

26 April 2019

Mr. Bryan Kelly GO Broome, LLC c/o Gotham 432 Park Avenue South, 2<sup>nd</sup> Floor New York, NY 10016

RE: Preliminary Geotechnical Investigation Report

**Proposed GO Broome Development** 

50 and 60 Norfolk Street (Block 346, Tax Lots 37 and 75)

New York, New York

Langan Project No.: 100646803

### Dear Bryan:

We have completed our preliminary geotechnical investigation for the proposed GO Broome Development project located at 50 and 60 Norfolk Street in New York, NY. The purposes of this investigation were to: 1) research and review available site and neighboring building information; 2) obtain subsurface information by drilling borings at the site; and 3) provide preliminary recommendations for foundation design and other geotechnical aspects of construction for the proposed development. This report has been prepared in accordance with our 3 March 2017 (last revised 22 February 2019) proposal and your subsequent authorization. No environmental investigation was performed as part of this work.

All elevations given in this report are referenced to the North American Vertical Datum of 1988 (NAVD88), unless otherwise noted.

### PROJECT DESCRIPTION

### **Existing Site Conditions**

The approximate 32,401 ft² project site is located at Block 346, Lots 37 and 75 in New York, New York. The site is currently occupied by an asphalt-paved parking lot and a community yard and outdoor garden area on Lot 75; see Figures 1 and 2. The remains of the former 1-story Beth Hamedrash Hagodol (BHH) Synagogue that was damaged in a fire in May 2017 is located on Lot 37. The site is bordered by Broome Street to the north, Suffolk Street to the east, the 5-story 384 Grand Street building and the 14-story 50 Norfolk Street Hong Ning residential building to the south, and Norfolk Street to the west. The BHH Synagogue is a NYC Landmark-designated building.

Based on the 29 December 2017, last revised 14 May 2018, Topographic, Boundary, and Utility Survey prepared by Langan:

- Site grades are generally level and range from approximate el 31 to el 34.
- Broome Street sidewalk grades adjacent to the site range from approximate el 31.5 to el 32.6, from east to west.
- Suffolk Street sidewalk grades adjacent to the site range from approximate el 31.5 to el 33.6, from north to south.
- Norfolk Street sidewalk grades adjacent to the site range from approximate el 32.6 to el 33.7, from north to south.

### **Proposed Development**

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Based on the 8 June 2018 65% Schematic Design architectural drawing set prepared by Dattner Architects, we understand that the proposed development scheme consists of constructing:

- A new 12-to-16-story mid-rise senior housing building with one cellar level in the west portion of the site adjacent to Norfolk Street. We understand that stabilized portions of the BHH Synagogue remnants are to remain in place and will be incorporated into the new development;
- A new 3-to-30-story low-to-high-rise residential building with one cellar level and retail space at the ground floor level in the north and east portions of the site adjacent to Broome Street and Suffolk Street.

We understand that the development will have a combined footprint area of approximately 25,275 ft<sup>2</sup>. At the time of this report, preliminary structural loads provided by DeSimone Consulting Engineers for the proposed development are as follows:

• Low- to mid-rise interior columns: 1,000 kips

Low- to mid-rise exterior columns: 700 kips

• High-rise interior columns: 2,800 kips

High-rise exterior columns: 1,800 kips

### SUBSURFACE INVESTIGATION

### **Review of Available Information**

We reviewed historical Sanborn maps, available New York City Department of Buildings (NYC DOB) information, NYC Landmarks information, emergency stabilization drawings for



the BHH synagogue, surficial and regional geologic bedrock information, and available flood data for the site vicinity. Pertinent information from the above documents is summarized in the following paragraphs.

### Historical Sanborn Maps

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We reviewed available historical Sanborn Maps dated 1894, 1905, 1913, 1922, 1950, 1968, 1976 through 1980, 1983, 1985, 1987, 1988, 1990, 1992 through 1996, and 2001 through 2005; see Appendix A.

- 1894 Sanborn Map: The 1894 Sanborn Map depicts the project site as being occupied by a Synagogue building in the west portion of the site, a Baptist Church building in the southeast portion of the site, and multiple three- to five-story buildings with yards in the north and east portions of the site. The map depicts the site as bordered by Broome Street to the north, Suffolk Street to the east, multiple two- to six-story buildings to the south, and Norfolk Street to the west.
- 1905 Sanborn Map: The 1905 Sanborn Map depicts additional free-standing one-story backyard structures in the central portion of the site. The map also identifies that the majority of the on-site buildings have a basement level. In addition, the map identifies the Synagogue building as a one-story building with a basement and the Baptist Church building as a one- to three-story building with a basement, and depicts the use of furnaces and gas lights in each of the buildings. The map also shows basement levels in the buildings that border the site to the south, along Norfolk Street.
- <u>1913 Sanborn Map</u>: The 1913 Sanborn Map no longer shows the free-standing one-story backyard structures in the central portion of the site.
- 1922 Sanborn Map: The 1922 Sanborn Map shows the Synagogue building as a one- to two-story building with a basement and shows a one- to two-story "N.Y. Post Office Station B" building in the place of the former Baptist Church building. The New York Post Office building is shown to have a basement.
- 1950 Sanborn Map: The 1950 Sanborn Map no longer shows one of the former five-story buildings in the northern-central portion of the site and no longer identifies the New York Post Office building as such, now identifying the building as flats with an elevator. In addition, the map no longer shows one of the former five-story buildings that bordered the site to the south, along Norfolk Street, and no longer shows one of the former four-story buildings, a portion of which was partially located within the southern portion of the site.
- <u>1968 and 1976 Sanborn Maps</u>: The 1968 Sanborn Map no longer shows one of the former 5-story buildings within the northwest portion of the site. The 1976 Sanborn Map no longer shows the remaining former 5-story buildings that occupied the northwest portion of the site and shows the northern-central portion of the site as being used for



parking. The 1976 Sanborn Map also no longer shows some of the five- to six-story buildings that bordered the site to the south. The 1968 and 1976 maps indicate that many buildings on the block/adjacent blocks have been removed or have become vacant in this time period.

- <u>1977 Sanborn Map</u>: The 1977 Sanborn Map no longer shows the former New York Post Office building and shows parking in place of the former 5-story building that bordered the site to the south, along Norfolk Street.
- <u>1978 and 1979 Sanborn Maps</u>: The 1978 and 1979 Sanborn Maps show no significant changes to the site relative to the 1977 Sanborn Map.
- 1980 Sanborn Map: The 1980 Sanborn Map no longer shows the former three- to five-story buildings in the east portion of the site, showing the Synagogue building and the five-story building in the northeast corner of the site as the only two remaining onsite buildings.
- 1983 Sanborn Map: The 1983 Sanborn Map shows the Synagogue building as the only building on the site and shows the previous parking/vacant building area bordering the site to the south, along Norfolk Street, as being occupied by a 14-story building identified as "Hong Ming Housing for the Elderly" that was built in 1982.
- 1985, 1987, and 1988 Sanborn Maps: The 1985, 1987, and 1988 Sanborn Maps show no significant changes to the site relative to the 1983 Sanborn Map.
- <u>1990 Sanborn Map</u>: The 1990 Sanborn Map shows the northwest corner of the site as an additional parking area.
- 1992 through 1996 and 2001 through 2005 Sanborn Maps: The 1992 through 1996 and 2001 through 2005 Sanborn Maps show few changes to the site relative to the 1990 Sanborn Map, with the exception of an additional area in the central area of the site shown as a parking area starting with the 2003 Sanborn Map.

### New York City Department of Buildings (NYC DOB) Search

We reviewed available structural and foundation-related information obtained from the NYC DOB for the on-site structures and neighboring buildings; see Appendix B for select records and building plans.

• 50 Norfolk Street "Hong Ning" Building (Block 346, Lot 1): Based on our records search and an undated Certificate of Occupancy for property, a 14-story building was completed in 1983; the Certificate of Occupancy does not list any below-grade levels.

According to a 16 June 1982 "Site Plan, Site Symbols, Planting Schedule, Zoning" drawing prepared for the Chinatown Planning Council, the proposed 1<sup>st</sup> floor elevation of the 14-story residential building was el 32.61; the elevation datum is not specified on



the drawing. This drawing shows the block in a configuration that is similar to the present configuration, however different block and lot designations are assigned.

According to a 13 January 2005 "Low Pressure Oil Filled Boiler" drawing prepared by New York Engineering Associates, P.C., two boilers are located in the central portion of the ground floor of the building.

According to a 25 January 2005 drawing set prepared by New York Engineering Associates, P.C., the boiler room is shown to be within a cellar level and a buried 10,000 gallon tank is identified to the east of the building; this tank is indicated to be abandoned in place. The drawing set includes details for a new fuel oil tank located at the northeast corner of the building, adjacent to the BHH Synagogue.

A 30 October 2006 "Oil Tank Foundation Plan & Details" drawing prepared by Anthony Gennaro, P.E. shows an oil tank foundation in the same location as that specified on the 25 January 2005 drawings described above. The oil tank foundation specified in the 30 October 2006 drawing is supported by 3-inch-diameter helical piles, each having a capacity of 60 kips. The 30 October 2006 drawing shows the helical piles as being installed within 12-inch sonotube forms that extended approximately 4-feet-4-inches below existing grade, and shows the foundation slab as 1-foot-6-inches thick and comprised of 5,000 psi concrete.

No other structural or foundation-related information regarding this building was available at the time of our search.

- <u>60 Norfolk Street, BHH Synagogue (Block 346, Lots 37 and 75) (Project Site)</u>: No structural or foundation-related information was available at the time of our search.
- <u>384 Grand Street building (Block 346, Lot 95)</u>: Based on our records search and a 17 June 1987 Certificate of Occupancy, the building is 5-stories with one cellar level.

According to a 12 April 2000 "Roof Plan & Equipment Room" drawing prepared by Arcnet Architects, the cellar does not occupy the entire footprint area of the building and includes a light well that is located 11-feet below the Grand Street sidewalk grade. The depth of the cellar floor slab is not specified on the drawing.

According to 17 August 2006 drawings prepared by Glen Coben Architecture + Design P.L.L.C., existing "waste" is shown beneath the cellar floor slab, and implementation of new grease traps are shown at cellar level. The gas and sanitary riser diagrams identify the cellar level as the lowest below grade level within the existing building.

No other structural or foundation-related information regarding this building was available at the time of our search.



### NYC Landmarks Information

Based on current mapping data made available on the NYC Landmarks Preservation Commission website, the BHH Synagogue is a NYC Landmark-designated building. No other Landmarks are located within 90 feet of the project site.

### BHH Synagogue Emergency Stabilization Drawings

We reviewed the Emergency Stabilization Drawings prepared by Ancora Engineering for the BHH Synagogue, dated 05 March 2019; see Appendix C. According to the drawings, portions of the synagogue's perimeter brick walls and tower walls, stairways, concrete slab-on-grade, and below-grade footings and foundations are to be left in place, and the top of the stabilized walls are to be waterproofed. The current extents of walls and slabs to remain in place are as follows, generally including walls along the west, east, and south sides of the synagogue and slabs occupying the interior of the synagogue:

- Along the west elevation of the synagogue, along Norfolk Street, the remnants will be stabilized to configure a northwest tower wall 11-feet 6-inches tall and 13-feet 9inches wide, followed by the main entrance stairway 7-feet 11-inches tall and 31feet 10-inches wide, completed by a southwest tower wall 36-feet 6-inches tall and between 13-feet 9-inches and 13-feet 10-inches wide. Shoring and bracing of the southwest tower are to be filed under a separate set of drawings.
- Along the east elevation of the synagogue, parallel to Suffolk Street, the remnants will be stabilized to configure a centralized brick wall that is 6-feet tall and 57-feet 7inches wide.
- Along the south elevation of the synagogue, parallel to Grand Street, the remnants will be stabilized to configure a southwest tower wall that is 36-feet 6-inches tall and between 13-feet 4-inches and 13-feet 5-inches wide, followed by a brick wall that is between 6-feet and 13-feet 10-inches tall and extends to near the eastern property line (width not specified). Shoring and bracing of the southwest tower are to be filed under a separate set of drawings.
- Along the north elevation of the synagogue, parallel to Broome Street, the remnants will be stabilized to configure a northwest tower wall that is 11-feet 6-inches tall (width not specified) and a section of brick wall near the eastern property line that is 6-feet tall (width not specified).
- The cellar concrete slab-on-grade will be cracked/perforated to permit drainage, and berms (1V:3H to 1V:4H) will be created from the cellar slab up to existing grade,



behind the stabilized remnants, where present. The backfill material for the berm is to consist of concrete/masonry demolition debris and/or clean soil.

• The top of the lowest level slab of the BHH synagogue appears to be located approximately 2 feet below adjacent Norfolk Street sidewalk grade.

We understand that you are currently in discussions to allow for the final stabilized height of the southwest BHH Synagogue tower to be lowered to 11-feet 6-inches tall to match the height of the northwest tower.

### Surficial Geology

We reviewed the Surficial Geologic Map of New York (Lower Hudson Sheet) dated 1991 and published by the New York State Geological Survey. The surficial geologic map indicates that soils beneath the project site consist of till of variable textures. A nearby region of lacustrine sand deposits is shown to the north and west of the site; see Figure 3.

We also reviewed the Sanitary and Topographical Map of the City and Island of New York prepared for the Council of Hygiene and Public Health by Egbert L. Viele, dated 1865. This map shows that the project site is located within an area historically characterized as meadow land; see Figure 4.

### Bedrock Geology

We reviewed the Bedrock and Engineering Map of New York County by Charles A. Baskerville, dated 1994. The bedrock geology map indicates that bedrock beneath the site is part of the Ravenswood Granodiorite formation and consists of medium- to dark-gray sillimanite-garnet-pink microcline-plagioclase-biotite-muscovite-quartz and/or biotite-hornblende-orthoclase layered gneiss. This map shows Cameron's fault line nearby to the west of the site, where the bedrock transitions to Manhattan Schist, which consists of gray, medium- to coarse-grained, layered sillimanite-muscovite-biotite-kyanite schist and gneiss interlayered with layered tourmaline-garnet-plagioclase-biotite-quartz schist and gneiss with black amphibolite layers 3-feet or more thick; see Figure 5.

We also reviewed the Rock Data Map of Manhattan prepared by the Office of the President Borough of Manhattan Topographical Bureau, Third Edition, dated 1944. The map indicates that the top of bedrock at the core boring located closest to the project site (about 300 feet southwest of the project site's southern border) was found to be located at el -133 MBPD (el -131.4 NAVD88), or approximately 164 feet below existing sidewalk grade in the vicinity of the project site.



### Flood Data Research

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According to the Federal Emergency Management Agency (FEMA) Preliminary Flood Insurance Rate Map (FIRM) Number 3604970203G, dated 5 December 2013, the project site lies outside of the 1% and 0.2% annual chance floodplains; see Figure 6.

### Field Subsurface Investigation

The geotechnical field investigation for this study consisted of drilling eight borings identified as LB-2, LB-4 through LB-8, LB-11, and LB-12. The borings were performed at accessible site areas within the existing asphalt-paved parking area. Proposed borings LB-1, LB-3, LB-9, and LB-10 were not accessible at the time of our investigation work and are to be performed at a late date once access to these locations can be provided; see Figure 2.

The borings were completed under the full-time Special Inspection of a field engineer from Langan and under the direct supervision of our project Professional Engineer. Our field engineer marked out the boring locations in the field, maintained logs of the explorations, classified soil, assigned NYC Building Code material classifications, and obtained representative material samples. Boring logs are provided in Appendix D and generalized subsurface profiles A-A and B-B are provided in Figure 7. Permission to access the site was obtained from the Chinese American Planning Council (CPC) prior to performing our work.

The borings for this study were drilled by Craig Geotechnical Drilling Co. between 8 and 14 February 2019. The borings were performed within the existing asphalt-paved parking area using truck-mounted drilling equipment. The borings were advanced using mud rotary drilling techniques to depths ranging from 77 feet to 102 feet below existing grades.

A standard 2-inch-outer-diameter split spoon sampler was used to obtain samples of the underlying soil strata. The Standard Penetration Test (SPT)<sup>1</sup> was accomplished as part of the sampling procedure in accordance with ASTM D1586, and the SPT results were recorded by our inspecting engineer. As part of the SPT test, a safety hammer was used; in boring LB-8, an automatic hammer was used to advance the split spoon sampler between ground level and 77 feet below grade due to rainy weather conditions at the time of sampling. SPT soil samples were obtained to the termination depth of each boring. Soil drilling was advanced using a 3-7/8-inch tri-cone roller bit. All boreholes were backfilled with soil cuttings and patched with asphalt upon completion.

<sup>&</sup>lt;sup>1</sup> The Standard Penetration Test (SPT) is a measure of the soil density and consistency. The SPT N-value is defined as the number of blows required to drive a 2-inch O.D. split-barrel sampler 12 inches, after an initial penetration of 6 inches using a 140 pound hammer falling freely for 30 inches.



### **Laboratory Testing**

Selected soil samples were transmitted to a specialty testing laboratory where the following laboratory tests were performed:

- 9 Natural Water Content Analyses (ASTM D2216)
- 6 Grain Size Distribution Analyses (ASTM D422)
- 3 Atterberg Limit Analyses (ASTM (D4318)

A summary of the laboratory tests performed and their results are included below and in Appendix E.

### SUBSURFACE CONDITIONS

The site subsurface conditions encountered in the borings generally consist of surface materials overlying successive layers of fill, upper sand, silt, lower sand, and clay. The following sections describe the encountered subsurface and groundwater conditions.

### **Surface Materials**

An approximately 1- to 3-inch thick layer of asphalt paving was encountered at the surface in all borings.

### Fill

A fill layer consisting of gray to brown sand with varying proportions of gravel, silt, and construction debris (red brick and asphalt) was encountered directly below the surface materials in all borings. The fill was observed to be approximately 10 feet to 16 feet thick. The fill was found to be very loose to very dense as evidenced by SPT N-values ranging from 2 blows/foot to refusal (over 100 blows/foot), with an average SPT N-value of 24 blows/foot, and is classified as NYC Building Code Class 7 material.

### **Upper Sand**

A stratum consisting of brown sand with varying amounts of gravel and silt was encountered below the fill layer in all borings. The stratum was first encountered at depths ranging from approximately 10 feet to 16 feet below existing grades, corresponding to approximate el 15 to el 23, and was observed to be approximately 9 feet to 25 feet thick. In boring LB-8, an approximately 5-foot thick zone of brown silt was encountered below the fill layer and above the sand at approximately 16 feet below existing grade, corresponding to approximate el 16.5. The upper sand stratum was found to be loose to dense as evidenced by SPT N-values ranging from 6 blows/foot to 55 blows/foot, with an



average SPT N-value of 18 blows/foot, and is generally classified as NYC Building Code Class 6, Class 3b, and Class 3a material.

Based on laboratory test results for samples obtained from the upper sand stratum, water contents range from 23.2% to 26.3%.

### Silt

A stratum consisting of brown to gray silt with varying proportions of gravel, sand and clay was encountered below the upper sand stratum in all borings. The silt was first encountered at depths ranging from approximately 25 feet to 35 feet below existing grades, corresponding to approximate el -3 to el 7.5, and was observed to be approximately 20 feet to 60 feet thick. In borings LB-2 and LB-8, an approximately 5- to 10-foot thick zone of sand with varying amounts of silt was encountered interbedded within the silt at approximately 50 feet to 80 feet below existing grades, corresponding to approximate el -47.5 to el -17. The silt stratum was found to be loose to dense as evidenced by SPT N-values ranging from 7 blows/foot to refusal (over 100 blows/foot), with an average SPT N-value of 27 blows/foot, and is generally classified as NYC Building Code Class 6, Class 5b and Class 5a material.

Based on laboratory test results for samples obtained from the silt stratum, water contents range from 28.7% to 30.8%, liquid limit (LL) ranges from 23 to 26, plastic limit (PL) ranges from 20 to 21, and plasticity index (PI) ranges from 3 to 5.

### **Lower Sand**

A stratum consisting of brown to gray sand with varying proportions of gravel and silt was encountered below the silt stratum in all borings. The sand was first encountered at depths ranging from approximately 45 feet to 95 feet below existing grades, corresponding to approximate el -62.5 to el -12.5, and was encountered to the termination depth of all borings except LB-12, where gray clay was encountered at a depth of 95 feet below existing grade and extended to the termination depth of the boring. The lower sand stratum was found to be medium dense to dense as evidenced by SPT N-values ranging from 20 blows/foot to refusal (over 100 blows/foot), with an average SPT N-value of 67 blows/foot, and is generally classified as NYC Building Code Class 3b, Class 3a, and Class 2a material. The clay encountered in boring LB-12 was found to be hard as evidenced by SPT N-values of 62 blows/foot to refusal (over 100 blows/foot) and is classified as NYC Building Code Class 4a material.

Based on laboratory test results for samples obtained from the lower sand stratum, water content ranges from 20.6% to 22.5%.



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### Groundwater

Groundwater was measured when first encountered in each boring and was also measured in temporary groundwater level observation wells during the January 2019 Langan Phase II Environmental Investigation at the site. Groundwater was observed to be approximately 26.5 feet to 29 feet below existing site grades, or at approximate el 2.5 to el. 7.

### PRELIMINARY FOUNDATION RECOMMENDATIONS

Based on the results of our subsurface investigation, natural sand and silt soils were encountered below the fill materials at the site. Where new building loads are relatively low, primarily within the low-to mid-rise portions of the proposed buildings, we anticipate that shallow foundations bearing on natural upper sand soils could be utilized.

In areas where new building loads are relatively high within the proposed high-rise building, or in other areas of the development where higher capacity elements are needed than can be achieved using shallow foundations, deep foundations consisting of driven piles or drilled micropiles deriving their support within the natural silt and lower sand soils can be utilized.

The following sections provide our recommendations for the foundation system, seismic design parameters, below-grade walls and floor slabs, groundwater control, soil excavation, temporary excavation support, and other geotechnical aspects of the proposed construction.

For the purposes of the preliminary recommendations provided in this report, we have assumed that the cellar level floor slab would be located 16 feet below existing grade, or at approximate el 16, and that general excavations for the proposed cellar level would need to extend deeper to reach proposed foundation subgrades.

### **Shallow Foundations**

The NYC Building Code Class 3b sand located below the anticipated foundation level is satisfactory bearing material that can be used for foundation support. The allowable soil bearing pressure for design of shallow foundations bearing on the above referenced natural sand soils or approved, compacted fill is 3 tons per square foot (tsf).

Individual spread footings should be at least 3 feet wide and continuous wall footings should be at least 2 feet wide. Lateral loads can be resisted by shear resistance developed at the base of the footings; a static friction coefficient of 0.25 can be used to calculate sliding resistance.



If portions of the fill layer are locally encountered at a new footing elevation, then the excavation will have to be deepened to reach the natural sand soil. In this case, the footings may need to be lowered to reach suitable bearing material.

New shallow foundations constructed adjacent to existing neighboring building foundations should bear at the same elevations as adjacent footings. If new foundations are anticipated to bear at lower elevations than adjacent existing foundations, existing foundations should be underpinned to the depth of the new foundations, if the adjacent existing foundations are above a 1H:1V theoretical influence line extending upwards from the base of the proposed below-grade footings.

In accordance with the NYC Building Code, Special Inspection of the foundation subgrades should be performed to verify the soil is suitable to provide the design bearing pressure. Bearing surfaces should be level and free of debris and standing water. After the bearing surface has been approved, it should be protected until the footing is poured.

### **Deep Foundations**

### Concrete-Filled Pipe Piles

As previously discussed, in areas where higher capacity elements are needed, the proposed building can be supported on closed-end pipe piles driven into the silt and lower sand and filled with concrete. We preliminarily expect that a 14-inch or 16-inch-dia Grade 3 (Modified 50 ksi) steel closed-end pipe pile having a 0.5 inch wall thickness driven into the lower sand and filled with concrete having a 28-day compressive strength of 4,000 lbs/in² could be suitable pile types to achieve a 150 ton compression design capacity. If available, more economical and generally equivalent 13.375-inch-dia by 0.48-inch wall thickness, or 16-inch-dia by 0.5-inch wall thickness, new secondary oil casing pipe piles can be utilized as a substitute for the 14-inch-dia and 16-inch dia piles identified above. We preliminarily anticipate the 14-inch-dia or 16-inch-dia piles or equivalent oil casing pipe piles can sustain an uplift design capacity of 15 tons. The basic allowable lateral load capacity for the driven pile is 2 kips.

At this time, we understand that raising site grades is not anticipated to be required. If the proposed development scheme will include raising site grades, the individual pile compression capacity given above will need to be reviewed to include an allowance for downdrag.

Based on the available information and assuming that piles would be installed from approximately 15 feet below existing sidewalk grade, we anticipate pile lengths may vary from about 60 feet to 75 feet at the site. We recommend that a minimum 1-1/4-inch-thick



end plate be used in an effort to prevent damage to the pile toe during driving. The minimum pile center-to-center spacing should be 3 feet for a 14-inch-dia pile and 3.5 feet for a 16-inch dia pile. Piles battered at 6V:1H can be used to resist lateral loads, if necessary.

Index piles and pile load tests are necessary to confirm and optimize the final pile capacity, pile diameter, wall thickness, length, and final driving criteria prior to the start of construction. In accordance with the provisions of the NYC Building Code, we recommend that two full-scale compression load tests be performed on each pile type/capacity, and one uplift and lateral load test be performed on each pile type/capacity. The load testing should be done as soon as possible so that modifications, if necessary, can be made prior to production pile installation.

The index piles should be driven at locations selected by the Geotechnical Engineer and should be the same in every respect to the production piles. The index piles to be load tested should also be selected by the Geotechnical Engineer. Dynamic testing of the index piles using the Pile Driving Analyzer (PDA) should be performed to measure pile stresses during driving, to estimate pile capacities, and to finalize driving criteria. Frictional resistance provided by the fill should be discounted when determining the termination driving criteria.

We recommend that driven piles not be utilized within 25 feet of the existing BHH Synagogue remnants, the adjacent neighboring buildings or neighboring structures/utilities unless the foundation contractor can successfully demonstrate during the index pile/pile load testing program prior to construction that driven piles can be installed as a viable foundation support alternative to drilled micropiles (as discussed in the following section) without negatively impacting these structures or inducing excessive vibrations in them. In addition, obstructions including shallow foundations and slabs associated with former structures which occupied the site, are anticipated to be encountered within the fill layer. At a minimum, driven pile locations within 25 feet of the property line and the existing BHH Synagogue remnants should be pre-drilled through the fill materials in order to reduce impact to adjacent structures and utilities due to vibrations induced during pile driving.

### Drilled Micropiles

In areas located within 25 feet of the existing BHH Synagogue remnants, the adjacent neighboring buildings or neighboring structures/utilities, building loads can be supported on drilled-in micropiles deriving their load-carrying capacity by means of a bond zone in NYC Building Code Class 3b and 5b, or better, sand and silt. Drilled micropiles are capable of simultaneously providing compressive, uplift, and lateral resistance.



Drilled micropiles are installed by drilling a steel casing to a depth required to generate the required load capacity. The steel reinforcement is then set and grout is placed under pressure while the casing is withdrawn. A permanent steel casing is left in place penetrating the fill and extending to a minimum depth of 2 feet below adjacent building foundations' zone of influence. An individual pile design compression capacity on the order of 100 tons for a 9-5/8-inch-diameter drilled micropile and 150 tons for a 13-3/8-inch-diameter drilled micropile is anticipated to be achievable. Drilled micropile grout and reinforcement should be designed in accordance with the provisions of the NYC Building Code. We have developed the following micropile option to assist in foundation design:

Compression Load Capacity	100 tons	150 tons
Micropile Diameter	9.625 inch	13.375 inch
Minimum Bond Zone Length (into NYC Building Code Class 3b, 5b, or better material) <sup>1</sup>	55 feet	60 feet
Minimum Reinforcement	1 #18 full length bar	1 #24 full length bar
Minimum Concrete Compressive Strength	5,000 lbs/in <sup>2</sup>	5,000 lbs/in <sup>2</sup>
Minimum Center-to-Center Micropile Spacing	4 feet	4 feet
Uplift Load Capacity <sup>1</sup>	30 tons	50 tons

### Notes:

- 1. The design minimum bond zone length assumes the drilled micropile will be pressure grouted during construction.
- 2. Uplift capacities are based on maximum structural capacity of the reinforcement and maximum geotechnical capacity of the soil socket. The actual capacity may need to be lowered to account for structural stiffness requirements.

The drilled micropile capacities must be verified in the field by means of at least two (2) compression load tests for each pile type/capacity performed in accordance with the provisions of the NYC Building Code. Conventional top-load testing procedures could be used for the drilled test piles. In addition, strain gauges should be installed at selected depths within the drilled piles to measure the load distribution along the pile shafts during the performance of the load tests. The information gathered with the strain gauges is critical in order to analyze the load testing results in an attempt to optimize the design embedment into the bearing stratum.



The basic allowable lateral load for a drilled micropile is 2 kips. Uplift loads should not exceed the values provided in the table herein. If higher lateral and uplift resistance (if any) is needed, lateral and uplift field load tests must be performed in accordance with the provisions of the NYC Building Code to substantiate the higher required resistance.

### **Below Grade Walls and Floor Slabs**

Below grade walls are presumed to be fixed against rotation, and should therefore be designed to resist the soil and surcharge pressures as shown on Figure 8. In addition, the below grade walls bordering Norfolk Street, Broome Street, and Suffolk Street should be checked against the potential for a temporary water level near street level should a utility break occur.

The below-grade walls along the south side of the site should be designed to withstand lateral loading from neighboring buildings, calculated as a surcharge, if the neighboring building foundations are supported above a 1H:1V theoretical influence line extending upwards from the base of the proposed below-grade wall foundations. Lateral loading from neighboring building footings need not be considered if these footings are to be underpinned to a level below the above-mentioned influence line.

In areas where deep foundation support is utilized, the lowest level floor slabs should be designed as a structural slab supported by the deep foundation elements. In areas where shallow foundation support is utilized and natural sand soils are encountered at the floor slab subgrade elevation, the floor slabs can be designed as a slab-on-grade using a modulus of subgrade reaction of 150 lbs/in<sup>3</sup>.

Although the groundwater level is not expected to be encountered during basement level excavation, we recommend that the below grade walls, lowest level floor slabs, and pits be fully waterproofed, and water stops be placed between all below-grade joints to limit the potential for wetness due to nearby utility breaks or heavy precipitation events.

### **Settlements**

Settlements of shallow foundations and slabs-on-grade are estimated to be one inch or less, with differential settlements between adjacent footings of ¾-inch or less. Total and differential settlements for deep foundation supported foundations are estimated to be less than ¾-inch.

The foundations will need to be sized and designed appropriately, so total and differential settlements are kept within tolerable limits. Once the column / foundation layout and column loading are finalized, we will need to perform additional geotechnical settlement analysis to assist the Structural Engineer in foundation design.



## **Seismic Design Parameters**

Based on the results of our subsurface investigation, we recommend the building be designed using the following seismic parameters obtained from the 2014 NYC Building Code:

- Site Class = D
- Maximum Considered Earthquake Spectral Response Acceleration Parameters:
  - o At 0.2 Second Period:  $S_S = 0.278 g$
  - o At 1.0 Second Period:  $S_1 = 0.071$  g

The above ground motions should be adjusted for site class effects using coefficients  $F_a = 1.57$  and  $F_v = 2.40$ .

The NYC Building Code requires an evaluation of the liquefaction potential of noncohesive soils or clays, silts, and clayey silts with plasticity index less than 20 that are located below the groundwater table and less than 50 feet below the ground surface. Based on an initial screening of the SPT N-value data from the borings, some points fall within the "Liquefaction Evaluation Required" on the NYC Liquefaction Assessment Diagram.

To evaluate the potential for soil liquefaction, the procedure outlined by Youd et al (2001) was used. This evaluation presents an empirical relationship between the earthquake demand, represented by the Cyclic Stress Ratio (CSR), and the soil resistance to dynamic loading, represented by the Cyclic Resistance Ratio (CRR). The CSR is correlated to the Peak Ground Acceleration (PGA) of the design earthquake event, as well as the in situ soil stresses, whereas the CRR is correlated to SPT N-values obtained in the field. Based on these analyses, liquefaction of the on-site soils is unlikely and need not be considered.

### **Excavation and Groundwater Control During Construction**

At this time, it is our understanding that the cellar level floor slab would be located 16 feet below existing grade, or at approximate el 16, and that general excavations for the proposed cellar level would need to extend deeper to reach proposed foundation subgrades.

Excavation will occur within the fill layer and the natural soil deposits. We anticipate that building foundation remnants, such as concrete block, brick, and foundation walls, footings, and concrete slabs may be present in the fill and will need to be removed and properly disposed of. Underground storage tanks, if encountered, would also need to be properly disposed of in accordance with the governing environmental regulations.



We anticipate that the excavation for one below-grade cellar level would be made above the groundwater level and that seepage water and stormwater that accumulates in the excavation could be collected using conventional sumps and pumps. Pumping of construction dewatering effluent should be done in a manner that will not adversely affect adjacent areas and buildings or cause settlements, loss of ground, or loss of support to adjacent areas.

