerox Corporation (Xerox) facility located at 800 Phillips Road in Webster, New York (Site). The sections below describe the background, landfill soil cover assessment objective and field activities, grain size analysis, and Community Air Monitoring Plan (CAMP) results.

Background

Xerox and TM Montante Solar Development LLC (Montante) are in the process of developing a commercial-scale solar photovoltaic facility north of Caracas Drive, a portion of which will be within the footprint of the closed and capped C&D Landfill. The landfill was closed under direction of the NYSDEC in 1988 and is designated Solid Waste Management Unit (SWMU) #88 – Former Disposal Area North of W200 and described as C&D Landfill under the Xerox Part 373 RCRA Permit; this SWMU is classified as No Further Action Required. Landfill closure plans and details were prepared by Bergmann Associates, and a drawing is included in Attachment A for reference.

In June 2025, the Site submitted a 60-Day Advance Notification of Site Change of Use to NYSDEC. Following preliminary review, Xerox and NYSDEC met to discuss the proposed construction and operation of the solar project. At this time, the NYSDEC requested additional assessment of the soil cover of this closed C&D Landfill at the Site prior to the solar project development construction related activities.

Objective of C&D Landfill Cover Assessment

The assessment of the landfill cover was conducted on October 30, 2025, to evaluate the soil condition of the cover at the C&D Landfill at the Site including soil type, estimated hydraulic conductivity, and cover thickness. The landfill closure and cover details on the Attachment A, C&D Landfill closure design drawing (Bergmann Associates, 1988) included grading of the landfill surface, installation of a minimum 18-inch thick soil cover consisting of low permeability soils (hydraulic conductivity (K) ≤ 1×10^{-5} centimeters per second (cm/sec) and 6 inches of overlying topsoil to support vegetative cover graded to promote surface water runoff.

C&D Landfill Cover Assessment Field Activities

Five test pits (TP-01 through TP-05) were hand excavated to 2 feet below ground surface (bgs) within the C&D Landfill extent (Figure 1). The limits of the C&D Landfill have been approximated based on interpolation of existing ground surface topography provided by Montante, in correlation with review of the landfill closure design and visual review conducted in the field. The soil stratigraphy was visually assessed for each test pit, and composition and cover thickness recorded. The test pit logs and associated photographs from the site assessment are included in Attachments B and C, respectively.

A topsoil layer to a depth of approximately 6 inches was observed at each test pit. A soil sample was collected between approximately 6 inches and 24 inches from the underlying cover soil (below topsoil) at each location and submitted to CME Associates, Inc. in East Syracuse, New York, a certified geotechnical materials testing laboratory. The samples were submitted for grain size analysis in accordance with ASTM D422. The lab report is included in Attachment D.

Following sample collection, each test pit location was backfilled with the excavated material and hand-tamped for compaction. The soil was replaced in kind, with the 18-inches of subsoil replaced first and then the 6-inches of topsoil placed back at the surface on top of the subsoil. Each location was surveyed with a handheld global positioning system (GPS) unit and approximate locations were incorporated into Figure 1**.**

C&D Landfill Cover Assessment Grain Size Analysis

The soil cover was determined to be primarily sandy silt and to a lesser extent silty sand at one test pit location, TP-02, based on the grain size distribution and field observations. Approximately 10 to 15 percent of the overall soil matrix was observed to be cobbles (larger than 3 inches). A stray brick fragment (4 inches) was observed at approximately 18 inches bgs at TP-02. Given the limited observation of this type of anthropogenic material, it may have been reworked into the cover soils during placement or introduced into the soil source material during handling or placement. No other non-native materials were observed within the soil matrix.

Based on the grain size distribution of the sandy silt or silty sand samples, the soil cover material is consistent with the target specification of hydraulic conductivity of the cover included in the C&D

Landfill closure design drawing (low-permeability soils, $K \leq 1 \times 10^{-5}$ cm/sec) [1]. The soil cover thickness was observed to meet the minimum thickness specification included in the C&D Landfill closure design drawing.

Air Monitoring

Hand-held air monitoring was completed at the test pit locations for personnel safety during the investigation. There was no detection of volatile organic compounds (VOCs) and no detections or deviations of four gas meter conditions (O2, CO, H2S or LEL) observed.

Continuous monitoring using community air monitoring instruments, including two TSI 8530 DustTrak II Aerosol Monitoring Units, were utilized during hand-excavation activities to comply with NYSDEC DER-10 guidance for community air monitoring during ground intrusive activities. VOCs were monitored continuously downwind and upwind. Vapor levels remained below work area/exclusion zone thresholds for 15-minute averages. Results from CAMP are included in Attachment E.

Conclusion

The SWMU #88 – Former Disposal Area North of W200, described as C&D Landfill, was previously closed and has No Further Action status in the Xerox Site NYSDEC Part 373 RCRA Permit. Given the previously closed, No Further Action status condition and the results of this investigation, we conclude the existing soil cover of the C&D Landfill is consistent with the design specifications for cover thickness and soils meeting the low-permeability hydraulic conductivity requirements as specified.

Sincerely yours,
H & A OF NEW YORK ENGINEERING AND GEOLOGY, LLP

Steven Phillips, P.G. (NY)
Senior Associate

Johanna Lang-Bentley, P.E. (NY)
Senior Technical Specialist

Enclosures:

- Report Certification
- Figure 1: Solar Project Cover Investigation Test Locations
- Attachment A: Bergmann Associates C&D Landfill Closure Plan & Details
- Attachment B: Test Pit Logs
- Attachment C: Photo Log
- Attachment D: Grain Size Analysis Lab Report
- Attachment E: Community Air Monitoring Results

[1] Soils grain size results were screened by the Hazen method empirical formula. *HydrogeoSieveXL: An Excel-based tool to estimate hydraulic conductivity from grain-size analysis, Hydrogeology Journal,* Devlin, J.F. (2015).

REPORT CERTIFICATION

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Consulting and Engineering Firm: H & A of New York Engineering and Geology, LLP

Name: Johanna Lang-Bentley, P.E. (NY)

Title: Senior Technical Specialist

Signature: Johanna Lang-Bentley

Date: February 24, 2026



REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Site Owner: Xerox Corporation

Name: John Barrett

Title: Manager - Western Hemisphere Real Estate Portfolio

Signature: 

Date: February 24, 2026

Docusign Envelope ID: 78989487-F70B-4024-ABAD-B6E1F7E1C3A8

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Operator (Project Developer): TM Montante Solar Development LLC

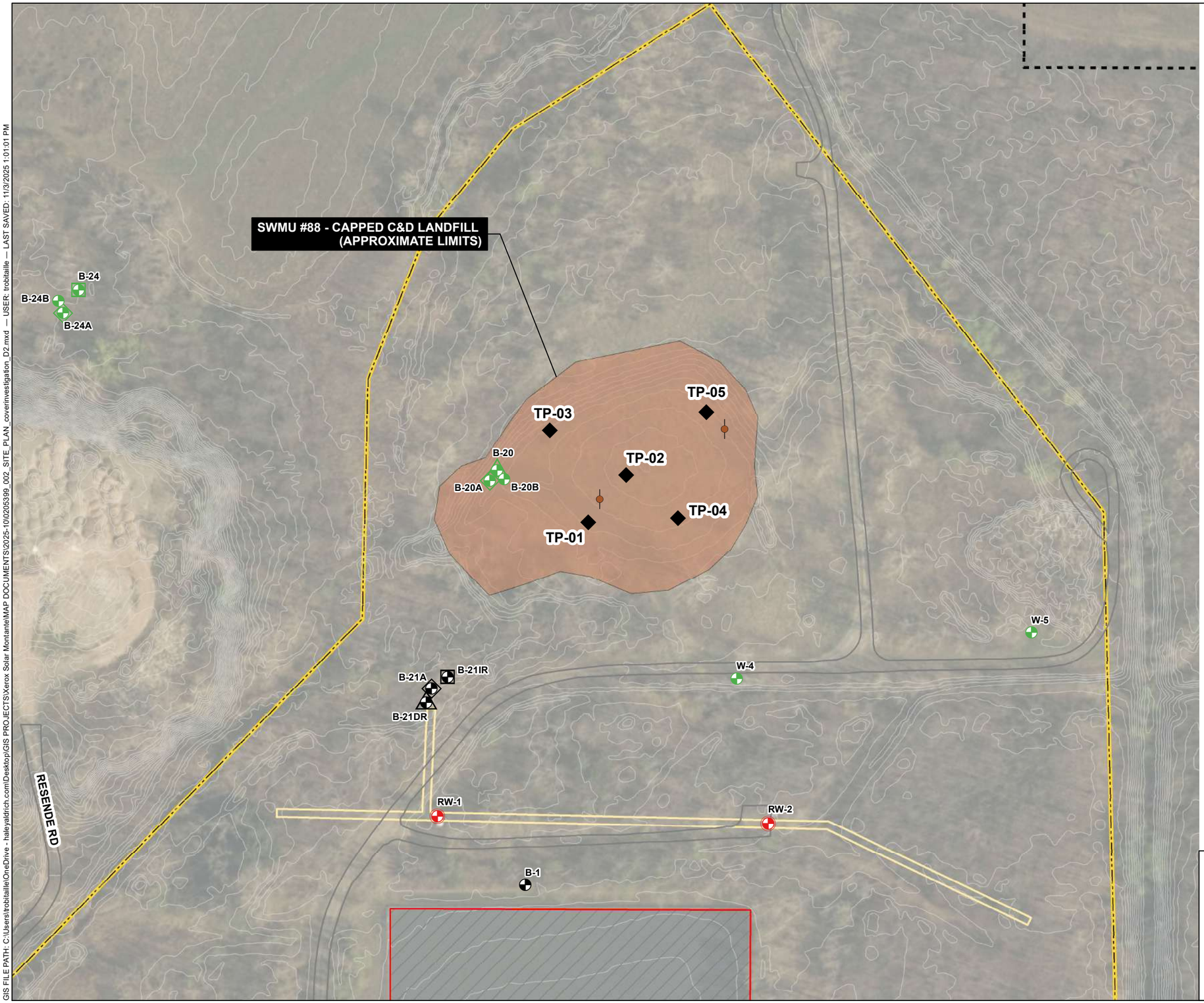
Name: Dan Montante

Title: Member

Signature: Signed by: Dan Montante 842D93661ABC439...

Date: 2/25/2026

FIGURE



LEGEND

- SOLAR PROJECT STUDY AREA
- TEST PIT LOCATIONS
- XEROX PROPERTY BOUNDARY
- 1 FT TOPOGRAPHIC CONTOURS
- APPROX EXTENTS OF C&D LANDFILL
- CAPPED INACTIVE LANDFILL
- LANDFILL GAS VENTS

OTHER SITE FEATURES

- GROUNDWATER PUMPING WELL
- GROUNDWATER QUALITY MONITORING (GQM) WELL
- TRIGGER WELL
- LINE OF COMPLIANCE WELL
- BLASTED TRENCH IN BEDROCK

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY
3. WELL LOCATION DATA SOURCE: XEROX CORPORATION
4. TOPOGRAPHIC DATA SOURCE: MONROE COUNTY LIDAR DOWNLOAD SITE

N

0 200

SCALE IN FEET

HALEY ALDRICH

TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

SOLAR PROJECT
COVER INVESTIGATION
TEST LOCATIONS

SCALE: AS SHOWN
LAST UPDATED: NOVEMBER 2025

FIGURE 1

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2025-10\0205399_002_SITE_PLAN_coverinvestigation_D2.mxd — USER: trobitaille — LAST SAVED: 11/3/2025 1:01:01 PM

ATTACHMENT A
Bergmann Associates C&D Landfill
Closure Plan & Details

BERGMANN ASSOCIATES
CONSULTING ENGINEERS & PLANNERS
ROCHESTER NEW YORK


STAKED STRAW BALES
EXISTING LANDFILL LIMITS
DIRT ROAD
364.92
365
367.18
369.75
369.67
370
374.9
369.58
376.92
375
376.5
376.89
377.44
377.20
376.21
376.0
380
380.0
376.55
377.26
377.88
376.97
373.08
376.55
375.8
375.83
377.08
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B-29DDDDDDDDDDDDDDDDDDDDDDDDDD
B-29EEEEEEEEEEEEEEEEEEEEEEEEEE
B-29FFFFFFFFFFFFFFFFFFFFFFFFFF
B-29GGGGGGGGGGGGGGGGGGGGGGGGGG
B-29HHHHHHHHHHHHHHHHHHHHHHHHHH
B-29IIIIIIIIIIIIIIIIIIIIIIIIII
B-29JJJJJJJJJJJJJJJJJJJJJJJJJJ
B-29KKKKKKKKKKKKKKKKKKKKKKKKKK
B-29LLLLLLLLLLLLLLLLLLLLLLLLLL
B-29MMMMMMMMMMMMMMMMMMMMMMMMMM
B-29NNNNNNNNNNNNNNNNNNNNNNNNNN
B-29OOOOOOOOOOOOOOOOOOOOOOOOOO
B-29PPPPPPPPPPPPPPPPPPPPPPPPPP
B-29QQQQQQQQQQQQQQQQQQQQQQQQQQ
B-29RRRRRRRRRRRRRRRRRRRRRRRRRR
B-29SSSSSSSSSSSSSSSSSSSSSSSSSS
B-29TTTTTTTTTTTTTTTTTTTTTTTTTT
B-29UUUUUUUUUUUUUUUUUUUUUUUUUU
B-29VVVVVVVVVVVVVVVVVVVVVVVVVV
B-29WWWWWWWWWWWWWWWWWWWWWWWWWW
B-29XXXXXXXXXXXXXXXXXXXXXXXXXX
B-29YYYYYYYYYYYYYYYYYYYYYYYYYY
B-29ZZZZZZZZZZZZZZZZZZZZZZZZZZ
B-29AAAAAAAAAAAAAAAAAAAAAAAAAAA
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B-29CCCCCCCCCCCCCCCCCCCCCCCCCCC
B-29DDDDDDDDDDDDDDDDDDDDDDDDDDD
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B-29FFFFFFFFFFFFFFFFFFFFFFFFFFF
B-29GGGGGGGGGGGGGGGGGGGGGGGGGGG
B-29HHHHHHHHHHHHHHHHHHHHHHHHHHH
B-29IIIIIIIIIIIIIIIIIIIIIIIIIII
B-29JJJJJJJJJJJJJJJJJJJJJJJJJJJ
B-29KKKKKKKKKKKKKKKKKKKKKKKKKKK
B-29LLLLLLLLLLLLLLLLLLLLLLLLLLL
B-29MMMMMMMMMMMMMMMMMMMMMMMMMMM
B-29NNNNNNNNNNNNNNNNNNNNNNNNNNN
B-29OOOOOOOOOOOOOOOOOOOOOOOOOOO
B-29PPPPPPPPPPPPPPPPPPPPPPPPPPP
B-29QQQQQQQQQQQQQQQQQQQQQQQQQQQ
B-29RRRRRRRRRRRRRRRRRRRRRRRRRRR
B-29SSSSSSSSSSSSSSSSSSSSSSSSSSS
B-29TTTTTTTTTTTTTTTTTTTTTTTTTTT
B-29UUUUUUUUUUUUUUUUUUUUUUUUUUU
B-29VVVVVVVVVVVVVVVVVVVVVVVVVVV
B-29WWWWWWWWWWWWWWWWWWWWWWWWWWW
B-29XXXXXXXXXXXXXXXXXXXXXXXXXXX
B-29YYYYYYYYYYYYYYYYYYYYYYYYYYY
B-29ZZZZZZZZZZZZZZZZZZZZZZZZZZZ
B-29AAAAAAAAAAAAAAAAAAAAAAAAAAAA
B-29BBBBBBBBBBBBBBBBBBBBBBBBBBBB
B-29CCCCCCCCCCCCCCCCCCCCCCCCCCCC
B-29DDDDDDDDDDDDDDDDDDDDDDDDDDDD
B-29EEEEEEEEEEEEEEEEEEEEEEEEEEEE
B-29FFFFFFFFFFFFFFFFFFFFFFFFFFFF
B-29GGGGGGGGGGGGGGGGGGGGGGGGGGGG
B-29HHHHHHHHHHHHHHHHHHHHHHHHHHHH
B-29IIIIIIIIIIIIIIIIIIIIIIIIIIII
B-29JJJJJJJJJJJJJJJJJJJJJJJJJJJJ
B-29KKKKKKKKKKKKKKKKKKKKKKKKKKKK
B-29LLLLLLLLLLLLLLLLLLLLLLLLLLLL
B-29MMMMMMMMMMMMMMMMMMMMMMMMMMMM
B-29NNNNNNNNNNNNNNNNNNNNNNNNNNNN
B-29OOOOOOOOOOOOOOOOOOOOOOOOOOOO
B-29PPPPPPPPPPPPPPPPPPPPPPPPPPPP
B-29QQQQQQQQQQQQQQQQQQQQQQQQQQQQ
B-29RRRRRRRRRRRRRRRRRRRRRRRRRRRR
B-29SSSSSSSSSSSSSSSSSSSSSSSSSSSS
B-29TTTTTTTTTTTTTTTTTTTTTTTTTTTT
B-29UUUUUUUUUUUUUUUUUUUUUUUUUUUU
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B-29XXXXXXXXXXXXXXXXXXXXXXXXXXXX
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B-29AAAAAAAAAAAAAAAAAAAAAAAAAAAAA
B-29BBBBBBBBBBBBBBBBBBBBBBBBBBBBB
B-29CCCCCCCCCCCCCCCCCCCCCCCCCCCCC
B-29DDDDDDDDDDDDDDDDDDDDDDDDDDDDD
B-29EEEEEEEEEEEEEEEEEEEEEEEEEEEEE
B-29FFFFFFFFFFFFFFFFFFFFFFFFFFFFF
B-29GGGGGGGGGGGGGGGGGGGGGGGGGGGGG
B-29HHHHHHHHHHHHHHHHHHHHHHHHHHHHH
B-29IIIIIIIIIIIIIIIIIIIIIIIIIIIII
B-29JJ

ATTACHMENT B
Test Pit Logs

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-01

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor, moist to dry -TOPSOIL-
		0.5	ML	Light-brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry, interbedded red sandy SILT
1				
2	0.0	2.0		BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-02

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Dark brown TOPSOIL, no structure, no odor, moist to dry -TOPSOIL-
		0.5	ML-SM	Dark brown silty SAND (ML-SM), mps 6 in., no structure, no odor, moist to dry
1			ML-SM	Similar as above, except light-brown Note: 4 in brick fragment observed at 18 in. bgs
2		2.0		BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

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HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-03

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Dark brown TOPSOIL, no structure, no odor, moist to dry
				-TOPSOIL-
		0.5	ML	Dark brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.8; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

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HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-04

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor, moist to dry
				-TOPSOIL-
	0.0	0.5	ML	Light brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry, traces of red sandy SILT
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

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HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-05

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor
				-TOPSOIL-
	0.0	0.5	ML	Light brown to brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.0; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

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ATTACHMENT C
Photo Log

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 1
Test Pit- TP-01 - hand augured to 2 feet bgs.



Photo Number: 2
Typical cobble sizes greater than 3 inches observed between 10-15% makeup of soil matrix within test pit excavations.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 3
Typical soil matrix observed in test pit excavations



Photo Number: 4
Test Pit - TP-02 - Brick fragment observed at 1.5 feet bgs.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 5
Test Pit - TP-02 - hand augured to 2 feet bgs.



Photo Number: 6
Test Pit - TP-03 - hand augured to 2 feet bgs.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 7
Test Pit - TP-04 - hand augured to 2 feet bgs.



Photo Number: 8
Test Pit - TP-05 - hand augured to 2 feet bgs.

ATTACHMENT D
Grain Size Analysis Lab Report

CME Associates, Inc.
6035 Corporate Drive
East Syracuse, NY 13057

Material Test Report

Report ID: MAT:02-25-11337-01
Issue No: 1
This issue replaces all previous issues of this report

Client: Haley & Aldrich of New York Engineering and Geology, LLP
CC: Steve Phillips, Syracuse Admin, Thomas Robitaille

Project: 2500581 - Montante Xerox Solar Development

Location: Webster, NY

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Submitted By: Niel W. Zuern, Vice President of Quality
Date of Issue: 11/12/2025

Sample Details

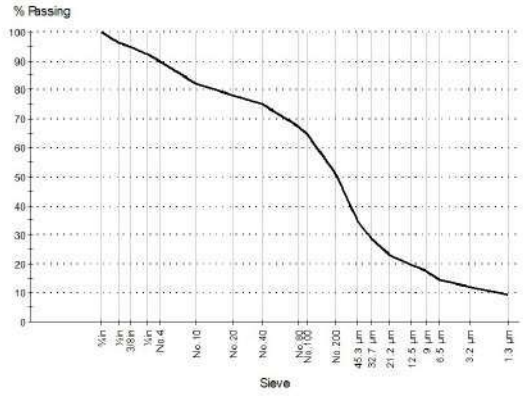
Sample ID	02-25-11337-01
Date Sampled	11/4/2025
Material	TP-01
Specification	Hydrometer Sieve - D422
Sampling Method	Client sampled
Location	TP-01

Other Test Results

Description	Method	Result	Limits
Dispersion device	ASTM D 422	Mechanical	
Dispersion time (min)		1	
Shape		Round	
Hardness		Soft	

Particle Size Distribution

ASTM D 422



Date Tested: 11/12/2025
Tested By: Hannah Kloiber

Sieve Size	% Passing	Limits
¾in	100	
½in	96	
3/8in	95	
¼in	92	
No.4	90	
No.10	82	
No.20	78	
No.40	75	
No.80	67	
No.100	65	
No.200	51	
45.3 µm	34.9	
32.7 µm	29.0	
21.2 µm	23.2	
12.5 µm	19.4	
9.0 µm	17.7	
6.5 µm	14.5	
3.2 µm	12.0	
1.3 µm	9.5	

Comments

This material is classified as Brown cmf SAND and SILT, little CLAY, little mf GRAVEL.

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CME Associates, Inc.
6035 Corporate Drive
East Syracuse, NY 13057

Material Test Report

Report ID: MAT:02-25-11337-02
Issue No: 1
This issue replaces all previous issues of this report

Client: Haley & Aldrich of New York Engineering and Geology, LLP
CC: Steve Phillips, Syracuse Admin, Thomas Robitaille

Project: 2500581 - Montante Xerox Solar Development

Location: Webster, NY

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Submitted By: Niel W. Zuern, Vice President of Quality
Date of Issue: 11/12/2025

Sample Details

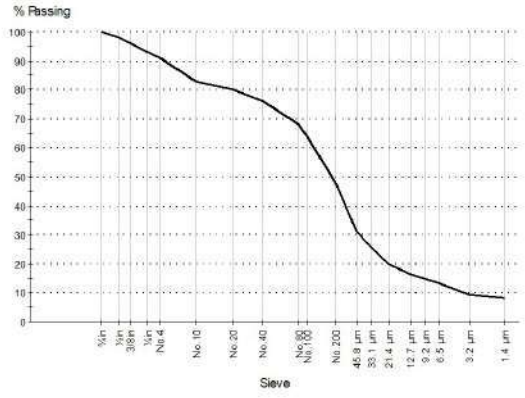
Sample ID	02-25-11337-02
Date Sampled	11/4/2025
Material	TP-02
Specification	Hydrometer Sieve - D422
Sampling Method	Client sampled
Location	TP-02

Other Test Results

Description	Method	Result	Limits
Dispersion device	ASTM D 422	Mechanical	
Dispersion time (min)		1	
Shape		Round	
Hardness		Soft	

Particle Size Distribution

ASTM D 422



Date Tested: 11/12/2025
Tested By: Hannah Kloiber

Sieve Size	% Passing	Limits
¾in	100	
½in	98	
3/8in	96	
¼in	93	
No.4	91	
No.10	83	
No.20	80	
No.40	76	
No.80	68	
No.100	64	
No.200	48	
45.8 µm	31.4	
33.1 µm	25.7	
21.4 µm	19.9	
12.7 µm	16.6	
9.2 µm	14.8	
6.5 µm	13.2	
3.2 µm	9.4	
1.4 µm	8.1	

Comments

This material is classified as Brown cmf SAND and SILT, little CLAY, trace mf GRAVEL.

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CME Associates, Inc.
6035 Corporate Drive
East Syracuse, NY 13057

Material Test Report

Report ID: MAT:02-25-11337-03
Issue No: 1
This issue replaces all previous issues of this report

Client: Haley & Aldrich of New York Engineering and Geology, LLP
CC: Steve Phillips, Syracuse Admin, Thomas Robitaille

Project: 2500581 - Montante Xerox Solar Development

Location: Webster, NY

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Submitted By: Niel W. Zuern, Vice President of Quality
Date of Issue: 11/12/2025

Sample Details

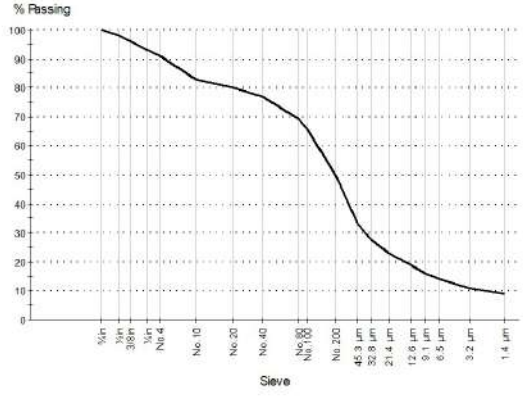
Sample ID	02-25-11337-03
Date Sampled	11/4/2025
Material	TP-03
Specification	Hydrometer Sieve - D422
Sampling Method	Client sampled
Location	TP-03

Other Test Results

Description	Method	Result	Limits
Dispersion device	ASTM D 422	Mechanical	
Dispersion time (min)		1	
Shape		Round	
Hardness		Soft	

Particle Size Distribution

ASTM D 422



Date Tested: 11/12/2025
Tested By: Hannah Kloiber

Sieve Size	% Passing	Limits
¾in	100	
½in	98	
3/8in	96	
¼in	93	
No.4	91	
No.10	83	
No.20	80	
No.40	77	
No.80	69	
No.100	66	
No.200	50	
45.3 µm	33.3	
32.8 µm	27.6	
21.4 µm	22.9	
12.6 µm	18.7	
9.1 µm	16.1	
6.5 µm	14.0	
3.2 µm	11.0	
1.4 µm	9.1	

Comments

This material is classified as Brown cmf SAND and SILT, little CLAY, trace mf GRAVEL.

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CME Associates, Inc.
6035 Corporate Drive
East Syracuse, NY 13057

Material Test Report

Report ID: MAT:02-25-11337-04
Issue No: 1
This issue replaces all previous issues of this report

Client: Haley & Aldrich of New York Engineering and Geology, LLP
CC: Steve Phillips, Syracuse Admin, Thomas Robitaille

Project: 2500581 - Montante Xerox Solar Development

Location: Webster, NY

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Submitted By: Niel W. Zuern, Vice President of Quality
Date of Issue: 11/12/2025

Sample Details

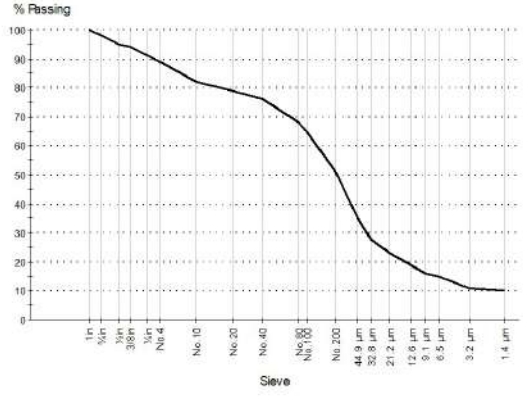
Sample ID	02-25-11337-04
Date Sampled	11/4/2025
Material	TP-04
Specification	Hydrometer Sieve - D422
Sampling Method	Client sampled
Location	TP-04

Other Test Results

Description	Method	Result	Limits
Dispersion device	ASTM D 422	Mechanical	
Dispersion time (min)		1	
Shape		Round	
Hardness		Soft	

Particle Size Distribution

ASTM D 422



Date Tested: 11/12/2025
Tested By: Hannah Kloiber

Sieve Size	% Passing	Limits
1in	100	
¾in	98	
½in	95	
3/8in	94	
¼in	91	
No.4	89	
No.10	82	
No.20	79	
No.40	76	
No.80	68	
No.100	65	
No.200	51	
44.9 µm	35.1	
32.8 µm	27.7	
21.2 µm	23.0	
12.6 µm	18.6	
9.1 µm	16.2	
6.5 µm	14.7	
3.2 µm	10.9	

Comments

This material is classified as Brown cmf SAND and SILT, little CLAY, little mf GRAVEL.

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CME Associates, Inc.
6035 Corporate Drive
East Syracuse, NY 13057

Material Test Report

Report ID: MAT:02-25-11337-05
Issue No: 1
This issue replaces all previous issues of this report

Client: Haley & Aldrich of New York Engineering and Geology, LLP
CC: Steve Phillips, Syracuse Admin, Thomas Robitaille

Project: 2500581 - Montante Xerox Solar Development

Location: Webster, NY

This report and the results contained herein are the exclusive property of CME Associates, Inc. and shall only be reproduced in full when written consent is provided by CME Associates, Inc. The results in this report relate only to the items inspected or tested.

Submitted By: Niel W. Zuern, Vice President of Quality
Date of Issue: 11/12/2025

Sample Details

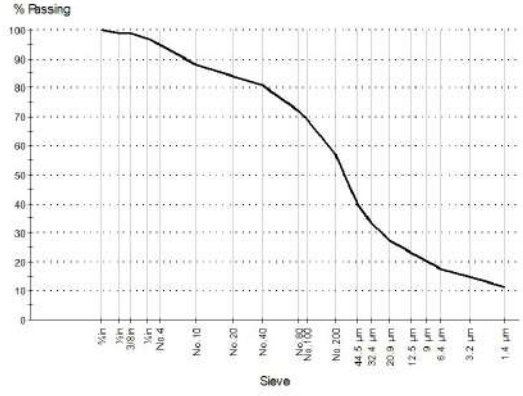
Sample ID	02-25-11337-05
Date Sampled	11/4/2025
Material	TP-05
Specification	Hydrometer Sieve - D422
Sampling Method	Client sampled
Location	TP-05

Other Test Results

Description	Method	Result	Limits
Dispersion device	ASTM D 422	Mechanical	
Dispersion time (min)		1	
Shape		Round	
Hardness		Soft	

Particle Size Distribution

ASTM D 422



Date Tested: 11/12/2025
Tested By: Hannah Kloiber

Sieve Size	% Passing	Limits
¾in	100	
½in	99	
3/8in	99	
¼in	97	
No.4	95	
No.10	88	
No.20	84	
No.40	81	
No.80	72	
No.100	69	
No.200	57	
44.5 µm	39.7	
32.4 µm	33.5	
20.9 µm	27.4	
12.5 µm	23.2	
9.0 µm	20.2	
6.4 µm	17.7	
3.2 µm	14.7	
1.4 µm	11.3	

Comments

This material is classified as Brown SILT and cmf SAND, little CLAY, trace mf GRAVEL.

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ATTACHMENT E
Community Air Monitoring Results

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
1	10/30/2025	9:36:03	0.016
2	10/30/2025	9:37:03	0.015
3	10/30/2025	9:38:03	0.014
4	10/30/2025	9:39:03	0.015
5	10/30/2025	9:40:03	0.014
6	10/30/2025	9:41:03	0.013
7	10/30/2025	9:42:03	0.013
8	10/30/2025	9:43:03	0.013
9	10/30/2025	9:44:03	0.012
10	10/30/2025	9:45:03	0.013
11	10/30/2025	9:46:03	0.013
12	10/30/2025	9:47:03	0.013
13	10/30/2025	9:48:03	0.012
14	10/30/2025	9:49:03	0.012
15	10/30/2025	9:50:03	0.012
16	10/30/2025	9:51:03	0.012
17	10/30/2025	9:52:03	0.012
18	10/30/2025	9:53:03	0.012
19	10/30/2025	9:54:03	0.013
20	10/30/2025	9:55:03	0.013
21	10/30/2025	9:56:03	0.012
22	10/30/2025	9:57:03	0.013
23	10/30/2025	9:58:03	0.013
24	10/30/2025	9:59:03	0.013
25	10/30/2025	10:00:03	0.012
26	10/30/2025	10:01:03	0.013
27	10/30/2025	10:02:03	0.013
28	10/30/2025	10:03:03	0.014
29	10/30/2025	10:04:03	0.014
30	10/30/2025	10:05:03	0.014
31	10/30/2025	10:06:03	0.014
32	10/30/2025	10:07:03	0.016
33	10/30/2025	10:08:03	0.016
34	10/30/2025	10:09:03	0.016
35	10/30/2025	10:10:03	0.018
36	10/30/2025	10:11:03	0.018
37	10/30/2025	10:12:03	0.019
38	10/30/2025	10:13:03	0.019
39	10/30/2025	10:14:03	0.019
40	10/30/2025	10:15:03	0.02

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
41	10/30/2025	10:16:03	0.021
42	10/30/2025	10:17:03	0.023
43	10/30/2025	10:18:03	0.021
44	10/30/2025	10:19:03	0.017
45	10/30/2025	10:20:03	0.017
46	10/30/2025	10:21:03	0.019
47	10/30/2025	10:22:03	0.02
48	10/30/2025	10:23:03	0.02
49	10/30/2025	10:24:03	0.02
50	10/30/2025	10:25:03	0.02
51	10/30/2025	10:26:03	0.02
52	10/30/2025	10:27:03	0.02
53	10/30/2025	10:28:03	0.019
54	10/30/2025	10:29:03	0.019
55	10/30/2025	10:30:03	0.018
56	10/30/2025	10:31:03	0.017
57	10/30/2025	10:32:03	0.016
58	10/30/2025	10:33:03	0.017
59	10/30/2025	10:34:03	0.017
60	10/30/2025	10:35:03	0.016
61	10/30/2025	10:36:03	0.015
62	10/30/2025	10:37:03	0.015
63	10/30/2025	10:38:03	0.014
64	10/30/2025	10:39:03	0.013
65	10/30/2025	10:40:03	0.012
66	10/30/2025	10:41:03	0.012
67	10/30/2025	10:42:03	0.013
68	10/30/2025	10:43:03	0.013
69	10/30/2025	10:44:03	0.012
70	10/30/2025	10:45:03	0.013
71	10/30/2025	10:46:03	0.012
72	10/30/2025	10:47:03	0.013
73	10/30/2025	10:48:03	0.012
74	10/30/2025	10:49:03	0.012
75	10/30/2025	10:50:03	0.012
76	10/30/2025	10:51:03	0.012
77	10/30/2025	10:52:03	0.012
78	10/30/2025	10:53:03	0.012
79	10/30/2025	10:54:03	0.013
80	10/30/2025	10:55:03	0.013

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
81	10/30/2025	10:56:03	0.013
82	10/30/2025	10:57:03	0.013
83	10/30/2025	10:58:03	0.012
84	10/30/2025	10:59:03	0.011
85	10/30/2025	11:00:03	0.012
86	10/30/2025	11:01:03	0.012
87	10/30/2025	11:02:03	0.012
88	10/30/2025	11:03:03	0.011
89	10/30/2025	11:04:03	0.011
90	10/30/2025	11:05:03	0.011
91	10/30/2025	11:06:03	0.011
92	10/30/2025	11:07:03	0.011
93	10/30/2025	11:08:03	0.01
94	10/30/2025	11:09:03	0.011
95	10/30/2025	11:10:03	0.011
96	10/30/2025	11:11:03	0.011
97	10/30/2025	11:12:03	0.01
98	10/30/2025	11:13:03	0.011
99	10/30/2025	11:14:03	0.01
100	10/30/2025	11:15:03	0.01
101	10/30/2025	11:16:03	0.01
102	10/30/2025	11:17:03	0.01
103	10/30/2025	11:18:03	0.01
104	10/30/2025	11:19:03	0.009
105	10/30/2025	11:20:03	0.01
106	10/30/2025	11:21:03	0.01
107	10/30/2025	11:22:03	0.01
108	10/30/2025	11:23:03	0.01
109	10/30/2025	11:24:03	0.009
110	10/30/2025	11:25:03	0.009
111	10/30/2025	11:26:03	0.01
112	10/30/2025	11:27:03	0.01
113	10/30/2025	11:28:03	0.01
114	10/30/2025	11:29:03	0.009
115	10/30/2025	11:30:03	0.009
116	10/30/2025	11:31:03	0.009
117	10/30/2025	11:32:03	0.009
118	10/30/2025	11:33:03	0.009
119	10/30/2025	11:34:03	0.009
120	10/30/2025	11:35:03	0.009

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
121	10/30/2025	11:36:03	0.009
122	10/30/2025	11:37:03	0.008
123	10/30/2025	11:38:03	0.008
124	10/30/2025	11:39:03	0.008
125	10/30/2025	11:40:03	0.008
126	10/30/2025	11:41:03	0.008
127	10/30/2025	11:42:03	0.008
128	10/30/2025	11:43:03	0.008
129	10/30/2025	11:44:03	0.009
130	10/30/2025	11:45:03	0.009
131	10/30/2025	11:46:03	0.008
132	10/30/2025	11:47:03	0.009
133	10/30/2025	11:48:03	0.008
134	10/30/2025	11:49:03	0.008
135	10/30/2025	11:50:03	0.008
136	10/30/2025	11:51:03	0.009
137	10/30/2025	11:52:03	0.008
138	10/30/2025	11:53:03	0.008
139	10/30/2025	11:54:03	0.009
140	10/30/2025	11:55:03	0.008
141	10/30/2025	11:56:03	0.008
142	10/30/2025	11:57:03	0.007
143	10/30/2025	11:58:03	0.007
144	10/30/2025	11:59:03	0.007
145	10/30/2025	12:00:03	0.007
146	10/30/2025	12:01:03	0.008
147	10/30/2025	12:02:03	0.008
148	10/30/2025	12:03:03	0.008
149	10/30/2025	12:04:03	0.007
150	10/30/2025	12:05:03	0.007
151	10/30/2025	12:06:03	0.008
152	10/30/2025	12:07:03	0.008
153	10/30/2025	12:08:03	0.007
154	10/30/2025	12:09:03	0.007
155	10/30/2025	12:10:03	0.007
156	10/30/2025	12:11:03	0.007
157	10/30/2025	12:12:03	0.008
158	10/30/2025	12:13:03	0.007
159	10/30/2025	12:14:03	0.007
160	10/30/2025	12:15:03	0.007

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
161	10/30/2025	12:16:03	0.007
162	10/30/2025	12:17:03	0.008
163	10/30/2025	12:18:03	0.008
164	10/30/2025	12:19:03	0.007
165	10/30/2025	12:20:03	0.007
166	10/30/2025	12:21:03	0.006
167	10/30/2025	12:22:03	0.006
168	10/30/2025	12:23:03	0.006
169	10/30/2025	12:24:03	0.007
170	10/30/2025	12:25:03	0.007
171	10/30/2025	12:26:03	0.007
172	10/30/2025	12:27:03	0.006
173	10/30/2025	12:28:03	0.006
174	10/30/2025	12:29:03	0.006
175	10/30/2025	12:30:03	0.006
176	10/30/2025	12:31:03	0.006
177	10/30/2025	12:32:03	0.006
178	10/30/2025	12:33:03	0.006
179	10/30/2025	12:34:03	0.006
180	10/30/2025	12:35:03	0.005
181	10/30/2025	12:36:03	0.007
182	10/30/2025	12:37:03	0.006
183	10/30/2025	12:38:03	0.005
184	10/30/2025	12:39:03	0.006
185	10/30/2025	12:40:03	0.005
186	10/30/2025	12:41:03	0.005
187	10/30/2025	12:42:03	0.006
188	10/30/2025	12:43:03	0.005
189	10/30/2025	12:44:03	0.005
190	10/30/2025	12:45:03	0.005
191	10/30/2025	12:46:03	0.005
192	10/30/2025	12:47:03	0.005
193	10/30/2025	12:48:03	0.005
194	10/30/2025	12:49:03	0.005
195	10/30/2025	12:50:03	0.005
196	10/30/2025	12:51:03	0.005
197	10/30/2025	12:52:03	0.005
198	10/30/2025	12:53:03	0.005
199	10/30/2025	12:54:03	0.005
200	10/30/2025	12:55:03	0.005

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
201	10/30/2025	12:56:03	0.006
202	10/30/2025	12:57:03	0.026
203	10/30/2025	12:58:03	0.005
204	10/30/2025	12:59:03	0.006
205	10/30/2025	13:00:03	0.006
206	10/30/2025	13:01:03	0.006
207	10/30/2025	13:02:03	0.005
208	10/30/2025	13:03:03	0.004
209	10/30/2025	13:04:03	0.004
210	10/30/2025	13:05:03	0.004
211	10/30/2025	13:06:03	0.004
212	10/30/2025	13:07:03	0.004
213	10/30/2025	13:08:03	0.004
214	10/30/2025	13:09:03	0.004
215	10/30/2025	13:10:03	0.004
216	10/30/2025	13:11:03	0.004
217	10/30/2025	13:12:03	0.005
218	10/30/2025	13:13:03	0.004
219	10/30/2025	13:14:03	0.004
220	10/30/2025	13:15:03	0.005
221	10/30/2025	13:16:03	0.009
222	10/30/2025	13:17:03	0.009
223	10/30/2025	13:18:03	0.01
224	10/30/2025	13:19:03	0.01
225	10/30/2025	13:20:03	0.01
226	10/30/2025	13:21:03	0.01
227	10/30/2025	13:22:03	0.009
228	10/30/2025	13:23:03	0.01
229	10/30/2025	13:24:03	0.009
230	10/30/2025	13:25:03	0.01
231	10/30/2025	13:26:03	0.01
232	10/30/2025	13:27:03	0.01
233	10/30/2025	13:28:03	0.01
234	10/30/2025	13:29:03	0.01
235	10/30/2025	13:30:03	0.01
236	10/30/2025	13:31:03	0.009
237	10/30/2025	13:32:03	0.009
238	10/30/2025	13:33:03	0.009
239	10/30/2025	13:34:03	0.009
240	10/30/2025	13:35:03	0.009

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
241	10/30/2025	13:36:03	0.009
242	10/30/2025	13:37:03	0.009
243	10/30/2025	13:38:03	0.008
244	10/30/2025	13:39:03	0.008
245	10/30/2025	13:40:03	0.008
246	10/30/2025	13:41:03	0.008
247	10/30/2025	13:42:03	0.008
248	10/30/2025	13:43:03	0.009
249	10/30/2025	13:44:03	0.008
250	10/30/2025	13:45:03	0.008
251	10/30/2025	13:46:03	0.008
252	10/30/2025	13:47:03	0.008
253	10/30/2025	13:48:03	0.008
254	10/30/2025	13:49:03	0.008
255	10/30/2025	13:50:03	0.008
256	10/30/2025	13:51:03	0.008
257	10/30/2025	13:52:03	0.008
258	10/30/2025	13:53:03	0.008
259	10/30/2025	13:54:03	0.007
260	10/30/2025	13:55:03	0.007
261	10/30/2025	13:56:03	0.008
262	10/30/2025	13:57:03	0.008
263	10/30/2025	13:58:03	0.007
264	10/30/2025	13:59:03	0.007
265	10/30/2025	14:00:03	0.007
266	10/30/2025	14:01:03	0.007
267	10/30/2025	14:02:03	0.007
268	10/30/2025	14:03:03	0.008
269	10/30/2025	14:04:03	0.007
270	10/30/2025	14:05:03	0.007
271	10/30/2025	14:06:03	0.007
272	10/30/2025	14:07:03	0.007
273	10/30/2025	14:08:03	0.007
274	10/30/2025	14:09:03	0.008
275	10/30/2025	14:10:03	0.007
276	10/30/2025	14:11:03	0.007
277	10/30/2025	14:12:03	0.008
278	10/30/2025	14:13:03	0.008
279	10/30/2025	14:14:03	0.008
280	10/30/2025	14:15:03	0.007

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
281	10/30/2025	14:16:03	0.007
282	10/30/2025	14:17:03	0.007
283	10/30/2025	14:18:03	0.007
284	10/30/2025	14:19:03	0.007
285	10/30/2025	14:20:03	0.007
286	10/30/2025	14:21:03	0.007
287	10/30/2025	14:22:03	0.007
288	10/30/2025	14:23:03	0.007
289	10/30/2025	14:24:03	0.007
290	10/30/2025	14:25:03	0.007
291	10/30/2025	14:26:03	0.007
292	10/30/2025	14:27:03	0.007
293	10/30/2025	14:28:03	0.007
294	10/30/2025	14:29:03	0.006
295	10/30/2025	14:30:03	0.007
296	10/30/2025	14:31:03	0.007
297	10/30/2025	14:32:03	0.007
298	10/30/2025	14:33:03	0.007
299	10/30/2025	14:34:03	0.007
300	10/30/2025	14:35:03	0.007
301	10/30/2025	14:36:03	0.006
302	10/30/2025	14:37:03	0.007
303	10/30/2025	14:38:03	0.007
304	10/30/2025	14:39:03	0.007
305	10/30/2025	14:40:03	0.007
306	10/30/2025	14:41:03	0.007
307	10/30/2025	14:42:03	0.007
308	10/30/2025	14:43:03	0.007
309	10/30/2025	14:44:03	0.007
310	10/30/2025	14:45:03	0.006
311	10/30/2025	14:46:03	0.007
312	10/30/2025	14:47:03	0.006
313	10/30/2025	14:48:03	0.006
314	10/30/2025	14:49:03	0.006
315	10/30/2025	14:50:03	0.006
316	10/30/2025	14:51:03	0.006
317	10/30/2025	14:52:03	0.006
318	10/30/2025	14:53:03	0.006
319	10/30/2025	14:54:03	0.006
320	10/30/2025	14:55:03	0.006

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
321	10/30/2025	14:56:03	0.006
322	10/30/2025	14:57:03	0.006
323	10/30/2025	14:58:03	0.006
324	10/30/2025	14:59:03	0.006
325	10/30/2025	15:00:03	0.006
326	10/30/2025	15:01:03	0.006
327	10/30/2025	15:02:03	0.006
328	10/30/2025	15:03:03	0.006
329	10/30/2025	15:04:03	0.006
330	10/30/2025	15:05:03	0.006
331	10/30/2025	15:06:03	0.006
332	10/30/2025	15:07:03	0.006
333	10/30/2025	15:08:03	0.006
334	10/30/2025	15:09:03	0.006
335	10/30/2025	15:10:03	0.006
336	10/30/2025	15:11:03	0.005
337	10/30/2025	15:12:03	0.006
338	10/30/2025	15:13:03	0.005
339	10/30/2025	15:14:03	0.005
340	10/30/2025	15:15:03	0.005
341	10/30/2025	15:16:03	0.006
342	10/30/2025	15:17:03	0.006
343	10/30/2025	15:18:03	0.006
344	10/30/2025	15:19:03	0.005
345	10/30/2025	15:20:03	0.005
346	10/30/2025	15:21:03	0.005
347	10/30/2025	15:22:03	0.005
348	10/30/2025	15:23:03	0.005
349	10/30/2025	15:24:03	0.005
350	10/30/2025	15:25:03	0.005
351	10/30/2025	15:26:03	0.005
352	10/30/2025	15:27:03	0.005
353	10/30/2025	15:28:03	0.004
354	10/30/2025	15:29:03	0.004
355	10/30/2025	15:30:03	0.004
356	10/30/2025	15:31:03	0.004
357	10/30/2025	15:32:03	0.004
358	10/30/2025	15:33:03	0.004
359	10/30/2025	15:34:03	0.004
360	10/30/2025	15:35:03	0.004

DUSTRAK ID: FA03605 (DOWNWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
361	10/30/2025	15:36:03	0.004
362	10/30/2025	15:37:03	0.006
363	10/30/2025	15:38:03	0.005
364	10/30/2025	15:39:03	0.006
365	10/30/2025	15:40:03	0.005
366	10/30/2025	15:41:03	0.005
367	10/30/2025	15:42:03	0.005
368	10/30/2025	15:43:03	0.004
369	10/30/2025	15:44:03	0.005
370	10/30/2025	15:45:03	0.004
371	10/30/2025	15:46:03	0.023

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
1	10/30/2025	9:29:56	0.018
2	10/30/2025	9:30:56	0.015
3	10/30/2025	9:31:56	0.014
4	10/30/2025	9:32:56	0.015
5	10/30/2025	9:33:56	0.015
6	10/30/2025	9:34:56	0.015
7	10/30/2025	9:35:56	0.014
8	10/30/2025	9:36:56	0.014
9	10/30/2025	9:37:56	0.013
10	10/30/2025	9:38:56	0.013
11	10/30/2025	9:39:56	0.011
12	10/30/2025	9:40:56	0.011
13	10/30/2025	9:41:56	0.011
14	10/30/2025	9:42:56	0.011
15	10/30/2025	9:43:56	0.01
16	10/30/2025	9:44:56	0.01
17	10/30/2025	9:45:56	0.01
18	10/30/2025	9:46:56	0.01
19	10/30/2025	9:47:56	0.01
20	10/30/2025	9:48:56	0.01
21	10/30/2025	9:49:56	0.01
22	10/30/2025	9:50:56	0.01
23	10/30/2025	9:51:56	0.01
24	10/30/2025	9:52:56	0.01
25	10/30/2025	9:53:56	0.009
26	10/30/2025	9:54:56	0.01
27	10/30/2025	9:55:56	0.01
28	10/30/2025	9:56:56	0.01
29	10/30/2025	9:57:56	0.011
30	10/30/2025	9:58:56	0.011
31	10/30/2025	9:59:56	0.011
32	10/30/2025	10:00:56	0.01
33	10/30/2025	10:01:56	0.01
34	10/30/2025	10:02:56	0.011
35	10/30/2025	10:03:56	0.011
36	10/30/2025	10:04:56	0.011
37	10/30/2025	10:05:56	0.012
38	10/30/2025	10:06:56	0.013
39	10/30/2025	10:07:56	0.012

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
40	10/30/2025	10:08:56	0.013
41	10/30/2025	10:09:56	0.015
42	10/30/2025	10:10:56	0.016
43	10/30/2025	10:11:56	0.017
44	10/30/2025	10:12:56	0.017
45	10/30/2025	10:13:56	0.018
46	10/30/2025	10:14:56	0.018
47	10/30/2025	10:15:56	0.018
48	10/30/2025	10:16:56	0.019
49	10/30/2025	10:17:56	0.02
50	10/30/2025	10:18:56	0.02
51	10/30/2025	10:19:56	0.016
52	10/30/2025	10:20:56	0.016
53	10/30/2025	10:21:56	0.016
54	10/30/2025	10:22:56	0.016
55	10/30/2025	10:23:56	0.016
56	10/30/2025	10:24:56	0.016
57	10/30/2025	10:25:56	0.016
58	10/30/2025	10:26:56	0.016
59	10/30/2025	10:27:56	0.016
60	10/30/2025	10:28:56	0.015
61	10/30/2025	10:29:56	0.015
62	10/30/2025	10:30:56	0.014
63	10/30/2025	10:31:56	0.014
64	10/30/2025	10:32:56	0.014
65	10/30/2025	10:33:56	0.013
66	10/30/2025	10:34:56	0.013
67	10/30/2025	10:35:56	0.013
68	10/30/2025	10:36:56	0.011
69	10/30/2025	10:37:56	0.011
70	10/30/2025	10:38:56	0.01
71	10/30/2025	10:39:56	0.01
72	10/30/2025	10:40:56	0.01
73	10/30/2025	10:41:56	0.01
74	10/30/2025	10:42:56	0.01
75	10/30/2025	10:43:56	0.01
76	10/30/2025	10:44:56	0.009
77	10/30/2025	10:45:56	0.01
78	10/30/2025	10:46:56	0.01

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
79	10/30/2025	10:47:56	0.01
80	10/30/2025	10:48:56	0.01
81	10/30/2025	10:49:56	0.01
82	10/30/2025	10:50:56	0.01
83	10/30/2025	10:51:56	0.009
84	10/30/2025	10:52:56	0.01
85	10/30/2025	10:53:56	0.01
86	10/30/2025	10:54:56	0.011
87	10/30/2025	10:55:56	0.01
88	10/30/2025	10:56:56	0.01
89	10/30/2025	10:57:56	0.01
90	10/30/2025	10:58:56	0.01
91	10/30/2025	10:59:56	0.009
92	10/30/2025	11:00:56	0.009
93	10/30/2025	11:01:56	0.009
94	10/30/2025	11:02:56	0.009
95	10/30/2025	11:03:56	0.008
96	10/30/2025	11:04:56	0.009
97	10/30/2025	11:05:56	0.008
98	10/30/2025	11:06:56	0.009
99	10/30/2025	11:07:56	0.008
100	10/30/2025	11:08:56	0.008
101	10/30/2025	11:09:56	0.008
102	10/30/2025	11:10:56	0.008
103	10/30/2025	11:11:56	0.008
104	10/30/2025	11:12:56	0.008
105	10/30/2025	11:13:56	0.008
106	10/30/2025	11:14:56	0.008
107	10/30/2025	11:15:56	0.009
108	10/30/2025	11:16:56	0.009
109	10/30/2025	11:17:56	0.008
110	10/30/2025	11:18:56	0.007
111	10/30/2025	11:19:56	0.007
112	10/30/2025	11:20:56	0.007
113	10/30/2025	11:21:56	0.007
114	10/30/2025	11:22:56	0.007
115	10/30/2025	11:23:56	0.007
116	10/30/2025	11:24:56	0.007
117	10/30/2025	11:25:56	0.008

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
118	10/30/2025	11:26:56	0.007
119	10/30/2025	11:27:56	0.007
120	10/30/2025	11:28:56	0.007
121	10/30/2025	11:29:56	0.007
122	10/30/2025	11:30:56	0.006
123	10/30/2025	11:31:56	0.007
124	10/30/2025	11:32:56	0.007
125	10/30/2025	11:33:56	0.007
126	10/30/2025	11:34:56	0.006
127	10/30/2025	11:35:56	0.006
128	10/30/2025	11:36:56	0.006
129	10/30/2025	11:37:56	0.005
130	10/30/2025	11:38:56	0.005
131	10/30/2025	11:39:56	0.006
132	10/30/2025	11:40:56	0.006
133	10/30/2025	11:41:56	0.006
134	10/30/2025	11:42:56	0.006
135	10/30/2025	11:43:56	0.007
136	10/30/2025	11:44:56	0.006
137	10/30/2025	11:45:56	0.006
138	10/30/2025	11:46:56	0.006
139	10/30/2025	11:47:56	0.006
140	10/30/2025	11:48:56	0.006
141	10/30/2025	11:49:56	0.006
142	10/30/2025	11:50:56	0.006
143	10/30/2025	11:51:56	0.006
144	10/30/2025	11:52:56	0.006
145	10/30/2025	11:53:56	0.007
146	10/30/2025	11:54:56	0.007
147	10/30/2025	11:55:56	0.006
148	10/30/2025	11:56:56	0.006
149	10/30/2025	11:57:56	0.005
150	10/30/2025	11:58:56	0.006
151	10/30/2025	11:59:56	0.006
152	10/30/2025	12:00:56	0.006
153	10/30/2025	12:01:56	0.006
154	10/30/2025	12:02:56	0.006
155	10/30/2025	12:03:56	0.006
156	10/30/2025	12:04:56	0.006

