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Geotechnical Report Astoria Cove Development

Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York

Submitted to:

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Executive Summary

GEI Consultants, Inc., P.C. prepared this report to present the results of a subsurface exploration program and recommendations for the design of foundation supports for the design of Phase 1 of a redevelopment project in the Astoria neighborhood of Queens, New York. Phase 1 of the redevelopment project consists of four new mixed-use buildings between 6 and 26 stories high with cellar/basement footprints between 14,630 ft² and 37,700 ft², designated as Building 3A/3B, Building 4, and Building 5.

The top of the cellar/basement slab elevations for Buildings 3A/3B, Building 4, and Building 5 are at El. 6.00, El. 8.46, and El. 22.50, respectively. Localized top of mat for elevator and mechanical pits are called out as El. 2.46 in Building 4 and either El. 22.00 or El. 12.50 in Building 5. General excavations between 11 to 16 feet, 12 to 35 feet, and 2 to 28 feet will be required for Building 3A/3B, Building 4, and Building 5, respectively.

The geotechnical exploration program consisted of 27 geotechnical test borings advanced between about 55 and 102 feet below the ground surface. Subsurface conditions encountered in the borings were generally medium dense to dense miscellaneous granular fill, over discontinuous loose sand, over discontinuous stiff silt/clay, over medium dense sand, over very dense glacial till, over very dense soil-like weathered/decomposed rock, over metamorphic gneiss bedrock. Till soils were generally between 49 and 64 feet thick.

We recommend that the proposed buildings be supported on either a mat foundation or spread and continuous wall footings bearing on the Stratum III dense glacial till soils (Class 3a) or the overlying Stratum IIc sandy soils (Class 3b) or compacted Crushed Stone extending to Stratum III soils or compacted Structural Fill extending to Stratum IIc soils. The maximum allowable design bearing pressures that are recommended range from 3 to 8 tons/ft² as discussed in greater detail in this report. We recommend a unit value for the modulus of vertical subgrade reaction (for a 1-foot loaded area) of 300 tons/ft³ and 130 tons/ft³ for mat foundations bearing in Stratum III and Stratum IIc soils, respectively.

Alternatively, drilled pressure-grouted micropiles also can be considered to support the tower or podium cores for Building 3A/3B deriving support capacity in the dense glacial till soils. Micropiles can be designed to achieve 200-ton compression and 100-ton tension capacity and could also be used as tiedown anchors for the tower core for tension capacity if shallow foundations are used for the entire building footprint.

Groundwater levels were generally observed between El. 1.1 and El. 4 for wells located near Building 3A/3B, between El. 2.9 and El. 4.8 for wells located near Building 4, and between

El. 3.5 and El. 6.2 for wells located near Building 5. Therefore, for the design of the floor slabs and foundation walls, we recommend design groundwater elevations for Building 3A/3B, Building 4, and Building 5 as El. 5, El. 6, and El. 7, respectively. Additionally, well point or sump pit dewatering may be required during construction if excavations are performed to reach bearing strata.

We recommend using Site Class D (Stiff Soil Profile) for seismic design purposes, in accordance with Building Code § 1613.5.2. Based on the site geology, topography and subsurface investigations reported herein, it is our opinion that the proposed buildings are not susceptible to damage from liquefaction, slope instability, lateral spreading, or surface rupture due to faulting.

1. Introduction

GEI Consultants, Inc., P.C. prepared this report to present the results of a subsurface exploration program and foundation recommendations for the design of Phase 1 of a redevelopment project in the Astoria neighborhood of Queens, New York (Fig. 1). Phase 1 of the redevelopment project consists of four new mixed-use buildings, designated Building 3A/3B, Building 4, and Building 5. Slim Astoria 2468 LLC of Brookyln, New York engaged GEI to provide geotechnical services for this project.

1.1 Scope of Services

GEI completed the following scope of services for this report. These services were performed to investigate the subsurface conditions at the site.

- Reviewed site plans and preliminary building layout drawings.
- Engaged a test boring contractor to drill 27 geotechnical test borings and install six observation wells in the support of Building 3A/3B, Building 4, Building 5 for geotechnical design.
- Performed split spoon sampling and cumulatively collected about 68 feet of rock cores.
- Evaluated soil and rock samples recovered from the test borings and prepared test boring logs.
- Performed geotechnical index testing on soil samples from the test borings.
- Evaluated the data obtained and prepared this Geotechnical Report.

We performed these services in general accordance with the 2014 edition of the New York City Building Code (Building Code), which is an adaptation of the 2009 International Building Code.

1.2 Authorization

Our work was performed in general accordance with our revised proposal dated November 6, 2023, executed and authorized by Mr. Dov Strohli of Slim Astoria 2468 LLC.

1.3 Elevation Datum and Horizontal Coordinate System

Elevations provided in this report are in feet and are referenced to the North American Vertical Datum of 1988 (NAVD88). Horizontal coordinates (northing and easting) are U.S. State Plane coordinates in U.S. survey feet and are referenced to the North American Datum of 1983 (NAD83), New York State Plane Long Island Zone (3104).

1.4 Site Description

The site is currently used by various contractors to store equipment and materials and is irregularly shaped. Two vacant 1-story buildings are present on the northern portion of the site. The site is bounded to the north by the East River, to the west by one open lot and three buildings between 1- and 13-stories high (13-story building under construction), to the south by a 4-story building with three cellar levels and 27th Avenue beyond, and to the east by 9th Street, and eight buildings between 3- and 6-stories high.

Existing ground surface elevations vary between about El. -3 near the East River and up to El. 55 near the south portions of the site. An approximately 3H:2V slope exists from the East River to the vacant 1-story buildings and another approximately 2H:1V slope existing from south of 26th Avenue to north end of 8th Street. Fig. 2 depicts the general elevations at the site.

A Phase I Environmental Site Assessment (ESA) for the site indicates that the two vacant buildings were constructed in 1935 and 1943. Historical Sanborn Fire Insurance Maps for the site included in the Phase I ESA indicate that in 1898, 1915, and 1928, as many as 20 structures were once located at the site. It is likely that the foundations for many of those structures remain buried at the site after the demolition of the buildings.

1.5 FEMA Flood Mapping

The Federal Emergency Management Agency's (FEMA) Effective Flood Insurance Rate Map (FIRM), number 3404970093F dated September 5, 2007, shows that the site is predominantly outside the Special Flood Hazard Area (SFHA) and the Area of Minimal Flood Hazard (Zone X). There is a portion of the site closer to the East River that is identified as the 1 percent annual chance flood Zone AE (base flood elevation for inundation at El. 12). Properties to the west of the site are also in the SFHA designated as Zone AE (base flood elevation for inundation at El. 12) and Zone X.

From the New York City Department of City Planning updated flood maps (based on preliminary updated FIRMs, number 3604970093G, from FEMA in 2013), the site is in an Area of Minimal Flood Hazard (Prelim Shaded Zone X) with a 0.2 percent annual chance of

flooding (moderate flood hazard). There is a portion of the site closer to the East River that is identified as the 1 percent annual chance flood Zone AE (base flood elevation for inundation at El. 14) with a limited area for Moderate Waver Action. Properties to the west of the site are also in the SFHA designated as Zone AE (base flood elevation for inundation at El. 12) and Zone X.

The site is classified as Flood Design Class 2 per ASCE 24. However, none of the proposed structures are in the flood zone and not subject to Appendix G of the 2014 Building Code. Regardless, if ownership decides to design buildings for design flood elevations, any structures located below the base flood elevation should be floodproofed in accordance with the Building Code, New York State Building Code, ASCE 24, and all other agency requirements having jurisdiction. At a minimum, 1 foot of free board must be provided above the controlling base flood elevation.

1.6 **Project Description**

The proposed Phase 1 redevelopment involves construction of four new mixed-use buildings, designated Building 3A/3B, Building 4, and Building 5. Buildings 3A/3B will be located on Block 906 and Lots 1 (08-01 26th Avenue) and 5 (08-51 26th Avenue); Building 4 will be located on Block 909 and Lot 35 (4-34 26th Avenue); and Building 5 is location on Block 908 and Lot 12 (26-10 9th Avenue).

<u>Buildings 3A/3B</u> – Buildings 3A/3B will include a 26-story tower in the northwest corner of the building and an 8- to 9-story building podium with a full cellar. Buildings 3A/3B cellar footprint is approximately 37,700 ft² in area. Top of slab elevation for proposed the basement/cellar is at El. 6.00. The cellar will be used for parking, possible residential amenities, and mechanical rooms; the 1st floor will be used for parking, commercial retail space, and residential space; and floors 4 through 26 will be used for residential space and amenities.

The excavation for the cellar will extend down to approximately El. 6, which is approximately 7 to 15 feet below the existing ground surface.

The project Structural Engineer, Desimone Consulting Engineers, has provided the following preliminary service loads:

- Building 3A
 - Podium West Core: 3,000 kips Dead / 600 kips Live
 - o Podium East Core: 3,000 kips Dead / 600 kips Live
 - o Typical Podium Column: 600 kips Dead / 220 kips Live

- o East Wall: 1,600 kips Dead / 400 kips Live
- Building 3B:
 - Tower Core: 13,000 kips Dead / 600 kips Live
 - Typical Tower Interior Column: 1,150 kips Dead / 450 kips Live
 - o Podium East Core: 2,100 kips Dead / 450 kips Live
 - o Typical Podium Column: 600 kips Dead / 220 kips Live
 - o West Wall: 3,800 kips Dead / 800 kips Live

<u>Building 4</u> – Building 4 will consist of an 8-story building with a full cellar (about 21,192 ft² in area) below most of the building footprint. Building 4 has a footprint area of approximately 23,650 ft². Top of mat elevation for the proposed cellar is at El. 8.46 with a localized elevator pit top of mat at El. 2.46. The cellar will be used for parking and mechanical rooms; the 1st floor will be used for mechanical rooms, commercial retail space, and residential space; and floors 2 through 8 will be used for residential space.

The excavation for the cellar will extend down to approximately El. 8.5, which is approximately 12.5 to 39 feet below the existing ground surface.

Desimone Consulting Engineers has provided the following preliminary service loads:

- Building 4:
 - Tower Core: 3,600 kips Dead / 450 kips Live
 - Typical Tower Interior Column: 550 kips Dead / 110 kips Live
 - Podium Core: 800 kips Dead / 100 kips
 - Typical Tower Perimeter Column: 250 kips Dead / 50 kips Live
 - o Typical Interior Column: 400 kips Dead / 800 kips Live
 - o Typical Podium Perimeter Column: 200 kips Dead / 30 kips Live

<u>Building 5</u> – Building 5 will consist of a 6-story building (with penthouse) with a walk-out basement at grade in the northern portion of the building and extending to the southern portion of the building as a full cellar approximately 28.5 feet below the existing ground surface, due to the rise in elevation on the southern portion of the site. Building 5 will have a footprint of approximately 14,630 ft². Top of slab elevation for the proposed basement/cellar is at El. 22.50 with a localized elevator pit top of mat at El. 22.00 and a mechanical pit top of mat at El. 12.50. There will be a mechanical pit in the northeast corner of the building with an approximate footprint of 820 ft². The cellar/basement will be used for mechanical rooms

and residential space, and floors one through six and the penthouse will be used for residential space.

The excavation for the basement/cellar will extend to approximately El. 21.5 for the majority of the building except for the mechanical pit where an excavation to about El. 11.5 will be required, which is approximately 3 to 27.5 feet below the existing ground surface.