### **Subgrade Preparation**

The footing and slab subgrade areas should be level and proof-rolled using an approved compactor such as a Bomag BW75 double-drum walk behind compactor, or equivalent. The footing and slab subgrades should be inspected and approved by a qualified geotechnical engineer prior to stone, steel, or concrete placement. Any soft, loose, or unsuitable soils identified by the inspecting geotechnical engineer during proof-rolling should be removed and replaced with approved compacted fill. Any foundation remnants associated with former site structures or boulders encountered within excavations should be fully removed and any void spaces that may be created should be backfilled with approved, compacted fill.

Controlled fill placed in any over-excavation should be placed in 8-inch loose lifts and compacted using a double drum walk behind compactor such as a Bomag BW75 or equivalent. All fill should be compacted to at least 95% of the material's maximum dry density as determined by the Modified Proctor Compaction Test (ASTM D 1557). The water content at the time of compaction should be within 3 percentage points of the optimum water content. All fill placement should be subject to controlled engineering inspection by the geotechnical engineer. No fill material should be placed on areas where free water is standing, on frozen subgrade areas, or on surfaces which have not been approved by the inspecting geotechnical engineer.

The Contractor should be responsible for maintaining all footing and slab subgrades in their as-approved condition until concrete is poured and the excavations are properly backfilled. Fill placement, backfilling, and compaction of footing and slab subgrades should be performed in accordance with the recommendations provided above for controlled fill. Footings and slabs should be constructed as soon as possible following subgrade approval by the geotechnical engineer. Rainwater, snow, ice or trash/debris should not be allowed to accumulate in the excavations.

### Temporary Excavation Support and Underpinning

Temporary excavation support will be needed to support the adjacent street and sidewalk areas, and temporary excavation support and/or underpinning will be needed for the



BHH Synagogue remnants and the adjacent buildings located to the south of the site if the foundation subgrade elevation extends to a lower elevation than the adjacent buildings.

At this time, we preliminarily anticipate that temporary support of excavation for a one cellar level development scheme could consist of the following:

- Adjacent Streets and Sidewalks: Along the Norfolk Street, Broome Street, and Suffolk Street frontages, we anticipate that temporary support of excavation could consist of drilled-in or driven solider pile and timber lagging walls that are laterally braced with drilled-in tie-back anchors and/or interior rakers/bracing. Driven soldier piles should not be utilized within 25 feet of the BHH Synagogue remnants, adjacent neighboring buildings or neighboring structures/utilities, as pile driving operations could cause unacceptable vibration levels and could induce settlements in these structures.
- Adjacent 50 Norfolk Street and 384 Grand Street Buildings: Considering that these buildings are located to the south of the site and approximately 12 feet to 30 feet or more away from the proposed development footprint, we do not anticipate that underpinning of these buildings would be required, and that temporary support of excavation consisting of drilled-in or driven solider pile and timber lagging walls that are laterally braced with interior rakers/bracing could be utilized. Driven soldier piles should not be utilized within 25 feet of adjacent neighboring buildings. Drilled-in tie-back anchors, if utilized, would need to be designed to avoid conflicts with the neighboring building foundations.

The need for underpinning, while not anticipated at this time, should be further assessed once the proposed building design is further developed. Concrete pier underpinning of the existing buildings located to the south of the site may be required to properly support the buildings during construction if the development scheme calls for new building foundation construction within a 1H:1V influence line projected beneath the adjacent building lowest level.

• BHH Synagogue Remnants: Temporary support of excavation and/or underpinning requirements for the synagogue remnants will be highly dependent on the depth and material of the remnant synagogue foundations, as well and the depth and plan extents of the proposed cellar level excavation. At this time, we anticipate that a proposed cellar level that is offset from the remnant synagogue foundations by approximately five feet or more could be facilitated by a temporary support of excavation system consisting of drilled-in soldier pile and lagging walls that are laterally braced with interior rakers/bracing. For a proposed cellar level located within approximately five feet of the remnant synagogue foundations, we anticipate



that underpinning of the synagogue remnants would be required to facilitate below-grade construction.

Considering that the BHH Synagogue is Landmark-designated structure, any specific requirements pertaining to support of excavation, monitoring, or settlement tolerances of the synagogue remnants should be confirmed with the Landmarks Commission prior to progressing a temporary support of excavation or underpinning design.

Based on our review of the BHH Synagogue Emergency Stabilization drawings, the top of the lowest level slab of the Synagogue appears to be located approximately 2 feet below adjacent Norfolk Street sidewalk grade; Elevations of BHH Synagogue remnants and neighboring buildings' lowest level or basement slabs should be confirmed by a Professional Land Surveyor Licensed in the State of New York to assist in identifying the extent of underpinning required, if any. Preconstruction test pits should be performed at selected locations adjacent to the BHH Synagogue remnants and foundation walls associated with the neighboring buildings to investigate foundation depths, types, and bearing materials for the purpose of evaluating underpinning requirements.

If underpinning is required, we recommend that during foundation excavation the bottom of the neighboring buildings' foundation walls should be carefully exposed to determine the material on which the walls bear for the purpose of confirming underpinning requirements. This work should be done in such a manner so as not to damage or cause loss of support to the neighboring building.

All excavation support and underpinning should be designed by a Professional Engineer licensed in the State of New York. Temporary and permanent surcharge loading due to cranes, equipment, buildings, and vehicular traffic should be considered while designing excavation support and underpinning. As required by the NYC Bldg Code, a Professional Engineer licensed in the State of New York and retained by the Owner should provide Special Inspection of this work.

### **Protection of Adjacent Structures**

The BHH Synagogue remnants, neighboring buildings and all utilities, sidewalks, and streets surrounding the site should be protected against loss of support or any other damage during excavation and subsequent construction. Special care will be required during dewatering, soil excavation and removal, and excavation support and underpinning construction activities to ensure excessive vibrations or movements are not induced in these structures, and site activities do not result in their loss of support or instability of the neighboring structures.



A pre-construction conditions documentation should be performed to document existing conditions of the BHH Synagogue remnants that are to remain in place and structures neighboring the site. The survey should include at a minimum a photographic and video documentation of the exposed exterior and select interior facades of the neighboring structures. Documentation should be done in accordance with the requirements of the NYC Building Code. In addition, survey monitoring points and crack monitoring gauges should be established at select locations on the BHH Synagogue remnants and neighboring structures prior to the start of excavation and new foundation construction. Ambient vibrations at the site and in the neighboring structures should also be recorded as part of the pre-construction conditions documentation to establish a baseline of vibrations for comparison with vibrations recorded during construction activities.

The BHH Synagogue remnants and neighboring buildings should be continuously monitored during construction in accordance with the applicable requirements of the NYC Landmarks Commission as outlined in TPPN 10/88 and the NYC Building Code. At a minimum, monitoring should include crack monitoring gauges and survey monitoring points established over select facades to determine if the structures are adversely impacted by the on-site work. Adjacent sidewalks should be similarly monitored on a periodic basis during excavation and foundation construction activities. Vibration levels in neighboring structures should be monitored using seismograph vibration monitors placed at strategic locations in the building lowest levels or basements.

### **ADDITIONAL SERVICES**

At this time, we recommend the following additional engineering services be performed for the proposed development:

- 1. For a proposed 25,275 ft² deep foundation supported building, the NYC Building Code requires a total of twelve drilled borings be performed at the site to satisfy the minimum investigation requirements in the Code. Considering that eight borings have been performed at the site as part of this investigation, we recommend that the remaining four borings be performed at accessible locations at the site within the existing BHH Synagogue remnants and at the existing yard area in the southeast portion of the site to satisfy the NYC Building Code requirements and to confirm the preliminary foundation recommendations provided herein. Borings performed within the extents of the BHH Synagogue remnants will need to be coordinated with the on-going synagogue stabilization efforts.
- 2. A pre-construction conditions documentation of the BHH Synagogue remnants and adjacent neighboring buildings should be performed prior to commencing any construction activity at the site.



3. Special inspections should be performed during foundation construction, including subgrade preparation, driven pile and drilled-in micropile installation and load testing in accordance with requirements of the NYC Building Code. Special inspection engineering services should be provided by representatives from our office. Our firm should be provided with and review any foundation Contractor submittals for conformance with the recommendations given in this report and the project Construction Documents.

To maintain the continuity of our responsibility on this project, we recommend that the above work be performed by our firm.

### **CLOSURE/LIMITATIONS**

This report is based on conditions that existed at the time the study was performed. The findings and conclusions of this report may be affected by the passage of time, by manmade events such as construction on or adjacent to the site, or by natural events such as floods, earthquakes, slope instability, or groundwater fluctuations. Our interpretation of subsurface conditions is based on field observations from widely spaced sampling locations at the site. Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Langan reviewed field data and then applied our professional judgment to render an opinion about subsurface conditions throughout the site. Our report, conclusions, and interpretations should not be construed as a warranty of the subsurface conditions.

This report presents our preliminary recommendations regarding the geotechnical aspects of design and construction for the proposed GO Broome Development project in New York, New York. These preliminary recommendations are based on our site subsurface investigation and our current understanding of the proposed development scheme. Recommendations given are contingent upon one another and no recommendation should be followed independent of the others. This report has been prepared to assist the Site/Civil Engineer, Architect, and Structural Engineer in their design. Any changes in structures, locations, or loading information should be brought to our attention so that we may determine how such changes may affect our recommendations. Langan Engineering, Environmental, Surveying, Landscape Architecture, and Geology, D.P.C. cannot assume responsibility for the use of this report to generate foundation design other than at the specific site addressed in this report.



We thank you for allowing us to assist you on this project. If you have any questions regarding this report, please call.

Sincerely,

Langan Engineering, Environmental, Surveying, Landscape Architecture and Geology, D.P.C.

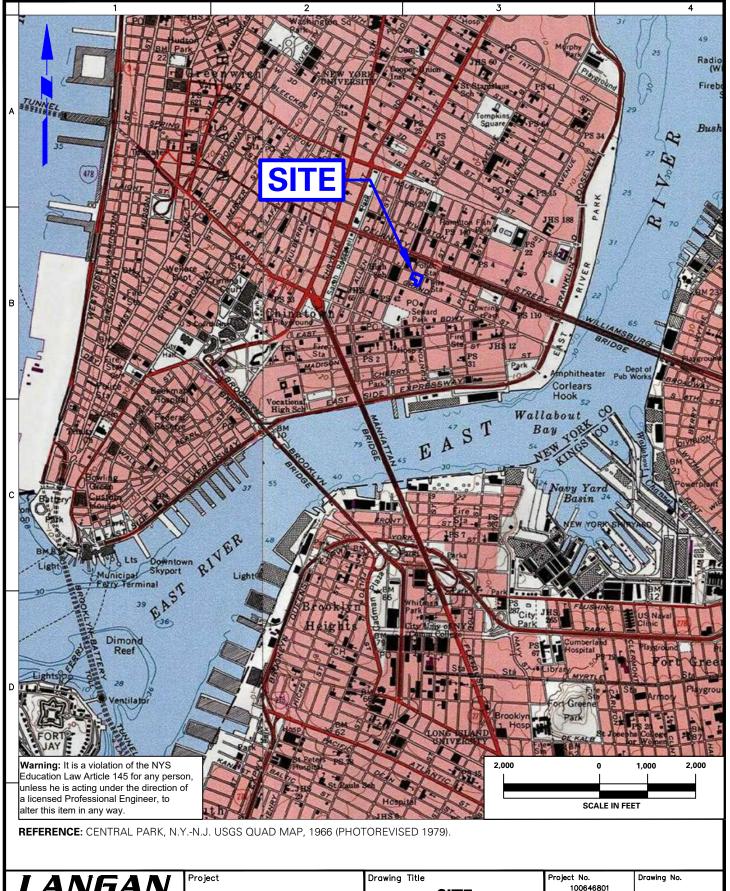
Nicholas Starzynski, P.E. Senior Project Manager

Ronald D. Boyer, P.E. Principal / Vice President

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# **FIGURES**

- Figure 1 Site Location Map
- Figure 2 Boring Location Plan
- Figure 3 Surficial Geology Map
- Figure 4 Historic Sanitary and Topographic Map
- Figure 5 Bedrock Geology Map
- Figure 6 Flood Map
- Figure 7 Subsurface Profiles A-A and B-B
- Figure 8 Lateral Earth Pressure Diagram



Langan Engineering, Environmental, Surveying, Landscape Architecture and Geology, D.P.C. 300 Kimball Drive

Parsippany, NJ 07054
T: 973.560.4900 F: 973.560.4901 www.langan.com NJ Certificate of Authorization No.24GA27996400

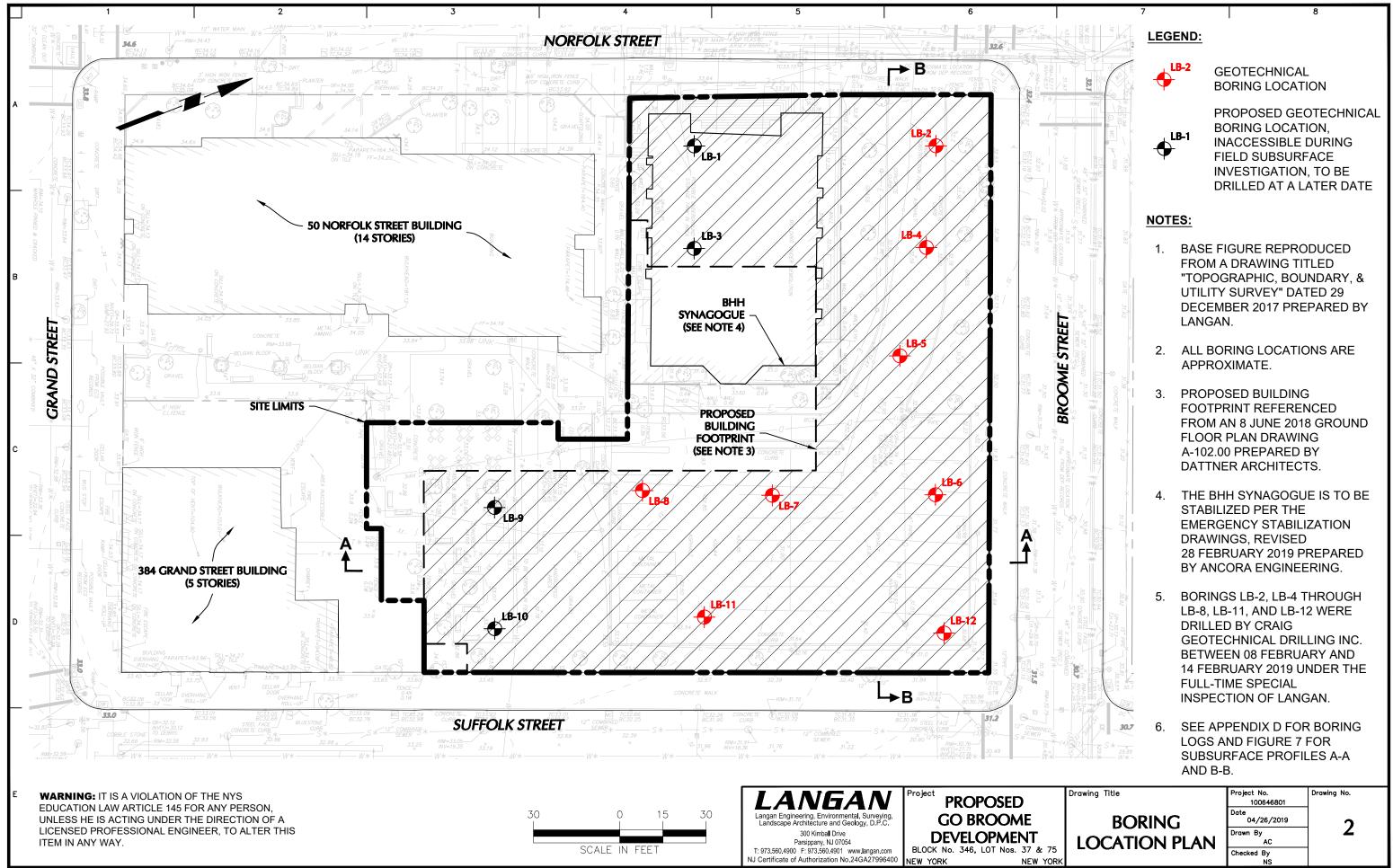
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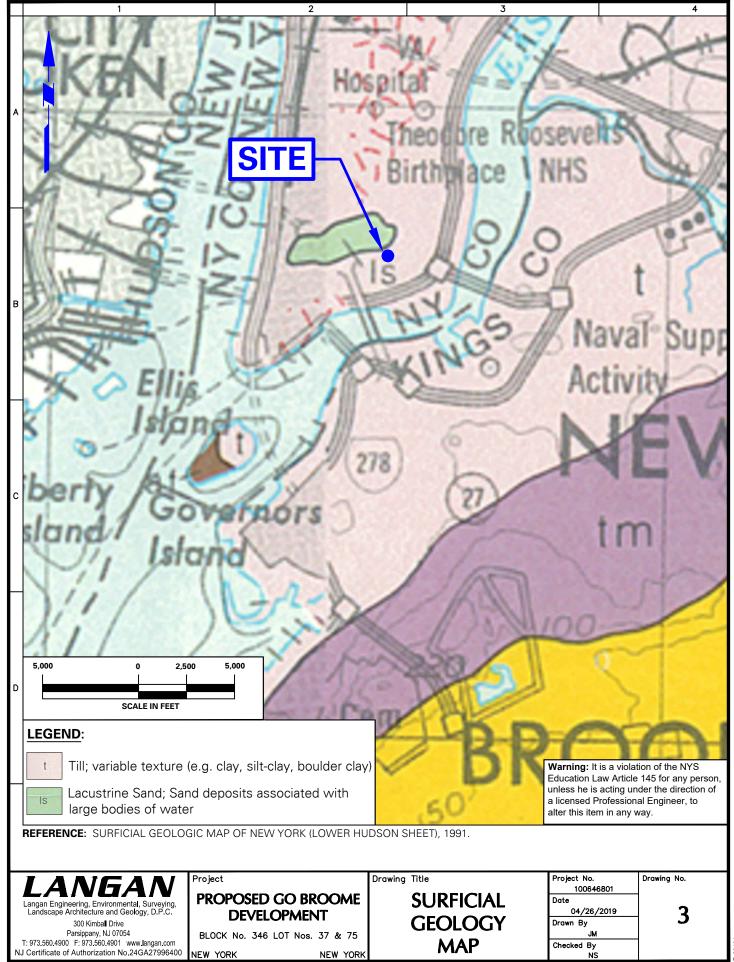
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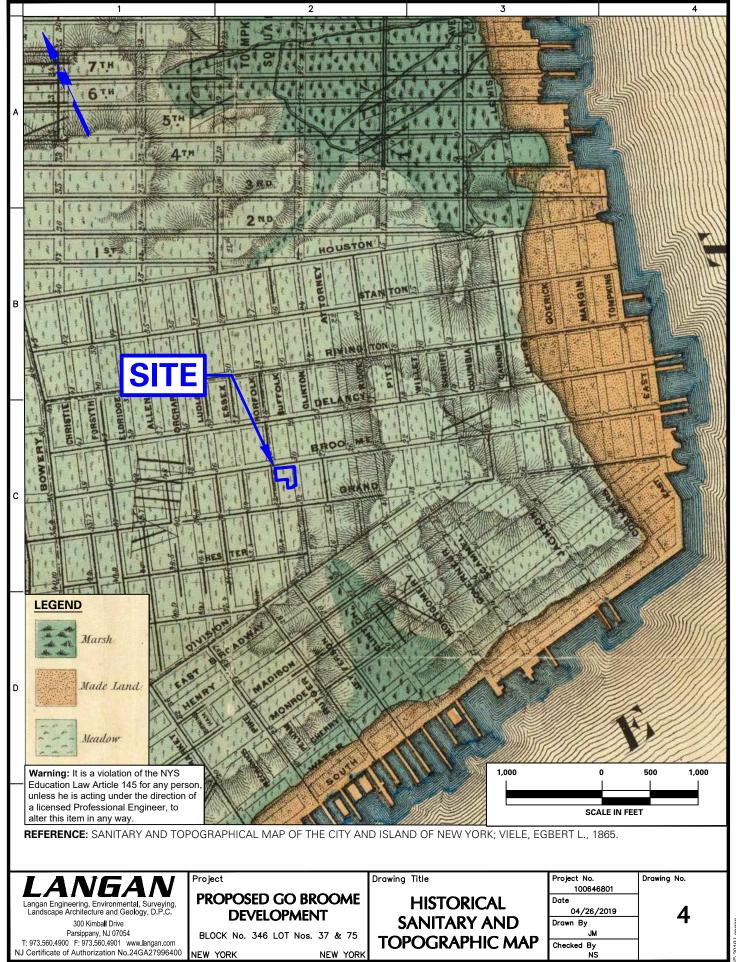
**SITE LOCATION MAP** 

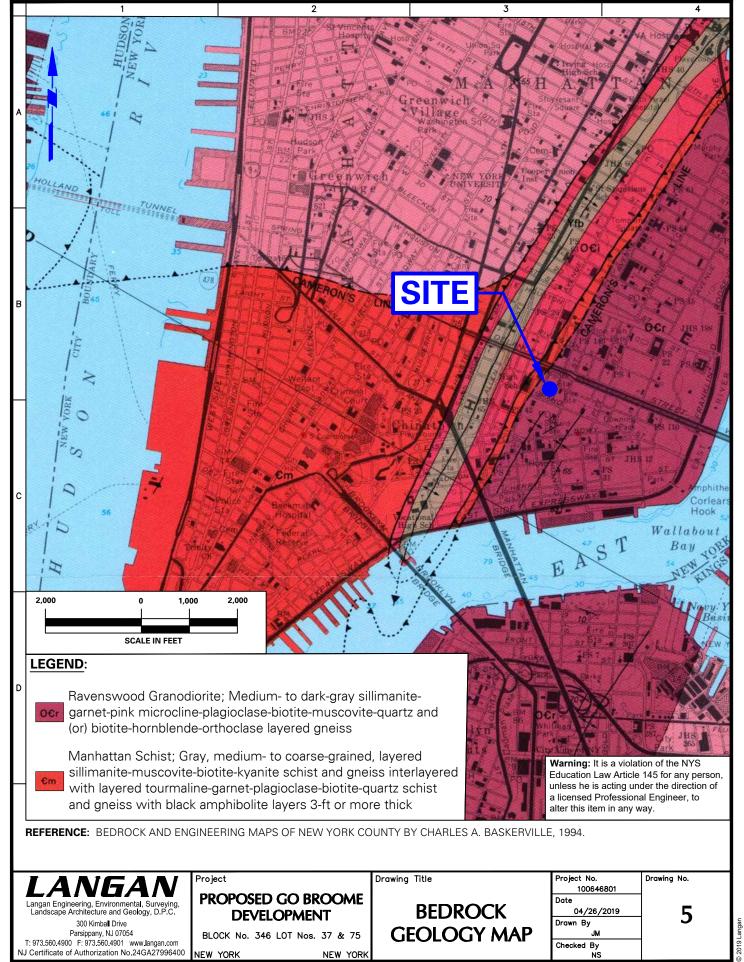
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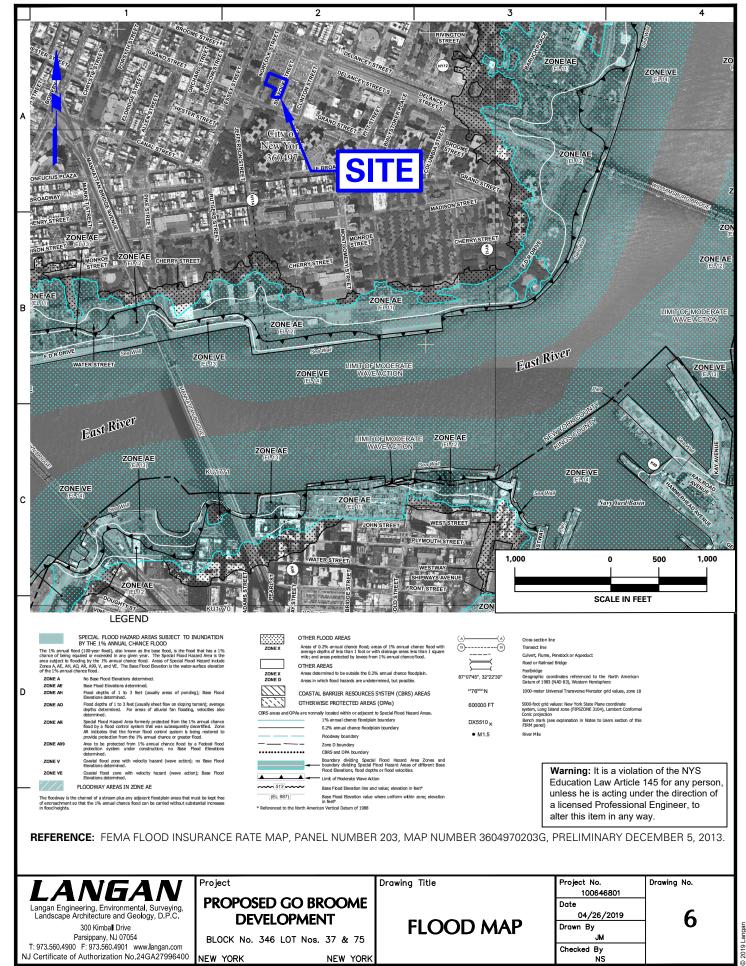
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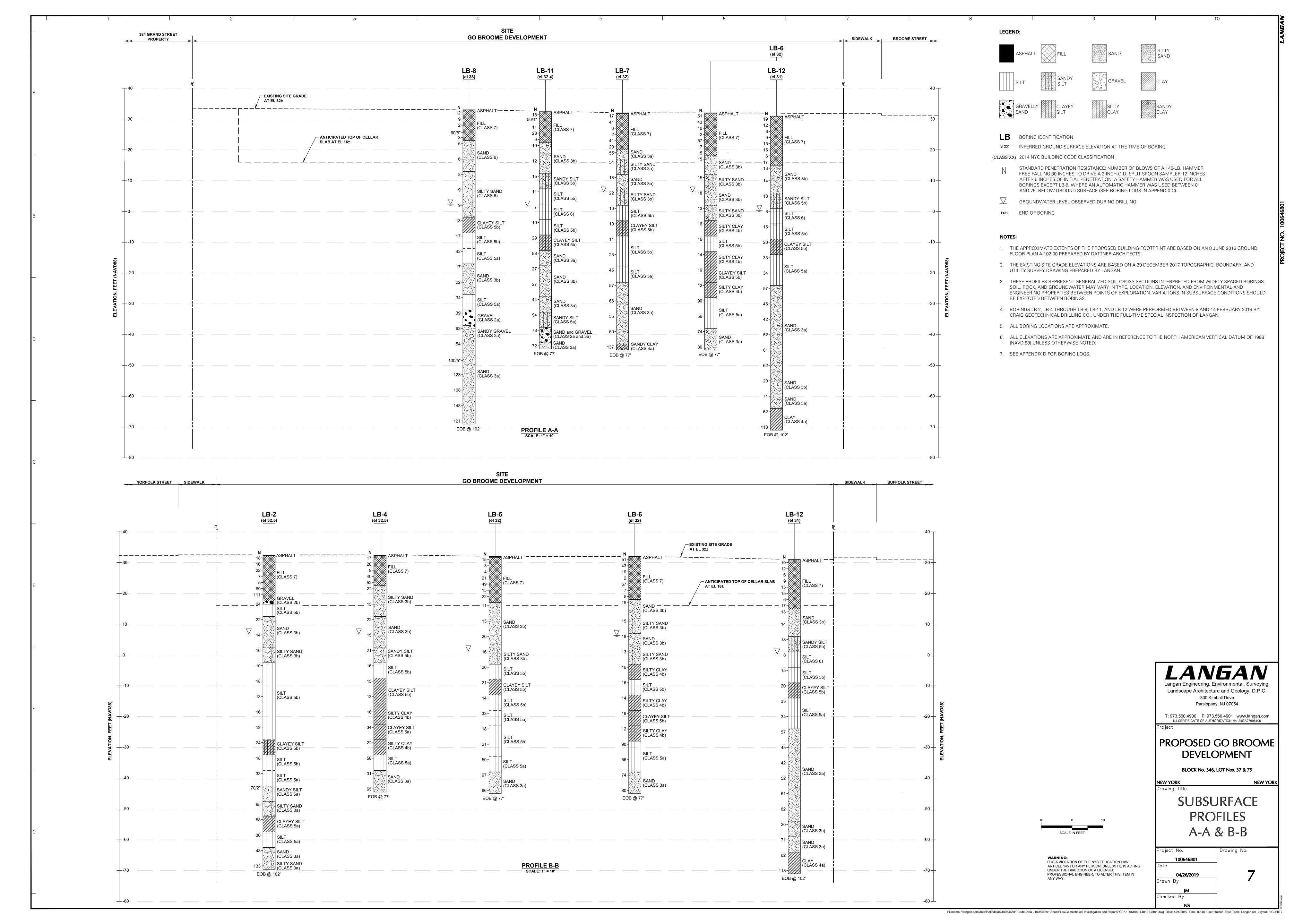