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
157	10/30/2025	12:05:56	0.006
158	10/30/2025	12:06:56	0.006
159	10/30/2025	12:07:56	0.006
160	10/30/2025	12:08:56	0.006
161	10/30/2025	12:09:56	0.006
162	10/30/2025	12:10:56	0.006
163	10/30/2025	12:11:56	0.006
164	10/30/2025	12:12:56	0.006
165	10/30/2025	12:13:56	0.006
166	10/30/2025	12:14:56	0.006
167	10/30/2025	12:15:56	0.006
168	10/30/2025	12:16:56	0.006
169	10/30/2025	12:17:56	0.006
170	10/30/2025	12:18:56	0.006
171	10/30/2025	12:19:56	0.005
172	10/30/2025	12:20:56	0.005
173	10/30/2025	12:21:56	0.005
174	10/30/2025	12:22:56	0.005
175	10/30/2025	12:23:56	0.005
176	10/30/2025	12:24:56	0.005
177	10/30/2025	12:25:56	0.005
178	10/30/2025	12:26:56	0.005
179	10/30/2025	12:27:56	0.005
180	10/30/2025	12:28:56	0.005
181	10/30/2025	12:29:56	0.005
182	10/30/2025	12:30:56	0.005
183	10/30/2025	12:31:56	0.005
184	10/30/2025	12:32:56	0.005
185	10/30/2025	12:33:56	0.005
186	10/30/2025	12:34:56	0.004
187	10/30/2025	12:35:56	0.005
188	10/30/2025	12:36:56	0.004
189	10/30/2025	12:37:56	0.004
190	10/30/2025	12:38:56	0.004
191	10/30/2025	12:39:56	0.004
192	10/30/2025	12:40:56	0.004
193	10/30/2025	12:41:56	0.004
194	10/30/2025	12:42:56	0.004
195	10/30/2025	12:43:56	0.004

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
196	10/30/2025	12:44:56	0.004
197	10/30/2025	12:45:56	0.009
198	10/30/2025	12:46:56	0.008
199	10/30/2025	12:47:56	0.003
200	10/30/2025	12:48:56	0.006
201	10/30/2025	12:49:56	0.005
202	10/30/2025	12:50:56	0.004
203	10/30/2025	12:51:56	0.004
204	10/30/2025	12:52:56	0.004
205	10/30/2025	12:53:56	0.003
206	10/30/2025	12:54:56	0.003
207	10/30/2025	12:55:56	0.004
208	10/30/2025	12:56:56	0.004
209	10/30/2025	12:57:56	0.004
210	10/30/2025	12:58:56	0.004
211	10/30/2025	12:59:56	0.004
212	10/30/2025	13:00:56	0.004
213	10/30/2025	13:01:56	0.003
214	10/30/2025	13:02:56	0.004
215	10/30/2025	13:03:56	0.003
216	10/30/2025	13:04:56	0.003
217	10/30/2025	13:05:56	0.003
218	10/30/2025	13:06:56	0.003
219	10/30/2025	13:07:56	0.003
220	10/30/2025	13:08:56	0.003
221	10/30/2025	13:09:56	0.003
222	10/30/2025	13:10:56	0.003
223	10/30/2025	13:11:56	0.003
224	10/30/2025	13:12:56	0.003
225	10/30/2025	13:13:56	0.003
226	10/30/2025	13:14:56	0.004
227	10/30/2025	13:15:56	0.007
228	10/30/2025	13:16:56	0.007
229	10/30/2025	13:17:56	0.008
230	10/30/2025	13:18:56	0.008
231	10/30/2025	13:19:56	0.008
232	10/30/2025	13:20:56	0.008
233	10/30/2025	13:21:56	0.008
234	10/30/2025	13:22:56	0.007

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
235	10/30/2025	13:23:56	0.008
236	10/30/2025	13:24:56	0.008
237	10/30/2025	13:25:56	0.008
238	10/30/2025	13:26:56	0.008
239	10/30/2025	13:27:56	0.008
240	10/30/2025	13:28:56	0.008
241	10/30/2025	13:29:56	0.009
242	10/30/2025	13:30:56	0.008
243	10/30/2025	13:31:56	0.008
244	10/30/2025	13:32:56	0.007
245	10/30/2025	13:33:56	0.007
246	10/30/2025	13:34:56	0.007
247	10/30/2025	13:35:56	0.007
248	10/30/2025	13:36:56	0.007
249	10/30/2025	13:37:56	0.007
250	10/30/2025	13:38:56	0.006
251	10/30/2025	13:39:56	0.006
252	10/30/2025	13:40:56	0.006
253	10/30/2025	13:41:56	0.006
254	10/30/2025	13:42:56	0.006
255	10/30/2025	13:43:56	0.006
256	10/30/2025	13:44:56	0.006
257	10/30/2025	13:45:56	0.006
258	10/30/2025	13:46:56	0.006
259	10/30/2025	13:47:56	0.006
260	10/30/2025	13:48:56	0.006
261	10/30/2025	13:49:56	0.006
262	10/30/2025	13:50:56	0.006
263	10/30/2025	13:51:56	0.006
264	10/30/2025	13:52:56	0.006
265	10/30/2025	13:53:56	0.006
266	10/30/2025	13:54:56	0.006
267	10/30/2025	13:55:56	0.006
268	10/30/2025	13:56:56	0.006
269	10/30/2025	13:57:56	0.006
270	10/30/2025	13:58:56	0.006
271	10/30/2025	13:59:56	0.005
272	10/30/2025	14:00:56	0.005
273	10/30/2025	14:01:56	0.006

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
274	10/30/2025	14:02:56	0.006
275	10/30/2025	14:03:56	0.006
276	10/30/2025	14:04:56	0.006
277	10/30/2025	14:05:56	0.006
278	10/30/2025	14:06:56	0.005
279	10/30/2025	14:07:56	0.006
280	10/30/2025	14:08:56	0.006
281	10/30/2025	14:09:56	0.006
282	10/30/2025	14:10:56	0.006
283	10/30/2025	14:11:56	0.006
284	10/30/2025	14:12:56	0.006
285	10/30/2025	14:13:56	0.006
286	10/30/2025	14:14:56	0.006
287	10/30/2025	14:15:56	0.005
288	10/30/2025	14:16:56	0.006
289	10/30/2025	14:17:56	0.005
290	10/30/2025	14:18:56	0.005
291	10/30/2025	14:19:56	0.005
292	10/30/2025	14:20:56	0.005
293	10/30/2025	14:21:56	0.005
294	10/30/2025	14:22:56	0.005
295	10/30/2025	14:23:56	0.005
296	10/30/2025	14:24:56	0.005
297	10/30/2025	14:25:56	0.005
298	10/30/2025	14:26:56	0.005
299	10/30/2025	14:27:56	0.005
300	10/30/2025	14:28:56	0.005
301	10/30/2025	14:29:56	0.005
302	10/30/2025	14:30:56	0.005
303	10/30/2025	14:31:56	0.005
304	10/30/2025	14:32:56	0.005
305	10/30/2025	14:33:56	0.005
306	10/30/2025	14:34:56	0.005
307	10/30/2025	14:35:56	0.005
308	10/30/2025	14:36:56	0.005
309	10/30/2025	14:37:56	0.005
310	10/30/2025	14:38:56	0.005
311	10/30/2025	14:39:56	0.005
312	10/30/2025	14:40:56	0.005

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
313	10/30/2025	14:41:56	0.006
314	10/30/2025	14:42:56	0.005
315	10/30/2025	14:43:56	0.005
316	10/30/2025	14:44:56	0.005
317	10/30/2025	14:45:56	0.005
318	10/30/2025	14:46:56	0.005
319	10/30/2025	14:47:56	0.005
320	10/30/2025	14:48:56	0.005
321	10/30/2025	14:49:56	0.005
322	10/30/2025	14:50:56	0.005
323	10/30/2025	14:51:56	0.005
324	10/30/2025	14:52:56	0.005
325	10/30/2025	14:53:56	0.005
326	10/30/2025	14:54:56	0.005
327	10/30/2025	14:55:56	0.005
328	10/30/2025	14:56:56	0.005
329	10/30/2025	14:57:56	0.005
330	10/30/2025	14:58:56	0.005
331	10/30/2025	14:59:56	0.005
332	10/30/2025	15:00:56	0.005
333	10/30/2025	15:01:56	0.005
334	10/30/2025	15:02:56	0.005
335	10/30/2025	15:03:56	0.004
336	10/30/2025	15:04:56	0.005
337	10/30/2025	15:05:56	0.005
338	10/30/2025	15:06:56	0.005
339	10/30/2025	15:07:56	0.005
340	10/30/2025	15:08:56	0.005
341	10/30/2025	15:09:56	0.004
342	10/30/2025	15:10:56	0.005
343	10/30/2025	15:11:56	0.004
344	10/30/2025	15:12:56	0.005
345	10/30/2025	15:13:56	0.005
346	10/30/2025	15:14:56	0.004
347	10/30/2025	15:15:56	0.004
348	10/30/2025	15:16:56	0.004
349	10/30/2025	15:17:56	0.004
350	10/30/2025	15:18:56	0.004
351	10/30/2025	15:19:56	0.004

DUSTRAK ID: FA02156 (UPWIND)			
DATA POINTS	**Date**	**Time**	**Aerosol (mg/m^3)**
352	10/30/2025	15:20:56	0.004
353	10/30/2025	15:21:56	0.004
354	10/30/2025	15:22:56	0.004
355	10/30/2025	15:23:56	0.004
356	10/30/2025	15:24:56	0.004
357	10/30/2025	15:25:56	0.004
358	10/30/2025	15:26:56	0.004
359	10/30/2025	15:27:56	0.003
360	10/30/2025	15:28:56	0.003
361	10/30/2025	15:29:56	0.004
362	10/30/2025	15:30:56	0.004
363	10/30/2025	15:31:56	0.004
364	10/30/2025	15:32:56	0.003
365	10/30/2025	15:33:56	0.004
366	10/30/2025	15:34:56	0.003
367	10/30/2025	15:35:56	0.004
368	10/30/2025	15:36:56	0.004
369	10/30/2025	15:37:56	0.003
370	10/30/2025	15:38:56	0.003
371	10/30/2025	15:39:56	0.004
372	10/30/2025	15:40:56	0.003
373	10/30/2025	15:41:56	0.003
374	10/30/2025	15:42:56	0.003
375	10/30/2025	15:43:56	0.003
376	10/30/2025	15:44:56	0.003
377	10/30/2025	15:45:56	0.003
378	10/30/2025	15:46:56	0.003
379	10/30/2025	15:47:56	0.003
380	10/30/2025	15:48:56	0.003

APPENDIX H
PROJECT SPECIFIC O&M PLAN

Operations & Maintenance Plan:
Webster Solar Gardens LLC



Montante Solar

Table of Contents:

1. Overview
2. Project Description
3. O&M Plan Goals
4. Scope of Services
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    b. Regulatory Compliance
    c. Monitoring & Performance Analysis
        i. Appendix A: Annual Solar Site Inspection Checklist
    d. Vegetation Maintenance Plan
        i. General Mowing
        ii. Pollinator Plantings
        iii. Landscape Screening
    e. Site Inspections
    f. Corrective Maintenance
5. Emergency Management
    a. Local Fire Department Training
    b. Remote Site Monitoring, Redundant Systems
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Operations and Maintenance Plan

1. Overview

This O&M Plan outlines the strategies and procedures for maintaining optimal performance, safety, and longevity of the 5 MW solar photovoltaic system. The plan includes preventative maintenance, corrective maintenance, monitoring, reporting, and safety protocols.

2. Project Description

Project Name:	Webster Solar Garden LLC
Location:	Caracas Drive, Webster NY (within Xerox Campus)
System Capacity:	5 MW AC / 6.6 MW DC
Racking Configuration:	Ground-mounted, fixed-tilt
Modules:	VSUN 610W, Bifacial Panel
Inverters:	Chint 275kW
Commissioning Date:	TBD

3. Goals of the O&M Program

- Ensure maximum system uptime and energy production
- Prolong the lifespan of system components
- Maintain safety for workers and the public
- Comply with local, state, and utility regulations
- Provide transparent reporting to stakeholders and owners

4. Scope of Services

General Operations:

- Effective implementation and control of O&M activities including document management, equipment inventories, owners and operating manuals, and warranties
- Reliable process operations to achieve the optimum balance between cost of scheduled maintenance, yield, and cash flow through the life of the system
- Specification of rules and provisions to ensure that maintenance is performed safely and efficiently

Regulatory & Environmental Compliance:

The Project Owner shall be responsible for:

- Comply with local, state, and federal laws and regulations
- Maintain stormwater controls (SWPPP compliance)
- Ensure compliance with interconnection agreement and utility requirements
- Waste handling (e.g., module replacement or oil disposal) in accordance with NYSDEC or EPA guidelines

In addition, Xerox Corporation has on-going monitoring and regulatory reporting and compliance requirements with NYSDEC in accordance with the 6 NYCRR 373 Permit.

Monitoring & Performance Analysis:

Webster Solar Garden LLC will have two monitoring platforms for redundancy. NYSERDA will have access to one of the monitoring portals and is able to pull reporting information. Webster Solar Garden LLC's O&M team will have live "smart" reporting with both systems, which will send immediate alerts.

Monitoring Platform(s):

NYSERDA Community Solar Reporting:	AlsoEnergy
Owner/O&M Team Additional Monitoring:	Chint Energy Portal

Owner, Operator, and O&M Team shall have 24/7 Remote Access to monitoring portals & 24/7 Physical Access to site

Alerts: Automated alerts for deviations in expected output, string-level faults, or inverter errors

Vegetative Maintenance Plan:

Following construction of the solar facility, disturbed grounds will be re-established with low growth / low maintenance ground cover (per requirements of the New York SPDES construction general permit coverage (≥ 80% re-vegetation). Inspection frequency during this phase of post-construction monitoring will be in accordance with the SPDES construction general permit requirements. Following the confirmation of the Project obtaining necessary re-vegetation, and close out of the SPDES construction permit, a vegetative maintenance contractor will be contracted and will be responsible for inspecting and maintaining the vegetative integrity of the solar facility. The

contractor will conduct on-site activities during the growing months at the frequency of approximately 2-3 times per year but a minimum of 2 times per year. The contractor is expected to adjust site maintenance frequency based on time of year and weather conditions. To avoid rutting, erosion, soil compaction and damage to pollinator and/or perennial plant species, weather forecasts will be consulted and on-site field inspections will be conducted prior to mowing or cutting to ensure that these practices occur when the site is able to withstand this type of activity.

Annual Landfill Cap Vegetative Site Inspections:

Vegetative clearing is required as part of the solar installation process. All trees will be hand felled. As part of annual maintenance a visual site check of the C&D Landfill will be completed to look for areas of depression or other signs of decomposition that may affect the amount of topsoil cap. Should this occur, NYSDEC will be notified within 14 days and the cover fill will be restored in accordance with the final closure plan.

General Mowing:

Mowing is a three-step process. First, the mower or bush hog trims the large areas. Second, trimmers are used to cut around structural elements and other places the mower couldn't reach. Finally, any vegetation that was thrown and stuck to the modules will be cleaned off.

Pollinator Plantings:

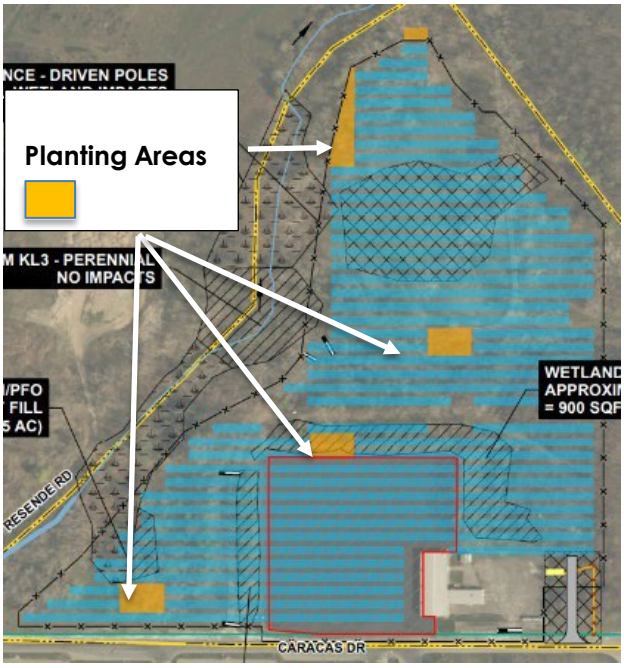
Pollinator planting sites have been strategically implemented in three areas to promote biological diversity and create additional habitat for the monarch butterfly. These areas total approximately 19,350 square feet
(0.44 acre); this will help to provide new potential monarch butterfly habitat.

Proposed pollinator plantings will be comprised of swamp milkweed (*Asclepias incarnata*), spotted trumpetweed (*Eutrochium maculatum*), New England American aster (*Symphyotrichum novae-angliae*)

During annual mowing, pollinator planting sites must be walked to ensure the concrete apron around the monitoring wells are visible. Mowing and trimming will be completed in these areas to ensure wells are fully accessible for routine/non-routine monitoring and maintenance.

The replacement of declining plants will be completed on an as-needed basis. Water and weeding will be completed during the first two growing seasons, based on weather conditions.

Pollinator Planting Areas shown below:



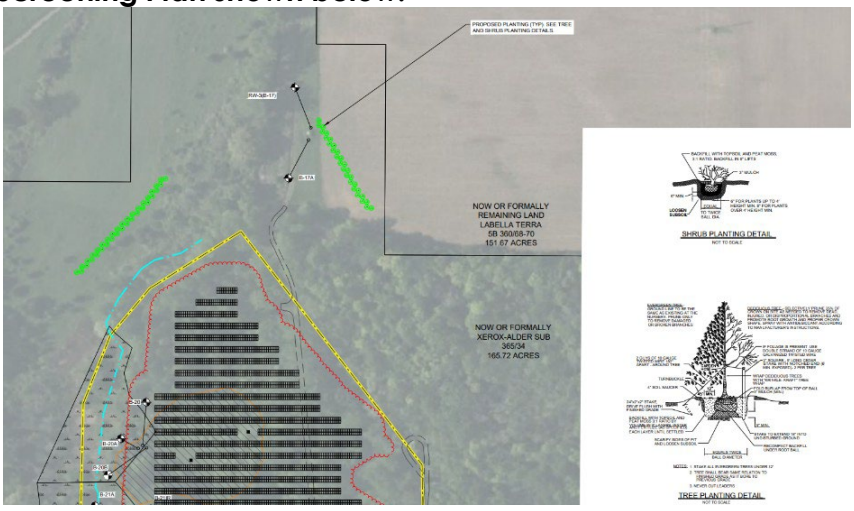
Landscape Screening:

Any landscape screening implemented as part of this project must be maintained appropriately. New plantings shall be watered as necessary during the first two growing seasons to ensure establishment and plant health.

Any dead or severely declining plants shall be replaced and be consistent with the original planting plan. Replacement of plantings shall be done in the next available planting season (spring or fall). Active watering will take place during establishment.

Landscape screening must remain in-tact throughout the life of the Solar Facility, or until the project is fully decommissioned.

Landscape Screening Plan shown below:



Site Inspection:

Routine site inspections will be conducted to ensure the Solar Facility remains in safe, efficient, and compliant operating condition. Inspections will focus on electrical equipment, structural integrity, vegetation management, stormwater controls, access roads, perimeter fencing, and landscape screening.

Inspection Frequency

- During active construction: In accordance with the New York SPDES construction general permit requirements (inspections expected to occur once every 7 calendar days at a minimum). Following active construction (and during vegetation re-growth monitoring stages prior to permit closeout), this frequency could be dropped to once every 30 days with notification to NYSDEC Division of Water and/or MS4 (Town of Webster).
- Post-construction Inspections: (Appendix A: Comprehensive, Post-Event, and Targeted Inspection Forms)
- **Comprehensive Site Inspection (post-construction):** At least semi-annually during the first two years year of operation, and annually thereafter.
- **Post-Event Inspections:** Following significant weather events (e.g., high winds, heavy snow, flooding, hail, or lightning) or other events that could damage facility infrastructure.
- **Targeted Inspections:** As needed, in response to alarms, performance issues, or community/agency concerns.

Inspection Scope: Webster Solar Gardens LLC shall share all inspection reports with Xerox Corp., who will incorporate any necessary findings into their annual NYSDEC Reporting. Each Comprehensive and Post-Event Site inspection will include, at a minimum:

1. **Solar Arrays** – Check for broken, cracked, or dislodged modules; assess racking alignment and stability.
2. **Electrical Systems** – Inspect inverters, transformers, combiner boxes, and wiring for visible damage, corrosion, or overheating.
3. **Structural & Safety Features** – Verify integrity of perimeter fencing, gates, locks, signage, and access control systems.
4. **Vegetation & Grounds** – Confirm compliance with vegetation management plan, assess potential shading issues, and check for erosion or invasive species.
5. **Stormwater Management** – Inspect drainage swales, basins, and erosion control measures to ensure proper function.
6. **Landscape Screening** – Review condition of screening plantings and replace or maintain as needed.
7. **Access Roads** – Assess condition for safe access by operations and emergency vehicles.

Xerox Site Inspections: In addition to Project Owner Site Inspections, Xerox will perform its own site inspections as part of ongoing NYSDEC compliance. As an appropriate example, a copy of Xerox's most recent annual landfill inspection and repair form can be found as part of the Xerox Site Management Plan.

Corrective Maintenance

Any deficiencies identified will be addressed promptly. Safety-related issues will be prioritized and corrected immediately. Non-critical maintenance items will be scheduled and tracked until resolution.

Response Time:

Emergency:	Immediate
Critical Failures (>50% System Down):	<48 Hours
Non-Critical Failures (<50% System Down)	<96 Hours

Documentation: All issues, repairs, and replacements documented in Maintenance Reports.

4. Emergency Management

Landfill Contingency Plan: Both the Hazardous Waste Landfill and the C&D Landfill have an Annual Inspection that includes a Monitoring & Sampling Plan as part of the Xerox Webster Facility Site Management Plan.

Local Fire Department Training: Prior to the commissioning of the solar site, Webster Solar Garden LLC will meet with the Fire Department and other local agencies to review emergency procedures. Annual training and safety refresh will also be offered.

Remote Site Monitoring: Any maintenance and emergency repair shall be performed by a qualified and licensed electrical contractor.

- The site will be monitored 24/7 using a remote DAS system
    - RG&E (Local Utility) has access to the system and is able to take the system offline immediately in case of an emergency
    - A local site contractor will be dispatched immediately in case of an emergency
    - In the case of an emergency threatening safety or loss of property, local emergency services will also be contacted

Emergency Contractor Contact:

Montante Solar
Steven Erck, Vice President
Cell: 716-609-0011
Email: serck@montante.com

Webster Fire Department: FOR EMERGENCY DIAL 911

North East Joint Fire District:

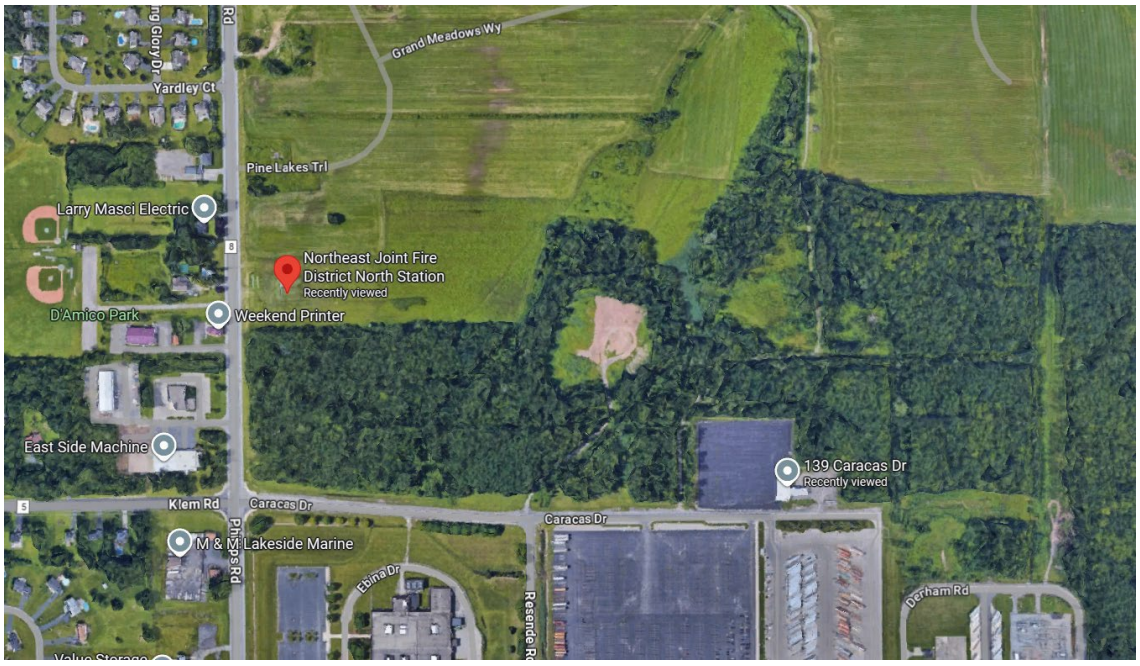
The **North East Joint Fire District** provides fire and rescue services to the east side of the town, including the village, and is provided with manpower by the Webster Volunteer Fireman's Association Inc.

Contact Information:

P.O. Box 361
Webster, NY 14580
Ph: (585) 872-9526

Station 1: Enderlin Station - 35 South Avenue
Station 2: Roseland Station - 1391 Salt Road (at Plank Road)
Station 3: North Station - 600 Phillips Road

The closest station to the project is located at 600 Phillips Road, governed by Webster's North East Joint Fire District. The location is approximately 1300ft from the project site, (approximately 2500ft of surface road to the site).



Appendix A:
Annual Operations & Maintenance Checklist

Date(s) Maintenance was performed: _____
Inspection Time periods: _____
Weather conditions during inspection: _____
Maintenance performed by:_____

Site Info
Site Name: _____
Site Address: _____
Solar Meter Reading: _____ kWh/MWH (circle one)

Visually inspect the following components, perform the listed procedures, note conditions of component, any visible damage or issues, as well as if any repair was performed.
***** Note: Take IR photography of the array BEFORE turning the system off for maintenance *****

- **1. STRUCTURE**
  - VISUAL INSPECTION
    - Verify grounding/bonding
    - Integrity of foundations (ballast, concrete, soil condition)
    - Racking system for corrosion or broken components
    - Verify operation of trackers, if applicable.
    - Debris, vegetation growth, standing water, or animal nests. (remove nests, remove debris if possible. Weed abatement and module washing are not included and performed only by special request for an additional fee)

  - MAINTENANCE
    - Replace or repair damaged mechanical components. Order replacement parts if needed.
    - Sample 1% of all racking structure mechanical connections for torque value. Record 10 sample results

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

  - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

- **2. MODULES**
  - VISUAL INSPECTION
    - Verify grounding/bonding
    - DC Connectors for damage, melting, corrosion, non-mated pairs and gaps between the male and female connectors which may allow water intrusion.
    - DC wire management, avoiding sharp edges or other potential ground fault hazards.
    - Capture IR imagery under load
    - Note degree of soiling (but do not clean)
    - Verify operation of trackers, if applicable
    - Debris, vegetation growth, standing water, or animal nests underneath panels, remedy as necessary (remove nests, remove debris if possible. Weed abatement and module washing are not included and performed only by special request for an additional fee)
  - MAINTENANCE
    - Replace any defective, damaged, or broken connectors.
    - Sample 1% of all module clamp connections for torque value. Record 10 sample results

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

  - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

- **3. INVERTERS**
  - VISUAL INSPECTION
    - Corrosion, water damage, pest nests, or metal shavings.
    - Verify grounding/bonding
    - Verify labels are correct and legible
    - Capture IR imagery under load
    - Check surge suppressors, is applicable
    - Check 20% of the dc fuses and replace as needed.
    - Inspect inverter fans.
    - Inspect and seal conduit penetrations.

  - MAINTENANCE
    - Verify operation of disconnects and switches.
    - Clean enclosure of debris, metal shavings, pest nests, and seal openings where water intrusion may occur.
    - Clean or replace air filters, if applicable.
    - Spot check for loose conductors and conduit.
    - Record and clear all faults at HMI screen.
    - Verify additional maintenance per manufacturer specifications.
    - Check 20% of conductor terminals for torque value. Record 10 sample results.

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

  - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

_____

- **4. DC COMBINERS and DC DISCONNECTS**
  - VISUAL INSPECTION
    - Corrosion, water damage, pest nests, or metal shavings.
    - Verify grounding/bonding
    - Verify labels are correct and legible
    - Capture IR imagery under load
    - If accessible to the public, ensure locks are installed.
    - Surge protectors, if applicable.
    - Check 20% of the dc fuses and replace as needed.

  - MAINTENANCE
    - Verify operation of disconnects and switches.
    - Clean enclosure of debris, metal shavings, pest nests, and seal openings where water intrusion may occur
    - Sample 20% of all conductor terminals for torque value. Record 10 sample results

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

  - DOCUMENTATION

Condition of component: _____

_____

What corrective actions were taken: _____

_____

Comments: _____

_____

- **5. AC COMBINERS and AC DISCONNECTS**
  - VISUAL INSPECTION
    - Corrosion, water damage, pest nests, or metal shavings.
    - Verify grounding/bonding
    - Verify labels are correct and legible
    - Capture IR imagery under load
    - If accessible to the public, ensure locks are installed.
    - Surge protectors, if applicable.

  - MAINTENANCE
    - Verify operation of disconnects and switches.
    - Clean enclosure of debris, metal shavings, pest nests, and seal openings where water intrusion may occur
    - Sample 20% of all conductor terminals for torque value. Record 10 sample results

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

Required torque: ___
Torque reading(s): 1.___ 2.___ 3.___ 4.___ 5.___ 6.___ 7.___ 8.___ 9.___ 10.___

  - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

- **6. WEATHER STATION / DAS**
  - VISUAL INSPECTION
    - Per manufacturer's specifications
    - Corrosion, water damage, pest nests, or metal shavings.
    - Verify grounding/bonding

- Verify labels are correct and legible
- If accessible to the public, ensure locks are installed.
- Inspect meter wiring, ensure fuses, CT shorting block, and wiring configuration are installed and operating correctly.

- MAINTENANCE
  - Verify operation of disconnects and switches.
  - Clean enclosure of debris, metal shavings, pest nests, and seal openings where water intrusion may occur
  - Clean pyranometer
  - Verify sensor operation
  - Record the net energy recorded by the meter and any tare value listed
  - Update firmware if needed. Recalibration of weather station is not included by the Service Provider. Upon Owner's written request, Service Provider can perform this activity as a Non-Standard System Service in accordance with Exhibit C of the Agreement

- DOCUMENTATION

Condition of component: _____

_____

What corrective actions were taken: _____

_____

Comments: _____

_____

- **7. TRANSFORMERS, if applicable.**
  - VISUAL INSPECTION
    - Per manufacturer's specifications
    - Corrosion, water damage, pest nests, or metal shavings.
    - Verify labels are correct and legible
    - Grounding/bonding
    - Concrete equipment pad

- MAINTENANCE
  - Clean enclosure of debris, metal shavings, pest nests, and seal openings where water intrusion may occur

- DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

- **8. OTHER**
  - **Raceways and Conduits**
    - VISUAL INSPECTION
      - Verify dc labels are legible
      - Grounding/bonding on metallic conduit.
      - Ensure expansion joints are not overextended or collapsed
      - Check for damage in high-traffic paths where landscapers may potentially collide with equipment

    - MAINTENANCE
      - Repair, supplement, or seal as necessary.

    - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

- **Site/Grounds**
  - VISUAL INSPECTION
    - Verify padlocks on all personnel gates and entry points.
    - Verify fence continuity and grounding/bonding
    - Verify Signage (general safety, owner and service provider contact info) is correct and visible on entry points.
    - Ground erosion by concrete equipment pads, vertically-driven post and piles.
  - MAINTENANCE
    - Replace locks if required
    - Order replacement signage if inadequate.
  - DOCUMENTATION

Condition of component: _____

What corrective actions were taken: _____

Comments: _____

Signature: _____ **Date:** _____

APPENDIX I
C&D LANDFILL GEOTECHNICAL REPORT (REVISED FEBRUARY 2026)

H & A OF NEW YORK ENGINEERING AND
GEOLOGY, LLP
260 E. Main Street
Suite 2100
Rochester, NY 4604
585.359.9000

Revised February 19, 2026
December 10, 2025
File No. 0205399-000-006-01

TM Montante Solar Development, LLC
2760 Kenmore Avenue
Buffalo, New York 14150

Attention: Katie Soscia, Director of Project Development

Subject: **Geotechnical Assessment for the C&D Soil-Capped Landfill**
Webster Solar Garden Project
750/800 Phillips Road
Webster, New York

Dear Ms. Soscia,

H & A of New York Engineering and Geology, LLP (Haley & Aldrich of New York) was retained by TM Montante Solar Development, LLC (Montante) to perform this geotechnical assessment of landfill cap stability at the proposed Webster Solar Garden Project (Project) located at 750/800 Phillips Road, Webster, Monroe County, New York.

The purpose of this assessment is to address the New York State Department of Environmental Conservation (NYSDEC) Division of Materials Management (DMM-4) Guidance: Photovoltaic Solar Projects at Closed Solid Waste Landfills.[1] Specifically, the assessment evaluates potential settlement and loading impacts associated with the installation of photovoltaic (PV) arrays on an existing soil-capped former construction & demolition (C&D) closed landfill.

The objectives of this assessment are to:

- Evaluate the settlement of the landfill cap based on the plate load testing program undertaken and the proposed PV loads and temporary construction equipment loads.
- Assess the potential impact to the landfill cap system from the PV array foundation units and the foundation unit installation methods.
- Review the DMM-4 requirements for documentation of landfill stability as it relates to the proposed development loads.

[1] New York State Department of Environmental Conservation (NYSDEC) Division of Materials Management DMM-4 Photovoltaic Solar Projects at Solid Waste Landfills (dated 1/4/2023) policy is being utilized by NYSDEC staff as guidance to review the Change Of Use Notice to allow PV Solar development under Xerox's Part 373 Permit.

Site Background and Landfill Description

The proposed solar development project will be located within the northern, unoccupied portions of Xerox Corporation's (Xerox's) Webster Campus (Tax Parcel No. 065.02-1-40.11). The project area includes an approximately 1.5-acre soil-capped inactive, closed C&D landfill. The landfill reportedly operated for approximately 15 years and primarily received grass clippings, leaves, and brush debris. The landfill was closed under direction of the NYSDEC in 1988 and is designated Solid Waste Management Unit (SWMU) #88 – Former Disposal Area North of W200 and described as C&D landfill under the Xerox Part 373 Resource Conservation and Recovery Act (RCRA) Permit (Site Code 828178); this SWMU is classified as No Further Action Required. The location of SWMU #88 is shown on Figure 1.

Remedial closure activities were reportedly completed in 1988 and included installation of a multi-layered engineered soil cap designed to limit infiltration and promote positive drainage. The cap system consists of approximately 6 inches of topsoil and vegetative cover overlying 18 inches of low-permeability soil ($K \leq 1x10^{-5}$ centimeters per second) placed atop the graded waste materials. The landfill slopes were graded to a maximum of 2H:1V, with a minimum 3 percent grade at the landfill center to facilitate drainage and minimize erosion and infiltration. Current grades across the landfill reach approximately 8 percent in localized areas, primarily along the northern slopes. The landfill closure design plan is included as Appendix A.

The landfill is maintained by Xerox under its existing RCRA permit and *Site Management Plan*. The surface is predominantly grass-covered, with limited areas of brush and small trees. Two vent pipes, installed as part of the closure, are present within the landfill footprint, along with a cluster of groundwater monitoring wells (B-20, 20A, and 20B) that are routinely sampled as part of compliance monitoring under the NYSDEC RCRA permit.

As part of Project construction, Montante plans to remove existing brush and small trees from the landfill surface. Stumps will be cut or ground to grade or slightly below the surface, and affected areas will be re-vegetated as necessary to maintain surface stability and erosion control. If the soil cover is impacted by tree removal, the cover will be re-established to closure requirements, including required cover material and sloping.

In support of project planning and NYSDEC coordination, Xerox conducted a soil cover investigation in October 2025 to verify the thickness and condition of the existing soil cover system. At the five test locations, Haley & Aldrich of New York observed the 24-inch cover, consisting of approximately 6 inches of topsoil overlying 18 inches of low-permeability soil, consistent with the original closure design. The soil cover investigation field observations are provided in Appendix B. The test pit locations are shown on Figure 1.

PROPOSED SOLAR DEVELOPMENT

The proposed development consists of a 6.6-megawatt, direct current, fixed-tilt PV system utilizing Geoballast-mounted foundations placed on the ground surface to avoid penetration of the existing landfill cap. No penetrations of the landfill soil cap are proposed as part of the PV Project. Montante

may utilize a compacted gravel leveling surface pad beneath the Geoballasts, if determined necessary during construction (along the northern edge of the C&D landfill where localized grades of up to 8 percent exist). These triangular-section gravel pads are a best management practice on slopes up to 15 percent (8.5 degrees) and may require between 0.1 to 0.2 cubic yards of gravel (where utilized) depending on degree of slope. Based on layout and design, it is currently estimated that approximately 40 gabion basket locations may require these leveling pads and that the average thickness of the gravel at these locations would be up to approximately 4-inches thick. Where utilized, the gravel leveling pads could total up to an additional 800 pounds each.

The Geoballast (gabion-style foundation) themselves (without gravel leveling pads) will contain approximately 2,250 pounds of stone aggregate, with an estimated total permanent load at ballast foundations of approximately 450 pounds per square foot (psf) or 3.1 pounds per square inch (psi) without the gravel leveling pads and up to 507 psf/ 3.5 psi at locations with gravel leveling pads, including the racking components, modules, and anticipated snow/wind design loads.

The maximum temporary construction load, which is based on the heaviest proposed construction equipment (a Kubota SVL97-2 track loader) is approximately 650 psf (4.5 psi), which is below the 10-psi threshold established by NYSDEC for construction over capped landfills.

Geotechnical Testing Procedures

To assess stability of the C&D landfill surface, Haley & Aldrich of New York subcontracted CME Associates, Inc. (CME) to perform static load testing in general conformance with ASTM D1194-94 – Standard Test Method for Bearing Capacity of Soil for Static Load and Spread Footings. Although ASTM D1194-94 was formally withdrawn in 2003, it continues to be referenced for informational and comparative purposes within the geotechnical field. A new standard (ASTM WK91344) addressing static soil response under loading is currently under development but has not yet been published.

Field testing for this Project was conducted on October 29 and 31, 2025; no testing was performed on October 30 due to inclement weather. Testing employed a rubber-tracked excavator, a 13.8-ton hydraulic jack assembly, and a 1-inch-thick, 12-inch-square steel plate instrumented with two dial gauges mounted on a separate reference beam. At the test locations, an 18-inch-wide excavator bucket with a blade tip was utilized to remove the grass and topsoil (to an estimated depth of 3-6-inches below ground surface). A square point shovel was utilized to expose and level the undisturbed underlying cover soil. Coarse gravel, where present in the soil matrix, was removed and where voids were present these were filled in with native soil and compacted by hand. There was minimal amounts of gravel removed from the locations, and all materials removed were replaced in kind following testing. The steel plate was then placed directly on the level subgrade, and the dial indicator was zeroed prior to loading. The subgrade was tested as opposed to the ground surface in order to perform the test on an even, flat surface and avoid data irregularities from minor variations in ground surfaces. The load was set up using the base of the excavator boom arm (the excavator King Post). This configuration was used for all tests. Incremental loads were applied in 200-pound steps, with each increment maintained for approximately 15 minutes while deflection readings were recorded at each interval. Testing was performed at three locations, CDL-1 through CDL-3, within the C&D landfill footprint (see Figure 1). The

maximum applied load was 2,000 psf, approximately three times greater than the anticipated construction equipment loading, providing a factor of safety for evaluation purposes. The measured corrected settlement was 0.0226 and 0.0367 inches at CDL-1 and CDL-3, respectively.

The ground conditions at load test CDL-2, were wet due to the previous day's inclement weather as indicated on Figures 2 and 3 in Appendix C. Negative settlements (or uplift) were observed during testing. In follow up conversations with CME:

- It is possible that the reference beam to which the dial gauges were connected may have experienced uplift due to wind during the load test.
- It is also possible that the 2x4 wooden stakes (driven through the top few inches of soil) that were utilized to create a flat surface for the reference beam to secure that in place may have shifted slightly due to softened surface soils from the inclement weather. CME suspects that the stakes shifted during the test given the saturated ground conditions during this test.
- A combination of both above is also possible.

For purposes of this letter report, the results for CDL-2 will not be used.

The load versus settlement curves are included as Appendix C. Following the termination of each test, the excavation spoils were replaced and compacted back in each of the testing locations.

Variations from ASTM D1194-94 were implemented during testing. A single steel bearing plate was used rather than multiple bearing plates of varying diameters. This deviation from the standard was documented and evaluated for applicability to site conditions. CME determined that use of a single bearing plate was appropriate for this test program based on the small test loads as compared to typical test loads under the ASTM standard. A 13.8-ton jack assembly, rather than the 50-ton assembly referenced in the standard, was used; CME determined this to be adequate for the relatively small test loads involved. The reference beam, 8 feet in total length, was supported 4 feet from the center of the loaded area, instead of the 8 feet specified in Section 4.5 of the ASTM standard due to the reduced scale of the load test set up overall. This modification was not expected to affect the testing accuracy, given the limited load magnitude.

Evaluation for Division of Materials Management (DMM-4) Guidance

Five subsurface explorations (test pits) were conducted within the boundaries of the soil-capped landfill as discussed previously. These investigations were confined to shallow test pits to evaluate the soil cover thickness; copies of the field observations are provided in Appendix B.

Boring logs for a cluster of groundwater monitoring wells (B-20, 20A, and 20B cluster), that were installed in 1985 as part of compliance monitoring under the NYSDEC RCRA permit, are provided in Appendix D.

Our assessments of potential settlement, slope instability, and slippage are based on the following:

- available topographic data (past and present);
- historical boring logs from the B-20 well cluster within the landfill footprint;
- findings from the landfill cover test pit investigations;
- information and product specification sheets from the Geoballast manufacturer
- observations made during the static load testing program.

SLOPE STABILITY ANALYSIS

The ground surface elevations (El.) for the C&D soil-capped landfill range between approximately El. 370 and El. 381 feet above mean sea level. The maximum aerial extents of the irregularly shaped C&D capped landfill footprint is approximately 400 feet by 200 feet. The ground surface was graded as part of the closure, as noted above, with side slopes graded to an approximate 7 horizontal to 1 vertical (7H:1V) or flatter, and a minimum 3 percent slope at the center to promote surface water runoff. The section used for slope stability analysis generally bisects the landfill from the northwest to the southeast. Figure 2 generally shows the section in plan view that was evaluated.

Haley & Aldrich of New York performed a slope stability analysis using SLIDE. A summary of the evaluations is provided below. The results are provided in Appendix E.

- Based on the landfill geometry described above, the calculated factors of safety ranged from 4 to 13 for the existing landfill, as-is, with no additional load. This range of factors of safety are greater than the recommended minimum factor of safety of 1.5.
- A slope stability analysis for the temporary construction load was also performed to assess the impact of equipment load. Using the equipment loading data provided (for a Kubota SVL97-2 track loader – which is the heaviest construction equipment which will operate on the landfills) and distributing it over the area of the landfill, the resulting factor of safety was greater than 4.
- A slope stability analysis for the proposed solar development load was also performed. The 450 psf Geoballast load was distributed over the area of the landfill as a 22.4 psf load due to the spacing of the Geoballast. The resulting factor of safety was greater than 4.

SETTLEMENT ANALYSIS

The estimated contact pressure for each Geoballast foundation is approximately 450 psf/3.1 psi, which includes the racking components, modules, and anticipated snow/wind design loads. With the gravel leveling pads included (only estimated to be needed at 40 locations), the estimated contact pressure would be 507 psf/3.5 psi. The applied load for the plate load testing was 2,000 pounds over a 12-inch by 12-inch plate, which is an equivalent applied pressure of 2,000 psf. The settlement observed due to the 2,000 psf load was approximately 0.02 to 0.04 inches.

Additionally, Haley & Aldrich of New York performed a settlement analysis using SETTLE3 to evaluate settlements under the proposed loading conditions. A summary of the evaluations is provided below. The results are provided in Appendix E.

- A series of uniform loads of 450 psf applied over the Geoballast area. The calculated settlements were less than ¼ inch at the edge of an individual Geoballast and the center of the Geoballast.
- A uniform load of 22.4 psf along a row of solar panels distributed over the panel's tributary area, which was calculated using the east–west distance between baskets and the north–south spacing between solar panel rows. The calculated settlements were very small, estimated to be less than 1/10 inch.
- A settlement analysis was also performed for the temporary construction loading, assuming a 650 psf applied load over the equipment footprint. The calculated settlement for this case was up to about a ½ inch. We note that the settlement is unlikely to occur instantaneously as the equipment traverses the site.
- Based on the load test results, and the settlement analyses based on historical information, we anticipate that the estimated settlements will be less than 1 inch.

DIFFERENTIAL SETTLEMENT

The proposed Geoballast foundation system has a tolerance for differential settlement of 1 inch of displacement over 1 foot of length. The Geoballast foundation system also has telescoping posts that can be adjusted, if needed, to reposition the solar panels in the event that differential settlement occurs. Additionally, Haley & Aldrich of New York performed settlement analysis using SETTLE3 to evaluate settlement under actual loading, as described in the settlement analysis section above.

The calculated differential settlement across the proposed Geoballast is 0.03 inches. A separate settlement analysis was also conducted for temporary construction loading (equipment loads), which resulted in a calculated differential settlement of less than ¼ inch. Based on the load testing results, the settlement analyses, and the anticipated load from the proposed construction, we expect differential settlement across the site to remain below a ½ inch. See Appendix E for settlement results of different cases.

BEARING CAPACITY ANALYSIS

Haley & Aldrich of New York conducted bearing capacity calculations to assess the potential for punching shear failure. Assuming a 2-foot-thick layer soil over the landfill (inclusive of low permeability soils), the calculated allowable bearing capacity was about 1,000 psf assuming a factor of safety of 3. When compared to the Geoballast contact pressures of 450 to 507 psf, the calculated factor of safety for the permanent condition is between 1.7 to 1.9, which is considered acceptable. See Appendix E for detailed bearing capacity calculations.

Discussion and Conclusions

The test results and analyses indicate that:

- The landfill cap can accommodate loads associated with the PV Geoballasts and array equipment (approximately 450-507 psf) and temporary construction equipment loads (approximately 650 psf).
- Temporary construction equipment loads and permanent loads from the system are within allowable limits (less than 10 psi) per NYSDEC DMM-4 guidance.
- No visual damage to the soil surface occurred during testing. Refer to the photographs in Appendix B.

Based on the load test results, we anticipate settlement of the soil-capped C&D landfill will be less than 1 inch. under the future loads. Accordingly, adverse impacts to landfill stability or cap integrity are not anticipated from the proposed development, provided that construction equipment, methods of construction, construction loads, and site conditions remain consistent with those evaluated herein. Localized condition variability may occur; however, given the soil cap construction materials, proposed Project loads, testing results, and factor of safety applied, we expect this assessment's conclusions address the DMM-4 guidance requirements for the proposed Change of Use at the site. Ongoing monitoring of the cap condition will continue post-construction, as part of the NYSDEC approvals for the Project.

Closing

We appreciate the opportunity to provide engineering services on this Project. Please do not hesitate to contact us if you have any questions or comments.

Sincerely yours,
H & A OF NEW YORK ENGINEERING AND GEOLOGY, LLP

Carrie A. Layhee, P.E. (NY)
Partner

Steven H. Phillips P.G. (NY)
Senior Associate

Enclosures:
Report Certification
Figure 1: Test Location Plan
Figure 2: C&D Landfill Existing Topography
Appendix A: C&D Landfill Closure Design Plan
Appendix B: Soil Cover Assessment – Test Pit Logs and Photolog
Appendix C: CME Associates, Inc. Test Data and Load vs. Settlement Curve Results and Field Photographs

Appendix D: B-20 Well Cluster Boring logs
Appendix E: Stability and Settlement Analysis Results

https://haleyaldrich.sharepoint.com/sites/TMMontanteSolarDevelopmentLLC/Shared Documents/0205399.Montante Xerox Webster Solar/Deliverables/Geotech/Task 1 - Geotech Scope/Reporting Deliverables/REPORT REVISIONS - 2026/C&D LANDFILL/2026_0219_GeotechReport_C&DLandfill-FR1.docx

REPORT CERTIFICATION

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Consulting and Engineering Firm: H & A of New York Engineering and Geology, LLP

Name: Carrie A. Layhee, P.E. (NY)

Title: Partner

Signature: Carrie A. Layhee

Date: February 24, 2026



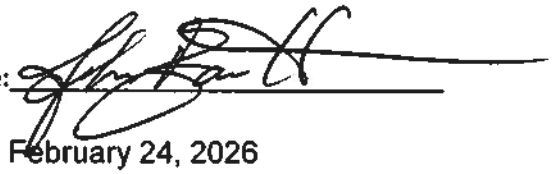
REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Site Owner: Xerox Corporation

Name: John Barrett

Title: Manager - Western Hemisphere Real Estate Portfolio

Signature: 

Date: February 24, 2026

Docusign Envelope ID: 78989487-F70B-4024-ABAD-B6E1F7E1C3A8

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Operator (Project Developer): TM Montante Solar Development LLC

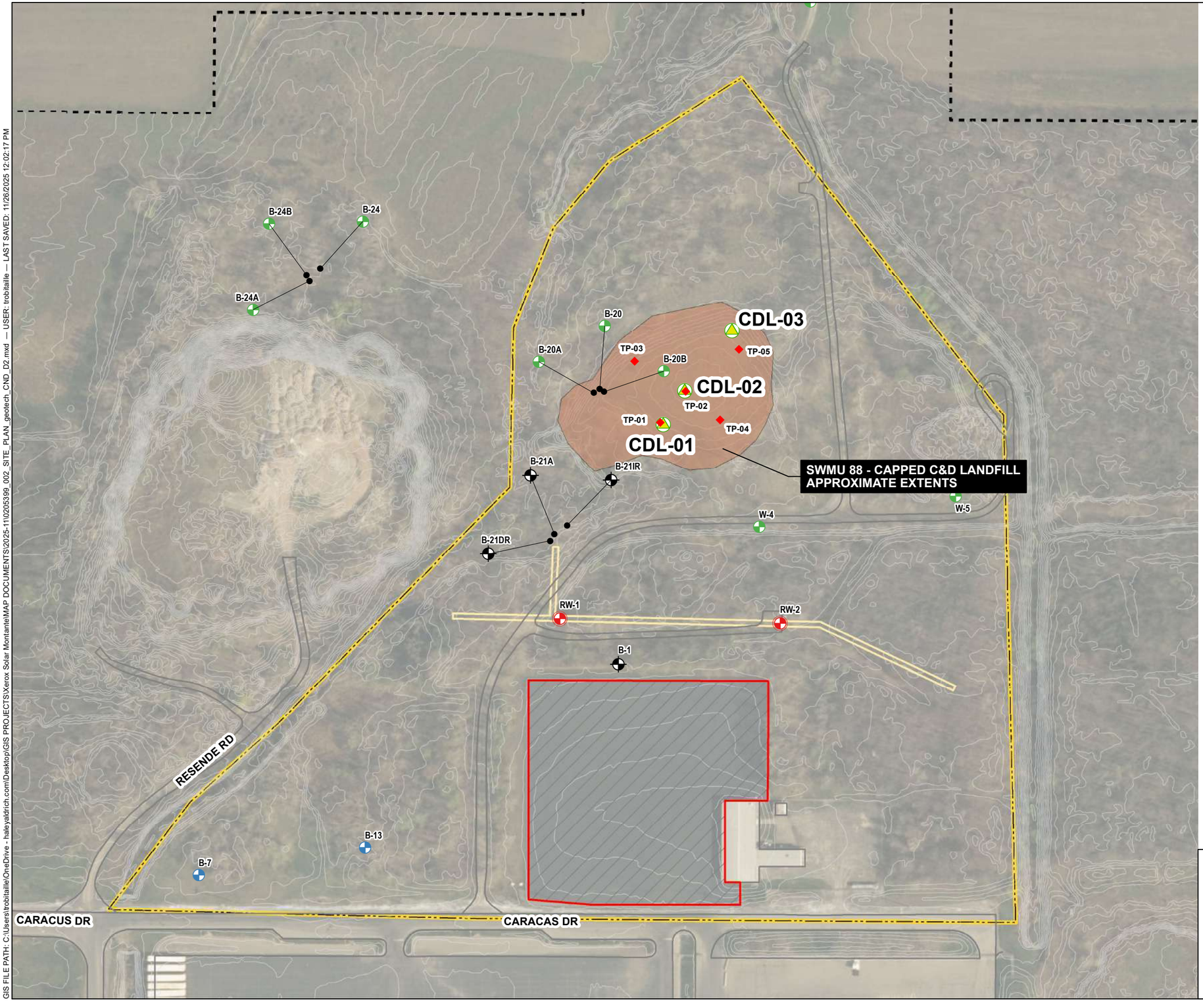
Name: Dan Montante

Title: Member

Signature: Signed by: Dan Montante 842D93661ABC439...