Desimone Consulting Engineers has provided the following preliminary services reactions:

- Building 5:
 - o Tower Core: 2,000 kips Dead / 250 kips Live
 - o Typical Tower Interior Column: 250 kips Dead / 50 kips Live
 - Podium Core: 600 kips Dead / 50 kips
 - Typical Tower Perimeter Column: 200 kips Dead / 25 kips Live
 - o Typical Interior Column: 550 kips Dead / 100 kips Live
 - Typical Podium Perimeter Column: 120 kips Dead / 25 kips Live

The anticipated foundation supports for Buildings 3A/3B, Building 4, and Building 5 are shallow foundations under most of the footprint with deep foundation element supports under the 26-story tower at Building 3B.

2. Subsurface Explorations

2.1 Exploration Program

The geotechnical exploration program consisted of 27 test boring that were performed in two mobilizations. Geotechnical test borings B1 through B23, B29, and B53 (B000-series borings) were drilled by Craig Geotechnical Drilling Co., Inc. of Mays Landing, New Jersey between March 2 and June 12, 2020. Geotechnical test borings B101 and B102 (B100-series borings) also were drilled by Craig Drilling on November 23 and 24, 2021.

The borings were drilled to depths between 55 and 102 feet below the existing ground surface, which translates to between El. -53.2 to El. -81.9. Approximate exploration locations are depicted in Fig. 2 and the exploration program is summarized in Table 1. Test boring logs as well as GEI's visual-manual description guide are included in Appendix A.

The test borings were advanced using either a CME 75 truck-mounted drilling rig or a CME 55LC track-mounted drilling rig with mud rotary drilling (bentonite-based), and 4-inch I.D. casing used to support the borehole.

A GEI geotechnical field representative observed and documented the geotechnical drilling and sample collection.

Standard Penetration Testing (SPT) and split-spoon sampling, in accordance with ASTM D1586, were generally performed continuously in the upper 12 feet of each boring and at 5-foot intervals, thereafter, using an automatic hammer.

Representative samples of the soils obtained from the borings were classified in accordance with ASTM D2488. Rock coring was performed using NX barrels in boreholes B8, B10, B11, B14, B15, B20, B23, and B101. Split-spoon samples were placed in appropriately identified sealed glass jars and rock cores were placed in wood core boxes. All samples were stored on-site.

Six groundwater observation wells were installed in completed borings B5, B8, B16, B29, B48 (off site well), and B53. Well construction details are presented in Appendix B.

An additional eight monitoring wells were installed in subsequent environmental investigations conducted in 2020 and are shown in Fig. 2. Well construction details are presented in Appendix B.

As-drilled boring locations were documented based on taped off measurements from existing site features. Approximate ground surface elevations at the exploration locations were estimated from the contours provided on Survey Number 44888-6 titled, "Survey of City of New York, County Queens, Tax Blocks as Shown," prepared by Montrose Surveying Co., LLP and revised October 27, 2023.

The boreholes were backfilled with drill cuttings and grout, with excess cuttings spread at the surface in the vicinity of the borehole.

3. Subsurface Conditions

3.1 Subsurface Conditions

Seven generalized strata were encountered in the test borings we performed and are described below, starting from the ground surface, and summarized in Table 1. Subsurface profiles based on the exploration information are presented in Figs. 3 through 6.

The drilling contractor used either a safety hammer with rope and cathead lifting system or an automatic hammer to perform the SPTs to measure the N-values in the field (termed N_m), which resulted in a variation in the efficiency of delivering the theoretical energy of a 140-pound weight freefalling 30 inches.

To use the data set in design, the N_m values for the automatic hammer were normalized to 60 percent of the theoretical energy delivered in the SPT (termed N_{60}), assuming that the automatic hammer was conservatively 80 percent efficient. The corrected "design" N-values are provided in Table 2. Since the N_m values obtained from a safety hammer with rope and cathead lifting system are typically 60 percent of the theoretical energy delivered in the SPT, no corrections were made on those SPTs.

The soil conditions are known only at the exploration locations. Conditions between exploration locations may vary significantly from the descriptions given below. The Building Code Material Classifications are given for each layer in paratheses.

- Stratum I Fill (Class 7)
- Stratum IIa Loose Sand (Class 6)
- Stratum IIb Silt/Clay (Class 4b/5b)
- Stratum IIc Sand (Class 3b)
- Stratum III Till (Class 3a)
- Stratum IV Decomposed/Weathered Rock (Class 1d)
- Stratum V Bedrock (Class 1b/1c)

<u>Stratum I – Fill (Class 7)</u> – Fill soils were encountered at all exploration location below up to 8 inches of asphalt or 12 inches of concrete. The thickness of the fill ranged from about 6 inches to 23.5 feet, but was generally 2 to 10 feet thick. The fill generally consisted of brown silty sand with varying amounts of glass, wood, bricks, and other construction debris.

SPT N_{60} -values ranged from 3 blows per foot to split spoon refusal, but were generally between 16 and 41 blows per foot, and averaged approximately 34 blows per foot, indicating generally dense soil conditions.

<u>Stratum IIa – Loose Sand (Class 6)</u> – Loose sand deposits were encountered in 10 of the 25 test borings performed below the fill and were observed to be between about 4 and 11.5 feet thick at the boring locations. The loose sand deposits consist primarily of narrowly and widely graded sand with up to about 10 percent nonplastic fines and up to 10 percent fine gravel. In B21, we observed that the loose sand layer contained up to 20 percent low plasticity clayey fines and up to about 30 percent gravel.

SPT N_{60} -values ranged from 3 to 24 blows per foot, but were generally between 7 and 11 blows per foot, and averaged approximately 9 blows per foot, indicating generally loose soil conditions.

<u>Stratum IIb – Silt/Clay (Class 4b/5b)</u> – Silt and clay deposits were encountered in B5, B9, and B21 below the fill and loose sand and were observed to be 9.5, 2, and 5 feet thick at the respective boring locations. The silt deposit observed at B5 consists of light brown nonplastic fines with up to 40 percent fine sand and included about a 3-foot-thick silty sand interlayer containing up to 40 percent nonplastic fines. The clay deposit observed at B21 consists of medium plasticity black fat clay with up to 10 percent gravel and up to 5 percent fine sand.

This stratum is likely discontinuous since the silt deposit encountered in B5 at about El. 21 and the clay deposit was encountered in B21 at about El. -14.5.

SPT N_{60} -values ranged from 9 to 25 blows per foot, but were generally between 13 and 19 blows per foot, and averaged approximately 16 blows per foot, indicating generally stiff to very stiff soil conditions.

<u>Stratum IIc – Sand (Class 3b)</u> – Sand deposits were encountered in all but four test borings performed below the fill and were observed to be between about 4 and 11.5 feet thick at the boring locations. Stratum IIc generally consists of orange-brown to tan narrowly graded sand with up to 40 percent nonplastic fines and up to 30 percent fine gravel. We observed the sand grade towards a silt before transitioning to Till in B3 and B5.

The top of sand contour plan presented in Fig. 7 depicts the estimated sand stratum surface as the top of sand drops in elevation towards the north portions of the site.

SPT N_{60} -values ranged from 7 blows per foot to split spoon refusal, but were generally between 14 and 21 blows per foot, and averaged approximately 20 blows per foot, indicating generally medium dense soil conditions.

<u>Stratum III – Till (Class 3a)</u> – Below the Stratum II deposit, a layer of dense glacial till was encountered at depths between 8 and 33.5 feet below grade corresponding to between El. -19.5 and El. 37.4. The top of till contour plan presented in Fig. 8 depicts the estimated till stratum surface as the top of till drops in elevation towards the north and northeast portions of the site.

Stratum III generally consists of brown silty sand, narrowly graded sand, widely graded sand, and narrowly graded gravel with up to 45 percent nonplastic to low plasticity fines and up to 60 percent gravel. The till was observed to be between 39 and 87 feet thick, but was generally between 49 and 64 feet thick and averaged 60 feet thick. Eleven test borings (B1, B3, B6, B7, B14, B17, B19 through B22, and B102) were terminated in the till.

Cobbles and boulders were encountered in the till in all test boring locations observed through rock core runs or observation of drilling rig reaction.

Light brown nonplastic silt was encountered in B3, B6, B16 with up to 25 percent sand.

We encountered spilt spoon refusal on a majority of the SPTs performed and assumed that this was due to the cobbles and boulders present in the till. Excluding the SPTs that met split-spoon refusal, SPT N_{60} -values ranged from 17 to over 100 blows per foot, but were generally between 49 and over 100 blows per foot, and averaged approximately 77 blows per foot, indicating very dense soil conditions.

<u>Stratum IV – Decomposed/Weathered Rock (Class 1d)</u> – Decomposed/weathered rock underlies the till layer and was encountered in 14 test borings at depths between 53 and 98 feet below grade corresponding to between El. -30.7 and El. -68.3. The decomposed/weathered rock was observed to be between 8.5 and 30 feet thick. Ten test borings (B2, B4, B5, B8, B9, B12, B13, B16, B18, and B101) were terminated in the decomposed/weathered rock.

The weathered rock primarily consists of rock that has decomposed to silt, sandy silt, silty sand, and narrowly graded sand with silt, all of varying colors (black, white, tan, blue, green, red, orange, brown, and yellow). Some samples contain friable rock, gravel, and clay.

SPT N_{60} -values ranged from 27 blows per foot to split spoon refusal. Most N_{60} -values were over 50 blows per foot, indicating very dense soil-like material. The weathered rock transitions to a more competent gneiss with depth but is defined as rock with Rock Quality

Designations (RQD) values less than or equal to 35 percent. In B15, we collected a 5-foot-long NQ core run with a RQD value of 0 percent, indicating very poor-quality rock.

<u>Stratum V – Bedrock (Class 1b/1c)</u> – Metamorphic gneiss bedrock was encountered in five borings (B8, B10, B11, B15, and B23) at 75 to 85 feet below the ground surface, corresponding to between El. -53.1 and El. -70.1. The gneiss was gray, black, and white, banded, medium to fine grained, hard with slight to moderate weathering. We collected eight 5-foot-long NQ cores, with recoveries that ranged from 63 to 100 percent, and averaged about 91 percent. RQD values ranged from 38 to 100 percent and averaged 75 percent, indicating fair to good-quality rock.

3.2 Groundwater Conditions

Groundwater levels were gauged in six observation wells installed in geotechnical test borings in March 2020 as identified in Fig. 2 as B5(OW), B8(OW), B16(OW), B29(OW), and B53(OW), with one of the wells, B48(OW), located about 400 feet northwest of B29(OW).

On a November 15, 2023 site visit, we could not find B16(OW), B29(OW), and B53(OW), and are assumed destroyed; B5(OW) and B8(OW) were found damaged and without surface casing; B48(OW) was found undamaged and was gauged. Historical groundwater data from these six observation wells are presented in Table 3.

Groundwater levels were also gauged on February 1 and February 2, 2022 at eight temporary monitoring wells installed on January 25 and January 26, 2022, as identified in Fig. 2 as MW-1 through MW-6, MW-11, and MW-12. Groundwater data from these eight monitoring wells are presented in Table 4.

Building	Observed Elevation Range in Wells	Top of Floor Slab Elevation	Recommended Design Groundwater Elevation
Building 3A/3B	1.1 to 4	6.00	5
Building 4	2.9 to 4.8	8.46	6
Building 5	3.5 to 6.2	22.50	7

Recommended design groundwater elevations are provided in the table below:

Groundwater levels are expected to fluctuate with season, precipitation, temperature, construction activity in the area, and other factors. Groundwater at the site also appears to be influenced by tidal fluctuations and we understand that the tidal range in the East River is about 3.6 to 4.7 feet. Groundwater level measurements represent conditions at the times and

locations when the measurements were made. Different groundwater elevations will occur at other times and locations.