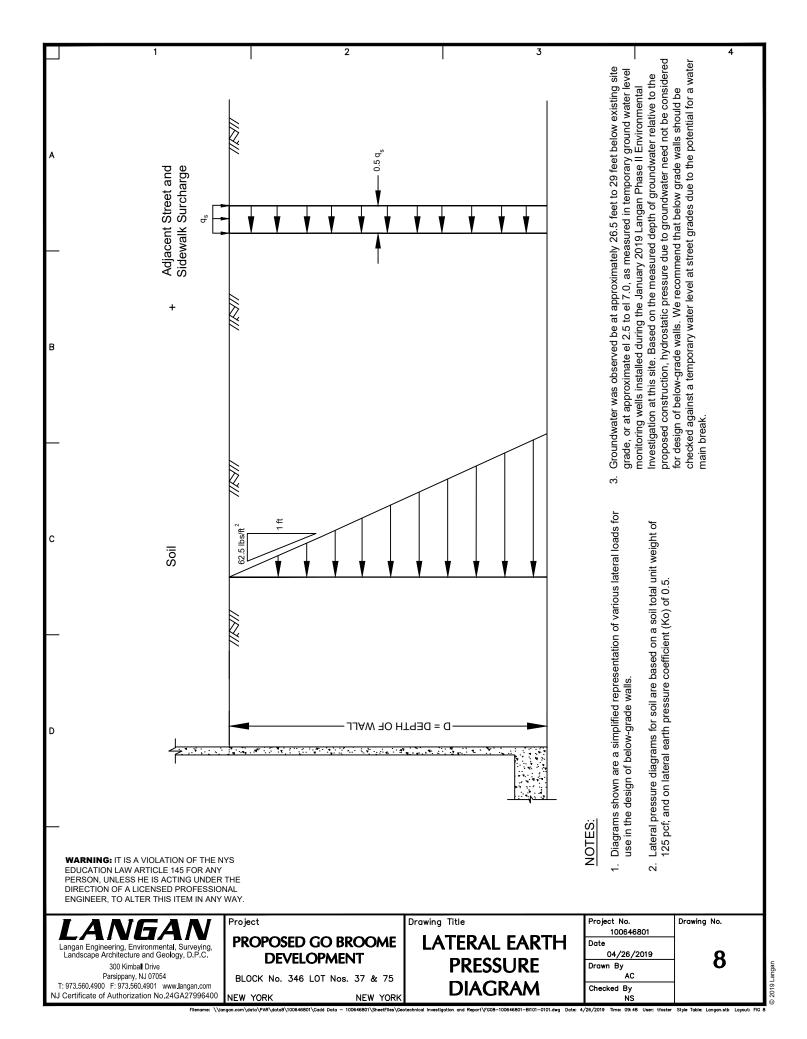




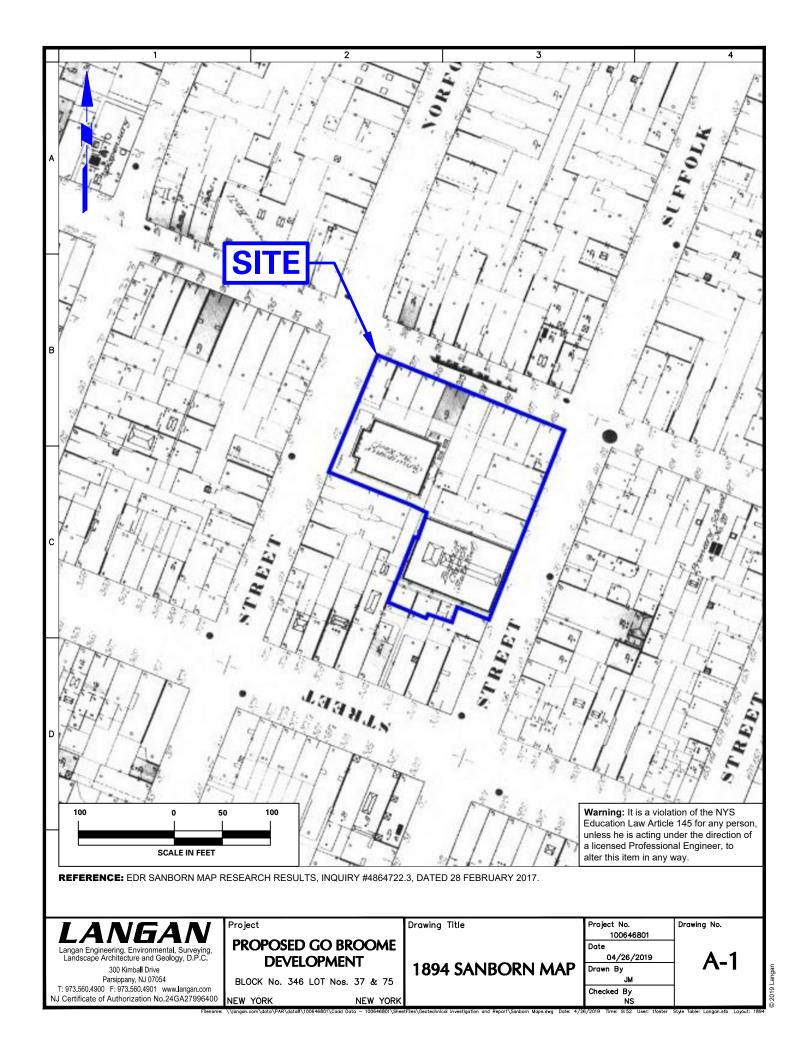


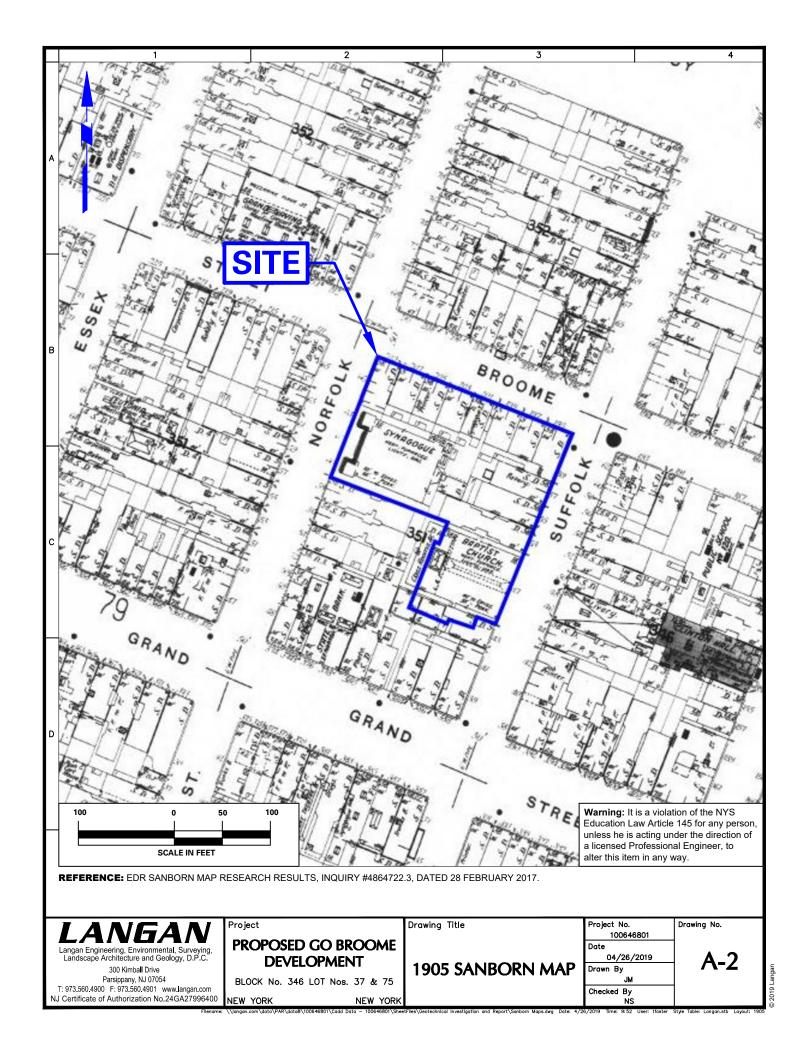


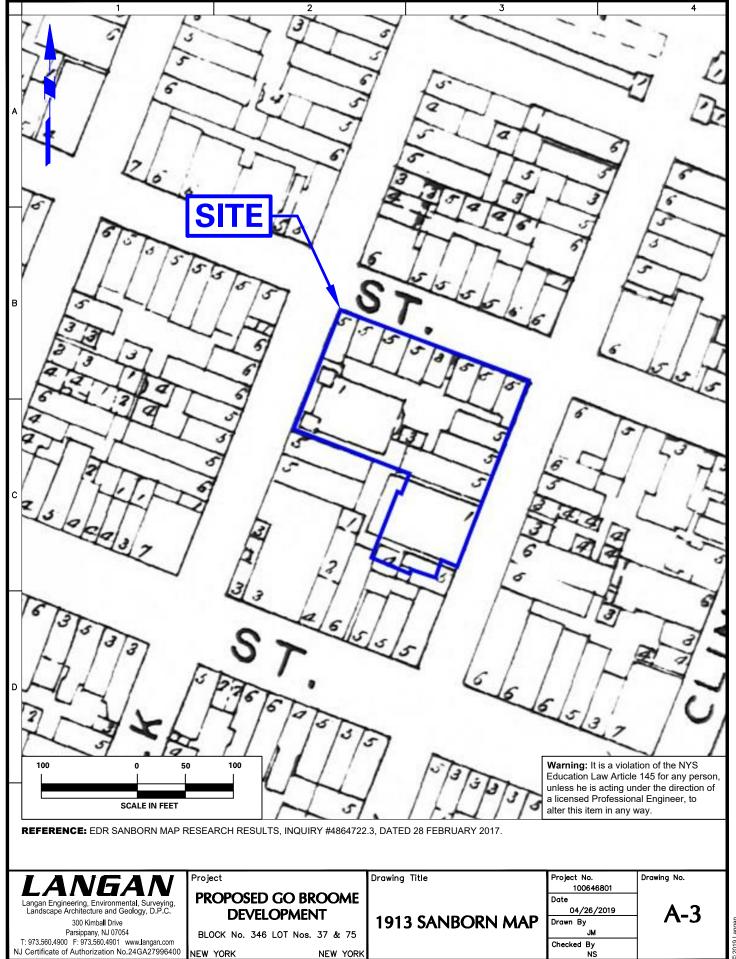




# APPENDIX A Historical Sanborn Maps

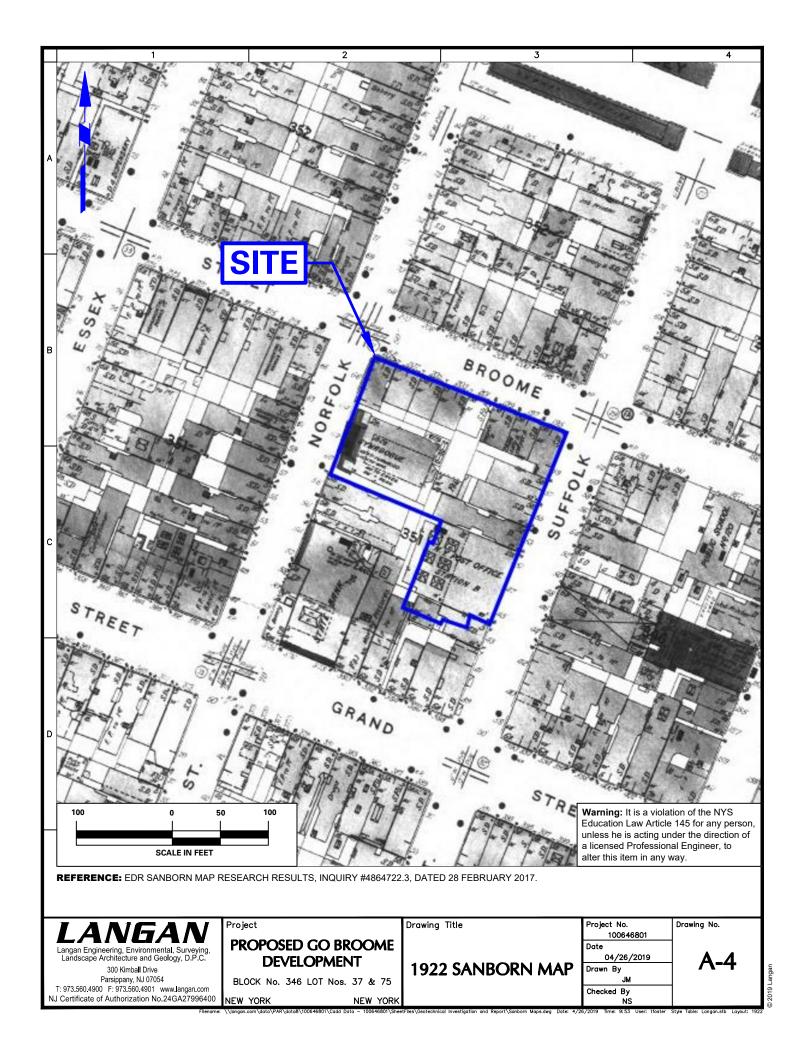


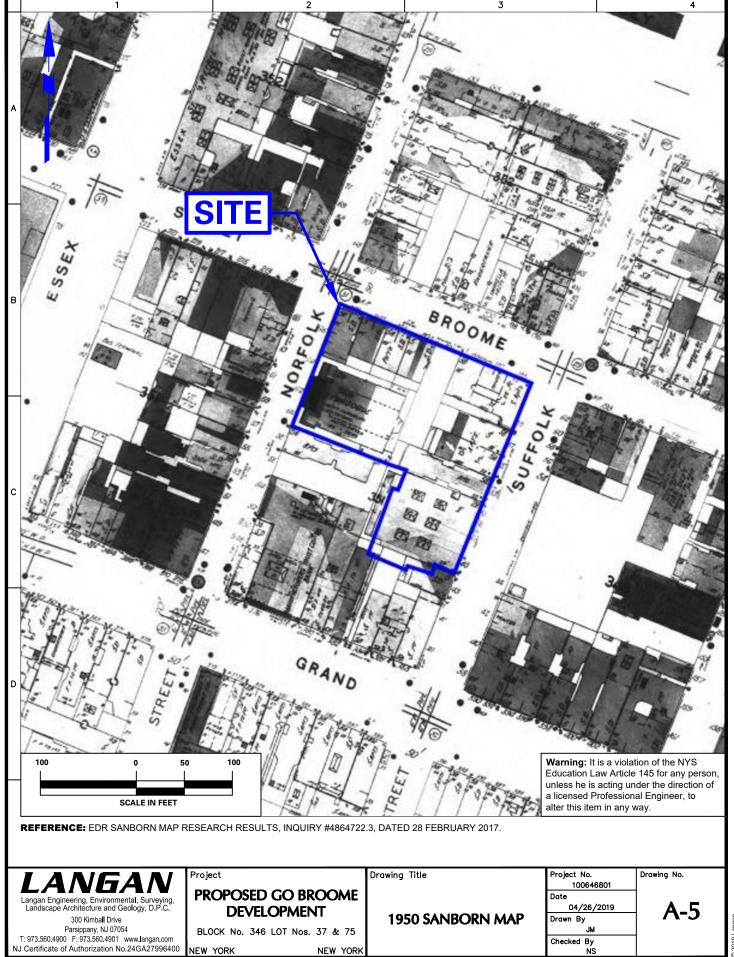




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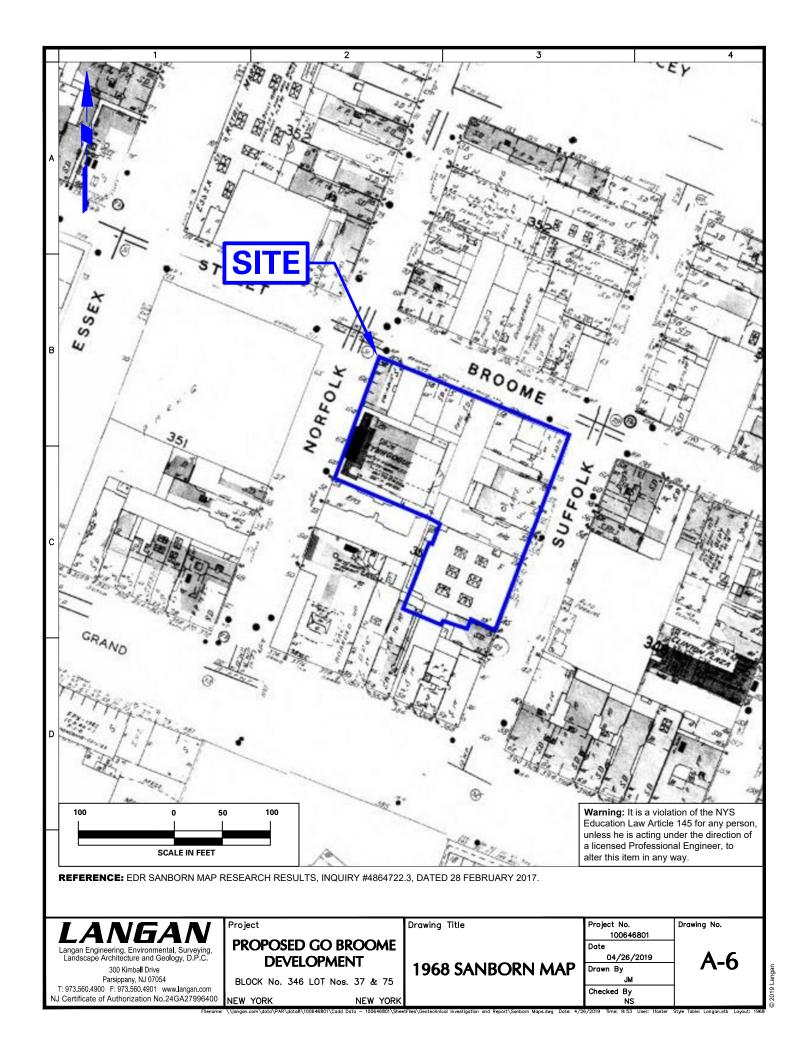


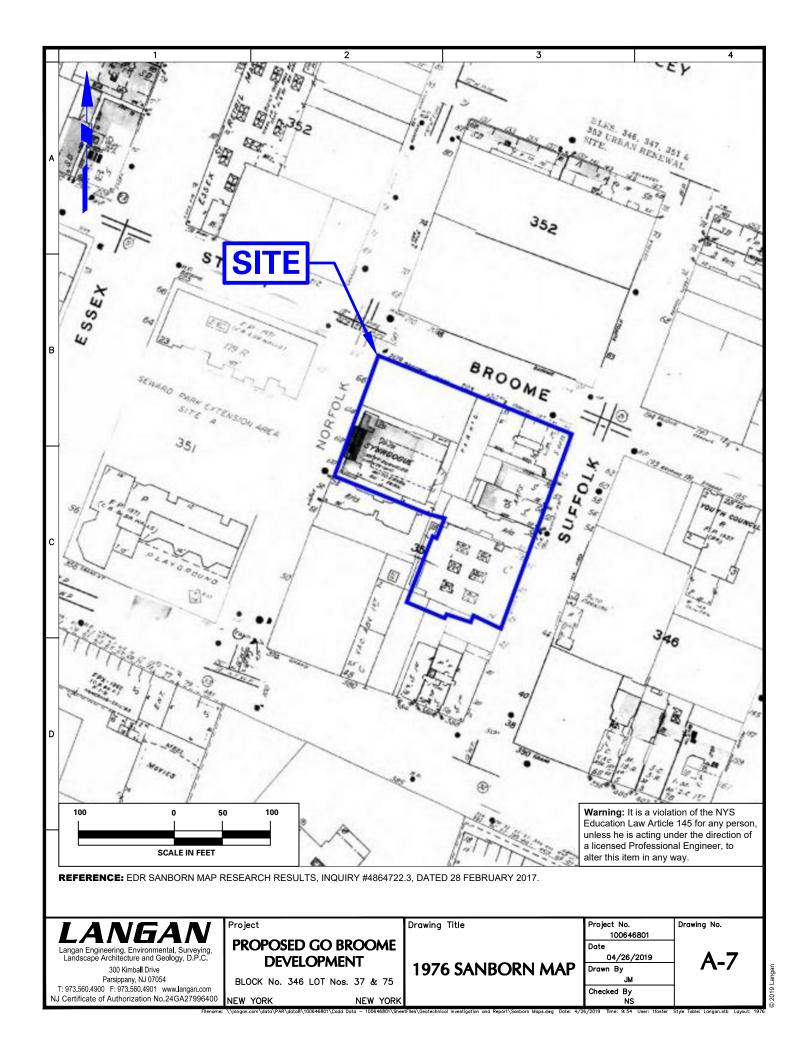
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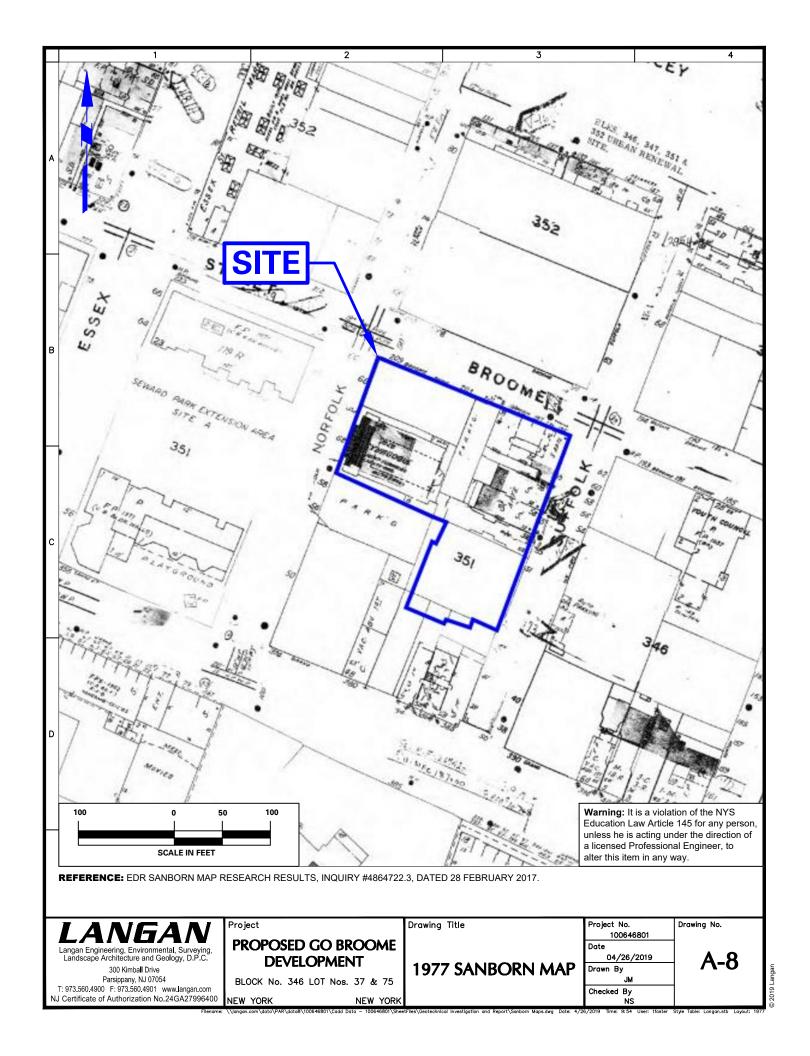
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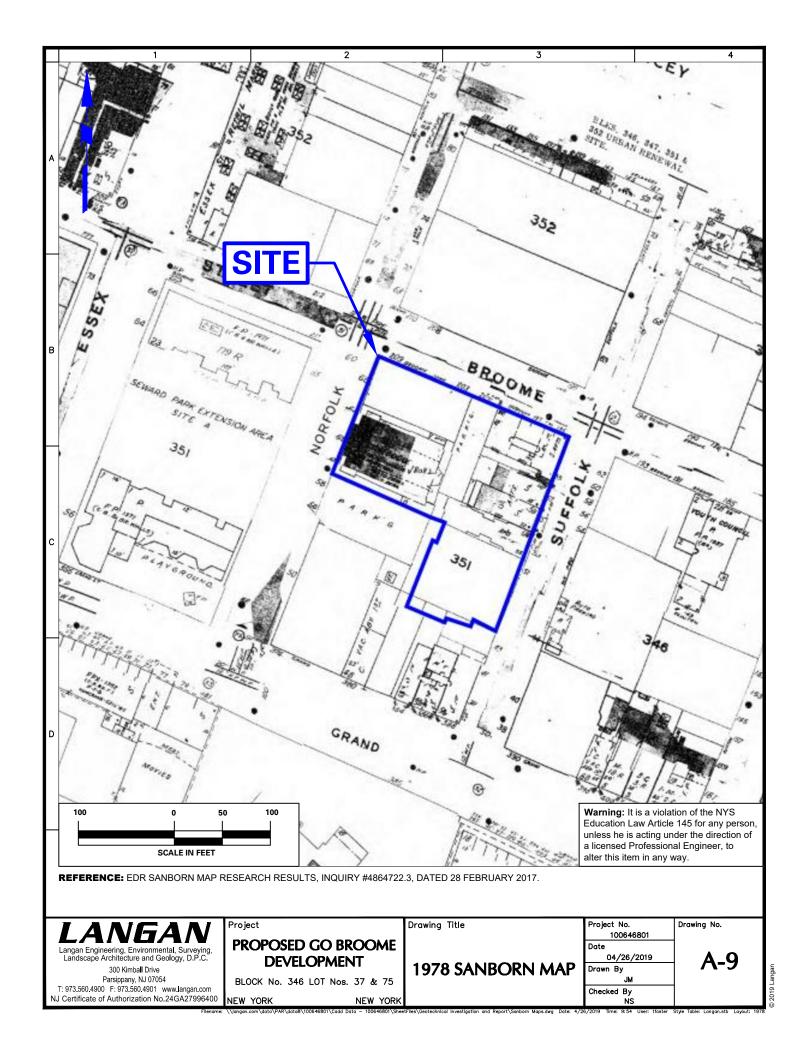
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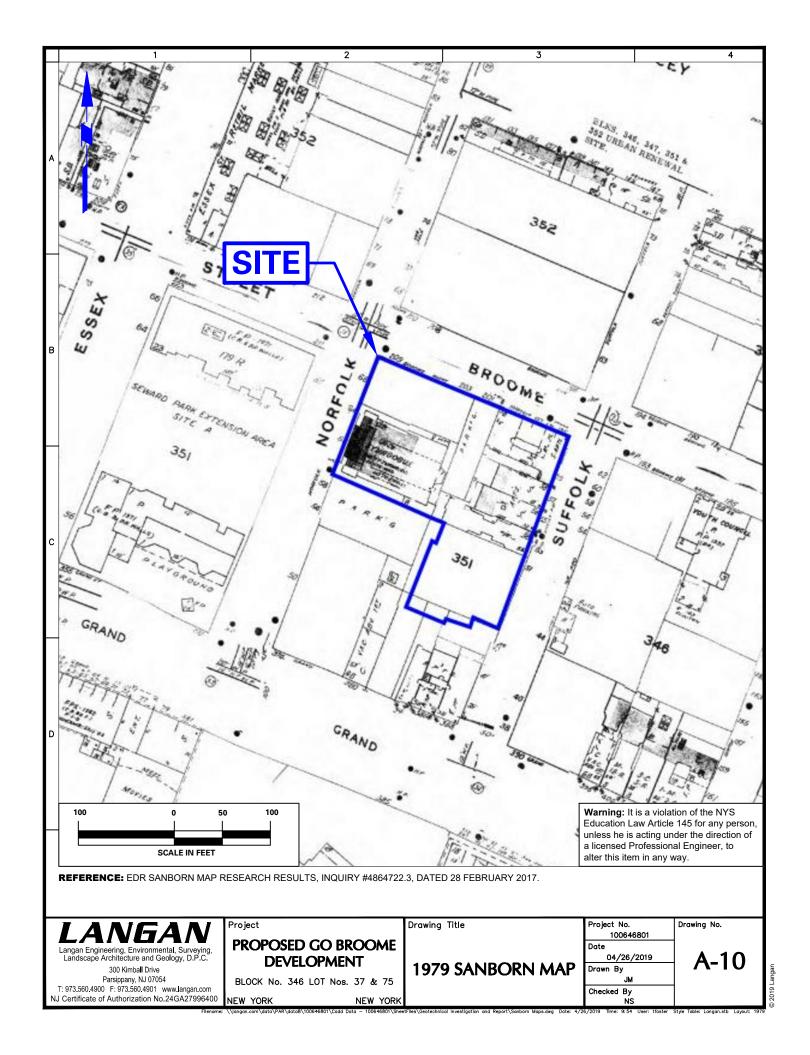
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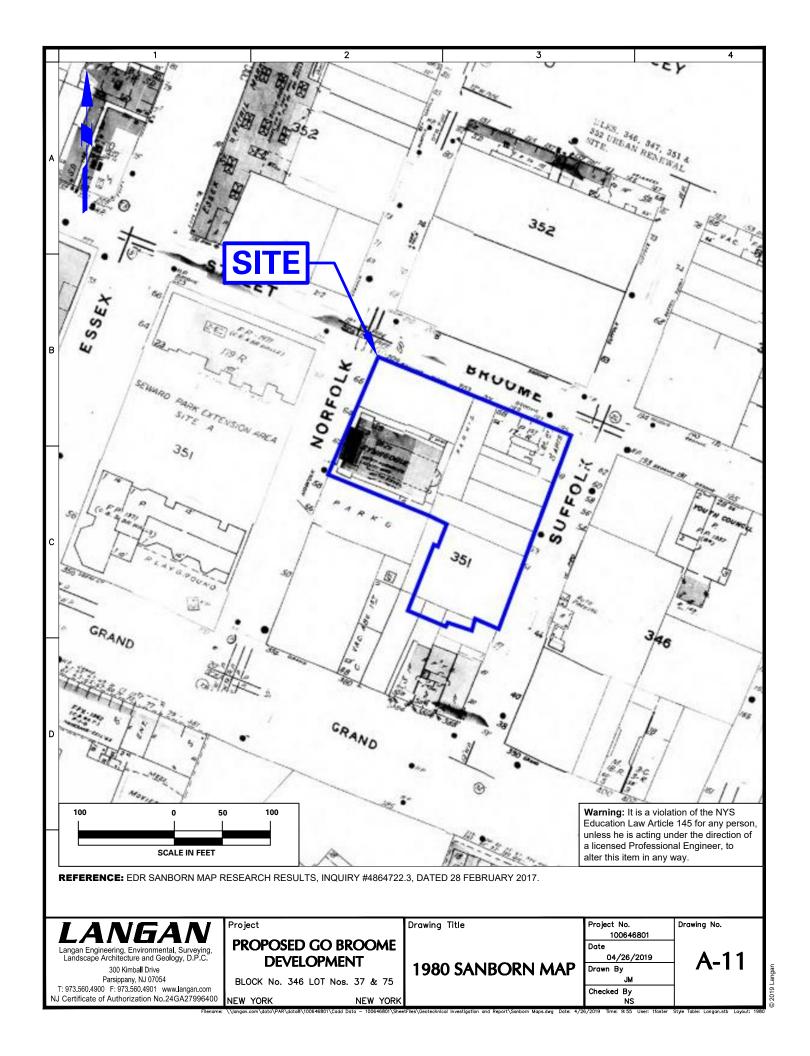