Date: 2/25/2026

FIGURES



LEGEND

- GEOTECHNICAL TEST LOCATIONS
- COVER INVESTIGATION TEST PIT LOCATIONS
- SOLAR PROJECT STUDY AREA
- XEROX PROPERTY BOUNDARY
- 1 FT TOPOGRAPHIC CONTOURS
- APPROX EXTENTS OF C&D LANDFILL
- CAPPED INACTIVE LANDFILL

OTHER SITE FEATURES

- GROUNDWATER PUMPING WELL
- GROUNDWATER QUALITY MONITORING (GQM) WELL
- TRIGGER WELL
- LINE OF COMPLIANCE WELL
- WELL LEADER LINES
- BLASTED TRENCH IN BEDROCK

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY
3. WELL LOCATION DATA SOURCE: XEROX CORPORATION
4. TOPOGRAPHIC DATA SOURCE: MONROE COUNTY LIDAR DOWNLOAD SITE

N

0 300

SCALE IN FEET

HALEY ALDRICH

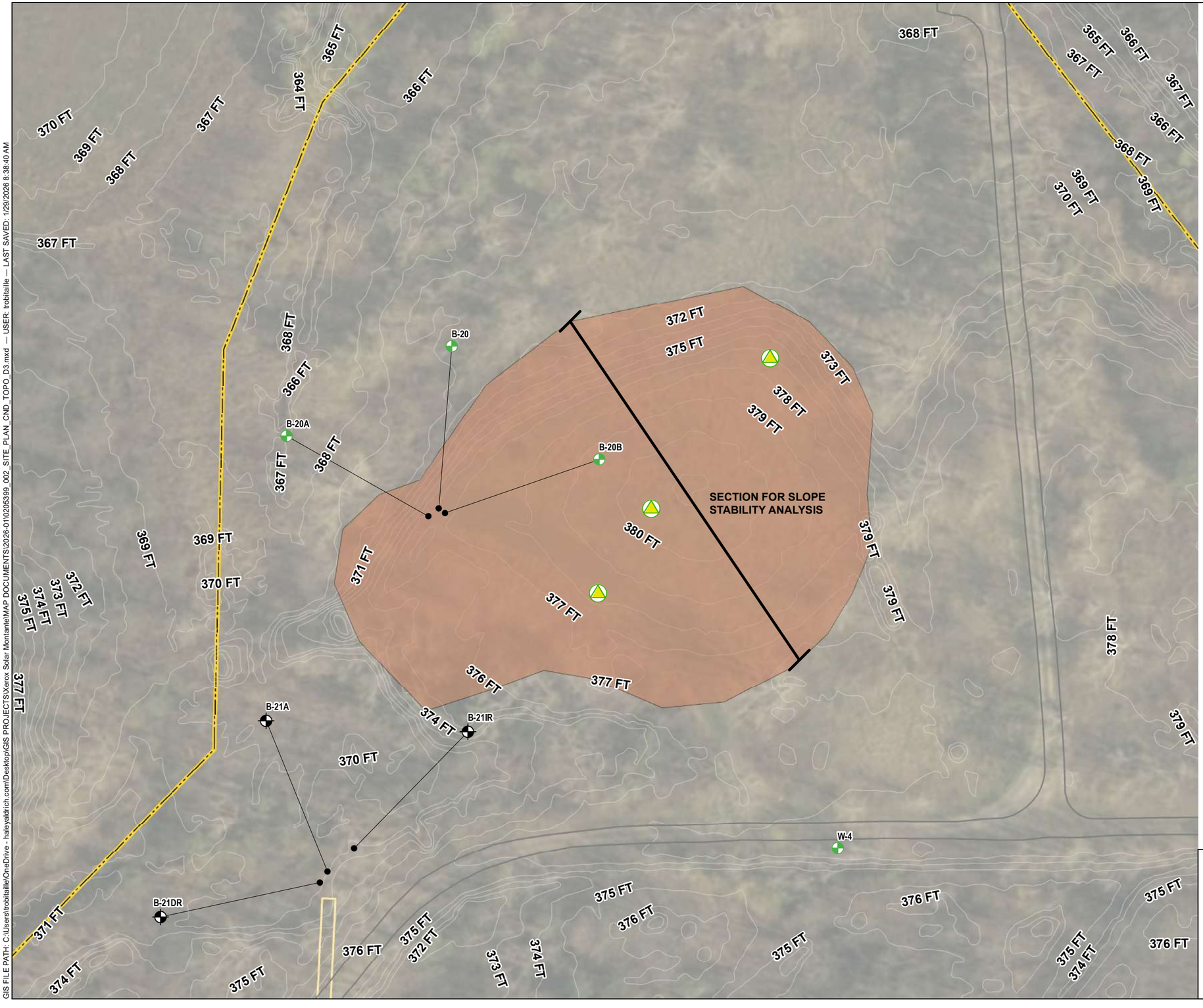
TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

TEST LOCATION PLAN

SCALE: AS SHOWN
LAST UPDATED: NOVEMBER 2025

FIGURE 1

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2025-1110205399_002_SITE_PLAN_geotech_CND_D2.mxd — USER: trobitaille — LAST SAVED: 11/26/2025 12:02:17 PM



LEGEND

- SOLAR PROJECT STUDY AREA
- 1 FT TOPOGRAPHIC CONTOURS
- TEST LOCATIONS
- APPROXIMATE EXTENTS OF C&D LANDFILL
- GROUNDWATER QUALITY MONITORING (GQM) WELL
- LINE OF COMPLIANCE WELL
- WELL LEADER LINES
- BLASTED TRENCH IN BEDROCK

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY
3. WELL LOCATION DATA SOURCE: XEROX CORPORATION
4. TOPOGRAPHIC DATA SOURCE: MONROE COUNTY LIDAR DOWNLOAD SITE

N

0 60

SCALE IN FEET

HALEY ALDRICH

TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

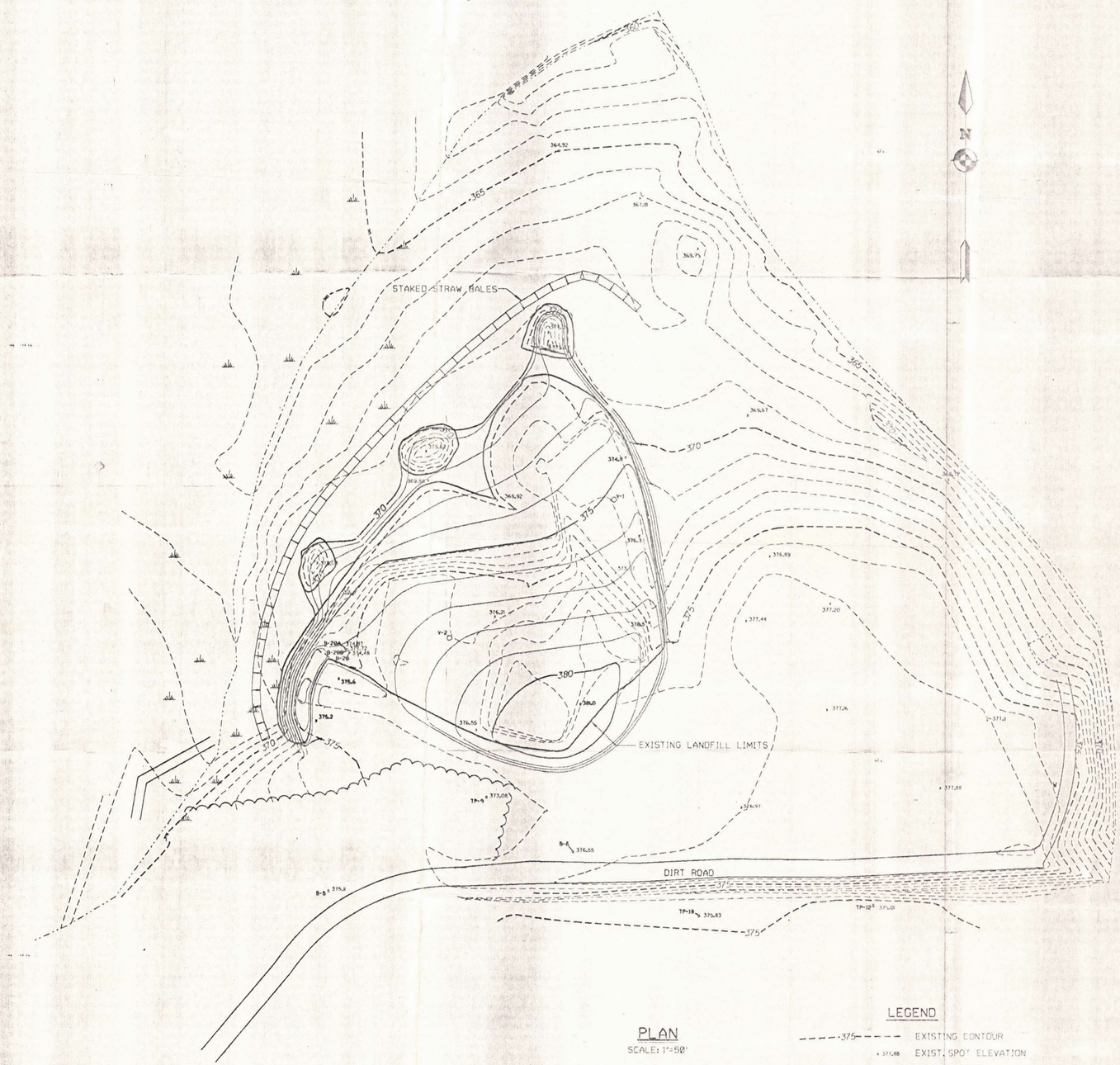
C&D LANDFILL
EXISTING TOPOGRAPHY

SCALE: AS SHOWN
LAST UPDATED: JANUARY 2026

FIGURE 2

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2026-0110205399_002_SITE_PLAN_CND_TOPO_D3.mxd — USER: trobitaille — LAST SAVED: 1/29/2026 8:38:40 AM

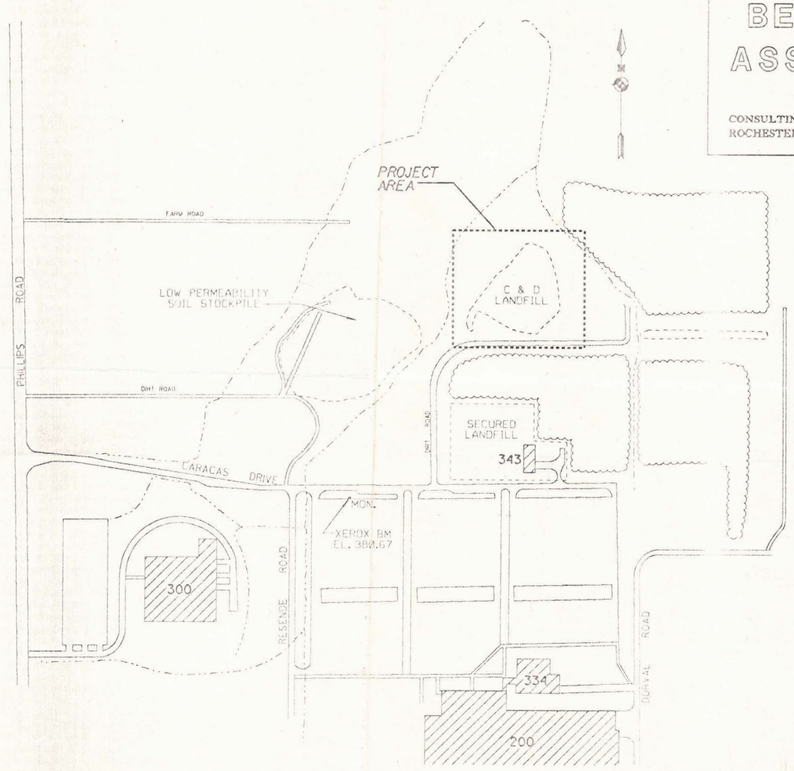
APPENDIX A
C&D Landfill Closure Design Plan



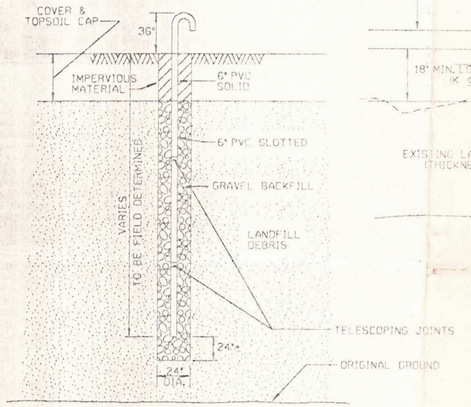
PLAN
SCALE: 1"=50'

LEGEND

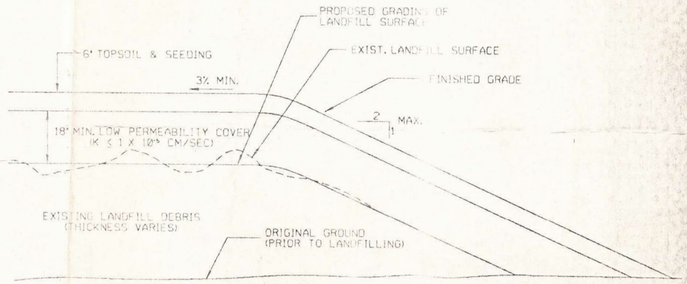
- EXISTING CONTOUR
- EXIST. SPOT ELEVATION
- PROPOSED CONTOUR
- PROPOSED SPOT ELEVATION
- EXIST. MONITORING WELL
- PROPOSED PVC VENT PIPE
- EXIST. LANDFILL LIMITS
- STAKED STRAW BALES



LOCATION PLAN
NO SCALE



LANDFILL VENT DETAIL
NO SCALE



TYPICAL SECTION
NO SCALE



12/14/88

N
ORIENTATION

BERGMANN ASSOCIATES
CONSULTING ENGINEERS & PLANNERS
ROCHESTER NEW YORK

ISSUE	DATE	REVISIONS	BY

XEROX FACILITIES OPERATIONS
JOSEPH C. WILSON CENTER FOR TECHNOLOGY
ROCHESTER, NEW YORK 14644

LAND/SITE
C & D LANDFILL CLOSURE
PLAN & DETAILS

DRAWN BY	DATE	SHEET NO	PROJECT NO
DMB	12-5-88	1 OF 1	
CHKD / DATE	CHKD / DATE		
APPROV / DATE	APPROV / DATE	ENVIRONMENTAL APPROV / DATE	DRAWING NO / ISSUE
SCALE	FILE	ENTRY NO	L-1 / A
AS NOTED			

APPENDIX B
Soil Cover Assessment – Test Pit Logs and Photolog

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-01

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor, moist to dry -TOPSOIL-
		0.5	ML	Light-brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry, interbedded red sandy SILT
1				
2	0.0	2.0		BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-02

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Dark brown TOPSOIL, no structure, no odor, moist to dry -TOPSOIL-
		0.5	ML-SM	Dark brown silty SAND (ML-SM), mps 6 in., no structure, no odor, moist to dry
1			ML-SM	Similar as above, except light-brown Note: 4 in brick fragment observed at 18 in. bgs
2		2.0		BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-03

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Dark brown TOPSOIL, no structure, no odor, moist to dry
				-TOPSOIL-
		0.5	ML	Dark brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.8; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-04

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor, moist to dry
				-TOPSOIL-
	0.0	0.5	ML	Light brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry, traces of red sandy SILT
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.9; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

HALEY ALDRICH

TEST PIT LOG

Test Pit No. TP-05

Project	XEROX - C&D LANDFILL - COVER INVESTIGATION
Location	WEBSTER, NEW YORK
Client	XEROX CORPORATION
Contractor	NA
Equipment Used	Hand Auger & Shovel
File No.	134970-105
H&A Rep	K. BARTLETT, Z. LYNCH
Date	10/30/2025
Weather	Sunny

Ground El.:
El. Datum:
Location: See Plan
Groundwater depths/entry rates (in./min.):

Depth (ft)	Sample Data / PID Readings (ppm)	Stratum Change Elev./ Depth (ft)	USCS Symbol	VISUAL-MANUAL IDENTIFICATION AND DESCRIPTION (color, natural grain size and artificial component percentage estimates, maximum particle size, manual test properties, structure, odors, moisture, other descriptions and observations GEOLOGIC INTERPRETATION)
0	0.0		OL/ OH	Brown-dark brown TOPSOIL, no structure, no odor
				-TOPSOIL-
	0.0	0.5	ML	Light brown to brown sandy SILT (ML), mps 5 in., no structure, no odor, moist to dry
1				
		2.0		
2				BOTTOM OF EXPLORATION AT 2.0 FT

Obstructions:

Remarks: Oxy: 20.0; LEL: 0%; CO: 0 ppm; H2S: 0.0 ppm; CHy: 0% vol

Field Tests

Dilatancy	R - Rapid S - Slow N - None
Toughness	L - Low M - Medium H - High
Plasticity	N - Nonplastic L - Low M - Medium H - High
Dry Strength	N - None L - Low M - Medium H - High V - Very High

Standing Water in Completed Pit

at depth ft
measured after hours elapsed

Boulders

Diameter (in.)	Number	Approx. Vol. (cu.ft)
12 to 24		=
over 24		=

Test Pit Dimensions (ft)

Pit Length x Width (ft) 1 x 1
Pit Depth (ft) 2.0

NOTE: Soil identification based on visual-manual methods of the USCS system as practiced by Haley & Aldrich, Inc.

HA TESTPIT-09-W PID PLOG-HA-LIB09-BOS STANDARD ONLY-MARCH 2024.GLB HA-TP09+FENCE.GDT \\HALEYALDRICH.COM\SHARE\CF\PROJECTS\134970\GINT\134970-105-TP-D1.GPJ Nov 24, 25

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 1
Test Pit- TP-01 - hand augured to 2 feet bgs.



Photo Number: 2
Typical cobble sizes greater than 3 inches observed between 10-15% makeup of soil matrix within test pit excavations.

Note: Cobbles were observed distributed throughout the soil matrix (6 inch bgs to 24-inch bgs) and were not limited to one specific horizon.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 3
Typical soil matrix observed in test pit excavations



Photo Number: 4
Test Pit - TP-02 - Brick fragment observed at 1.5 feet bgs.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 5
Test Pit - TP-02 - hand augured to 2 feet bgs.



Photo Number: 6
Test Pit - TP-03 - hand augured to 2 feet bgs.

PHOTO LOG

Project:	Xerox (Webster) - C&D Landfill Cover Soil Assessment	**Date of Photos:**	10/30/2025
Client:	Xerox Corporation	**Weather:**	Sunny
Location:	Webster, NY	**Temperature:**	50
Staff:	K. Bartlett, Z. Lynch	**File No.**	134970-105



Photo Number: 7
Test Pit - TP-04 - hand augured to 2 feet bgs.

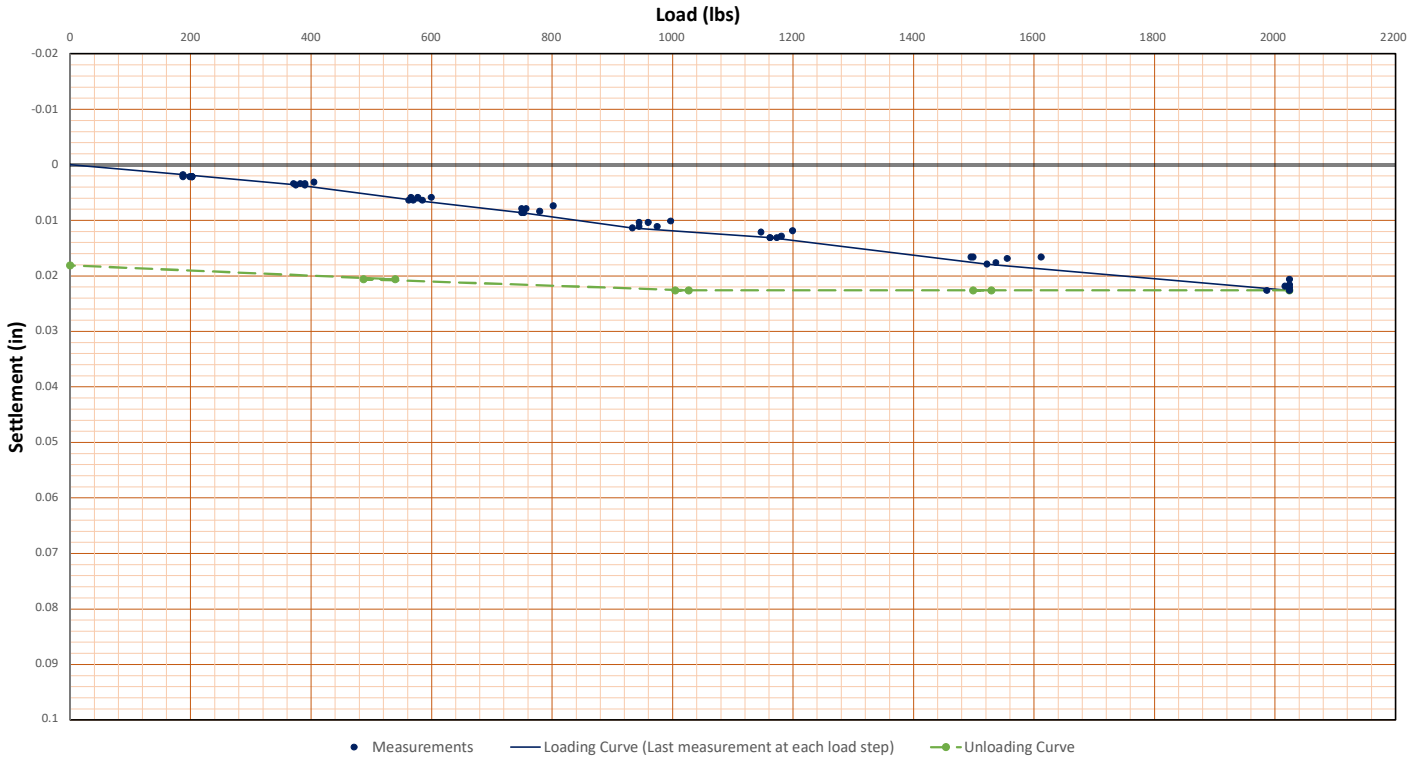


Photo Number: 8
Test Pit - TP-05 - hand augured to 2 feet bgs.

APPENDIX C
CME Associates, Inc.
Test Data and Load vs. Settlement Curve Results and Field Photographs

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3]	Measured Load (lbs)
200 lbs	0	2.1	202
	1	2.1	202
	2	2.1	202
	4	2.1	199
	8	2.1	187
	15	1.7	187
400 lbs	0	3.1	405
	1	3.4	390
	2	3.4	382
	4	3.4	371
	8	3.6	390
	15	3.6	375
600 lbs	0	5.9	600
	1	5.9	577
	2	5.9	566
	4	6.4	585
	8	6.4	562
	15	6.4	570
800 lbs	0	7.4	802
	1	7.9	757
	2	7.9	750
	4	8.4	780
	8	8.6	753
	15	8.6	750
1000 lbs	0	10.1	997
	1	10.4	960
	2	10.4	945
	4	11.1	975
	8	11.1	945
	15	11.4	933
1200 lbs	0	11.9	1199
	1	12.1	1147
	2	12.9	1181
	4	13.1	1173
	8	13.1	1162
	15	13.1	1162
1400 lbs	0	Note 4	
	1		
	2		
	4		
	8		
	15		
1600 lbs	0	16.6	1612
	1	16.6	1499
	2	16.6	1496
	4	16.9	1555
	8	17.6	1537
	15	17.9	1522
1800 lbs	0	Note 4	
	1		
	2		
	4		
	8		
	15		
2000 lbs	0	20.6	2024
	1	21.6	2024
	2	21.9	2017
	4	22.1	2024
	8	22.6	1987
	15	22.6	2024
1500 lbs	0	22.6	1499
	1	22.6	1529
1000 lbs	0	22.6	1005
	1	22.6	1027
500 lbs	0	20.6	487
	1	20.6	540
0 lbs	0	18.1	0

Load vs Settlement Curve (Test Location CDL-1)



Burmister Classification: Orange/Brown cmf SAND, some cmf GRAVEL, little SILT, trace CLAY

Sample Depth: 1 foot
Sample Date: 10/31/2025
Sample Time: 2:45 PM to 5:00 PM
Temperature and Weather: Drizzle and Wind, ~40-45°F
Set Load: 120 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

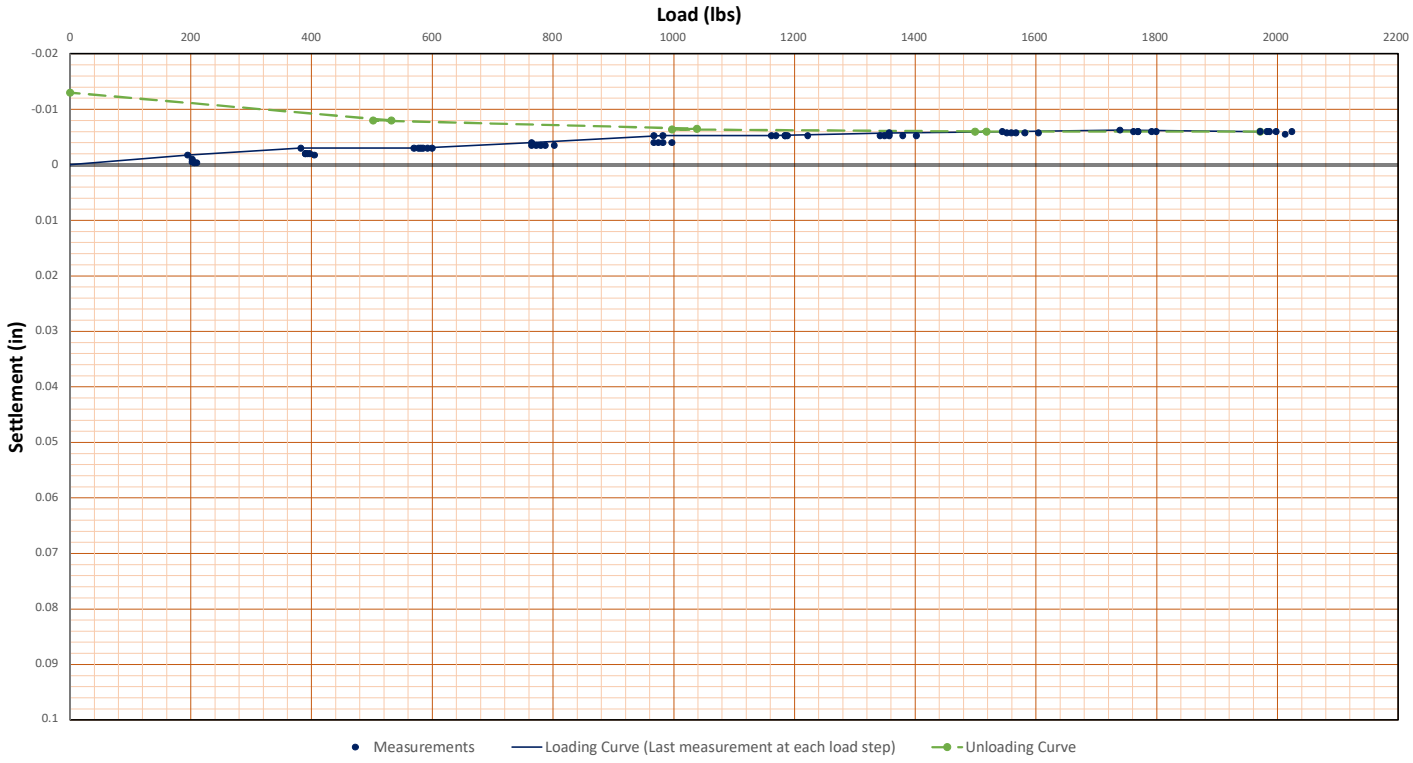
Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.
b. Readings were corrected for surface irregularities and initial zeroing at the set load. This correction was performed by shifting the settlement readings so that the linear fit of the early loading curve reports zero settlement at zero load.

Note 4: Load step skipped due to forecast inclement weather.

Note 5: Water noted at bearing surface from drizzling rain.

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3, 4]	Measured Load (lbs)
200 lbs	0	-0.5	202
	1	-0.4	210
	2	-0.4	206
	4	-0.4	206
	8	-1.0	202
	15	-1.8	195
400 lbs	0	-1.8	405
	1	-2.0	397
	2	-2.0	394
	4	-2.0	390
	8	-2.0	390
	15	-3.0	382
600 lbs	0	-3.0	600
	1	-3.0	585
	2	-3.0	577
	4	-3.0	570
	8	-3.0	592
	15	-3.0	581
800 lbs	0	-3.5	802
	1	-3.5	780
	2	-3.5	772
	4	-3.5	765
	8	-3.5	787
	15	-4.0	765
1000 lbs	0	-4.0	997
	1	-4.0	975
	2	-4.0	967
	4	-4.0	982
	8	-5.3	967
	15	-5.3	982
1200 lbs	0	-5.3	1222
	1	-5.3	1188
	2	-5.3	1184
	4	-5.3	1169
	8	-5.3	1184
	15	-5.3	1162
1400 lbs	0	-5.3	1402
	1	-5.3	1357
	2	-5.3	1349
	4	-5.3	1342
	8	-5.3	1379
	15	-5.8	1357
1600 lbs	0	-5.8	1604
	1	-5.8	1559
	2	-5.8	1552
	4	-5.8	1582
	8	-5.8	1567
	15	-6.0	1544
1800 lbs	0	-6.0	1799
	1	-6.0	1769
	2	-6.0	1762
	4	-6.0	1792
	8	-6.0	1769
	15	-6.3	1739
2000 lbs	0	-6.0	2024
	1	-6.0	1998
	2	-6.0	1983
	4	-5.5	2013
	8	-6.0	1987
	15	-6.0	1972
1500 lbs	0	-6.0	1499
	1	-6.0	1518
1000 lbs	0	-6.4	997
	1	-6.5	1038
500 lbs	0	-8.0	502
	1	-8.0	532
0 lbs	0	-13.0	0

Load vs Settlement Curve (Test Location CDL-2)



Burmister Classification: Orange/Brown cmf SAND, some cmf GRAVEL, little SILT, trace CLAY

Sample Depth: 1 foot
Sample Date: 10/31/2025
Sample Time: 10:15 AM to 1:15 PM
Temperature and Weather: Drizzle and Wind, ~40 - 45°F
Set Load: 120 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

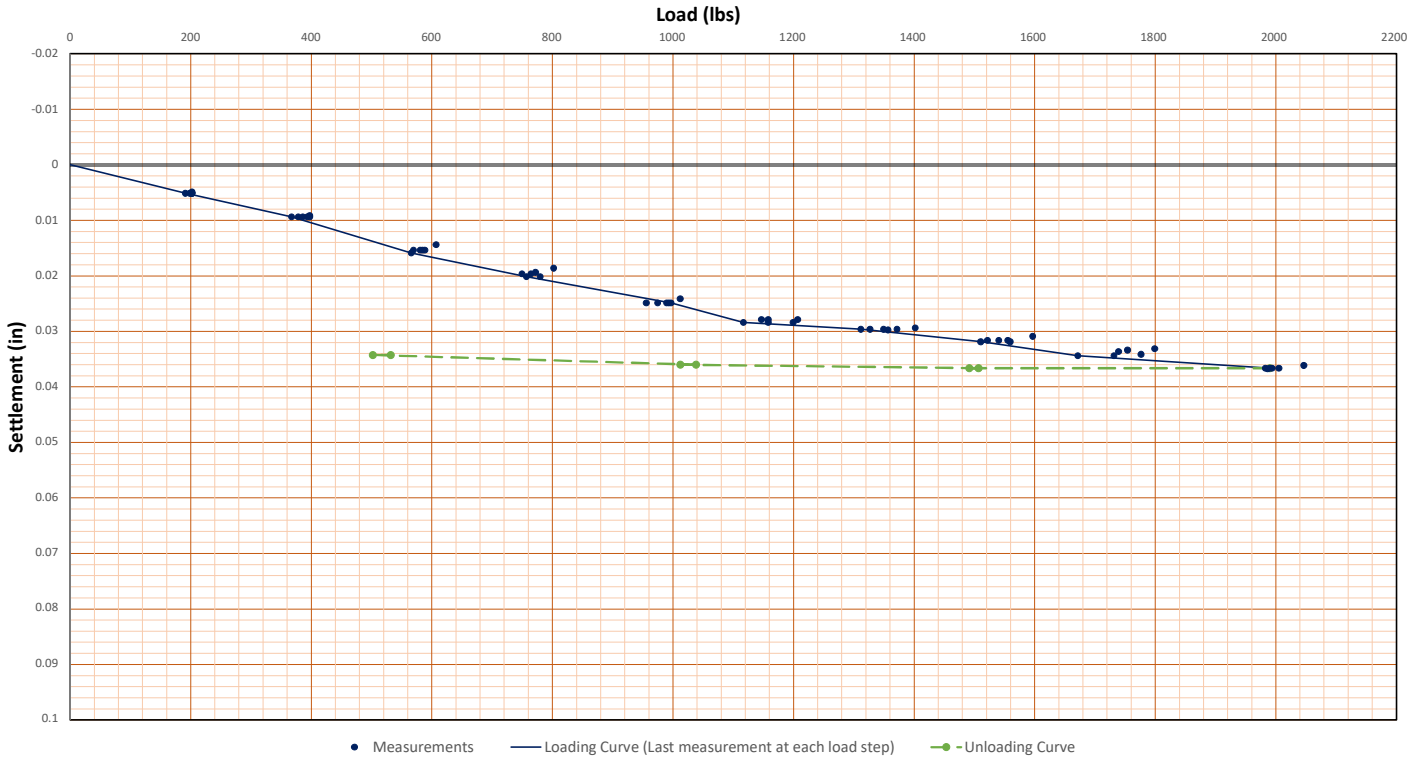
Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.

Note 4: Unusual negative settlement was recorded during these measurements. It was noted that the weather was windy.
This unusual negative settlement was possibly caused by settlement of the reference beam, possibly caused by wind and weak soil under the reference beam stakes.*

*Haley & Aldrich of New York - Supplemental Note: 2x4 wooden stakes were driven a few inches into the topsoil to provide a stable surface for the reference beam to sit on. CME suspects these stakes shifted during testing given saturated ground conditions and that stakes may not have been driven deep enough.

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3]	Measured Load (lbs)
200 lbs	0	4.9	202
	1	5.1	202
	2	5.1	202
	4	5.1	202
	8	5.1	199
	15	5.1	191
400 lbs	0	9.1	397
	1	9.4	397
	2	9.4	394
	4	9.4	386
	8	9.4	379
	15	9.4	367
600 lbs	0	14.4	607
	1	15.4	588
	2	15.4	581
	4	15.4	570
	8	15.4	585
	15	15.9	566
800 lbs	0	18.6	802
	1	19.4	772
	2	19.6	765
	4	19.6	750
	8	20.1	780
	15	20.1	757
1000 lbs	0	24.1	1012
	1	24.9	993
	2	24.9	990
	4	24.9	975
	8	24.9	956
	15	24.9	997
1200 lbs	0	27.9	1207
	1	27.9	1158
	2	27.9	1147
	4	28.4	1199
	8	28.4	1158
	15	28.4	1117
1400 lbs	0	29.4	1402
	1	29.6	1372
	2	29.6	1349
	4	29.6	1327
	8	29.7	1357
	15	29.6	1312
1600 lbs	0	30.9	1597
	1	31.6	1555
	2	31.6	1540
	4	31.6	1522
	8	31.9	1559
	15	31.9	1511
1800 lbs	0	33.1	1799
	1	33.4	1754
	2	33.6	1739
	4	34.1	1777
	8	34.4	1732
	15	34.4	1672
2000 lbs	0	36.1	2046
	1	36.6	1994
	2	36.7	1987
	4	36.6	2005
	8	36.6	1983
	15	36.6	1990
1500 lbs	0	36.6	1492
	1	36.6	1507
1000 lbs	0	36.0	1012
	1	36.0	1038
500 lbs	0	34.2	502
	1	34.2	532
0 lbs	Note 4		

Load vs Settlement Curve (Test Location CDL-3)



Burmister Classification: Brown cmf SAND and cmf GRAVEL, little SILT, trace CLAY

Sample Depth: 1 foot
Sample Date: 10/29/2025
Sample Time: 2:15 PM to 5:30 PM
Temperature and Weather: Cloudy, ~50°F
Set Load: 60 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.
b. Readings were corrected for surface irregularities and initial zeroing at the set load. This correction was performed by shifting the settlement readings so that the linear fit of the early loading curve reports zero settlement at zero load.

Note 4: Measurements at final 0 lb unloading step could not be obtained.

CME Report #2500581E-01-0226 Recorded Field Data

CME Associates, Inc.

Target Load	Measurement Time (minutes after application of load)	CDL-1			CDL-2			CDL-3			HWL-1			HWL-2			HWL-3		
		Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]
Set Load	0	100.00	100.00	0.165	100.00	100.00	0.155	100.00	100.00	0.085	100.00	100.00	0.220	100.00	100.00	0.250	100.00	100.00	0.025
200 lbs	0	100.00	100.00	0.270	100.00	101.00	0.270	97.50	98.50	0.270	100.00	100.00	0.275	101.50	98.00	0.270	98.00	98.00	0.270
	1	99.75	100.25	0.270	99.75	101.00	0.280	97.50	98.00	0.270	100.00	100.00	0.295	102.50	97.00	0.255	97.50	97.00	0.250
	2	99.75	100.25	0.270	99.75	101.00	0.275	97.50	98.00	0.270	100.00	100.00	0.320	101.00	95.25	0.400	96.50	96.00	0.270
	4	99.75	100.25	0.265	99.75	101.00	0.275	97.50	98.00	0.270	99.50	100.00	0.370	101.00	95.00	0.390	96.00	95.50	0.260
	8	99.75	100.25	0.250	100.00	102.00	0.270	97.50	98.00	0.265	98.50	99.50	0.450	101.00	95.00	0.370	95.00	95.00	0.240
	15	99.75	101.00	0.250	100.50	103.00	0.260	97.50	98.00	0.255	97.00	99.00	0.570	101.00	95.00	0.355	94.50	94.50	0.260
400 lbs	0	99.00	99.00	0.540	100.50	103.00	0.540	93.00	94.50	0.530	97.00	99.00	0.580	101.00	95.00	0.540	93.00	93.50	0.530
	1	98.50	99.00	0.520	101.00	103.00	0.530	93.00	94.00	0.530	97.00	99.00	0.590	101.00	95.00	0.525	93.00	93.00	0.495
	2	98.50	99.00	0.510	101.00	103.00	0.525	93.00	94.00	0.525	96.75	98.75	0.610	101.00	95.00	0.520	92.50	92.50	0.485
	4	98.50	99.00	0.495	101.00	103.00	0.520	93.00	94.00	0.515	96.50	98.75	0.640	101.00	95.00	0.530	91.00	91.00	0.520
	8	98.00	99.00	0.520	101.00	103.00	0.520	93.00	94.00	0.505	95.00	97.50	0.700	101.00	95.00	0.515	91.00	91.00	0.490
	15	98.00	99.00	0.500	102.00	104.00	0.510	93.00	94.00	0.490	94.00	97.50	0.780	101.00	95.00	0.530	89.00	89.00	0.505
600 lbs	0	96.00	96.50	0.800	102.00	104.00	0.800	88.00	89.00	0.810	93.50	97.00	0.800	101.50	94.75	0.800	87.00	87.00	0.800
	1	96.00	96.50	0.770	102.00	104.00	0.780	87.00	88.00	0.785	93.50	96.50	0.815	101.50	94.50	0.755	87.00	87.00	0.760
	2	96.00	96.50	0.755	102.00	104.00	0.770	87.00	88.00	0.775	93.00	96.50	0.830	101.50	94.50	0.785	87.00	87.00	0.750
	4	95.50	96.00	0.780	102.00	104.00	0.760	87.00	88.00	0.760	93.00	96.50	0.850	101.50	94.50	0.760	87.50	87.00	0.770
	8	95.50	96.00	0.750	102.00	104.00	0.790	87.00	88.00	0.780	92.50	96.25	0.890	101.50	94.00	0.760	87.00	86.00	0.760
	15	95.50	96.00	0.760	102.00	104.00	0.775	86.50	87.50	0.755	92.50	96.25	0.890	102.50	94.00	0.760	84.00	83.00	0.770
800 lbs	0	94.50	95.00	1.070	102.50	104.50	1.070	84.00	84.50	1.070	92.00	96.00	1.070	102.50	94.00	1.080	84.00	83.00	1.090
	1	94.00	94.50	1.010	102.50	104.50	1.040	83.00	84.00	1.030	92.00	96.00	1.010	102.50	94.00	1.040	82.00	81.00	1.040
	2	94.00	94.50	1.000	102.50	104.50	1.030	83.00	83.50	1.020	92.00	96.00	0.990	102.50	94.00	1.030	82.00	80.00	1.030
	4	93.50	94.00	1.040	102.50	104.50	1.020	83.00	83.50	1.000	91.50	96.00	1.050	102.50	94.00	1.005	81.00	79.50	1.000
	8	93.00	94.00	1.005	102.50	104.50	1.050	82.50	83.00	1.040	91.00	96.00	1.045	102.50	94.00	1.040	79.00	79.50	1.040
	15	93.00	94.00	1.000	103.00	105.00	1.020	82.50	83.00	1.010	90.50	95.50	1.090	101.50	92.00	1.050	80.00	79.00	1.050
1000 lbs	0	92.00	92.00	1.330	103.00	105.00	1.330	78.50	79.00	1.350	89.50	95.00	1.340	101.50	92.00	1.340	80.00	79.00	1.340
	1	91.50	92.00	1.280	103.00	105.00	1.300	78.00	78.00	1.325	89.50	95.00	1.315	101.50	92.00	1.290	80.00	79.00	1.300
	2	91.50	92.00	1.260	103.00	105.00	1.290	78.00	78.00	1.320	89.50	95.00	1.310	101.50	92.00	1.325	80.00	78.00	1.320
	4	91.00	91.00	1.300	103.00	105.00	1.310	78.00	78.00	1.300	89.50	95.00	1.305	101.50	92.00	1.290	77.00	76.00	1.290
	8	91.00	91.00	1.260	103.50	107.00	1.290	78.00	78.00	1.275	89.25	95.00	1.295	101.50	92.00	1.300	74.00	72.00	1.310
	15	90.50	91.00	1.245	103.50	107.00	1.310	78.00	78.00	1.330	89.00	95.00	1.285	101.50	91.50	1.285	73.00	71.00	1.280
1200 lbs	0	90.00	90.50	1.600	103.50	107.00	1.630	74.00	76.00	1.610	89.00	95.00	1.610	102.00	91.50	1.600	72.00	70.00	1.600
	1	90.00	90.00	1.530	103.50	107.00	1.585	74.00	76.00	1.545	89.00	95.00	1.570	102.00	91.50	1.560	71.50	69.50	1.570
	2	89.00	89.50	1.575	103.50	107.00	1.580	74.00	76.00	1.530	89.00	95.00	1.560	102.00	91.50	1.590	71.00	69.25	1.555
	4	89.00	89.00	1.565	103.50	107.00	1.560	73.00	76.00	1.600	88.50	95.00	1.590	102.00	91.00	1.560	71.00	69.00	1.585
	8	89.00	89.00	1.550	103.50	107.00	1.580	73.00	76.00	1.545	88.50	95.00	1.570	101.00	90.00	1.570	70.00	68.50	1.560
	15	89.00	89.00	1.550	103.50	107.00	1.550	73.00	76.00	1.490	88.00	95.00	1.575	101.00	91.00	1.570	68.00	65.00	1.570
1400 lbs	0	Load step skipped due to forecast inclement weather			103.50	107.00	1.870	72.00	75.00	1.870	88.00	95.50	1.870	102.00	91.00	1.870	67.00	64.50	1.870
	1				103.50	107.00	1.810	71.50	75.00	1.830	88.00	95.50	1.540	102.00	91.00	1.820	66.50	64.50	1.840
	2				103.50	107.00	1.800	71.50	75.00	1.800	88.00	95.50	1.525	102.00	91.00	1.845	65.00	64.00	1.860
	4				103.50	107.00	1.790	71.50	75.00	1.770	88.00	95.50	1.810	102.00	91.00	1.830	65.00	64.00	1.830
	8				103.50	107.00	1.840	71.25	75.00	1.810	87.50	95.50	1.840	101.50	90.00	1.800	65.00	62.50	1.835
	15				103.50	108.00	1.810	71.50	75.00	1.750	87.00	95.00	1.820	100.00	88.50	1.810	64.00	61.00	1.810
1600 lbs	0	86.00	85.00	2.150	103.50	108.00	2.140	70.00	74.00	2.130	86.50	95.00	2.130	100.00	88.50	2.130	63.00	61.00	2.130
	1	86.00	85.00	2.000	103.50	108.00	2.080	69.50	73.00	2.075	86.50	95.00	2.090	100.00	88.50	2.060	62.50	60.00	2.090
	2	86.00	85.00	1.995	103.50	108.00	2.070	69.00	73.50	2.055	86.50	95.00	2.085	100.00	88.00	2.095	62.00	60.00	2.115
	4	85.50	85.00	2.075	103.50	108.00	2.110	69.00	73.50	2.030	86.00	95.00	2.080	99.50	87.50	2.060	61.50	59.00	2.085
	8	85.00	84.00	2.050	103.50	108.00	2.090	69.00	73.00	2.080	85.00	94.00	2.110	98.00	86.00	2.085	60.00	58.00	2.085
	15	84.50	84.00	2.030	103.00	109.00	2.060	69.00	73.00	2.015	84.00	93.00	2.120	98.00	86.00	2.075	59.00	56.00	2.090
1800 lbs	0	Load step skipped due to forecast inclement weather			103.00	109.00	2.400	67.50	72.00	2.400	84.00	93.00	2.400	97.50	85.50	2.400	58.00	55.50	2.400
	1				103.00	109.00	2.360	67.00	72.00	2.340	84.00	93.00	2.370	97.50	85.50	2.340	58.00	55.00	2.350
	2				103.00	109.00	2.350	67.00	71.50	2.320	84.00	93.00	2.360	97.50	85.50	2.380	58.00	55.00	2.380
	4				103.00	109.00	2.390	66.50	71.00	2.370	84.00	93.00	2.390	97.00	85.00	2.370	57.00	54.00	2.350
	8				103.00	109.00	2.360	66.00	71.00	2.310	83.00	93.00	2.380	97.00	85.00	2.330	56.00	53.50	2.350
	15				103.00	109.50	2.320	66.00	71.00	2.230	83.00	93.00	2.370	97.00	85.00	2.305	55.50	52.50	2.250
2000 lbs	0	81.00	82.00	2.700	103.00	109.00	2.700	64.50	69.00	2.730	82.50	93.00	2.700	97.00	85.00	2.700	53.50	50.50	2.700
	1	81.00	80.00	2.700	103.00	109.00	2.665	64.00	68.50	2.660	82.50	93.00	2.675	96.50	85.00	2.640	53.00	50.00	2.670
	2	81.00	79.50	2.690	103.00	109.00	2.645	64.00	68.25	2.650	82.50	93.00	2.665	96.50	84.50	2.670	53.00	50.00	2.660
	4	81.00	79.00	2.700	102.00	109.00	2.685	63.50	69.00	2.675	82.50	93.00	2.660	96.50	84.50	2.665	51.50	48.00	2.690
	8	80.00	79.00	2.650	102.00	110.00	2.650	63.50	69.00	2.645	81.50	93.00	2.685	96.50	84.50	2.655	50.00	46.50	2.660
	15	80.00	79.00	2.700	102.00	110.00	2.630	63.50	69.00	2.655	81.00	92.50	2.680	94.00	81.00	2.665	48.00	45.00	2.670
	20 (Discretionary extended hold)													93.50	80.00	2.670	47.00	44.00	2.680
	25 (Discretionary extended hold)													92.50	79.00	2.685	47.00	44.00	2.680
	30 (Discretionary extended hold)													92.50	79.00	2.680			
1500 lbs	0	80.00	79.00	2.000	102.00	110.00	2.000	63.50	69.00	1.990	82.00	93.50	2.010	93.00	79.50	1.980	47.00	44.00	2.000
	1	80.00	79.00	2.040	102.00	110.00	2.025	63.50	69.00	2.010	82.00	94.00	2.050	93.00	79.50	1.960	47.00	44.00	2.020
1000 lbs	0	80.00	79.00	1.340	102.50	110.25	1.330	64.50	69.25	1.350	84.00	95.50	1.330	96.00	82.50	1.250	47.50	44.00	1.340
	1	80.00	79.00	1.370	102.50	110.50	1.385	64.50	69.25	1.385	84.00	95.50	1.390	97.00	82.50	1.290	47.50	44.00	1.400
500 lbs	0	81.00	82.00	0.650	104.00	112.00	0.670	66.75	70.50	0.670	86.00	97.00	0.690	99.00	85.50	0.680	50.00	46.50	0.670
	1	81.00	82.00	0.720	104.00	112.00	0.710	66.75	70.50	0.710	86.00	97.50	0.760	99.00	86.00	0.760	50.00	46.50	0.730
0 lbs	0	81.00	87.00	0.000	109.00	117.00	0.000	Could not be obtained			94.50	104.50	0.000	Could not be obtained			53.00	50.00	0.000



Figure 2: Static Load Test Setup at CDL-1.



Figure 3: Bearing surface after completion of the Static Load Test at CDL-1.

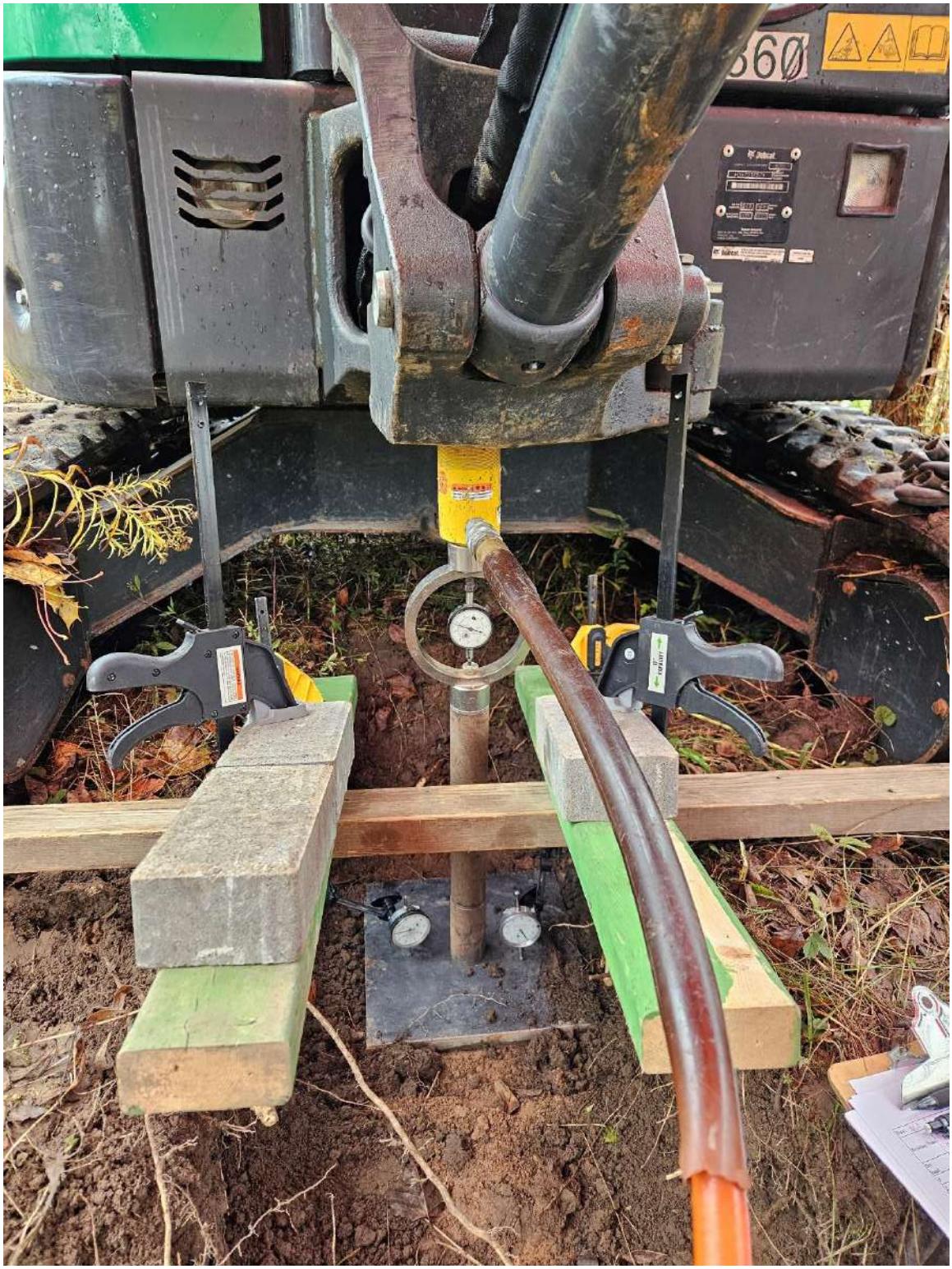


Figure 4: Static Load Test Setup at CDL-2.



Figure 5: Bearing surface after completion of the Static Load Test at CDL-2.



Figure 6: Static Load Test Setup at CDL-3.



Figure 7: Bearing surface after completion of the Static Load Test at CDL-3.



Supplemental Photo 1: Rig setup for the Static Load Test at CDL-2. Setup is typical of test locations for C&D landfill.

APPENDIX D
B-20 Well Cluster Boring Logs

O'BRIEN & GERE ENGINEERS INC.

TEST BORING LOG

REPORT OF BORING NUMBER 20, 20A, 21
SHEET 1 OF 2
DATE FILE 336.017

PROJECT LOCATION Webster, NY
CLIENT: XEROX

SAMPLER
TYPE: Split Spoon
HAMMER FALL 140 lb 30"

GROUNDWATER READINGS

DATE	DEPTH	BGL	
[illegible]	7.52	20B	7.97
	8.65	20A	

BORING CO. Parratt-Wolff, Inc.
FOREMAN M. Ellingsworth
OBG ENGINEER J. C. Tomik

BORING LOCATION North of Landfill
GROUND ELEV. 375.0
DATE STARTED 7/17/85 DATE ENDED 7/19

DEPTH	"N" Value	SAMPLE NO.	PEN./REC.	DEPTH	BLOWS / 6"	SAMPLE DESCRIPTION	STRA. CHG. GEN. DESC.	EQUIPMENT INSTALLED	FIELD TESTING	REMKS
		1	2'	0-2'	Auger Sample	Brown, moist, SILT, little fine SAND, Wood Fragments (FILL)	(FILL)			
		2	2'	2-4'	Auger Sample	Brown, moist, SILT & f. SAND	6'			
5	11	3	2'	4-6'	4-4 7-9	Brown, moist-wet, dense fine SAND, some SILT, lit. f. GRAVEL	LAC. SAND/ SILT			
	31	4	2'	6-8'	11-13 18-20	Brown, moist stratified -AS ABOVE-	10'			
10	60	5	2'	8-10'	12-18 42-50/1	Reddish brown, moist-wet, very dense f. SAND, some SILT, little f. GRAVEL 12.5-13.5' BOULDER -AS ABOVE- except medium to fine SAND.	Sandy Till			
	>100	6	1'	10-10.6'	28-50/1"					
	>100	7	6"	13.5-14'	52/.5					
15	>100	8	6"	15-15.5	50/.5		17'			
	>100	9	6'	17.5-18'	78/.5					
20	>100	10	1'	20-21	100/.4	Brown-Reddish Brown, wet, very dense f. SAND & SILT with little embedded GRAVEL fragments, occas. COBBLES				
25	>100	11	6"	25-25.5	80/.5					
30	>100	12	6"	30-30.5	85/.5	Some Red SHALE, ROCK fragments	32'			
						-Weathered Bedrock-	Weath Bdrk			

REMARKS:

O'BRIEN & GERE ENGINEERS INC.