3.3 Geotechnical Laboratory Testing

Our laboratory in Woburn Massachusetts performed the following tests on soil samples obtained from the test borings to confirm field classifications and to estimate engineering properties:

- 23 Grain size distribution tests. [ASTM D6913 and D7928]
- 2 Atterberg limits tests. [ASTM D4318]
- 25 moisture content tests. [ASTM 2216]

The results of laboratory testing are provided in Appendix C and have been incorporated into the soil descriptions on the boring logs in Appendix A.

3.4 Sample Storage and Disposition

Representative samples of the soils and cored rock samples were classified by the on-site GEI Field Inspector. Soil and rock samples collected as a result of our explorations are being temporarily stored at the on-site warehouse in Astoria, New York. The samples should be retained by the project until 1 year after the issuance of this report or after installed foundations have been approved by the NYC Department of Buildings, whichever is longer.

Building Code §1802.5.3 requires that:

"Soil and rock samples shall be maintained in an accessible location by the permit holder or owner and made available to the engineer responsible for the geotechnical investigation and to the department, until the foundation work has been completed and accepted, or until 1 year after the investigation is complete, whichever is longer."

4. Geotechnical Recommendations

The following sections of this report present the recommendations for the design of the foundation support for the proposed buildings for Phase 1 of the Astoria Cove development project. We understand that spread footings or mat foundations will be used under the majority of the building footprints, with the possible exception of deep foundations to support the tower at Building 3B.

4.1 Soil Properties

Recommended values of soil properties and design parameters are presented in Table 5. The properties were estimated using data from correlations with SPT N_{60} -values, laboratory testing results, published empirical correlations, and our engineering judgement.

4.2 Foundation Design Selection Approach

Underground program space is proposed for each building and there will be as much as 35 feet of overburden soil removed to reach floor slab subgrades. Based on the subsurface conditions encountered at the boring locations, the planned floor slab subgrades are generally within 2 feet, vertically, of Stratum III (Class 3a) soil deposits for most of the building footprint areas. The northern portions of Buildings 3A/3B and Building 5 would need up to about 11 feet and 18 feet, respectively, of overexcavation below the floor slab subgrades to reach the Stratum III deposits. For the northern portions of Building 3A/3B and Building 5, Stratum IIc soils are approximately 4 feet and 11 feet below the proposed floor slab subgrades, respectively.

The proposed buildings can be supported on either a mat foundation or spread and continuous wall footings bearing on the Stratum III soils or Stratum IIc soils or compacted Crushed Stone (refer to Recommended Material Specifications section of this report) extending to Stratum III soils or compacted Structural Fill extending to Stratum IIc soils.

Building Code Table 1804.1 provides the maximum allowable bearing pressure for material class. As stipulated in the Building Code, the allowable bearing pressure may be increased due to foundation embedment greater than 4 feet. As indicated above, construction of cellars/basements at each proposed building will require the removal of overburden soils and the modified allowable bearing pressures are presented for each building below.

4.2.1 Buildings 3A/3B

The foundations for Buildings 3A/3B will require the removal of at least 11 feet and up to approximately 16 feet of overburden soil across the footprint areas of the buildings. Based on the existing topography at the site and approximate cellar floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.7 and 1 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated two allowable bearing pressures zones (Zones 1 and 2) in Fig. 9 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 6.9 and 7 tons/ft², as depicted for each zone.

We also approximated two allowable bearing pressures zones (Zones 3 and 4) in Fig. 9 for support in the Stratum IIc soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 3.9 and 3.7 tons/ft², as depicted for each approximate zone.

Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations are to derive geotechnical capacity from either the Stratum IIc soils (overexcavating up to about 5 feet) or Stratum III soils (overexcavating up to about 11 feet). In the overexcavation areas identified in Fig. 9 compacted Structural Fill extending to Stratum IIc soils or compacted Crushed Stone extending to Stratum III soils would require allowable bearing pressures of 3 tons/ft² and 6 tons/ft², respectively. No overburden credit can be applied to those foundations bearing on Structural Fill or Crushed Stone (Zone 5).

Alternatively, drilled pressure-grouted micropiles can be used to support the tower or podium cores deriving their load carrying capacity in the till soils. Refer to micropile recommendations section below for additional details.

4.2.2 Building 4

The foundations for Building 4 will require the removal of at least 12.5 feet and up to approximately 35 feet of overburden soil across the footprint area of the building. Based on the existing topography at the site and approximate cellar floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.8 and 2 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated four allowable bearing pressures zones in Fig. 10 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure between 6.8 and 8 tons/ft², as depicted for each zone.

4.2.3 Building 5

The foundations for Building 5 will require the removal of at least 2.5 feet and up to approximately 27.5 feet of overburden soil across the footprint area of the building. Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations will derive geotechnical capacity from either the Stratum IIc soils (11 feet) or Stratum III soils (22 feet).

Based on the existing topography at the site and approximate cellar/basement floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.3 and 1.5 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated one allowable bearing pressures zone (Zone 1) in Fig. 11 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 7.5 tons/ft², as depicted.

We also approximated two allowable bearing pressures zones (Zones 2 and 3) in Fig. 11 for support in the Stratum IIc soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 4.3 and 3.5 tons/ft², as depicted, for each zone.

Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations are to derive geotechnical capacity from either the Stratum IIc soils (overexcavating up to about 11 feet) or Stratum III soils (overexcavating up to about 22 feet). In the overexcavation areas approximated in Fig. 11, compacted Structural Fill extending to Stratum IIc soils or compacted Crushed Stone extending to Stratum III soils would offer allowable bearing pressures of 3 tons/ft² and 6 tons/ft², respectively. No overburden credit can be applied to those foundations bearing on Structural Fill or Crushed Stone (Zone 4).

4.3 Settlement

From our experience with similar structures/foundation systems, and assuming subgrades are prepared and inspected as recommended in this report, we estimate total settlement of the structure will be less than 1 inch, with differential settlements less than ¹/₂ inch. Most of the expected settlements should occur during construction.

To minimize settlement after construction, we recommend the site grades be excavated to (or raised to) the finished floor elevations and then excavated to build foundations, as necessary. If grades need to be raised, the fill should be left in place 30 to 60 days prior to the construction of the building foundation.

Foundations should be designed and constructed in accordance with Building Code §1805. Shallow foundations should also satisfy the requirements contained in Building Code §1809.

We recommend that all fill placed below buildings meet the gradation and compaction requirements for Crushed Stone or Structural Fill provided in the Recommended Material Specifications Section.

4.4 Modulus of Subgrade Reaction for Foundation Mats

Terzaghi (1955) presents the following recommended values for modulus of vertical subgrade reaction, in tons/ft³, for a 1-foot loaded area (1-ft² or 1-foot-wide strip) bearing on sand.

Relative Density of Sand	Loose	Medium	Dense
Dry or moist sand	40	130	500
Submerged sand	25	80	300

The values for dense submerged sand are appropriate for Stratum III soils, so the recommended unit value for modulus of vertical subgrade reaction (for a 1-foot loaded area) is 300 tons/ft³.

The values for medium dense dry or moist sand are appropriate for Stratum IIc soils, so the recommended unit value for modulus of vertical subgrade reaction (for a 1-foot loaded area) is 130 tons/ft³.

The modulus of vertical subgrade reaction decreases as the size of the loaded area increases. The above unit value for a 1-foot loaded area must be adjusted based on the actual size of the loaded area. For a loaded area of width B (in feet), the unit value should be multiplied by a reduction factor of $[(B+1)/2B]^2$ per Terzaghi (1955). For a loaded area 10 feet wide, the reduction value is 0.30. For a loaded area of infinite width, the reduction factor is 0.25.

For a mat foundation, the effective loaded area is a function of the area of the loading applied to the mat and the mat stiffness (which distributes the load to the soil over a larger area). Foundation mats are typically designed as a relatively stiff reinforced concrete slab. For a

conservative value, we recommend using a reduction factor of 0.25. This gives a conservative area-adjusted modulus of vertical subgrade reaction of:

 $0.25 \ge 300 \text{ tons/ft}^3 = 75 \text{ tons/ft}^3 = 87 \text{ lb/in}^3$.

4.5 Footings and Floor Slab

We understand that portions of the cellar/basement floor may be designed as a framed slab or slab-on-grade over spread and continuous footings.

Footings on soil should be at least 3 feet wide. Foundations should bear at least 48 inches below the adjacent grade for frost protection, in accordance with Building Code §1809.3.1. Interior footings bearing on the Stratum III deposit or Structural Fill should bear at least 18 inches below the bottom of the floor slab.

We recommend that contraction joints be incorporated between the slab-on-grade and the columns and perimeter walls of the proposed buildings.

The floor slab should not rest directly on boulders. Protruding boulders should be removed and backfilled as needed to allow a minimum 9-inch-thick compacted Structural Fill or Crushed Stone "cushion" to be placed below the slab. Eliminating protruding boulders will reduce cracking of the slab that may result from stress concentrations.

We recommend that, at a minimum, a vapor retarder be installed below the slab-on-grade and to consult with the manufacturer of the proposed flooring system regarding vapor transmission and vapor retarder/waterproofing requirements.

4.6 Micropile Foundation Recommendations

Driven piles may prove difficult to install in the dense Stratum III (Class 3a) soil deposits and it may be difficult to achieve 200-ton compression and 100-ton tension capacity with driven piles. Therefore, we recommend that the northern portions of the Building 3B area, inclusive of the tower footprint, be supported by pressure-grouted micropiles bearing in Stratum III soils.

During installation, these micropiles are drilled with steel-casing to the design tip-elevation. The soil socket is achieved by filling the entire casing with grout, by a tremie pipe, and extracting the amount of casing needed for the length of the soil socket as measured from the bottom of the pile. As casing is extracted, the grout is pressurized to about 50 lb/in², or as determined by the pile designer. Micropiles should be designed by a professional engineer licensed in the State of New York with experience on similar projects.

We recommend a preliminary micropile design consisting of:

- 50 kips/in² (ksi) steel casing with the dimensions of 13.375 inches outer diameter and ¹/₂ inch in thickness.
- Two 75 ksi #18 steel-reinforcing-bars for the whole length of the pile.
- 5,000 lb/in² compressive strength grout.
- Minimum soil socket of 35 feet in length installed entirely in Stratum III.
- 3 feet minimum center-to-center spacing of piles.

Drilled pressure-grouted micropiles can be used to achieve the 200-ton compression and 100-ton tension capacity and could also be used as tiedown anchors for the towner core for tension capacity if shallow foundations are used for the entire building footprint.

A minimum of two compression load tests will be required for the micropiles. To substantiate loads larger than the allowable loads of deep foundations listed in the Building Code, the piles will either need to be instrumented using strain gauges or be subject to cyclical loading per ASTM D1143. Additionally, the final load increment will need to be held for a minimum period of 24 hours. A minimum of two uplift load tests are recommended. The number of uplift load tests will be determined by the professional engineer of record. Lateral load testing is required per the Building Code if the design lateral load exceeds 1 ton.

Micropiles should be installed under full-time Special Inspection in accordance with the Building Code. Use of air hammers should be prohibited for drilling through the overburden soils. Internal flushing of drilling fluids should be required and a positive head shall be always maintained within the casing. Observation of the soil cutting quantity in the wash water should be closely monitored during installation.

4.7 Seismic Design

We initially recommend using Site Class D (Stiff Soil Profile) for seismic design purposes, in accordance with Building Code §1613.5.2, based on the N_{60} -values and the subsurface profile encountered at the site. The site is borderline Site Class C (Very Dense Soil and Soft Rock Profile), which can be further refined and proven out by performing geophysical shear wave velocity studies of the subsurface to evaluate whether Site Class C can be assigned to this site. Using the seismic design values associated with Site Class C, the project design tea

m may be able to find cost savings relative to seismic reinforcement requirements compared with Site Class D recommendations.