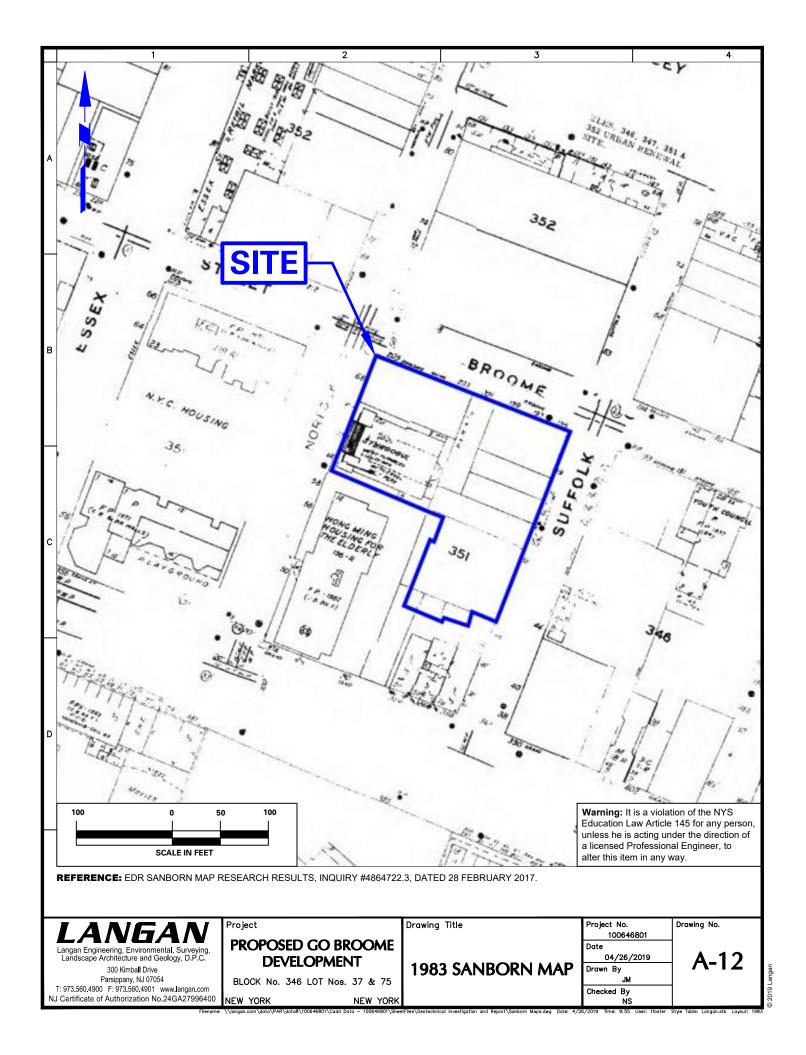


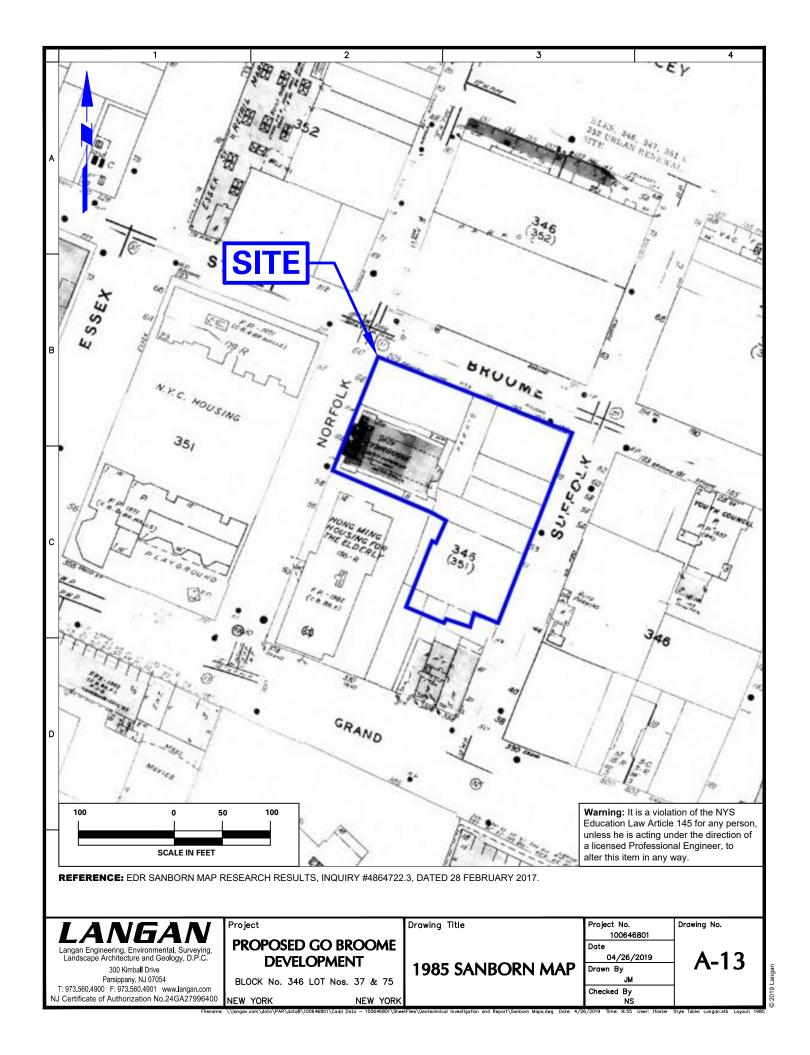


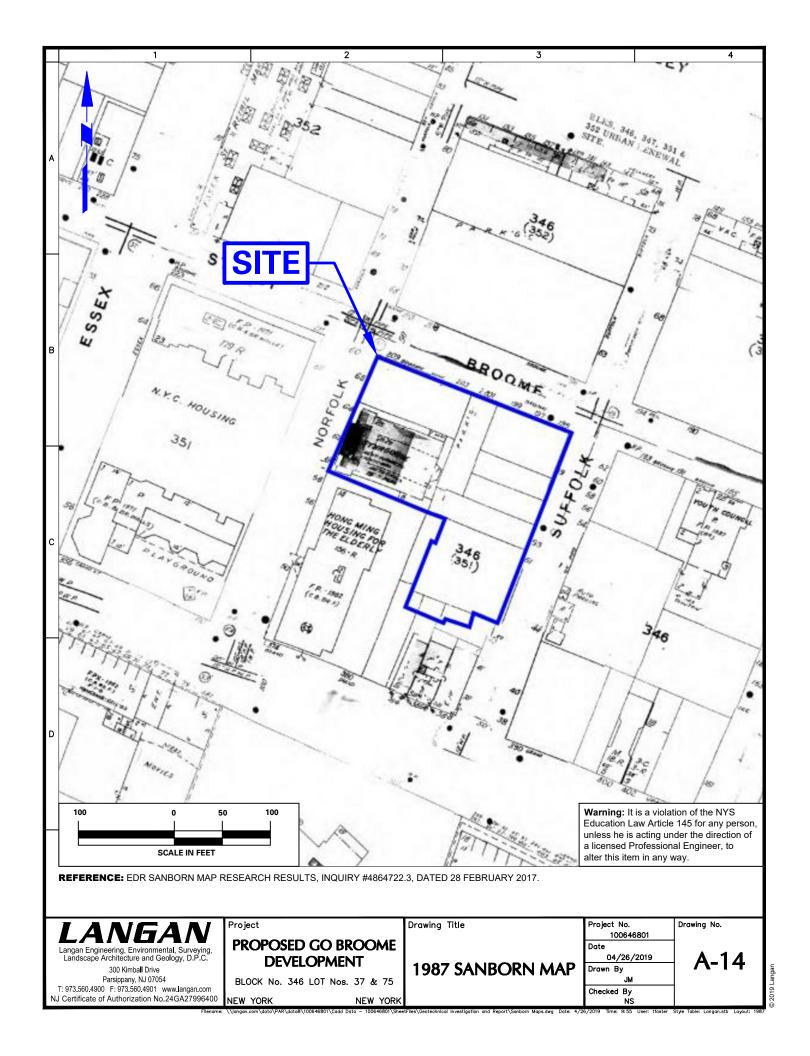


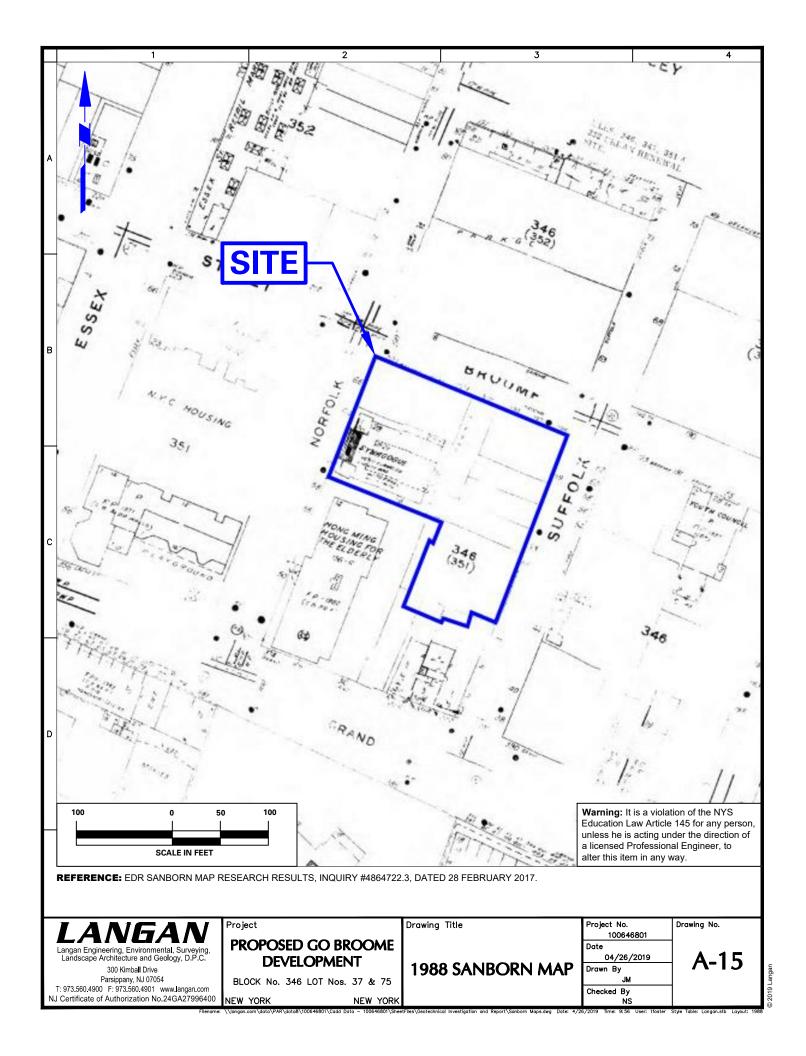


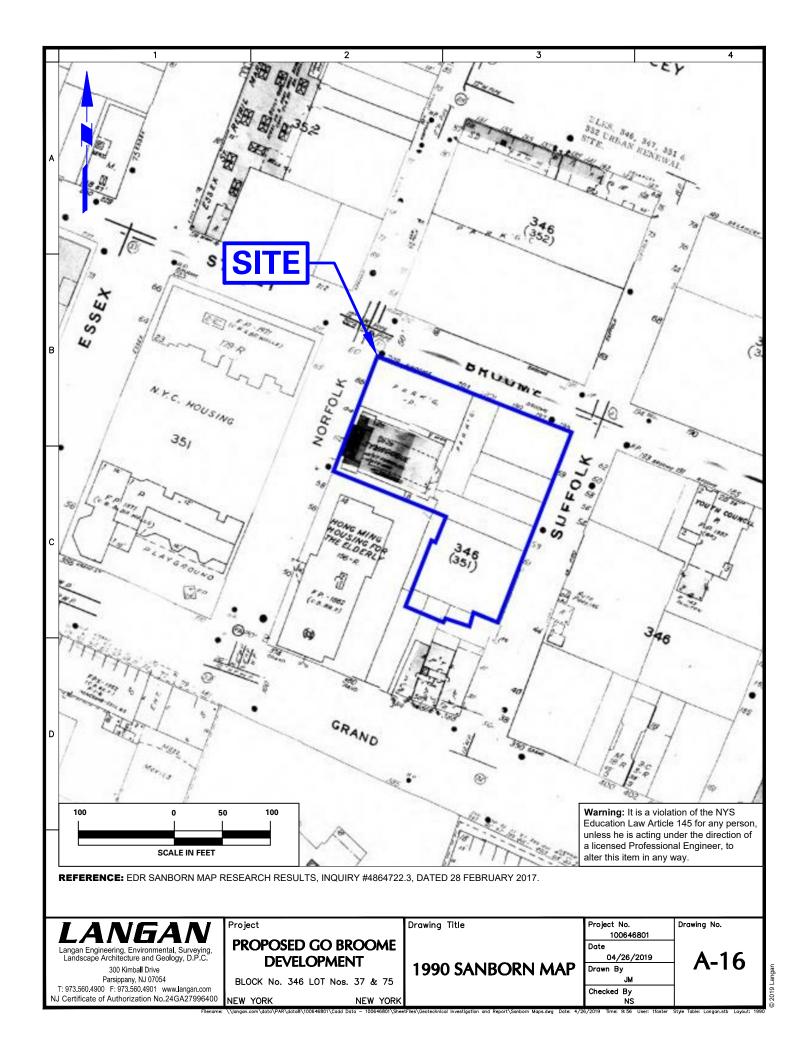


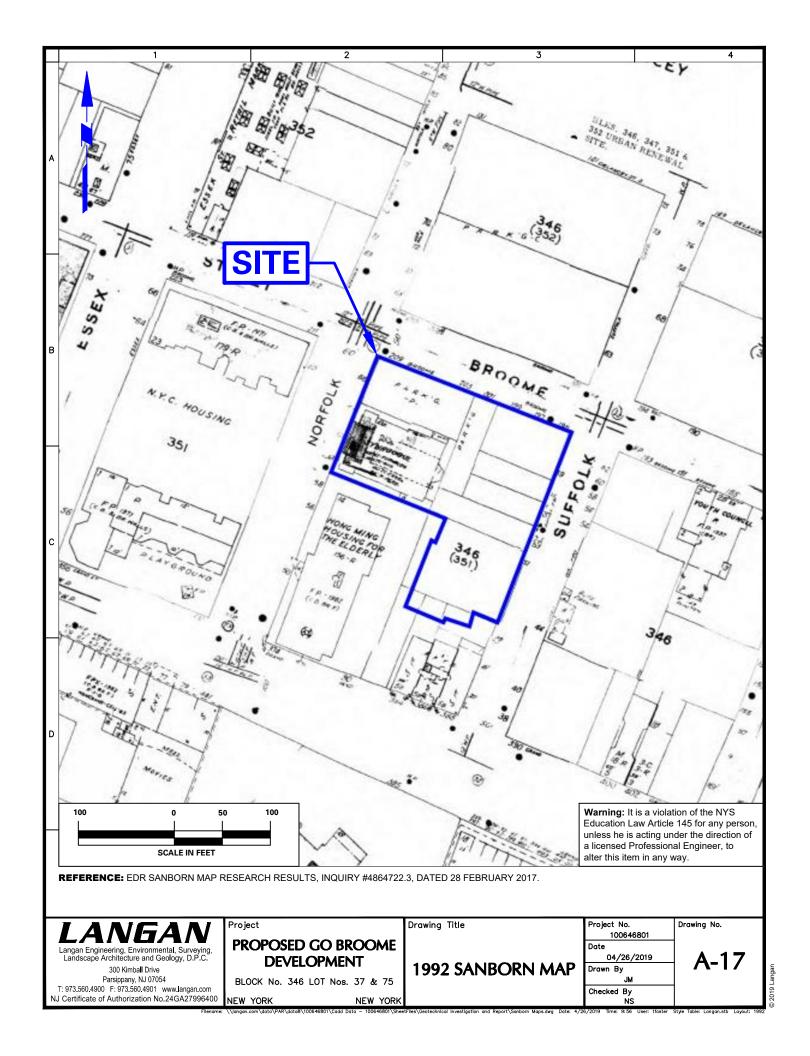


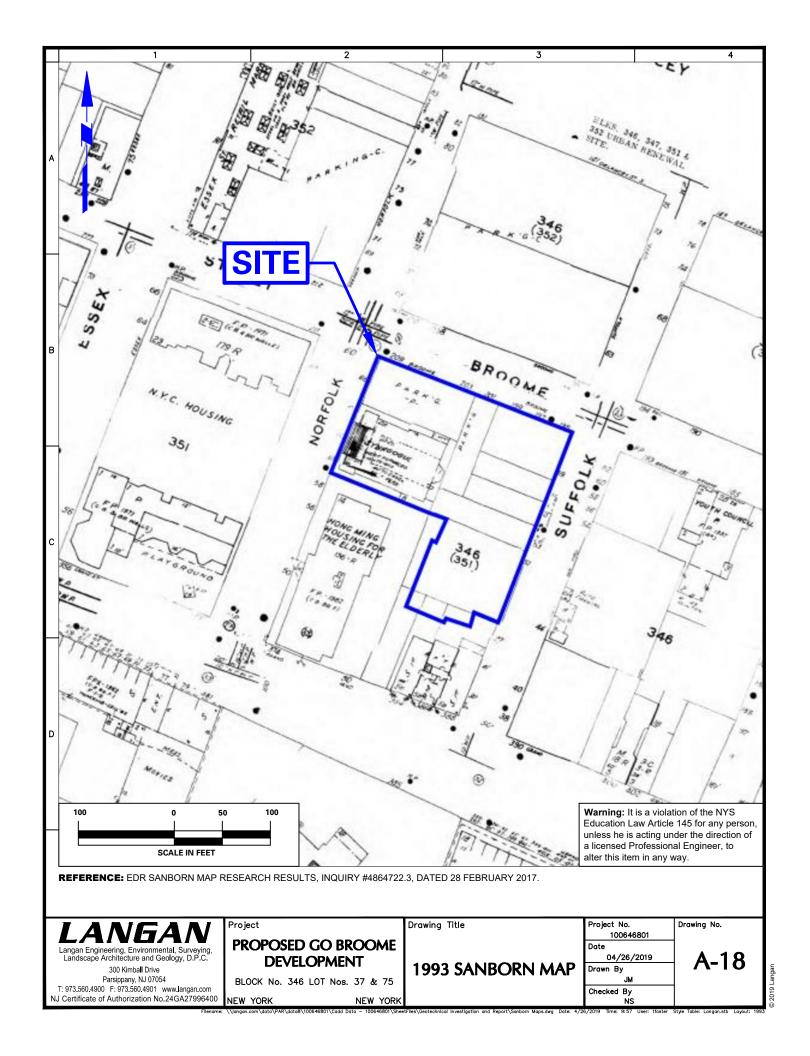


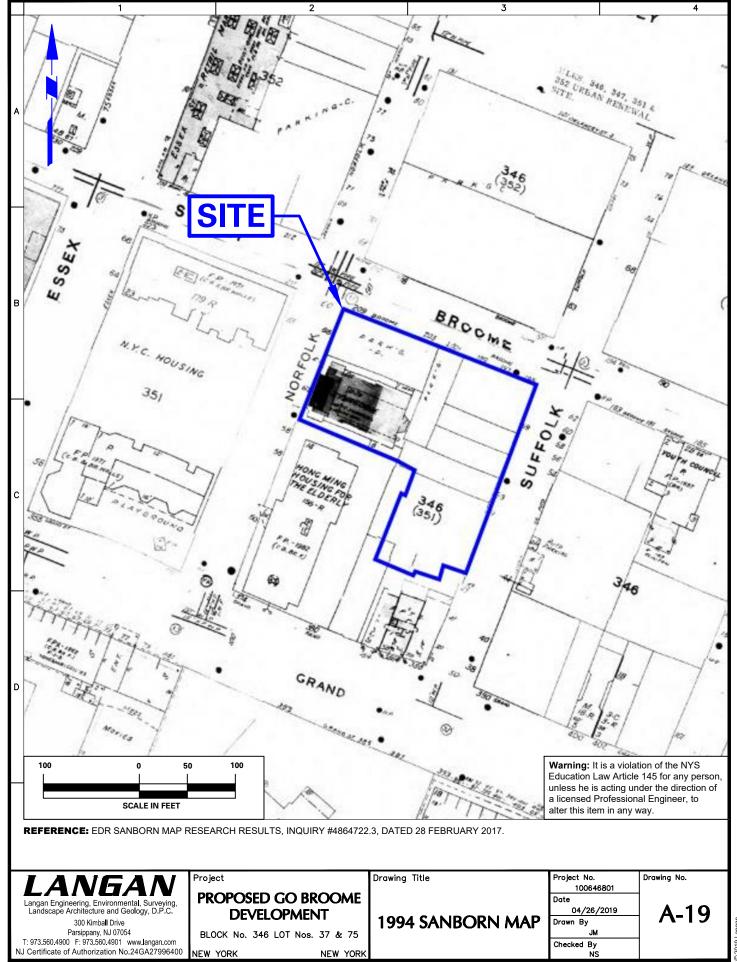


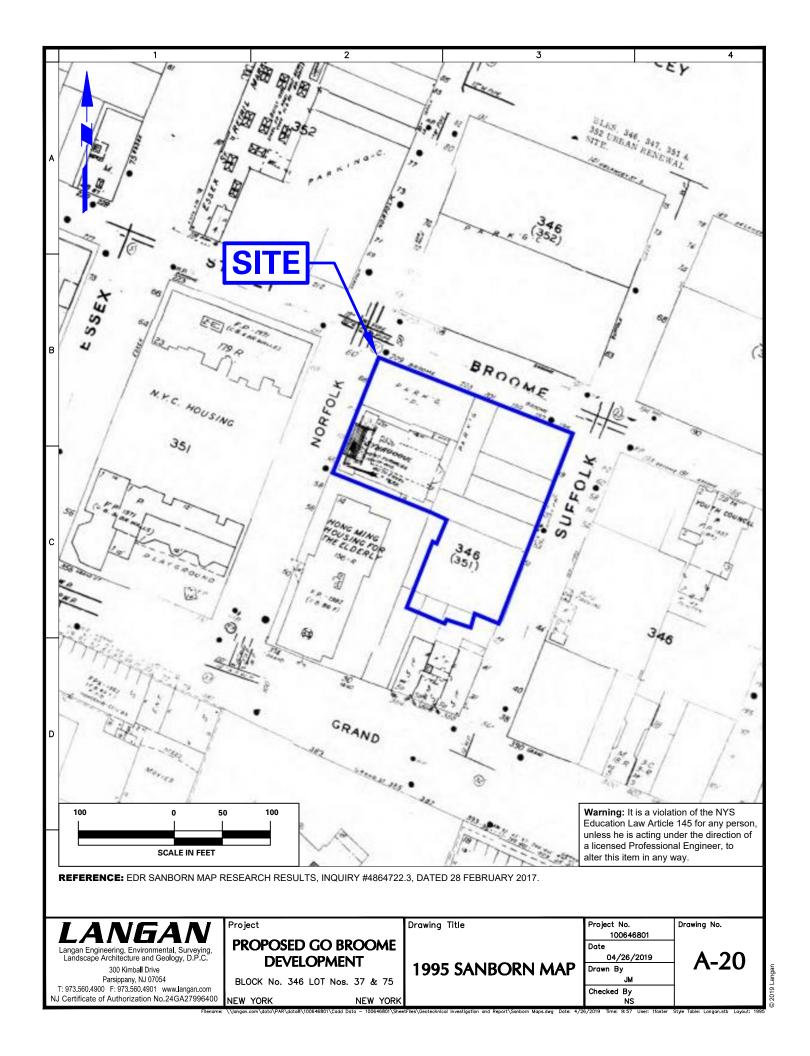


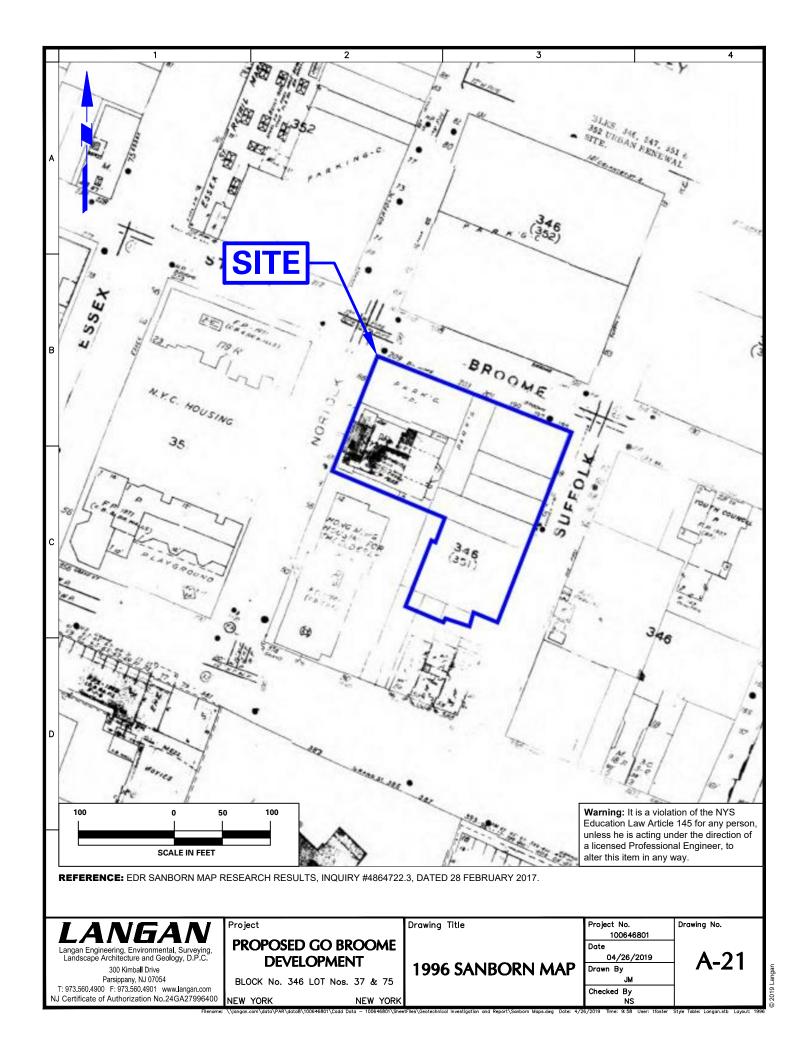


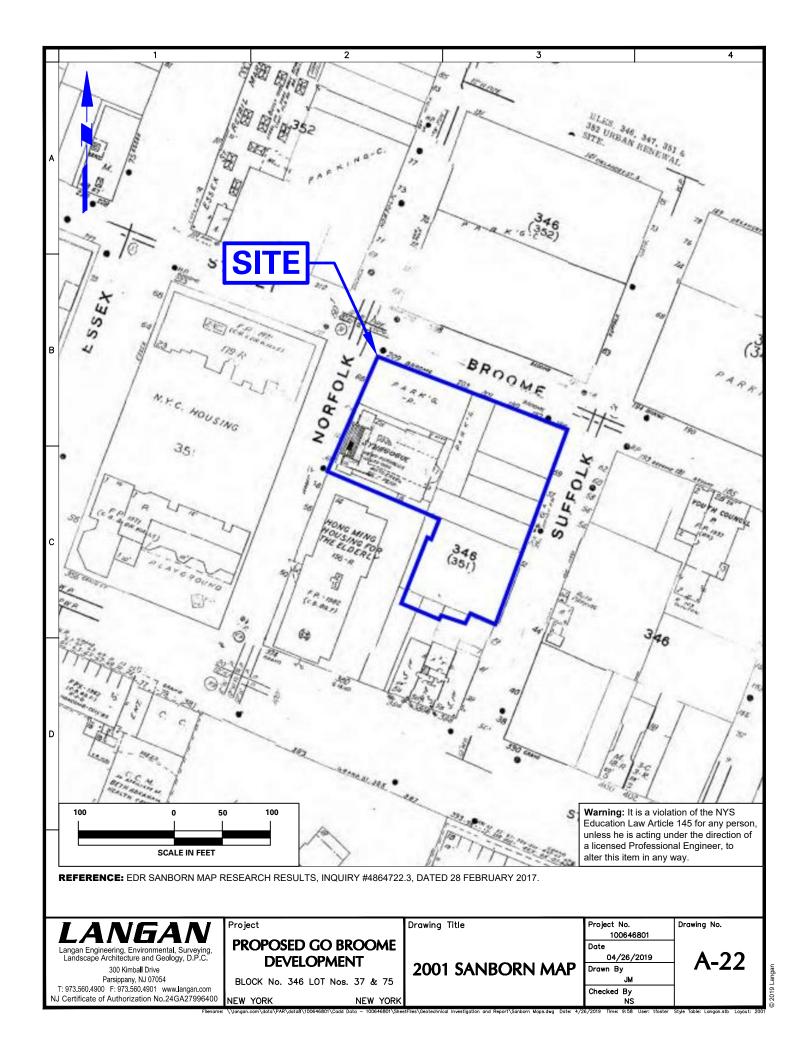


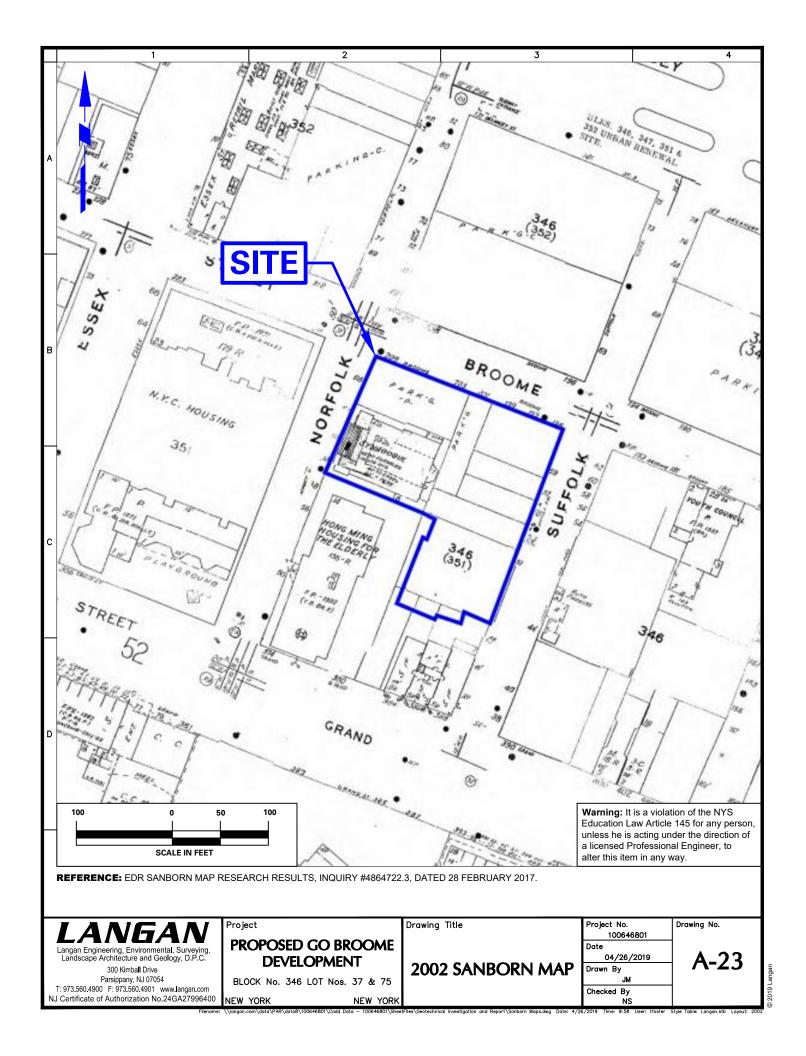


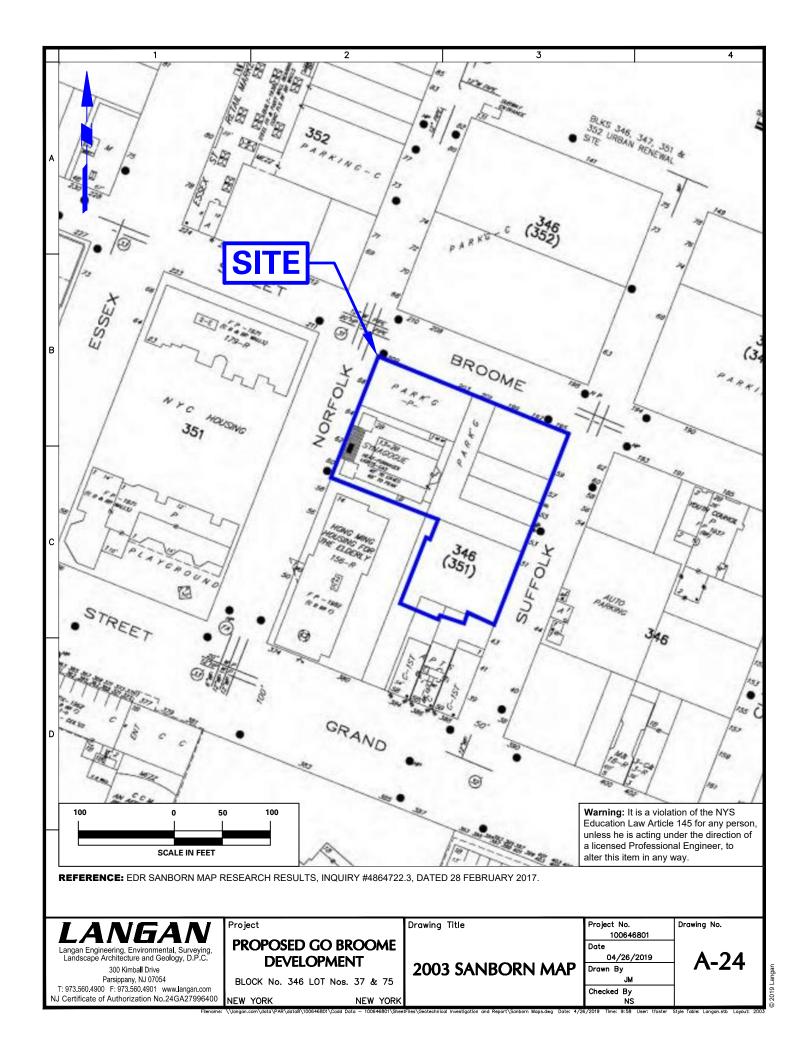


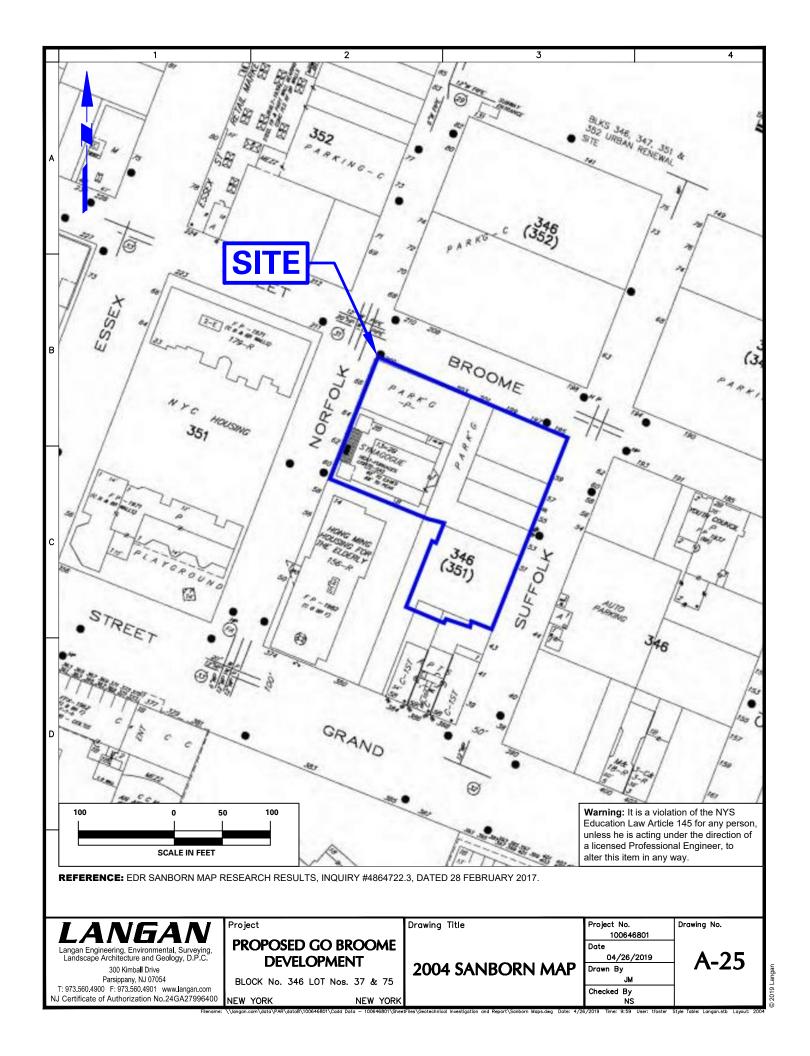


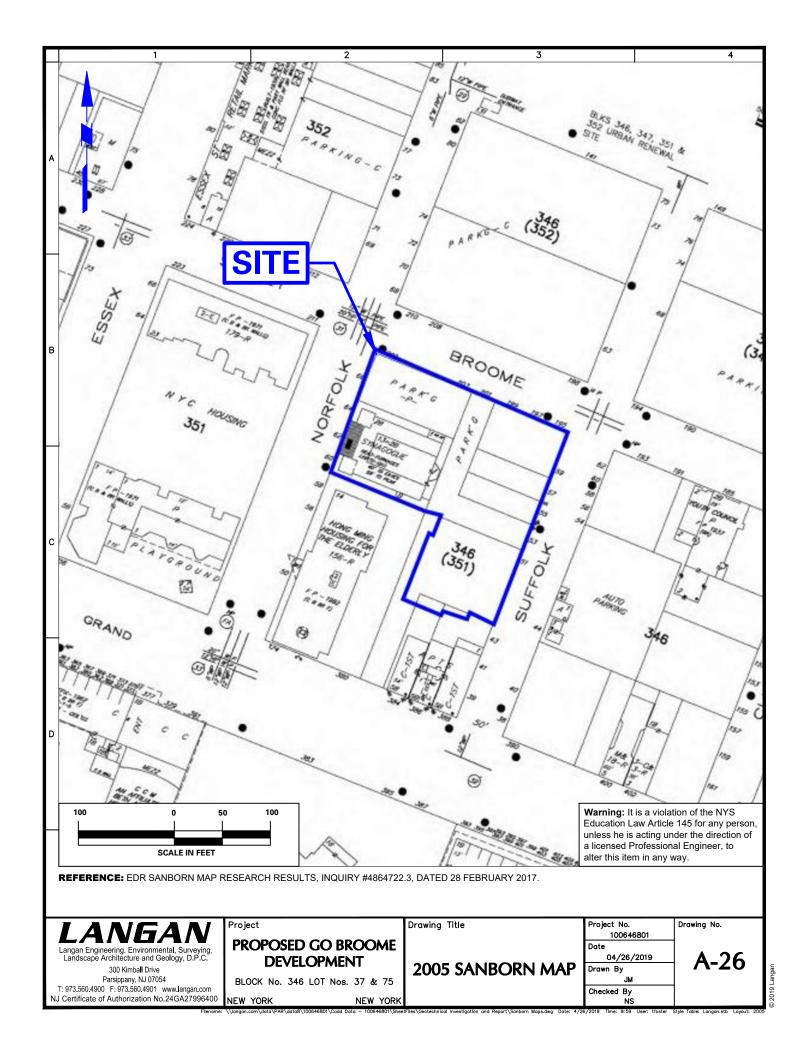












# APPENDIX B NYC DOB Documents

374 Grand Street (Block 346, Lot 1)

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supersedes C.O. NoT 83086

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	PERMISSIBLE USE AND OCCUPANCY									
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ist	floor	0.G.	6	0	0	2	B <b>-</b> 2	Tenant storage; maintenane room; kitchen storage; boi room		
lst	floor	0.G.	1	0	0	2	D <b>-</b> 1	Gas meter room		
lst	floor	0.G.	15	0	0	2	0-2	Utility service rooms; boi room; compactor room; fire pump room; kitchen		
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lst	floor	o.G.	31	0	0	2	P3 & 4	Meeting room		
lst	floor	0.G.	125	0	0	2	P4.	Dining room		
lst	floor	0.G.	11	C	0	2	Acc.	Lobby, mail room, laundry, toilets (Accessory to J-2)		
Fls.	2-14	40	-	12/FL	26	2	J <b>-</b> 2	12 Class "A" apartments pe floor		
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OPEN SPACE USES 13, 206. 34 Square Feet Parking 33 cars (12,040 Square Feet (SPECIFY-PARKING SPACES, LOADING BERTHS, OTHER USES, NONE)

Other Open Spaces: Planting, gardens, seating recreation (31,166.