TEST BORING LOG

REPORT OF BORING NUMBER 20, 20A, 20B
SHEET 2 OF 2
DATE FILE

PROJECT LOCATION Webster, NY
CLIENT: XEROX

SAMPLER
TYPE: Split Spoon
HAMMER FALL 140 lb

GROUNDWATER READINGS

DATE	DEPTH		

BORING CO. Parratt-Wolff, Inc
FOREMAN M. Ellingsworth
OBG ENGINEER J. C. Tomik

BORING LOCATION North of Landfill
GROUND ELEV. 375.0
DATE STARTED _____ DATE ENDED _____

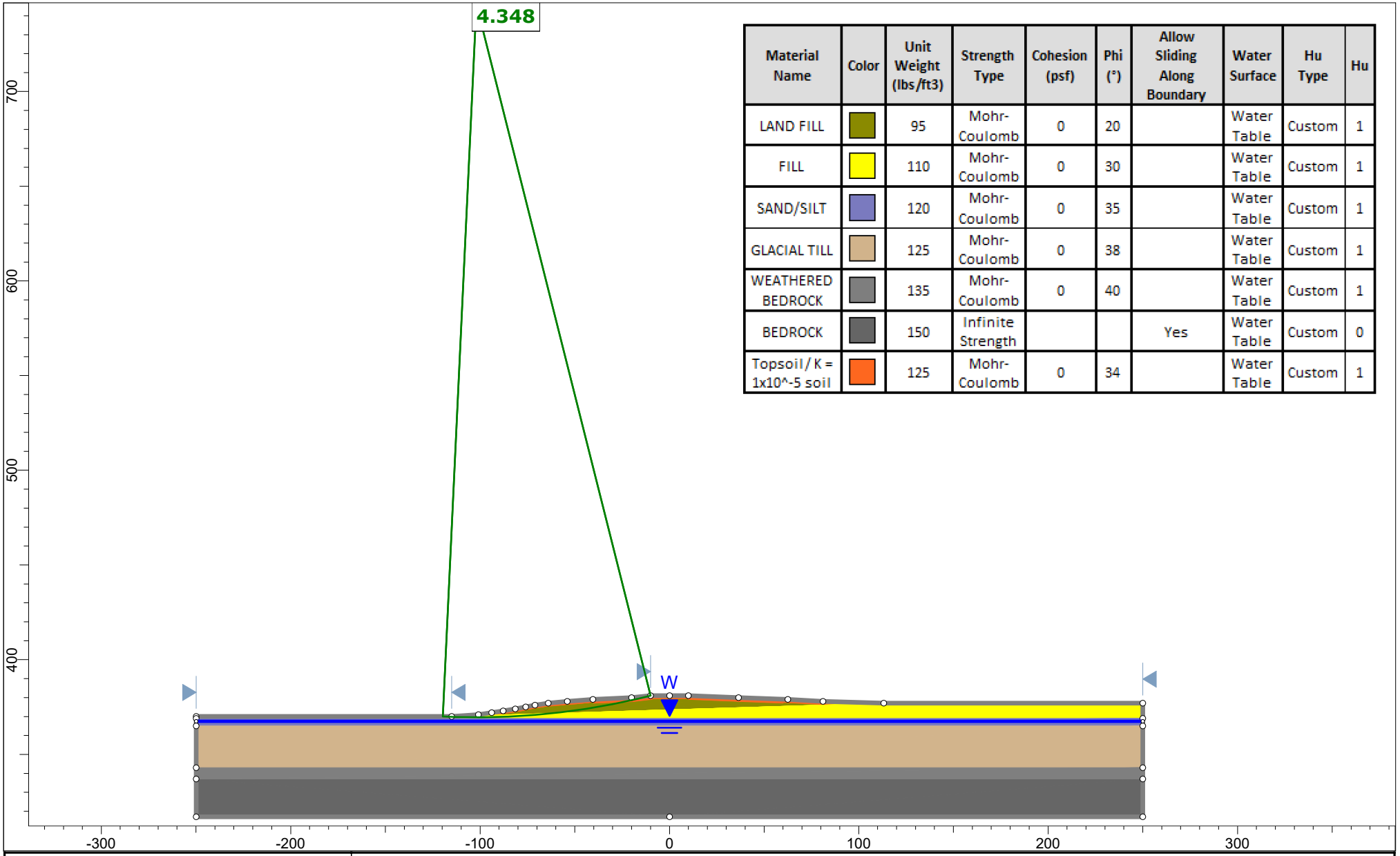
DEPTH	"N" Value	SAMPLE NO.	SAMPLE PEN./REC.	SAMPLE DEPTH	SAMPLE BLOWS/6"	SAMPLE DESCRIPTION	STRA. CHG. GEN. DESC.	EQUIPMENT INSTALLED	FIELD TESTING	RMKS
40		R1	60"/ 32"	38-43'		Purplish red-Greenish gray, interbedded SILTSTONE & f. grained crystalline SANDSTONE, frequent hor. fractures every 1"-3" @ 40-43'. Upper 2' section highly weathered.				
45							Competent Bedrck			
50										
55										
60										
		R2	60"/ 55"	60-65'		Purplish red, f. grained SANDSTONE 60-62' hor. fractures every 3-12" Brownish red SILTSTONE 62-64' w/fractures again 1-3" SANDSTONE 64-65'				
65										
		R3	60"/ 55"	65-70'		Purplish red, f. grained SANDSTONE w/occ. layers of SILTSTONE 6" long. Hor. fractures every 4-16".				
70										

REMARKS:

Bottom of Exploration @ 70'

APPENDIX E
Stability and Settlement Analysis Results

SLOPE STABILITY RESULTS
(CURRENT CONDITIONS, NO LOAD)



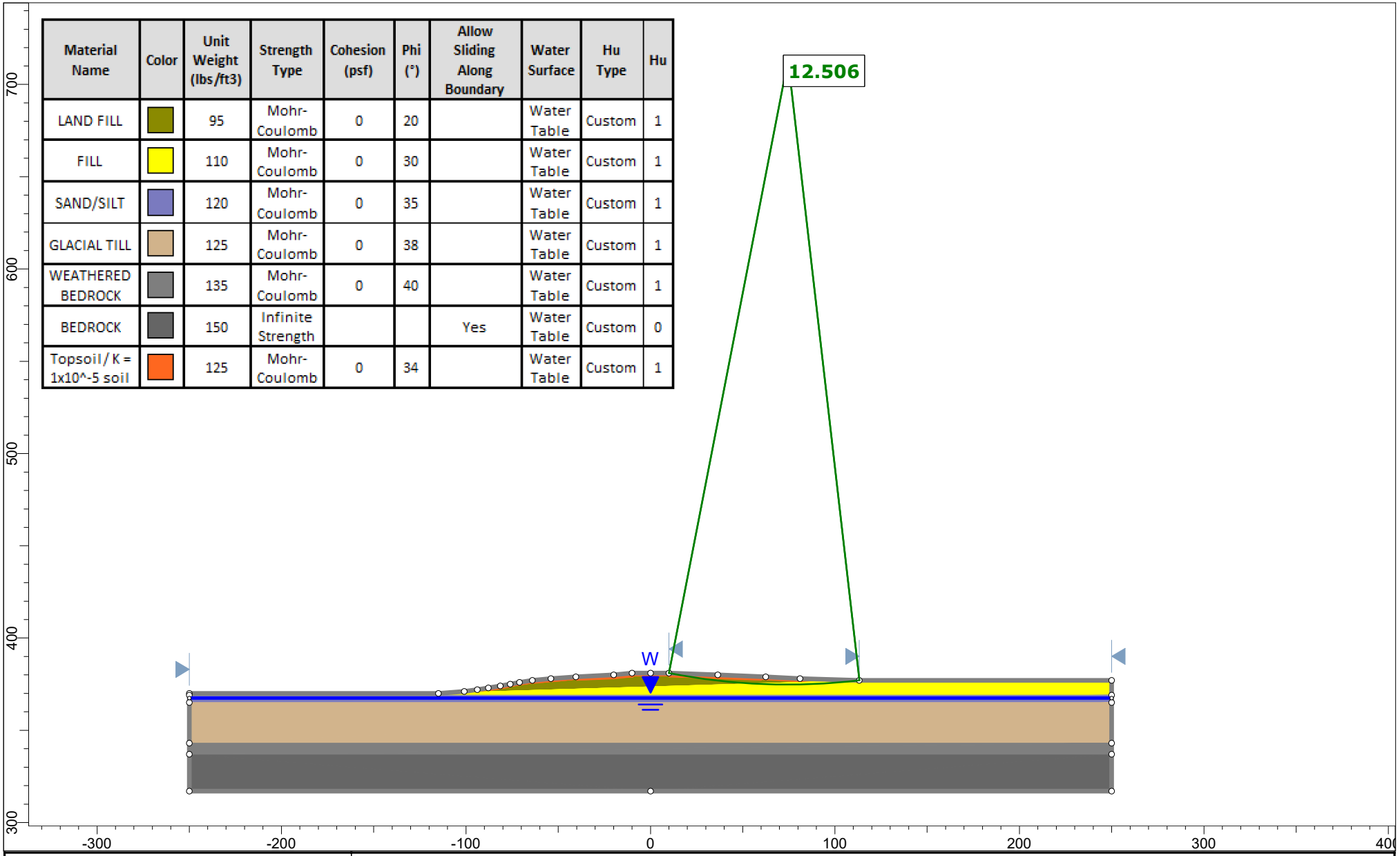
Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0
Topsoil / K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1

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SLIDEINTERPRET 9.029

Project	Slide2 - An Interactive Slope Stability Program		
Group	Right to Left-W/O Surcharge	*Scenario*	Master Scenario
Drawn By		*Company*	
Date	11/17/2025, 4:53:50 PM	*File Name*	2025-1119-Left to Right and Right to Left-D1.slmd

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0
Topsoil / K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1

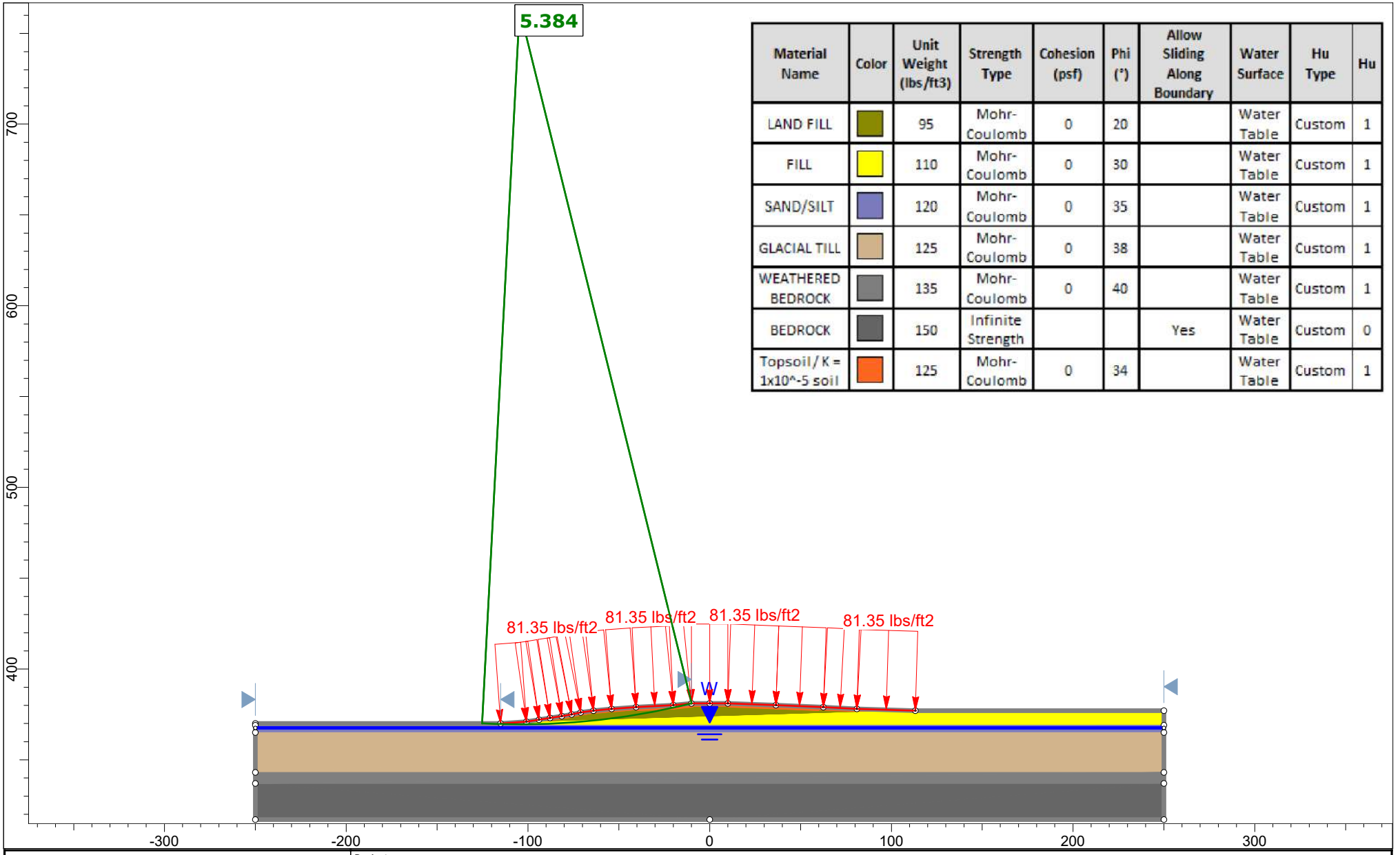


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SLIDEINTERPRET 9.029

Project	Slide2 - An Interactive Slope Stability Program		
Group	Left to Right-W/O Surcharge	Scenario	Master Scenario
Drawn By		Company	
Date	11/17/2025, 4:53:50 PM	File Name	2025-1119-Left to Right and Right to Left-D1.slmd

SLOPE STABILITY RESULTS (DISTRUBUTED LOAD, TEMPORARY CONSTRUCTION EQUIPMENT)



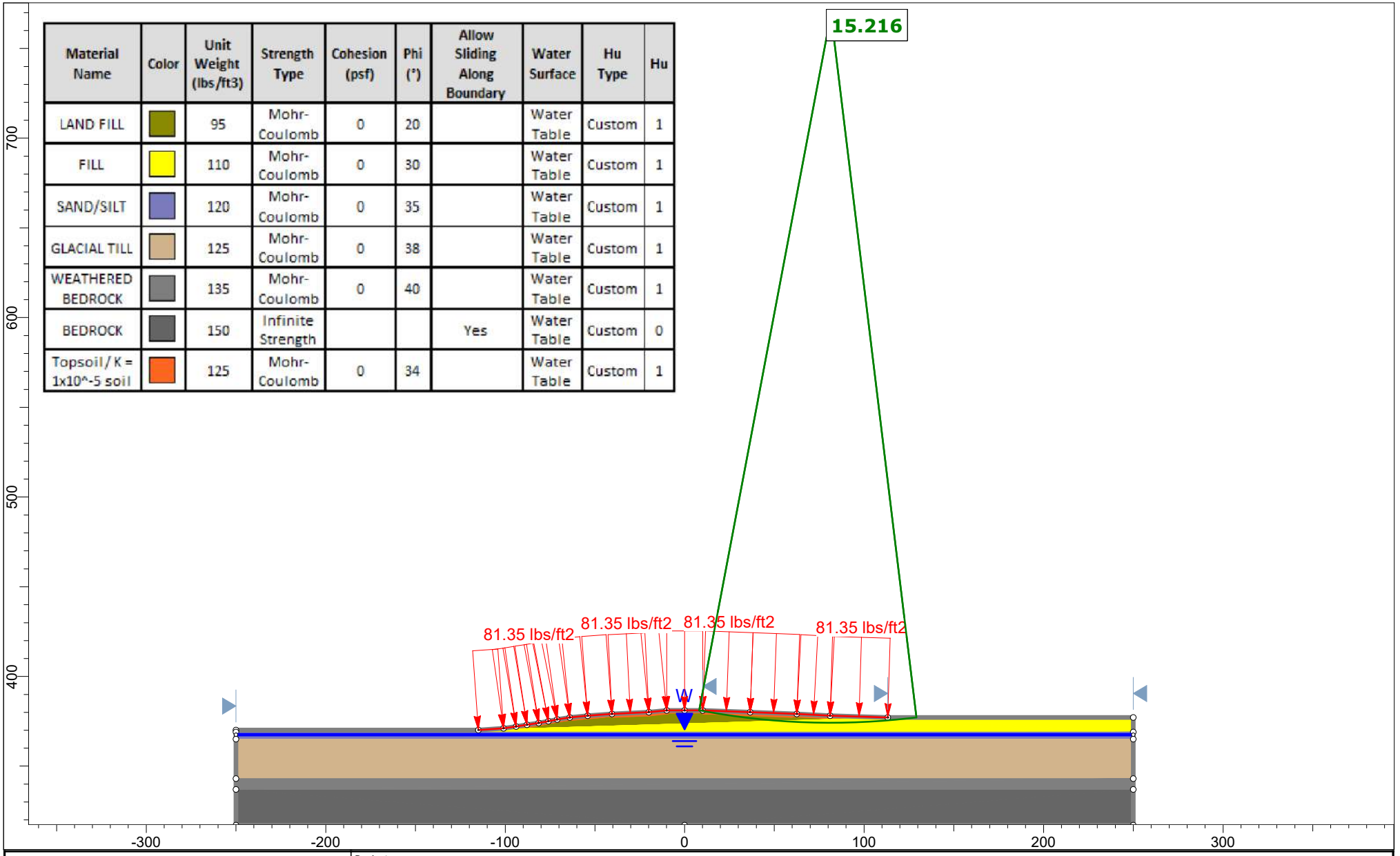
Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0
Topsoil / K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1

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SLIDEINTERPRET 9.029

Project	Slide2 - An Interactive Slope Stability Program		
Group	Right to Left-With Surcharge	Scenario	Master Scenario
Drawn By		Company	
Date	11/17/2025, 4:53:50 PM	File Name	2025-1124-Left to Right and Right to Left-D1.slmd

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0
Topsoil / K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1

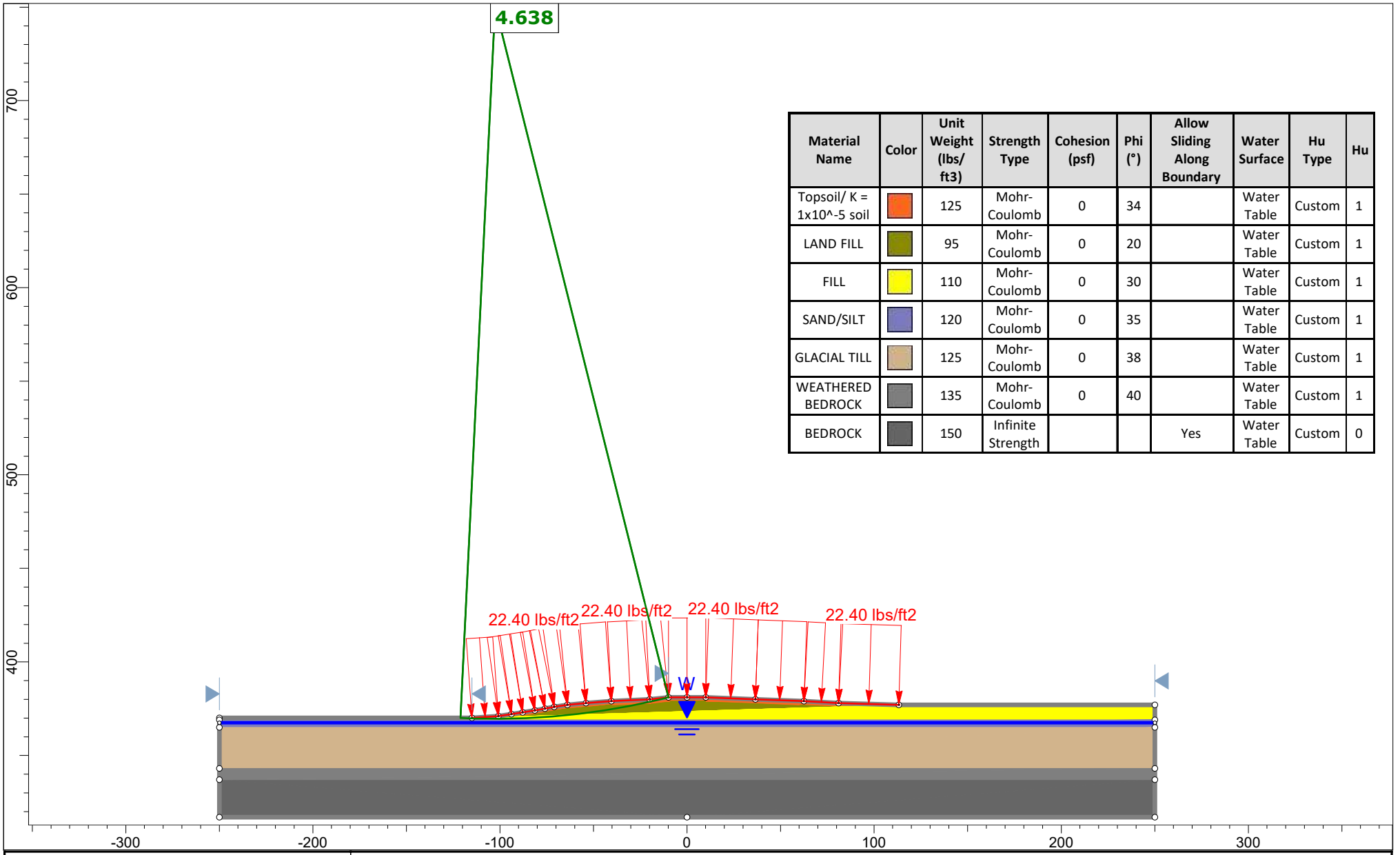


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SLIDEINTERPRET 9.029

Project	Slide2 - An Interactive Slope Stability Program		
Group	Left to Right-With Surcharge	Scenario	Master Scenario
Drawn By		Company	
Date	11/17/2025, 4:53:50 PM	File Name	2025-1124-Left to Right and Right to Left-D1.slmd

SLOPE STABILITY RESULTS (DISTRUBUTED LOAD DUE TO SOLAR PANELS)



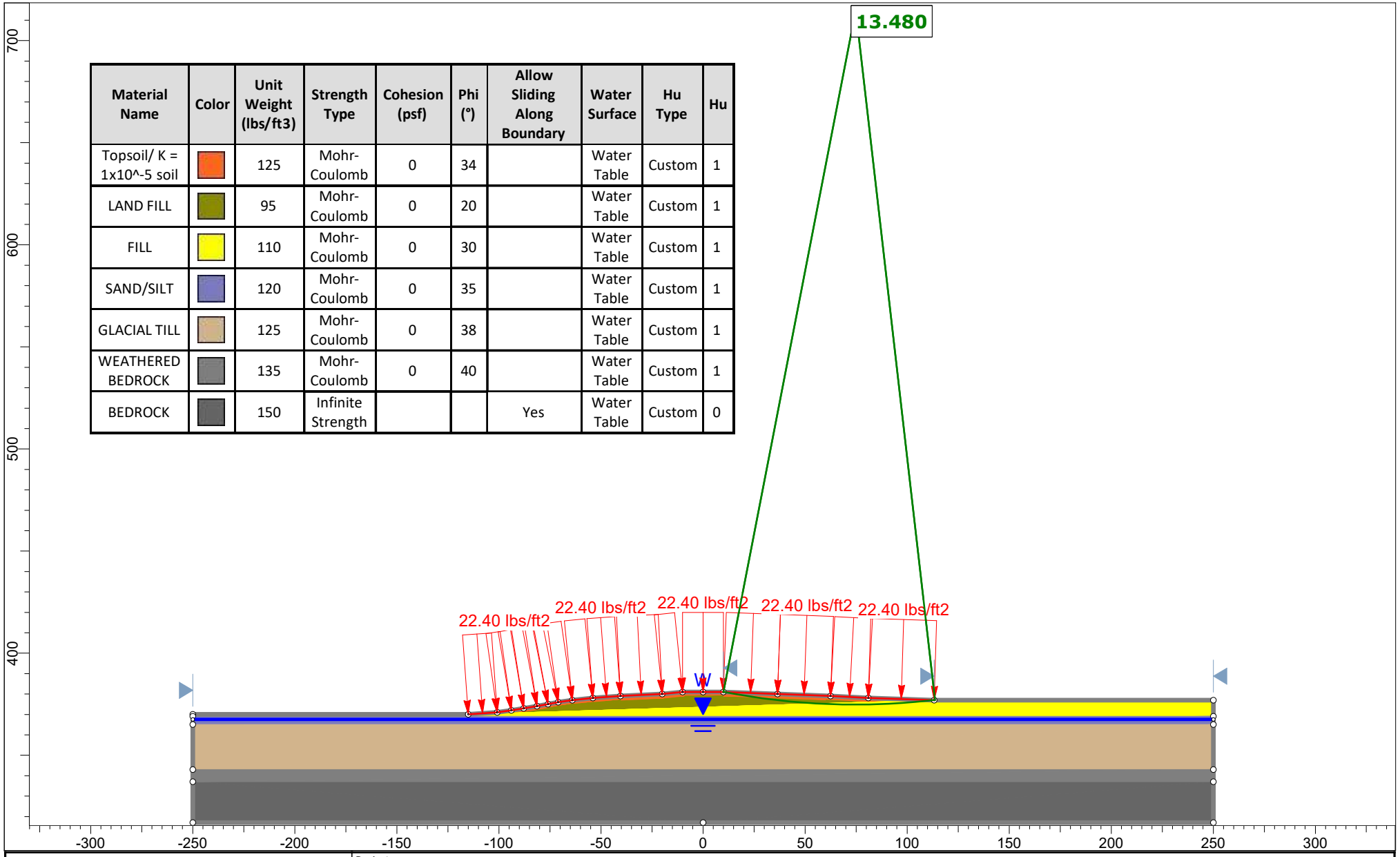
Material Name	Color	Unit Weight (lbs/ ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Topsoil/ K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0

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SLIDEINTERPRET 9.029

Project	Slide2 - An Interactive Slope Stability Program		
Group	Right to Left-With Surcharge	*Scenario*	Master Scenario
Drawn By		*Company*	
Date	02/09/2026, 10:53:50 PM	*File Name*	2026-0203-Left to Right and Right to Left-D1.slmd

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Topsoil/ K = 1x10^-5 soil		125	Mohr-Coulomb	0	34		Water Table	Custom	1
LAND FILL		95	Mohr-Coulomb	0	20		Water Table	Custom	1
FILL		110	Mohr-Coulomb	0	30		Water Table	Custom	1
SAND/SILT		120	Mohr-Coulomb	0	35		Water Table	Custom	1
GLACIAL TILL		125	Mohr-Coulomb	0	38		Water Table	Custom	1
WEATHERED BEDROCK		135	Mohr-Coulomb	0	40		Water Table	Custom	1
BEDROCK		150	Infinite Strength			Yes	Water Table	Custom	0

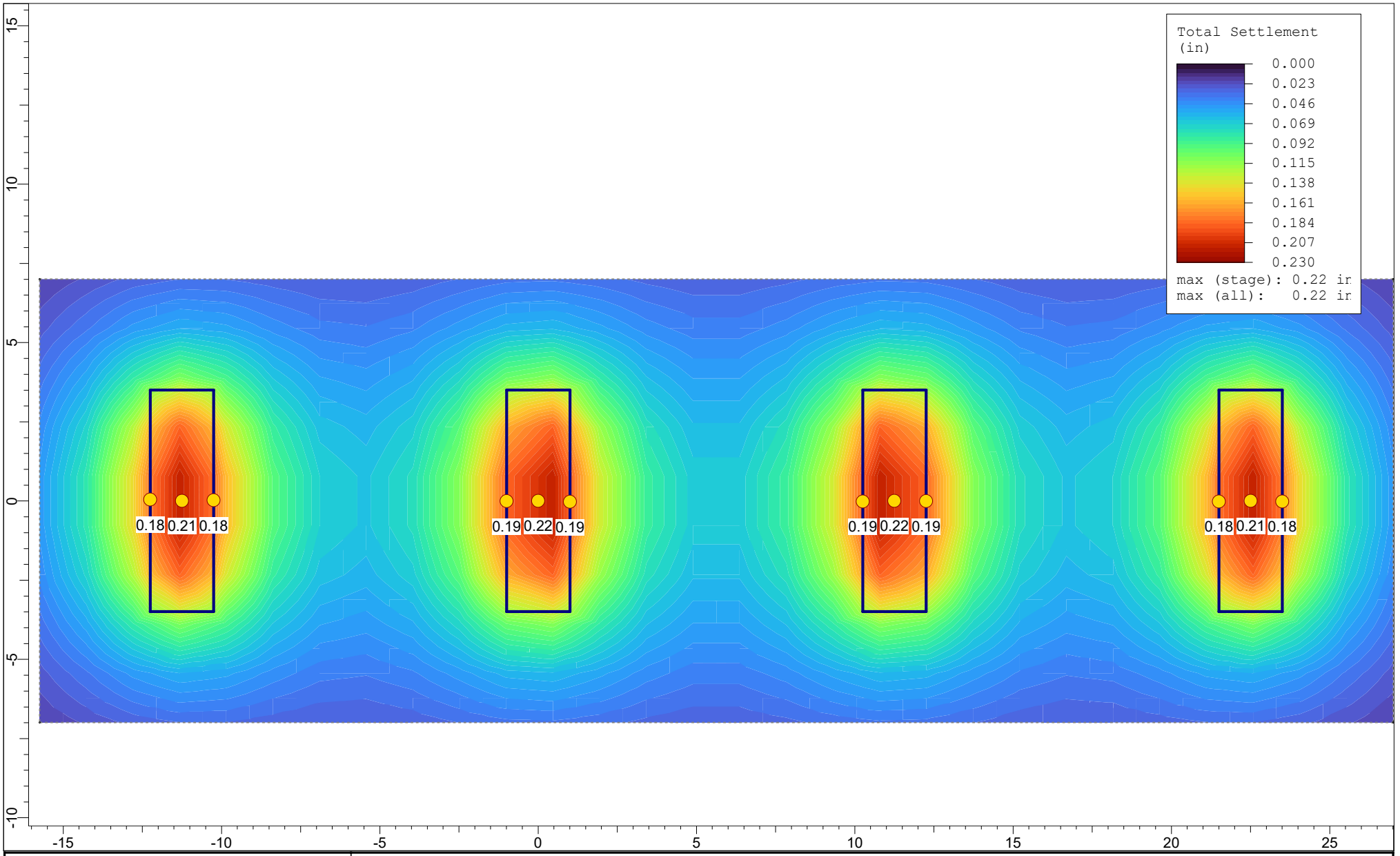


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SLIDEINTERPRET 9.029

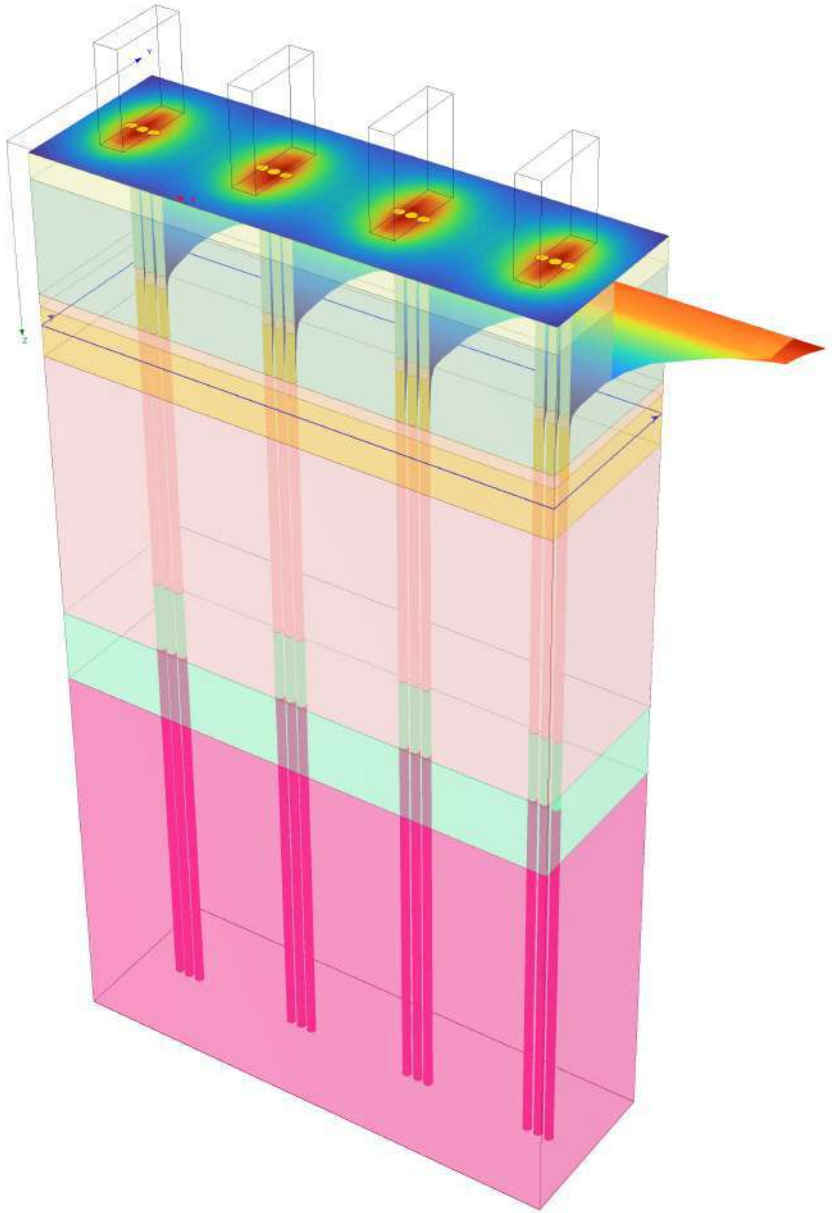
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Group	Left to Right-With Surcharge	Scenario	Master Scenario
Drawn By		Company	
Date	02/09/2026, 10:53:50 PM	File Name	2026-0203-Left to Right and Right to Left-D1.slmd

SETTLEMENT ANALYSIS (LOAD DUE TO INDIVIDUAL SOLAR PANELS)



rocscience	Project: Webster Solar Garden Project		
	Analysis Description:		
	Drawn By: SSM	Company: Haley and Aldrich	
	Date: 02/09/2026, 1:40:20 PM	File Name: 2026-0203-Settlement Analysis-D1.s3z	

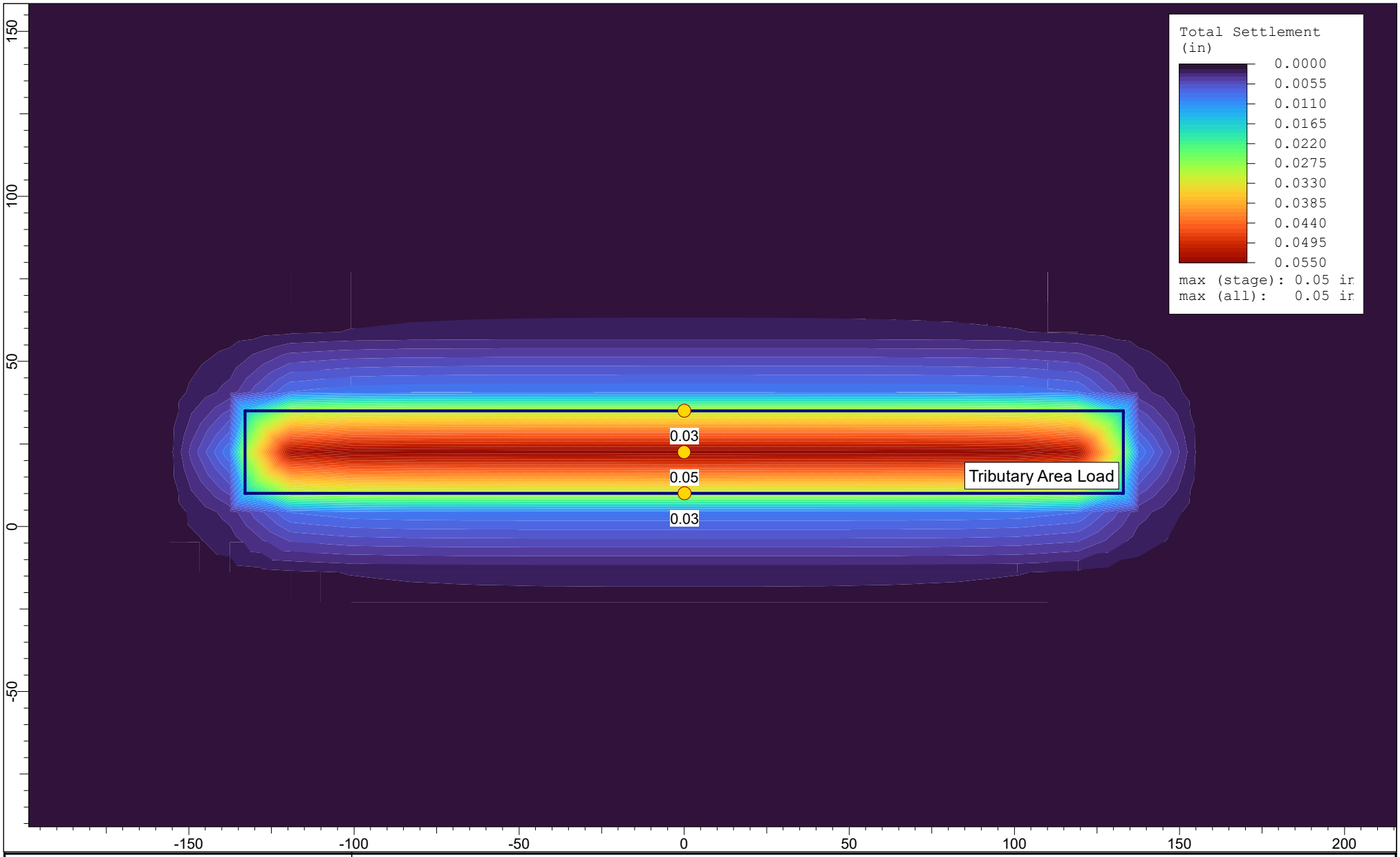
SETTLE3 5.022



rocscience	*Project*	Webster Solar Garden Project	
	Analysis Description		
	Drawn By	SSM	*Company* Haley and Aldrich
	Date	02/09/2026, 1:40:20 PM	*File Name* 2026-0203-Settlement Analysis-D1.s3z

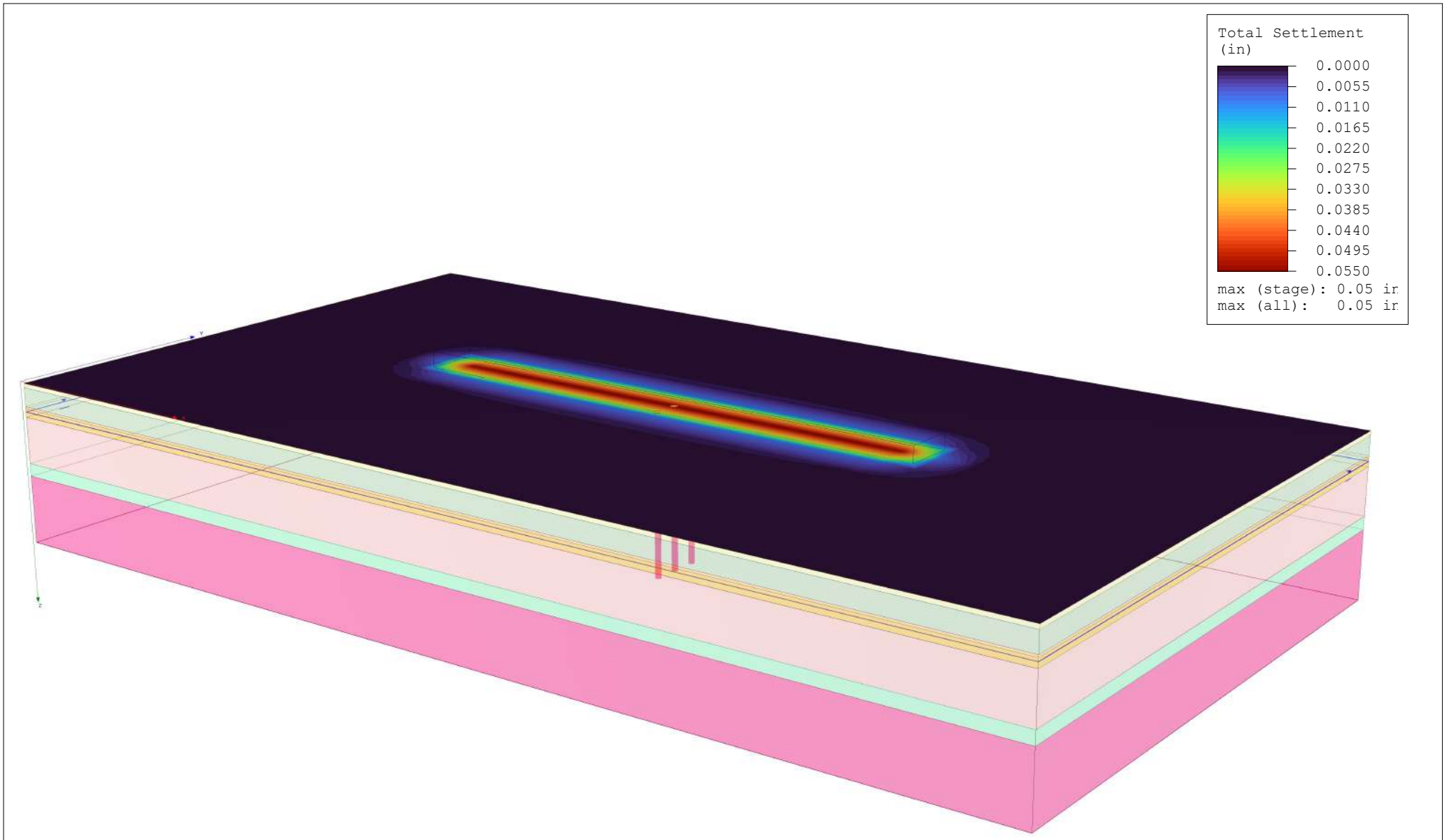
SETTLE3 5.022

SETTLEMENT ANALYSIS (DISTRUBUTED LOAD DUE TO SOLAR PANELS)



rocscience	*Project*	Webster Solar Garden Project	
	Analysis Description		
	Drawn By	SSM	*Company* Haley and Aldrich
	Date	02/09/2026, 10:40:48 PM	*File Name* 2026-0204-Settlement Analysis-D1.s3z

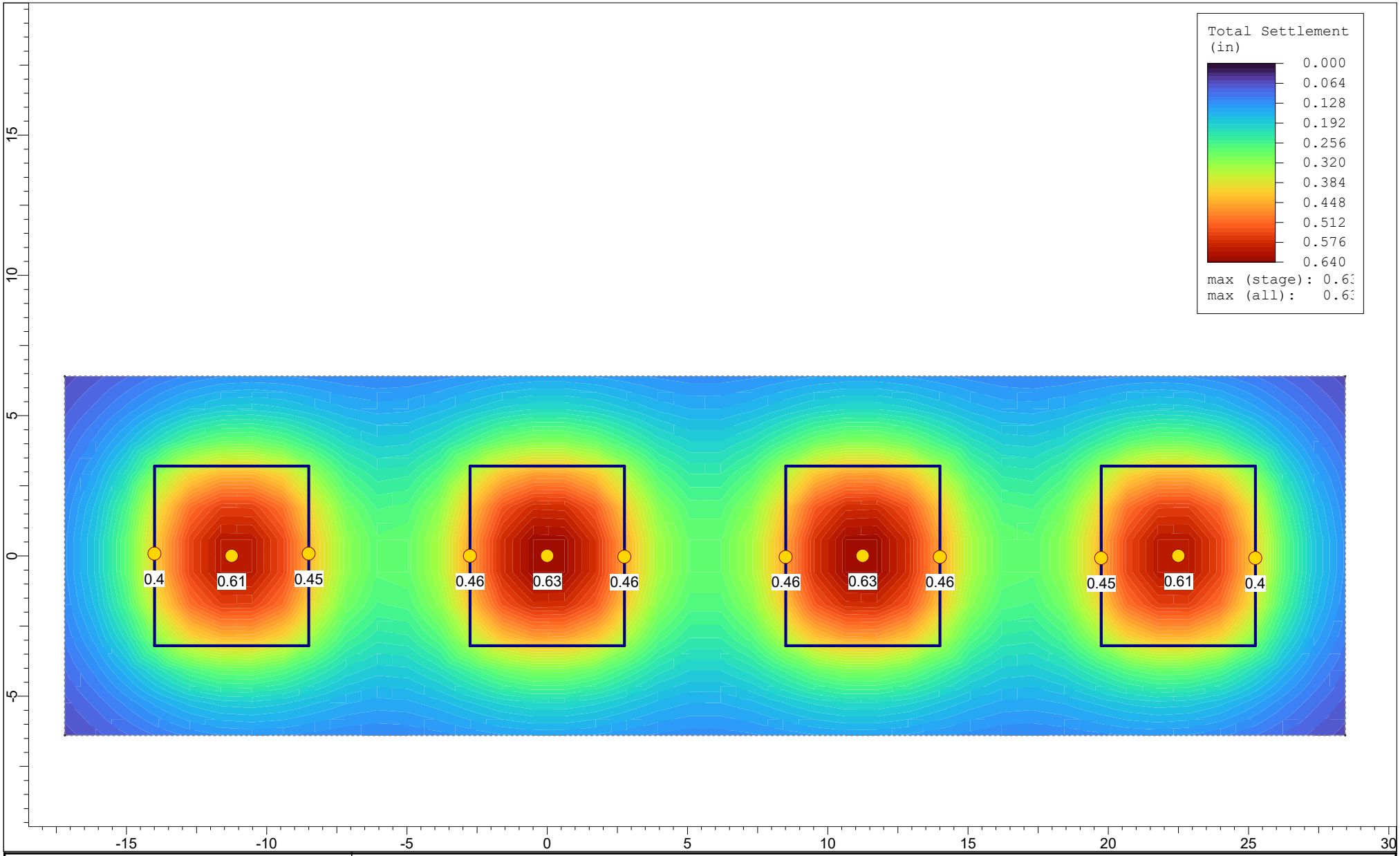
SETTLE3 5.022



rocscience	*Project*	Webster Solar Garden Project	
	Analysis Description		
	Drawn By	SSM	*Company* Haley and Aldrich
	Date	02/09/2026, 10:40:48 PM	*File Name* 2026-0204-Settlement Analysis-D1.s3z

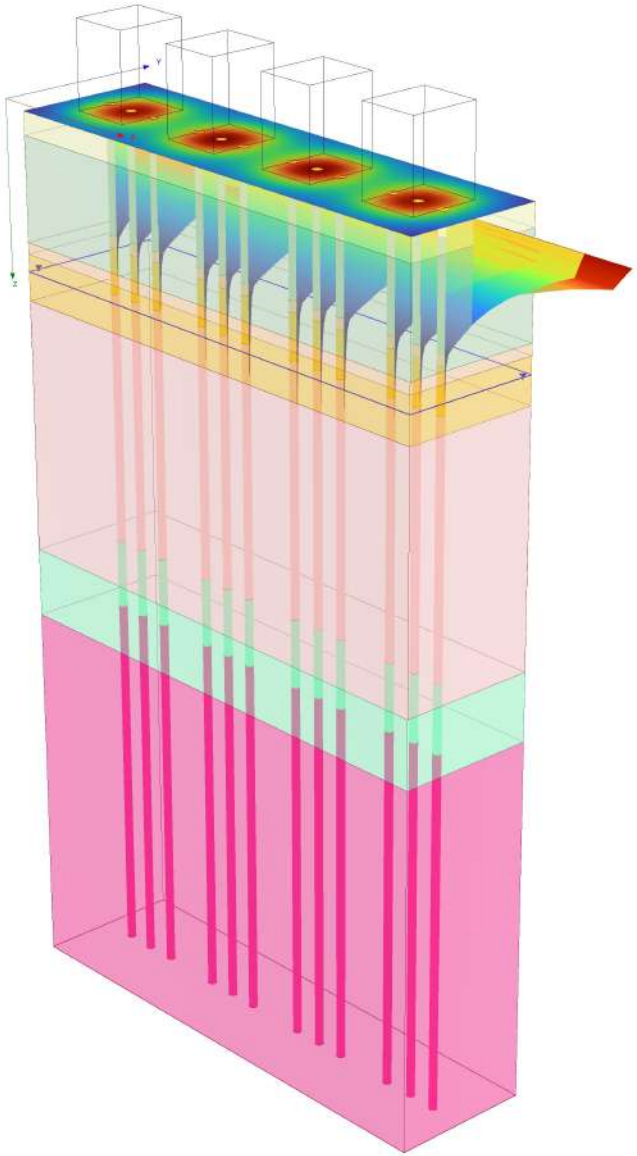
SETTLE3 5.022

SETTLEMENT ANALYSIS
(EQUIPMENT LOAD)



rocscience	*Project*	Webster Solar Garden Project	
	Analysis Description		
	Drawn By	SSM	*Company* Haley and Aldrich
	Date	11/17/2025, 12:56:48 PM	*File Name* 2025-1124-Settlement Analysis-D1.s3z

SETTLE3 5.022



rocscience	*Project*	Webster Solar Garden Project	
	Analysis Description		
	Drawn By	SSM	*Company* Haley and Aldrich
	Date	11/17/2025, 12:56:48 PM	*File Name* 2025-1124-Settlement Analysis-D1.s3z

SETTLE3 5.022

BEARING CAPACITY ANALYSIS (PUNCHING FAILURE)

CALCULATIONS

For rectangular foundations:

$q_{ult} = q_b + (1+(B/L))(2c_a'H/B) + \Upsilon_{top}H^2(1+(B/L))(1+ 2D_f/H)(K_s\tan\phi_{top}/B) - \Upsilon_{top}H \leq q_t$

B = width of foundation
L = length of foundation
c_a' = adhesion
q_b = bearing capacity of the bottom soil layer
q_t = bearing capacity of the top soil layer
δ = inclination of the passive force P_p with the horizontal
D_f = depth of foundation
H = depth from bottom of footing to top of weaker soil layer
K_s = punching shear coefficient
$K_s = f(q_2/q_1, \phi_{top})$

$q_1 = c_{top}N_{c,top}+0.5B\Upsilon_{top}N_{\Upsilon,top}$
$q_2 = c_{bottom}N_{c,bottom}+0.5B\Upsilon_{bottom}N_{\Upsilon,bottom}$
$q_b = c_{bottom}N_{c,bottom}F_{cs,bottom} + \Upsilon_{top}(D_f+H)N_{q,bottom}F_{qs,bottom} + 0.5\Upsilon_{bottom}BN_{\Upsilon,bottom}F_{\Upsilon s,\ bottom}$
$q_t = c_{top}N_{c,top}F_{cs,top} + \Upsilon_{top}D_fN_{q,top}F_{qs,top} + 0.5\Upsilon_{top}BN_{\Upsilon,top}F_{\Upsilon s,top}$

designates input cell (hard coded)

	Unit Weight, Υ (pcf)	Friction Angle, ϕ (degrees)	Cohesion, c (psf)
Top Layer	125	34	0
Bottom Layer	95	20	0

$N_{c,\ top}$ =	42.16	$N_{c,bottom}$ =	14.83
$N_{q,\ top}$ =	29.44	$N_{q,bottom}$ =	6.40
$N_{\Upsilon,top}$ =	41.06	$N_{\Upsilon,bottom}$ =	5.39
$F_{cs,\ top}$ =	1.20	$F_{cs,\ bottom}$ =	1.12
$F_{qs,\ top}$ =	1.19	$F_{qs,\ bottom}$ =	1.10
$F_{\Upsilon s,\ top}$ =	0.89	$F_{\Upsilon s,\ bottom}$ =	0.89

H =	2	ft	
B =	2	ft	
L =	7	ft	
D_f =	0	ft	
q_b =	2,220	psf	
q_t =	4,546	psf	
q_1 =	5,133	psf	
q_2 =	512	psf	
q_2/q_1 =	0.10		
K_s =	3	from Figure 4.8	
c_a' =	0	psf, from Figure 4.9	
q_{ult} =	2,620	psf	OK
F.S. =	3		
q_{all} =	873	psf	
q_{all} =	**0.87**	ksf	

REFERENCE

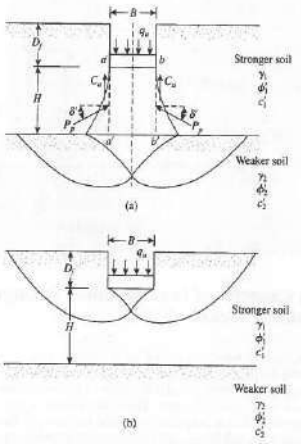
* Principle Foundation Engineering-Sixth Edition/ Braja M. Das

Table 3.3 Bearing Capacity Factors

ϕ'	N_c	N_q	N_γ	ϕ'	N_c	N_q	N_γ
0	5.14	1.00	0.00	26	22.25	11.85	12.54
1	5.38	1.09	0.07	27	23.94	13.20	14.47
2	5.63	1.20	0.15	28	25.80	14.72	16.72
3	5.90	1.31	0.24	29	27.86	16.44	19.34
4	6.19	1.43	0.34	30	30.14	18.40	22.40
5	6.49	1.57	0.45	31	32.67	20.63	25.99
6	6.81	1.72	0.57	32	35.49	23.18	30.22
7	7.16	1.88	0.71	33	38.64	26.00	35.19
8	7.53	2.06	0.86	34	42.16	29.44	41.06
9	7.92	2.25	1.03	35	46.12	33.30	48.03
10	8.35	2.47	1.22	36	50.59	37.75	56.31
11	8.80	2.71	1.44	37	55.63	42.92	66.19
12	9.28	2.97	1.69	38	61.35	48.93	78.03
13	9.81	3.26	1.97	39	67.87	55.96	92.25
14	10.37	3.59	2.29	40	75.31	64.20	109.41
15	10.98	3.94	2.65	41	83.86	73.90	130.22
16	11.63	4.34	3.06	42	93.71	85.38	155.55
17	12.34	4.77	3.53	43	105.11	99.02	186.54
18	13.10	5.26	4.07	44	118.37	115.31	224.64
19	13.93	5.80	4.68	45	133.88	134.88	271.76
20	14.83	6.40	5.39	46	152.10	158.51	330.35
21	15.82	7.07	6.20	47	173.64	187.21	403.67
22	16.88	7.82	7.13	48	199.26	222.31	496.01
23	18.05	8.66	8.20	49	229.93	265.51	613.16
24	19.32	9.60	9.44	50	266.89	319.07	762.89
25	20.72	10.66	10.88				

Layer	Unit weight	Soil friction angle	Cohesion
Top	γ_1	ϕ_1'	c_1'
Bottom	γ_2	ϕ_2'	c_2'

At ultimate load per unit area (q_u), the failure surface in soil will be as shown in the figure. If the depth H is relatively small compared with the foundation width B, a punching shear failure will occur in the top soil layer, followed by a general shear failure in the bottom soil layer. This is shown in Figure 4.7(a). However, if the depth H is relatively large, then the failure surface will be completely located in the top soil layer, which is the upper limit for the ultimate bearing capacity. This is shown in Figure 4.7b.