-			
Building Code			
Seismic Design Values			
Site Class	С	D	
Risk Category	II	II	
Ss	0.279 g	0.279 g	
S ₁	0.072 g	0.072 g	
S _{DS}	0.223 g	0.293 g	
S _{D1}	0.082 g	0.115 g	
PGA _M	0.20 g	0.24 g	
Seismic Design Category	В	В	

Corresponding spectral response design parameters are as follows:

We calculated the spectral response parameters for the Site using general procedures outlined in Building Code §1613.5. Peak ground acceleration (PGA_M) is adjusted for Site Class effects, per ASCE 7-10 §11.4.7, and were selected from Building Code Table 1813.2.1.

Based on a Site Class D and Risk Category II for the proposed building, and in accordance with Building Code Tables 1613.5.6(1) and (2), Seismic Design Category B should be assigned to the structure for design.

Based on the site geology, topography and subsurface investigations reported herein, it is our opinion that the proposed building is not susceptible to damage from liquefaction, slope instability, lateral spreading, or surface rupture due to faulting.

4.8 Waterproofing and Dampproofing

Groundwater levels observed in wells were in close proximity to the proposed cellar floor slab elevation in Building 3A/3B. In addition, groundwater levels could temporarily rise during flood conditions. Therefore, we recommend that all floors and walls of the cellar should be waterproofed in accordance with Building Code §1807.

A vapor retarder consisting of polyethylene membrane should also be installed below all moisture sensitive slabs (i.e. occupied interior spaces). The installed vapor retarder should be sealed at the foundation walls, columns, and utility penetrations, and panels should be overlapped and joints sealed. We also recommend that waterstops be provided at all joints in the concrete basement walls and floor slab, and that the cellar/basement walls be waterproofed/dampproofed.

We recommend that isolated structures that extend below the slab and near or below the design groundwater elevation in any of the four buildings, such as elevator pits and sump pits, be waterproofed in accordance with Building Code §1807 and designed for hydrostatic uplift pressures.

4.9 Permanent Below Grade Walls

We recommend that cellar/basement walls be designed for the lateral pressures shown in Fig. 12.

Below grade walls used for this project should be designed for at-rest earth pressures of 60 lb/ft^2 per foot of depth above the groundwater level. Based on the groundwater conditions observed at the site, we do not expect hydrostatic pressures will act on cellar/basement wall. Although hydrostatic pressures are not expected, the cellar/basement walls should be waterproofed. Rainwater and water falling on impermeable surfaces should be tied to subgrade stormwater sewers and directed away from cellar/basement walls.

An earthquake pressure of 10.8 lb/ft² per foot of depth should be applied to the walls and be distributed as an inverse triangle over the height of the wall.

In addition to the lateral loads exerted by the soil against the walls, allowance should be included for lateral stresses imposed by any temporary or long-term surcharge loads, such as cars or trucks adjacent to the walls. The wall design shall also include any surcharge loads that may develop within a 45° slope from the toe of the wall using 50 percent of the adjacent surcharge load over the height of the wall. Design of cellar walls should not incorporate the lateral support provided by temporary shoring works that are used to facilitate excavation to the cellar level.

4.10 Temporary Support of Excavation (SOE)

Permanent cellar/basement walls and temporary Support of Excavation (SOE) walls will be required to construct the proposed below-grade space at all four building sites. Temporary SOE might be required to perform the undercut to Statum III soils within the footprint of the buildings.

Temporary SOE is required for vertical excavations deeper than 4 feet to support the cut face. Temporary walls should be designed to withstand lateral earth pressures and surcharge loads from construction staging, equipment, stockpiles, and adjacent traffic loads.

Based on the existing grades and the proposed lowest level of the structure, cut heights up to 35 feet are expected. An SOE system consisting of cantilevered soldier piles and lagging is

therefore feasible. Alternatively, a braced SOE system consisting of soldier piles and rakers with heel blocks can be used..

4.11 Site Retaining Wall Designs

All permanent earth retaining structures used on the project across the site should be designed using the following earth pressure parameters:

Parameter	Value
Total Unit Weight of Granular Backfill (γ)	125 lb/ft ³
Angle of Internal Friction (φ)	32°
Cohesion (c)	0 lb/ft ²
At-Rest Earth Pressure Coefficient, (K _o)	0.47 ⁽¹⁾
Active Earth Pressure Coefficient, (K _a)	0.31 ⁽¹⁾
Passive Earth Pressure Coefficient, (K _p)	3.0 ⁽¹⁾
Allowable coefficient of friction between rough concrete footing and granular bearing soil	0.50 ⁽¹⁾

⁽¹⁾ = Ultimate values, with no Factor of Safety applied

Retaining walls free to rotate at the top should be designed for active earth pressures. In addition to the lateral loads exerted by the soil against the walls, allowance should be included for lateral stresses imposed by any temporary or long-term surcharge loads, such as cars or trucks adjacent to the walls or adjacent footing loads.

The recommended wall design parameters do not consider the development of hydrostatic pressure behind the walls. As such, positive wall drainage must be provided for all earth retaining structures. These drainage systems can be constructed of open-graded washed stone isolated from the soil backfill with a geosynthetic filter fabric and drained by perforated pipe, or several wall drainage products made specifically for this application. Where backfill soils are not drained using an appropriately designed drainage system, the lateral soil pressure on proposed retaining walls must consider hydrostatic forces and submerged soil unit weight.

The earth pressure parameters given in the above table assume placement and compaction of the backfill in accordance with recommendations elsewhere in this report. Compact backfill directly behind walls with light, hand-operated compactors. Heavy compactors and grading equipment should not be allowed to operate within 10 feet of the walls during backfilling to avoid developing excessive temporary or long-term lateral soil pressures.

The soil parameters used in the design of the walls should be based on tests performed on the actual soil materials that will be used to construct the walls.

5. Construction Considerations

5.1 Excavation, Shoring, and Dewatering

Excavations should be made in accordance with Occupational Safety and Health Administration (OSHA) standards and the Building Code pertaining to the protection of property and worker safety. Any excavation greater than 4 feet in depth should be inclined for safety unless shoring is used. Excavations adjacent to the existing structure foundations should not extend below the bottom of the existing foundations without prior underpinning of the existing structure foundations, if required.

Because of the depth of excavation, we recommend that temporary excavation support systems be used for the cellar/basement excavations and foundation construction at each building site. A Professional Engineer licensed in the State of New York and experienced in the design of excavation support systems should design the excavation support.

The excavation support system should be designed to minimize settlement and lateral movement of surrounding structures including buildings and utilities. Vibrations to nearby structures should be limited during installation of the excavation support elements. We recommend monitoring adjacent structures for vibration during installation of excavation support.

Design groundwater elevations for Building 3A/3B, Building 4, and Building 5 are El. 5, El. 6, and El. 7, respectively. We understand that, in general, proposed cellar/basement levels and mechanical pits will be above groundwater levels. In addition, overexcavation to reach target bearing stratum will likely extend below the groundwater table. The contractor will need to design and install groundwater dewatering systems appropriate to lower groundwater levels to maintain stable subgrades.

Localized dewatering of subgrades can be achieved through a temporary dewatering wellpoint system. Groundwater levels shall be a minimum of 2 feet below final subgrade elevations prior to excavation for final subgrade.

The designer of the dewatering systems should consider the effects of drawdown outside the limits of excavation. The temporary dewatering system shall be designed by a Professional Engineer from the State of New York and in compliance with NYCDEP water discharge guidelines.

Site runoff may infiltrate the excavations. Removal of runoff may be accomplished with filtered sumps and pumps. The sumps should consist of a perforated pipe wrapped in Geotextile and surrounded by Pea Stone. The site should be graded to route stormwater runoff away from excavations.

5.2 Subgrade Preparation

Foundations may bear directly on the Stratum IIc or Stratum III soils or compacted Structural Fill extending to Stratum IIc or Stratum III soils. Lean concrete, Controlled Low Strength Material (CLSM), or tamped Crushed Stone wrapped in Geotextile may be substituted for Structural Fill. Any loose or disturbed soil should be removed from the bottom of the excavation, and the subgrade should be proof compacted with a vibratory compactor.

We recommend the following for preparing subgrades below foundations:

- Remove any loose or disturbed soil from the bottom of the excavation and compact the subgrade with a vibrator compactor weighing at least 200 pounds and imparting an impact load of at least 2.5 tons. Any observed soft or 'pumping' zones in the subgrades should be overexcavated and replaced with either compacted Structural Fill or with Crushed Stone wrapped in Geotextile.
- 2. If encountered, uncontrolled fill is not suitable for structural support and should be excavated from below all foundations and replaced with compacted Structural Fill.
- 3. Concrete for foundations may be placed directly on the soil subgrade. Bearing surfaces should be free of standing water, frost, loose soil, and soil softened by water intrusion before placement of reinforcing steel and concrete. Areas of the subgrade disturbed by traffic or surface water should be re-compacted.
- 4. A 6-inch layer of compacted crushed stone over geotextile fabric or a 3- to 4-inch-thick lean concrete mud slab may be required to stabilize foundation subgrades, protect subgrades against weather, and provide a working platform.

We recommend that a GEI engineer observe the final preparation of subgrades prior to foundation construction. The subgrade compaction should be observed continuously by a qualified Special Inspector, a service that GEI can provide.

5.3 Freezing Conditions

The soils at the site are frost susceptible. Therefore, if construction is performed during freezing weather, special precautions will be required to prevent the subgrade soils from freezing.

All subgrades should be free of frost before placement of concrete. Frost-susceptible soils that have frozen should be removed and replaced with compacted Structural Fill.

Soil placed as fill should be free of frost, as should the ground on which it is placed.

If mud slabs or the foundation mat are built and left exposed during the winter, precautions should be taken to prevent freezing of the underlying soil. Freezing of the soil beneath the foundation during construction may result in subsequent settlement of the structure.

5.4 Backfilling and Compaction

Any fill placed within the building limits should meet the gradation and compaction requirements for Structural Fill. Lean concrete, CLSM, or Crushed Stone wrapped in Geotextile may be substituted for Structural Fill. Fill outside the structure limits should meet the gradation and compaction requirements for Ordinary Fill. Ordinary Fill may be susceptible to frost heave. The potential for frost heave can be reduced by grading outside areas for proper drainage and by using Structural Fill rather than Ordinary Fill for the top 1 to 2 feet.

The lateral pressures given in Fig. 12 assume placement and compaction of the backfill in accordance with recommendations in this report. Compact backfill directly behind walls with light, hand-operated compactors. Heavy compactors and grading equipment should not be allowed to operate within 10 feet of the walls during backfilling to avoid developing excessive temporary or long-term lateral soil pressures.

5.5 Adjacent Structures

In accordance with Building Code §107.7.1, we recommend indicating foundation elevations of adjacent structures on any structural foundation plans. We understand that underpinning of adjacent structures will not be required on this project.

We recommend that the ownership team retain an instrumentation consultant, independent of the construction contractor, to perform optical survey monitoring of the support of excavation systems and any existing adjacent structures that might be affected by the excavations. GEI can provide these services, if requested.

5.6 Site Disturbance

Project specifications should require the contractor to maintain stable site conditions at all times. Specific measures to stabilize equipment traffic areas could include compacted aggregate placed over a Geotextile, temporary drainage, or other appropriate measures.