NO CHANGES OF USE OR OCCUPANCY SHALL BE MADE UNLESS A NEW AMENDED CERTIFICATE OF OCCUPANCY IS OBTAINED

THIS CERTIFICATE OF OCCUPANCY IS ISSUED SUBJECT TO FURTHER LIMITATIONS, CONDITIONS AND

PECIFICATIONS NOTED ON THE REVERSE SIDE.

BOROUGH SUPERINTENDENT

COMMISSIONER

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STORM DE		E DISCHAR	GES INTO: 8) COMBIN	IED S	EWEF	C) PRIVATE SEWAGE DISPOSAL	SYSTEM	

was the same of the same that are not and the same of ZONE: R-8 (ZONING MAP (RC) SITE 2A, SEWARD PARK EXTENSION URBAN RENEWAL AREA 50 NORFOLK ST., NEW YORK, N.Y. ONING DATA 11 11 10/2" 68-B KEY TO SITE PLAN SYMBOLS HTEM 3-67.8-0 PROPUSED ALLOWABLE OR REFERENCE REQUIRED CALCULATIONS: PROPERTY LNE SITE AREA COVERAGE FLOOR AREA :44492 - DIAGRAM HERECN 309,2GI ALLOWABLE 23-14 Z.R. LOT COVERAGE : 9762,77 SQ.FT. 1764 17 50.FT СВ CATCH BASIN TOTAL FAR STOTAL FLOOR AREA-LOT AREA 150 418 TE LEGISLA 5.0" HEIGHT FACTOR : FLOOR AREA - GOVERAGE - 150 476 16 - 9762 77 - 14 YARD DRAIN BLUC LINE BFH FIRE HYDRAN OPEN SPACE SPACE - LOT AREA - COVERAGE - 9141.77 - 0016 OER O.S.R. . OPEN SPACE + FLOOR AREA - @ EXISTING LIGHT POLE FLIGWARLE FLOOR AREA - ALLOWABLE F.A.R. X LOT AREA - 5.81 X EFFECT - PM EXISTING SEVER MANHOLE COVER ALLOWABLE ZONING ROOMS = 44 SQ. FT. LOT AREA /RM. = 44492 -44 - LOTTIE TO CT.S. Осль. CON ED ELECTRICAL MANHOLE COVER 52.76 KEQUIRED 25-25 Z.R. (CHART) PARKING: 21% OF NO. OF APTS - 156 X 21% = 32. TO REQUIRED (33 PROVIDED)
PROJECT IS PUBLICALLY ASSISTED NON-PROFIT HOUSING FOR THE ELDERLY. 33 SPACES 67.55 5.0 35.35 8.0 7.87 EXISTING ELEVATION PROPOSED ELEVATION NEW EXTERIC LIGHTING BLOCK 351 LOTS: 35, 36, 40, 41, 42, 43, 44, 47, 48, 50, 51, 54, 86, 58, 59, 60, 63, 31,32 GROSS FLOOR AREA FOR HUD CALCULATIONS \_ wr WASTE RECEITACLE CROSS FLOOR AR A FROM DIAGRAM AT RIGHT + ARRA OF ELEVATOR BULLHEAD! [85 X 15 5] + [8.5 X 15.5] - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 165.75 + 17.5 (8.5 X 15.5) - 36075 + 17.5 (8.5 X 15.5) : GAME TABLE NO SEATS GROSS SQ. FT. SUSENIG 2.54 GROSS FLOOR AREA PER FLOOR - [69,0 X163-33]-1607 - 11269 - 507 - 97
X 14 FLOORS - 7,762.77 X 14 - 1806.070-70 50, FT. - GROSS FLOOR AREA BENCH, LENGTH AS NOTED AT GRAND STREET 5-37 . 916 2 17 SQ FT. A COVERAGE (WIDE ST.) 4" CONC. PAVIG SCORED 2"-0" O.C. BOTH WAYS E-P ON THINGBEE SHOWN THE O'C. MAX. SUFFOLK STREET ( DE-MAPPED) 4" CONC. PAVIS SCORED 4'-0"OC. BOTH WAYS. BUR JOINTS WHIZE SHOWN 28'-0"OC YOUR • TREE PIT F. B. SHRUB, SEE FANTING SCHEDULE ON THIS-DWG. GARDEN PLOT COAL. GRASS ON TO SOIL 14 GROUND COVE AREA O DELETED LOT 10-70 - 125.0 = 25.7 ob 30.0 = 1.35.0 = 25.7 ob 31.0 = 1.5 ob = 25.7 ob 31.0 = 1.5 ob = TREES, SEE PANTING SCHEDULE ASSHALT PAVING 6-0" HIGH CHA STEEL FENCE S NOTED SEE IA SEA SOF SEE DEENLES. PLANTING SCHEDULE NOTATION BOTANICAL NAME \$<del>-</del> 52: PROPOSED NEW 14 STORY RESIDENTIAL BUILDING T.S. IST FLOOR EL. 32.61 PLATANUS ACERIFOL O'-O"LINE OF ZERO COORDING GINKGO BILDEA C GLIDITSIA TRIACATHO INERMIS BETULA PENDUA GRACIÉ RHANNUS FRANCIULA COL COTONEASTER DIVARIGATE FORSITHIA INT. SHE GLOR 100 horo EDONTHUS KINJASCHOVICE KOLKWITZIA AMABILIS HODO, CAROLINIANUM P.J NORFOLK SINCET MERBERIS MENTORENSIS HEDERA WELLY "BALTICA ENVIYMUS FORT, WOUTUS! 5CA( E : 1'= 16'-0'

SKYPLANES -BLDG LINE AT NORFOLK STREET ( NARROW ST. ) SKYPLANE DIAGRAMS AUTHORIZATIONS BY CITY PLANNING COMMISSION AND THE BOARD OF ESTIMATE: AS PER PAR. 78-912 (3) Z.R. BY SPECIAL PERMIT TO AUTHORIZE MINOR VARIATIONS IN THE FRONT HEIGHT AND SETBACK REGULATIONS ON THE PERIPHERY OF SUCH DEVELOPMENT....AS REQUIRED BY 23-632 AND 23-64 Z.R.

25 PLANT 3'0 APART LANT G'-C' APART PLANT & APLET PLANT & APART

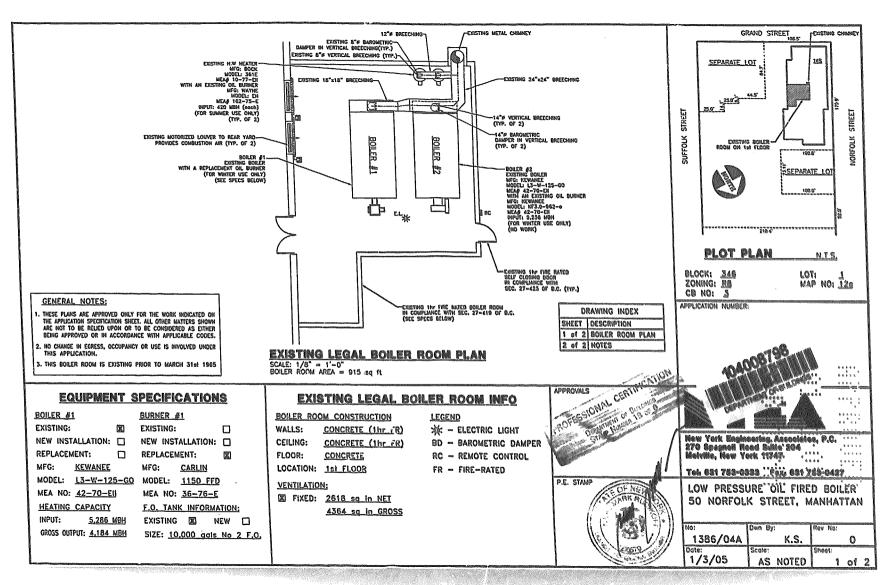
3 SEPT. F2 DIMENSIONS ACRES AT EXISTS BLDG CORNER LOT DELETED. 1 JULY 82 ZONING CALCULATIONS REVISED.

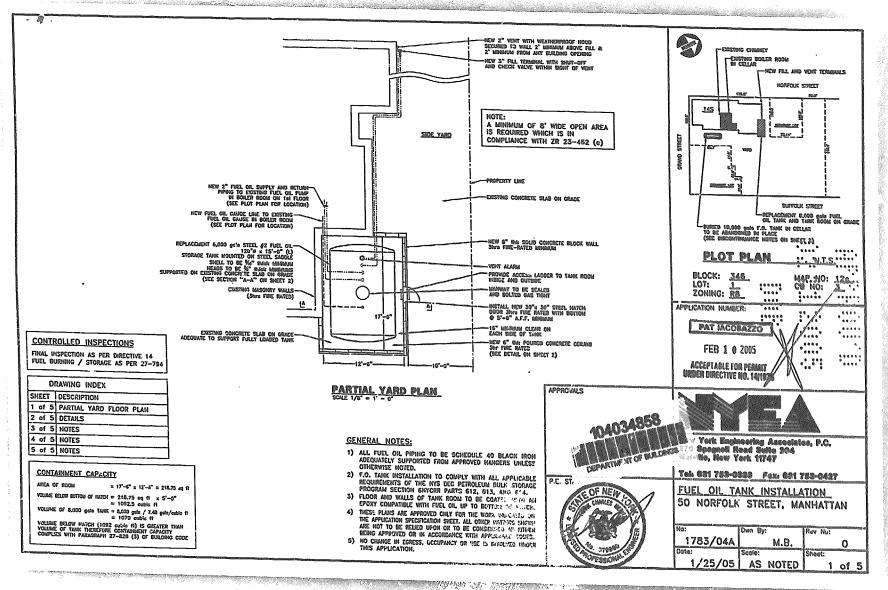
134 Sixth Avenue, New York N.Y. 10011 218-924-48

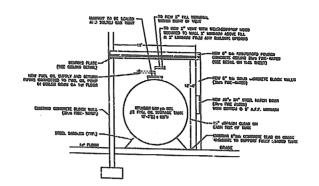
CHINATOWN PLANNING COUNCIL OPEN DOOR SENIOR CITIZENS' HOUSING



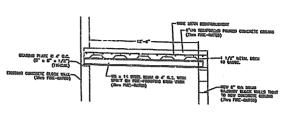
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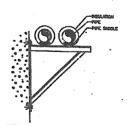






(DETAIL A)





- COPTES: ALL COMPONENCE TO ASSESSMENT YOUR SERVICE LAND.

  2. VIEW DESCRIPTION SERVICES FOR MOT LAND.

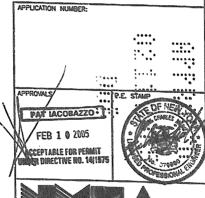
  3. FOR COPPER WORKS LAZ COPPER PLAND.

  MANUFACTURE SERVICES COPPER PLAND.

PER CURPORT DATAIL (DETAIL "C")

# GENERAL NOTES:

- These Plans are approved only for the work directed on the application specification sheet, all other matters shown are not to be relied upon or to be considered as either BEING APPROVED OR IN ACCORDANCE WITH APPLICABLE CODES.
- NO CHANGE IN EGRESS, OCCUPANCY OR USE IS INVOLVED UNDER THIS APPLICATION.



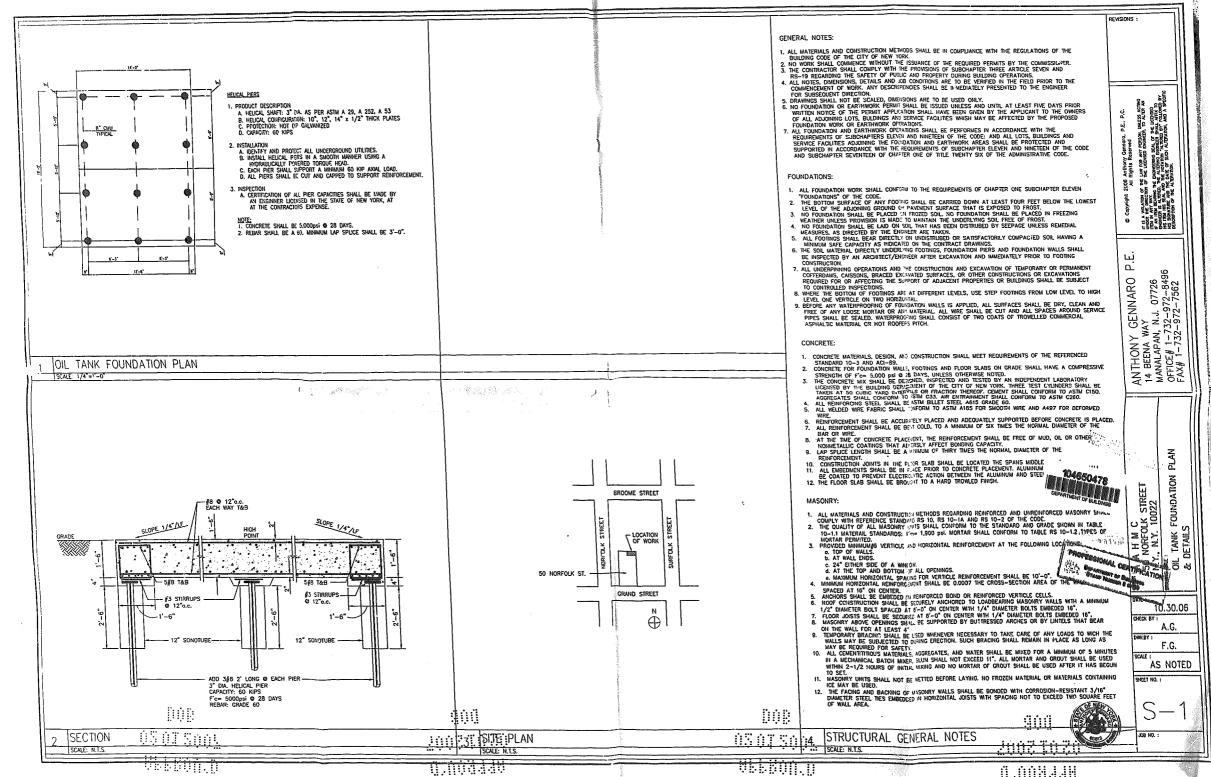


New York Engineering Accordance, P.G. 270 Spagnall Hood Sulta 204 Melville, New York 11747

Tel. 631 703-0033 Fam 631 733-0427

DETAILS 50 NORFOLK STREET, MANHATTAN

Na:	Den by:	I Rest Max
1783/048	M.D.	0
Dete:	Scale:	Sheet:
1/25/05	NONE	2 of 5



384 Grand Street (Block 346, Lot 95)



# DEPARTMENT OF BUILDINGS ALL. #231/83 CERTIFICATE OF OCCUPANCY

90477

BOROUGH HARRINGTAN

DATE: JUN 1 7 1987 NO.

ZONING DISTRICT R8

This certificate supersedes C.O. No.

THIS CERTIFIES that the new-altered-guisting-building-premises located at 384 Grand Street

Block 346 Lot 56

CONFORMS SUBSTANTIALLY TO THE APPROVED PLANS AND SPECIFICATIONS AND TO THE REQUIREMENTS OF ALL APPLICABLE LAWS, RULES, AND REGULATIONS FOR THE USES AND OCCUPANCIES SPECIFIED HEREIN

# PERMISSIBLE USE AND OCCUPANCY

	FEMALOSIDE OF AND OCCO, MIC.							
	STORY.	LIVE LOAD LBS PER SQ. FT.	MANINUM NO OF PERSONS PERMITTED	CHING DWELLING DWELLING	GUILDING COUS HABITAGLS ROOMS	BAINGS SUCAD 33U	GEONE BOOS BOILDING	DESCRIPTION OF USE
ellar		0.0.						Boiler room and storage
Lst		100		2	5	6/2	E/J2	2 Apartments and 3 stores
2nd		110		6	15	2	J2	6 Apartments Class "A"
3ra 'i		1 <sub>t</sub> O		6	15	2	J2	6 Apartments Class "A"
hth		1 <sub>4</sub> O		6	15	2'	J2	6 Apartments Class "A"
5th		110		6	15	2	J2	6 Apartments Class "A"
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The Committee of Cocupancy Must be Pould SALMIN THE BAILDING IN VCCOLDVNCE MILK THE KARA M I'M DEPARTMENT PROMULGATED WARRY MICK AND

**OPEN SPACE USES** 

ISPECIFY-PARKING SPACES, LOADING BERTHS, OTHER USES WONED

NO CHANGES OF USE OR OCCUPANCY SHALL BE MADE UNLESS A NEW AMENDED CERTIFICATE OF OCCUPANCY IS OBTAINED

THIS GERTIFICATE OF OCCUPANCY IS ISSUED SUBJECT

ICATIONS NOTED ON THE REVERSE SIDE.

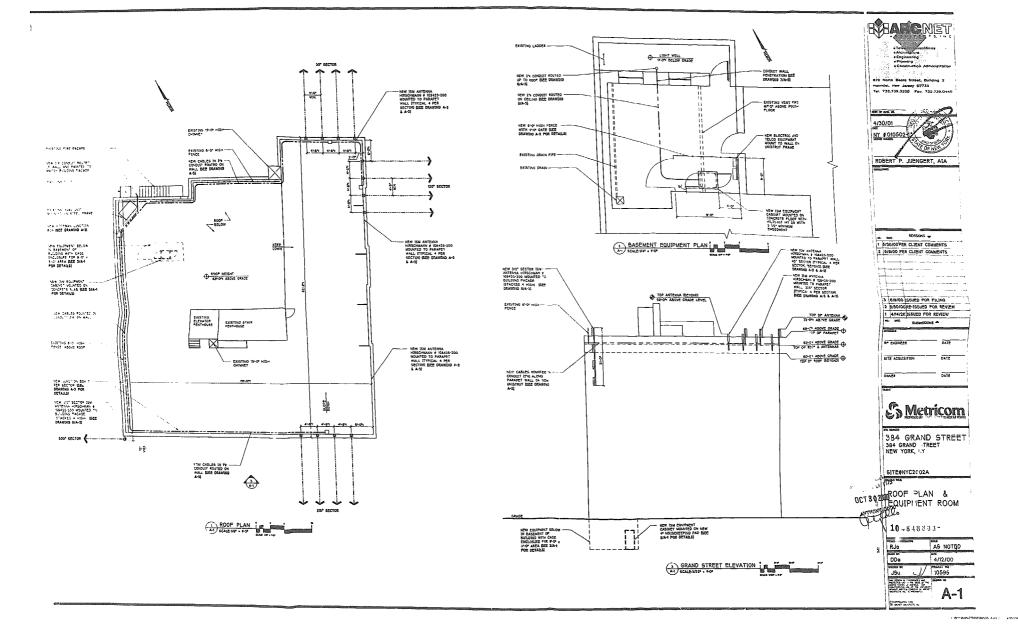
BOROUGH SUPERINTENDENT

COMMISSIONER

D ORIGINAL

☐ OFFICE COPY-DEPARTMENT OF BUILDINGS

☐ COPY



- DRAWNOS ARE DISSPANIATIC AND ROBERT CHIERRAL ARRANDATION OF PRINCIPAL PROPRIES CALL RECESSARY SUPPORTS, POPE, DITEGES AND VALVET HAT ARE REQUERD FOR COMMINION OF THIS WORK WITH ALL OTHER TRANS AND THE EXCENSE OF THE STATE
- CONFIRM SIZES AND LOCATIONS OF ALL EXISTING PIPING PRIOR TO
- CONNECTIONS TO EXISTING WORK. AT ODD HOURS TO INSURE MANNING INTERFERENCE WITH REGULAR OPERATION OF EXISTING FACULTIES BIAIN APPROVAL OF BUILDING MANAGEMENT PRIOR TO INSPECTION OR COMMENCING WORK
- PROVIDE WORKMANSHIP OF HIGHEST GRADE. INSTALLATION SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT BUILDING CODE
- PROVIDE ONE YEAR GUARANTEE AGAINST DEFECTIVE WORKWANSHIP AND MATERIAL
- ALL MATERIAL, PLUMBING ITEMS, ETC., SHALL BE NEW AND BEST OF ITS KIND, UNLESS OTHERWISE NOTED.

# SCOPE OF WORK

- ALL REQUIRED LABOR, MATERIALS AND CONTRACTOR'S SERVICES RECESSARY FOR COMPLETE SAFE INSTALLATION OF WORK IS FULL CONFORMITY WITH THE REQUIREMENTS OF ALL AUTHORITIES HAVING JURISDICTION, AS INDICATED ON DRAWINGS AND/OR HEREBY SECTIONS OF THE PROPERTY OF THE PROPE HEREW ST CIFIED, INCLUDING THE FOLLOWING
  - SANTARY DRAWAGE AND VENE SYSTEM, INCLUDING CONNECTIONS TO EXISTING PIPING, PLUMBING FIXTURES EQUIPMENT AND INDIRECT WASTE PIPING.
  - DOMESTIC WATER SUPPLY SYSTEM, INCLUDING CONNECTIONS TO EXISTING PIPING, VALVES, PLUMBING FIXTURES AND EQUIPMENT
  - 4. PLUMBING FIXTURE ROUGHING
  - PLUMBING EQUIPMENT
  - INSULATIO:
  - 7 ROUGH CUTTING AND PATCHING.
  - 8. SHOP DRAWNOS
  - 9 ACCESS DOORS
  - TO TESTING.
  - "I DISINFECTION OF POTABLE WATER SYSTEM.

# 3) GENERAL ITEMS

- AP CHAMBERS: 12 INCHES HIGH, PROMDE ON INDIMPUAL HOT AND COLD WATER BRANCHES TO FIXTURY J.
- SHOCK ABSORBERS: PROMDE ON MODIFICIAL HOT AND COLD WATER BRANCHES TO EQUIPMENT AND ON WATER LINES EQUIPPED WITH SOLENOID VALVES. J.R. SMITH SERIES 5000.
- C. ESCUTCHEONS: PROVIDE EXPOSED PIPMS BOTH BARE AND COVERED, WITH CP CAST BRASS ESCUTCHEONS ON BOTH SIDES WHEFE PASSING THROUGH CEILINGS, WALLS OR PARTITIONS.
- WY JUM BREAKERS. PROVIDE VACUUM BREAKERS SIMILAR 10 ZURM.
- VALVE TAGS AND CHARTS: PROVIDE 2 INCH SQUAPE, 188 AND S GAIDE ALUMINUM WITH STAMPED NUMBERED TAGS, FILL IN WITH BLACK PAINT. PROVIDE ON CONTROL VALVES EXCEPT THOSE CONTROLLING A SINGLE FIXTURE AND LOCATED ADJACENT TO XTURES. FASTEN WITH BRASS HOOK OR CHAIN. PROVIDE PRAMED CHARTS LISTING OPERATION OF EACH SYSTEM, LOCATION AND PURPOSE OF VALVES. COORDINATE WITH EXISTING BUILDING
- F. SLEDYS: PROVEE NO. 22 USSC CALVANIED FRON SLEDVES
  DEFLORED PHROUGH CONSTRUCTION IN COLUMNS, WALLS AND
  PARTITIONS. FOR INSULATION PRINCE, SEED TO ALON INSULATION
  TO PASS THROUGH SLEEVE. PROVIDE 1/2 INCH SPACE BETWEEN
  PRE AND/OR RISULATION AND SLEEVE. SEA, ALL SLEDYS IN
  ACCIDENMENTS.

# G Supports

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- SLEVEPT PERMIC AT DISTANCES AS REQUIRED BY CODE.
- ALL PIPING SHALL BE INDEPENDENTLY SUPPORTED FROM STRUCTURE. PIPE SUPPORTED FROM PIPE, CHAIN, STRAP, PERFORATED BAP, OR WIRE HANGERS VIID ARE NOT PERMITTED. SUPPORT PIPING FROM WALL BRACKETS WHERE
- FCP HORZONTAL PIPING 3 INCH AND LARGER, GALVANIZED "CLEVIS" HANGERS, GRIMBELL 260. TRAPEZE FOR MULTIPLE
- PIPING 2 BICH OR LESS PROVIDE ELECTROPLATED SOLID BAND HANGER, GRINNELL AUTO GRIP F730.

- SUPPORT HORIZONDAL BRANCH PIPING IN PIPE CHASES FROM WALL BRACKETS SUPPORTED FROM STRUCTURE
- SUPPORT MATERIALS: SAME AS PIPING MATERIAL, EXCEPT AL NOTED

## H ACCESS LIDORS

12 PL X 12 IN. NO. 4 FINSH STATRESS SIEEL FLUSH TYPE, LOCATE AND SET AFTER REVIEW. DOOR AND FRAME WITH METAL WINGS FOR KEYING INTO CONSTRUCTION WITH CONCEALED HINGES AND SCREWDRIVER OPERATED STAINLESS STEEL CAM LOCK. KARP STYLL DSC-214M OR KRP-150-FR IN TIRE RATED CONSTRUCTION.

### LABELING

ON ALL PIPING IN CEILING, 10 FT. ON CENTERS, INDICATING SYSTEM, SIZE AND DIRECTION OF FLOW.

# J. DISSIMILAR METALS

PROMDE BRASS OR COPPER WATER PIPING CONNECTED TO CALVANZED PIPE AND FOR ANY OTHER DISSIMILAR METALS WITH DIELECTRIC FITTINGS.

- SHOP DRAWINGS: BEFORE COMMENCING WORK, THE CONTRACTOR SHALL SUBUIT A LIST OF MANUFACTURE SUBSTITUTIONS, FOR REVIEW AND ACCEPTANCE. SUBSTITUTION WILL NOT BE ACCEPTED AT ANY OTHER TIME.
  - A) AFTER VERIFYING ALL FIELD CONDITIONS CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL
  - LAYOUT OF DRAWINGS SHOWING DEVIATIONS SHALL HAVE BEEN COORDINATED WITH THE OTHER TRADES AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMISSION FOR APPROVAL.
  - AT THE TIME OF EACH SUBMISSION CONTRACTOR IS TO IDENTIFY ANY DEVIATION FROM THE CONTRACT DOCUMENTS. DETERMINE DEVIATIONS FROM SPECIFICATIONS AND DRAWINGS AND INDICATE BY 'CLOUDING' ON SHOP DRAWINGS.
  - D) SUBMIT THE FOLLOWING:
    - (1) PIPE, FITTINGS AND JOINTS.
    - (2) VALVES.
    - (3) SUPPORTS
    - (4) PLUMBING FIXTURES, TRIM, TRAPS, ETC.
    - (5) INSULATION
    - (6) SLEEVES.
    - (7) SOLDER.

- A. SANIFARY DRAMAGE AND VENT PIPING
- SANITARY DRAWAGE AND VENT PIPING BON NO-HUB WITH COUPLINGS SIMILAR TO HUSKY SERIES AS PER SISPI 301 OR ASIM A888.

## B WATER PIDING

.) MATERIALS

- COPPER TUBING TIPE 'L' AND WROUGHT COPPER OR CAST BRONZE FITTINGS WITH 95-5 SOLDERED JOINES AS PER ASTM B88
- ALL EXPOSED WATER PIPING TO PLUMBING FIXTURES (AND KITCHEN/BAR EQUIPMENT) SHALL BE CHROWE PLATED.
- C. VALVES-DONESTIC WATER
  - SHUT-OFF VALVES: BROWZE RISING STEM GATE TYPF 125 PSI WSP, SOLDER JOINT END, STOCKHAM NO. B-109 OR BALL VALVES SIMILAR TO APOLLO SERIES.
- D. NON-LUBRICATED GAS COCKS
  - BRONZE, 125 PSI WOG, SIMILAR TO CRANE # 252.

- INSULATE WATER PIPING, VALVES 4 19. GENSITY FIBERGIASS ONE PIECE MOLDED SECTIONAL PIPE COVERING MAXIMUM K FACTOR. 0.26 AT 75 DEG F MEAN
- FIRE RETARDANT ALL SERVICE JACKET ON PIPE INSULATION.
  MAXIMUM FLAMESPREAD AND SMOKE RATING 25/50.
- VALVES AND FITTINGS. PREMOLDED FIGERGLASS FITTINGS OR RADAL MITTERD SECURITIS OF PIPE INSULATION TAPED ON, PISULATION TAPED ON, PISULATION CENENT, FIBERCLASS REINFORCING CLOTH, AND MASTIC. VAPOR SEAL RISULATION ON CW PIPING, THICKNESS, ALLE AS REGISTED OF THE PIPE OF THE OFFICE OFFICE OFFICE OFFICE OFFICE OFFICE OFFICE O SAME AS REQUIRED ON PIPING.
- D. CW PIPING: 1/2 INCH THICK FIBERGLISS SECTIONAL PIPE COVERING WITH VAPOR BARRIER JACKET. NO STIPLES PERMITTED.
- E. HW PIPING: 1 INCH FIBERCLASS SECTIONAL PIPE COVERING.
- F. HEAT TRACING IS PROVIDED BY ELECTRICAL CONTRACTOR.

1" CW 3/47-W UD

- A. TEST ALL SYSTEMS AS REQUIRED BY BUILDING CODE.
- DISINFECTION OF POTABLE WATER SUPPLY SISTEM
- DISINFECT DOMESTIC WATER PIPING AS REQUIRED BY BUILDING
- 6) WORK NOT INCLUDED
  - A. FINAL PAYCHING AND PAINTING
- 7) PLUMBING FIXTURES
  - A. ALL PLUMBING FIXTURES WILL BE SPECIFED BY ARCHITECT.
  - B. PROVIDE CHAIR CARRIER SUPPORT FOR YEW HANDICAPPED WATER CLOSET WALL HUNG LAVATORY-SIMILAR TO J.R. SMITH

# ADDITIONAL CONSIDERATIONS:

- ANY EXISTING PIPING SYSTEMS REQUIRING MODIFICATIONS AND/OR ALTERATIONS WILL REQUIRE FIELD VERIFICATION PRIOR TO WORK STARTING, THE NEW INSTALLED PIPING SHALL BE ISOLATED FROM THE EXIST'G SYSTEM DURING THE PERFORMANCE OF REQUIRED
- PRIOR TO FINAL PIPING CONNECTIONS TO EXIST'C PIPING, ALL NEW PIPING SHALL BE THOROUGHLY BLOWN 201 AND FLUSHED AT LEAST WICE, TO REMOVE ALL ACCUMULATION OF DRIT AND DEERIS.
- 4. PROVIDE WATER HANNER APPRESTOR ON EQUIPMENT WITH QUECK CLUSING
- 5. PROVIDE AND INSTALL PEQUIRED AR VENTS AT HIGH SPOTS  $\mathcal W$  THE PIPING
- 6 SANTARY/CREASE LINES SHALL BE "EFRODICALLY TREATED WITH A BACTIFIA ENZYMES TO KEEP THE LINES FREE FLOWING.
- 7. PROVIDE SUITABLE SIGN INDICATING LOCATION WHERE CONTROL VALVES ARE
- 8. PROVIDE SECONDARY CONTAINMENT PAN FOR HOT WATER HEATER AND PIPE RELIEF TO MOP SINK

GREASE TRAPS SHALL BE COMPLETELY ACCESSIBLE FOR CLEANING AND MAINTENANCE

FLOOR DRAIN (FD) - J.R. SMITH OR APPROVED EQUAL, PROVIDE 3" DRAIN, BRONZE STRAINER, PROVIDE FLASHING TO EX. FLR SLAB.