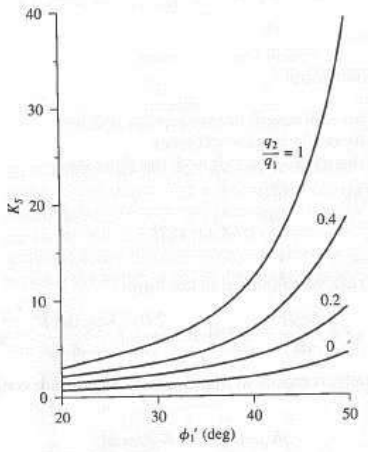


Figure 4.8 Meyerhof and Hanna's punching shear coefficient K_s

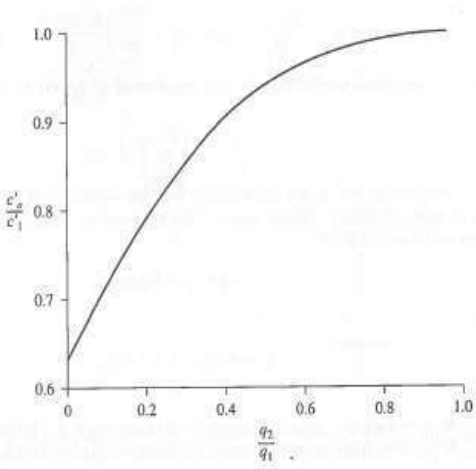


Figure 4.9 Variation of c_2'/c_1' with q_2/q_1 based on the theory of Meyerhof and Hanna (1978)

APPENDIX J
ASPHALT LANDFILL GEOTECHNICAL REPORT (REVISED FEBRUARY 2026)

H & A OF NEW YORK ENGINEERING AND
GEOLOGY, LLP
260 E. Main Street
Suite 2100
Rochester, NY 4604
585.359.9000

Revised February 19, 2026
November 10, 2025
File No. 0205399-000-006-01

TM Montante Solar Development, LLC
2760 Kenmore Avenue
Buffalo, New York 14150

Attention: Ms. Katie Soscia, Director of Project Development

Subject: Geotechnical Assessment for the Asphalt-Capped Landfill
Webster Solar Garden Project
750/800 Phillips Road
Webster, New York

H & A of New York Engineering and Geology, LLP (Haley & Aldrich of New York) was retained by TM Montante Solar Development, LLC (Montante) to perform this geotechnical assessment of landfill cap stability at the proposed Webster Solar Garden Project (Project) located at 750/800 Phillips Road, Webster, Monroe County, New York.

The purpose of this assessment is to address the New York State Department of Environmental Conservation (NYSDEC) Division of Materials Management (DMM-4) guidance: Photovoltaic Solar Projects at Closed Solid Waste Landfills.[1] Specifically, the assessment evaluates potential settlement and loading impacts associated with the installation of photovoltaic (PV) arrays on the existing asphalt-capped landfill.

The objectives of this assessment are to:

- Evaluate the settlement of the landfill cap based on the plate load testing program undertaken and the proposed PV loads and temporary construction equipment loads.
- Assess the potential impact to the landfill cap system from the PV array foundation units and the foundation unit installation methods.
- Review the DMM-4 requirements for documentation of landfill stability as it relates to the proposed development loads.

[1] New York State Department of Environmental Conservation (NYSDEC) Division of Materials Management DMM-4 Photovoltaic Solar Projects at Solid Waste Landfills (dated 1/4/2023) policy is being utilized by NYSDEC staff as guidance to review the Change Of Use Notice to allow PV Solar development under Xerox's Part 373 Permit.

Site Background and Landfill Description

The solar development Project is proposed within the northern unoccupied portions of Xerox Corporation's Webster Campus (Tax Parcel No. 065.02-1-40.11) and includes a 2.8-acre asphalt-capped inactive landfill (NYSDEC Site ID: 828013) designated Solid Waste Management Unit (SWMU) #62 – Inactive Landfill (Closed Solid Waste Management Facility). This landfill reportedly operated between circa 1960 and 1971 and is currently managed under the corrective action portion of Xerox's Part 373 Permit (Site Code 828178). The location of the SWMU #62 Inactive Landfill is shown on Figure 1.

Remedial closure actions for this landfill were reportedly completed prior to 1990 by others and included the installation of a multi-layered cap system consisting of the following components[2]:

- A 4-inch asphalt surface cap comprised of 2.5 inches New York State Department of Transportation [NYSDOT] Type 3 and 1.5 inches NYSDOT Type B bituminous mixture.
- A minimum of 6 to 8 inches of stabilized NYSDOT Type I Subbase Course (Section 304-2.02)
- Underlying geotextile fabric (Trevira 1145), which also reportedly extends around the terminal sections of the gravel subbase.
- Up to 2 feet of soil cover underlying the cap system and 6 inches of compacted clay over the waste materials.

The asphalt cap was designed to limit infiltration. Annual inspections and routine groundwater monitoring (as required under the Site Permit) by others continue to assess its integrity and performance. The landfill's design also includes a groundwater migration control system consisting of a 20-foot-deep bedrock trench located to the north of the landfill surface footprint with associated recovery wells that discharge to the Town of Webster publicly owned treatment works under Xerox's Industrial Sewer Use Permit.

Prior to the construction of the Project, and at the request of the Town, Xerox plans to add an additional 1.5 inches of asphalt surface ahead of construction to minimize potential future maintenance needs. Post-construction, the asphalt cap inspections and groundwater monitoring will continue (as required by the Site Permit).

PROPOSED SOLAR DEVELOPMENT

The proposed development consists of a 6.6-megawatt, direct current, fixed-tilt PV system utilizing geoballast-mounted foundations placed directly on the ground surface to avoid penetration of the existing landfill cap. No penetrations of the landfill asphalt cap are proposed as part of the PV Project.

[2] The cap system details, including the asphalt surface, subbase, and geotextile fabric, are based on Figure LM-5 of the Remedial Program Inactive Landfill Site Closure, As-Built Set, prepared for Xerox Corporation by Woodward-Clyde Consultants and dated July 7, 1989. Information regarding the underlying soil cover is derived from the Feasibility Study for the Inactive Waste Landfill, prepared for Xerox Corporation by O'Brien & Gere Engineers, Inc., dated April 1986.

Each geoballast (gabion-style foundation) will contain approximately 2,250 pounds of stone aggregate, resulting in an estimated total permanent load at each ballast foundation of approximately 450 pounds per square foot (psf), or 3.1 pounds per square inch (psi). This load estimate includes the racking components, photovoltaic modules, and anticipated snow and wind design loads.

The maximum temporary construction load, which is based on the heaviest proposed construction equipment (a Kubota SVL97-2 track loader) is approximately 650 psf (4.5 psi), which is below the 10-psi threshold established by NYSDEC for construction over capped landfills.

Geotechnical Testing Procedures

To assess stability of the asphalt-capped surface, Haley & Aldrich of New York subcontracted CME Associates, Inc. (CME) to perform static load testing in general conformance with ASTM D1194-94 – Standard Test Method for Bearing Capacity of Soil for Static Load and Spread Footings. Although ASTM D1194-94 was formally withdrawn in 2003, it continues to be referenced for informational and comparative purposes within the geotechnical field. A new standard (ASTM WK91344) addressing static soil response under loading is currently under development but has not yet been published.

Field testing for this Project was conducted between October 28 and 29, 2025. Testing employed a rubber-tracked excavator, a 13.8-ton hydraulic jack assembly, and a 1-inch-thick, 12-inch-square steel plate instrumented with two dial gauges mounted on a separate reference beam. The steel plate was placed directly on the asphalt surface without any excavation, and the dial indicator was zeroed prior to loading at each tested location. The load was set up using the base of the excavator boom arm (the excavator King Post). This configuration was used for all tests. Incremental loads were applied in 200-pound steps, with each increment maintained for approximately 15 minutes while deflection readings were recorded at each interval. Testing was performed at three locations, HWL-1 through HWL-3, within the asphalt-capped landfill footprint (see Figure 1). The maximum applied load was 2,000 psf, approximately three times greater than the anticipated construction equipment loading, providing a factor of safety for evaluation purposes. The measured corrected settlement was 0.0162, 0.0151, and 0.0554 inches at HWL-1, HWL-2, and HWL-3, respectively. The load versus settlement curves are provided in Appendix A.

Variations from ASTM D1194-94 were implemented during testing. A single steel bearing plate was used rather than multiple bearing plates of varying diameters. This deviation from the standard was documented and evaluated for applicability to site conditions. CME determined that use of a single bearing plate was appropriate for this test program based on the small test loads as compared to typical test loads under the ASTM standard. A 13.8-ton jack assembly, rather than the 50-ton assembly referenced in the standard, was used; CME determined this to be adequate for the relatively small test loads involved. The reference beam, 8 feet in total length, was supported 4 feet from the center of the loaded area, instead of the 8 feet specified in Section 4.5 of the ASTM standard due to the reduced scale of the load test set up overall. This modification was not expected to affect the testing accuracy, given the limited load magnitude.

Evaluation for Division of Materials Management (DMM-4) Guidance

Since geotechnical subsurface explorations were not permitted within the limits of the asphalt-capped landfill, the evaluations for settlement, slope failure, and slippage are based on the available topographic information and the observations made during the static load test program.

DMM-4 requires that a geotechnical assessment be performed to evaluate site stability and potential impacts to landfill integrity, including considerations of settlement, slope stability, and shear resistance, using methods appropriate to site constraints. Where intrusive subsurface investigations are not feasible due to landfill cap restrictions, DMM-4 allows the use of non-intrusive data, surface observations, historical information, and engineering judgment to assess geotechnical performance. The approach used for this evaluation is consistent with those requirements and reflects the physical limitations imposed by the capped landfill.

SLOPE STABILITY ANALYSIS

The ground surface elevations (Els.) for the asphalt-capped landfill range between approximately El. 379 and El. 382 feet above mean sea level (AMSL). The plan extents of the asphalt-capped landfill are roughly 300 feet by 300 feet. As designed, the landfill surface generally exhibits gentle slopes on the order of approximately 2 percent, to promote surface water runoff. Therefore, by inspection, slope stability is not applicable to the proposed construction.

SETTLEMENT ANALYSIS

The estimated contact pressure for each geoballast foundation is approximately 450 psf (3.1 psi), accounting for the racking components, modules, and a conservate assumption for anticipated snow and wind design loads. Final wind and design load information will be provided upon ordering of project components, and geotechnical analyses can be re-evaluated to confirm these loads do not differ materially from the values assumed. The applied load for the plate load testing was 2,000 pounds over a 12-inch by 12-inch plate, which is an equivalent applied pressure of 2,000 psf. The settlement observed due to the 2,000 psf load was approximately 0.02 to 0.06 inches. Based on the load testing results, we anticipate that the estimated settlement will be less than 0.5 inches.

DIFFERENTIAL SETTLEMENT

The proposed geoballast foundation system has a tolerance for differential settlement of 1 inch of displacement over 1 foot of length. The geoballast foundation system also has telescoping posts that can be adjusted, if needed, to reposition the solar panels in the event that differential settlement occurs.

Based on the plate load testing results and the anticipated load of the new construction, we anticipate that differential settlement will be less than 0.5 inches over the site.

Discussion and Conclusions

The testing results indicate that:

- The landfill cap can accommodate loads associated with the PV geoballasts and array equipment (approximately 450 psf or 3.1 psi) and temporary construction equipment loads (approximately 650 psf or 4.5 psi).
- Temporary construction equipment loads and permanent loads from the system are within allowable limits (less than 10 psi) per NYSDEC DMM-4 guidance.
- No visual damage or cracking of the asphalt surface occurred during testing. Refer to the photographs in Appendix B.

Based on the load test results, we anticipate settlement of the asphalt-capped landfill will be less than 0.5 inches under the future loads. Accordingly, adverse impacts to landfill stability or cap integrity are not anticipated from the proposed development, provided that construction equipment, methods of construction, construction loads, and site conditions remain consistent with those evaluated herein. Localized condition variability may occur; however, given the asphalt cap construction materials, proposed Project loads, testing results, and factor of safety applied, we expect this assessment's conclusions address the DMM-4 guidance requirements for the proposed Change of Use at the site. Ongoing monitoring of the cap condition will continue consistent with the Site Permit requirements.

We appreciate the opportunity to provide engineering services on this Project. Please do not hesitate to contact us if you have any questions or comments.

Sincerely yours,
H & A OF NEW YORK ENGINEERING AND GEOLOGY, LLP

Carrie A. Layhee, P.E. (NY)
Partner

Steven H. Phillips P.G. (NY)
Senior Associate

Enclosures:
Report Certification
Figure 1 – Test location Plan
Appendix A – CME Associates, Inc. Test Data and Load vs. Settlement Curve Results
Appendix B – CME Associates, Inc. Field Photographs

REPORT CERTIFICATION

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Consulting and Engineering Firm: H & A of New York Engineering and Geology, LLP

Name: Carrie A. Layhee, P.E. (NY)

Title: Partner

Signature: Carrie A. Layhee

Date: February 24, 2026



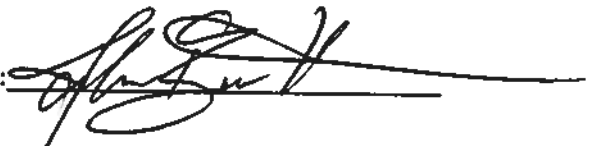
REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Site Owner: Xerox Corporation

Name: John Barrett

Title: Manager - Western Hemisphere Real Estate Portfolio

Signature: 

Date: 2 / 24 / 2026

Docusign Envelope ID: 78989487-F70B-4024-ABAD-B6E1F7E1C3A8

REPORT CERTIFICATION

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision according to a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Operator (Project Developer): TM Montante Solar Development LLC

Name: Dan Montante

Title: Member

Signature: Signed by: Dan Montante 842D93661ABC439...

Date: 2/25/2026

FIGURE

GIS FILE PATH: C:\Users\trobitaille\OneDrive - haleyaldrich.com\Desktop\GIS PROJECTS\Xerox Solar Montante\MAP DOCUMENTS\2025-10\0205399_002_SITE_PLAN_geotech_hazlandfill_d2.mxd — USER: trobitaille — LAST SAVED: 11/6/2025 1:37:18 PM

B-17A
B-24B
B-24
B-24A
B-20
B-20A
B-20B
LF-B-37-I
B-21A
B-21IR
B-21DR
W-5
W-4
RW-1
RW-2
B-1
HWL-03
HWL-02
HWL-01
B-22B
B-22A
B-22
SWMU #62 - CAPPED INACTIVE LANDFILL
B-13
B-7
CARACUS DR
RESENDE RD
CARACAS DR
B-18A
B-18
B-18SR
B-25A
B-26SR
B-27
B-27I
B-27A
B-28
B-28A
DENHAM RD
EBINA DR

LEGEND

- SOLAR PROJECT STUDY AREA
- TEST LOCATIONS
- XEROX PROPERTY BOUNDARY
- 1 FT TOPOGRAPHIC CONTOURS
- CAPPED INACTIVE LANDFILL

OTHER SITE FEATURES

- GROUNDWATER PUMPING WELL
- GROUNDWATER QUALITY MONITORING (GQM) WELL
- TRIGGER WELL
- LINE OF COMPLIANCE WELL
- WELL LEADER LINES
- BLASTED TRENCH IN BEDROCK

NOTES

1. ALL LOCATIONS AND DIMENSIONS ARE APPROXIMATE.
2. AERIAL BASE MAP SOURCE: ESRI WORLD IMAGERY
3. WELL LOCATION DATA SOURCE: XEROX CORPORATION
4. TOPOGRAPHIC DATA SOURCE: MONROE COUNTY LIDAR DOWNLOAD SITE

N

0 200 400
SCALE IN FEET

HALEY ALDRICH

TM MONTANTE SOLAR DEVELOPMENT LLC
PROPOSED SOLAR PROJECT - 139 CARACAS DRIVE
TOWN OF WEBSTER
MONROE COUNTY, NEW YORK

TEST LOCATION PLAN

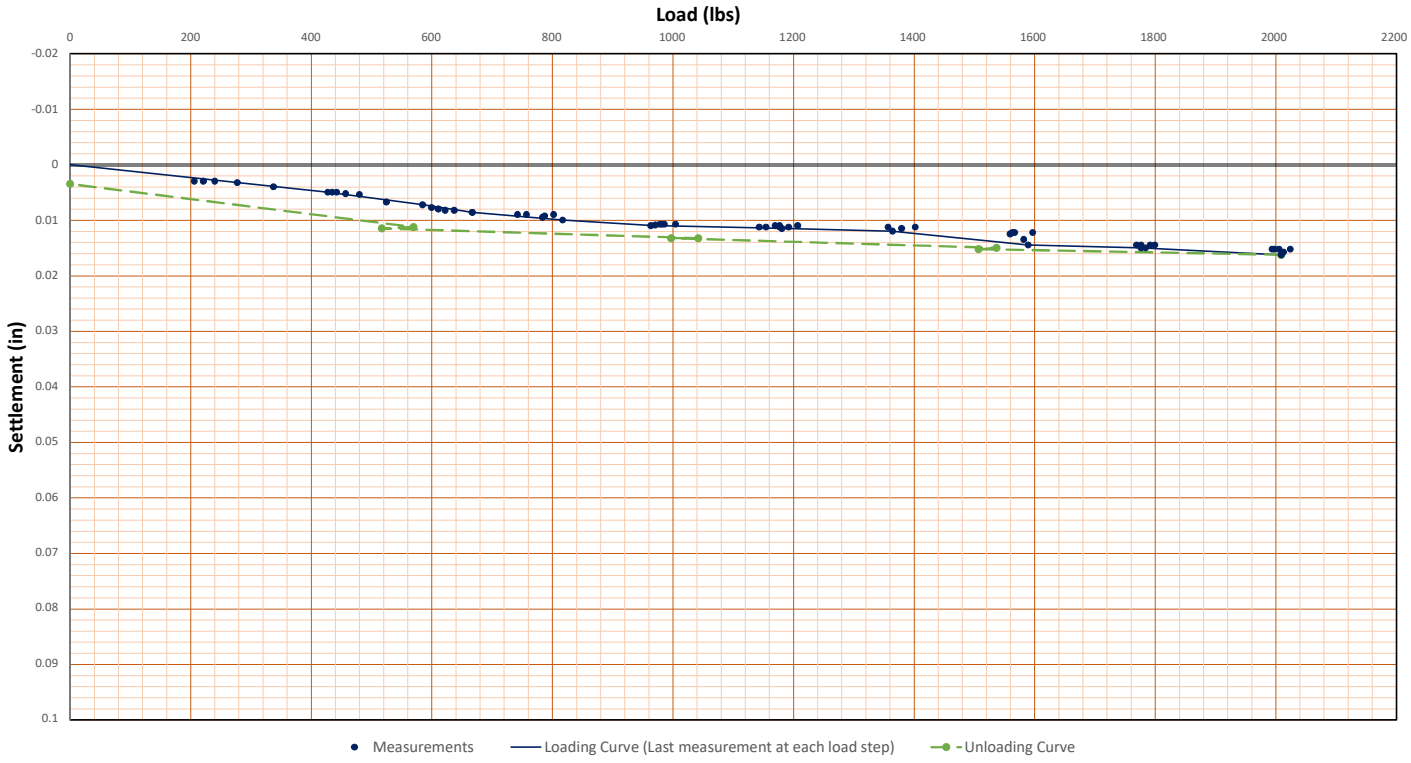
SCALE: AS SHOWN
LAST UPDATED: NOVEMBER 2025

FIGURE 1

APPENDIX A
CME Associates, Inc.
Test Data and Load vs. Settlement Curve Results

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3]	Measured Load (lbs)
200 lbs [Note 4]	0	2.9	206
	1	2.9	221
	2	2.9	240
	4	3.2	277
	8	3.9	337
	15	4.9	427
400 lbs [Note 4]	0	4.9	435
	1	4.9	442
	2	5.2	457
	4	5.3	480
	8	6.7	525
	15	7.2	585
600 lbs [Note 4]	0	7.7	600
	1	7.9	611
	2	8.2	622
	4	8.2	637
	8	8.6	667
	15	8.6	667
800 lbs	0	8.9	802
	1	8.9	757
	2	8.9	742
	4	9.2	787
	8	9.4	783
	15	9.9	817
1000 lbs	0	10.7	1005
	1	10.7	986
	2	10.7	982
	4	10.7	978
	8	10.8	971
	15	10.9	963
1200 lbs	0	10.9	1207
	1	10.9	1177
	2	10.9	1169
	4	11.2	1192
	8	11.2	1177
	15	11.4	1181
1400 lbs	0	11.2	1402
	1	11.2	1154
	2	11.2	1143
	4	11.2	1357
	8	11.4	1379
	15	11.9	1364
1600 lbs	0	12.2	1597
	1	12.2	1567
	2	12.2	1563
	4	12.4	1559
	8	13.4	1582
	15	14.4	1589
1800 lbs	0	14.4	1799
	1	14.4	1777
	2	14.4	1769
	4	14.4	1792
	8	14.9	1784
	15	14.9	1777
2000 lbs	0	15.2	2024
	1	15.2	2005
	2	15.2	1998
	4	15.2	1994
	8	15.7	2013
	15	16.2	2009
1500 lbs	0	15.2	1507
	1	14.9	1537
1000 lbs	0	13.2	997
	1	13.2	1042
500 lbs	0	11.4	517
	1	11.2	570
0 lbs	0	3.4	0

Load vs Settlement Curve (Test Location HWL-1)



Bearing Surface Classification: Asphalt

Sample Depth: At-Grade
Sample Date: 10/29/2025
Sample Time: 9:15 AM to 12:15 PM
Temperature and Weather: Cloudy, ~40°F - 50°F
Set Load: 165 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

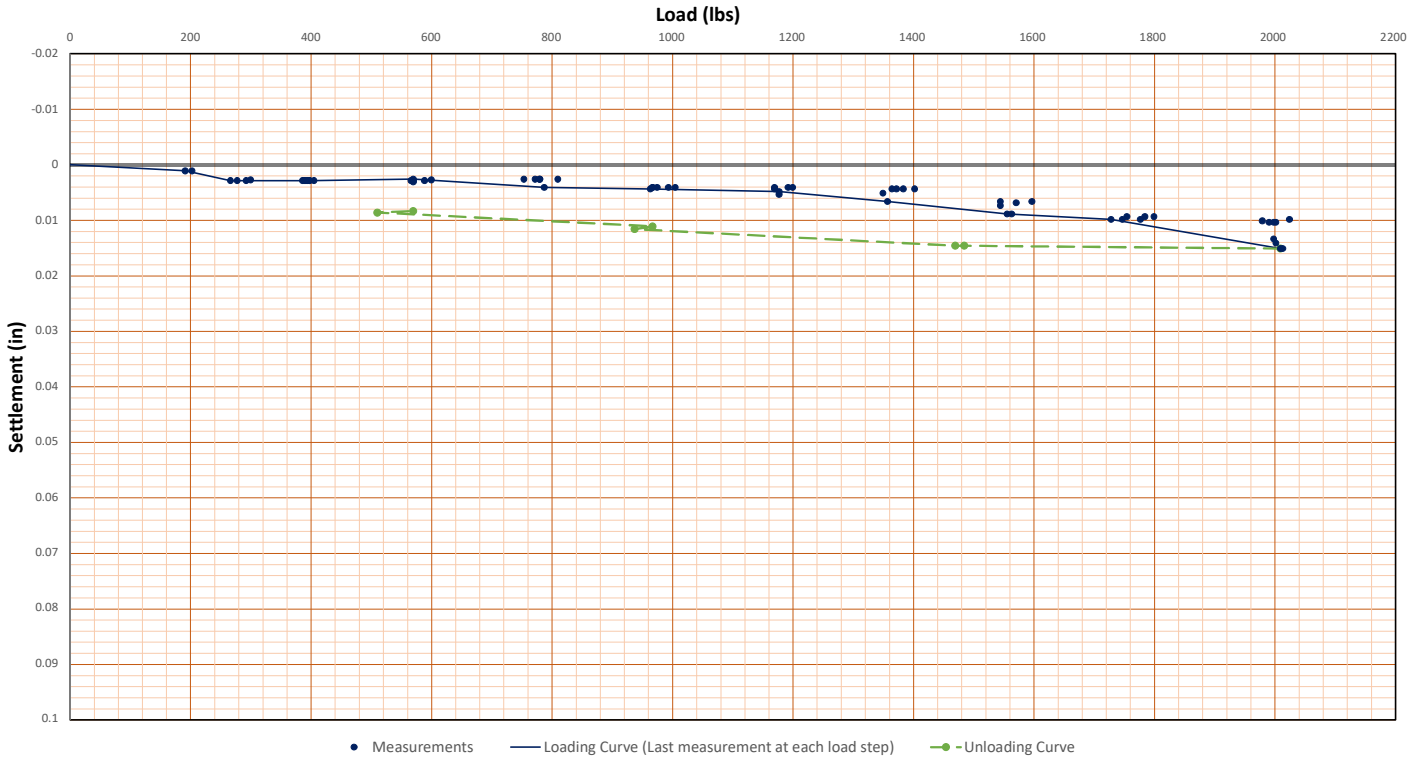
Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.
b. Readings were corrected for surface irregularities and initial zeroing at the set load. This correction was performed by shifting the settlement readings so that the linear fit of the early loading curve reports zero settlement at zero load.

Note 4: Measured Load continuously increased during these target load steps, possibly due to hydraulics in the excavator relaxing and placing more load on the load column.*

*Haley & Aldrich of New York - Supplemental Note: The excavator bucket was reportedly placed on the ground with minimal force prior to testing, inadvertently lifting the front of the excavator a few centimeters. After the load test was set up and in progress under the boom arm, the hydraulics in the boom arm slowly relaxed, causing the excavator to relax back to a neutral position on the ground.

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3]	Measured Load (lbs)
200 lbs	0	1.1	202
	1	1.1	191
	2 [Note 4]	2.7	300
	4	2.8	292
	8	2.8	277
	15	2.8	266
400 lbs	0	2.8	405
	1	2.8	394
	2	2.8	390
	4	2.8	397
	8	2.8	386
	15	2.8	397
600 lbs	0	2.7	600
	1	2.8	566
	2	2.8	588
	4	2.8	570
	8	3.1	570
	15	2.6	570
800 lbs	0	2.6	810
	1	2.6	780
	2	2.6	772
	4	2.6	753
	8	2.6	780
	15	4.1	787
1000 lbs	0	4.1	1005
	1	4.1	967
	2	4.1	993
	4	4.1	967
	8	4.1	975
	15	4.3	963
1200 lbs	0	4.1	1199
	1	4.1	1169
	2	4.1	1192
	4	4.3	1169
	8	5.3	1177
	15	4.8	1177
1400 lbs	0	4.3	1402
	1	4.3	1364
	2	4.3	1383
	4	4.3	1372
	8	5.1	1349
	15	6.6	1357
1600 lbs	0	6.6	1597
	1	6.6	1544
	2	6.8	1570
	4	7.3	1544
	8	8.8	1563
	15	8.8	1555
1800 lbs	0	9.3	1799
	1	9.3	1754
	2	9.3	1784
	4	9.8	1777
	8	9.8	1747
	15	9.8	1728
2000 lbs [Note 5]	0	9.8	2024
	1	10.1	1979
	2	10.3	2002
	4	10.3	1998
	8	10.3	1990
	15	13.3	1998
	20	14.1	2002
	25	15.1	2013
	30	15.1	2009
1500 lbs	0	14.6	1484
	1	14.6	1469
1000 lbs	0	11.6	937
	1	11.1	967
500 lbs	0	8.6	510
	1	8.3	570
0 lbs	Note 6		

Load vs Settlement Curve (Test Location HWL-2)



Bearing Surface Classification: Asphalt

Sample Depth: At-Grade
Sample Date: 10/28/2025
Sample Time: 10:00 AM to 1:15 PM
Temperature and Weather: Clear, ~50°F
Set Load: 190 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.
b. Readings were corrected for surface irregularities and initial zeroing at the set load. This correction was performed by shifting the settlement readings so that the linear fit of the early loading curve reports zero settlement at zero load.

Note 4: Measured Load jumped to 300 lbs when reapplying pressure to maintain target load at this measurement step.

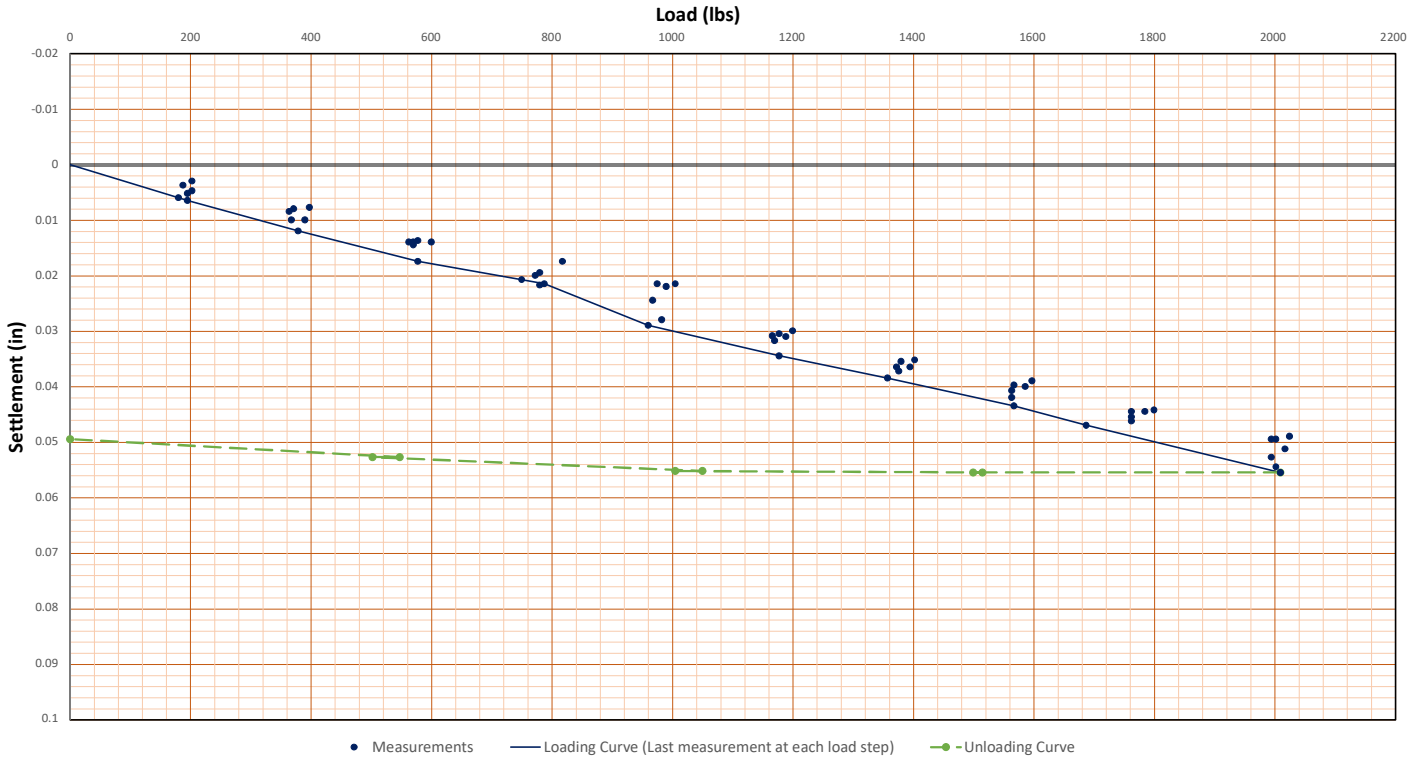
Note 5: An extended hold was performed at this load step to measure creep.

Note 6: Measurements at final 0 lb unloading step could not be obtained.*

*Haley & Aldrich of New York - Supplemental Note: When the load drops below the set-load, the loading test setup is no longer held tight in compression, and becomes unstable. At this testing location, a part of the load test setup fell over and disrupted the dial gauges before the final reading could be obtained.

Target Load	Measurement Time (minutes after application of load)	Corrected Settlement (1/1000 in) [Note 3]	Measured Load (lbs)
200 lbs	0	2.9	202
	1	3.7	187
	2	4.7	202
	4	5.2	195
	8	5.9	180
	15	6.4	195
400 lbs	0	7.7	397
	1	7.9	371
	2	8.4	364
	4	9.9	390
	8	9.9	367
	15	11.9	379
600 lbs	0	13.9	600
	1	13.9	570
	2	13.9	562
	4	13.7	577
	8	14.4	570
	15	17.4	577
800 lbs	0	17.4	817
	1	19.4	780
	2	19.9	772
	4	20.7	750
	8	21.7	780
	15	21.4	787
1000 lbs	0	21.4	1005
	1	21.4	975
	2	21.9	990
	4	24.4	967
	8	27.9	982
	15	28.9	960
1200 lbs	0	29.9	1199
	1	30.4	1177
	2	30.8	1166
	4	30.9	1188
	8	31.7	1169
	15	34.4	1177
1400 lbs	0	35.2	1402
	1	35.4	1379
	2	36.4	1394
	4	36.4	1372
	8	37.2	1376
	15	38.4	1357
1600 lbs	0	38.9	1597
	1	39.7	1567
	2	39.9	1585
	4	40.7	1563
	8	41.9	1563
	15	43.4	1567
1800 lbs	0	44.2	1799
	1	44.4	1762
	2	44.4	1784
	4	45.4	1762
	8	46.2	1762
	15	46.9	1687
2000 lbs [Note 4]	0	48.9	2024
	1	49.4	2002
	2	49.4	1994
	4	51.2	2017
	8	52.7	1994
	15	54.4	2002
	20	55.4	2009
	25	55.4	2009
1500 lbs	0	55.4	1499
	1	55.4	1514
1000 lbs	0	55.2	1005
	1	55.2	1049
500 lbs	0	52.7	502
	1	52.7	547
0 lbs	0	49.4	0

Load vs Settlement Curve (Test Location HWL-3)



Bearing Surface Classification: Asphalt

Sample Depth: At-Grade
Sample Date: 10/28/2025
Sample Time: 1:45 PM to 4:45 PM
Temperature and Weather: Clear, ~50°F
Set Load: 20 lbs

Comments:

Note 1: A 13.8 ton jack was utilized instead of the 50-ton jack specified in ASTM D1194-94. This jack was more than sufficient for the 2000 lbs test load specified by Client.

Note 2: The 8 foot reference beam was only able to be supported 4 feet from the center of the loaded area, short of the 8 foot support distance specified in ASTM D1194-94. In CME's professional opinion, this deviation will not adversely affect the test readings due to the relatively small specified test load.

Note 3: The following corrections were applied to the raw recorded data to obtain the Corrected Settlement:
a. Readings from both settlement dial gauges were averaged together.
b. Readings were corrected for surface irregularities and initial zeroing at the set load. This correction was performed by shifting the settlement readings so that the linear fit of the early loading curve reports zero settlement at zero load.

Note 4: An extended hold was performed at this load step to measure creep.

CME Report #2500581E-01-0226 Recorded Field Data

CME Associates, Inc.

Target Load	Measurement Time (minutes after application of load)	CDL-1			CDL-2			CDL-3			HWL-1			HWL-2			HWL-3		
		Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]	Displacement Dial 1 Raw Reading (1/1000th inch)	Displacement Dial 2 Raw Reading (1/1000th inch)	Proving Ring Dial Raw Reading (revolutions) [749.63 lbs per revolution]
Set Load	0	100.00	100.00	0.165	100.00	100.00	0.155	100.00	100.00	0.085	100.00	100.00	0.220	100.00	100.00	0.250	100.00	100.00	0.025
200 lbs	0	100.00	100.00	0.270	100.00	101.00	0.270	97.50	98.50	0.270	100.00	100.00	0.275	101.50	98.00	0.270	98.00	98.00	0.270
	1	99.75	100.25	0.270	99.75	101.00	0.280	97.50	98.00	0.270	100.00	100.00	0.295	102.50	97.00	0.255	97.50	97.00	0.250
	2	99.75	100.25	0.270	99.75	101.00	0.275	97.50	98.00	0.270	100.00	100.00	0.320	101.00	95.25	0.400	96.50	96.00	0.270
	4	99.75	100.25	0.265	99.75	101.00	0.275	97.50	98.00	0.270	99.50	100.00	0.370	101.00	95.00	0.390	96.00	95.50	0.260
	8	99.75	100.25	0.250	100.00	102.00	0.270	97.50	98.00	0.265	98.50	99.50	0.450	101.00	95.00	0.370	95.00	95.00	0.240
	15	99.75	101.00	0.250	100.50	103.00	0.260	97.50	98.00	0.255	97.00	99.00	0.570	101.00	95.00	0.355	94.50	94.50	0.260
400 lbs	0	99.00	99.00	0.540	100.50	103.00	0.540	93.00	94.50	0.530	97.00	99.00	0.580	101.00	95.00	0.540	93.00	93.50	0.530
	1	98.50	99.00	0.520	101.00	103.00	0.530	93.00	94.00	0.530	97.00	99.00	0.590	101.00	95.00	0.525	93.00	93.00	0.495
	2	98.50	99.00	0.510	101.00	103.00	0.525	93.00	94.00	0.525	96.75	98.75	0.610	101.00	95.00	0.520	92.50	92.50	0.485
	4	98.50	99.00	0.495	101.00	103.00	0.520	93.00	94.00	0.515	96.50	98.75	0.640	101.00	95.00	0.530	91.00	91.00	0.520
	8	98.00	99.00	0.520	101.00	103.00	0.520	93.00	94.00	0.505	95.00	97.50	0.700	101.00	95.00	0.515	91.00	91.00	0.490
	15	98.00	99.00	0.500	102.00	104.00	0.510	93.00	94.00	0.490	94.00	97.50	0.780	101.00	95.00	0.530	89.00	89.00	0.505
600 lbs	0	96.00	96.50	0.800	102.00	104.00	0.800	88.00	89.00	0.810	93.50	97.00	0.800	101.50	94.75	0.800	87.00	87.00	0.800
	1	96.00	96.50	0.770	102.00	104.00	0.780	87.00	88.00	0.785	93.50	96.50	0.815	101.50	94.50	0.755	87.00	87.00	0.760
	2	96.00	96.50	0.755	102.00	104.00	0.770	87.00	88.00	0.775	93.00	96.50	0.830	101.50	94.50	0.785	87.00	87.00	0.750
	4	95.50	96.00	0.780	102.00	104.00	0.760	87.00	88.00	0.760	93.00	96.50	0.850	101.50	94.50	0.760	87.50	87.00	0.770
	8	95.50	96.00	0.750	102.00	104.00	0.790	87.00	88.00	0.780	92.50	96.25	0.890	101.50	94.00	0.760	87.00	86.00	0.760
	15	95.50	96.00	0.760	102.00	104.00	0.775	86.50	87.50	0.755	92.50	96.25	0.890	102.50	94.00	0.760	84.00	83.00	0.770
800 lbs	0	94.50	95.00	1.070	102.50	104.50	1.070	84.00	84.50	1.070	92.00	96.00	1.070	102.50	94.00	1.080	84.00	83.00	1.090
	1	94.00	94.50	1.010	102.50	104.50	1.040	83.00	84.00	1.030	92.00	96.00	1.010	102.50	94.00	1.040	82.00	81.00	1.040
	2	94.00	94.50	1.000	102.50	104.50	1.030	83.00	83.50	1.020	92.00	96.00	0.990	102.50	94.00	1.030	82.00	80.00	1.030
	4	93.50	94.00	1.040	102.50	104.50	1.020	83.00	83.50	1.000	91.50	96.00	1.050	102.50	94.00	1.005	81.00	79.50	1.000
	8	93.00	94.00	1.005	102.50	104.50	1.050	82.50	83.00	1.040	91.00	96.00	1.045	102.50	94.00	1.040	79.00	79.50	1.040
	15	93.00	94.00	1.000	103.00	105.00	1.020	82.50	83.00	1.010	90.50	95.50	1.090	101.50	92.00	1.050	80.00	79.00	1.050
1000 lbs	0	92.00	92.00	1.330	103.00	105.00	1.330	78.50	79.00	1.350	89.50	95.00	1.340	101.50	92.00	1.340	80.00	79.00	1.340
	1	91.50	92.00	1.280	103.00	105.00	1.300	78.00	78.00	1.325	89.50	95.00	1.315	101.50	92.00	1.290	80.00	79.00	1.300
	2	91.50	92.00	1.260	103.00	105.00	1.290	78.00	78.00	1.320	89.50	95.00	1.310	101.50	92.00	1.325	80.00	78.00	1.320
	4	91.00	91.00	1.300	103.00	105.00	1.310	78.00	78.00	1.300	89.50	95.00	1.305	101.50	92.00	1.290	77.00	76.00	1.290
	8	91.00	91.00	1.260	103.50	107.00	1.290	78.00	78.00	1.275	89.25	95.00	1.295	101.50	92.00	1.300	74.00	72.00	1.310
	15	90.50	91.00	1.245	103.50	107.00	1.310	78.00	78.00	1.330	89.00	95.00	1.285	101.50	91.50	1.285	73.00	71.00	1.280
1200 lbs	0	90.00	90.50	1.600	103.50	107.00	1.630	74.00	76.00	1.610	89.00	95.00	1.610	102.00	91.50	1.600	72.00	70.00	1.600
	1	90.00	90.00	1.530	103.50	107.00	1.585	74.00	76.00	1.545	89.00	95.00	1.570	102.00	91.50	1.560	71.50	69.50	1.570
	2	89.00	89.50	1.575	103.50	107.00	1.580	74.00	76.00	1.530	89.00	95.00	1.560	102.00	91.50	1.590	71.00	69.25	1.555
	4	89.00	89.00	1.565	103.50	107.00	1.560	73.00	76.00	1.600	88.50	95.00	1.590	102.00	91.00	1.560	71.00	69.00	1.585
	8	89.00	89.00	1.550	103.50	107.00	1.580	73.00	76.00	1.545	88.50	95.00	1.570	101.00	90.00	1.570	70.00	68.50	1.560
	15	89.00	89.00	1.550	103.50	107.00	1.550	73.00	76.00	1.490	88.00	95.00	1.575	101.00	91.00	1.570	68.00	65.00	1.570
1400 lbs	0	Load step skipped due to forecast inclement weather			103.50	107.00	1.870	72.00	75.00	1.870	88.00	95.50	1.870	102.00	91.00	1.870	67.00	64.50	1.870
	1				103.50	107.00	1.810	71.50	75.00	1.830	88.00	95.50	1.540	102.00	91.00	1.820	66.50	64.50	1.840
	2				103.50	107.00	1.800	71.50	75.00	1.800	88.00	95.50	1.525	102.00	91.00	1.845	65.00	64.00	1.860
	4				103.50	107.00	1.790	71.50	75.00	1.770	88.00	95.50	1.810	102.00	91.00	1.830	65.00	64.00	1.830
	8				103.50	107.00	1.840	71.25	75.00	1.810	87.50	95.50	1.840	101.50	90.00	1.800	65.00	62.50	1.835
	15				103.50	108.00	1.810	71.50	75.00	1.750	87.00	95.00	1.820	100.00	88.50	1.810	64.00	61.00	1.810
1600 lbs	0	86.00	85.00	2.150	103.50	108.00	2.140	70.00	74.00	2.130	86.50	95.00	2.130	100.00	88.50	2.130	63.00	61.00	2.130
	1	86.00	85.00	2.000	103.50	108.00	2.080	69.50	73.00	2.075	86.50	95.00	2.090	100.00	88.50	2.060	62.50	60.00	2.090
	2	86.00	85.00	1.995	103.50	108.00	2.070	69.00	73.50	2.055	86.50	95.00	2.085	100.00	88.00	2.095	62.00	60.00	2.115
	4	85.50	85.00	2.075	103.50	108.00	2.110	69.00	73.50	2.030	86.00	95.00	2.080	99.50	87.50	2.060	61.50	59.00	2.085
	8	85.00	84.00	2.050	103.50	108.00	2.090	69.00	73.00	2.080	85.00	94.00	2.110	98.00	86.00	2.085	60.00	58.00	2.085
	15	84.50	84.00	2.030	103.00	109.00	2.060	69.00	73.00	2.015	84.00	93.00	2.120	98.00	86.00	2.075	59.00	56.00	2.090
1800 lbs	0	Load step skipped due to forecast inclement weather			103.00	109.00	2.400	67.50	72.00	2.400	84.00	93.00	2.400	97.50	85.50	2.400	58.00	55.50	2.400
	1				103.00	109.00	2.360	67.00	72.00	2.340	84.00	93.00	2.370	97.50	85.50	2.340	58.00	55.00	2.350
	2				103.00	109.00	2.350	67.00	71.50	2.320	84.00	93.00	2.360	97.50	85.50	2.380	58.00	55.00	2.380
	4				103.00	109.00	2.390	66.50	71.00	2.370	84.00	93.00	2.390	97.00	85.00	2.370	57.00	54.00	2.350
	8				103.00	109.00	2.360	66.00	71.00	2.310	83.00	93.00	2.380	97.00	85.00	2.330	56.00	53.50	2.350
	15				103.00	109.50	2.320	66.00	71.00	2.230	83.00	93.00	2.370	97.00	85.00	2.305	55.50	52.50	2.250
2000 lbs	0	81.00	82.00	2.700	103.00	109.00	2.700	64.50	69.00	2.730	82.50	93.00	2.700	97.00	85.00	2.700	53.50	50.50	2.700
	1	81.00	80.00	2.700	103.00	109.00	2.665	64.00	68.50	2.660	82.50	93.00	2.675	96.50	85.00	2.640	53.00	50.00	2.670
	2	81.00	79.50	2.690	103.00	109.00	2.645	64.00	68.25	2.650	82.50	93.00	2.665	96.50	84.50	2.670	53.00	50.00	2.660
	4	81.00	79.00	2.700	102.00	109.00	2.685	63.50	69.00	2.675	82.50	93.00	2.660	96.50	84.50	2.665	51.50	48.00	2.690
	8	80.00	79.00	2.650	102.00	110.00	2.650	63.50	69.00	2.645	81.50	93.00	2.685	96.50	84.50	2.655	50.00	46.50	2.660
	15	80.00	79.00	2.700	102.00	110.00	2.630	63.50	69.00	2.655	81.00	92.50	2.680	94.00	81.00	2.665	48.00	45.00	2.670
	20 (Discretionary extended hold)													93.50	80.00	2.670	47.00	44.00	2.680
	25 (Discretionary extended hold)													92.50	79.00	2.685	47.00	44.00	2.680
	30 (Discretionary extended hold)													92.50	79.00	2.680			
1500 lbs	0	80.00	79.00	2.000	102.00	110.00	2.000	63.50	69.00	1.990	82.00	93.50	2.010	93.00	79.50	1.980	47.00	44.00	2.000
	1	80.00	79.00	2.040	102.00	110.00	2.025	63.50	69.00	2.010	82.00	94.00	2.050	93.00	79.50	1.960	47.00	44.00	2.020
1000 lbs	0	80.00	79.00	1.340	102.50	110.25	1.330	64.50	69.25	1.350	84.00	95.50	1.330	96.00	82.50	1.250	47.50	44.00	1.340
	1	80.00	79.00	1.370	102.50	110.50	1.385	64.50	69.25	1.385	84.00	95.50	1.390	97.00	82.50	1.290	47.50	44.00	1.400
500 lbs	0	81.00	82.00	0.650	104.00	112.00	0.670	66.75	70.50	0.670	86.00	97.00	0.690	99.00	85.50	0.680	50.00	46.50	0.670
	1	81.00	82.00	0.720	104.00	112.00	0.710	66.75	70.50	0.710	86.00	97.50	0.760	99.00	86.00	0.760	50.00	46.50	0.730
0 lbs	0	81.00	87.00	0.000	109.00	117.00	0.000	Could not be obtained			94.50	104.50	0.000	Could not be obtained			53.00	50.00	0.000

APPENDIX B
CME Associates, Inc.
Field Photographs



Figure 8: Static Load Test Setup at HWL-1.



Figure 9: Bearing surface after completion of the Static Load Test at HWL-1.



Figure 10: Static Load Test Setup at HWL-2.

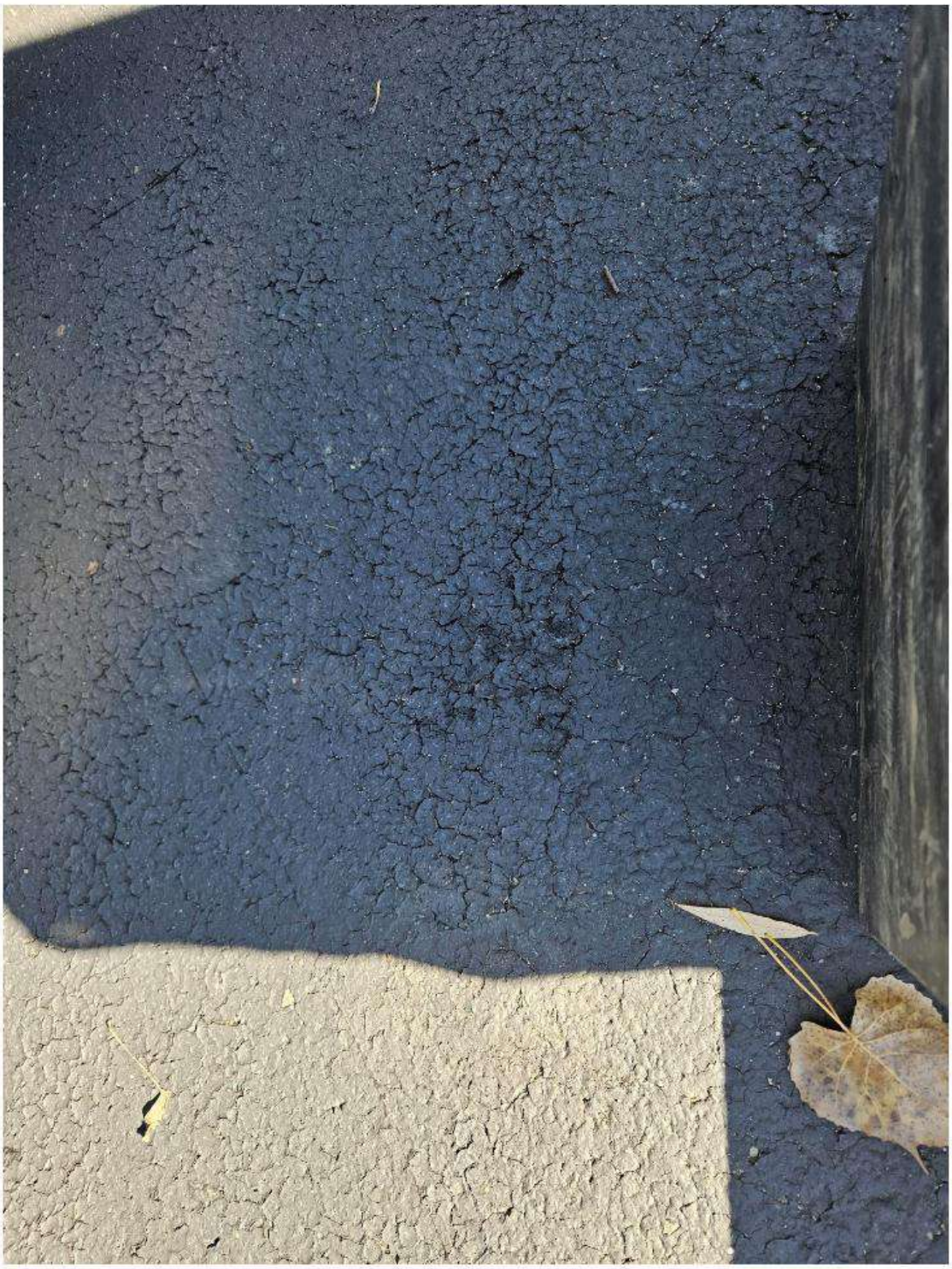


Figure 11: Bearing surface after completion of the Static Load Test at HWL-2.