5.7 Wet Site Conditions

The contractor should be prepared to handle excessively over-optimum (wet) soil moisture conditions during earthwork activities in the winter or spring as well as during or following periods of rain. Site soils could easily become disturbed, and the ground surface could become excessively muddy and unstable due to traffic by construction equipment. Colder months of the year generally provide fewer opportunities to dry soils that have become wet. Wet soil subgrades or fill can make proper compaction difficult or impossible. Possible mitigating measures for wet soil conditions are as follows:

- 1. Frequent spreading and mixing during warm dry weather.
- 2. Mixing with drier materials.
- 3. Mixing with a lime, lime-fly ash, or cement product.
- 4. Stabilizing with aggregate, geotextile stabilization fabric, or both.

Options 3 and 4 should be evaluated and approved by the Geotechnical Engineer prior to implementation.

6. Limitations

This report was prepared for the exclusive use of Slim Astoria 2468 LLC and the design team. Our recommendations are based on the project information provided to us at the time of this report and may require modification if there are any changes in the nature, design, or location of the proposed structure. We cannot accept responsibility for designs based on our recommendations unless we are engaged to review the final plans and specifications to assess whether any changes in the project affect the validity of our recommendations and whether our recommendations have been properly implemented in the design.

The recommendations in this report are based in part on the data obtained from the subsurface explorations. The nature and extent of variations between explorations may not become evident until construction. If variations from the anticipated conditions are encountered, it may be necessary to revise the recommendations in this report. We, therefore, recommend that GEI be engaged to make site visits during construction to: a) check that the subsurface conditions exposed during construction are in general conformance with our design assumptions, and b) ascertain that, in general, the work is being performed in compliance with the contract documents.

Our professional services for this project have been performed in accordance with generally accepted engineering practices; no warranty, express or implied, is made. This report should be, per Building Code §1802.6, submitted to the building official by any permit applicant at the time of permit application.

Recommended Material Specifications

Structural Fill and Ordinary Fill shall consist of hard, durable sand and gravel, free of clay, organic matter, surface coatings, and other deleterious materials. Soil finer than the No. 200 sieve (the "fines") should be nonplastic. On-site material can be re-used as Structural Fill or Ordinary Fill, provided they can meet the appropriate gradation and compaction requirements indicated below and do not contain deleterious materials. Soils to be used as fill imported from off site should also meet the gradation requirements given below.

Structural Fill

Sieve Size	Percent Passing by Weight
3 inches	100
0.5 inch	50 to 100
No. 4	35 to 85
No. 16	20 to 65
No. 50	5 to 40
No. 200 (fines)	0 to 8

Structural Fill shall meet the following gradation requirements:

Structural Fill shall be compacted in maximum 9-inch-thick, loose lifts to at least 95 percent of the maximum dry density determined in accordance with ASTM D1557 (Modified AASHTO Compaction). The moisture content should be held to within ± 3 percent of optimum moisture content (as determined by ASTM D1557).

Ordinary Fill

Ordinary Fill shall meet the following gradation requirements:

Sieve Size	Percent Passing by Weight
6 inches	100
3 inches	80 to 100
No. 4	20 to 100
No. 200 (fines)	0 to 20

Ordinary Fill shall be compacted in maximum 12-inch-thick, loose lifts to at least 92 percent of the maximum dry density determined in accordance with ASTM D1557 (Modified AASHTO Compaction). The moisture content should be held to within ± 3 percent of optimum moisture content (as determined by ASTM D1557).

Crushed Stone

Crushed Stone shall consist of clean, durable, sharp-angled fragments of gravel free from surface coatings and shall conform to the requirements of material designation 703-0201 No. 1 Crushed Stone in the May 1, 2020 Standard Specifications prepared by New York State Department of Transportation (NYSDOT). Materials that break up when alternatively, frozen and thawed or wetted and dried shall not be used. Crushed Stone shall meet the following gradation requirements or approved equal:

Sieve Size	Percent Passing by Weight
1 inch	100
0.5 inch	90 to 100
0.25 inch	0 to 15
No. 200 (fines)	0 to 1.0

Crushed Stone should be compacted with at least four passes of a static vibratory compactor.

Pea Stone

Pea Stone shall be screened gravel conforming to the requirements of ASTM D 448 for stone size No. 8 and shall consist of durable, inert rounded stone, free from loam or clay, surface coatings, and deleterious materials. Pea Stone shall meet the following gradation requirements:

Sieve Size	Percent Passing by Weight
0.5 inch	100
0.375 inch	85 to 100
No. 4	10 to 30
No. 8	0 to 10
No. 16	0 to 5

Controlled Low Strength Material

Controlled Low Strength Material (CLSM) shall conform to material designation 733-01 Type 1/2 in the May 1, 2020 Standard Specifications prepared by the NYSDOT.

Geotextile

Geotextile should be a non-woven fabric, consisting of Mirafi 140N or an approved equal product.
Geotechnical Report Astoria Cove Development Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York December 2023

Tables

Table 1. Subsurface Exploration DataGeotechnical ReportAstoria Cove DevelopmentAstoria, Queens, New York

							Approximate Top of Layer ^(2,3,4) (feet)													
Exploration		Northing ⁽¹⁾	Easting ⁽¹⁾	Ground Surface Elevation ⁽²⁾	Total Depth of Exploration	Bottom of Exploration Elevation	Stra F (Cla	itum I Fill Iss 7)	Strat Loos (Cla	um Ila e Sand iss 6)	Strat Silt (Class	um IIb /Clay s 4b/5b)	Strat Sa (Clas	um IIc and ss 3b)	Strat T (Clas	tum III 'ill ss 3a)	Strati Decomp athere (Clas	um IV osed/We d Rock <u>s 1d)</u>	Strat Bed (Class	tum V Irock 5 1b/1c)
ID	Building ID	(feet)	(feet)	(feet)	(feet)	(feet)	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.
B1	Bldg. 5	221,653	1,003,263	48.8	102.0	-53.2	0.0	48.8	2.0	46.8			8.0	40.8	28.5	20.3				
B2	Bldg. 5	221,695	1,003,280	48.4	102.0	-53.6	0.0	48.4					7.0	41.4	11.0	37.4	97.7	-49.3		
B3	Bldg. 5	221,718	1,003,305	46.0	102.0	-56.0	0.0	46.0					10.0	36.0	28.5	17.5				
B4	Bldg. 5	221,775	1,003,310	24.7	102.0	-77.3	0.0	24.7					2.0	22.7	10.0	14.7	93.0	-68.3		
B5(OW)	Bldg. 5	221,770	1,003,343	24.8	102.0	-77.2	0.0	24.8			4.0	20.8	13.5	11.3	24.0	0.8	85.6	-60.8		
B6	Bldg. 4	221,715	1,003,183	46.2	102.0	-55.8	0.0	46.2	10.0	36.2			16.0	30.2	28.5	17.7				
B7	Bldg. 4	221,766	1,003,203	40.1	102.0	-61.9	0.0	40.1					18.5	21.6	33.5	6.6				
B8(OW)	Bidg. 4	221,825	1,003,207	23.6	85.0	-61.4	0.0	23.6					8.0	15.6	17.0	6.6	/6.1	-52.5	80.0	-56.4
B9	Bldg. 4	221,855	1,003,277	22.3	102.0	-79.7	0.0	22.3	0.7	21.7	6.0	16.3	8.0	14.3	10.5	11.8	88.5	-66.2		
<u>B10</u>	Bidg. 4	221,877	1,003,222	21.9	85.0	-63.1	0.0	21.9	2.0	19.9					13.5	8.4	62.7	-40.8	75.0	-53.1
<u>B11</u>	Bldg. 4	221,898	1,003,171	21.7	90.0	-68.3	0.0	21.7	4.0	17.7			10.0	11.7	13.5	8.2	/1.5	-49.8	80.0	-58.3
B12	Bldg. 3A/3B	222,043	1,003,593	14.8	77.0	-62.2	0.0	14.8					13.0	1.8	23.5	-8.7	67.7	-52.9		
B13	Bldg. 3A/3B	221,884	1,003,427	23.5	102.0	-78.5	0.0	23.5	6.0	17.5			10.0	13.5	18.5	5.0	82.7	-59.2		
<u>B14</u>	Bldg. 3A/3B	221,930	1,003,368	21.8	77.0	-55.2	0.0	21.8	4.0	17.8			10.0	11.8	13.5	8.3				
B15	Bldg. 3A/3B	221,889	1,003,534	21.9	93.0	-/1.1	0.0	21.9	2.0	19.9			6.0	15.9	13.5	8.4	52.6	-30.7	83.0	-61.1
B16(UW)	Bldg. 3A/3B	221,903	1,003,499	21.9	102.0	-80.1	0.0	21.9					4.0	17.9	10.0	11.9	73.5	-51.0		
B17	Bldg. 3A/3B	221,948	1,003,379	21.5	/5.3	-53.8	0.0	21.5					1.0	20.5	13.5	8.0				
B18	Bldg. 3A/3B	221,937	1,003,535	20.6	700.2	-79.6	0.0	20.6					2.0	18.0	23.5	-2.9	68.5	-47.9		
B19	Bldg. 3A/3B	221,997	1,003,398	21.4	75.4	-54.0	0.0	21.4					8.0	13.4	13.5	7.9				
B20	Bldg. 3A/3B	222,000	1,003,420	20.2	75.1	-54.9	0.0	20.2	 10 E						8.U	12.2				
B21	Bldg. 3A/3B	222,090	1,003,573	14.0	72.0	-58.0	0.0	14.0	18.5	-4.5	28.5	-14.5			33.5	-19.5				
B22	Bldg. 3A/3B	222,064	1,003,531	17.5	75.1	-57.0	0.0	17.5					10.0	7.5	18.5	-1.0				
B23	BIOG. 3A/3B	222,133	1,003,480	15.0	90.1	-75.1	0.0	15.0							23.5	-6.5	73.5	-36.5	85.1	-70.1
		222,000	1,003,320	21.9	80.U	-03.1	0.0	21.9					8.U	13.9	23.3	-1.0	07.0	-45.1	80.0	-38.1
D03(UVV)		221,/09	1,003,421	29.4	55.U 102.0	-20.0	1.0	20.4					4.0	20.4	13.5	10.9	30.0	-9.1	45.0	-15.0
B102	Bldg 3A/3B	222,007	1,003,490	20.1	102.0 86.8	-01.9	0.0	20.1	0.0	12.1			19.0	7.0	23.3 18.5	-3.4	07.0	-00.9		
DIVZ	Diuy. SAISD	221,943	1,003,439	Z1.4	00.0	-00.4	0.0	Z1.4					13.5	1.9	10.0	۲.۶				

Footnotes:

1. Horizontal Coordinates are U.S. State Plane Coordinates in U.S. Survey feet and are referenced to the North American Datum of 1983 (NAD83).

2. Elevations are referenced to the North American Vertical Datum of 1988 (NAVD88) and estimated from topographic plan prepared by Montrose Surveying.

3. "--" indicates that layer was not encountered.