FLOOR SINK (FS) – J.R. SMITH OR APPROVED EQUAL  $8^*x8^*x8^*$  PORCELAIN, WITH  $3^*$  WASTE LINE. PROVIDE FLASHING TO EXIST'G FLR SLAB.

FLOOR DRAINS/SINKS SHALL BE VISIBLE AND EASILY ACCESSIBLE FOR CLEANING AND MAINTENANCE WATER CONNECTION NOTE:

- PROVIDE STOP VALVES AT ALL WATER CONNECTIONS TO PIXTURE AND EQUIPMENT.
   PROVIDE VACUUM GREAKERS AT WATER SUPPLY CONN. TO ICE MAKER, COFFEE MAKERS AND MOP SINK.
- 3. PROWDE HOT WATER MIXING VALVE AT ALL HAND SINKS.
  HW DISCHARGE TEMPERATURE TO BE SET AT 110 DEGREES F.
  4. DOMESTIC HOT WATER HEATER SHALL BE SET TO 140 DEGREES F.

# TWO BOOTS

OWNER PHILIP HARTMAN Avinue A 10003 Te: 212-XXX-XXXX

384 GRAND STREET NEW YORK, NY 10002

DESIGN ARCHITECT GLEN COBEN ARCHITECTURE + DESIGN PLLC

13 Eard 37th Street new scribing 10016 161 212 685 2770 #AX 212 689 1033

CONSULTANTS

Rakshan Malek Associates

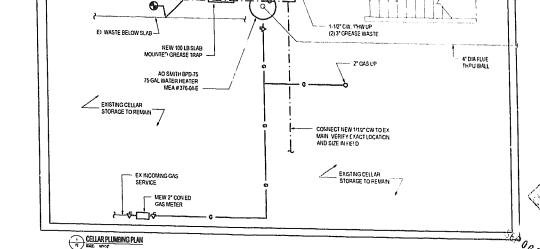
26 Oza Road Saddle Rhen füll (1258 YEL 217 818 (265 PRAS (1898) (2678)

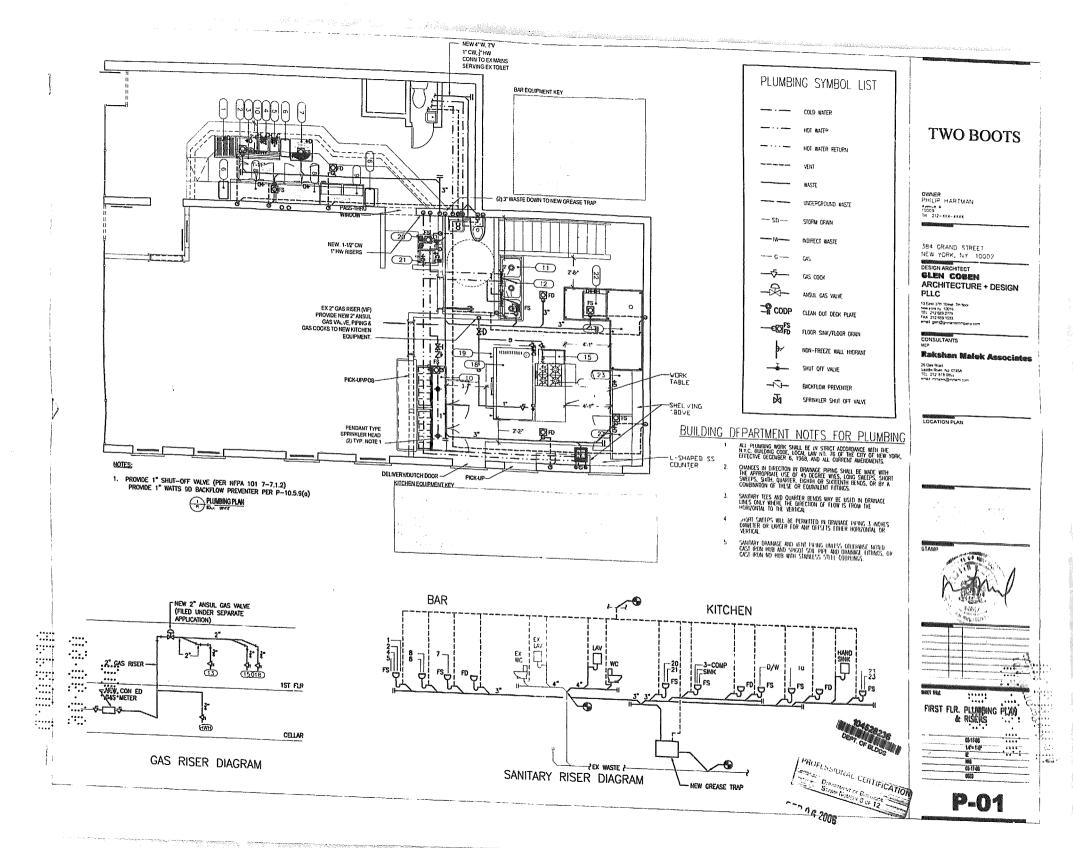
LOCATION PLAN



ii J CELLAR PLUMBING PLANE & NOTES THOP SOUTH CONTROL TON 14:146 ..... 14214 ....

P - 02





### **APPENDIX C**

**BHH Synagogue Stabilization Drawings** 

## EMERGENCY STABILIZATION

(DOB JOB #: 123199653)

# 60 NORFOLK STREET

NEW YORK, NY 10002



(212) 461-1987

ISSUE

BETH HAMEDRASH HAGADOL (BHH) **60 NORFOLK STREET** NEW YORK, NY 10002

MASPETH, NEW YORK 11378

'ABILIZ STREE 1Y 10002 000 EMERGENCY 60 NORF( NEW YOF

REVISIONS DATE: DESCRIPTION \_02/28/19 LPC NEW LIMITS

DOB ADDRESS: 60 NORFOLK STREET NEW YORK, NY 10002 BLOCK: 346 LOT: 37

ZONING DISTRICT: R8 BIN #: 1004155 HEIGHT: 1 STORY @ 65'-5"±

DOB USE ONLY

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**COVER SHEET** 

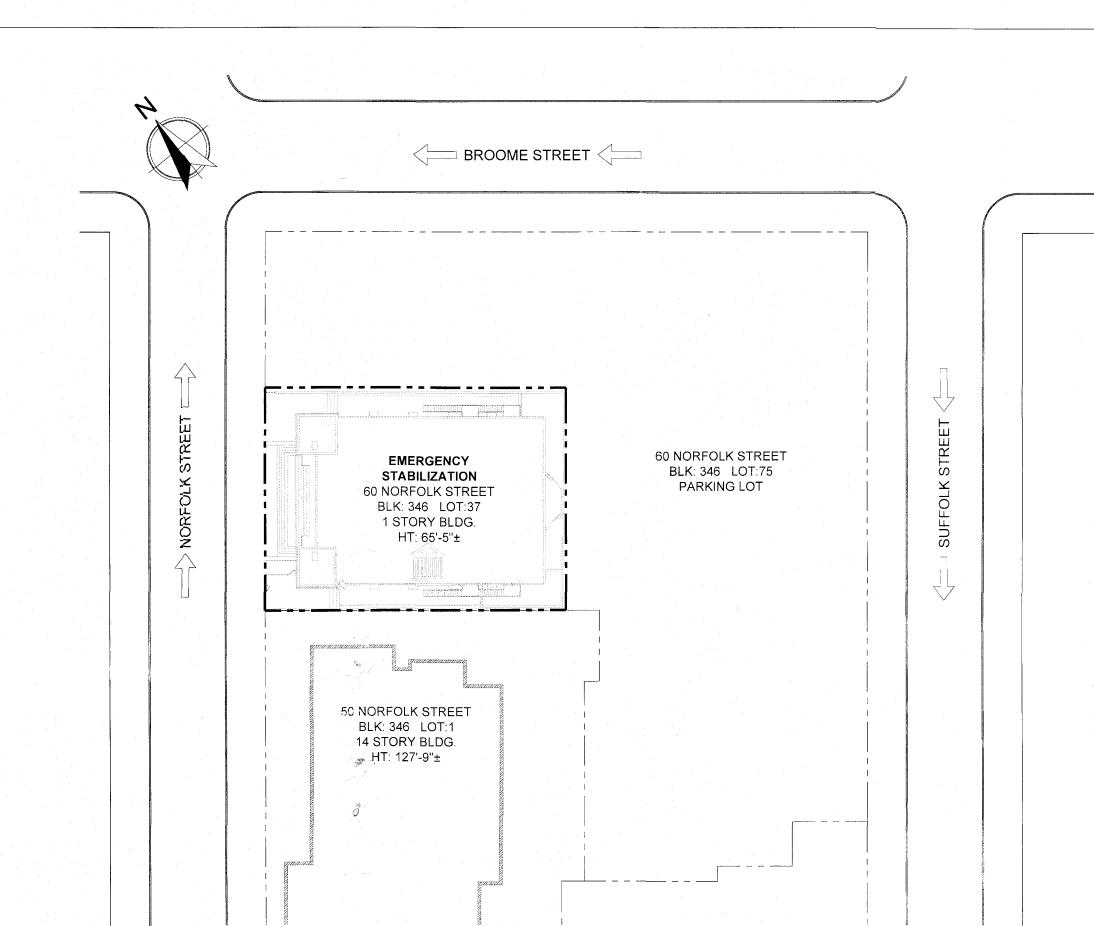
APPLICANT: JAMES M. PATTERSON, P.E. Sectional the OPEN HIMM Professional Certification

MANHATTAN

MAR 0 7 2019

PROJECT NO: BETHAM1827 DRAWN BY: R.F. CHECKED BY: J.P. DWG NUMBER:

OT-000.01 CADD FILE NAME:



## SITE INFORMATION

COMPLETED

1. THE NYC BUILDING CODE REQUIRES A SPECIAL INSPECTOR TO PERFORM A MINIMUM OF

3. ALL WORK CONTAINED HEREIN SHALL BE SUBJECT TO SPECIAL INSPECTION IN ACCORDANCE

4.2. MECHANICAL DEMOLITION (BC 1704.20.4 & BC 3306.6)

4.4. POST-INSTALLED ANCHORS (BB# 2014-018, 2014-019, BC 1704.32)

2.7. STRUCTURE DESCRIPTION: THE BUILDING IS CONSTRUCTED OF MASONRY LOAD BEARING

THREE (3) SPECIAL INSPECTIONS, ONE BEFORE, DURING AND AFTER DEMOLITION 2. CONTRACTOR SHALL NOTIFY ENGINEER 72 HOURS PRIOR TO START OF WORK AND SHALL PROVIDE ENGINEER WITH REGULAR PROGRESS NOTIFICATION UNTIL STRUCTURAL WORKS IS WITH CHAPTER 17 OF THE NEW YORK CITY BUILDING CODE. 4. REQUIRED SPECIAL INSPECTIONS INCLUDE: 4.1. STRUCTURAL STABILITY - EXISTING BUILDINGS (BC 1704.20.1)

LEGEND

INDICATES LANDMARKED STRUCTURE MONITORING TO BE PROVIDED IN ACCORDANCE WITH TPPN 10/88

PAGE # DWG #

GENERAL OT-000.01 COVER SHEET

DRAWING INDEX

DESCRIPTION

OT-001.01 | PLOT PLAN & NOTES OT-002.00 NYC BLDG. CODE

SURVEYING AND MONITORING OT-003.00 | MONITORING PROTOCOL

**EXISTING CONDITIONS** OT-004.00 EXISTING LOGISTICS PLAN

DEMOLITION SITE LOGISTICS PLAN

SITE LOGISTICS ELEVATIONS A & B OT-102.00 SITE LOGISTICS ELEVATIONS C & D OT-103.00 SITE LOGISTICS SECTIONS A & B OT-104.00 SITE LOGISTICS SECTIONS C

OT-105.00 DEMO. PLAN OT-106.00 DEMO. ELEVATIONS A & B OT-107.00 DEMO. ELEVATIONS C & D OT-108.00 DEMO. SECTIONS A & B

OT-109.00 DEMO. SECTION C

OT-110.01 DEMO. PLAN OT-111.01 DEMO. SECTIONS A & B OT-112.00 | LPC OT-113.00 LPC

OT-114.00 LPC DETAILS OT-200.00 TYPICAL DETAILS I

MECHANICAL EQUIPMENT

OT-300.00 MECHANICAL EQUIPMENT SPECIFICATIONS

4.3. FINAL (28-116.2.4.2, BC 110.5, DIRECTIVE 14 OF 1975, AND 1 RCNY §101-10)

WALLS/PIERS; A WOOD FRAME ROOF STRUCTURE; WOOD FRAME MEZZANINE AND GROUND FLOOR; AND MASONRY OR CONCRETE FOUNDATION. 2.8. **BUILDING CONDITION**. THE BUILDING EXPERIENCED A 3X ALARM FIRE ON MAY 14, 2017 RENDERING PORTIONS OF THE BUILDING UNSTABLE AND STRUCTURAL UNSOUND. DEMOLITION OPERATIONS SHALL BE PERFROMED AS EMERGENCY WORK ORDER ORDER

OF DOB PER CLASS 1 VIOLATION CONTROL No. 334695. 3. BASE DRAWINGS REPRODUCED FROM AS-BUILT PLANS PREPARED BY LI SALTZMAN ARCHITECTS, P.C. ARCHITECTURE AND RESERVATION, 62 WHITE STREET, NEW YORK, NY 10013.3598. DATED (-). ALL LOCATIONS ARE APPROXIMATE AND ARE SUBJECT TO VERIFICATION IN THE FIELD.

1. THE PLANS ENCLOSED DEPICT THE EXTENT, SEQUENCE, MEANS AND METHODS FOR THE

AND STRUCTURAL UNSOUND PORTIONS OF THE BUILDING TO A SAFE LEVEL.

2.4. LOT SIZE: 7,438 SF; LOT FRONTAGE: 100.75 FT; GROSS FLOOR AREA: 4,600 SF

PROPOSED EMERGENCY STABILIZATION OF THE SYNAGOGUE BUILDING LOCATED AT 60

NORFOLK STREET, NEW YORK, NY. THE OBJECTIVE OF THE WORK IS TO REMOVE UNSTABLE

**GENERAL NOTES** 

2. BUILDING/PROPERTY DESCRIPTION

2.1. BLOCK: 346; LOT: 37; BIN: 1004155

2.6. THE BUILDING IS CURRENT VACANT

2.3. BUILDING HEIGHT: 1 STORY 66 FT ± HIGH

2.5. LAND USE: PUBLIC FACILITIES AND INSTITUTIONS

2.2. ZONING DISTRICT: R8; MAP 12C

PLOT PLAN SCALE: 1/32"=1'-0"

#### **MEANS AND METHODS**

THE PLANS ENCLOSED DEPICT THE EMERGENCY STABILIZATION OF THE 1 STORY BUILDING LOCATED AT 60 NORFOLK STREET, BLOCK: 346, LOT: 37, BIN: 1004155. DEMOLITION BY HAND MEANS. THE ENGINEER SHALL BE NOTIFIED BY THE CONTRACTOR OF ANY DEVIATION FROM THE APPROVED PLAN OR UNEXPECTED CONDITIONS.

#### PRE-STABILIZATION

THE FOLLOWING MEASURES SHALL BE COMPLETED PRIOR TO THE START OF

DEMOLITION: 1. PROVIDE CONSTRUCTION SIGNAGE, SOLID CONSTRUCTION FENCE, SIDEWALK SHED, PIPE SCAFFOLDING, PIPE SCAFFOLDING ON NEEDLE, DEBRIS NETTING AND OTHER SAFETY PROTECTION TO ESTABLISH A SAFETY ZONE AROUND THE DEMOLITION AREA AS SHOWN IN THE PLANS

#### **DEMOLITION SEQUENCE**

- PROVIDE CONTROLLED ACCESS SAFETY ZONE AS SHOWN. 1.1. MAINTAIN CONTROLLED ACCESS SAFETY ZONE AT A MINIMUM
- HORIZONTAL DISTANCE EQUAL TO ONE HALF THE HEIGHT OF THE BUILDING (2V:1H). 1.2. DELINEATE BOUNDARY OF CONTROLLED ACCESS SAFETY ZONE
- BY USE OF SOLID CONSTRUCTION FENCE, MOVEABLE BARRIERS, OR STANDARD GUARDRAIL AND A FLAG MAN DIRECTING
- 1.3. NO PERSONNEL PERMITTED WITHIN CONTROLLED ACCESS SAFETY ZONE DURING MECHANICAL DEMOLITION EXCEPT ESSENTIAL PERSONNEL, E.G., CONTRACTORS PERFORMING DEMOLITION
- 2. CLEAR BUILDING AND CONTROLLED ACCESS SAFETY ZONE OF ALL PERSONNEL EXCEPT WORKERS PERFORMING DEMOLITION.
- 3. DEMOLITION 3.1. DEMOLITION SHALL PROGRESS FROM THE TOP DOWN, STARTING AT THE HIGHEST POINT OF THE STRUCTURE.
- 3.2. DEMOLITION SHALL BE PERFORMED BY HAND, OPERATING BY ARTICULATED BOOM LIFT, UTILIZING CONVENTIONAL HAND TOOLS. WORKERS ARE PROHIBITED FROM WORKING BELOW
- STRUCTURALS TO TO BE DEMOLISHED. 3.3. EXCAVATORS MAY BE USED TO ASSISTING HAND DEMOLITION BY GRASPING STRUCTUAL MEMBERS, .E.G., WOOD /STEEL, TO FACILIATE CUTTING BY HAND; AND LOWERING CUT STRUCTUAL MEMBERS. STRUCTURAL MEMBERS SHALL NOT BE PULLED OR TWISTED BY THE EXCAVATOR.
- DEBRIS 4.1. DEBRIS SHALL BE FALL TO GRADE; AND WHERE POSSIBLE AT THE
- EXTERIOR OF THE BUILDING. 4.2. ALL DEBRIS SHALL BE CLEARED FROM THE SITE.
- UPON COMPLETION OF DEMOLITITION OPERATIONS THE ENGINEER OF RECORD SHALL PERFORM A STRUCTURAL STABILITY ASSESSMENT AND DETERMINE STABILIZATON MEASURES, E.G., SHORING AND BRACING, NECESSARY TO MAINTIN STRUCTURAL STABILITY.
- 4. THE ENGINEER SHALL BE NOTIFIED BY THE CONTRACTOR OF ANY DEVIATION FROM THE APPROVED PLAN OR UNEXPECTED CONDITIONS **DEMOLITION REQUIREMENTS**
- NOTIFICATIONS. PRIOR TO THE COMMENCEMENT OF DEMOLITION OPERATIONS, AND NOT LESS THAN 72 HOURS IN ADVANCE, THE PERMIT HOLDER SHALL NOTIFY THE FOLLOWING:
- 1.1. DOB, FORESNIC ENGINEERING UNIT, 280 BROADWAY, 4TH FLOOR, NEW YORK, NY. 10007, ATTN: ANTHONY DEVITO, 212-393-2137 EMAIL: ADEVITO@BUILDINGS.NYC.GOV.
- 1.2. FDNY, BUREAU OF FIRE INVESTIGATION 1.3. LANDMARKS PRESERVATION COMMISSION (LPC).
- 1.4. ENGINEER OF RECORD, DOMANI TECHNICAL & DESIGN SERVICES. 2. ALL ALTERATION, DEMOLITION AND STABILIZATION WORK SHALL BE PERFORMED UNDER FULL TIME SUPERVSION OF A PROFESSIONAL ENGINEER AS PER NYC DOB CONTROL NO. 334695 DATED 11/03/2017.

### FINAL CONDITIONS

- 1. THE REMAINING STRUCTURE WILL REMAIN IN PLACE TO BE INCORPORATED INTO THE PROPOSED NEW CONSTRUCTION WHICH IS CURRENTLY IN PRELIMANRY DESIGN PHASES.
- 1. STRUCTURE(S) LEFT-IN-PLACE
- 1.1. PERIMETER BRICK WALLS AND TOWER WALLS.
- 1.2. CONCRETE SLAB-ON-GRADE 1.3. SUB-GRADE FOOTINGS AND FOUNDATIONS.
- 2. ALL DEBRIS SHALL BE CLEARED FROM THE SITE.
- 3. BACKFILL REQUIREMENTS
- 3.1. BACKFILL SHALL BE COMPRISED OF CONCRETE/MASONRY DEMOLITION DEBRIS (FREE OF ORGANIC MATERIAL) AND/OR
- 3.2. PROVIDE 1V:3H BERM AGAINST PERIMETER WALLS AS SHOWN AT INTERIOR OF BUILDING FROM CELLAR SLAB-ON-GRADE UP TO MATCH EXTERIOR GRADE ELEVATION. 3.3. GRADE TO MATCH EXISTING CONTOURS, DRAIN THE LOT,
- ELIMINATE POCKETS IN THE FILL AND PREVENT THE ACCUMULATION OF WATER.
- 4. CONTRACTOR TO PROVIDE GUARDRAIL AT LEADING EDGE, AT LIMITS OF CONTROLLED ACCESS ZONE; AND AT TOP OF BERMS LESS THAN A THIRTY-THREE PERCENT SLOPE (1V:3H) NYC BC 3304.4.4. 5. SITE DRAINAGE
- 5.1. PRIOR TO BACKFILL, THE CONTRACTOR MUST CRACK/PERFORATE THE CONCRETE SLAB-ON-GRADE TO PERMIT FREE DRAINAGE AND PREVENT THE ACCUMULATION OF WATER.
- 6.1. PROVIDE A 1V:4H EARTH RAMP, IN ACCORDANCE WITH NYC BC REQUIREMENTS, FROM CELLAR SLAB-ON-GRADE TO EXTERIOR GRADE.
- 7. WATERPROOFING
- 7.1. EXPOSED TOP OF WALLS SHALL BE WATERPROOFED. 7.2. CLEAR DEBRIS, DUST AND LOOSE MATERIAL FROM TOP OF WALL.
- 7.3. APPLY SELF ADHESIVE BITUTHENE MEMBRANE ALONG TOP OF ALL MASONRY WALLS.
- 8. INSPECTIONS.
- 8.1. IF THE PROPOSED NEW CONSTRUCTION DOES NOT BEGIN WITHIN THREE MONTHS OF COMPLETION OF DEMOLITION OPERATIONS A REGISTERED DESIGN PROFESSIONAL MUST PERFORM PERIODIC INSPECTIONS OF STRUTCTURAL STABILITY AS PER NYC BC 3303.13.3.
- 9. THE APPROVED DM PLANS WILL BE PROVIDED TO THE OWNER, ILLUSTRATING SITE CONDITIONS AT THE COMPLETION OF DEMOLITION, TO BE INCORPORATED INTO THE SUPPORT-OF-EXCAVATION DESIGN FOR NEW DEVELOPMENT.

#### MECHANICAL EQUIPMENT USE RESTRICTIONS

- 1. PROFESSIONAL ENGINEER OR REPRESENTATIVE MUST BE ON SITE AT START OF WORK TO OBSERVE STRUCTURE'S BEHAVIOR DURING MECHANICAL EQUIPMENT OPERATIONS.
- 2. MECHANICAL EQUIPMENT OF SIMILAR WEIGHT AND DIMENSIONS OF THOSE LISTED ABOVE MAY BE SUBSTITUTED FOR USE AS APPROVED BY THE ENGINEER OF RECORD.
- 3. MECHANICAL EQUIPMENT MUST BE OPERATED BY A QUALIFIED OPERATOR EXPERIENCED IN THE USE OF THE MACHINE.
- 4. NO MECHANICAL EQUIPMENT OPERATION PERMITTED WITHIN "NO LOAD ZONE" AS SHOWN IN PLANS 5. MECHANICAL EQUIPMENT SHALL MAINTAIN A MINIMUM SEPARATION
- DISTANCE OF 1 COLUMN BAY OR 20 FT BETWEEN MACHINES AT ALL TIMES. WHICHEVER IS GREATER S MECHANICAL EQUIPMENT SHALL NOT BE OPERATED IN A MANNER
- THAT A WHEEL IS UPLIFTED OR IN A MANNER THAT IMPARTS EXTREME DYNAMIC LOADING ON THE DECK. . ANY EVENT, CHANGE OF METHOD OR CONDITION THAT PROMPTS THE CONCERN ABOUT THE INTEGRITY OF THE STRUCTURE TO SUPPORT
- THE EQUIPMENT SHALL BE REPORTED TO THE ENGINEER OF RECORD IMMEDIATELY AND WORK SHALL NOT PROCEED IN SUSPECT AREA(S) ARTICULATED BOOM LIFTS
- 8.1. SHALL BE OPERATED IN STRICT ACCORDANCE WITH THE MANUFACTURE'S RECOMMENDATIONS AND BY TRAINED AND COMPETENT OPERTORS.
- 8.2. SHALL BE USED TO FACILITATE HAND DEMOLITION OPERATIONS. 8.3. ARE STRICTLY PROHIBITED FOR LOWERING DEBRIS TO GRADE.
- EXCAVATOR(S) MAY BE USED FOR THE FOLLOWING OPERATIONS
- 9.1. OPERATIION ON GRADE/BACKFILL ONLY 9.2. LOAD OUT OF DEBRIS, EQUIPMENT OR OTHER MATERIALS
- 9.3. ASSISTING HAND DEMOLITION BY GRASPING STRUCTUAL MEMBERS, .E.G., WOOD /STEEL BEAMS/COLUMNS, TO FACILIATE CUTTING BY HAND; AND LOWERING CUT STRUCTUAL MEMBERS. STRUCTURAL MEMBERS SHALL NOT BE PULLED OR TWISTED BY
- THE EXCAVATOR. 9.4. CRACKING CONCRETE SLAB-ON-GRADE
- 9.5. BACKFILL AND GRADING 9.6. OTHER OPERATIONS APPROVED BY PROFESSIONAL ENGINEER ON-SITE.
- 10. SKID STEER(S) MAY BE USED FOR THE FOLLOWING OPERATIONS 10.1.OPERATION ON GRADE/BACK FILL ONLY
- 10.2.LOAD OUT OF DEBRIS, EQUIPMENT OR OTHER MATERIALS 10.3.BACKFILL AND GRADING
- 1. ANY EVENT, CHANGE OF METHOD OR CONDITION THAT PROMPTS THE CONCERN ABOUT THE INTEGRITY OF THE STRUCTURE TO SUPPORT THE EQUIPMENT SHALL BE REPORTED TO THE ENGINEER OF RECORD IMMEDIATELY AND WORK SHALL NOT PROCEED IN SUSPECT AREA(S)

#### ONG/HIGH REACH EXCAVATOR USE RESTRICTIONS

- SHALL BE OPERATED IN STRICT ACCORDANCE WITH THE MANUFACTURE'S RECOMMENDATIONS AND BY TRAINED AND COMPETENT OPERTORS.
- HIGH REACH EXCAVATORS SHOULD BE OPERATED FACING IN FRONT OF THE TRACKS, NOT SIDE-ON. SIDE-ON WILL RESULT IN ROLLOVER OR OVERLOADING. OPERATOR MUST AVOID OVERLOADING WORKING TOOL AS THIS CAN CAUSE A ROLLOVER.SOME LONG-REACH EXCAVATORS ARE FITTED WITH A BOOM ANGLE WARNING SYSTEM TO ALERT OF THIS POSSIBILITY.
- 3. MACHINES ARE TO BE FITTED WITH ROPS/FOPS. FULL WRITTEN INSPECTION CHECKS MUST BE PERFORMED DAILY BY A COMPETENT PERSON BEFORE USE.
- 4. ANY EVENT, CHANGE OF METHOD OR CONDITION THAT PROMPTS THE CONCERN ABOUT THE INTEGRITY OF THE STRUCTURE TO SUPPORT THE EQUIPMENT SHALL BE REPORTED TO THE ENGINEER OF RECORD IMMEDIATELY AND WORK SHALL NOT PROCEED IN SUSPECT AREA(S) 5. HIGH REACH MACHINES ARE NOT DESIGNED AS CRANES, NOR SHOULD THEY BE USED TO CARRY OUT ANY FORM OF LIFTING DUTIES EXCEPT WHERE THEY ARE REQUIRED TO GRAB ELEMENTS OF THE STRUCTURE
- BEING DEMOLISHED. 5.1. ATTACHMENTS SHOULD BE MATCHED TO THE WEIGHT AND TYPE
- OF WORK CARRIED OUT. 5.2. MACHINES HAVE SAFE OPERATING LIMITS AND DISTANCES WHICH MUST BE ADHERED TO IN ORDER TO CARRY OUT DEMOLITION.
- 6. WORKING PLATFORM MUST BE CHECKED FOR STABILITY BEFORE ANY WORK CAN BE PERFORMED TO PREVENT ROLLOVERS. ANY RAMPS OR WORKING PLATFORMS CONSTRUCTED FROM RECYCLED MATERIAL SUCH AS CONCRETE AND STONE, SHOULD BE INSPECTED BY THE OPERATOR REGULARLY TO IDENTIFY ANY PROTRUDING STEEL OR REINFORCING THAT MAY OBSTRUCT OR LOCK UP IN THE EXCAVATOR
- TRACKS. THE OPERATOR MUST ENSURE THAT THE MACHINE REMAINS WITHIN THE PRESCRIBED SAFE WORKING RADIUS AT ALL TIMES. RADIUS CHARTS SHOULD BE AVAILABLE FOR THE OPERATOR TO REFER TO AT ALL TIMES WITHIN THE CABIN OF THE EXCAVATOR.
- THE OPERATOR MUST WATCH THE FALL ZONE, AS FALLING DEBRIS/STEEL CAN SPEAR OR BOUNCES TOWARDS OPERATING POSITION.
- 8.1. IT IS IMPORTANT TO WORK WITHIN THE SAFE DISTANCES OF THE MACHINE AND ALSO MAINTAIN A FALL ZONE FOR THE DEBRIS TO
- TRAVEL TO THE GROUND. 8.2. A TIER SYSTEM AND PERIODIC CLEANING OF DEBRIS FROM FALL

#### EQUIPMENT DISTE-BYANDOTO OLIS MACHINE MAY BE REQUIRED.

- ELECTRIC CHIPPING GUN
- JACK-HAMMER
- 3. OXY-ACETYLENE BURNING OUTFIT
- 4. ELECTRICAL GRINDER
- 5. GAS-POWERED DEMOLITION SAW
- 6. CHAINSAW
- GENERATOR
- 8. COMPRESSOR
- 9. CHAIN FALL
- 10.CONCRETE SAW
- 11.HAND GANTRY
- 12.DRILL
- 13. MISC. HANDTOOLS

#### **EQUIPMENT LIST - MECHANICAL EQUIPMENT**

TYPE MAKE/MODEL OPERATING WEIGHT (LBS) FLOOR LEVEL PLACEMENT SEPARATION DISTANCE FOUPMENT OR OTHER MATERIALS FULL MECHANICAL MECHANI					MIN.	USE DESCRIPTION							
MINI EXCAVATOR         BOBCAT 329         7,098         GRADE ONLY         NA         •            MINI EXCAVATOR         BOBCAT 331         7,185         GRADE ONLY         NA         •            SKID STEER LOADER         BOBCAT 570         2,795         GRADE ONLY         NA         •            SKID STEER LOADER         BOBCAT 5100         4,091         GRADE ONLY         NA         •            SKID STEER LOADER         BOBCAT 226B         5,822         GRADE ONLY         NA         •            SKID STEER LOADER         BOBCAT 5150         5,935         GRADE ONLY         NA         •            MINI EXCAVATOR         CAT 301.8C         3,936         GRADE ONLY         NA         •         •	ТҮРЕ	MAKE/MODEL	OPERATING WEIGHT (LBS)	FLOOR LEVEL PLACEMENT	SEPARATION	EQUIPMENT OR OTHER	MECHANICAL						
MINI EXCAVATOR         BOBCAT 331         7,185         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT S70         2,795         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT S100         4,091         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT 226B         5,822         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT 5150         5,935         GRADE ONLY         NA         •         MINI EXCAVATOR         CAT 301.8C         3,936         GRADE ONLY         NA         •         •	MINI EXCAVATOR	BOBCAT 325	6,145	GRADE ONLY	NA	•							
SKID STEER LOADER         BOBCAT \$70         2,795         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT \$100         4,091         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT 226B         5,822         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT \$150         5,935         GRADE ONLY         NA         •         SKID STEER LOADER         CAT 301.8C         3,936         GRADE ONLY         NA         •         •         •	MINI EXCAVATOR	BOBCAT 329	7,098	GRADE ONLY	NA	•							
SKID STEER LOADER         BOBCAT \$100         4,091         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT 226B         5,822         GRADE ONLY         NA         •         SKID STEER LOADER         BOBCAT \$150         5,935         GRADE ONLY         NA         •         MINI EXCAVATOR         CAT 301.8C         3,936         GRADE ONLY         NA         •         •         •	MINI EXCAVATOR	BOBCAT 331	7,185	GRADE ONLY	NA	•							
SKID STEER LOADER         BOBCAT 226B         5,822         GRADE ONLY         NA         •           SKID STEER LOADER         BOBCAT 5150         5,935         GRADE ONLY         NA         •           MINI EXCAVATOR         CAT 301.8C         3,936         GRADE ONLY         NA         •         •	SKID STEER LOADER	BOBCAT S70	2,795	GRADE ONLY	NA	•							
SKID STEER LOADER BOBCAT \$150 5,935 GRADE ONLY NA • MINI EXCAVATOR CAT 301.8C 3,936 GRADE ONLY NA • •	SKID STEER LOADER	BOBCAT S100	4,091	GRADE ONLY	NA	•							
MINI EXCAVATOR CAT 301.8C 3,936 GRADE ONLY NA •	SKID STEER LOADER	BOBCAT 226B	5,822	GRADE ONLY	NA	•							
	SKID STEER LOADER	BOBCAT S150	5,935	GRADE ONLY	NA	•							
MINI EXCAVATOR CAT 303.5D CR 7,800 GRADE ONLY NA •	MINI EXCAVATOR	CAT 301.8C	3,936	GRADE ONLY	NA	•		•					
	MINI EXCAVATOR	CAT 303.5D CR	7,800	GRADE ONLY	NA	•		•					
MINI EXCAVATOR CAT 304E CR 8,838 GRADE ONLY NA •	MINI EXCAVATOR	CAT 304E CR	8,838	GRADE ONLY	NA	•		•					
EXCAVATOR KOMATSU PC70 15,829 GRADE ONLY NA •	EXCAVATOR	KOMATSU PC70	15,829	GRADE ONLY	NA	•		•					
EXCAVATOR KOMATSU PC200 44,114 GRADE ONLY NA •	EXCAVATOR	KOMATSU PC200	44,114	GRADE ONLY	NA	•		•					
BOOM LIFT         GENIE S-120         44,340         GRADE ONLY         NA         •	BOOM LIFT	GENIE S-120	44,340	GRADE ONLY	NA			•					
BOOM LIFT GENIE S-125 44,640 GRADE ONLY NA	BOOM LIFT	GENIE S-125	44,640	GRADE ONLY	NA			•					