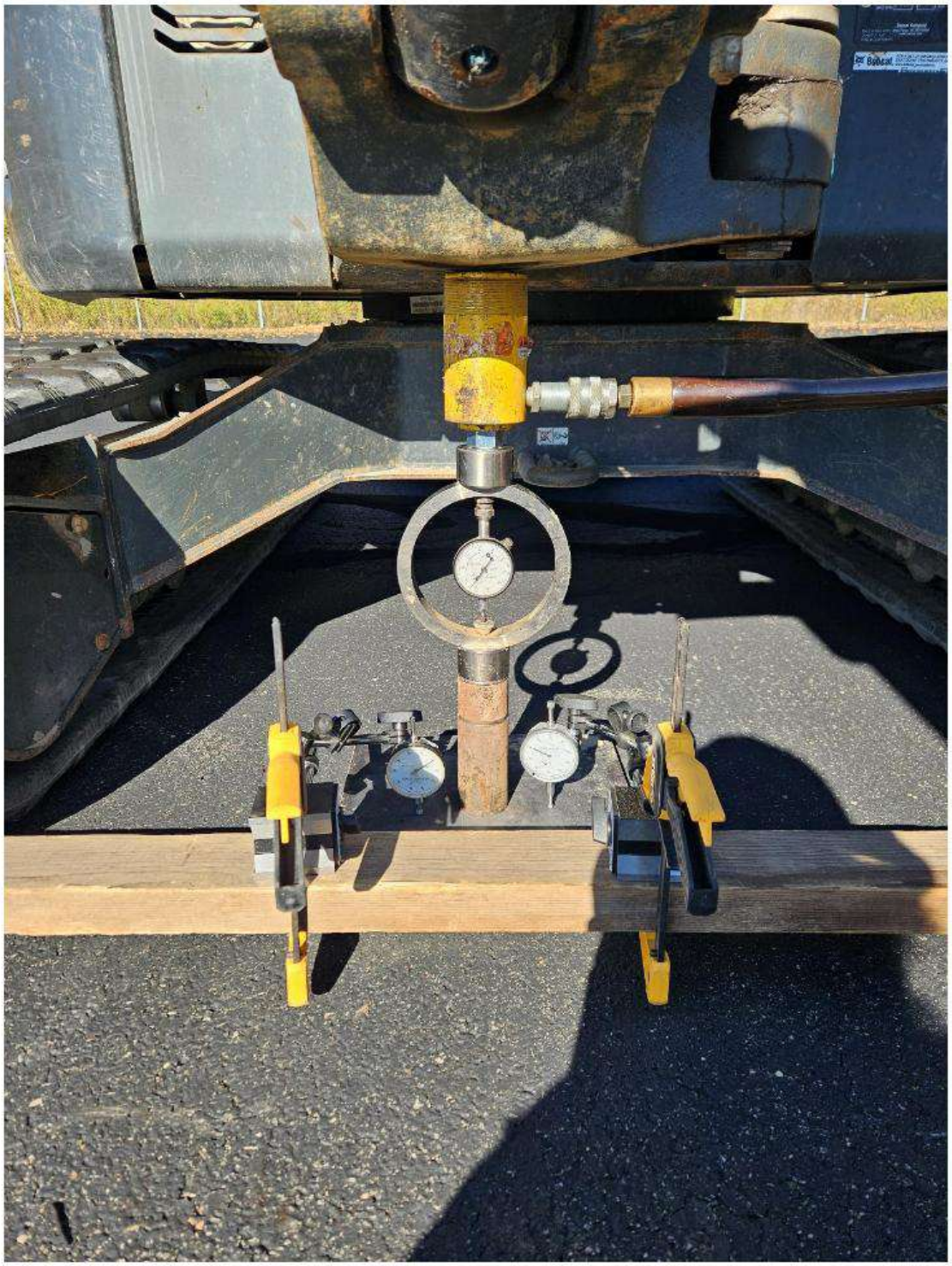
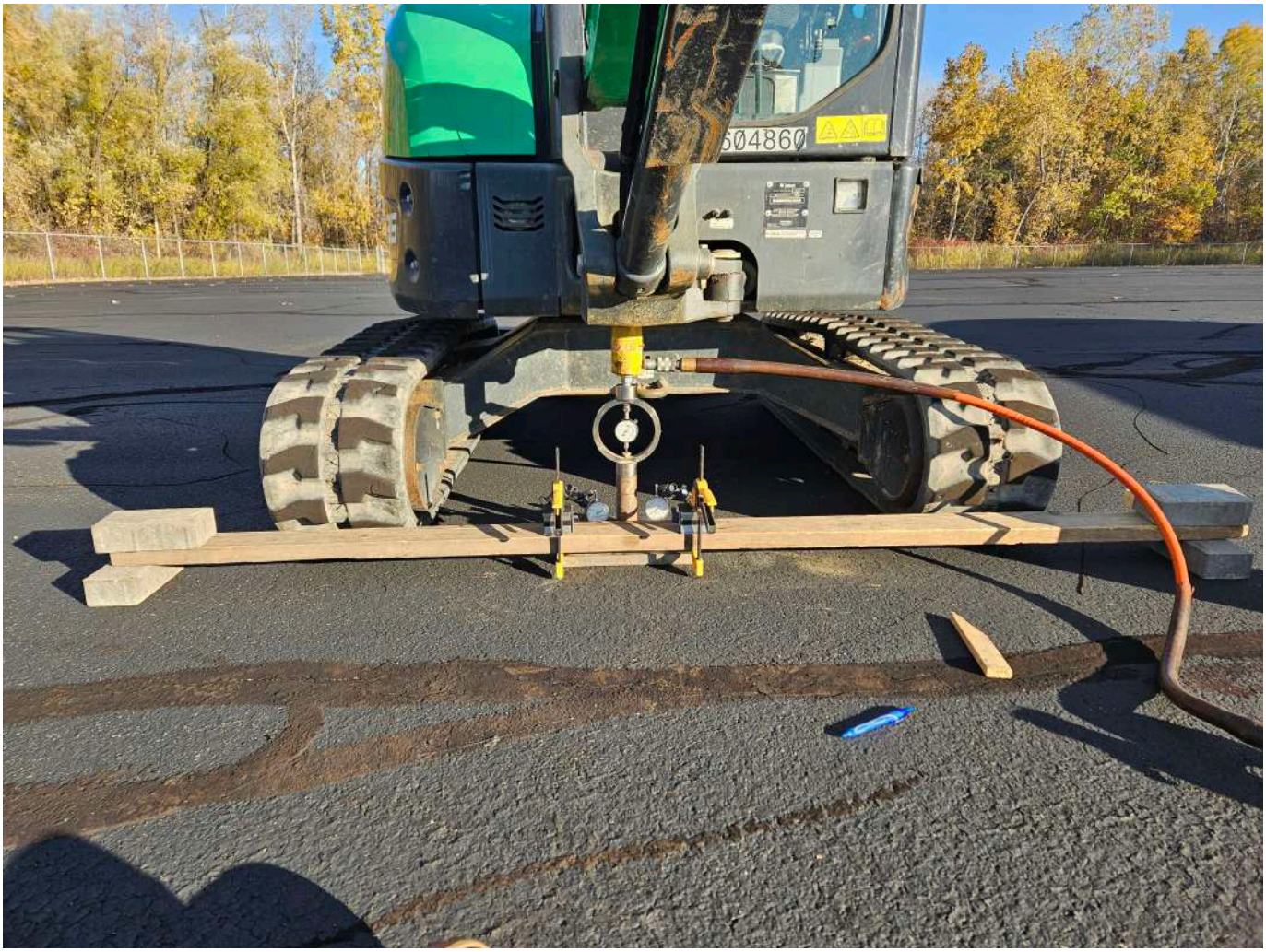


Figure 12: Static Load Test Setup at HWL-3.



Figure 13: Bearing surface after completion of the Static Load Test at HWL-3.



Supplemental Photo 1: Rig Setup at test location HWL-3. Setup typical of all test locations on asphalt landfill.

APPENDIX K
ASPHALT LANDFILL AS-BUILT DRAWINGS

Remedial Program
Inactive Landfill Site Closure

prepared for

Xerox Corporation

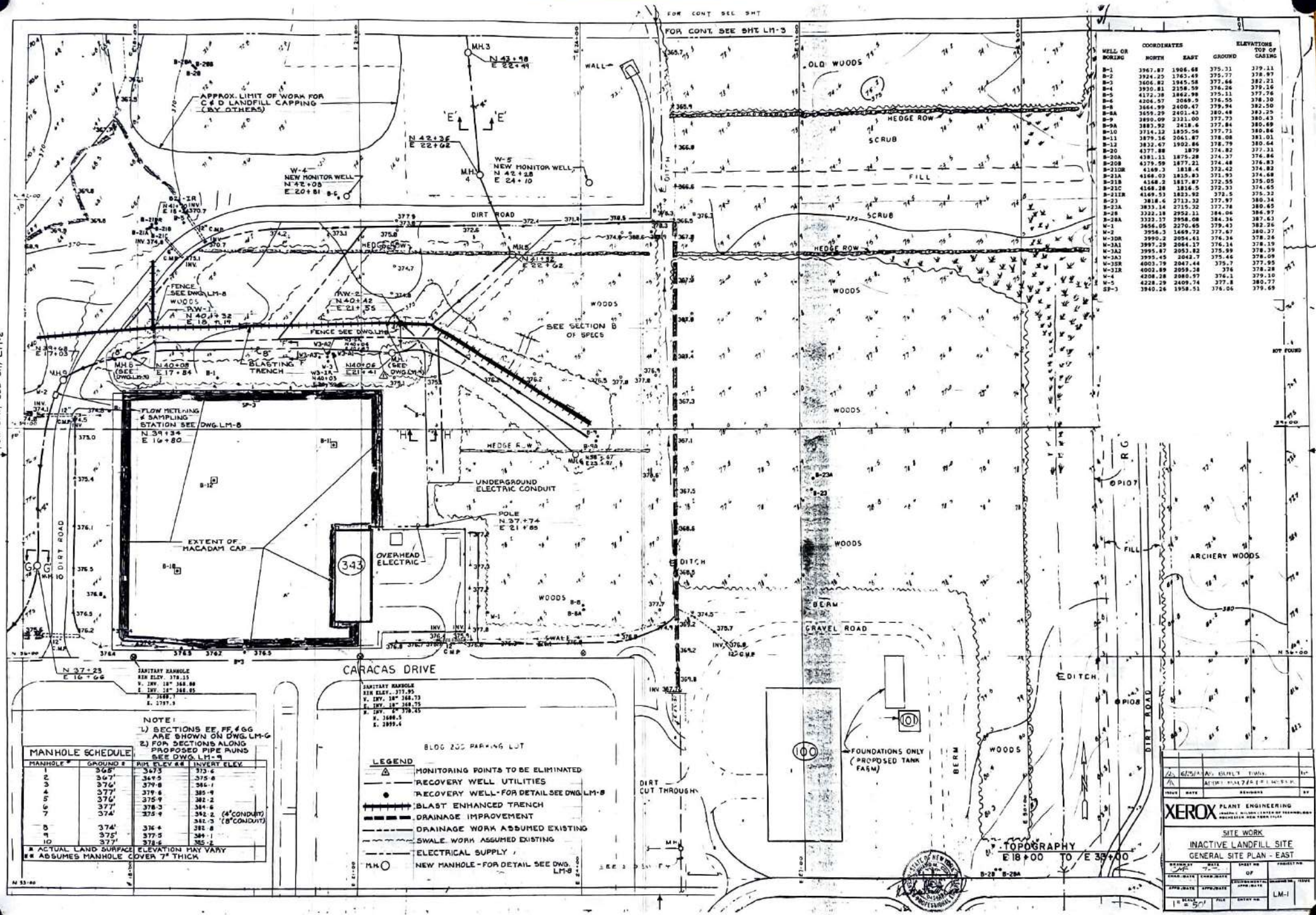
Webster, New York

As-Built Set

LIST OF DRAWINGS

Drawing Number:	Title:
LM-1	GENERAL SITE PLAN - EAST
LM-2	GENERAL SITE PLAN - WEST
LM-3	GENERAL SITE PLAN - NORTH
LM-4	CAP LAYOUT
LM-5	CAP DETAILS
LM-6	MISCELLANEOUS SITE DETAILS
LM-7	SEE SECTION B OF SPECIFICATIONS
LM-8	GROUNDWATER RECOVERY SYSTEM DETAILS
LM-9	PROFILES
E-1	ELECTRICAL SITE PLAN AND DETAILS
E-2	ELECTRICAL ONE LINE DIAGRAM

Prepared by:
Woodward-Clyde Consultants



WELL OR BORING	COORDINATES NORTH	EAST	ELEVATIONS GROUND	TOP OF CASING
B-1	3967.87	1906.68	375.31	379.11
B-2	3924.25	1763.49	375.77	378.97
B-3	3606.82	1945.58	377.46	381.21
B-4	3930.81	2158.59	376.26	379.16
B-5	4172.38	1862.98	375.11	377.76
B-6	4204.57	2069.9	376.55	378.30
B-8	3664.99	2400.67	379.54	382.50
B-8A	3659.29	2401.43	380.48	383.25
B-9	3890.09	2321.00	377.73	380.43
B-9A	3883.92	2418.6	377.84	380.69
B-10	3714.12	1855.56	377.71	380.86
B-11	3879.16	2061.87	378.08	381.01
B-12	3832.67	1902.86	378.79	380.54
B-20	4377.88	1879	374.82	377.33
B-20A	4381.11	1875.28	374.37	376.86
B-20B	4379.59	1877.21	374.48	376.83
B-21BR	4169.3	1818.4	372.42	374.82
B-21A	4168.03	1815.83	371.93	374.68
B-21B	4168.2	1817.87	372.55	375.05
B-21C	4168.28	1816.5	372.33	374.65
B-21BR	4169.51	1823.92	372.5	375.32
B-23	3818.6	2713.32	377.97	380.34
B-23A	3833.34	2715.32	377.74	380.65
B-28	3322.18	2952.11	384.06	386.97
B-28A	3323.17	2958.08	384.36	387.63
W-1	3656.05	2270.65	379.43	382.26
W-2	3956.3	1669.72	377.67	380.37
W-3DR	3990.2	2054.61	376.34	378.26
W-3A1	3997.29	2064.17	376.14	378.19
W-3A2	3995.87	2053.82	375.99	378.39
W-3A3	3995.45	2042.7	375.46	378.09
W-3SR	4003.79	2047.44	375.7	377.95
W-3IR	4002.89	2059.38	376	378.28
W-4	4208.28	2080.97	376.1	379.10
W-5	4228.29	2409.74	377.8	380.77
SP-3	3940.26	1958.51	376.06	379.69

NOTE:
1) SECTIONS EE, FF, & GG ARE SHOWN ON DWG. LM-6
2) FOR SECTIONS ALONG PROPOSED PIPE RUNS SEE DWG. LM-9

MANHOLE SCHEDULE

MANHOLE #	GROUND #	RIM ELEV. ##	INVERT ELEV.	
1	368'	373.5	373.6	
2	367'	369.5	375.6	
3	376'	379.8	386.1	
4	377'	379.6	385.9	
5	376'	375.9	382.2	
6	377'	378.3	384.6	
7	374'	375.9	382.2	(4"CONDUIT)
			382.3	(8"CONDUIT)
8	374'	376.4	382.8	
9	375'	377.5	384.1	
10	377'	378.5	385.2	

ACTUAL LAND SURFACE ELEVATION MAY VARY
ASSUMES MANHOLE COVER 7" THICK

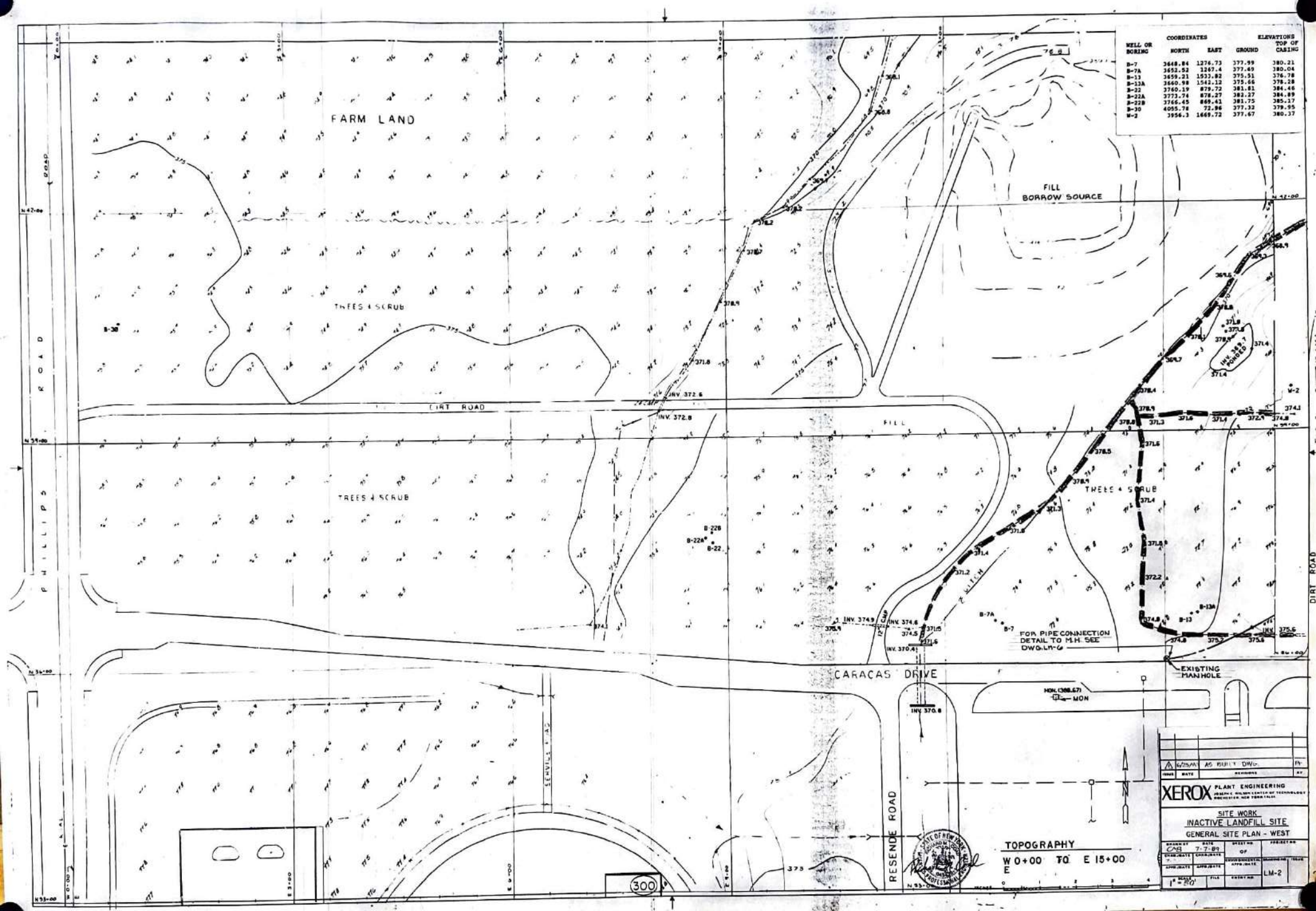
LEGEND
- MONITORING POINTS TO BE ELIMINATED
- RECOVERY WELL UTILITIES
- RECOVERY WELL - FOR DETAIL SEE DWG. LM-8
- BLAST ENHANCED TRENCH
- DRAINAGE IMPROVEMENT
- DRAINAGE WORK ASSUMED EXISTING
- SWALE. WORK ASSUMED EXISTING
- ELECTRICAL SUPPLY
- MH NEW MANHOLE - FOR DETAIL SEE DWG. LM-8

6/25/81 AS BUILT THRU
ISSUE DATE REVISIONS BY

XEROX PLANT ENGINEERING
ROCHESTER, NEW YORK 14626

SITE WORK
INACTIVE LANDFILL SITE
GENERAL SITE PLAN - EAST

DRAWN BY MP DATE 7-81 SHEET NO OF PROJECT NO
CHKD. DATE CHKD. DATE ENVIRONMENTAL APPR. DATE DRAWING NO. LM-1 ISSUE
APPR. DATE APPR. DATE
1" = 50' FILE ENTRY NO.



WELL OR BORING	COORDINATES NORTH	COORDINATES EAST	ELEVATIONS GROUND	ELEVATIONS TOP OF CASING
B-7	3648.84	1276.73	377.99	380.21
B-7A	3652.52	1347.4	377.69	380.04
B-13	3659.21	1533.82	375.51	376.78
B-13A	3660.98	1542.12	375.66	378.28
B-22	3760.19	879.72	381.81	384.46
B-22A	3773.74	878.27	382.27	384.89
B-22B	3766.45	869.41	381.75	385.17
B-36	4055.78	72.96	377.32	379.95
W-2	3956.3	1669.72	377.67	380.37

TOPOGRAPHY

W 0+00 TO E 15+00

4/25/89	AS BUILT DWG.		
ISSUE DATE	REVISIONS		BY

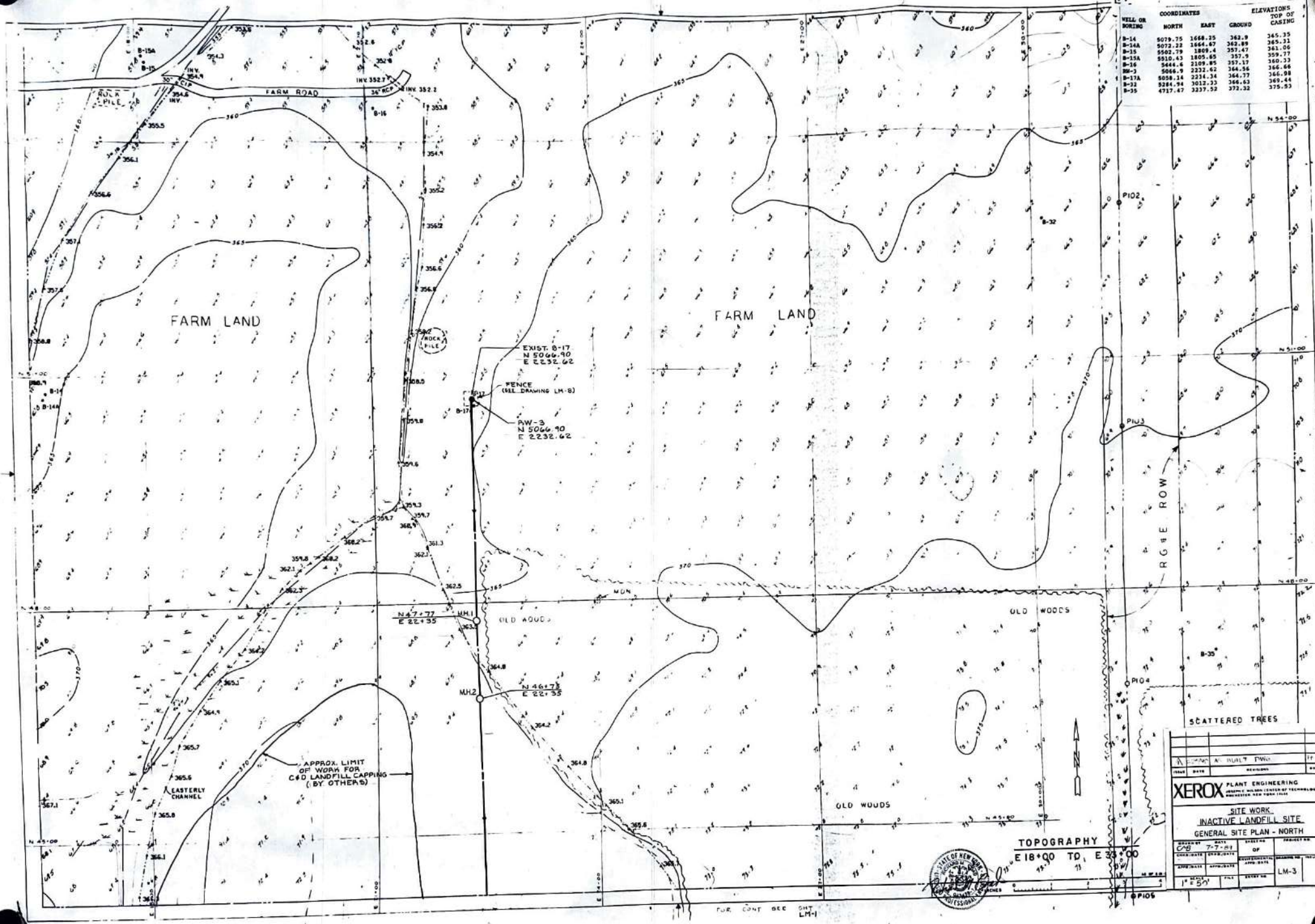
XEROX PLANT ENGINEERING

SITE WORK

INACTIVE LANDFILL SITE

GENERAL SITE PLAN - WEST

DRAWN BY CMB	DATE 7-7-89	SHEET NO. OF	PROJECT NO.
CHKD./DATE	CHKD./DATE	COVER SHEET NO. APPD./DATE	DRAWING NO. LM-2
APPD./DATE	APPD./DATE		
SCALE 1" = 50'	FILE	EVENT NO.	



WELL OR BORING	COORDINATES NORTH	EAST	ELEVATIONS GROUND	TOP OF CASING
B-14	5079.75	1668.25	362.9	365.35
B-14A	5072.22	1664.67	362.89	365.31
B-15	5502.79	1809.4	357.47	361.06
B-15A	5510.43	1805.65	357.9	359.77
B-16	5444.6	2109.85	357.17	360.33
PW-3	5066.9	2232.62	364.56	366.66
B-17A	5058.34	2234.34	364.77	366.98
B-32	5284.94	3012.33	366.63	369.44
B-35	4717.47	3237.52	372.32	375.53

AS BUILT DWG.

ISSUE	DATE	REVISIONS	BY

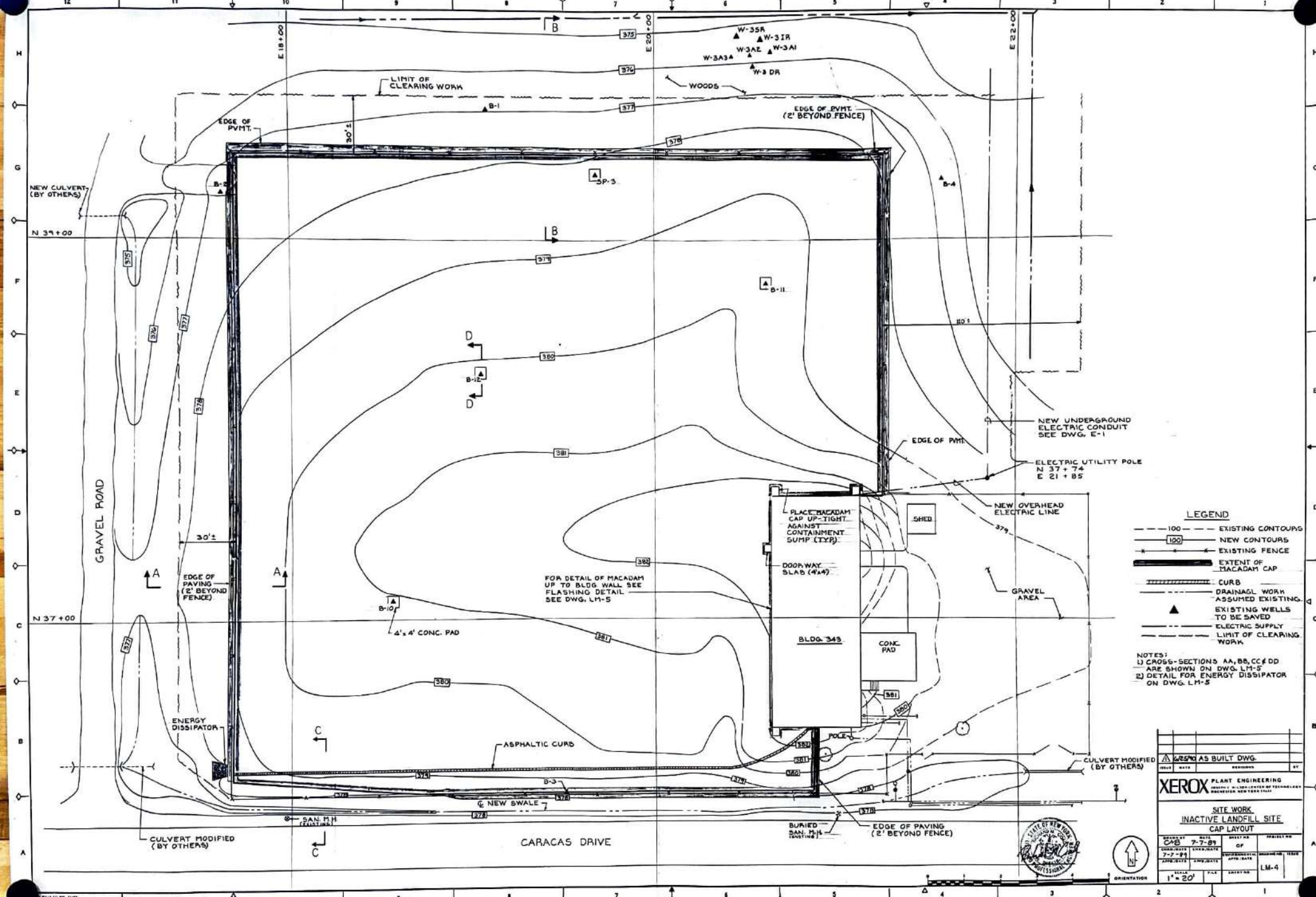
XEROX PLANT ENGINEERING
JOSEPH C. WILSON CENTER OF TECHNOLOGY
ROCHESTER NEW YORK 14644

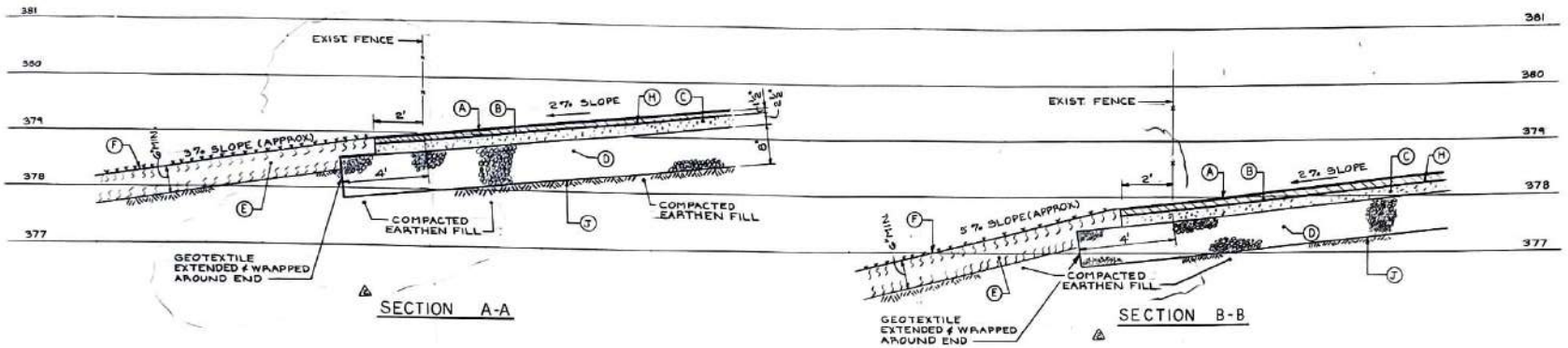
SITE WORK
INACTIVE LANDFILL SITE
GENERAL SITE PLAN - NORTH

DRAWN BY	DATE	SHEET NO.	PROJECT NO.
CNP	7-7-81	OF	
CHKD/DATE	ENGR./DATE	ENGINEERING APP. DATE	DRAWING NO. / ISSUE
APP./DATE	APPR./DATE		LM-3
SCALE 1" = 50'	FILE	ENTRY NO.	

TOPOGRAPHY
E 18+00 TO E 33+00

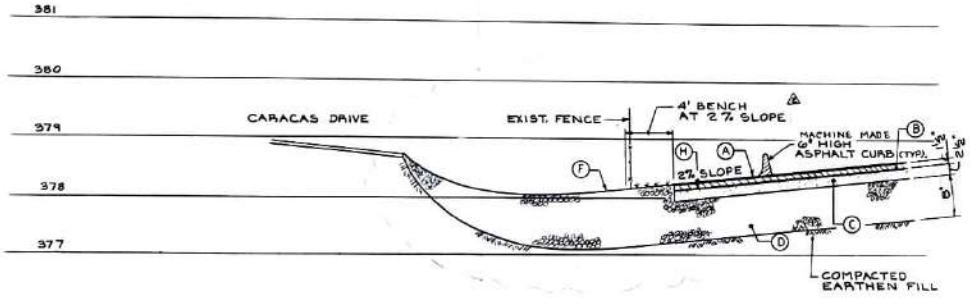
STATE OF NEW YORK
RICHARD M. PRATT
LICENSED PROFESSIONAL ENGINEER


E 18+00
E 20+00
E 22+00
N 39+00
N 37+00
LIMIT OF CLEARING WORK
WOODS
EDGE OF PVMT.
EDGE OF PVMT. (2' BEYOND FENCE)
30'±
NEW CULVERT (BY OTHERS)
GRAVEL ROAD
EDGE OF PAVING (2' BEYOND FENCE)
B-1
B-2
B-3
B-4
B-10
B-11
B-12
SP-3
W-3SA
W-3IR
W-3AZ
W-3AI
W-3A3A
W-3DR
NEW UNDERGROUND ELECTRIC CONDUIT SEE DWG. E-1
ELECTRIC UTILITY POLE N 37 + 74 E 21 + 85
NEW OVERHEAD ELECTRIC LINE
PLACE MACADAM CAP UP-TIGHT AGAINST CONTAINMENT SUMP (TYP.)
SHED
DOORWAY SLAB (4'x4')
FOR DETAIL OF MACADAM UP TO BLDG. WALL SEE FLASHING DETAIL SEE DWG. LM-5
GRAVEL AREA
4'x 4' CONC. PAD
BLDG. 343
CONC. PAD
POLE
ENERGY DISSIPATOR
ASPHALTIC CURB
℄ NEW SWALE
SAN. M.H (EXISTING)
BURIED SAN. M.H (EXISTING)
CULVERT MODIFIED (BY OTHERS)
CARACAS DRIVE
LEGEND
EXISTING CONTOURS
NEW CONTOURS
EXISTING FENCE
EXTENT OF MACADAM CAP
CURB
DRAINAGE WORK ASSUMED EXISTING
EXISTING WELLS TO BE SAVED
ELECTRIC SUPPLY
LIMIT OF CLEARING WORK
NOTES:
1) CROSS-SECTIONS AA, BB, CC & DD ARE SHOWN ON DWG. LM-5
2) DETAIL FOR ENERGY DISSIPATOR ON DWG. LM-5
6/25/90 AS BUILT DWG.
XEROX PLANT ENGINEERING
SITE WORK
INACTIVE LANDFILL SITE
CAP LAYOUT
CMB 7-7-89
7-7-89
1" = 20'
LM-4



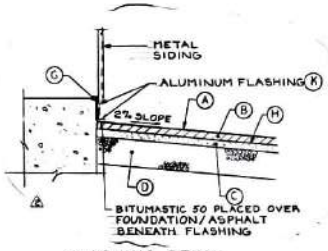
SECTION A-A

SECTION B-B



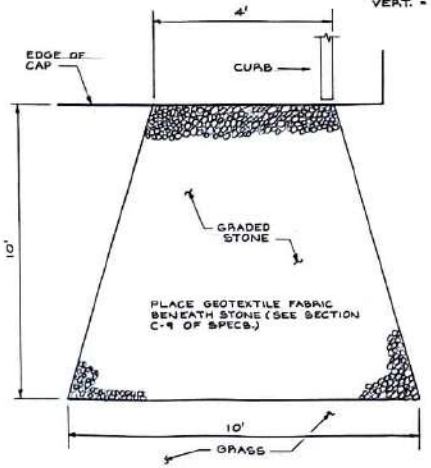
SECTION C-C

SCALE:
HORIZ. = 1" = 5'
VERT. = 1" = 1'



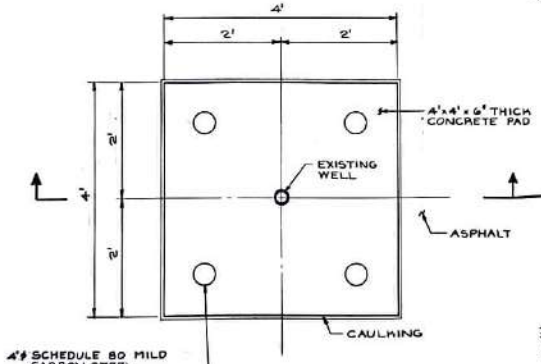
FLASHING DETAIL

N.T.S.

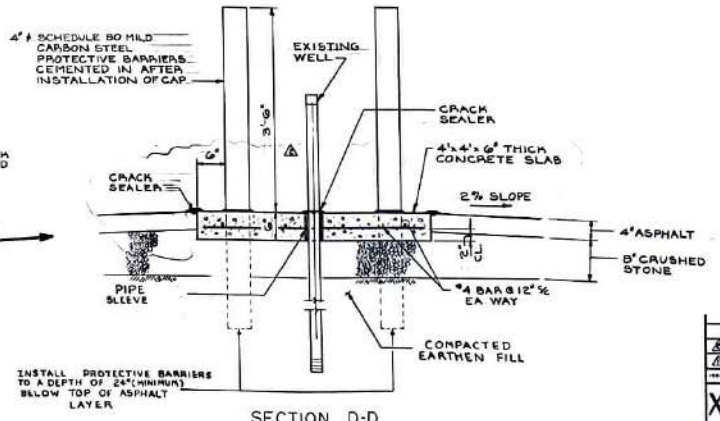


ENERGY DISSIPATOR DETAIL

N.T.S.



PLAN



SECTION D-D

MONITORING WELL CAP CONNECTION

SCALE: 1" = 1'

LEGEND

- (A) SPRAY SEALANT APPLICATION (ASPHALT CEMENT AC-10 / AC-20) AT RATE OF 0.20 GALLONS PER SQUARE YARD RESIDUAL ASPHALT. MANUAL APPLICATION AT INCREASED RATE AROUND PERIMETER OF CAP TO SEAL FENCE POSTS AND WELL CAP CONNECTIONS.
- (B) NYSDOT TYPE 7F MARSHALL TOP (MODIFIED) BITUMINOUS PAVEMENT MIXTURE (SEE NYSDOT TABLE 401-1)
- (C) NYSDOT TYPE 3 MIXTURE (TABLE 401-1) COMPACTED TO AT LEAST 90% OF THEORETICAL MAXIMUM OBTAINABLE DENSITY.
- (D) MINIMUM 6 - 8 INCHES, STABILIZED SUBBASE MATERIAL NYSDOT TYPE 1 SUBBASE COURSE SECTION 304-2.02
- (E) MINIMUM 6 INCHES, TOPSOIL STRIPPED FROM THE SITE CONFORMING TO NYS DOT SECTION 713-01
- (F) SEEDING IS TO CONFORM TO NYSDOT SECTION 713-04
- (G) ALUMINUM FLASHING IN CONTACT WITH CONCRETE TO BE PROTECTED WITH BITUMASTIC 50 (KOPPERS CO.) OR APPROVED SUBSTITUTE. PLACE FLASHING EITHER FLUSH WITH UNDERSIDE OF WALL OR BETWEEN WALL AND EXISTING METAL SIDING.
- (H) TACK COAT APPLICATION (IN CONFORMANCE WITH NYSDOT SECTION 407) AT A RATE OF 0.03 TO 0.07 GALLONS PER SQUARE YARD.
- (J) GEOTEXTILE (TREVIRA HMS) AS MANUFACTURED BY HOECHST CELANESE (SEE SECTION C-9.0 IN SPECIFICATIONS).
- (K) ALUMINUM FLASHING TO CONFORM WITH SHEET METAL AND AIR CONDITIONING NATIONAL ASSOCIATION (SMACNA) GUIDELINES. ALUMINUM FLASHING TO BE FASTENED TO CONCRETE WALL USING APPROPRIATE RAMSET TOOL OR EQUIVALENT.

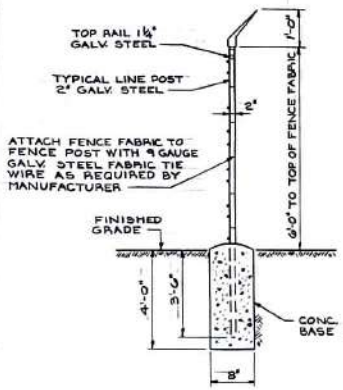
4	6/25/91	AS BUILT DWG. (CHANGES)	
3	8-16-89	CHANGE ALUMINUM FLASHING DETAIL	PDT
ISSUE	DATE	REVISIONS	BY

XEROX PLANT ENGINEERING
WEBSTER, NEW YORK 14580

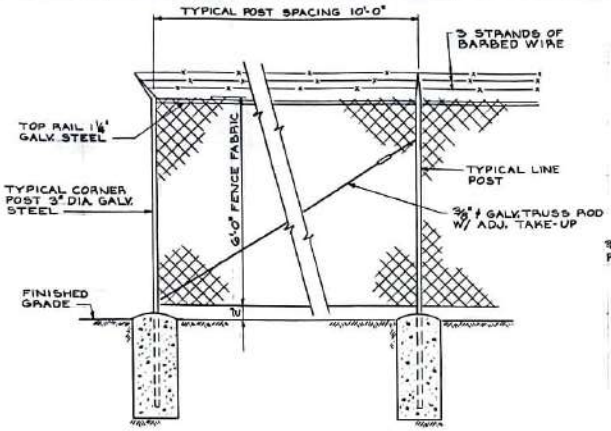
SITE WORK
INACTIVE LANDFILL SITE
CAP DETAILS

DESIGNED CHS	DATE 7-7-89	SHEET NO. OF	PROJECT NO.
DRAWN DATE 7-7-89	CHKD. DATE	SUPERVISION APPR. DATE	DRAWING NO. LM-5
APPR. DATE	APPR. DATE	SPEC. NO.	ISSUE
SCALE	FILE		

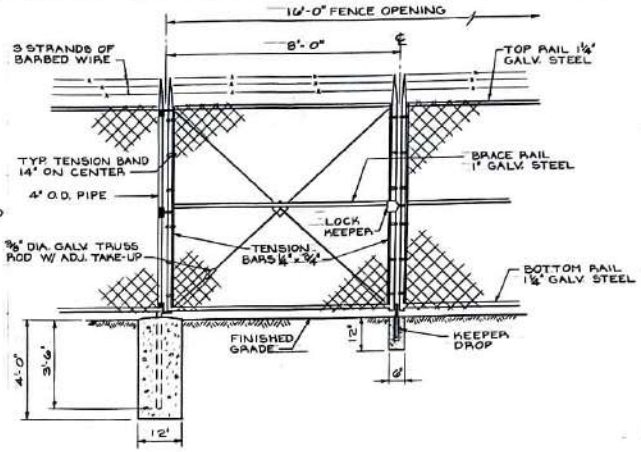
ORIENTATION



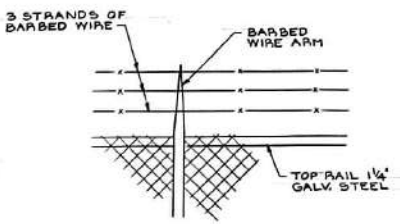
LINE POST DETAIL
N.T.S.



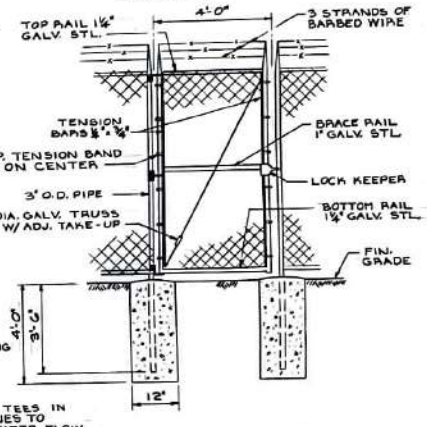
TYPICAL FENCING DETAIL
SCALE 1 1"=2'



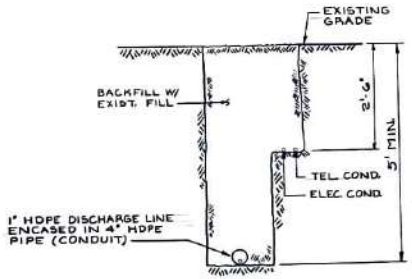
DOUBLE GATE DETAIL
N.T.S.



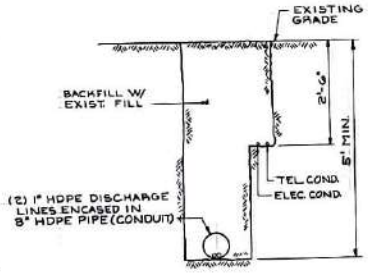
LINE POST TOP
SCALE : 1"= 1'



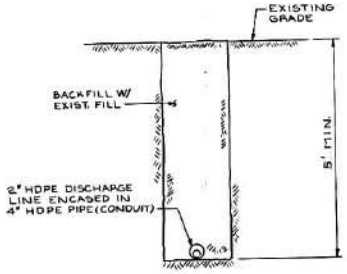
SINGLE GATE DETAIL
N.T.S.



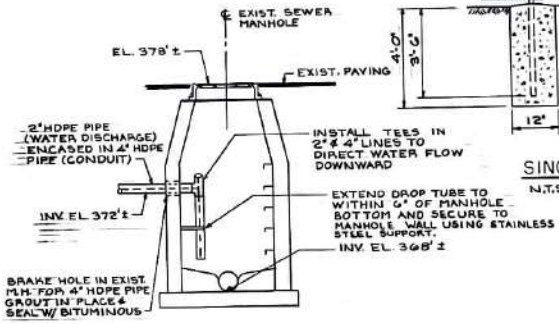
SECTION E-E
N.T.S.



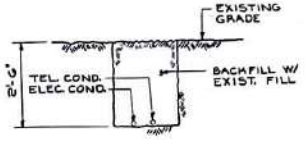
SECTION F-F
N.T.S.



SECTION G-G
N.T.S.



NEW PIPE TO EXISTING MANHOLE CONNECTION DETAIL
N.T.S.



SECTION H-H
N.T.S.

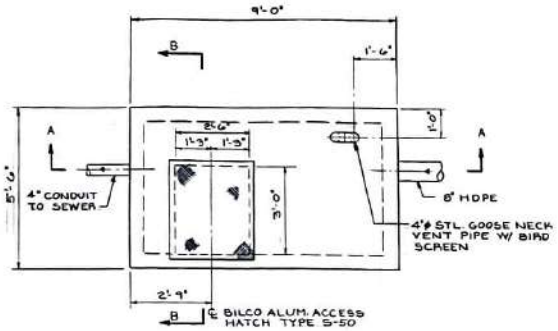
3	GRS/RG	AS BUILT DWG.	RF
2	8-16-89	REVISED PIPE CONNECTION DETAIL IN EXISTING MANHOLE	CAB
ISSUE	DATE	REVISIONS	BY

XEROX PLANT ENGINEERING
ROCHESTER, NEW YORK 14644

SITE WORK
INACTIVE LANDFILL SITE
MISCELLANEOUS SITE DETAILS

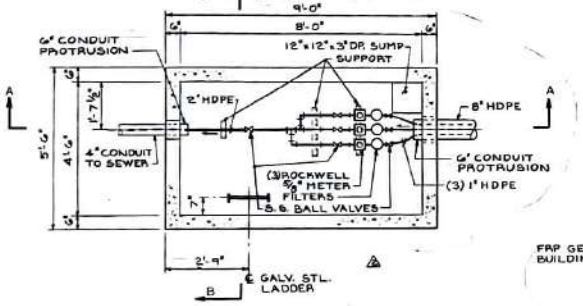
DRAWN BY CAB	DATE 7-7-89	SHEET NO. OF	PROJECT NO.
CHKD. BY/DATE 7-7-89	CHKD. DATE	ENVIRONMENTAL APPR./DATE	DRAWING NO. / ISSUE LM-6
APPR./DATE	APPR./DATE		
SCALE	FILE	ENTER NO.	

ORIENTATION



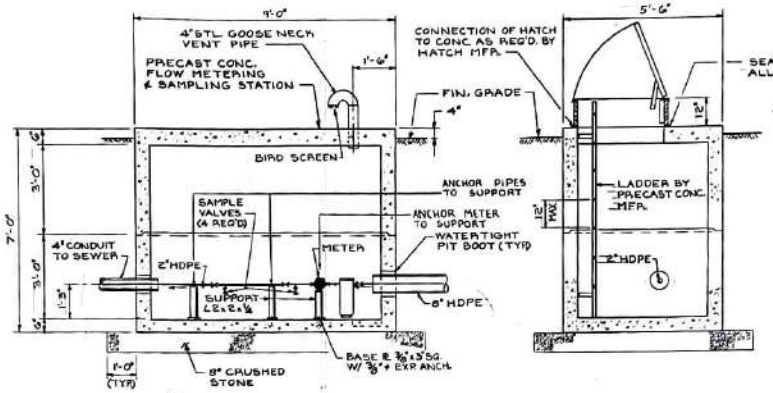
TOP PLAN

½" = 1'-0"



SECTIONAL PLAN

½" = 1'-0"

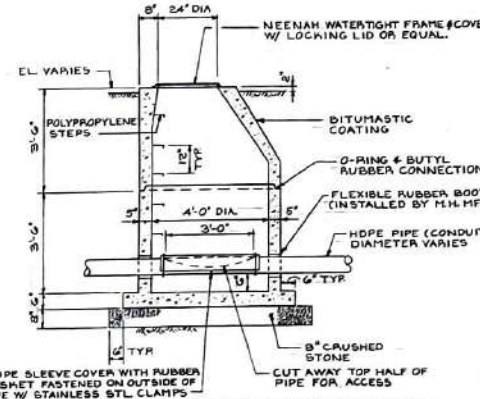


SECTION A-A

½" = 1'-0"

SECTION B-B

½" = 1'-0"

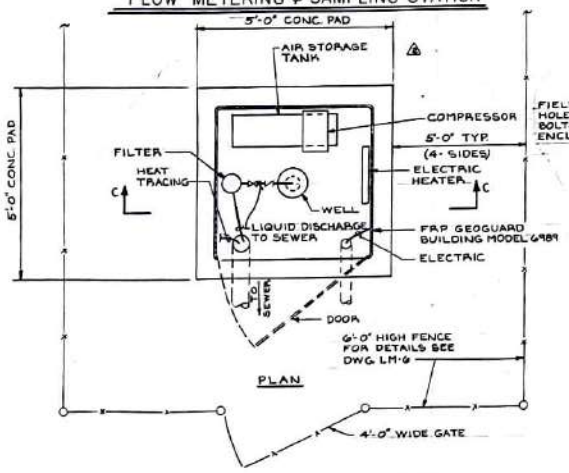


PRECAST CONCRETE MANHOLE DETAIL

½" = 1'-0"

NOTES:
1. LOCATION OF TOP SECTION OF MANHOLE & STEPS TO BE 90° TO PIPE RUN
2. CONTRACTOR TO INSTALL 8"x8"x4" HDPE TEE'S AS REQUIRED IN MH1 AND MH6
⚠ (SEE DETAIL 1 & 2, DWG. LM-9)

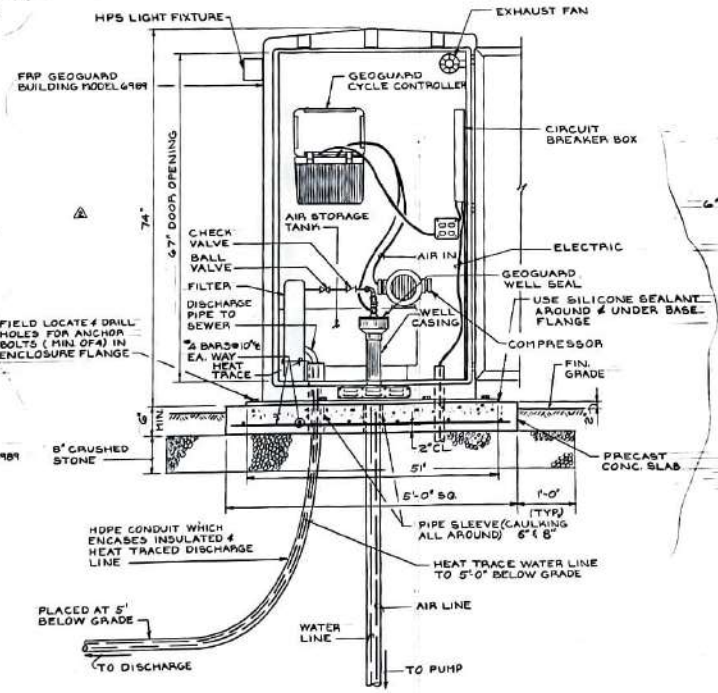
FLOW METERING & SAMPLING STATION



PLAN

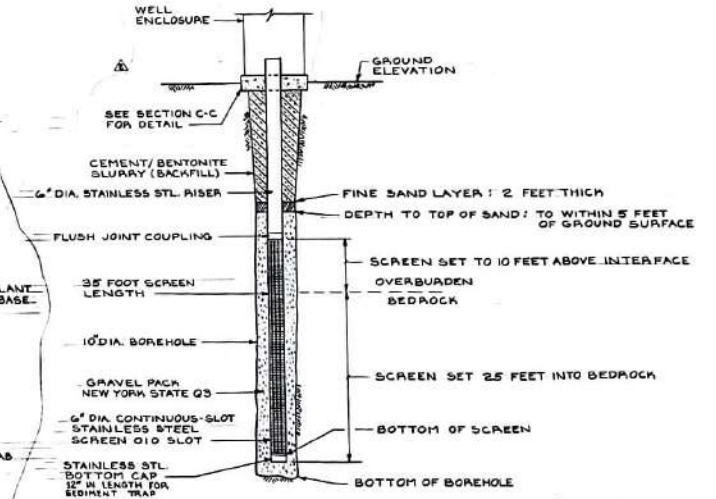
FRP RECOVEREY WELL ENCLOSURE

N.T.S.



SECTION C-C

N.T.S.



RECOVERY WELL DETAIL (RW-1 & RW-2) *

N.T.S.

* EXISTING B-17 WELL TO BE USED FOR RW-3

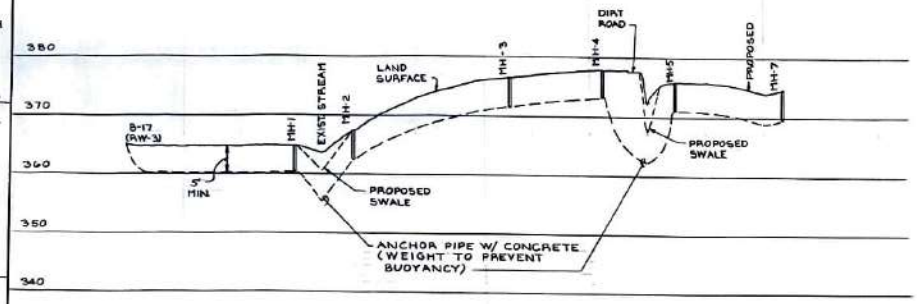
⚠	4/25/89	AS BUILT DWG. (CHANGES)
⚠		ADDED NOTE AT RC, M.H. DET.
ISSUE	DATE	REVISIONS BY

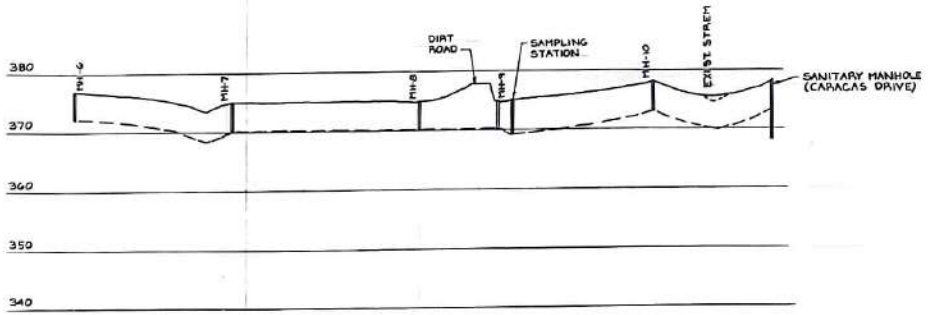
XEROX PLANT ENGINEERING
JOSEPH C. WILSON CENTER OF TECHNOLOGY ROCHESTER, NEW YORK 14644

SITE WORK
INACTIVE LANDFILL SITE
GROUNDWATER RECOVERY SYSTEM DETAILS

DRAWN BY: CMB	DATE: 7-7-89	SHEET NO. OF	PROJECT NO.
CHKD. BY: 7-7-89	CHKD. BY/DATE	ENVIRONMENTAL APPR./DATE	DRAWING NO. LM-8
APPR./DATE	APPR./DATE		ISSUE
SCALE	FILE	ENTRY NO.	