			Field Measured	Equivalent
Boring	Building	Sample	N-value. N.,	N-value. N _{co}
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 5	S1	33	44
	Bldg. 5	S2	12	16
	Bldg. 5	S3	7	9
	Bldg. 5	S4	7	9
	Bldg. 5	S5	13	17
	Bldg. 5	S6	11	15
	Bldg. 5	S7	12	16
	Bldg. 5	S8	13	17
	Bldg. 5	S9	25	33
	Bldg. 5	S10	33	44
	Bldg. 5	S11	48	64
R1	Bldg. 5	S12	160/9"	60
ы	Bldg. 5	S13	25	33
	Bldg. 5	S14	106/3"	100
	Bldg. 5	S15	117	100
	Bldg. 5	S16	100/5"	100
	Bldg. 5	S17	101/5"	100
	Bldg. 5	S18	100/5"	100
	Bldg. 5	S19	102/2"	100
	Bldg. 5	S20	101/4"	100
	Bldg. 5	S21	178/10"	100
	Bldg. 5	S22	102/5"	100
	Bldg. 5	S23	104/2"	100
	Bldg. 5	S24	111	100
	Bldg. 5	S1	19	25
	Bldg. 5	S2	9	12
	Bldg. 5	S3	7	9
	Bldg. 5	S4	11	15
	Bldg. 5	S5	20	27
	Bldg. 5	S6	45	60
	Bldg. 5	S7	13	17
	Bldg. 5	S8	31	41
	Bldg. 5	S9	41	55
	Bldg. 5	S10	115	100
	Bldg. 5	S11	150	100
B2	Bldg. 5	S12	76	100
DZ	Bldg. 5	S13	86	100
	Bldg. 5	S14	103/5"	100
	Bldg. 5	S15	103/5"	100
	Bldg. 5	S16	104/5"	100
	Bldg. 5	S17	62/0"	100
	Bldg. 5	S18	179/10"	100
	Bldg. 5	S19	101/2"	100
	Bldg. 5	S20	104/5"	100
	Bldg. 5	S21	101/3"	100
	Bldg. 5	S22	189/11"	100
	Bldg. 5	S23	102/5"	100
	Blda. 5	S24	71	95

			Field Measured	Equivalent
Boring	Building	Sample	N-value, N.,	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 5	S1	17	23
	Bldg. 5	S2	11	15
	Bldg. 5	S3	4	5
	Bldg. 5	S4	2	3
	Bldg. 5	S5	14	19
	Bldg. 5	S6	13	17
	Bldg. 5	S7	7	9
	Bldg. 5	S8	18	24
	Bldg. 5	S9	14	19
	Bldg. 5	S10	27	36
	Bldg. 5	S11	80	100
B3	Bldg. 5	S12	111	100
	Bldg. 5	S13	78	100
	Bldg. 5	S14	101/3"	100
	Bldg. 5	S15	100/3"	100
	Bldg. 5	S16	100/3"	100
	Bldg. 5	S17	124/5"	100
	Bldg. 5	S18	102/5"	100
	Bldg. 5	S19	103/5"	100
	Bldg. 5	S20	55/0"	100
	Bldg. 5	S21	178	100
	Bldg. 5	S22	100/4"	100
	Bldg. 5	S23	100/2"	100
	Bldg. 5	S24	173	100
	Bldg. 5	S1	21	28
	Bldg. 5	S2	9	12
	Bldg. 5	S3	9	12
	Bldg. 5	S4	8	11
	Bldg. 5	S5	9	12
	Bldg. 5	S6	47	63
	Bldg. 5	S7	107/3"	100
	Bldg. 5	S8	101/3"	100
	Bldg. 5	S9	39	52
	Bldg. 5	S10	100/5"	100
	Bldg. 5	S11	125	100
B4	Bldg. 5	S12	108	100
	Bldg. 5	S13	106/5"	100
	Bldg. 5	S14	97	100
	Bldg. 5	S15	101/2"	100
	Bldg. 5	S16	92	100
	Bldg. 5	<u>S1/</u>	100/5"	100
	Blag. 5	518	107/4"	100
	Blag. 5	S19 800	100/2"	100
	Blag. 5	S20	126/3"	100
	Blag. 5	521	100/2"	100
	Blug. 5	522	102/5	100
	Diuy. D	020 004	41	00 04
	Diuy. D	324	23	31

Boring ID	Building Package	Sample ID	Field Measured N-value, N _m (blows/foot)	Equivalent N-value, N ₆₀ (blows/foot)
	Bldg. 5	S1	24	32
	Bldg. 5	S2	14	19
	Bldg. 5	S3	15	20
	Bldg. 5	S4	19	25
	Bldg. 5	S5	13	17
	Bldg. 5	S6	10	13
	Bldg. 5	S7	7	9
	Bldg. 5	S8	18	24
	Bldg. 5	S9	13	17
	Bldg. 5	S10	32	43
	Bldg. 5	S11	75	100
	Bldg. 5	S12	100/4"	100
B5	Blda, 5	S13	77	100
	Blda, 5	S14	102	100
	Blda, 5	S15	101	100
	Blda, 5	S16	101/1"	100
	Blda, 5	S17	106/1"	100
	Blda, 5	S18	72	96
	Blda, 5	S19	154/10"	100
	Blda, 5	S20	107/5"	100
	Blda, 5	S21	108/3"	100
	Bldg. 5	S22	53	100
	Bldg. 5	S23	34	45
	Blda, 5	S24	34	45
	Blda, 5	S25	30	40
	Blda, 4	S1	18	24
	Bldg. 4	S2	9	12
	Bldg. 4	S3	15	20
	Bldg. 4	S4	7	9
	Bldg. 4	S5	5	7
	Bldg. 4	S6	5	7
	Bldg. 4	S7	14	19
	Bldg. 4	S8	15	20
	Bldg. 4	S9	15	20
	Bldg. 4	S10	23	31
	Bldg. 4	S11	28	37
PG	Bldg. 4	S12	27	36
БО	Bldg. 4	S13	51	68
	Bldg. 4	S14	81	100
	Bldg. 4	S15	100/4"	100
	Bldg. 4	S16	101/4"	100
	Bldg. 4	S17	102/5"	100
	Bldg. 4	S18	100/3"	100
	Bldg. 4	S19	107/4"	100
	Bldg. 4	S20	108/4"	100
	Bldg. 4	S21	100/2"	100
	Bldg. 4	S22	100/3"	100
	Bldg. 4	S23	100/4"	100
	Bldg. 4	S24	103/5"	100
Consultants		Proi	ect 2305370	

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			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 4	S1	45	60
	Bldg. 4	S2	15	20
	Bldg. 4	S3	27	36
	Bldg. 4	S4	18	24
	Bldg. 4	S5	22	29
	Bldg. 4	S6	18	24
	Bldg. 4	S7	77	100
	Bldg. 4	S8	11	15
	Bldg. 4	S9	15	20
	Bldg. 4	S10	12	16
	Bldg. 4	S11	32	43
B7	Bldg. 4	S12	38	51
D1	Bldg. 4	S13	101/5"	60
	Bldg. 4	S14	30	40
	Bldg. 4	S15	100/3"	100
	Bldg. 4	S16	57/0"	100
	Bldg. 4	S17	193/10"	100
	Bldg. 4	S18	138	100
	Bldg. 4	S19	102/1"	100
	Bldg. 4	S20	100/5"	100
	Bldg. 4	S21	101/4"	100
	Bldg. 4	S22	57/0"	100
	Bldg. 4	S23	100/5"	100
	Bldg. 4	S24	100/3"	100
	Bldg. 4	S1	24	32
	Bldg. 4	S2	55	73
	Bldg. 4	S3	24	32
	Bldg. 4	S4	24	32
	Bldg. 4	S5	8	11
	Bldg. 4	S6	9	12
	Bldg. 4	S7	11	15
	Bldg. 4	S8	61	81
	Bldg. 4	S9	69	92
B8	Bldg. 4	S10	64	85
20	Bldg. 4	S11	105/5"	100
	Bldg. 4	S12	143	100
	Bldg. 4	S13	102/3"	100
	Bldg. 4	S14	100/4"	100
	Bldg. 4	S15	101/3"	100
	Bldg. 4	S16	100/2"	100
	Bldg. 4	S17	111/3"	100
	Bldg. 4	S18	156/11"	100
	Bldg. 4	S19	167/8"	100
	Bldg. 4	S20	65/0"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 4	S1	13	17
	Bldg. 4	S2	6	8
	Bldg. 4	S3	8	11
	Bldg. 4	S4	7	9
	Bldg. 4	S5	9	12
	Bldg. 4	S6	56	75
	Bldg. 4	S7	38	51
	Bldg. 4	S8	114/4"	100
	Bldg. 4	S9	144	100
	Bldg. 4	S10	127	100
	Bldg. 4	S11	165/9"	100
RQ	Bldg. 4	S12	139	100
5	Bldg. 4	S13	216/9"	100
	Bldg. 4	S14	103/3"	100
	Bldg. 4	S15	115/3"	100
	Bldg. 4	S16	111/4"	100
	Bldg. 4	S17	121/4"	100
	Bldg. 4	S18	120/5"	100
	Bldg. 4	S19	100/5"	100
	Bldg. 4	S20	191/11"	100
	Bldg. 4	S21	82	100
	Bldg. 4	S22	21	28
	Bldg. 4	S23	20	27
	Bldg. 4	S24	40	53
	Bldg. 4	S1	26	35
	Bldg. 4	S2	13	17
	Bldg. 4	S3	8	11
	Bldg. 4	S4	5	7
	Bldg. 4	S5	7	9
	Bldg. 4	S6	8	11
	Bldg. 4	S7	44	59
	Bldg. 4	S8	50/0"	60
B10	Bldg. 4	S9	67	89
2.0	Bldg. 4	S10	128	100
	Bldg. 4	S11	100/3"	100
	Bldg. 4	S12	110/5"	100
	Bldg. 4	S13	100/3"	100
	Bldg. 4	S14	100/4"	100
	Bldg. 4	S15	110/3"	100
	Bldg. 4	S16	125/4"	100
	Bldg. 4	S17	111/3"	100
	Bldg. 4	S18	102/1"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 4	S1	14	19
	Bldg. 4	S2	72	96
	Bldg. 4	S3	8	11
	Bldg. 4	S4	5	7
	Bldg. 4	S5	6	8
	Bldg. 4	S6	12	16
	Bldg. 4	S7	37	49
	Bldg. 4	S8	42	56
	Bldg. 4	S9	61	81
B11	Bldg. 4	S10	100/3"	100
	Bldg. 4	S11	102/5"	100
	Bldg. 4	S12	107/5"	100
	Bldg. 4	S13	102/4"	100
	Bldg. 4	S14	145/10"	100
	Bldg. 4	S15	62/0"	100
	Bldg. 4	S16	100/5"	100
	Bldg. 4	S17	100/3"	100
	Bldg. 4	S18	122	100
	Bldg. 4	S19	100/1"	100
	Bldg. 3A/3B	S1	10	13
	Bldg. 3A/3B	S2	50/3"	60
	Bldg. 3A/3B	S3	40	53
	Bldg. 3A/3B	S4	14	19
	Bldg. 3A/3B	S5	4	5
	Bldg. 3A/3B	S6	8	11
	Bldg. 3A/3B	S7	20	27
	Bldg. 3A/3B	S8	43	57
B12	Bldg. 3A/3B	S9	83	100
DIZ	Bldg. 3A/3B	S10	86	100
	Bldg. 3A/3B	S11	100/2"	100
	Bldg. 3A/3B	S12	100	100
	Bldg. 3A/3B	S13	171/11"	100
	Bldg. 3A/3B	S14	100/5"	100
	Bldg. 3A/3B	S15	120	100
	Bldg. 3A/3B	S16	101/5"	100
	Bldg. 3A/3B	S17	103/3"	100
	Bldg. 3A/3B	S18	58	77