NEW YORK, NY 10001

(212) 461-1987

#### OWNER

BETH HAMEDRASH HAGADOL (BHH) SYNAGOGUE 60 NORFOLK STREET

#### **GENERAL CONTRACTOR**

NEW YORK, NY 10002

TITAN INDUSTRIAL SERVICES 66-00 LONG ISLAND EXPRESSWAY MASPETH, NEW YORK 11378 718-424-0300

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REVISIONS

DATE: DESCRIPTION 02/28/19 LPC NEW LIMITS

**LOCATION INFO** DOB ADDRESS: 60 NORFOLK STREET

NEW YORK, NY 10002 BLOCK: 346

ZONING DISTRICT: R8 MAP: 12C

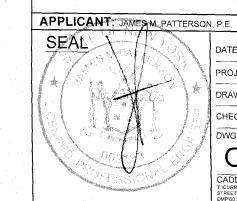
BIN #: 1004155 HEIGHT: 1 STORY @ 65'-5"±

DOB USE ONLY

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PLOT PLAN & NOTES



Vocarca for OPPN #1/54

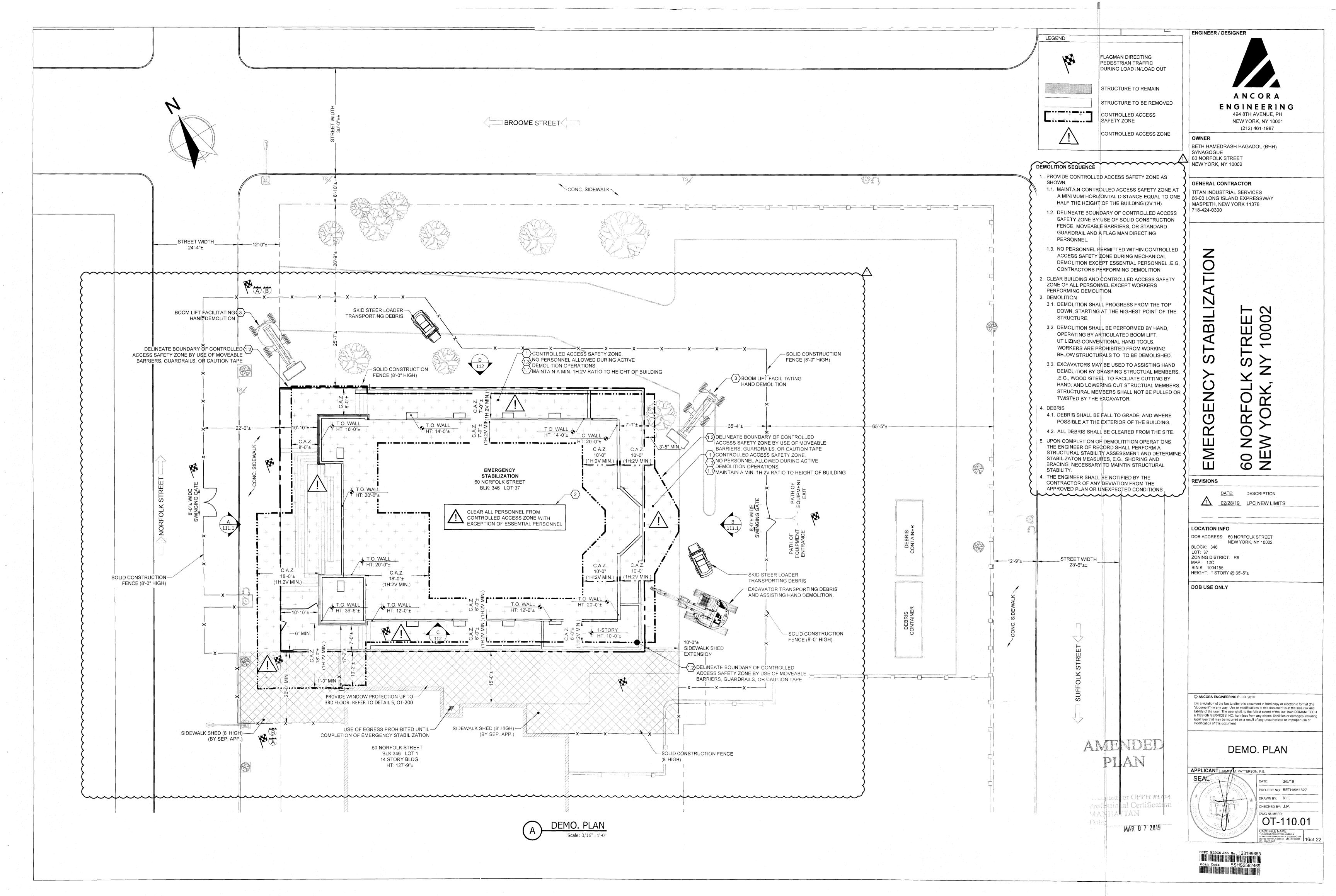
Asfessional Certification

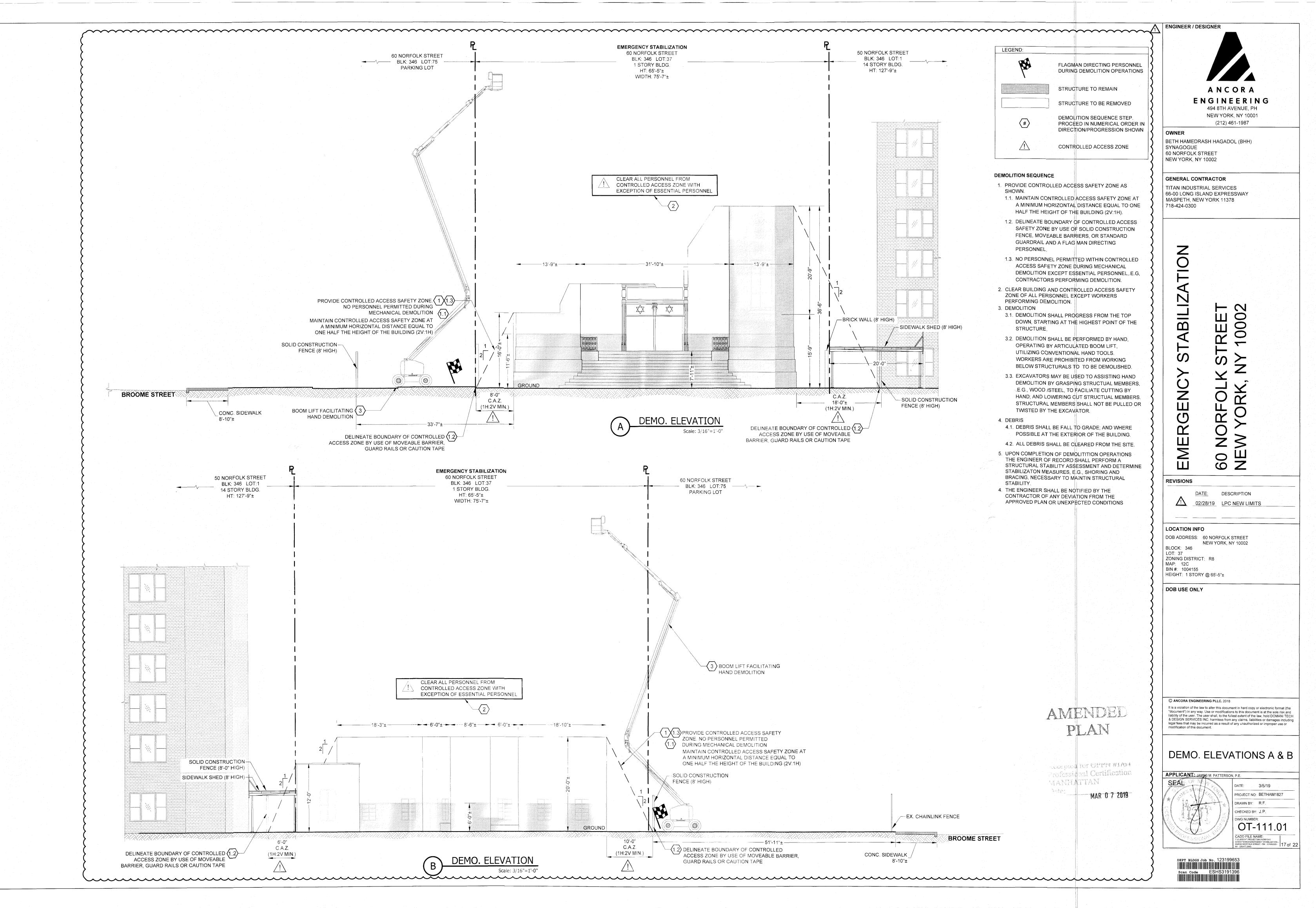
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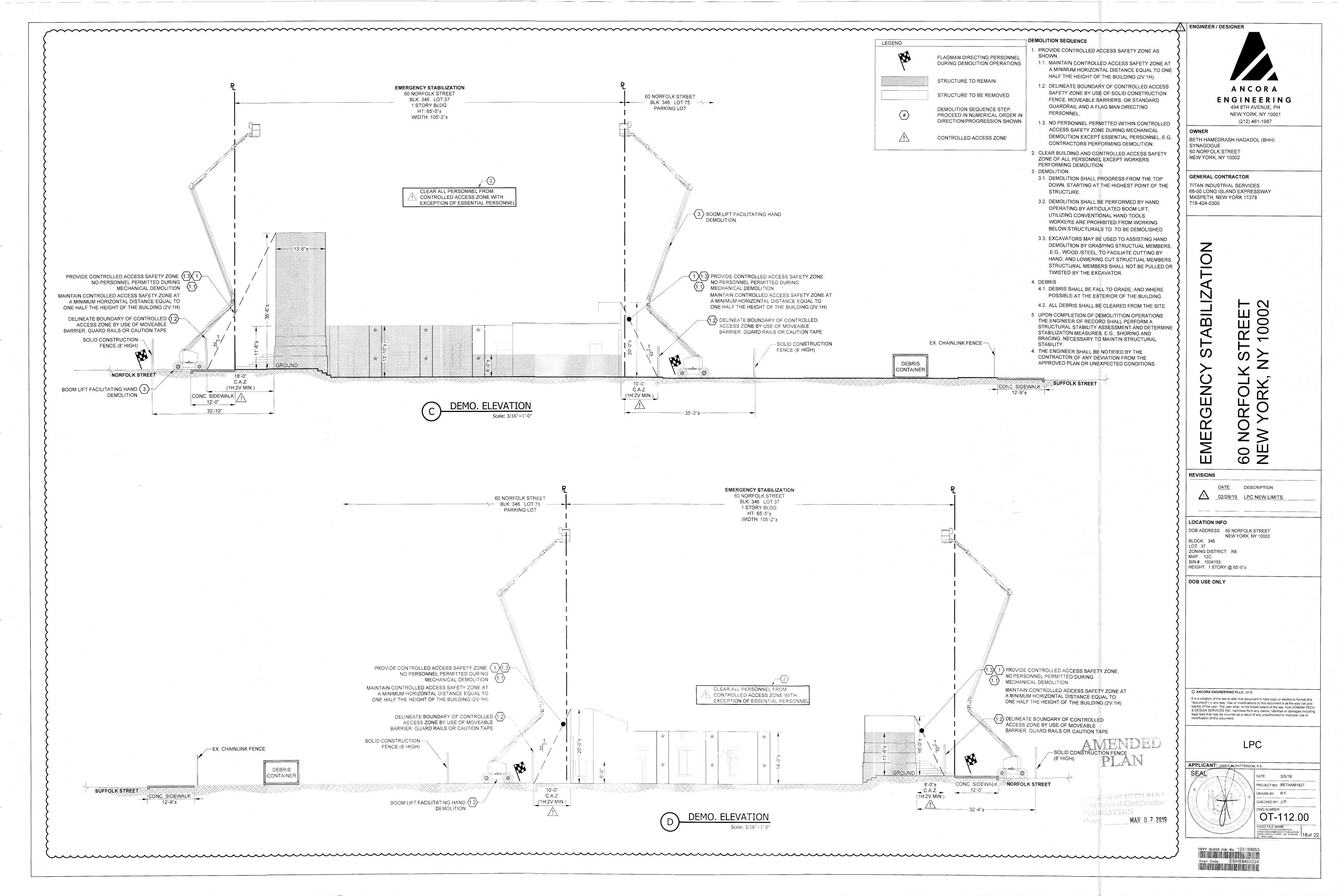
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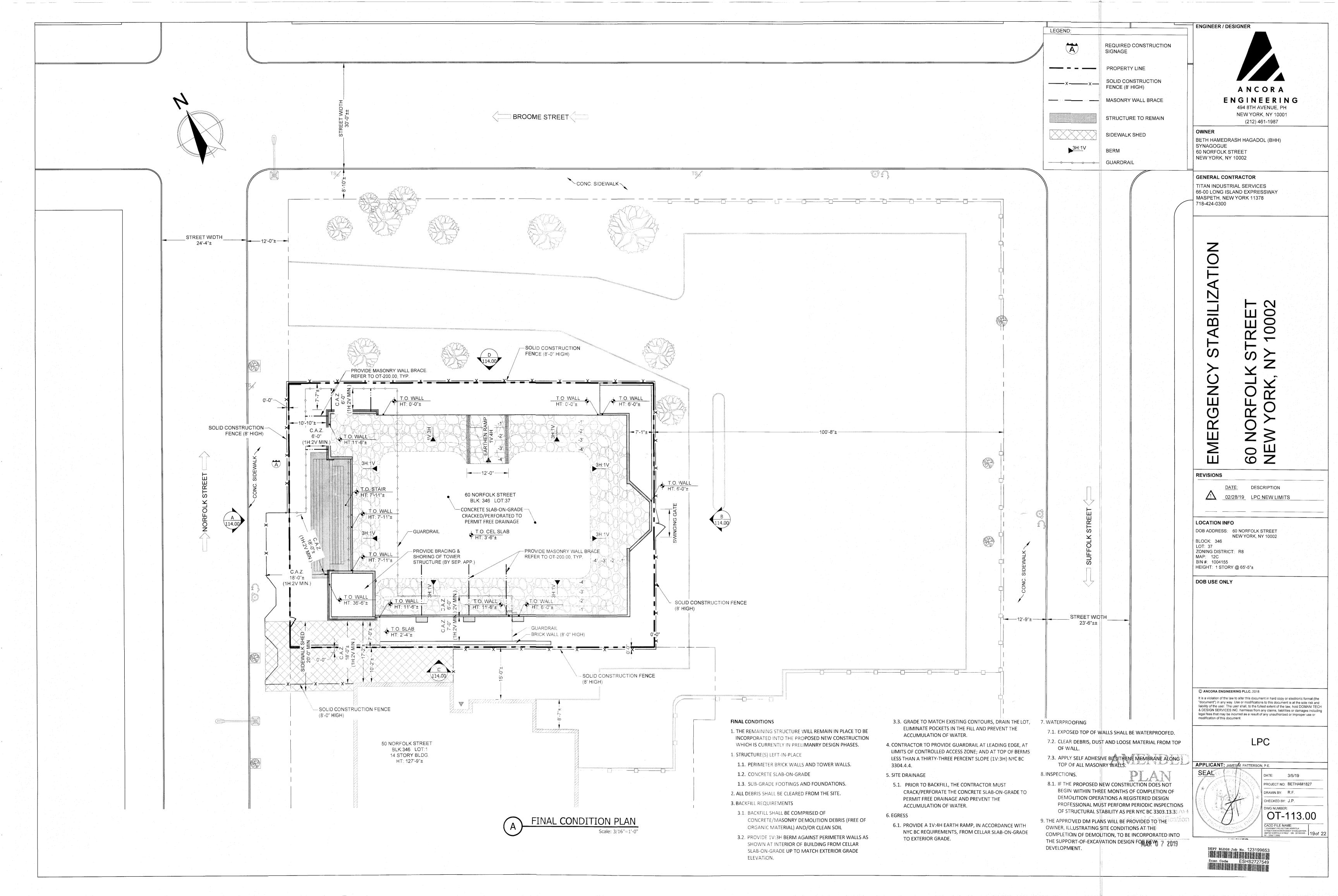
DATE: 3/5/19 PROJECT NO: BETHAM1827 DRAWN BY: R.F. CHECKED BY: J.P. DWG NUMBER: OT-001.01

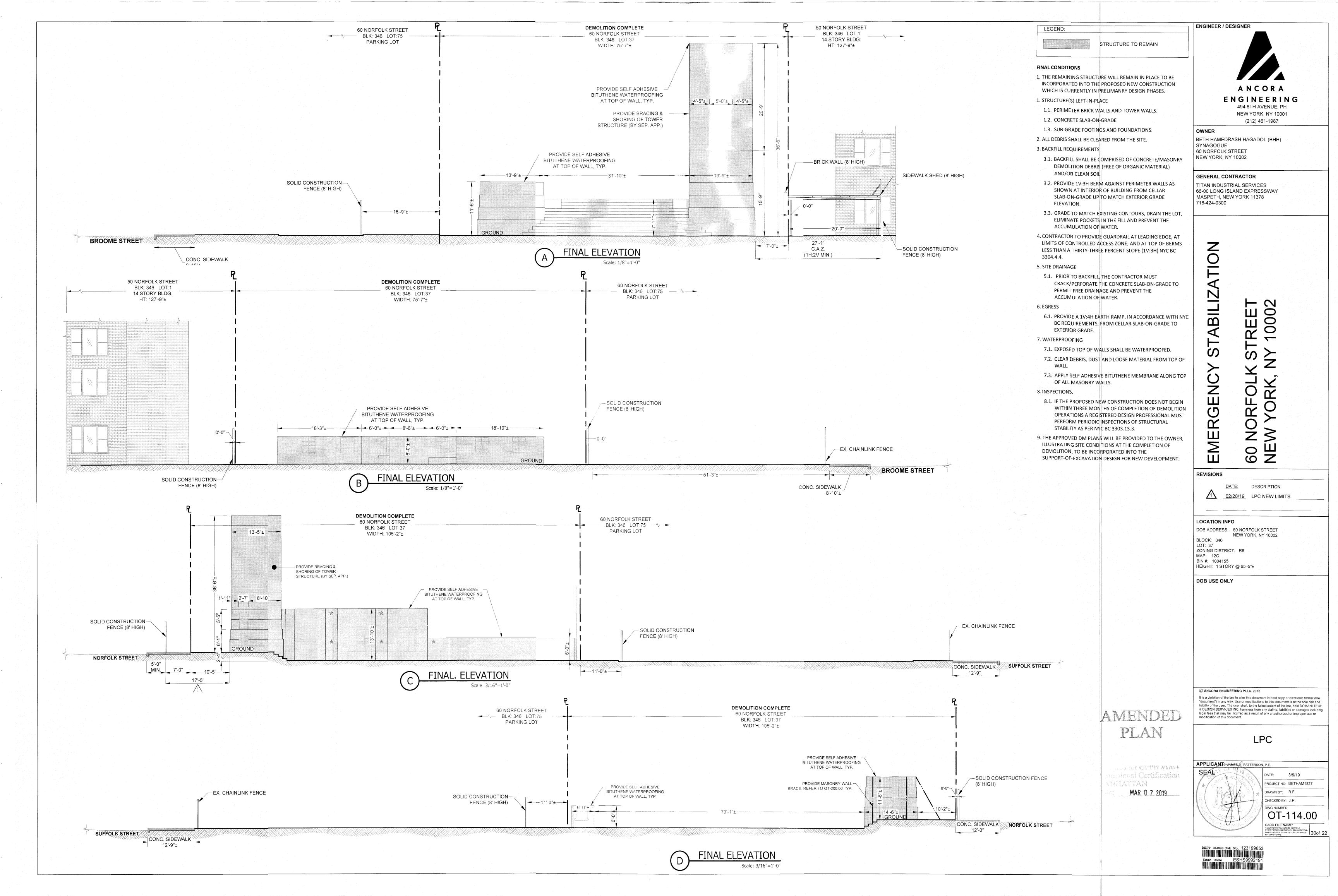
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**APPENDIX D** 

**Boring Logs** 

Log of Boring LB-2 Sheet of 5 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Date Started **Drilling Company** Date Finished Craig Geotechnical Drilling Co., Inc. 2/13/19 2/13/19 **Drilling Equipment** Completion Depth Rock Depth 102 ft CME-75 Truck-Mounted Drill Rig NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 25 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 25  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 140 Dave Cooke Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Drop (in) Sampler Hammer 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Number Sample Description Recov (in) (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) (Blows/ft) Scale +32. 10 20 30 40 ASPHALT (APPROX. 2-INCHES-THICK) +32. SS Started drilling at 8:00 AM. 30 Brown f-c SAND, some fine gravel, some asphalt, some SS 0' to 2' 12 red brick, trace silt (moist) [FILL] S-1 9 6 SS 2' to 4' SS Brown to gray f-c SAND, some red brick, some fine 11 gravel, trace silt (moist) [FILL] S-2 3 ω GO BROOME DEVELOPMENT 18 Drilled to 4'. Brown f-c SAND, some red brick, trace fine gravel, trace SS SS 4' to 6' silt (moist) [FILL] 13 S-3 9 5 9 3 6 SS 6' to 8' SS Brown f-c SAND, trace fine gravel, trace silt, trace red 2 brick (moist) [FILL] 3 S-4 DATA/GEOTECHNICAL/GINTLOGS/100610001 Class 7 8 Installed casing to 8'. SS Dark gray f-c SAND and RED BRICK, trace silt, trace 3 Drilled to 8' glass (moist) S-5 SS 8' to 10' 9 ω 3 2 10 SS 10' to 12' Gray f-c GRAVEL, some f-c sand, trace silt (moist) 16 SS 35 S-6 ω 34 8 12 SS SS 12' to 14' Gray-brown f-c SAND, some fine gravel, trace silt, trace 14 red brick (moist) S-7 13 9 50 26 14 15 Drilled to 15'. 51 Gray f-c GRAVEL (moist) Class SS 15' to 17' 11 Re-drove with 3" O.D. spoon \LANGAN.COM\DATA\PAR\DATA8\ 16 SS 13 for additional recovery. Brown SILT, trace fine sand (moist) 13 17 Class 18 19



3/13/2019 10:47:18 AM .

Log of Boring LB-2 Sheet of 5 2 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in)
Penetr. resist Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12. 20 Installed casing to 14'. Brown f-c SAND, trace silt (moist) Drilled to 20'. SS 11 S-9 SS 20' to 22' 21  $\infty$ 11 12 22 23 24 25 Drilled to 25'. Brown f-c SAND, trace silt (wet) SS 25' to 27' 6 10 26 12 27 Class - GO BROOME DEVELOPMENT.GPJ. 28 29 +2.5 30 Drilled to 30'. Brown Silty f-m SAND (wet) SS 30' to 32' 8 16 31 16 8 9 32 ILANGAN.COMIDATAIPARIDATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 33 34 -2.5 35 SS Drilled to 35'. Brown SILT, some fine sand (wet) SS 35' to 37' 5 9 36 37 38 39 Class 5b Drilled to 40'. SS Brown SILT, some fine sand (wet) SS 40' to 42' 9 15 10 43

LANGAN Log of Boring LB-2 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -12. 10 20 30 40 SS Drilled to 45'. Brown SILT, trace clay (wet) SS 45' to 47' S-14 5 24 46 8 DATA/GEOTECHNICAL/GINTLOGS/100610001 - GO BROOME DEVELOPMENT.GPJ ... 3/13/2019 10:47:18 AM ... Report: Log - LANGAN 48 49 50 Drilled to 50'. Brown SILT, trace fine sand, trace clay (wet) SS 50' to 52' 20 11 52 53 54 55 Drilled to 55'. Gray SILT, some clay, trace fine sand (wet) SS 55' to 57' 5 9 56 9 Class 58 59 -27.5 60 SS Drilled to 60'. Gray Clayey SILT (wet) SS 60' to 62' 9 9 61 15 16 62 63 64 \\LANGAN.COM\DATA\PAR\DATA8\10064680 -32.5 Drilled to 65'. SS Brownish-gray SILT, some clay (wet) SS 65' to 67' 20 66 11 67 68

69



Log of Boring LB-2 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -37.5 10 20 30 40 -37. SS Drilled to 70'. Bronwish-gray SILT, trace fine sand, trace clay (wet) SS 70' to 72' S-19 11 9 33 22 - GO BROOME DEVELOPMENT.GPJ ... 3/13/2019 10:47:19 AM ... Report: Log - LANGAN 21 72 73 74 Class -42. Drilled to 75. 33 SS Brownish-gray Sandy SILT, trace fine gravel (wet) 9 SS 75' to 75'8" 70/2 76 77 78 79 -47 5 80 Drilled to 80'. Brown Silty f-m SAND (wet) 28 SS 80' to 82' S-21 31 5 81 34 31 82 DATA/GEOTECHNICAL/GINTLOGS/100610001 Class 83 -52. 85 SS Drilled to 85'. Brown Clayey SILT, trace fine sand (wet) 24 SS 85' to 87' S-22 26 86 32 31 87 88 89 \\LANGAN.COM\DATA\PAR\DATA8\10064680 Class 5a -57.5 Drilled to 90'. SS 15 Brown SILT, some clay, trace fine sand (wet) SS 90' to 92' 14 24 16 18 92 93 94



Log of Boring LB-2 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -62. 10 20 30 40 95 Drilled to 95'. SS Brown f-c SAND, some f-c gravel, some silt, trace clay SS 95' to 97' (wet) S-24 19 7 96 29 52 97 98 Class 3a 99 3/13/2019 10:47:19 AM . -67.5 100 Drilled to 100'. 56 Brown Silty f-m SAND (wet) SS 100' to 102' 61 18 72 77 -69.6 102 Finished drilling at 12:30PM. "ILANGAN, COMIDATA\PAR\DATA8/100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ .. End of Boring at 102'. Backfilled boring with drill cuttings and patched surface 103 with asphalt upon completion. 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119

1		4			<u>1/V</u>		Log		Boring			LB	-4		_	;	Sheet	1	of	4
F	Project	Go Broome Development						Project No. 100646801												
ī	ocation	·						Elevation and Datum												
1	Orilling C	60 Norfolk Street, New York, New York g Company							Approx. el 32.5 N								lnished			
Craig Geotechnical Drilling Co., Inc.								2/14/19 2/14/19    Completion Depth   Rock Depth												
Drilling Equipment  CME-75 Truck-Mounted Drill Rig								mpieuon	Бер	un		77 ft		Ro	CK L	ерип		NE		
Size and Type of Bit 3-7/8" O.D. Tri-Cone Roller Bit							Nι	Number of Samples Disturbed Undisturbed Core								_				
Ţ	Casing E	Diame	ter (in)	). Steel		Ca	asing Depth (ft)	w	ater Leve	l (ft.)		First		25		Con	pletion	_	24 HR.	_
	Casing F	lamm	er Autor	matic	Weight (lbs)	140	Drop (in) 30	Dr	illing For	emar		_				<u> </u>	-		<u> </u>	
	Sampler							Dave Cooke Field Engineer												
7 [-	Sampler	Hamr	ner	Safety	Weight (lbs)	140	Drop (in) 30				Pa		Lenih							
AIM NE	MATERIAL SYMBOL	Elev.	Building Code		Sample Des	scription			Depth Scale	Number	Туре		Penetr. resist aldu BL/6in Q	N-V	/alue ws/ft	)	(Drillir Fluid Los	Rem ng Fluid, D ss, Drilling	arks epth of Cas Resistance	ing, , etc.)
4/ .20 X	××××	+32.5 +32.2		ASPHALT (AI	PPROX. 3-INCH-	-THICK)		_	0 -	_			32	10 20	30 4	40	Started drilling at 7:55 AM on			M on
				Light brown to fine gravel, tra	ne red brick, trace	•	1 -	S-1	SS	12	10 7	17 •			2/14/2019. SS 0' to 2' SS 2' to 4'					
)   N				Light brown f-	race fine gravel,		2 -				7		$\setminus$							
75.FX				trace silt (moi		_ 3 -	S-2	SS SS	9	11 17	2	8								
						_ 4 -				9		/		Drilled to 4'. SS 4' to 6'						
				Gray to brown trace fine grav		Ė .	3			5 6										
			Class 7			5 -	S-3	SS	8	3	9									
				Brown f-c SAND, some red brick, trace fine gravel, trac silt (moist) [FILL]					6 -				14				SS 6' to 8' Spoon broke off down the			
				Siit (moist) [i i		7 -	S-4	SS SS	4	24 16		40	$\left  \cdot \right $	hole. Hole was offset approximately			mately			
000116					ne silt, some red		8 -				9			$ \cdot $	1 foot and redrilled to 6'. Took sample from 6' to 8 auto hammer.			8' with		
				brick (moist) [		9 -	S-5	SS	9	28 24			52	SS 8' to 10'						
		+22.5.							10		_		23			I				
				Brown Silty fir	ne SAND (moist)				10 -		SS		9 11		/		Drilled	l to 10'.	g to 10'.	
									_ 11 -	S-6	SS	12	11	22			SS 10	to 12		
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בון פון פון									13 -											
ב ב									_ 14 -											
			Class 3b Brown Silty fine SAND (moist)					<u> </u>		1			15				Drilled	rilled to 15'.		
00110									16	<u>'</u> -	SS	16	10	45			SS 15' to 17'			
¥ .									- 16 -	S-7	S	_	5	15						
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3/13/2019 10:47:27 AM .

| 100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 - GO BROOME DEVELOPMENT.GPJ

Log of Boring LB-4 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12. 20 +12. Brown f-m SAND, some silt, trace fine gravel (moist) SS 20' to 22' SS 11 S-8 21 က 11 12 22 23 24 Class 25 Drilled to 25'. 10 Brown f-c SAND, some silt (wet) SS 25' to 27' 10 26 13 27 28 29 +2.5 30 Drilled to 30'. 11 Brown Sandy SILT (wet) SS 30' to 32' 11 9 31 10 9 32 33 34 -2.5 35 SS Drilled to 35'. Brown SILT, trace fine sand, trace clay (wet) SS 35' to 37' 8 9 36 11 37 Class 5b 38 39 -7.5 Drilled to 40'. SS Brown Clayey SILT (wet) SS 40' to 42' 8 18 10 42 43



Log of Boring LB-4 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -12. 10 20 30 40 Drilled to 45'. Brown Clayey SILT (wet) SS 45' to 47' S-13 20 46 6 Class 5b 48 49 -17.5 50 Drilled to 50'. Brownish-gray Silty CLAY (wet) SS 50' to 52' 22 10 52 53 54 -22.5 55 Drilled to 55'. 18 Brownish-gray Clayey SILT (wet) SS 55' to 57' 15 56 20 21 Class 58 59 -27. 60 SS Drilled to 60'. Brownish-gray Silty CLAY (wet) SS 60' to 62' 10 20 61 12 12 62 Class 4b 63 64 \\LANGAN.COM\DATA\PAR\DATA8\10064680` -32.5 Drilled to 65'. SS 19 Brownish-gray SILT, trace fine sand, trace clay (wet) SS 65' to 67' 25 16 66 38 67 Class 5a 68 69



Log of Boring LB-4 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.5 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -37. 10 20 30 40 Brownish-gray f-c SAND, some silt, some clay, trace SS Drilled to 70'. SS 70' to 72' fine gravel (wet) S-18 13 16 18 27 73 Class 3a 3/13/2019 10:47:28 AM . 75 Drilled to 75'. Brown f-m SAND, some silt (wet) 28 SS 75' to 77' 31 22 34 47 Finished drilling at 10:50 AM. "ILANGAN.COMIDATA"PAR'IDATA8/100646801/ENGINEERING DATA!GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ.. End of boring at 77'. Backfilled boring with drill cuttings and patched surface 78 with asphalt upon completion. 79 80 81 82 83 85 86 87 88 89 91 92 93 94

Log of Boring LB-5 Sheet of 4 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 **Drilling Company** Date Started Date Finished Craig Geotechnical Drilling Co., Inc. 2/14/19 2/14/19 Rock Depth **Drilling Equipment** Completion Depth CME-75 Truck-Mounted Drill Rig 77 ft NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 20 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 30  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 30 140 Mark Aquino Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Sampler Hammer Drop (in) 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Number Sample Description Recov (in) (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale (Blows/ft) +32.0 10 20 30 40 ASPHALT (APPROX. 3-INCH-THICK) +31. SS 10 Started drilling at 7:49 AM on Light gray f-c SAND, some fine gravel, trace silt, trace 2/14/2019. 9 S-1 red brick (moist) [FILL] SS 0' to 2' 9 6 3 Brown Gravelly f-c SAND, trace silt, trace red brick SS 2' to 4' 2 (moist) [FILL] S-2 SS 3  $\alpha$ GO BROOME DEVELOPMENT 4 Installed casing to 4'. No Recovery SS 2 Drilled to 4'. 3 S-3 SS 4' to 6' 0 5 6 SS 6' to 8' SS RED BRICK, some fine gravel, some f-c sand, trace silt 5 No Recovery with 2" O.D. split (moist) [FILL] 10 S-4 spoon, re-drove with 3" O.D. LANGAN.COM/DATA\PAR\DATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 11 split spoon and obtained 4" recovery. Class 7 20 Installed casing to 8'. SS Gray fine GRAVEL, some f-c sand, trace silt, trace glass 22 Drilled to 8' (moist [FILL] 27 S-5 SS 8' to 10' 9  $\sim$ 22 20 10 SS 10' to 12' Light brown f-c SAND, some silt (moist) [FILL] 10 SS S-6 12 8 7 12 SS SS 12' to 14' Light brown f-c SAND, some silt, trace fine gravel, trace 11 red brick, trace glass (moist) [FILL] 11 S-7 13 9 11 9 14 +17.0 Installed casing to 15'. SS Brown f-m SAND, some silt (moist) Drilled to 15'. 5 S-8 SS 15' to 17' ω 6 17 Class 18 19



3/13/2019 10:47:35 AM .