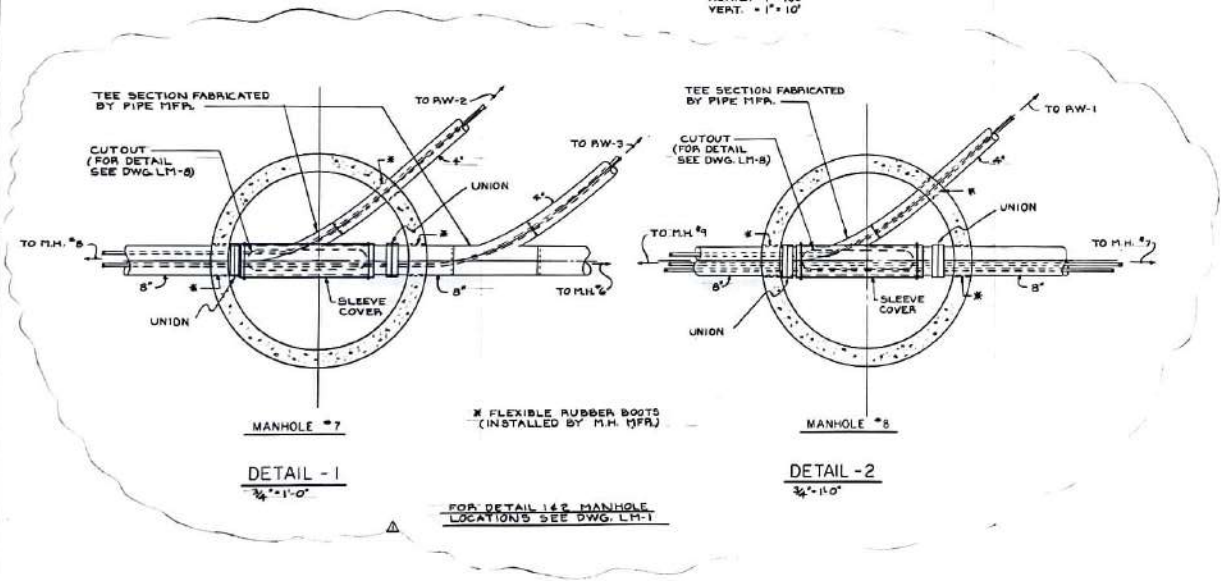
ORIENTATION





SECTIONS ALONG PROPOSED PIPE ROUTES

SCALE:
HORIZ. - 1" = 100'
VERT. = 1" = 10'



* FLEXIBLE RUBBER BOOTS (INSTALLED BY M.H. MFR.)

DETAIL - 1
3/4" = 1'-0"

DETAIL - 2
3/4" = 1'-0"

⚠ FOR DETAIL 1 & 2 MANHOLE LOCATIONS SEE DWG. LM-1

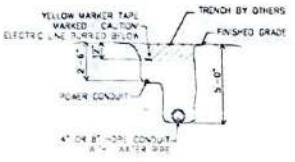
ISSUE	DATE	REVISIONS	BY
⚠	6/25/91	AS BUILT DWG.	A
⚠	8/16/89	ADDED DETAIL 1 & 2	CAB

XEROX PLANT ENGINEERING
WEBSTER, NEW YORK 14580

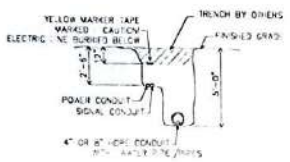
SITE WORK
INACTIVE LANDFILL SITE
PROFILES

DRAWN CAB	DATE 7-7-89	SHEET NO. OF	PROJECT NO.
CHAR. DATE 7-7-89	APPR. DATE	ENVIRONMENTAL APPR. DATE	DRAWING NO. LM-9
APPR. DATE	APPR. DATE		ISSUE
SCALE	FILE	ENTRY NO.	

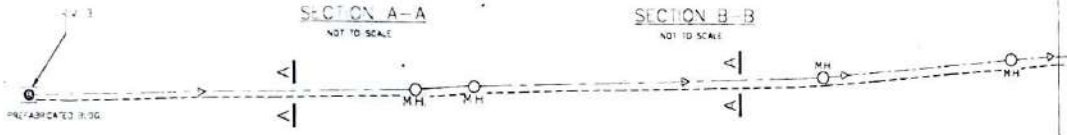
ORIENTATION

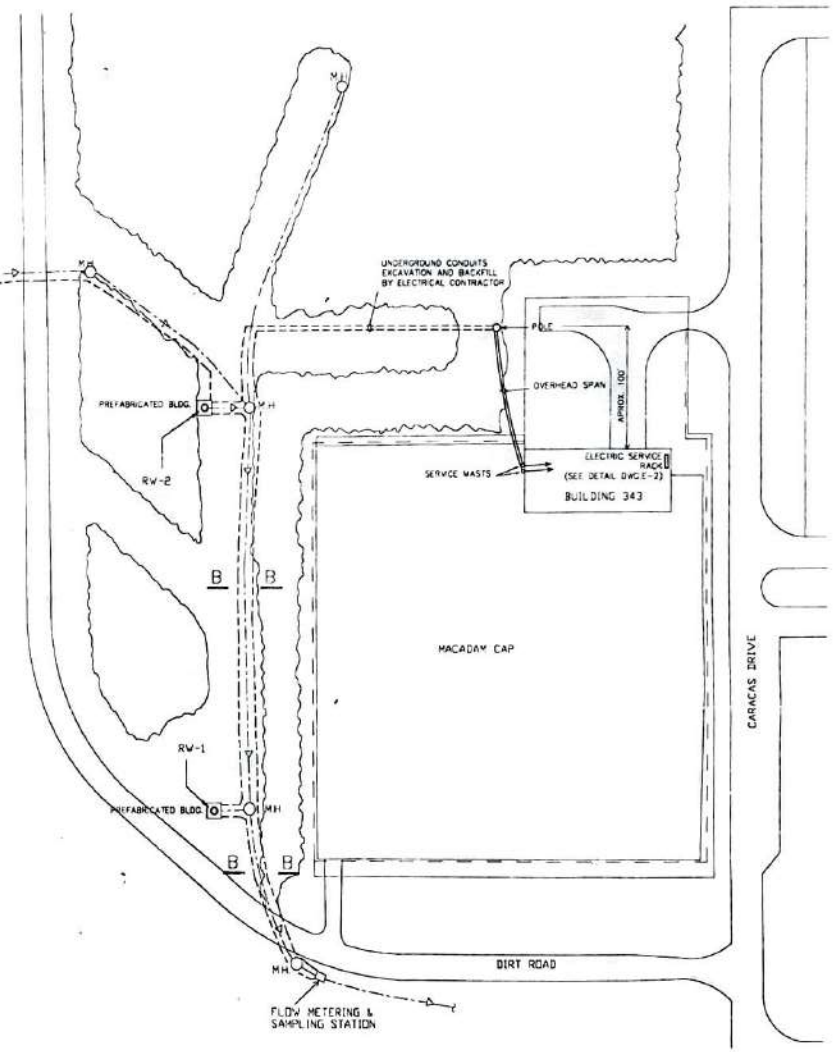


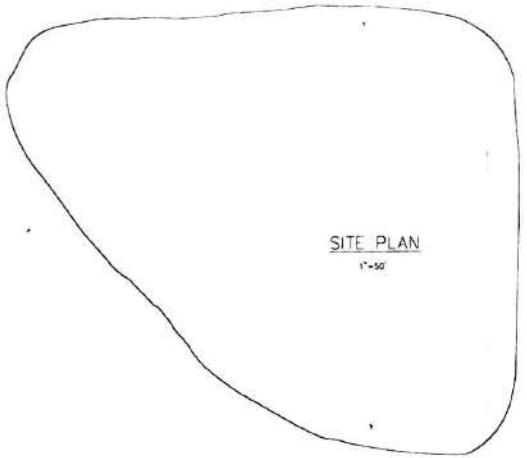
SECTION A–A
NOT TO SCALE

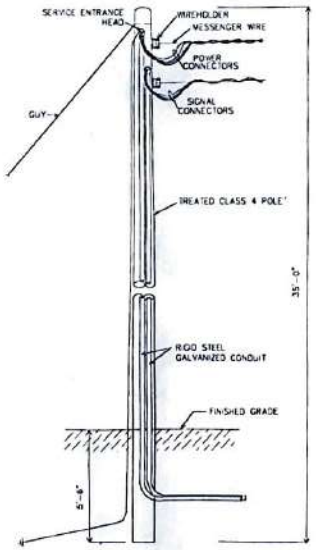


SECTION B–B
NOT TO SCALE

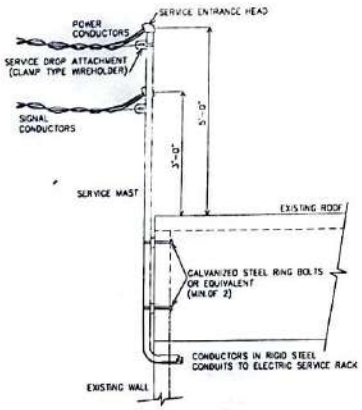




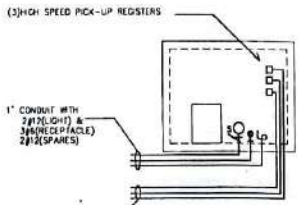




POLE DETAIL
NOT TO SCALE



SERVICE MAST INSTALLATION DETAIL
NOT TO SCALE



FLOW METERING STATION DETAIL
NOT TO SCALE

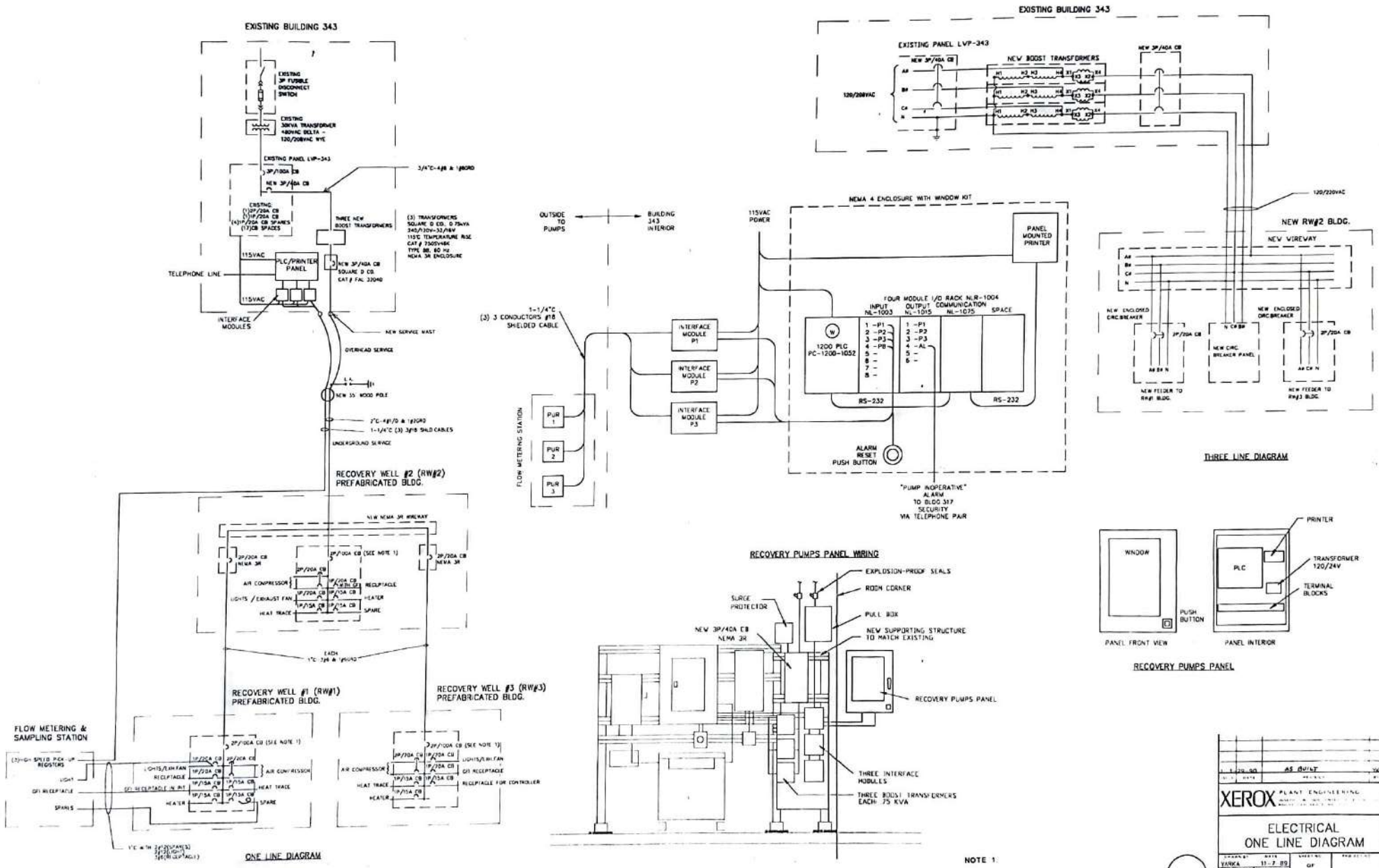
1	1-28-90	AS BUILT	YK
ISSUE	DATE	REVISIONS	BY

XEROX PLANT ENGINEERING

ELECTRICAL
SITE PLAN & DETAILS

YARDOK #	1797-89	SHEET NO.	PROJECT NO.
DRAW DATE	APPR DATE	OF	
APPR DATE	APPR DATE	ENVIRONMENTAL APPR DATE	DRAWING NO. / ISSUE
AS SHOWN	FILE	ENTITY NO.	E-1 1

ORIENTATION


EXISTING BUILDING 343
EXISTING 3P FUSIBLE DISCONNECT SWITCH
EXISTING 30KVA TRANSFORMER 480VAC DELTA - 120/208VAC WYE
EXISTING PANEL LVP-343
NEW 3P/40A CB
THREE NEW BOOST TRANSFORMERS
(3) TRANSFORMERS SQUARE D CO. 0 75kVA 340/330V-32/16V 115°C TEMPERATURE RISE CAT # 750SV48K TYPE BR, 60 Hz NEMA 3R ENCLOSURE
115VAC
PLC/PRINTER PANEL
NEW 3P/40A CB SQUARE D CO. CAT # FAL 32040
TELEPHONE LINE
INTERFACE MODULES
NEW SERVICE MAST
OVERHEAD SERVICE
NEW 35' WOOD POLE
UNDERGROUND SERVICE
RECOVERY WELL #2 (RW#2) PREFABRICATED BLDG.
NEW NEMA 3R WIREWAY
AIR COMPRESSOR
LIGHTS / EXHAUST FAN
HEAT TRACE
RECEPTACLE
HEATER
SPARE
EACH 1"C WITH 2#12(CU)AWG(S) 3#12(CU)AWG(G) 1#8(CU)AWG(G)
RECOVERY WELL #1 (RW#1) PREFABRICATED BLDG.
RECOVERY WELL #3 (RW#3) PREFABRICATED BLDG.
FLOW METERING & SAMPLING STATION
(2)HIGH SPEED PICK UP REGISTERS
LIGHT
GFI RECEPTACLE
SPARES
GFI RECEPTACLE IN PIT
RECEPTACLE FOR CONTROLLER
ONE LINE DIAGRAM
EXISTING BUILDING 343
EXISTING PANEL LVP-343
NEW BOOST TRANSFORMERS
120/208VAC
NEW RW#2 BLDG.
NEW WIREWAY
NEW ENCLOSED CIRC.BREAKER
NEW CIRC BREAKER PANEL
NEW FEEDER TO RW#1 BLDG.
NEW FEEDER TO RW#3 BLDG.
THREE LINE DIAGRAM
OUTSIDE TO PUMPS
BUILDING 343 INTERIOR
NEMA 4 ENCLOSURE WITH WINDOW KIT
115VAC POWER
PANEL MOUNTED PRINTER
1-1/4"C (3) 3 CONDUCTORS #18 SHIELDED CABLE
FLOW METERING STATION
PUR 1
PUR 2
PUR 3
INTERFACE MODULE P1
INTERFACE MODULE P2
INTERFACE MODULE P3
FOUR MODULE I/O RACK NLR-1004
INPUT NL-1003
OUTPUT NL-1015
COMMUNICATION NL-1075
SPACE
1200 PLC PC-1200-1052
RS-232
ALARM RESET PUSH BUTTON
"PUMP INOPERATIVE" ALARM TO BLDG 317 SECURITY VIA TELEPHONE PAIR
RECOVERY PUMPS PANEL WIRING
EXPLOSION-PROOF SEALS
ROOM CORNER
PULL BOX
NEW SUPPORTING STRUCTURE TO MATCH EXISTING
SURGE PROTECTOR
NEW 3P/40A CB NEMA 3R
RECOVERY PUMPS PANEL
THREE INTERFACE MODULES
THREE BOOST TRANSFORMERS EACH: 75 KVA
MODIFICATION OF EXIST.ELECT. SERVICE RACK
NOT TO SCALE
WINDOW
PUSH BUTTON
PANEL FRONT VIEW
PLC
PRINTER
TRANSFORMER 120/24V
TERMINAL BLOCKS
PANEL INTERIOR
RECOVERY PUMPS PANEL
NOTE 1:
100 A CB SERVES AS MAIN DISCONNECT ONLY
AS BUILT
XEROX PLANT ENGINEERING
ELECTRICAL ONE LINE DIAGRAM
VANKA
11-7-89
AS SHOWN
E-2 1

APPENDIX L
LANDFILL CAP MONITORING, MAINTENANCE, AND CONTINGENCY PLAN REQUIREMENTS

H & A OF NEW YORK ENGINEERING
AND GEOLOGY, LLP
260 E Main St, Ste 2100
Rochester, NY 14604
585.359.9000

MEMORANDUM

Revised February 25, 2026
February 18, 2026
File No. 0134970

TO: Kevin Chazhikattu, Assistant Engineer
Lynn Winterberger, Supervisor, HW Corrective Action and Post Closure
New York State Department of Environmental Conservation, Division of Materials Management, Bureau of Hazardous Waste and Radiation Management
625 Broadway, Albany, NY

FROM: Janice Szucs, Associate, H & A of New York Engineering and Geology, LLP
Thomas Robitaille, Project Manager, H & A of New York Engineering and Geology, LLP

C: Marcus Lathrop, Xerox Corporation
Katie Socia, Montante Solar

SUBJECT: Landfill Cap Monitoring, Maintenance, and Contingency Plan Requirements

On behalf of Xerox Corporation (Xerox), H & A of New York Engineering and Geology, LLP (Haley & Aldrich of New York) has prepared this memorandum to support the New York State Department of Environmental Conservation (NYSDEC) review of the Change of Use application for the proposed commercial-scale photovoltaic (PV) generation facility (Project) within portions of tax parcel (065.02-1-40.111) of the Xerox Webster Facility. This memorandum outlines the post-closure monitoring and maintenance, notification and contingency operations to be conducted to satisfy the Contingency Plan requirements outlined in DMM-4/Photovoltaic Solar Projects at Closed Solid Waste Landfills.

The proposed solar development area encompasses two separate landfill areas within the Xerox Webster Facility including an approximate 2.8-acre asphalt-capped inactive landfill with associated monitoring and remedial infrastructure and a separate approximate 1.5-acre soil-capped closed construction and demolition (C&D) Landfill. An approximate 20-acre site (encompassing both landfill areas) will be leased by Montante for the purposes of the construction and operation of the solar Project. The Project construction start date is planned for Spring 2026 (pending all applicable approvals) with site restoration and stabilization to follow during the same construction season.

The proposed development areas will be located within the larger Xerox Webster Facility, which is a NYSDEC RCRA Facility (Site Code 828178) for identified groundwater and soil contamination currently managed under a RCRA Part 373 Hazardous Waste Management Permit (No. 8-2654-00064/00040). The Xerox Webster Facility, including the proposed development areas, are subject to a Site Management Plan (SMP), which is currently undergoing revisions. The monitoring, maintenance, and emergency

notifications requirements for the two landfills to be included in the revised SMP are described below. These requirements are subject to change based on the ongoing SMP revisions and any future change to the SMP, subject to NYSDEC approval.

LANDFILL COVER SYSTEM

Specific design elements were incorporated into the construction of both landfill cover systems to minimize the potential for damage relating to environmental exposure or erosion. Ongoing inspection, maintenance, and repair activities further protect the integrity of the cover systems. Details of these elements are provided below.

- **Asphalt-Capped Inactive Landfill:** A cap system was installed over the entire surface of the inactive landfill and is comprised of an existing 4-inch thick asphalt cap at the surface (in addition to underlying cap elements – 6 to 8 inches of aggregate subbase material underlain by geotextile fabric) to minimize infiltration of surface water and to prevent exposure to contaminated fill. This existing asphalt cap will be further supplemented with an additional 1.5 inches of asphalt to be laid on top of the existing surface prior to the construction of solar facilities. Drainage ditches were installed around the surface of the landfill to convey surface water away from the inactive landfill area. A security fence was installed around the cap to prevent unauthorized access and will be modified and expanded to encompass the solar Project.
- **C&D Landfill Area:** The C&D Landfill area has been capped with a minimum of 18-inches of low permeability soil cover and further topped with a minimum of 6-inches of topsoil. The side slopes of the landfill have been graded to a maximum of 2/1 with a 3% minimum grade at the landfill center to minimize infiltration of surface water and direct surface water off the cap. The C&D Landfill closure construction also included two 6-inch PVC landfill gas vents.

Annual Landfill Cap Inspections: Xerox will conduct an annual inspection of the Asphalt-Capped Inactive Landfill and C&D Landfill to assess the structural integrity of the cover systems. The proposed inspection and maintenance activities are outlined in Table 1. An example of the Xerox site-wide inspection form which includes review of the landfill cap/cover systems is included as Appendix A. Separate inspections will also be completed by Montante as part of the post-construction monitoring of the Project (as is noted in Montante's Project Operations and Maintenance [O&M] Plan). Replacement, repairs, and corrective maintenance to the cover systems will be performed by Montante and/or Xerox (with approvals as necessary from the NYSDEC) to maintain the structural integrity of the cover systems. The results of the inspections, maintenance, and repairs will be included in the Xerox Webster Facility Annual Report.

Table 1. Proposed Xerox Responsibilities: Annual Inspection and Maintenance Activities for the Landfill Cover Systems

Asphalt-Capped Inactive Landfill Area	C&D Landfill Area
• Asphalt Cap: The asphalt cap will be inspected for cracking, holes, depressions and ponding, and vegetation penetrating through the cap or encroaching onto the cap from the	• Soil Cap and Vegetation: The soil cap will be inspected for holes, depressions, and ponding, and the surface will be inspected for overgrown vegetation. Repairs and maintenance will be

Asphalt-Capped Inactive Landfill Area	C&D Landfill Area
perimeter. Repairs to the cap will be conducted as needed. • Surface Water Drainage Structures: The condition of the drainage structures will be inspected, and repairs will be made as needed. Weed growth will be removed as needed. • Fencing: Fencing, gates, and locks will be inspected for damage. Repairs and replacements will be conducted as needed. *Please note that Montante will be responsible for all fencing inspections, maintenance, and repairs for the duration of the Project. Upon decommissioning of the Project, responsibility for landfill fencing inspections and maintenance will return to Xerox.*	conducted as needed. These activities will also be conducted by Montante (as part of Montante's Project O&M Plan). When possible, the Xerox and Montante annual inspections may coincide as a single event. • Vents will be manually inspected for integrity (damage such as cracks, ensuring the vent is in-place and not knocked over, no obstruction and clear of vegetation)

The NYSDEC will be notified within 14 days of discovery of damage or depressions of either landfill cap. Notification will include a summary of actions taken, or to be taken, to repair the condition.

LANDFILL GAS

Geotechnical evaluations conducted in Fall 2025 indicated that the Asphalt-Capped Inactive Landfill and the C&D Landfill exhibit adequate cover design and/or surface bearing capacity to minimize the potential for settlement, slippage, or slope instability. Given the nature of the wastes and design of the Asphalt-Capped Inactive Landfill, gases are not presumed to be present or a concern for this landfill. Based on the age of the C&D Landfill and the reported organic wastes contained, this landfill may be a limited source of landfill gas. There will be no occupied structures on or adjacent to the C&D Landfill, and electrical lines and solar panels will be ballasted above grade without the need for trenching or penetrations through the C&D Landfill cap for these solar Project elements. Conditions that would result in interaction with unsafe landfill gas levels are not expected to occur.

The C&D landfill was constructed with two 6-inch PVC landfill gas vents – one that is currently intact and a second that will be repaired prior to solar development. The Project has been designed to maintain a 15-foot clearance radius (total of 30-feet diameter open space) around the landfill vents. This is a typical mitigation measure for solar projects on landfills and is expected to limit potential interactions of residual gas (if present) with electrical equipment. The gas vents will be protected in place (i.e., temporarily barricaded or marked with fencing, high-visibility flagging, or similar barriers) during construction activities. The 30-foot diameter open space will be maintained around the landfill vents to allow for future access, maintenance, and mitigation of potential interaction with landfill gas. Xerox will inspect the gas vents during annual inspection activities. Replacement, repairs and/or corrective maintenance will be performed as required.

If site conditions change (e.g., substantial subsidence, erosion, or physical damage to the soil cap is observed outside of the scope of minor maintenance requirements), monitoring for unsafe concentrations of explosive gas would be performed on an as-needed basis. If substantial damage or settlement to the cap were to occur, discussions with NYSDEC would be initiated and routine gas monitoring (for a frequency and duration as approved by NYSDEC) would ensue to ensure that the changed condition does not result in a potential for gas (or other) incidents. The duration and frequency of gas monitoring would be determined on a case-by-case basis. In the event that monitoring is required, Xerox will obtain a qualified contractor, and monitoring will be performed using appropriate gas monitoring equipment.

EMERGENCY NOTIFICATIONS AND RESPONSE

Per the proposed updates to the Notifications section (currently designated as Section 1.3) of the Xerox Webster Facility SMP, Xerox will provide a verbal alert to the NYSDEC by noon the following day of any emergency, such as fire, flood, or earthquake, that has the potential to damage the landfill cover systems and within 2 hours of discovery will notify NYSDEC Spills for any spills-related incident. Written confirmation will be provided to the NYSDEC within 7 days that includes a summary of actions taken, or to be taken, and the potential impacts to the environment or public. For any fire, explosion or release to the environment, except if the release is less than or equal to one (1) pound and immediately cleaned up, Xerox will within fifteen (15) calendar days of the incident, submit a written report to the NYSDEC in accordance with 6 NYCRR 373-2.4(g)(10) and include the information required therein. Follow-up status reports will describe and document actions taken to restore the effectiveness of the landfill cover systems.

APPENDICES:

APPENDIX A: Xerox Webster Facility Site Inspection Form

APPENDIX A
XEROX SITE-WIDE INSPECTION FORM

HALEY ALDRICH

Appendix A *Note: Xerox Webster Facility Site Inspection Form is subject to changes and updates based on the Site Management Plan, in accordance with NYSDEC review & approval*

Xerox Webster Facility Site Inspection Form **Reporting Period:** _____

Location (Site/Facility Name): _____
Location (Address): _____
Client: _____

Reason for Inspection: ☐ Annual Inspection
☐ Severe Weather Condition
☐ Other *(Describe)* _____

Date: _____
Performed By: _____

Weather/Site conditions at time of inspection: _____

1. Site Use & Ownership

a. Current Site Use & Owner _____

b. Has site use or owner changed since previous inspection? ☐ YES ☐ NO

If "yes", describe: _____

If "yes", was notification made to the NYSDEC? ☐ YES ☐ NO
Date of NYDEC Notification: _____ *Date Approved on:* _____

c. Have permits been issued for the property since previous inspection? ☐ YES ☐ NO

If "yes", describe: _____

d. Is the site currently undergoing development? ☐ YES ☐ NO

2a. Engineering Controls (Inactive Landfill Cover System)

a. Are engineering controls (cover system) in place and functioning as designed? ☐ YES ☐ NO

i. Asphalt Cap ☐ OK ☐ needs repair *describe:* _____

ii. Drainage Ditches ☐ OK ☐ needs repair/maintenance *describe:* _____

iii. Fencing ☐ OK ☐ needs repair *describe:* _____

2b. Engineering Controls (C&D Landfill Cover System)

a. Are engineering controls (cover system) in place and functioning as designed? ☐ YES ☐ NO

i. Soil cap ☐ intact ☐ needs repair *describe:* _____

ii. Vegetation ☐ OK ☐ needs maintenance *describe:* _____

iii. Vents ☐ intact ☐ needs repair *describe:* _____

2c. Engineering Controls (Site-Wide Migration Control and Treatment System)

a. Are engineering controls (recovery wells, treatment system) in place and functioning as designed? ☐ YES ☐ NO

i. Recovery Wells ☐ intact/ functional ☐ needs repair *describe:* _____

ii. Treatment System ☐ intact/ functional ☐ needs repair *describe:* _____

Xerox Webster Facility Site Inspection Form

Reporting Period: _____

Location (Site/Facility Name): _____

Location (Address): _____

Client: _____

Reason for Inspection: ☐ Annual Inspection
☐ Severe Weather Condition
☐ Other *(Describe)* _____

Date: _____

Performed By: _____

Weather/Site conditions at time of inspection: _____

3. Institutional Controls

a. Is the site in compliance with the environmental easement?	☐ YES	☐ NO	☐ UNKNOWN
b. Is the site being used for commercial or industrial use?	☐ YES	☐ NO	☐ UNKNOWN
c. Has groundwater been used for any purpose?	☐ YES	☐ NO	☐ UNKNOWN
If "yes", was NYSDEC & NYSDOH /County DOH notified, and did the department(s) approve? :	☐ YES	☐ NO	☐ UNKNOWN
d. Is work at the site being conducted in accordance with the SMP?	☐ YES	☐ NO	☐ UNKNOWN
g. Have there been any new enclosed structures constructed over areas that contain remaining contamination since the last inspection?	☐ YES	☐ NO	☐ UNKNOWN
If "yes", was an SVI evaluation performed? :	☐ YES	☐ NO	☐ UNKNOWN

4. Monitoring (indicate if/when monitoring was conducted during the reporting period)

a. Was indoor air monitoring conducted in accordance with the SMP?	☐ YES	☐ NO	☐ UNKNOWN	☐ NOT REQUIRED
b. Was groundwater and surface water monitoring conducted in accordance with the SMP?	☐ YES	☐ NO	☐ UNKNOWN	

5. Soil and Groundwater Management Activities During Reporting Period

a. Were soil management activities performed at the site during the reporting period? ☐ YES ☐ NO

See Soil and Groundwater Management Plan appendix to SMP.

If "yes", describe: _____

_____

_____

_____

If "yes", was SGMP followed (attach documentation to PRR)? ☐ YES ☐ NO

b. Were groundwater management activities performed at the site during the reporting period? ☐ YES ☐ NO

See Soil and Groundwater Management Plan appendix to SMP.

If "yes", describe: _____

_____

_____

_____

If "yes", was SGMP followed (attach documentation to PRR)? ☐ YES ☐ NO

Other Notes and Information:

_____

_____

_____

_____

_____

_____

_____

_____

_____

APPENDIX M
LEASE AGREEMENT WITH AMENDMENT

FIRST LEASE MODIFICATION AGREEMENT

This **First Lease Modification Agreement** ("Agreement") between **Xerox Corporation** ("Landowner") and **Webster Solar Garden LLC** ("Company"), dated as of February 12, 2026 ("Effective Date"), shall amend and modify the Lease (as defined below). All terms not otherwise defined herein shall have the same meaning as in the Lease.

WITNESSETH

WHEREAS, Landowner and Company entered into that certain Land Lease and Solar Easement dated November 10, 2025 (the "Lease"), regarding the lease of approximately 20 acres of land located on Caracas Drive, Town of Webster, Monroe County, as more particularly described in the Lease; and

WHEREAS, the Parties desire to modify and/or supplement certain terms of the Lease as a result of comments received from the New York State Department of Environmental Conservation; and

NOW THEREFORE, in consideration of the foregoing and the mutual covenants herein contained, the Parties agree as follows:

1. Section 1.1 is hereby amended as follows:

   The defined term "Landfill Permit" shall be deleted and replaced with "Corrective Action Permit." "Corrective Action Permit" shall be defined as follows:

   > The 6 NYCRR Part 373 Hazardous Waste Management Permit No. DEC #8-2654-00064/00040, issued to Landowner on April 12, 2012, with respect to NYSDEC Site Number 828178, USEPA ID Number NYD002211324, as modified during the active life of the permit and as extended pursuant to the New York State Administrative Procedures Act. The RCRA Permit covers, and governs the Landowner's ongoing RCRA corrective action obligations with respect to, all contiguous Landowner-owned land that is part of Landowner's Webster, New York campus, including the landfill area (the "Landfill") where the Leased Premises is situated. **Note**: The Landowner and the NYSDEC are in the process of negotiating a RCRA corrective action consent order (the "RCRA Order") that is intended to replace the RCRA Permit. Upon the effective date of the RCRA Order, all references to "RCRA Permit" in this Lease shall henceforth be regarded as references to the "RCRA Order."

2. The Lease is hereby amended so that each reference to "Landfill Permit" is replaced by "Corrective Action Permit."

3. Section 2.1A is hereby modified by adding the language underlined, bolded, and italicized below as follows:

   > … provided, however, that all construction, installation, reinstallation, operation and maintenance of such Solar Facilities shall conform in all respects to the New York State Department of Environmental Conservation's ***("NYSDEC")*** "Policy DMM-4

Guidance for Photovoltaic Solar Projects at Closed Solid Waste Landfills (as ***may be*** amended from time to time, "DMM-4") as applicable***, and shall conform with any Corrective Action Permit requirements or other state, local, or federal regulations Landowner may be subject to. Landowner expressly retains all rights to access the following areas at any time and for any reason, and Company shall not prevent Landlord's access to or use of such areas: (1) the building on the Leased Premises and the storage grounds immediately east of such building, (2) areas among the Solar Facilities reasonably necessary for ongoing environmental monitoring and groundwater pumping required by Landowner's agreement with the NYSDEC, and (3) the gravel road along the northerly side of the Leased Premises as reasonably necessary for Landowner's compliance with any state pollution discharge elimination system permit or program. If Company desires to enclose any such areas with a fence, it will ensure Landowner's ability to exercise such retained rights by providing access through such fence for Landowner or its designees and their personnel, vehicles and equipment by customary multi-lock arrangement on gates.***

4. Section 8.1 is hereby modified by adding the language underlined, bolded, and italicized below as follows:

> ...the Parties shall either amend this Lease to reflect any necessary relocation and/or modification of the Leased Premises or Solar Facilities which will preserve the value and benefit of the Lease to the Company and Landowner, together with any corresponding payments***, provided that any such relocation or modification shall be done in accordance with DMM-4 and with all laws and permits applicable to the Property, consistent with the requirements of Sections 2.1C and 4.2 above***, or, at either Party's option, this Lease shall terminate in which event neither Party shall have any further obligations hereunder except those which expressly survive the termination of this Lease and Company's obligation to decommission and remove the Solar Facilities and return the Leased Premises to its original condition pursuant to Section 4.5...

5. Each Party represents and warrants to the other that it has not dealt with any broker, finder, or agent in connection with the negotiation, execution, or delivery of this Agreement. Each Party shall indemnify, defend, and hold harmless the other party from and against any and all claims, demands, liabilities, losses, costs, and expenses (including reasonable attorneys' fees) arising from or related to any claim for brokerage commissions, finder's fees, or similar compensation asserted by any broker, finder, or agent claiming through or under the indemnifying party in connection with this Agreement.

6. In the event of any conflicts between the terms of this Agreement and the terms of the Lease, the provisions of this Agreement shall control. Except as modified herein, all terms and provisions of the Lease shall remain in full force and effect.

7. Each party signing this Agreement represents to the other that he has been duly authorized to execute this document by all requisite corporate action. This Agreement may be executed in any number of counterparts, including facsimile, electronic or scanned PDF documents. Each such counterpart, facsimile, electronic or scanned PDF document shall be deemed an original instrument and all of which, together, shall constitute one and the same instrument.

[Remainder of Page Intentionally Blank; Signature Page(s) Follow]

IN WITNESS WHEREOF, the parties hereto have executed this Agreement as of the Effective Date.

LANDOWNER

XEROX CORPORATION

By: _____

Name: John Barrett

Title: Manager - Western Hemisphere Real Estate Portfolio

02 / 12 / 2026

COMPANY

WEBSTER SOLAR GARDEN LLC

By: _____

Name: Daniel Montante

Title: Member

LAND LEASE AND SOLAR EASEMENT

This Land Lease and Solar Easement (the "Lease") is made and entered into as of the Effective Date (hereinafter defined) by and between Landowner (hereinafter defined) and Company (hereinafter defined). Landowner and Company may hereafter be referred to as, together, the "Parties" and each, a "Party".

RECITALS

A. Landowner is the owner of the Property (hereinafter defined).

B. Company wishes to obtain a land lease and easements from Landowner, on, along, over and under certain portions of the Property to be identified as the "Leased Premises" in the manner set forth in this Lease for the purpose of planning, constructing, operating and maintaining solar energy generating systems and related electrical or other energy distribution and transmission infrastructure (collectively, the "Project").

AGREEMENT

In consideration of the foregoing and other good and valuable consideration, the receipt and adequacy of which are hereby acknowledged, the Parties hereto agree as follows:

ARTICLE 1
DEFINITIONS

1.1 Definitions. The following terms have the definitions ascribed to such terms in the table below:

Term	Definition
"Effective Date"	The date this Lease is fully executed, as evidenced by the latest date below the signature lines of each Party
"Landowner" (and address)	Xerox Corporation Whose address for notice purposes hereunder is: 800 Phillips Rd., Webster, New York 14580 Mailstop 0111-04-99F Email: john.barrett@xerox.com Phone: 585-422-9052

"Company" (and address)	Webster Solar Garden LLC Whose address for notice purposes hereunder is: 2760 Kenmore Avenue, Tonawanda, NY 14150 Phone: 716 876-8899 Email: dmontante@montante.com Phone: 716-876-8899
"Leased Premises"	Approximately 20 acres of land, which shall be located on the Property as approximately shown on Exhibit "B" attached hereto; provided however, that Landowner and Company acknowledge that the actual location of the Leased Premises will be determined in accordance with Section 2.2B of this Lease. Notwithstanding anything to the contrary contained herein, the final location of the Leased Premises shall be agreed upon by Landowner and Company during the Due Diligence Period.
"Property"	That certain lot or tract of land located in Monroe County, New York described and/or depicted on Exhibit "A" attached hereto and made a part hereof for all purposes which is comprised approximately 20 acres of land on Caracas Drive, Town of Webster, Monroe County, SBL number 065.02-1-40.11.
"Lease Term"	The Lease Term is comprised of three components, the Due Diligence Period, the Construction Term and the Operations Term. 1. "Due Diligence Period": The Due Diligence Period will commence on the Effective Date and terminate on the earlier to occur of (i) the beginning of the Construction Term, or (ii) the termination of this Lease. 2. "Construction Term": The Construction Term will commence immediately upon the date the Company issues a notice to proceed to its construction contractor as provided in a written notice to the Landowner and approved by Landowner in writing subject to Section 2.2.B below, provided this Lease has not been terminated, and will extend until the date of commercial operation filed by the Company to the relevant utility company, but such date shall not be later than December 31, 2027. 3. "Operations Term": The Operations Term shall consist of twenty-five Lease Years commencing immediately upon conclusion of the Construction Term and ending on the last day of the 25th Lease Year Thereafter (the "Expiration Date"). The first Lease Year will commence on the 1st day of the Operations Term and will end on the last day of the twelfth (12th) calendar month

	thereafter. Subsequent Lease Years will each be for a period of twelve (12) consecutive full calendar months. If mutually agreed by the Landowner and Company, Company shall have the right (but not the obligation) to extend the Lease Term for two-five (5) Lease Year periods (each an "Extension Period") by giving written notice to Landowner no later than one Lease Year prior to the Expiration Date of either the initial term, if applicable, or the first Extension Period.
"Due Diligence Rent"	The sum of one dollar ($1.00) for the Leased Premises, commencing on the first day of the Due Diligence Period.
"Construction Rent"	Rent for the Construction Term shall be a one-time payment of $1.00 payable on or before the first day of the Construction Term. The Construction Rent is not refundable.
"Operation Rent"	If construction of the Project is completed within 365 days after the start of the Construction Term, the sum of one dollar ($1.00) for the Leased Premises for each Lease Year, commencing on the first day of the Operations Term. If construction of the Project is not completed within 365 days after the start of the Construction Term, a monthly payment of $2,833.96 until the date of commercial operation filed by the Company to RG&E, prorated for partial months, then, beginning on the following month and for each Lease Year thereafter, one dollar ($1.00).
"Landfill Permit"	That certain Hazardous Waste Management Permit No. DEC #8-2654-00064/00040, issued to Landowner on April 12, 2012, with respect to NYSDEC Site Number 828178, USEPA ID Number NYD002211324, as amended or modified from time to time. The area of the Premises covered by such Landfill Permit, the "Landfill".
"Power Purchase Agreement"	That certain Power Purchase Agreement dated as of the Effective Date by and between the Company and Landowner.

ARTICLE 2
NATURE AND TERM OF AGREEMENT

2.1 **Interests Granted.**

A. Grant of Leasehold Interest. Landowner hereby leases exclusively to Company the Leased Premises for the exclusive purpose of planning, constructing, installing, re-installing, operating, and maintaining solar energy conversion systems, including, without exclusion, solar photovoltaic panels (including concentrating solar photovoltaic equipment), solar resource measurement equipment, support structures, foundations, pads, footings, fencing, electrical inverters and transformers, meters, switches, breakers, fixtures, equipment storage, electric wiring, distribution and transmission lines, access roads, interconnection facilities and related structures and facilities and equipment as the foregoing pertain to the Project (collectively the "Solar Facilities") on the Property to the extent set forth in this Lease, and at Company's sole cost and expense; provided, however, that all construction, installation, reinstallation, operation and maintenance of such Solar Facilities shall conform in all respects to the New York State Department of Environmental Conservation's "Policy DMM-4 Guidance for Photovoltaic Solar Projects at Closed Solid

Waste Landfills" (as amended from time to time, "DMM-4") as applicable.

B. Grant of Access Easement. Landowner hereby grants to Company a non-exclusive easement (the "Access Easement") for vehicular and pedestrian ingress and egress to and from the Leased Premises over the Property (including areas outside the Leased Premises). The Access Easement shall, to the extent practicable, be across existing roadways and be located where reasonably acceptable to Landowner. Company agrees to work with Landowner to minimize impacts to Landowner's Property outside the Leased Premises, taking into consideration the operation of the Property as a commercial and industrial campus. The initial proposed location of the Access Easement is depicted on Exhibit "C", attached hereto and made a part hereof, and the final location shall be agreed upon by Landowner and Company during the Due Diligence Period consistent with all permits and applicable laws. The Access Easement shall not permit ingress or egress to or from the Leased Premises by any equipment not designed to not have a material impact on the Landfill. The Site Plan for the Project approved by the appropriate governmental authorities having jurisdiction over the Property shall become the final Exhibit C.

C. Grant of Utilities Easement. Landowner hereby grants to Company an easement for constructing, installing, re-installing, operating, and maintaining electric wiring, distribution and transmission lines, and communications lines over and across and under the Property (including areas outside the Leased Premises) at Company's sole cost and expense (the "Utility Easement"). Such easement shall, to the extent practicable, utilize existing utility corridors on the Property and be located where reasonably acceptable to Landowner and in compliance with all laws and permits applicable to the Property, including DMM-4. Company agrees to work with Landowner to minimize impacts to Landowner's Property outside the Leased Premises, taking into consideration the operation of the Property as a commercial and industrial campus. The initial proposed location of the Utility Easement is depicted on Exhibit "C", attached hereto and made a part hereof, and the final location shall be agreed upon by Landowner and Company during the Due Diligence Period consistent with all permits and applicable laws. The final agreed upon Utility Easement, if not shown on the approved Site Plan to be attached as Exhibit C, shall be attached as Exhibit C-1.

D. Intentionally Omitted.

E. Inspection Rights. During the Due Diligence Period, Landowner grants to Company the right to undertake any other activities on the Leased Premises, Access Easement and Utility Easement that Company determines are reasonably necessary, helpful, appropriate or convenient in connection with, incidental to, for the benefit of, or to accomplish the installation, operation, and maintenance of, the proposed Solar Facilities, including conducting surveys and geological, non-invasive environmental (including a Phase I environmental site assessment), biological, cultural and other non-invasive tests and studies, in each case by the use of such means and technologies as Company may reasonably choose consistent with prevailing standards in the applicable industry, whether or not currently known, and with the right of entry on the Property for such purposes. Landowner shall have the right to accompany Company on any inspections or activities. Company shall reasonably cooperate with Landowner to minimize disruption of Landowner's operations on the Property. If any damage results at the Property from Company's inspections or activities, including any violation of the Landfill Permit or damage that would cause a

violation of, or Landowner's inability to comply with, the Landfill Permit, Company shall promptly and at its sole cost and expense repair such damage as close to the condition that existed immediately prior to such damage as is to the extent reasonably practical. The repair obligation in this paragraph shall survive the expiration or earlier termination of this Lease. Notwithstanding anything to the contrary contained herein, Company may, upon written notice to Landowner, perform any invasive or non-invasive testing required by the New York State Department of Environmental Conservation ("NYSDEC") to obtain any permits or approvals required from NYSDEC for the Project. Except as permitted above the Company may not perform a Phase II environmental study or other intrusive environmental studies without first obtaining the prior written consent of Landowner, which such consent is not to be unreasonably withheld. If any damage results at the Property from Company's inspections or activities, Company shall promptly and at its sole cost and expense repair such damage as close to the condition that existed immediately prior to such damage as is to the extent reasonably practical. The repair obligation in this paragraph shall survive the expiration or earlier termination of this Lease.

2.2 **Notice to Landowner; Amendment to Leased Premises.**

A. Due Diligence Period. During the Due Diligence Period, if Company determines, in its discretion, for no reason or for any reason, that the Leased Premises is not appropriate for Company's intended use (or if Company decides for any reason or no reason, not to construct Solar Facilities on the Property), then Company may cancel and terminate this Lease upon written notice to Landowner at any time during the Due Diligence Period. Company makes no representation or warranty as to the likelihood that this Lease will extend into the Construction Term.

B. Notice to Landowner: Leased Premises. If Company determines that it wishes to construct Solar Facilities on the Property, prior to the expiration of the Due Diligence Period, Company shall provide written notice to Landowner of its intent to begin construction of the Solar Facilities ("Notice"). The Notice will specify the date that the Construction Term will begin, such date to occur not less than thirty (30) days following Landowner's receipt of the Notice. Landowner shall promptly respond to the Notice affirming its approval of such date or informing Company that it does not approve of such date which failure to approve shall be solely as a result of Landowner being unable to obtain any required consents from the NYSDEC, shall state the specific basis for such disapproval, and suggesting another start date for the Construction Term except that Landowner shall have the right to terminate this Lease in the event Landowner does not reasonably believe it will, despite making a good faith effort, obtain any such required NYS DEC consent. . Such Notice also shall include a legal description, survey and calculated acreage of each of the Leased Premises, Access Easement and Utility Easement, and which shall be attached to this Lease as a replacement of Exhibit "B" and "Exhibit "C" and shall constitute the "Leased Premises", "Access Easement" and "Utility Easement" for all purposes.

2.3 Termination of Lease. The occurrence of any of the following events shall terminate this Lease:

A. The expiration of the Lease Term, as same may be extended; or

B. The termination of this Lease as expressly provided for herein; or

C. The written agreement of both Parties to terminate this Lease; or

D. Company fails to provide Notice of its intent to construct Solar Facilities to Landowner prior to the end of the Due Diligence Period; or

E. the termination or expiration of the Power Purchase Agreement; or

F. A change in applicable law materially impacting the ability of either Party to directly or indirectly recognize reasonable economic benefits from the Solar Facility or materially impacting a Party's risk of liability or ability to comply with applicable law, unless the Parties agree to an amendment to this Lease satisfactorily addressing such impacts within ninety (90) days of the passage of such law.

ARTICLE 3
RENT, TAXES

3.1 **Rent.**

A. Due Diligence Period Rent. Due Diligence Rent shall be made as set forth in Section 1.1.

B. Construction Rent: Construction Rent shall be made as set forth in Section 1.1.

C. Operations Term Rent. Company shall pay to Landowner a rental payment equal to the Operation Rent on the commencement of the Operations Term and periodically thereafter as set forth in Section 1.1. Operation Rent payments shall be prorated for any partial year.

3.2 **Taxes, Assessments, and Utilities.**

A. Tax Obligations of Landowner. The Landowner shall pay, when due, all real property taxes and assessments levied against the land portion of Leased Premises (excepting therefrom any taxes, assessments and other charges and payments-in-lieu thereof related to any improvements on the Leased Premises, including without limitation the Solar Facilities).

B. Tax Obligations of Company. Company shall pay when due all personal and real property taxes and assessments and other impositions and payments-in-lieu thereof levied against the Solar Facilities and any other improvements constructed with respect thereto as well as any claw back of any exemption or changes in amounts of real property taxes associated with the loss of any exemption but only to the extent such loss of an exemption was caused by the Company (collectively the "Taxes"). Notwithstanding the forgoing, the Company's obligations with respect to Taxes are limited to paying PILOT payments and real property taxes to the extent they apply to the improvements constructed on the Property by Company, and Landowner is to pay any PILOT or other real property taxes on the assessed value of the Property, if any. To the extent PILOT or real property tax bills do not separately show the value of the Property from the value of the improvements constructed on the Property by Company, the parties will work in good faith to determine the assessed value of such

improvements and Company shall pay Taxes allocable to such value. If such value cannot be determined, the parties will hire and split the cost of a third-party licensed appraiser to determine the fair market value of such improvements, and the Company shall be responsible for the payment of Taxes on such amount. If the Property or any portions thereof experiences any increase in the amount of Taxes assessed as a result of the Project, including without limitation the installation of the Solar Facilities on the Property and any reclassification of the Property, Company shall pay or reimburse Landowner an amount equal to the increase no later than thirty (30) days prior to the date each year on which the applicable Taxes are due to be paid, provided that Landowner provides Company sufficient notice with copies of the applicable current and past statements of Taxes payable for the Property and any related information demonstrating the reasons for any increase in Taxes.

C. Contested Taxes. Either Party may contest the validity or amount of any taxes for which each is responsible under this Lease as long as such contest is pursued in good faith and with due diligence and the Party contesting the taxes has paid the obligation in question or established adequate reserves to pay the obligation in the event of an adverse determination.

D. Utilities. Company shall pay when due for all water, electric, telecommunications and any other utility services used by the Solar Facilities or Company on the Property.

ARTICLE 4
COMPANY'S OBLIGATIONS

4.1 **Liens.** Company shall keep the Property free and clear of all liens and claims of liens for labor, material, services, supplies and equipment performed on or furnished to Company or any of the components of the Solar Facilities or Project or in connection with Company's use of the Property, including, for the avoidance of doubt, and municipal lien filed in connection with the Company's failure to decommission the Solar Facility as required under applicable law (collectively, the "Liens"). Company may contest any such Lien, but shall post a bond or utilize other available means to remove or bond any Lien that is created during the contested proceeding. Company agrees to otherwise remove any Lien or encumbrance for which it is responsible pursuant to this paragraph within thirty (30) days of the receiving notice of the creation of any such Lien or encumbrance. Encumbrances incurred by Company in accordance with Section 5.1 are not subject to this Section 4.1. Notwithstanding the foregoing, in the event that any Lien is filed against the Property or any portion thereof, Landowner, at its option, after ten (10) days' notice to Company, may bond, discharge, defend or pay the Lien without inquiring into the validity thereof, and Company shall forthwith reimburse Landowner the total expense incurred by Landowner in discharging or bonding, paying or defending against the said lien within ten (10) days demand for same.

4.2 **Permits and Laws.** Company and its designees shall at all times, and at Company's sole cost and expense, comply with all applicable federal, state, and local laws, statutes, ordinances, rules, regulations, permits, judgments and other valid orders of any governmental authority with respect to the Property and Company's activities pursuant to this Lease and shall obtain all permits, licenses, approvals, determinations, clearances and orders required to conduct any such activities. Company shall not create, maintain or permit any nuisance (public or private) on the Leased Premises or Property. Company shall not commit any act of waste in, on or about the Leased Premises or Property.

4.3 **Company's Improvements.**

A. Ownership. All Solar Facilities constructed, installed or placed on the Property by Company pursuant to this Lease shall be the sole property of Company, and Landowner shall have no ownership or other interest in any Solar Facilities on the Property.

B. Construction and Maintenance. The Solar Facilities shall be constructed in compliance with all applicable laws and permit applicable to the Leased Property, including, without limitation, the New York State Department of Environmental Conservation's DMM-4 regulations regarding installation of photovoltaic projects on closed landfills, and shall be substantially consistent with the site plan approved by Landowner for submission to the local permitting authority attached hereto. Company shall not make any material changes to the Solar Facility without Landowner's prior written consent, not to be unreasonably withheld or delayed. Throughout the Lease Term, Company shall, at its sole cost and expense, maintain Company's Solar Facilities and the Leased Premises in good condition and repair, ordinary wear and tear excepted, and in compliance with all applicable laws, permits, rules and regulations and in accordance with standard industry practice. All Solar Facilities constructed, installed or placed on the Property by Company pursuant to this Lease may be moved, replaced, repaired or refurbished by Company at any time, without material deviation from the site plan discussed between the parties and with substantially the same or better materials, equipment and services. While on Landowner's property, Company and its subcontractors and agents shall comply with all of Landowner commercially reasonable policies of which Company has been informed in advance in writing.

4.4 **Insurance.** During the Lease Term, including during the Due Diligence Period with respect to general liability, workers' compensation, automotive, umbrella, and pollution policies, Company and each of its contractors, subcontractors and agents shall comply, at its sole cost and expense, with the Insurance requirements set forth in Exhibit "F" attached hereto and made a part hereof.