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	15	20
	Bldg. 3A/3B	S2	7	9
	Bldg. 3A/3B	S3	12	16
	Bldg. 3A/3B	S4	4	5
	Bldg. 3A/3B	S5	5	7
	Bldg. 3A/3B	S6	10	13
	Bldg. 3A/3B	S7	16	21
	Bldg. 3A/3B	S8	60	80
	Bldg. 3A/3B	S9	36	48
	Bldg. 3A/3B	S10	37	49
	Bldg. 3A/3B	S11	116	100
B13	Bldg. 3A/3B	S12	47	63
	Bldg. 3A/3B	S13	196	100
	Bldg. 3A/3B	S14	105	100
	Bldg. 3A/3B	S15	1/0/11"	100
	Bldg. 3A/3B	S16	101/4"	100
	Bldg. 3A/3B	<u>S17</u>	108/4"	100
	Bldg. 3A/3B	S18	104/3"	100
	Bldg. 3A/3B	S19	107/5"	100
	Bldg. 3A/3B	S20	100/4"	100
	Bldg. 3A/3B	S21	36	48
	Blag. 3A/3B	522	27	36
	Blag. 3A/3B	523	30	40
	Bldg. 3A/3B	524	31	41
	Diug. JA/JD	<u>ଁ</u>	32	40
	Bldg 24/3D	52	7	23
	Bldg 3A/3B	33 84	12	9
	Bldg 3A/3B	54 85	15	5
	Bldg 34/3B		15	20
	Bldg 34/3B	<u> </u>	24	32
	Bldg. 3A/3B	58	35	<u> </u>
	Bldg. 3A/3B	S9	85	100
B14	Bldg 3A/3B	S10	20/1"	100
	Bldg 3A/3B	S11	100/4"	100
	Bldg 3A/3B	S12	50/2"	100
	Bldg, 3A/3B	S13	100/5"	100
	Bldg. 3A/3B	S14	50/4"	100
	Bldg. 3A/3B	S15	150/9"	100
	Bldg, 3A/3B	S16	100/5"	100
	Bldg. 3A/3B	S17	100/3"	100
	Bldg. 3A/3B	S18	169/9"	100
	Bldg. 3A/3B	S19	100/2"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	19	25
	Bldg. 3A/3B	S2	9	12
	Bldg. 3A/3B	S3	5	7
	Bldg. 3A/3B	S4	16	21
	Bldg. 3A/3B	S5	13	17
	Bldg. 3A/3B	S6	10	13
	Bldg. 3A/3B	S7	32	43
	Bldg. 3A/3B	S8	27	36
	Bldg. 3A/3B	S9	36	48
B15	Bldg. 3A/3B	S10	129	100
	Bldg. 3A/3B	S11	80	100
	Bldg. 3A/3B	S12	120/5"	100
	Bldg. 3A/3B	S13	100/1"	100
	Bldg. 3A/3B	S14	100/2"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	49	65
	Bldg. 3A/3B	S17	100/1"	100
	Bldg. 3A/3B	S18	103/0"	100
	Bldg. 3A/3B	S19	36/0"	100
	Bldg. 3A/3B	S1	44/3"	60
	Bldg. 3A/3B	S2	23	31
	Bldg. 3A/3B	S3	8	11
	Bldg. 3A/3B	S4	11	15
	Bldg. 3A/3B	S5	13	17
	Bldg. 3A/3B	S6	69	92
	Bldg. 3A/3B	S7	54	72
	Bldg. 3A/3B	S8	28	37
	Bldg. 3A/3B	S9	32	43
	Bldg. 3A/3B	S10	134	100
	Bldg. 3A/3B	S11	150/11"	100
B16	Bldg. 3A/3B	S12	106/5"	100
-	Bldg. 3A/3B	S13	134	100
	Bldg. 3A/3B	S14	111	100
	Bldg. 3A/3B	S15	117	100
	Bldg. 3A/3B	S16	104/5"	100
	Bldg. 3A/3B	S17	79	100
	Bldg. 3A/3B	S18	113	100
	Bldg. 3A/3B	S19	87	100
	Bldg. 3A/3B	S20	62	83
	Bldg. 3A/3B	S21	48	64
	Bldg. 3A/3B	S22	35	47
	Bldg. 3A/3B	S23	56	75
	Bldg. 3A/3B	S24	40	53

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	16/6"	60
	Bldg. 3A/3B	S2	16	21
	Bldg. 3A/3B	S3	12	16
	Bldg. 3A/3B	S4	28	37
	Bldg. 3A/3B	S5	11	15
	Bldg. 3A/3B	S6	20	27
	Bldg. 3A/3B	S7	72	96
	Bldg. 3A/3B	S8	32	43
B17	Bldg. 3A/3B	S9	30	40
	Bldg. 3A/3B	S10	50/3"	100
	Bldg. 3A/3B	S11	100/5"	100
	Bldg. 3A/3B	S13	50/3"	100
	Bldg. 3A/3B	S14	47/4"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	100/3"	100
	Bldg. 3A/3B	S17	100/5"	100
	Bldg. 3A/3B	S18	100/4"	100
	Bldg. 3A/3B	S1	5	7
	Bldg. 3A/3B	S2	12	16
	Bldg. 3A/3B	S3	15	20
	Bldg. 3A/3B	S4	25	33
	Bldg. 3A/3B	S5	20	27
	Bldg. 3A/3B	S6	20	27
	Bldg. 3A/3B	S7	100/4"	60
	Bldg. 3A/3B	S8	13	17
	Bldg. 3A/3B	S9	94	100
	Bldg. 3A/3B	S10	100/1"	100
	Bldg. 3A/3B	S11	65	87
B18	Bldg. 3A/3B	S12	100/2"	100
Die	Bldg. 3A/3B	S13	100	100
	Bldg. 3A/3B	S14	177/8"	100
	Bldg. 3A/3B	S15	111/4"	100
	Bldg. 3A/3B	S16	100/5"	100
	Bldg. 3A/3B	S17	100	100
	Bldg. 3A/3B	S18	143	100
	Bldg. 3A/3B	S19	69	92
	Bldg. 3A/3B	S20	100/3"	100
	Bldg. 3A/3B	S21	114/3"	100
	Bldg. 3A/3B	S22	101/2"	100
	Bldg. 3A/3B	S23	100/2"	100
	Bldg. 3A/3B	S24	101/2"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	50/3"	60
	Bldg. 3A/3B	S2	100/5"	60
	Bldg. 3A/3B	S3	10	13
	Bldg. 3A/3B	S4	16	21
	Bldg. 3A/3B	S5	20	27
	Bldg. 3A/3B	S6	28	37
	Bldg. 3A/3B	S7	67	89
	Bldg. 3A/3B	S8	47	63
B19	Bldg. 3A/3B	S9	100/5"	100
	Bldg. 3A/3B	S10	50/1"	100
	Bldg. 3A/3B	S11	50/3"	100
	Bldg. 3A/3B	S12	100/4"	100
	Bldg. 3A/3B	S13	100/4"	100
	Bldg. 3A/3B	S14	100/5"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	50/2"	100
	Bldg. 3A/3B	S17	100/5"	100
	Bldg. 3A/3B	S1	35	47
	Bldg. 3A/3B	S2	57	76
	Bldg. 3A/3B	S3	32	43
	Bldg. 3A/3B	S4	18	24
	Bldg. 3A/3B	S5	42	56
	Bldg. 3A/3B	S6	29	39
	Bldg. 3A/3B	S7	37	49
	Bldg. 3A/3B	S8	71	95
B30	Bldg. 3A/3B	S9	42	56
B20	Bldg. 3A/3B	S10	51/4"	100
	Bldg. 3A/3B	S11	100/4"	100
	Bldg. 3A/3B	S12	100/5"	100
	Bldg. 3A/3B	S13	100/3"	100
	Bldg. 3A/3B	S14	100/4"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	100/4"	100
	Bldg. 3A/3B	S17	50/3"	100
	Bldg. 3A/3B	S18	100/1"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID.	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	25	33
	Bldg. 3A/3B	S2	32	43
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	23	31
	Bldg. 3A/3B	S5	54	72
	Bldg. 3A/3B	S6	100/5"	60
	Bldg. 3A/3B	S7	13	17
	Bldg. 3A/3B	S8	2	3
B21	Bldg. 3A/3B	S9	18	24
DZT	Bldg. 3A/3B	S10	10	13
	Bldg. 3A/3B	S11	32	43
	Bldg. 3A/3B	S12	45	60
	Bldg. 3A/3B	S13	52	69
	Bldg. 3A/3B	S14	192/8"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	134/11"	100
	Bldg. 3A/3B	S17	100/3"	100
	Bldg. 3A/3B	S18	165/11"	100
	Bldg. 3A/3B	S1	21	28
	Bldg. 3A/3B	S2	25	33
	Bldg. 3A/3B	S3	25	33
	Bldg. 3A/3B	S4	6	8
	Bldg. 3A/3B	S5	22	29
	Bldg. 3A/3B	S6	5	7
	Bldg. 3A/3B	S7	7	9
	Bldg. 3A/3B	S8	100/5"	100
	Bldg. 3A/3B	S9	89	100
B22	Bldg. 3A/3B	S10	156/11"	100
	Bldg. 3A/3B	S11	164/10"	100
	Bldg. 3A/3B	S12	113/5"	100
	Bldg. 3A/3B	S13	102/4"	100
	Bldg. 3A/3B	S14	101/4"	100
	Bldg. 3A/3B	S15	100/3"	100
	Bldg. 3A/3B	S16	100/5"	100
	Bldg. 3A/3B	S17	100/1"	100
	Bldg. 3A/3B	S18	102/3"	100
	Bldg. 3A/3B	S19	100/1"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	63	84
	Bldg. 3A/3B	S2	70	93
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	7	9
	Bldg. 3A/3B	S5	14	19
	Bldg. 3A/3B	S6	8	11
	Bldg. 3A/3B	\$7	13	17
	Bldg. 3A/3B	S8	5	/
	Bldg. 3A/3B	S9	27	36
B23	Bldg. 3A/3B	S10	14	19
	Bldg. 3A/3B	511	09	92
	Bldg 24/3D	S12 S12	102/1	100
	Bldg 3A/3B	S13 S14	127	100
	Bldg 34/3B	S14 S15	102/3"	100
	Bldg 3A/3B	S16	102/3	100
	Bldg. 3A/3B	S17	79	100
	Bldg 3A/3B	S18	145	100
	Bldg, 3A/3B	S19	50	67
	Bldg. 3A/3B	S20	105/1"	100
	N/A	S1	9	12
	N/A	S2	30	40
	N/A	S3	39	52
	N/A	S4	17	23
	N/A	S5	34	45
	N/A	S6	16	21
	N/A	S7	9	12
	N/A	S8	17	23
	N/A	S9	42	56
B29	N/A	S10	104/3"	100
	N/A	S11	101/3"	100
	N/A	S12	148	100
	N/A	S13	102/2"	100
	N/A	S14	199/10"	100
	N/A	S15	110/5"	100
	N/A	S16	102/2"	100
	N/A	S17	105/1"	100
	N/A	S18	100/4"	100
	N/A	S19	45/0"	100

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N _m	N-value, N ₆₀
ID	Package	ID	(blows/foot)	(blows/foot)
	N/A	S1	11	15
	N/A	S2	13	17
	N/A	S3	11	15
	N/A	S4	6	8
	N/A	S5	10	13
	N/A	S6	14	19
B53	N/A	S7	25	33
	N/A	S8	93	100
	N/A	S9	73	97
	N/A	S10	48	64
	N/A	S11	105	100
	N/A	S12	100/4"	100
	N/A	S13	53/0"	100
	Bldg. 3A/3B	S1	13	17
	Bldg. 3A/3B	S2	24	32
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	13	17
	Bldg. 3A/3B	S5	5	7
	Bldg. 3A/3B	S6	3	4
	Bldg. 3A/3B	S7	2	3
	Bldg. 3A/3B	S8	2	3
	Bldg. 3A/3B	S9	51	68
	Bldg. 3A/3B	S10	29	39
	Bldg. 3A/3B	S11	65	87
B101	Bldg. 3A/3B	S12	123	100
DIOI	Bldg. 3A/3B	S13	100/5"	100
	Bldg. 3A/3B	S14	122	100
	Bldg. 3A/3B	S15	101/4"	100
	Bldg. 3A/3B	S16	100/4"	100
	Bldg. 3A/3B	S17	100/5"	100
	Bldg. 3A/3B	S18	50/0"	100
	Bldg. 3A/3B	S19	51/4"	100
	Bldg. 3A/3B	S20	69	92
	Bldg. 3A/3B	S21	69	92
	Bldg. 3A/3B	S22	41	55
	Bldg. 3A/3B	S23	22	29
	Bldg. 3A/3B	S24	27	36