Log of Boring LB-5 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12.0 20 Installed casing to 20'. SS Brown f-m SAND, some silt, trace clay (moist) Drilled to 20'. 6 S-9 SS 20' to 22' 7 21 6 22 23 24 25 Drilled to 25'. Brown f-m SAND, some silt (moist) SS 25' to 27' 10 26 12 11 27 Class 3b - GO BROOME DEVELOPMENT.GPJ 28 29 +2.0 30 Drilled to 30'. Brown Silty fine SAND (wet) SS 30' to 32' 8 4 31 8 8 32 100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 33 34 -3.0 35 SS Drilled to 35'. Brown SILT, some fine sand, trace clay (wet) SS 35' to 37' 10 36 10 10 37 38 39 Class 5b -8.0 Drilled to 40'. Brown Clayey SILT (wet) SS 40' to 42' 18 13 43



Log of Boring LB-5 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -13.0 Drilled to 45' Brown SILT, some clay (wet) SS 45' to 47' S-14 SS 6 15 46 8 NLANGAN.COMIDATA/PARIDATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/1006/1006/10061 - GO BROOME DEVELOPMENT.GPJ... 3/13/2019 10:47:35 AM ... Report. Log - LANGAN 11 Class 5b 48 49 -18.0 50 Drilled to 50'. 13 Gray SILT, some clay (wet) SS 50' to 52' 16 20 22 52 Class 53 54 -23.0 55 Drilled to 55'. Gray SILT, some clay (wet) SS 55' to 57' 20 56 11 15 58 59 Class 60 SS Drilled to 60'. Gray SILT, some clay, trace fine sand (wet) SS 60' to 62' 11 9 61 10 13 62 63 64 -33.0 Drilled to 65'. SS 34 Brownish-gray SILT, trace fine sand, trace clay (wet) SS 65' to 67' 26 15 51 67 Class 5a 68 69



Log of Boring LB-5 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -38.0 10 20 30 40 -38.0 Drilled to 70'. Brown f-m SAND, some silt (wet) SS 70' to 72' S-19 SS 4 53 45 72 73 Class 3a 74 3/13/2019 10:47:36 AM . 75 Drilled to 75'. Brown f-m SAND, some silt (wet) SS 75' to 77' 47 18 49 39 -45.0 Finished drilling at 10:31 AM. ||LANGAN.COM|DATA\PAR\DATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ.. End of boring at 77'. Backfilled boring with drill cuttings and patched surface 78 with asphalt upon completion. 79 80 81 82 83 85 86 87 88 89 91 92 93 94

Log of Boring LB-6 Sheet of 4 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 **Drilling Company** Date Started Date Finished Craig Geotechnical Drilling Co., Inc. 2/13/19 2/13/19 Rock Depth **Drilling Equipment** Completion Depth CME-75 Truck-Mounted Drill Rig 77 ft NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 20 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 25  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 140 Mark Aquino Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Sampler Hammer Drop (in) 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Penetr. resist BL/6in Number Recov. (in) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale (Blows/ft) +32.0 0 ASPHALT (APPROX. 1-INCH-THICK) Started drilling at 9:40 AM. +31. 15 Grayish-brown f-c SAND, some fine gravel, trace silt, SS 0' to 2' SS 22 trace asphalt (moist) [FILL] S-1 9 Started drilling at 9:40 AM. 29 22 Brown to dark gray f-c SAND, some fine gravel, trace SS SS 2' to 4' 20 silt, trace red brick, trace asphalt (moist) [FILL] 28 S-2 10 3 GO BROOME DEVELOPMENT 15 9 Installed casing to 4'. Gray f-c GRAVEL and RED BRICK, trace f-c sand, trace SS 4 Drilled to 4'. silt (moist) [FILL] 5 S-3 SS 4' to 6' 5 4 5 3 6 SS 6' to 8' SS Brown fine GRAVEL, some f-c sand (moist) [FILL] 4 S-4 Class 7 DATA/GEOTECHNICAL/GINTLOGS/100610001 8 Installed casing to 8'. Light gray Gravelly f-c SAND, trace silt, trace wood, SS 8 Drilled to 8' trace red brick (moist) [FILL] 12 S-5 SS 8' to 10' 9 ω 45 6 SS SS 10' to 12' Grayish-brown f-c SAND, some fine gravel, trace silt, 4 trace red brick (moist) [FILL] 4 S-6 ω 3 2 12 SS SS 12' to 14' Grayish-brown f-c SAND, trace fine gravel, trace silt, 3 trace red brick (moist) [FILL] ANGAN. COM/DATA/PAR/DATA8/100646801/ENGINEERING S-7 13 က 3 2 +18.0 Installed casing to 14'. 5 Brown f-c SAND, trace silt, trace clay (moist) Drilled to 14'. 7 SS S-8 SS 14' to 16' 9 8 6 16 Class 17 18 19



Log of Boring LB-6 Sheet of 4 2 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12.0 20 Installed casing to 20'. +12. SS Brown Silty fine SAND (moist) Drilled to 20'. S-9 SS 20' to 22' 7 21 8 9 22 23 24 3/13/2019 10:47:43 AM . +7.0 25 Installed casing to 25'. 10 Brown f-c SAND, some silt, trace fine gravel (wet) Drilled to 25'. SS 25' to 27' 8 26  $\infty$ 10 10 27 Class - GO BROOME DEVELOPMENT.GPJ. 28 29 +2.0 30 Installed casing to 30'. Brown Silty f-m SAND (wet) Drilled to 30'. 5 SS 30' to 32' 9 13 31 8 9 32 DATA/GEOTECHNICAL/GINTLOGS/100610001 33 34 35 SS Installed casing to 35'. Brownish-gray Silty CLAY (wet) Drilled to 35'. 8 SS 35' to 37' 10 36 9 37 Class 4b 38 39 \\LANGAN.COM\DATA\PAR\DATA8\10064680` -8.0 Drilled to 40'. Brown SILT, some clay (wet) SS 40' to 42' 6 18 10 7 Class 5b 43



Log of Boring LB-6 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -13.0 SS Drilled to 45' Brown Silty CLAY (wet) SS 45' to 47' S-14 20 46 10 Class 4b 48 49 -18.0 Drilled to 50'. Brownish-gray Clayey SILT, trace fine sand (wet) SS 50' to 52' 4 12 12 52 Class 53 54 -23.0 55 Drilled to 55'. Grayish-brown Silty CLAY (wet) SS 55' to 57' 6 9 56 6 9 NILANGAN.COMIDATAIPARIDATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 Class 58 59 -28.0 SS Drilled to 60'. Brownish-gray SILT, some fine sand, trace fine gravel 56 SS 60' to 62' 46 16 61 44 47 62 63 64 Class Drilled to 65'. 17 Brown SILT, trace fine sand, trace clay (wet) SS 65' to 67' 25 31 27 67 68 69



Log of Boring LB-6 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -38.0 10 20 30 40 -38.0 Drilled to 70'. Brown f-m SAND, some silt (wet) SS 70' to 72' S-19 SS 33 16 41 35 72 73 Class 3a 74 3/13/2019 10:47:44 AM . 75 Drilled to 75'. Brown f-m SAND, some silt (wet) 35 SS 75' to 77' 16 40 46 -45.0 Finished drilling at 12:26PM. "ILANGAN.COMIDATA"PAR'IDATA8/100646801/ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ.. End of boring at 77'. Backfilled boring with drill cuttings and patched surface 78 with asphalt upon completion. 79 80 81 82 83 85 86 87 88 89 91 92 93 94

Log of Boring LB-7 Sheet of 4 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 **Drilling Company** Date Started Date Finished Craig Geotechnical Drilling Co., Inc. 2/11/19 2/13/19 Rock Depth **Drilling Equipment** Completion Depth CME-75 Truck-Mounted Drill Rig 77 ft NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 20 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 25  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 140 Mark Aquino Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Sampler Hammer Drop (in) 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Number Sample Description Recov (in) (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale (Blows/ft) +32.0 10 20 30 40 0 ASPHALT (APPROX. 2-INCH-THICK) +31. SS 48 Started drilling at 11:47 AM on Brown f-c SAND, some fine gravel, trace silt, trace red 2/11/2019. 8 S-1 brick (moist) [FILL] 12 SS 0' to 2' 9 12 SS 2' to 4' Brown to gray Gravelly f-c SAND, trace silt, trace asphalt 49 fragments (moist) [FILL] 32 S-2 SS 3 ω GO BROOME DEVELOPMENT 9 9 4 Installed casing to 4'. No Recovery SS 3 Drilled to 4'. S-3 SS 4' to 6' 0 5 6 Class 7 SS 6' to 8' SS Brown f-c SAND and f-c GRAVEL, trace silt, trace red No Recovery with 2" O.D. split brick (moist) [FILL] S-5 spoon, re-drove with 3" O.D. ANGAN.COMIDATAIPARIDATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 split spoon and obtained 4" recovery. 8 Installed casing to 8'. SS Brown to dark gray f-c SAND, some fine gravel, trace 15 Drilled to 8' silt, trace red brick (moist) [FILL] 24 S-5 SS 8' to 10' 9 ω 17 19 10 SS SS 10' to 12' Brown f-c SAND, trace fine gravel, trace silt, trace red 12 brick (moist) [FILL] 9 S-6 10 11 11 +20.0 12 Drilled to 12' 15 Dark brown f-m SAND, some silt, trace fine gravel SS 12' to 14' (moist) S-7 13 SS ω 32 29 14 +17.0 Installed casing to 15'. 15 Dark brown Silty f-c SAND, trace fine gravel (moist) Drilled to 15'. 25 Class S-8 SS 15' to 17' 16 SS 9 29 30 17 18 19



Log of Boring LB-7 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12.0 20 Installed casing to 20'. Brown f-c SAND, some silt (moist) Drilled to 20'. SS S-9 SS 20' to 22' 10 21 9 10 22 23 24 Class +7.0 25 Drilled to 25'. 10 Brown Silty fine SAND (wet) SS 25' to 27' Stopped drilling for the day at 1:10 PM on 2/11/2019. 16 26 13 Resumed drilling at 7:49 AM 13 on 2/13/2019. 27 DATA/GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ. 28 29 +2.0 30 Drilled to 30'. 4 Brown SILT, some clay, trace fine sand (wet) SS 30' to 32' ω 31 6 5 32 33 -3.0 35 SS Drilled to 35'. Brown Clayey SILT (wet) SS 35' to 37' 20 36 37 Class 5b 38 39 \\LANGAN.COM\DATA\PAR\DATA8\10064680 -8.0 Drilled to 40'. SS Brown SILT, trace clay (wet) SS 40' to 42' 3 18 10 42 43



Log of Boring LB-7 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -13.0 SS Drilled to 45' Grayish-brown SILT, some clay, trace fine sand (wet) SS 45' to 47' S-14 8 16 46 15 - GO BROOME DEVELOPMENT.GPJ ... 3/13/2019 10:47:52 AM ... Report: Log - LANGAN 15 Class 5b 48 49 -18.0 50 Drilled to 50'. Brown SILT, trace fine sand, trace clay (wet) SS 50' to 52' 20 18 25 19 52 Class 53 54 -23.0 Drilled to 55'. 23 Brown to dark gray f-c SAND, some silt, trace clay (wet) SS 55' to 57' 28 8 56 29 34 IILANGAN.COMIDATAIPARIDATA8/100646801/ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 58 59 60 SS Drilled to 60'. Brown f-c SAND, some silt (wet) 27 SS 60' to 62' 28 61 38 29 62 Class 3a 63 64 Drilled to 65'. Brown f-m SAND, some silt (wet) 17 SS 65' to 67' 22 16 66 27 67 68 69



Log of Boring LB-7 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -38.0 10 20 30 40 70 SS Drilled to 70'. Brown f-c SAND, some silt (wet) SS 70' to 72' S-19 25 15 25 30 72 Class 3a 73 74 -43.0 Drilled to 75'. 27 Grayish-brown Sandy CLAY, some fine gravel, some silt SS 75' to 77' (wet) 57 Class 4a 12 80 82 Finished drilling at 9:20 AM. "ILANGAN, COMIDATA\PAR\DATA8/100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ .. End of boring at 77'. Backfilled boring with drill cuttings and patched surface 78 with asphalt upon completion. 79 80 81 82 83 85 86 87 88 89 90 91 92 93 94

Log of Boring LB-8 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 33 NAVD88 Drilling Company Date Started Date Finished 2/8/19 Craig Geotechnical Drilling Co., Inc. 2/8/19 **Drilling Equipment** Completion Depth Rock Depth CME-75 Truck-Mounted Drill Rig 102 ft NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 24 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 30  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 30 140 Dave Cooke Sampler 2" O.D. Split Spoon Field Engineer Sampler Hammer Safety/Automatic Weight (lbs) Drop (in) 140 30 Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Penetr. resist BL/6in Number Sample Description Recov (in) (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale (Blows/ft) +33.0 0 ASPHALT (APPROX. 1-INCH-THICK) +32. SS Started drilling at 8:15 AM. 6 Brown f-c SAND, some fine gravel, trace silt, trace red Automatic hammer used due S-1 brick, trace tile (moist) [FILL] 9 to rain. SS 0' to 2' SS 2' to 4' Brown f-c SAND & RED BRICK, trace silt (moist) [FILL] 5 Drilled to 4'. GO BROOME DEVELOPMENT.GPJ 3 S-2 SS 3 4 6 6 8 SS 4' to 6' Light grayish-brown f-c SAND, trace fine gravel, trace 3 silt, trace red brick (moist) [FILL] S-3 ω 5 Class 7 6 SS 6' to 7'5" SS Top 4": Light grayish-brown f-c SAND, some fine gravel, 3 Drilled to 8'. trace silt, trace red brick (moist) [FILL] S-4 ω 2 Approx. 6" obstruction from LANGAN.COM/DATA\PAR\DATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 Bottom 4": ASPHALT 60/5" 7.7' to 8' Installed casing to 8'. 8 Drilled to 8'. Brown f-c SAND, trace fine gravel, trace silt, trace red SS 3 SS 8' to 10' brick (moist) [FILL] S-5 9 4 2 3 +23.0 10 SS 10' to 12' Brown f-m SAND, some silt (moist) 3 SS 3 S-6 10 3 3 12 13 14 Class 6 Installed casing to 15'. SS Brown f-c SAND, trace silt (moist) 3 Drilled to 15'. 2 S-7 SS 15' to 17' 16 ω 17 18 19



Log of Boring LB-8 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 33 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +13.0 20 +13. SS Drilled to 20' Brown Silty f-m SAND (moist) SS 20' to 22' S-8 4 21 5 22 23 24 25 Drilled to 25'. Brown Silty f-m SAND (moist) SS SS 25' to 27' S-9 15 26 5 27 - GO BROOME DEVELOPMENT.GPJ Class 6 28 29 30 Drilled to 30' Brown Silty f-m SAND (moist) SS 30' to 32' 5 12 31 32 33 34 35 SS Drilled to 35'. Reddish-brown Clayey SILT (wet) SS 35' to 37' 6 20 36 37 38 39 \\LANGAN.COM\DATA\PAR\DATA8\10064680 Class 5b -7.0 Drilled to 40'. SS 6 Grayish-brown SILT, trace fine sand, trace clay (wet) SS 40' to 42' 8 22 12 42 43



Log of Boring LB-8 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 33 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -12.0 45 -12. SS Drilled to 45'. Brown SILT, some f-m sand, some clay (wet) SS 45' to 47' S-13 20 20 46 22 3/13/2019 10:48:02 AM ... Report: Log - LANGAN 28 47 Class 5a 48 49 -17.0 50 Drilled to 50'. Brown f-c SAND, some silt (wet) SS 50' to 52' 9 15 9 52 - GO BROOME DEVELOPMENT.GPJ. 53 54 Class 55 Drilled to 55'. Brown f-c SAND, trace silt (wet) 11 SS 55' to 57' 10 56 12 12 6801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 58 59 -27.0 60 SS Drilled to 60'. Brown SILT, trace fine sand (wet) 12 SS 60' to 62' 15 15 61 19 19 62 Class 5a 63 64 -32.0 Drilled to 65'. SS 23 Brown f-c GRAVEL, some f-c sand, trace silt (wet) SS 65' to 67' 19 9 66 20 23 67 Class 2a 68 69



- GO BROOME DEVELOPMENT.GPJ.

/LANGAN.COM/DATA\PAR\DATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001

Log of Boring LB-8 Sheet 5 of Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 33 NAVD88 Sample Data Building Code Remarks Depth Scale N-Value (Blows/ft) Recov. (in)
Penetr. resist Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) -37.0 10 20 30 40 70 SS Drilled to 70'. Moderate rig Brown Sandy f-c GRAVEL, trace silt (wet) S-18 35 10 SS 70' to 72' 28 24 72 Class 2a 73 74 0 -42.0 Drilled to 75'. Moderate rig 46 Brown f-c SAND, some f-c gravel, trace silt (wet) chatter SS 75' to 77' 12 30 36 78 79 80 Drilled to 80'. Moderate rig Brown f-c SAND, some f-c gravel, trace silt (wet) 74 S-20 Switched to safety hammer for 81 100/5" SPT sampling. SS 80' to 81'5" 82 83 84 Class 85 SS Drilled to 85'. Brown f-c SAND, some fine gravel, trace silt (wet) 65 SS 85' to 87' 63 S-21 86 60 51 87 88 89 Drilled to 90'. Brown f-c SAND, trace fine gravel, trace silt (wet) 52 SS 90' to 92' 61 8 47 44 92 93 94

95



Log of Boring LB-8 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 33 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -62.0 10 20 30 40 95 SS Drilled to 95'. Brown f-c SAND, trace silt (wet) S-23 SS 95' to 96'7" 16 80 96 68 50/1" 97 98 Class 3a 99 3/13/2019 10:48:02 AM . 100 Drilled to 100'. Brown f-m SAND, some silt (wet) 67 SS 100' to 102' 18 58 54 -69.0 102 Finished drilling at 1:10 PM. ||LANGAN.COM|DATA\PAR\DATA8\100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ.. End of boring at 102'. Backfilled boring with soil cuttings and hole plug and 103 patched surface with asphalt upon completion. 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119

Log of Boring LB-11 Sheet of 4 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 32.4 NAVD88 Drilling Company Date Started Date Finished Craig Geotechnical Drilling Co., Inc. 2/11/19 2/11/19 Drilling Equipment Rock Depth Completion Depth 77 ft CME-75 Truck-Mounted Drill Rig NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 19 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 30  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 140 Mark Aquino Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Drop (in) Sampler Hammer 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks Elev Depth N-Value Number Sample Description Recov. (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) (Blows/ft) Scale +32. 10 20 30 40 ASPHALT (APPROX. 1-INCH-THICK) +32. SS Started drilling at 8:35 AM. 6 Brown f-c SAND, some f-c gravel, some red brick, trace SS 0' to 2' 6 silt, trace asphalt (moist) [FILL] S-1 ω 12 13 50/1 SS 2' to 4' S-2 SS No Recovery 50/1' GO BROOME DEVELOPMENT.GPJ 3 4 Drilled to 4'; heavy rig chatter. No Recovery SS 5 Installed casing to 4'. S-3 Drilled to 4'. 0 5 Class 7 SS 4' to 6' 6 SS 6' to 8' SS Brown f-c SAND, some fine gravel, some silt, some red 12 brick (moist) [FILL] 18 S-4 /LANGAN.COM/DATA/PAR/DATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 10 6 8 SS Installed casing to 8'. Brown f-c SAND, trace fine gravel, trace silt, trace red 3 Drilled to 8'. brick (moist) [FILL] S-5 SS 8' to 10' 9  $\alpha$ 8 +22. 10 SS SS 10' to 12' Brown f-m SAND, some silt (moist) 9 10 S-6 10 9 8 12 13 14 Class Installed casing to 15'. SS Brown f-m SAND, some silt (moist) Drilled to 15'. 6 S-7 SS 15' to 17' 9 16 7 17 18 19



Log of Boring LB-11 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.4 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +12.4 20 Installed casing to 20'. Brown Sandy SILT, trace fine gravel (moist) Drilled to 20'. SS S-8 SS 20' to 22' 21  $\infty$ 8 9 22 23 24 100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ....3/13/2019 10:46:56 AM ... Class +7.4 25 Drilled to 25'. SS Brown SILT, trace fine sand, trace clay (moist) SS 25' to 27' 5 10 26 6 6 27 28 29 +2.4 30 Drilled to 30'. 3 Brown SILT, trace fine sand, trace clay (wet) SS 30' to 32' 3 9 31 6 32 Class 6 33 34 -2.6 35 SS Drilled to 35'. Brown SILT, some clay, trace fine sand (wet) SS 35' to 37' 8 4 36 11 13 37 38 39 Class 5b Drilled to 40'. SS 15 Brownish-gray Clayey SILT (wet) SS 40' to 42' 15 9 13 42 43



3/13/2019 10:46:56 AM .

Log of Boring LB-11 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.4 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -12.6 10 20 30 40 45 -12.6 SS Drilled to 45'. Brown f-c SAND, some silt, trace clay (wet) SS 45' to 47' S-13 37 20 46 88 51 41 47 Class 48 49 50 Drilled to 50'. 12 Brown f-m SAND, trace silt (wet) SS 50' to 52' 18 14 13 52 - GO BROOME DEVELOPMENT.GPJ. 53 54 Class 55 Drilled to 55'. Brown f-c SAND, trace silt (wet) 10 SS 55' to 57' 12 16 56 15 16 ANGAN.COM/DATA/PAR/DATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 58 59 60 SS Drilled to 60'. Brown f-m SAND, trace silt (wet) 15 SS 60' to 62' S-16 21 61 23 22 62 Class 3a 63 64 -32.6 Drilled to 65'. SS 36 Brown Sandy SILT, trace fine gravel (wet) SS 65' to 67' 42 16 66 52 50 67 68 69



Log of Boring LB-11 Sheet of 4 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 32.4 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -37.6 SS Drilled to 70'. Reddish-brown f-c SAND and f-c GRAVEL, trace silt SS 70' to 72' (wet) S-18 48 10 30 38 72 Class 3a 73 74 Drilled to 75; slight rig chatter. SS 75' to 77' 30 Brown SAND, some fine gravel, trace silt (wet) 35 Class 3a 12 37 34 Finished drilling at 11:21 AM. "ILANGAN. COMIDATA\PAR\DATA8/100646801\ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ. End of boring at 77'. Backfilled boring with drill cuttings and patched surface 78 with asphalt upon completion. 79 80 81 82 83 85 86 87 88 89 91 92 93 94

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Log of Boring LB-12 Sheet of 5 Project Project No. Go Broome Development 100646801 Elevation and Datum Location 60 Norfolk Street, New York, New York Approx. el 31 NAVD88 **Drilling Company** Date Started Date Finished Craig Geotechnical Drilling Co., Inc. 2/11/19 2/11/19 Rock Depth **Drilling Equipment** Completion Depth 102 ft CME-75 Truck-Mounted Drill Rig NE Size and Type of Bit Disturbed Undisturbed Core Number of Samples 3-7/8" O.D. Tri-Cone Roller Bit 26 Casing Diameter (in) Casing Depth (ft) Completion 24 HR. First Water Level (ft.) 4" I.D. Steel 30  $\mathbf{V}$ Casing Hammer Automatic Drilling Foreman Weight (lbs) Drop (in) 30 140 Dave Cooke Sampler 2" O.D. Split Spoon Field Engineer Weight (lbs) Sampler Hammer Drop (in) 140 30 Safety Patrick Lenihan Sample Data MATERIAL SYMBOL Building Code Remarks N-Value Elev Depth Number Recov. (in) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale (Blows/ft) +31.0 10 20 30 40 ASPHALT (APPROX. 1-INCH-THICK) +30. SS Started drilling at 8:16 AM. 12 Gray to brown f-c SAND, some fine gravel, trace silt, SS 0' to 2' 10 trace red brick, trace asphalt (moist) [FILL] S-1 ω 5 SS SS 2' to 4' Dark gray f-c SAND and RED BRICK, trace fine gravel, trace silt (moist) [FILL] GO BROOME DEVELOPMENT.GPJ S-2 3 9 5 6 Drilled to 4'. Brown to tan f-c SAND and RED BRICK, trace fine SS SS 4' to 6' gravel, trace silt (moist) [FILL] 3 S-3 ω 5 3 11 6 SS 6' to 8' SS Brown to tan f-c SAND and RED BRICK, trace fine 8 gravel, trace silt, trace asphalt (moist) [FILL] 4 S-4 /LANGAN.COM/DATA/PAR/DATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 8 Class 7 SS Installed casing to 8'. Brown f-c SAND, some gravel, some red brick, trace silt, 9 Drilled to 8' trace asphalt (moist) [FILL] S-5 SS 8' to 10' 9 4 6 10 SS SS 10' to 12' RED BRICK, some fine gravel, trace silt (moist) [FILL] 8 S-6 က 12 SS Drilled to 12' Dark gray f-c SAND, some fine gravel, some red brick, 6 SS 12' to 14' trace silt (moist) [FILL] S-7 13 9 2 4 SS 14' to 16' Brown f-c SAND, some red brick, trace silt (moist) [FILL] 49 10 SS S-8 12 7 +15.0 Drilled to 16'. SS Brown f-m SAND, some silt (moist) SS 16' to 18' 10 17 8 Class 18 19



Log of Boring LB-12 Sheet of 5 2 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 31 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 +11.0 20 Installed casing to 20'. SS Brown f-m SAND, some silt (moist) Drilled to 20'. S-10 6 SS 20' to 22' 7 21 8 8 22 Class 3b 23 24 100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001 - GO BROOME DEVELOPMENT.GPJ ... 3/13/2019 10:47:06 AM +6.0 25 Drilled to 25'. SS Brown Sandy SILT (moist) SS 25' to 27' 9 18 26 10 27 Class 28 29 +1.0 30 Drilled to 30'. 3 Brown SILT, trace clay (wet) SS 30' to 32' 20 31 5 32 Class 6 33 34 -4.0 35 SS Drilled to 35'. Grayish-brown SILT, some clay (wet) SS 35' to 37' 6 9 36 9 17 37 38 39 Class 5b -9.0 Drilled to 40'. SS 10 Grayish-brown Clayey SILT (wet) SS 40' to 42' 10 2 16 42 43

LANGAN Log of Boring LB-12 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 31 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) 10 20 30 40 -14.0 -14. Drilled to 45'. Grayish-brown SILT, some clay (wet) SS 45' to 47' S-15 SS 16 16 46 33 17 - GO BROOME DEVELOPMENT.GPJ ... 3/13/2019 10:47:07 AM ... Report: Log - LANGAN 22 48 49 Class 50 Drilled to 50'. Grayish-brown SILT, some clay, trace fine sand (wet) SS 50' to 52' 18 18 27 52 53 54 -24.0 55 Drilled to 55'. 35 Brown f-c SAND, some silt (wet) SS 55' to 57' 28 8 56 29 32 IILANGAN.COMIDATAIPARIDATA8/100646801/ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 58 59 60 SS Drilled to 60'. Brown f-m SAND, some silt (wet) 18 SS 60' to 62' 23 61 22 23 62 Class 3a 63 64 SS Drilled to 65'. Brown f-c SAND, trace fine gravel, trace silt (wet) 19 SS 65' to 67' 22 7 66 25 67 68

69



3/13/2019 10:47:07 AM

/LANGAN.COM/DATA/PAR/DATA8/100646801/ENGINEERING DATA/GEOTECHNICAL/GINTLOGS/100610001

Log of Boring LB-12 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 31 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -39.0 10 20 30 40 SS Drilled to 70' Brown f-c SAND, some f-c gravel, trace silt (wet) SS 70' to 72' S-20 25 7 27 25 73 74 Drilled to 75; moderate rig Brown f-c SAND, trace fine gravel, trace silt (wet) 29 chatter. 32 SS 75' to 77' 12 29 30 - GO BROOME DEVELOPMENT.GPJ. 78 79 80 Drilled to 80'. Brown f-m SAND, some silt (wet) 22 SS 80' to 82' 81 33 36 82 83 85 SS Drilled to 85'. Brown f-m SAND, some silt (wet) SS 85' to 87' S-23 8 12 86 12 23 87 Class 3b 88 89 -59.0 Drilled to 90'. SS 32 Brown f-m SAND, some silt (wet) SS 90' to 92' 35 16 36 63 92 Class 3a 93 94



Log of Boring LB-12 Sheet of 5 Project Project No. Go Broome Development 100646801 Location Elevation and Datum 60 Norfolk Street, New York, New York Approx. el 31 NAVD88 Sample Data Building Code Remarks Elev (ft) Depth Scale N-Value (Blows/ft) Recov. (in) Penetr. resist BL/6in Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) -64.0 10 20 30 40 95 Drilled to 95'. -64. Gray CLAY, some silt (wet) SS 95' to 97' S-25 SS 30 16 96 32 36 97 98 Class 99 100 Drilled to 100'. Gray CLAY, trace silt (wet) 55 SS 100' to 102' 51 20 67 60 102 Finished drilling at 12:30PM. ILANGAN.COMIDATAIPARIDATA81100646801/ENGINEERING DATA\GEOTECHNICAL\GINTLOGS\100610001 - GO BROOME DEVELOPMENT.GPJ . End of boring at 102'. Backfilled boring with drill cuttings and patched surface 103 with asphalt upon completion. 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119

## **APPENDIX E**

**Laboratory Testing Results** 

## Langan Engineering #100646801 50 Norfolk St LABORATORY TESTING DATA SUMMARY

BORING	SAMPLE	DEPTH	IDENTIFICATION TESTS						REMARKS
			WATER	LIQUID	PLASTIC	PLAS.	USCS	SIEVE	
NO.	NO.		CONTENT	LIMIT	LIMIT	INDEX	SYMB.	MINUS	
							(1)	NO. 200	
		(ft)	(%)	(-)	(-)	(-)		(%)	
LB-2	S-17	60-62	30.8	26	21	5	CL-ML		
LB-4	S-7	15-17	26.3				SM	26.0	
LB-5	S-11	30-32	23.4				SM	21.8	
LB-5	S-14	45-47	29.8	23	20	3	ML		
LB-6	S-19	70-72	20.6				SM	12.5	
LB-7	S-10	25-27	23.2				SM	22.6	
LB-8	S-8	20-22	25.6				SM	20.6	
LB-11	S-12	40-42	28.7	25	21	4	CL-ML		·
LB-12	S-17	55-57	22.5				SP-SM	8.2	·
									·

Note: (1) USCS symbol based on visual observation and Sieve and Atterberg limits reported.

Prepared by: NG Reviewed by: CMJ Date: 3/6/2019

**TerraSense, LLC** 45H Commerce Way Totowa, NJ 07512 Project No.: 7920-910 File: Indx1.xlsx Page 1 of 1