4.5 **Surrender, Decommissioning and Removal.** Within twelve months after the expiration or earlier termination of the Lease Term (the "Decommissioning Period"), Company shall decommission, dismantle and remove the Solar Facilities and all other property of Company located on the Premises in accordance with the Town of Webster Solar Law, returning the Premises to its condition as of the Effective Date to the extent reasonably practical (reasonable wear and tear, non-Company caused casualty and condemnation excepted) (the "Decommissioning Obligations"). Landowner hereby grants to Company and its successors and assigns a license to enter upon the Leased Premises to perform the activities required to be performed by Company pursuant to this paragraph. No later than the earlier of (i) the date of commencement of the Construction Term, and (ii) the date required by applicable law, Company shall post a bond or letter of credit (the "Decommissioning Bond") as surety for 115% of the estimated gross cost of performing the Decommissioning Obligations, provided, to the extent that any governmental authority requires a decommissioning or restoration bond, letter of credit, cash deposit or other security to secure the Decommissioning Obligations, then Company shall instead comply with the requirements of such governmental authority and this Lease by obtaining a co-obligee Decommissioning Bond in favor of both Landowner and such governmental authority. In such event, Landowner acknowledges and agrees that Company's compliance with such governmental decommissioning and restoration security requirements shall fulfill, replace and satisfy Company's obligation to post the Decommissioning Bond under this Section 4.5. The Decommissioning Bond shall be irrevocable

and shall expressly state that it may not be terminated or cancelled and must be held in place by the surety during the Construction Term and Operations Term. Company shall provide Landowner with evidence of continuation or extension of the Decommissioning Bond no less than ten (10) business days prior to the expiration of the then-current term of such bond. Company shall cause the value of the Decommissioning Bond to be increased no less than every ten (10) years during the Term to 115% of the amount determined by Company and verified by an independent third party valuation engineering or consultant, and Owner shall be provided a copy of such valuation no less than thirty (30) days prior to the end of the then-current such ten-year period. The amount of such Decommissioning Bond shall increase by 2% from the last year's value in each year between such valuations. If Company fails to timely perform its Decommissioning Obligations set forth in this Section 4.5, upon ninety (90) days' notice to Company, the Landowner may perform, or cause a third party to perform, the Decommissioning Obligations on Company's behalf and draw upon the Decommissioning Bond to perform, or reimburse such third party for performing, the Decommissioning Obligations. In the event the Company is working to perform its Decommissioning Obligations set forth in this Section 4.5 but such obligations are not complete prior to the Decommissioning Period, then Company shall pay Landowner holdover monthly rent at 150% of the last month's payment made by Landowner under the Power Purchase Agreement prior to such expiration or termination.

ARTICLE 5
OBLIGATIONS OF THE PARTIES

5.1 **Title and Authority.**

A. Authority. Except to the extent otherwise stated in this Lease, Landowner represents it is the sole owner of the Property and each person signing the Lease on behalf of Landowner has the full and unrestricted authority to execute and deliver this Lease and to grant the easements and rights granted herein. All persons having any ownership interest in the Property are signing this Lease as Landowner. When signed by Landowner, this Lease constitutes a valid and binding agreement enforceable against Landowner in accordance with its terms. Company represents it has the full and unrestricted authority to execute and deliver this Lease and to perform all Company obligations herein. When signed by Company, this Lease constitutes a valid and binding agreement enforceable against Company in accordance with its terms.

B. Company's Title Examination. During the Due Diligence Period Company may notify Landowner in writing that it has disapproved any title exceptions which negatively impact marketability of the Leased Premises for Company's purposes hereunder and shall notify Landowner of any such disapproved title exceptions ("Disapproved Encumbrances"). If within ten (10) days after receipt of notice of the Disapproved Encumbrances, Landowner is unable or unwilling to, cure or cause the removal of against the Disapproved Encumbrances, then Company will have the option (such option exercised prior to the expiration of the Due Diligence Period) to either: (i) waive the Disapproved Encumbrances and proceed with this Lease; or (ii) terminate this Lease by written notice to Landowner.

C. Encumbrances. Landowner represents to the best of its knowledge that there are no encumbrances or liens against the Property except as recorded in the public records or as otherwise disclosed to Company in writing.

D. Subordination. This Lease shall be subject and subordinate to the lien of any mortgage or mortgages or deed or deeds of trust, which at any time may be placed upon Landowner's interest

in the Leased Premises or Property. Company agrees that in the event that any holder of any ground or underlying lease, mortgage or deed of trust succeeds to Landowner's interest in the Leased Premises, Company will attorn to such successor in interest without change in the terms or provisions of this Lease. Notwithstanding the foregoing, it is further expressly agreed and understood that any such successor to Landowner's interest in the Leased Premises shall not assume or be deemed to assume any liabilities of Landowner pursuant to this Lease or otherwise solely by virtue of such successor's acceptance of title to all or a portion of the Property, acceptance of Rent or otherwise. In the event that there are one or more mortgages encumbering the Property as of the Effective Date, Landowner shall obtain, following written request from Company, a written subordination and non-disturbance agreement on Landowner's lender's standard form, for themselves and their successors, not to disturb Company's and Company's Mortgagees' rights and possessory interests under this Lease in the event of foreclosure, judicial sale or other proceedings in connection with said mortgages. The non-disturbance agreement must be in a form reasonably acceptable to Company.

5.2 **Quiet Enjoyment.** Company shall have the quiet use and enjoyment of Leased Premises in accordance with the terms of this Lease without unreasonable interference by Landowner or any person claiming through Landowner. Landowner and its activities on the Property and any grant of rights Landowner makes to any other person shall not unreasonably interfere with Company's activities pursuant to this Lease.

5.3 **Cooperation.** Landowner shall cooperate with Company to obtain non-disturbance and subordination agreements from any person or entity with a lien, encumbrance, mortgage, lease, or other exception to Landowner's fee title to the Property to the extent necessary to eliminate any actual or potential interference by any such lienholder with any rights granted to Company under this Lease. Landowner shall also cooperate with Company to obtain and maintain any permits or authorizations needed for the Solar Facilities provided that Landowner incurs no expenses (other than fees of its attorneys, accountants and other professional advisors not exceeding $500) in doing so. Each Party shall provide the other with such further assurances that may be reasonably necessary for recording purposes or requested to confirm the terms hereof. Landowner shall also reasonably cooperate, at no out-of-pocket cost to Landowner, with Company's efforts, if any, to obtain access to water for purposes of construction, operations or maintenance of the Solar Facilities.

ARTICLE 6
INDEMNIFICATION, SURFACE DAMAGE, HAZARDOUS SUBSTANCES

6.1 **Indemnification.** To the greatest extent permitted by law, each Party (the "Indemnifying Party") agrees to defend, indemnify, and hold harmless the other Party and the other Party's officers, directors, employees, representatives, mortgagees and agents (collectively the "Indemnified Party") from and against any and all losses, damages, claims, actual out of pocket, reasonable and documented expenses, and liabilities for physical damage to property and for physical injury to any person (including death), including without limitation, reasonable attorneys' fees, to the extent resulting from or arising out of (i) any operations or activities of the Indemnifying Party on the Property; (ii) any negligent or intentional act or omission on the part of the Indemnifying Party; or (iii) any breach of this Lease by the Indemnifying Party beyond any applicable notice and cure periods. This indemnification shall not apply to losses, damages, claims, expenses, and liabilities to the extent caused by any negligent or intentional act or omission on the part of the Indemnified Party. This indemnification shall apply only to losses, damages, claims, expenses and liabilities accruing during the Lease Term and the

Decommissioning Period. The provisions of this paragraph shall survive the expiration or sooner termination of this Lease. Notwithstanding anything to the contrary in the above, each Party agrees that, except as otherwise provided herein, under no circumstances shall the Company be liable for (i) any conditions at, on in, or under the Leased Premises or Property that existed prior to the Effective Date, including any that arise from or is related to any environmental conditions that existed prior to the Effective Date, except to the extent such conditions are exacerbated by the Company or its contractors or agents, and then, only to the extent of the exacerbation, or (ii) any acts or omissions of the Landowner, no matter if prior to or after the Effective Date, including those related to the presence, exposure to, storage, disposal, release, management, or transportation of Hazardous Substances (as defined in Section 6.2(c) below), or any attendant claims. Further notwithstanding anything to the contrary in the above, each Party agrees that, except as otherwise provided herein, under no circumstances shall the Landowner be liable for (i) any conditions at, on in, or under the Leased Premises or Property, including its environmental condition, that was introduced or created by the Company or its contractors or agents, or (ii) any acts or omissions of Company or its contractors or agents, including those related to the presence, exposure to, storage, disposal, release, management, or transportation of Hazardous Substances (as defined in Section 6.2(c) below), or any attendant claims. Notwithstanding anything to the contrary contained herein, Company shall not undertake any remediation efforts or incur any remediation costs with respect to pre-existing environmental contamination without first obtaining Xerox's prior written consent

6.2 **Surface Damage, New Improvements.**

A. Damage. The Parties anticipate and acknowledge that Landowner may suffer damage to crops, grass, fences, and other property or improvements on the Leased Premises during Company's construction, installation, decommissioning, and maintenance of Solar Facilities. The, Company shall not be responsible for any losses of income, rent, business opportunities, profits or other losses arising out of Landowner's inability to otherwise use the Leased Premises, except to the extent caused by Company's actions causing noncompliance with, or the inability to comply with, the Landfill Permit.

B. Limitation on New Improvements. Notwithstanding Section 6.2A, Landowner shall not undertake any improvement on the Leased Premises inconsistent with Company's future use of the Leased Premises without Company's written consent or unless required by applicable law or in connection with the Landfill Permit. Any such improvement made without Company's prior written consent will be made at Landowner's sole risk.

ARTICLE 7
ASSIGNMENT; ENCUMBRANCE OF LEASE

7.1 **Right to Encumber** Company may at any time mortgage (a "Mortgage") all or any part of its interest in the Lease and rights under this Lease and/or otherwise encumber, grant security interests, and/or enter into a collateral assignment of all or any part of its interest in the Lease or rights under this Lease or the Solar Facilities or any part thereof (the "Solar Assets") to any entity (together with its assignees or designees, a "Mortgagee") without the consent of Landowner, except to an entity that is, or is an affiliate of, a business set forth on Exhibit G. Company shall promptly provide prior written notice to Landowner of a Mortgagee's interest in the Lease and specify the Mortgagee's address to be used for issuance of notices under this Lease.

7.2 **Assignment.** Company may assign, sublease, transfer or convey all or part of its interests in this Lease with Landowner's consent, such consent to not be unreasonably withheld or delayed; provided, however, that Company will remain liable for all obligations under this Lease unless the assignee expressly assumes such obligations in writing delivered to Landowner along with evidence reasonably acceptable to Landowner that such assignee has operating capabilities and a net worth at least similar to that of Company and that such assignee has also accepted assignment of the Power Purchase Agreement and is ready, willing and able to perform all of Company's obligations under this Lease and the Power Purchase Agreement. Company shall provide prior written notice to Landowner of any such assignment, sublease, transfer, or conveyance.

7.3 **Continuing Nature of Obligations.**

A. Easement in Gross. The Solar Easement is an easement in gross for the benefit of Company, its successors and assigns, as owner of the rights created by such easement.

B. Run with the Land. The burdens of the Access Easement, Utility Easement and Solar Easement, and all other rights granted to Company in the Lease shall run with and against the Property and shall be a charge and burden on the Property and shall be binding upon and against the Landowner and its successors, assigns, permittees, licensees, Company, employees and agents. The Lease and Solar Easement shall inure to the benefit of the Company and its successors, assigns, permittees, licensees and sub-tenants.

7.4 **Mortgagee Protection.**

A. Landowner's Obligations. Landowner agrees to negotiate in good faith a subordination, non-disturbance and attornment agreement with any Mortgagee containing customary terms and conditions. Provided Landowner has received notice of Mortgagee as required by Section 7.1 of this Lease, Landowner acknowledges that the following terms and conditions shall apply.

B. Mortgagee Cure Rights. Following acquisition of all or a portion of the Solar Assets by the Mortgagee as a result of either foreclosure or acceptance of an assignment in lieu of foreclosure, or by a purchaser at a foreclosure sale, this Lease shall continue in full force and effect and the Mortgagee or party acquiring title to the Solar Assets shall, as promptly as reasonably possible, commence the cure of all defaults under this Lease and thereafter diligently process such cure to completion. If a Mortgagee is prohibited by any process or injunction issued by any court or by reason of any action of any court having jurisdiction over any bankruptcy or insolvency proceeding involving Company from commencing or prosecuting the appropriate judicial or nonjudicial proceedings, then any cure period for commencing such proceedings shall be extended for the period of such prohibition. Each Mortgagee shall have the same amount of time to cure the default as to Company's entire interest or its partial interest in the Solar Assets as is given to Company and the same right to cure any default as Company (as addressed herein below) or to remove any property of Company, Mortgagees or Assignees located on the Leased Premises. The cure period for each Mortgagee shall begin to run at the end of the cure period given to Company in this Lease, but in no case shall the cure period for any Mortgagee be less than thirty (30) days after receipt of the Notice of Default. The Mortgagee shall have the absolute right, but not the obligation, to substitute itself for Company and perform the duties of Company

hereunder for purposes of curing such Event of Default. Landowner expressly consents to such substitution, agrees to accept such performance, and authorizes the Mortgagee (or its employees, agents, representatives or contractors) to enter upon the Leased Premises to complete such performance with all of the rights and privileges of Company hereunder. Any Mortgagee that does not directly hold an interest in the Solar Assets, or who interest is held solely for security purposes, shall have no obligation or liability under this Lease prior to the time the Mortgagee directly holds an interest in the Solar Assets, or succeeds to absolute title to Company's interest. A Mortgagee shall be liable to perform Company's obligations under this Lease only for and during the period it directly holds such interest or absolute title. Further in the event that a Mortgagee elects to (a) perform Company's obligations under this Lease, (b) continue operations on the Leased Premises, (c) acquire any portion of Company's right, title or interest in all or any of the Solar Assets or (d) enter into a new easement agreement as provided herein below, Landowner's sole recourse in the Event of Default by such Mortgagee shall be to execute against such Mortgagee's interest in the Solar Assets. Moreover, any Mortgagee or other party who acquires the Solar Assets pursuant to foreclosure or an assignment in lieu of foreclosure shall not be liable to perform any obligations under this Lease to the extent the same are incurred or accrue after such Mortgagee or other party no longer has ownership of the Solar Assets.

C. Mortgagee's Right to Possession. Right to Acquire and Right to Assign. A Mortgagee shall have the absolute right: (a) to assign its Mortgage; (b) to enforce its lien and acquire title to all or any portion of the Solar Assets by any lawful means; (c) to take possession of and operate all or any portion of the Solar Assets and to perform all obligations to be performed by Company under this Lease, or to cause a receiver to be appointed to do so; and (d) to acquire all or any portion of the Solar Assets by foreclosure or by an assignment in lieu of foreclosure and thereafter without Landowner' s consent to assign or transfer all or any portion of the Solar Assets to a third party. Landowner's consent shall not be required for any of the foregoing, and upon acquisition of the interests of all or any portion of the Solar Assets by a Mortgagee or any other third party who acquires the same from or on behalf of the Mortgagee, Landowner shall recognize the Mortgagee or such other party (as the case may be) as Company's proper successor, and this Lease and the Easements shall remain in full force andeffect, provided that Landowner has prior written notice of such action and the full name and contact information of all parties thereto.

D. Termination. Neither the bankruptcy nor the insolvency of Company shall be grounds for terminating this Lease or the Easements as long as all payments and all other monetary charges payable by Company under this Lease are timely paid by the Mortgagee, if any, in accordance with the terms of this Lease and Landowner continues to receive the benefits to which it is entitled under the Power Purchase Agreement. Notwithstanding the foregoing, in the event that this Lease (a) terminates because of Company's uncured Event of Default or (b) is rejected or disaffirmed pursuant to bankruptcy law or any other law affecting creditors' rights, then, so long as a Mortgagee has cured any such monetary Event of Default and is making commercially reasonable efforts to cure any such non-monetary Event of Default as provided herein, Landowner shall, immediately upon written request from such Mortgagee received within ninety (90) days after any such event, without demanding additional consideration therefor, enter into a new easement agreement in favor of such Mortgagee, which new easement agreement shall (i) contain the same covenants, agreements, terms, provisions and limitations as this Lease (except for any requirements that have been

fulfilled by Company prior to such termination, foreclosure, rejection or disaffirmance), (ii) be for a term commencing on the date of such termination, foreclosure, rejection or disaffirmance and continuing for the remaining Lease Term before giving effect to such termination, foreclosure, rejection or disaffirmance, and (iii) enjoy the same priority as the Lease has over any lien, encumbrance or other interest created by Landowner, and, until such time as such new easement agreement is executed and delivered, the Mortgagee may enter, use and enjoy the Easements and the Leased Premises and conduct operations thereon as if this Lease were still in effect. At the option of the Mortgagee, the new easement agreement may be executed by a designee of such Mortgagee, with the Mortgagee assuming the burdens and obligations of Company thereunder. If more than one Mortgagee makes a written request for a new easement agreement pursuant hereto, then the same shall be delivered to the Mortgagee whose lien is senior in priority.

E. Mortgagee Consent. Notwithstanding any provision of this Lease to the contrary, (a) Company shall not agree to a modification or amendment of this Lease if the same could reasonably be expected to materially reduce the rights or remedies of a Mortgagee or impair or reduce the security for its lien, and (b) Landowner shall not accept a surrender of the Leased Premises, the Easements or any part thereof or a termination of this Lease, in each such case without the prior written consent of each Mortgagee (conditioned upon Landowner receiving notice of such Mortgagee as required by Section 7.1 of this Lease, and limited to the information contained in such notice).

ARTICLE 8
CONDEMNATION/FORCE MAJEURE

8.1 **Condemnation.** If eminent domain proceedings are commenced against all or any material portion of the Leased Premises to the extent the taking and proposed use of such property would prevent or adversely affect Company's construction, installation or operation of Solar Facilities on the Leased Premises, the Parties shall either amend this Lease to reflect any necessary relocation and/or modification of the Leased Premises or Solar Facilities which will preserve the value and benefit of the Lease to the Company and Landowner, together with any corresponding payments, or, at either Party's option, this Lease shall terminate in which event neither Party shall have any further obligations hereunder except those which expressly survive the termination of this Lease and Company's obligation to decommission and remove the Solar Facilities and return the Lease Premises to its original condition pursuant to Section 4.5. In the event Company or Landowner is required by a governmental authority pursuant to the Landfill Permit to make changes to the Leased Premises substantially impacting all or a portion of the Solar Facility resulting in a material reduction to the value of the Leased Premises, such changes shall be deemed a condemnation under this Section 8.1 for which no Party shall be entitled to compensation but for which the Parties shall be entitled to amend or terminate this Lease as set forth above.

8.2 **Proceeds.** All payments made by a condemnor on account of a taking by eminent domain shall be the property of the Landowner, except that the Company shall be entitled to any award or amount paid for the reasonable costs of removing or relocating any of the Solar Facilities on the Leased Premises or the loss of any such Solar Facilities or the use of the Leased Premises pursuant to the Lease. Company shall have the right to participate in any condemnation proceedings to this extent.

8.3 **Force Majeure.** A "Force Majeure Event" shall mean, when used in connection with the performance of a Party's obligations under this Agreement, any act or event (to the extent not caused

by such Party or its agents or employees) which is unforeseeable, or being foreseeable, unavoidable and outside the control of the Party which invokes it, and which renders said Party unable to comply totally or partially with its obligations under this Agreement and fits within the following:

(a) war (whether or not war is declared), hostilities, revolution, rebellion, insurrection against any governmental authority, riot, terrorism, acts of a public enemy or other civil disturbance;
(b) acts of God or natural phenomena, including but not limited to, storms (ice, snow and wind) of severity greater than normal weather conditions, floods, lightning, earthquakes, hailstorms, tornados, typhoons, hurricanes, landslides, and fires (and unsafe or hazardous conditions arising from such acts of God or natural phenomena), sabotage or destruction by a third party (other than any contractor retained by or on behalf of the Party) of facilities and equipment relating to the performance by the affected Party of its obligations under this Agreement;
(c) strikes (whether local, regional or national), walkouts, lockouts or similar industrial or labor actions or disputes;
(d) acts or failures to act on the part of any governmental authority that materially restrict or limit Company's access to the Leased Premises and/or Solar Facility;
(e) pandemics or epidemics (including any coronavirus such as COVID-19);
(f) Utility caused delays outside the Company's control and outside the time periods built into the construction schedule (provided that the delays related to Company failing to timely comply with utility requests and procedures shall not be utility delays);
(g) late delivery of equipment required for Work caused outside the Company's control and outside the time periods built in to the construction schedule resulting from a vendor under contract to provide equipment being unable to provide such equipment (provided, however, that the that the delays related to Company failing to timely order and procure such equipment shall not be utility delays); and
(h) state and/or federal governmental incentives are terminated or materially reduced or tariffs are imposed by any governmental authority rendering the Project no longer economically viable.

Notwithstanding the foregoing, Force Majeure shall not include the following events, conditions or circumstances:
(i) shortages of supervisors, labor, or Equipment unless caused by a Force Majeure event; and

(ii) except as otherwise set forth above the late performance as a consequence of any violation of applicable law or decisions of a governmental authority related to the conduct of Company's or any subcontractor's business, including insolvency or any delay related to the economic, commercial or labor circumstances of Company, any subcontractor or other business conducted by Company or any Subcontractor.

Neither Landowner nor Company shall be liable to each other, or be permitted to terminate this Lease, for any failure to perform any obligation under this Lease to the extent such performance is caused by a Force Majeure Event; provided, however, that Landowner may terminate this Lease in the event that the Solar Facility is not producing any energy for more than twelve months following the cessation of a Force Majeure Event, and Company shall thereafter immediately decommission and remove the Solar Facility pursuant to Section 4.5.

ARTICLE 9
DEFAULT

9.1 **Events of Default.** Each of the following shall constitute an Event of Default (herein so called), which shall permit the non-defaulting Party to terminate this Lease or exercise any other remedy specifically set forth in this Lease:

A. Any failure by Company to pay Due Diligence Period Rent, Construction Rent or Operation Rent if the failure to pay continues for sixty (60) days after written notice ("Notice of Default") from Landowner;

B. Any other breach of the Lease by either Party which continues for thirty (30) days after receipt of a Notice of Default from the non-defaulting Party or, if the cure will take longer than thirty (30) days, the length of time reasonably necessary to affect cure as long as the defaulting Party commences such cure within such 30 day period and thereafter diligently prosecutes same to completion;

C. In the event there is no Mortgagee, (1) Company commences a voluntary case under title 11 of the United States Code or the corresponding provisions of any successor laws; (2) anyone commences an involuntary case against the Company under title 11 of the United States Code or the corresponding provisions of any successor laws and either (A) the case is not dismissed by midnight at the end of the 60th day after commencement or (B) the court before which the case is pending issues an order for relief or similar order approving the case; (3) a court of competent jurisdiction appoints, or the Company makes an assignment of all or substantially all of its assets to, a custodian (as that term is defined in title 11 of the United States Code or the corresponding provisions of any successor laws) for the Company or all or substantially all of its assets; or (4) the Company fails generally to pay its debts as they become due (unless those debts are subject to a good-faith dispute as to liability or amount) or acknowledges in writing that it is unable to do so;

D. Any failure of the Solar Facility to provide any amount of electricity to the utility grid for a period of twelve months or more, not due to a casualty event, provided that Company is diligently pursuing restoration of the Solar Facility after such event, or the act or failure to act by Landowner; and

E. Any breach of the Power Purchase Agreement by and between the Parties in connection with the Project which continues for thirty (30) days after receipt of a Notice of Default from the non-defaulting Party.

9.2 In the event of a Company default, the Landowner shall have the right to bring an action for all Rent due and owing through such date of termination of this Lease as well as for liquidated damages in the amount of the net value of the Credits and RECs provided to Landowner under the Power Purchase Agreement during the twelve (12) month period preceding the date of such default.

9.3 **Specific Performance.** Each Party acknowledges and agrees that should the other Party breach any of its obligations hereunder or otherwise fail to permit the other Party to exercise any of the rights and privileges granted herein, such non-defaulting Party shall have the right to seek specific enforcement of this Lease without the requirement to post any bond in connection with such enforcement.

ARTICLE 10
MISCELLANEOUS

10.1 **Notice.** Any notice to be given hereunder or which either Party wishes to give to the other shall be in writing and may be delivered personally to the other or given by overnight delivery service, or by depositing the same in the U.S. Mail with all postage and certification charges thereon prepaid in a sealed envelope and sent by registered or certified mail with return receipt requested, addressed as set forth in the table beneath such Party's name or to such other address as either Party shall hereafter specify by written notice to the other. Any notice shall be deemed delivered three days after deposit in the mail in accordance with the foregoing provision or upon personal delivery. Notices given by a Party's attorney shall have the same force and effect if given by the Party itself.

10.2 **Record.** Contemporaneously with execution of this Lease, Landowner and Company agree to execute a memorandum of this Lease substantially in the form attached hereto as Exhibit " D" (as may be revised to comply with local law or other recording requirements), which may be recorded by Company at Company's sole cost in the county where the Property is situated. Upon the termination or expiration of this Lease, Company, at its sole cost, shall ensure that appropriate termination of lease documentation is recorded.

10.3 **Estoppel**. Within fifteen (15) business days after either Party's written request therefor, the other Party shall deliver a written statement certifying (if such is the case) that this Lease is in full force and effect and has not been assigned, modified, supplemented or amended; that all covenants, conditions and agreements on the part of the Parties hereunder have been performed; and that there are no defenses or offsets to the enforcement of this Lease, or stating those claimed.

10.4 **No Third-Party Beneficiaries.** Except for the rights of the Mortgagees set forth above, no provision of this Lease is intended to nor shall it inure to the benefit of any third party so as to constitute any such person as a third-party beneficiary under this Lease, or of anyone or more of the terms of this Lease, or otherwise give rise to any cause of action in any person not a Party to this Lease.

10.5 **Entire Agreement.** It is mutually understood and agreed that this Lease and the Power Purchase Agreement constitute the entire agreement between Landowner and Company and supersede any and all prior oral or written understandings, representations or statements, and that no understandings, representations or statements, verbal or written, have been made which modify, amend, qualify or affect the terms of this Lease. This Lease may not be amended except in a writing executed by both Parties.

10.6 **Governing Law.** This Lease is made in and shall be governed by the laws of the State of New York.

10.7 **Additional Actions.** Each of the Parties, without further consideration, agrees to execute and deliver such additional documents and take such action as may be reasonably necessary to carry out the purposes and intent of this Lease and to fulfill the obligations of the respective Parties.

10.8 **Waiver.** Neither Party shall be deemed to have waived any provision of this Lease or any remedy available to it unless such waiver is in writing and signed by the Party against whom the waiver would operate. Any waiver at any time by either Party of its rights with respect to any rights arising in connection with this Lease shall not be deemed a waiver with respect to any subsequent or other matter.

10.9 **Relationship of Parties.** The duties, obligations and liabilities of each of the Parties are intended to be several and not joint or collective. This Lease shall not be interpreted or construed to create an association, joint venture, fiduciary relationship or partnership between Landowner and Company or to impose any partnership obligation or liability or any trust or agency obligation or relationship upon either Party. Landowner and Company shall not have any right, power, or authority to enter any agreement or undertaking for, or act on behalf of, or to act or be an agent or representative of, or to otherwise bind, the other Party. The Parties do not intend that this Lease grant to Landowner any rights in Company's Solar Facilities or output therefrom (including capacity, energy, and environmental attributes) or revenue therefrom except as may be expressly set forth herein.

10.10 **Confidentiality.** The Parties acknowledge that during the course of the performance of their respective obligations under this Lease, either Party may need to provide information to the other Party that the disclosing Party deems to be confidential, proprietary or a trade secret. Any such information that is marked confidential, including the terms of this Lease and the Power Purchase Agreement, shall be treated as confidential by the receiving Party and shall not be disclosed to any other person without the prior written consent of the disclosing Party. Notwithstanding the foregoing, such information may be disclosed to such Party's advisors and agents provided such third parties are bound by the confidentiality provisions hereof.

10.11 **Consequential Damages** Neither Party shall be liable to the other Party for incidental, consequential, special, punitive or indirect damages, including but not limited to loss of use or loss of profit or revenue, except as the result of such liable Party's gross negligence, intentional misconduct or fraud.

10.12 **Waiver of Jury Trial.** EACH PARTY HERETO WAIVES, TO THE FULL EXTENT PERMITTED BY LAW, THE RIGHT TO A JURY TRIAL IN ANY LITIGATION CONCERNING THIS LEASE OR ANY DEFENSE, CLAIM, COUNTERCLAIM, CLAIM OF SET-OFF OR SIMILAR CLAIM OF ANY NATURE.

10.13 **Partial Invalidity.** If any term or provision of this Lease is, to any extent, determined by a court of competent jurisdiction to be invalid or unenforceable, the remainder of this Lease shall not be affected thereby, and each remaining term and provision of this Lease shall be valid and enforceable to the fullest extent permitted by law.

10.14 **Counterparts.** This Lease may be executed in two or more counterparts and by different Parties on separate counterparts, all of which shall be considered one and the same agreement and each of which shall be deemed an original. In order to facilitate the agreements contemplated by this Lease, signatures transmitted by facsimile machine or signatures transmitted via e-mail in a "PDF" format may be used in place of original signatures on this Lease. Each Party intends to be bound by such Party's facsimile or "PDF" format signature on this Lease, is aware that the other Parties are relying on such Party's facsimile or "PDF" format signature, and hereby waives any defenses to the enforcement of this Lease based upon the form of signature.

[SIGNATURE PAGE FOLLOWS]

IN WITNESS WHEREOF, the Parties have hereunto set their hands and seals, the corporate parties by their proper officers thereunto duly authorized, as of the day and year last below written.

LANDOWNER:
XEROX CORPORATION

By: _____
John Barrett
Name: _____
Manager - Western Hemisphere Real Estate Portfolio
Date: 11/7/2025

COMPANY:
WEBSTER SOLAR GARDEN LLC

By: _____

Name: Daniel Montante

Date: 11/10/25

EXHIBIT A

THE PROPERTY

(LEASED PREMISES LEGAL DESCRIPTION)

ALL THAT TRACT OR PARCEL OF LAND situate in the Town of Webster, County of Monroe, State of New York, and being part of Lot 6 of the Salt Tract, Section 11, Township 14, Range 4 of the Phelps and Gorham Purchase (so-called), bounded and described as follows:

COMMENCING AT a northwest corner of lands conveyed to Tessy Webster Facility, LLC by deed recorded in the Monroe County Clerk's Office in Liber 12686 of Deeds at Page 125;

RUNNING THENCE: N-45°-05'-16"-E, a distance of 108.28 feet to the Point or Beginning of the parcel herein described;

RUNNING THENCE: N-34°-22'-21"-E, a distance of 63.04 feet to a point;

RUNNING THENCE: N-57°-55'-05"-E, a distance of 75.62 feet to a point;

RUNNING THENCE: N-53°-11'-36"-E, a distance of 78.21 feet to a point;

RUNNING THENCE: N-39°-46'-55"-E, a distance of 412.73 feet to a point;

RUNNING THENCE: N-66°-24'-36"-E, a distance of 138.20 feet to a point;

RUNNING THENCE: N-30°-43'-22"-E, a distance of 105.34 feet to a point;

RUNNING THENCE: N-05°-26'-03"-E, a distance of 456.50 feet to a point;

RUNNING THENCE: N-49°-53'-27"-E, a distance of 240.79 feet to a point;

RUNNING THENCE: S-22°-27'-44"-E, a distance of 156.07 feet to a point;

RUNNING THENCE: S-43°-23'-07"-E, a distance of 400.59 feet to a point;

RUNNING THENCE: S-05°-52'-37"-E, a distance of 185.72 feet to a point;

RUNNING THENCE: S-01°-33'-25"-E, a distance of 596.31 feet to a point;

RUNNING THENCE: S-88°-42'-34"-W, a distance of 681.82 feet to a point;

RUNNING THENCE: N-00°-53'-59"-W, a distance of 28.09 feet to a point;

RUNNING THENCE: S-89°-06'-01"-W, a distance of 522.22 feet to the POINT OR PLACE OF BEGINNING, containing 17.74 Acres, be the same, more or less.

SUBJECT to easements, rights of way and restrictions of record.

BEING AND INTENDED to be a portion of lands conveyed to Xerox Corporation by deed recorded in the Monroe County Clerk's Office in Liber _____ of Deeds at Page _____.

EXHIBIT B

LEASED PREMISES
(TO BE UPDATED PRIOR TO CONSTRUCTION TERM)

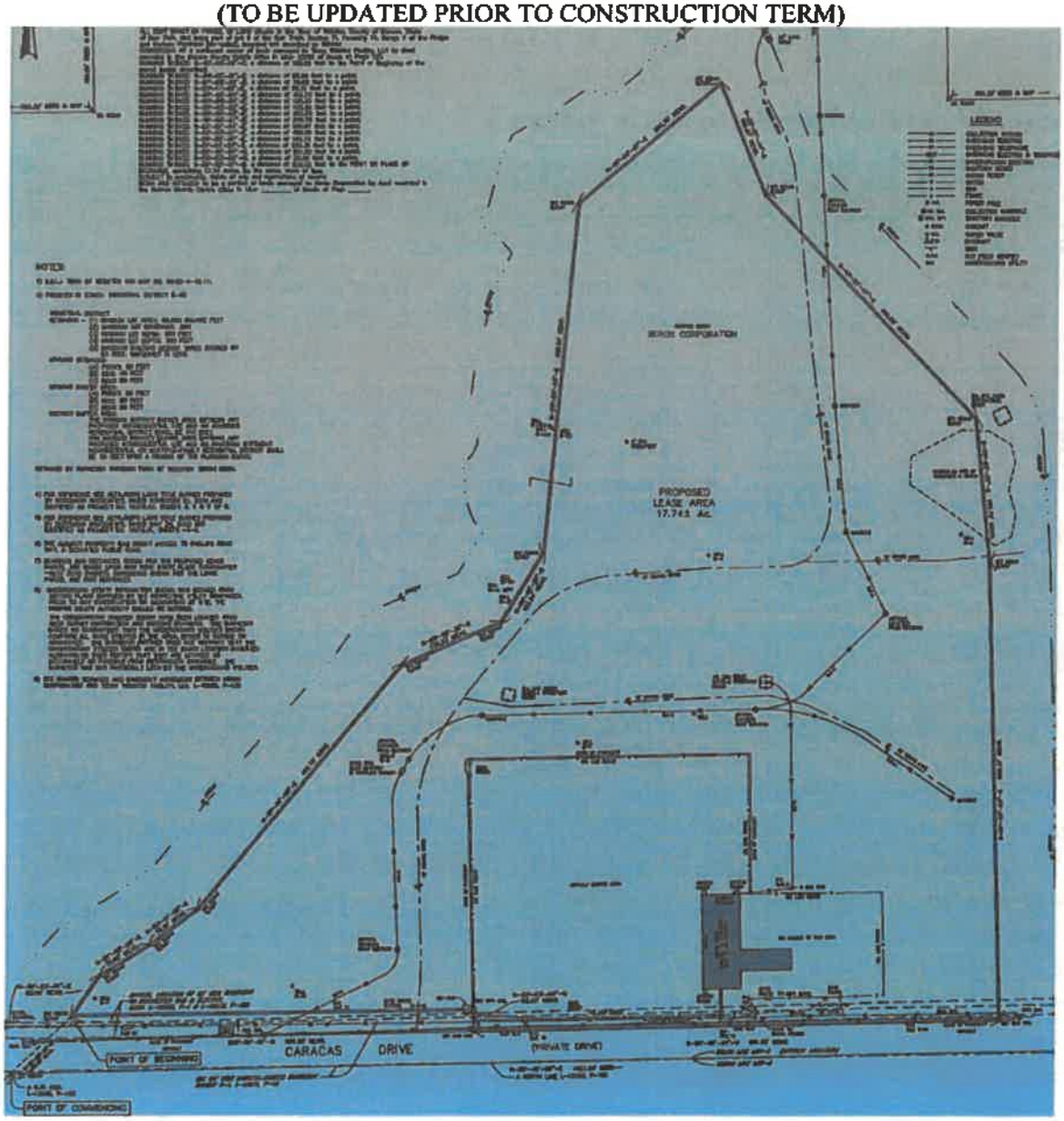


EXHIBIT C

ACCESS EASEMENT
UTILITY EASEMENT
(TO BE UPDATED PRIOR TO CONSTRUCTION TERM)

Approximate Easement Area Outlined in Yellow



EXHIBIT D

MEMORANDUM OF LEASE

MEMORANDUM OF LEASE

NAME AND ADDRESS OF LESSOR:	Xerox Corporation, 800 Phillips Rd., Webster, New York 14580 Mailstop 0111-04-99F
NAME AND ADDRESS OF LESSEE:	Webster Solar Garden LLC 2760 Kenmore Avenue, Tonawanda, NY 14150

DESCRIPTION OF DEMISED PREMISES: See Exhibit "A", attached hereto and incorporated herein.

DATE OF LEASE 11/7/2025

TERM OF LEASE: The Term of the Lease will commence on the Effective Date and continue through the Due Diligence Period, the Construction Term and for a period of twenty-five (25) Lease Years thereafter with two (2) options to extend the Lease for two (2) additional terms of five (5) years each.

OTHER PROVISIONS:

1. Company may assign, sublease, transfer or convey all or part of its interests in this Lease with Landowner's consent, such consent to not be unreasonably withheld or delayed; provided, however, that Company will remain liable for all obligations under this Lease unless the assignee expressly assumes such obligations in writing delivered to Landowner along with evidence reasonably acceptable to Landowner that such assignee has operating capabilities and a net worth at least similar to that of Company and that such assignee has also accepted assignment of the Power Purchase Agreement and is ready, willing and able to perform all of Company's obligations under this Lease and the Power Purchase Agreement. Company shall

provide prior written notice to Landowner of any such assignment, sublease, transfer, or conveyance.

2. Within twelve months after the expiration or earlier termination of the Lease Term (the "Decommissioning Period"), Company shall decommission, dismantle and remove the Solar Facilities and all other property of Company located on the Premises in accordance with the Town of Webster Solar Law, returning the Premises to its condition as of the Effective Date to the extent reasonably practical (reasonable wear and tear, non-Company caused casualty and condemnation excepted) (the "Decommissioning Obligations"). Landowner hereby grants to Company and its successors and assigns a license to enter upon the Leased Premises to perform the activities required to be performed by Company pursuant to this paragraph. No later than the earlier of (i) the date of commencement of the Construction Term, and (ii) the date required by applicable law, Company shall post a bond or letter of credit (the "Decommissioning Bond") as surety for 115% of the estimated gross cost of performing the Decommissioning Obligations, provided, to the extent that any governmental authority requires a decommissioning or restoration bond, letter of credit, cash deposit or other security to secure the Decommissioning Obligations, then Company shall instead comply with the requirements of such governmental authority and this Lease by obtaining a co-obligee Decommissioning Bond in favor of both Landowner and such governmental authority. In such event, Landowner acknowledges and agrees that Company's compliance with such governmental decommissioning and restoration security requirements shall fulfill, replace and satisfy Company's obligation to post the Decommissioning Bond under this Section 4.5. The Decommissioning Bond shall be irrevocable and shall expressly state that it may not be terminated or cancelled and must be held in place by the surety during the Construction Term and Operations Term. Company shall provide Landowner with evidence of continuation or extension of the Decommissioning Bond no less than ten (10) business days prior to the expiration of the then-current term of such bond. Company shall cause the value of the Decommissioning Bond to be increased no less than every ten (10) years during the Term to 115% of the amount determined by Company and verified by an independent third party valuation engineering or consultant, and Owner shall be provided a copy of such valuation no less than thirty (30) days prior to the end of the then-current such ten-year period. The amount of such Decommissioning Bond shall increase by 2% from the last year's value in each year between such valuations. If Company fails to timely perform its Decommissioning Obligations set forth in this Section 4.5, upon ninety (90) days' notice to Company, the Landowner may perform, or cause a third party to perform, the Decommissioning Obligations on Company's behalf and draw upon the Decommissioning Bond to perform, or reimburse such third party for performing, the Decommissioning Obligations. In the event the Company is working to perform its Decommissioning Obligations set forth in this Section 4.5 but such obligations are not complete prior to the Decommissioning Period, then Company shall pay Landowner holdover monthly rent at 150% of the last month's payment made by Landowner under the Power Purchase Agreement prior to such expiration or termination.

3. This memorandum contains only selected portions of the Lease and reference is made to the full Lease, executed on even date herewith, along with any approved amendments, for full terms and

conditions. This memorandum shall not, in any way, amend or supersede the terms and conditions of the Lease, copies of which are kept by the parties.

BALANCE OF PAGE INTENTIONALLY LEFT BLANK

IN WITNESS WHEREOF, the parties hereto have duly executed this Memorandum of Lease this 7th day of NOV 2025.

LESSOR:

XEROX CORPORATION

By: _____

Name: John Barrett

Its: Manager - Western Hemisphere Real Estate Portfolio

Date: 11/7/2025

LESSEE:

WEBSTER SOLAR GARDEN LLC

By: _____

Name: DANIEL MONTANTE

Its: Member

Date: 11/10/25

STATE OF NEW YORK)

SS:

COUNTY OF ERIE)

On the 10th day of November in the year 2025 before me, the undersigned, a notary public in and for said state, personally appeared Daniel Montante, personally known to me or proved to me on the basis of satisfactory evidence to be the individual whose name is subscribed to the within instrument and acknowledged to me that he executed the same in his capacity, and that by his signature on the instrument, the individual or the person upon behalf of which the individual acted, executed the instrument.

Jennifer L. Elmer
Notary Public, State of New York
Registration No. 01EL6005664
Qualified in Niagara County
Commission Expires April 20, 2026

Notary Public

STATE OF NEW YORK)

SS:

COUNTY OF MONROE)

On the 7 day of November in the year 2025 before me, the undersigned, a notary public in and for said state, personally appeared John Barrett, personally known to me or proved to me on the basis of satisfactory evidence to be the individual whose name is subscribed to the within instrument and acknowledged to me that he executed the same in his capacity, and that by his signature on the instrument, the individual or the person upon behalf of which the individual acted, executed the instrument.

TORI MALINAK
Notary Public, State of New York
No. 01MA6109774
Qualified in Monroe County
Commission Expires Jan. 12, 20 28

Notary Public

EXHIBIT E

Intentionally Omitted.

EXHIBIT F

INSURANCE REQUIREMENTS

1. INSURANCE TO BE MAINTAINED BY COMPANY

Prior to taking possession of the Leased Premises or commencing work or services under this Lease, Company, at its own cost and expense, shall procure and maintain insurance for the coverages listed below, written for not less than the limits specified for each coverage or required by law, whichever is greater, (except that if Company procures any policy limits greater than the amounts required herein, then the higher limits shall apply as though stated and required herein) and including the provisions enumerated below:

1.1. Commercial General Liability *(per ISO Form CG 00 01 or equivalent)*

Bodily Injury and Property Damage Limit $1,000,000 each occurrence

Products/Completed Operations Limit 1,000,000 aggregate

Personal Injury & Advertising Injury Limit 1,000,000 each person or organization

General Aggregate 2,000,000 per project

Coverage is to apply on an occurrence basis only. No endorsement or modification of this policy limiting the scope of coverage for Contractual Liability, Products/Completed Operations, Personal Injury, explosion, collapse and underground hazards, or operations of contractors or subcontractors of any tier shall be permitted. In addition, no designated Premises/Operations limitation shall be permitted. The Commercial General Liability coverage is to be maintained for a period of three (3) years after expiration or termination of this Lease and any decommissioning period.

1.2. Automobile Liability *(per ISO Form CA 00 01 or equivalent)*

Owned (if any), Hired and Non-Owned Autos

Combined Single Limit for Bodily Injury & Property Damage Limit $1,000,000 each accident

1.3. Workers' Compensation & Employers' Liability

Statutory coverage complying with law of New York State, with Employers' Liability limits of not less than $500,000 Each Accident and $500,000 Each Employee for Disease and $500,000 Policy Limit for Disease, or the minimum level required by Company's Excess Umbrella Liability insurance company, whichever is greater.

1.4. Excess Umbrella Liability

Combined Single Limit for Bodily Injury & Property Damage $5,000,000 each occurrence

5,000,000 aggregate

Coverage is to apply on an occurrence basis only, in excess of the other Liability coverages required in 1.1, 1.2, and 1.3 above and shall be no more restrictive than such scheduled underlying insurance. The Excess Umbrella Liability coverage is to be maintained for a period of three years (3) after expiration or termination of this Lease and any decommissioning period.

1.5. Pollution Liability

Bodily Injury and Property Damage Limit $2,000,000 each occurrence

Products/Completed Operations Limit 2,000,000 aggregate

General Aggregate 2,000,000 per project

This insurance shall be maintained on an occurrence basis unless otherwise agreed by Landowner and shall be maintained for a period of three (3) years after expiration or termination of this Lease and any decommissioning period or the completion of all work or services involving potential environmental risks. No endorsement or modification of this policy limiting the scope of coverage for Contractual Liability, Products/Completed Operations, explosion, or collapse and underground hazards shall be permitted. No exclusions or limitations that would, in any way, limit or restrict coverage for Company's environmental risks shall be permitted. No designated Premises/Operations limitation shall be permitted.

1.6. Property Insurance

Company and Landowner shall each keep their respective property at the Leased Premises insured against loss or damage by fire, explosion or other cause of loss normally covered by an All Risks or Special Causes of Loss (or equivalent) form of property insurance. During any construction, Company's property insurance shall cover all equipment, construction machinery,

mobile equipment, special equipment, scaffolding, trailers, and tools used or owned by Company and shall also require such property insurance for equipment, construction machinery, mobile equipment, special equipment, scaffolding, trailers, and tools used or owned by any contractor or subcontractor. Landowner shall in no way be responsible or liable for the loss or damage, or loss of use, to any property listed above used or owned by Company or any contractor or subcontractor. Company and Landowner hereby waive their rights, and the rights of their subsidiaries and affiliates, to recover from the other party and their subsidiaries and affiliates for loss or damage to such party's building, equipment, improvements and other property of every kind and description resulting from fire, explosion or other cause of loss to the extent covered in the required form of property insurance policies maintained by the respective parties. Company shall cause contractors and subcontractors to waive their rights of recovery against Landowner for loss or damage to equipment, construction machinery, mobile equipment, special equipment, scaffolding, trailers, and tools used or owned by any contractor or subcontractor.

1.7. Financial Rating of Insurers

The insurance companies providing the required coverages shall be licensed to do so in New York and shall be rated no lower than "A-" by the most recent Best's Key Rating Guide and shall have a Best's Financial Size Category of not less than VIII, unless otherwise agreed to by Landowner.

1.8. Deductibles or Self-Insured Retentions

Deductibles and self-insured retentions shall be permitted with the understanding that (i) Company (and not Landowner) shall be responsible for deductibles and self-insured retentions and (ii) all deductibles and self-insured retentions must be disclosed on the required certificate(s) of insurance. Deductibles and self-insured retentions greater than $50,000 shall require Landowner's approval.

1.9. Additional Insureds

Xerox Corporation and their respective Board Members, Officers, Employees and Agents (as well as such other person(s) or organization(s) as designated by Landowner in writing) (the Additional Insured Parties) shall be named as Additional Insureds under the policies required in 1.1, 1.2, 1.4 and 1.5 and providing coverage for both ongoing and completed operations under 1.1, 1.4 and 1.5. The insurance protection afforded to the Additional Insured Parties under all such policies shall apply on a primary basis and any insurance (or self-insurance program) maintained by the Additional Insured Parties shall not contribute with the insurance afforded to the Additional Insured Parties. The Excess Umbrella Liability policies must provide for what is known as a vertical exhaustion of limits so that all of Company's liability insurance affording the required additional insured status will apply before any insurance or self-insurance maintained by the Additional Insured

Parties is called upon to respond to a claim. This may necessitate an amendment to the Excess Umbrella Liability Policy's *Other Insurance* provision. Follow-form Excess Umbrella Liability policies with their own Other Insurance condition that negates the primary and non-contributory status for the additional insureds will not be accepted.

1.10. Waiver of Subrogation

As to every type and form of insurance coverage required from Company specified in Sections 1.1 through and including 1.6 above, there shall be no right of subrogation against the Additional Insured Parties. To the extent that any of Company's policies of insurance prohibit such a waiver of subrogation, Company shall secure the necessary endorsement(s) to accomplish this waiver.

1.11. Notice of Cancellation, etc.

Company will make every reasonable effort to have each required insurance policy endorsed to provide Landowner with 30 days advance notice of cancellation of coverage by the insurer, except in the case of cancellation or termination due to non-payment of premium, in which case only 10 days advance notice shall be required. Company is hereby further obligated to send by e-mail or fax Landowner a copy of any cancellation, material reduction in coverage or non-renewal notice received from the insurer for any policy affording the coverages required herein within five (5) days of Company's receipt of same. Company further agrees to provide Landowner with 30 days advance written notice of cancellation, non-renewal or material reduction in coverage initiated by Company with respect to any of the required insurance coverages. Notices to Landowner hereunder must be e-mailed to Manager, Western Hemisphere Real Estate Portfolio, Xerox Corporation – Global Real Estate & Facilities, 800 Phillip Road, Webster, New York 14580.

1.12. General Provisions

1.12.1. No Limitation. The types and limits of insurance to be purchased and maintained by Company pursuant to this Lease shall not be deemed to constitute a limitation of Company's liability or indemnification obligations hereunder.

1.12.2. Purchase of Insurance. If Company fails to purchase, maintain, and continue in force insurance in the types and with limits of liability required by this Lease, Landowner may purchase such insurance and the cost thereof shall be borne by Company.

1.12.3. Notice of Occurrence. Upon Company's knowledge of any occurrence, event, or claim that may reasonably exceed $100,000, or be expected to exceed $100,000, or which may reduce or materially affect the aggregate amount of insurance coverage available pursuant to this Lease, Company shall (a) immediately provide Landowner with written notice of such occurrence,

event or third-party claim(s) with reasonable detail, and (b) to the extent reasonably commercially available, promptly obtain replacement insurance for the eroded aggregate limit and provide Landowner with evidence thereof.

1.12.4. Claims-Made Coverage. For any liability coverages maintained on a claims-made basis, the following provisions apply unless otherwise agreed to by Landowner:

1.12.4.1. If the claims-made coverage terms designate a specific retroactive date, Company shall maintain a retroactive date which is not later than the date of the commencement of the Lease Term for each and every coverage provided on a claims-made basis;

1.12.4.2. Coverage must be maintained for the duration of this Lease and any decommissioning period and for a period of not less than three (3) years after the expiration or termination of this Lease and any decommissioning period (the Extended Term of Protection) without any advancement of the retroactive date or lapse of coverage. If coverage is cancelled or non-renewed before the Work is completed or during the Extended Term of Protection, Company must purchase an extended reporting period ("tail coverage") of three (3) years after the cancellation or non-renewal of coverage and secure replacement coverage that will not result in any gaps in coverage through the remainder of this Lease and the Extended Term of Protection;

1.12.4.3. At the conclusion of the Extended Term of Protection, Company shall, at its own cost, (i) purchase a three (3) year extended reporting period endorsement ("tail" coverage) or (ii) the existing claims-made coverage must be continued for an additional three (3) years without any lapse, interruption or advancement of the retroactive date. Company will provide any documentation reasonably requested by Landowner to confirm that no gap in coverage has occurred.

1.12.5. Adequacy of Insurance. Landowner does not in any way represent that the insurance specified herein, whether in scope of coverage or limits of coverage, is adequate or sufficient to protect the business or interest of Company.

1.13. Evidence of Insurance

Company shall deliver to Landowner, prior to taking possession of the Leased Premises or commencing work or services under this Lease, including any visitation of the Property during the Due Diligence Period, Certificates of Insurance acceptable to Landowner certifying that policies of insurance for the required coverages have been issued and are in effect and comply with the requirements herein. Upon expiration or cancellation of any policy during the period the coverages under such policy are required to be maintained, Company shall immediately deliver to Landowner a Certificate of Insurance evidencing proper renewal or replacement of the policy.

1.13.1 The NAIC Identification Number must be shown on the certificate after the name of each insurance company to allow accurate identification of the insurer(s).

1.13.2 The Contract Name must be clearly stated on the Certificate of Insurance.

1.13.3 Certificates must show the full limits of coverage provided under each required policy of insurance without regard to the minimum limits of insurance required herein.

1.13.4 Certificates must show the amount of any deductibles or self-insured retentions applicable to the required coverages.

1.13.5 Required waivers of subrogation must be shown on certificates.

1.13.6 Certificates evidencing Liability coverage under which Landowner is required to be named as an Additional Insured must clearly indicate that the Additional Insured Parties (as required herein) are included as Additional Insureds on a primary and non-contributory basis. The certificate must specify the policies under which such Additional Insured status has been granted and a copy of the Additional Insured Endorsement(s) or Policy Provision(s) that grant(s) the required Additional Insured status must be attached to the certificate. If such endorsements or policy provisions do not reflect the corresponding policy number, the certificate of insurance must specifically list the attached endorsements or policy provisions by form number and/or name.

1.13.7 For any coverage maintained on a claims-made basis, the certificate must reflect the Retroactive Date or specify that full prior acts coverage applies.

1.13.8 All Exclusionary or Limiting Endorsements attached to the required Commercial General Liability policy must be disclosed on the required certificate and, upon request, Company will provide a copy of any such endorsements to Landowner.

1.13.9 Failure of Landowner to demand such Certificate of Insurance or failure of Landowner to identify a deficiency in a certificate that is provided shall not be construed as a waiver of Company's obligation to maintain such insurance.

1.13.10 Landowner shall have the right, but not the obligation, to prohibit Company from entering the Leased Premises until such certificate indicating full compliance with the requirements herein has been received and approved by Landowner.

1.13.11 Certificates of Insurance shall be sent to Xerox Corporation, Manager, Assessment & Environmental Operations, Xerox Corporation, 800 Phillip Road, Webster, New York 14580 (and emailed to Marcus.Lathrop@xerox.com).

1.13.12 Company shall provide copies of any insurance policies required herein within 10 days of Landowner's written request for same. The copy of the policy or policies provided shall contain all applicable conditions, definitions, exclusions, forms and endorsements.

2. CONTRACTORS AND SUB-CONTRACTORS OF ANY TIER

All contractors and sub-contractors of Company, while providing work or services at the Leased Premises, including during the Due Diligence Period, shall maintain the insurance coverages outlined above and comply with all requirements set forth above, including the furnishing of separate insurance certificates and endorsements prior to said contractors or sub-contractors entering the

Leased Premises or commencing work, unless otherwise agreed to by Landowner. It is recognized that the nature of some work performed by a contractor or sub-contractor may warrant a waiver of some of the insurance requirements outlined above or the imposition of additional requirements beyond those required above. Such changes in the requirements are subject to review and approval by Landowner.

Company agrees that it will contractually obligate its contractors and sub-contractors to advise Company promptly of any changes or lapses in the requisite insurance coverages and Company agrees to promptly advise Landowner of any such notices Company receives from its contractors or subcontractors. Company assumes all responsibility for monitoring contractors' and sub-contractors' insurance documentation for compliance with the insurance and other provisions of this Lease.

Upon written request of Landowner, Company's contractors and sub-contractors must furnish separate insurance certificates to Landowner (per 1.13 above) with copies of the required endorsements prior to said contractors or sub-contractors entering the Leased Premises or commencing work.

Exhibit G

Competitor List

HP Inc.

Canon

Epson

Kyocera

Brother

Ricoh

Toshiba