Boring ID	Building Package	Sample ID	Field Measured N-value, N _m (blows/foot)	Equivalent N-value, N ₆₀ (blows/foot)	
	Bldg. 3A/3B	S1	12	16	
	Bldg. 3A/3B	S2	10	13	
	Bldg. 3A/3B	S3	7	9	
	Bldg. 3A/3B	S4	7	9	
	Bldg. 3A/3B	S5	84	100	
	Bldg. 3A/3B	S6	19	25	
	Bldg. 3A/3B	S7	35	47	
	Bldg. 3A/3B	S8	36	48	
	Bldg. 3A/3B	S9	109	100	
B102	Bldg. 3A/3B	S10	93	100	
D102	Bldg. 3A/3B	S11	100/5"	100	
	Bldg. 3A/3B	S12	50/0"	100	
	Bldg. 3A/3B	S13	184/9"	100	
	Bldg. 3A/3B	S14	173/11"	100	
	Bldg. 3A/3B	S15	100/3"	100	
	Bldg. 3A/3B	S16	100/2"	100	
	Bldg. 3A/3B	S17	101/5"	100	
	Bldg. 3A/3B	S18	100/3"	100	
	Bldg. 3A/3B	S19	100/3"	100	
	Bldg. 3A/3B	S20	165	100	

Table 3. Groundwater Level Data in Observation WellsGeotechnical ReportAstoria Cove DevelopmentAstoria, Queens, New York

	Location1 A			Approximate	Depth	Top of	Screen	Interval	Depth	Date of	Wat	ter Leve	I Reading #	1	Wa	ter Leve	el Reading #	‡ 2	Wa	ter Leve	I Reading #	‡ 3
Well ID	Building ID	Northing	Easting	Ground Surface Elevation ²	to Top of PVC	PVC Elev.	Top Depth	Bottom Depth	of Boring	Well Development	Date	Depth	Elevation	Notes	Date	Depth	Elevation	Notes	Date	Depth	Elevation	Notes
		_		(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	-		(feet)	(feet)			(feet)	(feet)			(feet)	(feet)	
B5(OW)	Bldg. 5	221,770	1,003,343	25.0	0.00	25.0	15.0	25.0	102.0	Note 3	03/10/20	18.8	6.2	-	-	-	-	-	11/14/23	Dry	Dry	5
B8(OW)	Bldg. 4	221,825	1,003,207	23.2	0.30	22.9	20.0	30.0	85.0	Note 3	03/05/20	18.3	4.6	-	03/09/20	18.1	4.8	-	11/14/23	17.0	Uncertain	5
B16(OW)	Bldg. 3A/3B	221,903	1,003,499	21.9	0.61	21.3	3.0	13.0	14.0	Note 3	03/05/20	17.3	4.0	-	03/10/20	17.3	4.0	-	-	-	-	6
B29(OW)	N/A	222,066	1,003,320	21.9	0.35	21.6	3.0	13.0	21.0	Note 3	03/18/20	16.9	5.0	-	-	-	-	-	-	-	-	6
B48(OW)	N/A	222,448	1,003,170	12.9	0.33	12.5	4.0	13.7	14.0	Note 3	05/21/20	11.1	1.4	-	-	-	-	-	11/14/23	10.8	2.1	-
B53(OW)	N/A	221,759	1,003,421	29.4	0.33	29.1	4.0	13.7	28.1	Note 3	05/15/20	23.5	5.6	-	-	-	-	-	-	-	-	6

Notes:

1. Coordinates for boring locations are taken from taped off distances frm survey site features and are referenced to NAD83.

2. Ground surface elevations are estimated from topographic data and are referenced to NAVD88.

3. Well development not required in observation wells.

4. Water level measured in casing at the completion of drilling.

5. Surface casing damaged/missing with PVC riser exposed; debris had fallen into well. Measurements made from exposed PVC casing are likely erroneous.

6. Well not located.

Table 4. Groundwater Level Data in Monitoring WellsGeotechnical ReportAstoria Cove DevelopmentAstoria, Queens, New York

Monitoring Wells	Building ID	Date	Approx. Screen Interval Depth, (feet)	Top of Casing Elevation, (feet)	Depth to Water, (feet)	Depth to Bottom, (feet)	Groundwater Elevation, (feet)
MW-1	Bldg. 3A/3B	2/1/2022	15 to 25	19.1	17.2	23.1	2.0
MW-2	Bldg. 3A/3B	2/2/2022	10 to 20	12.4	11.3	19.4	1.1
MW-3	Bldg. 3A/3B	2/2/2022	10 to 20	14.1	12.8	19.6	1.4
MW-4	Bldg. 3A/3B	2/1/2022	20 to 30	19.6	17.6	28.6	2.0
MW-5	Bldg. 4	2/2/2022	15 to 25	21.9	18.8	24.4	3.1
MW-6	Bldg. 4	2/2/2022	15 to 25	23.4	18.9	29.7	4.5
MW-11	Bldg. 5	2/1/2022	15 to 25	25.5	21.6	28.5	3.9
MW-12	Bldg. 5	2/1/2022	20 to 30	24.6	21.7	30.1	2.9

Notes:

1. Ground surface elevations are estimated from topographic data and are referenced to NAVD88.

Table 5. Recommended Geotechnical Design ParametersGeotechnical ReportAstoria Cove DevelopmentAstoria, Queens, New York

	Total U	nit Weight	Drained Parameters					
Soil Material ⁽¹⁾	Above Water Table		Drained Friction Angle	Effective Cohesion	Earth Pressure Coefficients ⁽²⁾			
	γ _t (pcf)	γ _{sat} (pcf)	φ' (degrees)	C' (ksf)	K _o	K _a	K_{p}	
Stratum I -Fill (Class 7)	120	122	34	0	0.44	0.28	3.5	
Stratum IIa - Loose Sand (Class 6)	115	118	30	0	0.50	0.33	3.0	
Stratum IIb - Silt/Clay (Class 4b/5b)	115	118	30	1	0.50	0.33	3.0	
Stratum IIc - Sand (Class 3b)	120	122	34	0	0.44	0.28	3.5	
Stratum III - Till (Class 3a)	130	135	40	0	0.36	0.22	4.6	
Stratum IV - Decomposed/Weathered Rock (Class 1d)	130	135	40	0	0.36	0.22	4.6	
Stratum V - Bedrock (Class 1b/1c)	150	150						
Ordinary Fill (92% Compaction) ⁽³⁾	120	125	32	0	0.47	0.31	3.3	
Structural Fill (95% Compaction) ⁽⁴⁾	120	125	35	0	0.43	0.27	3.7	

Footnotes:

- 1. The values of soil/rock properties in this table are based on site-specific field test results (where available), empirical correlations using the results of standard penetration tests and laboratory index tests, and engineering judgement.
- 2. Parameters to be used for footings and structures:
 - K_{o} = Coefficient of Earth Pressure at Rest (Rankine Method)
 - K_a = Active Earth Pressure Coefficient (Rankine Method)
 - K_p = Passive Earth Pressure Coefficient (Rankine)
- 3. For material compacted to ~92% of Modified Proctor maximum dry density in accordance with ASTM D1557.
- 4. For material compacted to ~95% of Modified Proctor maximum dry density in accordance with ASTM D1557.

Geotechnical Report Astoria Cove Development Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York December 2023

Figures



DIBARTOLO, SEAN B:\Working\CAD3 CONSTR AND DEVELOP\2305370 SLIM Astoria 2468 Ph1 Geotech\00_CAD\Figures\GTR\2305370-02 GTR-SLM.dwg - 12/8/2023





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(CLASS 7	LOOSE SAND (CLASS 6)		
9	USCS SYMBOLS		
	FILL CONCRETE ASPHALT WEATHEREI DECOMPOS BOULDER/C BEDROCK	SP SW SM D/ ED ROCK OBBLE	
В	ORING NUMBER		
E C E	LEVATION OF GROUND S FFSET OF BORING FROM XISTING GROUND SURFA	SURFACE AND M PROFILE LINE ACE	
G		EASURED AT COMPLETION OF DRILLIN	١G
U	NCORRECTED N-VALUE	FROM STANDARD PENETRATION	
T 0% R	EST, BLOWS PER FOOT OCK CORE RECOVERY/F	ROCK QUALITY DESIGNATION (RQD)	
	GEI Consultants	SUBSURFACE PROFILE D-D	
	Project 2305370	December 2023 Fig	. 6















limensions and be responsible for field fit and quantity of work No allowances shall be made on behalf of the contractor for any error or neglect on his part. In a conflict between sealed drawings nd electronic files the sealed drawings will gov

SCHEMATIC DESIGN











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Geotechnical Report Astoria Cove Development Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York December 2023

Appendix A

Exploration Logs





FINE-GRAINED SOILS

VISUAL-MANUAL DESCRIPTIONS

GROUP PATTERN	I & SYMBOL			GROUP NAME
	<30% plus No. 200	→ <15% plus No. 200		
/		🛰 15-25% plus No. 200 ——	──► % Sand <u>></u> % Gravel ───	LEAN CLAY WITH SAND
			% Sand <% Gravel	LEAN CLAY WITH GRAVEL
		% Sand >% of Gravel	──► <15 % Gravel	SANDY LEAN CLAY
	>30% plus No. 200		>15% Gravel	SANDY LEAN CLAY WITH GRAVEL
		➤ % Sand <% of Gravel ——		GRAVELLY LEAN CLAY
			<u>≥</u> 15% Sand	GRAVELLY LEAN CLAY WITH SAND
	<30% plus No. 200	→ <15% plus No. 200		► SILT
/		🛰 15-25% plus No. 200	→ % Sand >% Gravel	→ SILT WITH SAND
Шм			% Sand <% Gravel	SILT WITH GRAVEL
		✓ % Sand ≥% of Gravel <		SANDY SILT
×	>30% plus No. 200			SANDY SILT WITH GRAVEL
		➤ % Sand <% of Gravel ───	→ <15 % Sand →	GRAVELLY SILT
			<u>≥</u> 15% Sand	GRAVELLY SILT WITH SAND
<u>~</u> 50 % T INES	<30% plus No. 200	<15% plus No. 200		FAT CLAY
/		🛰 15-25% plus No. 200 👥	── > % Sand <u>></u> % Gravel ───	FAT CLAY WITH SAND
Сн			% Sand <% Gravel	FAT CLAY WITH GRAVEL
		✓ % Sand ≥% of Gravel		SANDY FAT CLAY
×	>30% plus No. 200			SANDY FAT CLAY WITH GRAVEL
		∽ % Sand <% of Gravel ───		GRAVELLY FAT CLAY
			<u>≥</u> 15% Sand	GRAVELLY FAT CLAY WITH SAND
	<30% plus No. 200 ———	→ <15% plus No. 200		ELASTIC SILT
/		🛰 15-25% plus No. 200 👡	──► % Sand <u>></u> % Gravel ───	ELASTIC SILT WITH SAND
Шмн			% Sand <% Gravel	ELASTIC SILT WITH GRAVEL
Ш		✓ % Sand ≥% of Gravel		SANDY ELASTIC SILT
×	>30% plus No. 200		>15% Gravel	SANDY ELASTIC CLAY WITH GRAVEL
		➤ % Sand <% of Gravel	→ <15 % Sand	GRAVELLY ELASTIC SILT
			<u>≥</u> 15% Sand	GRAVELLY ELASTIC SILT WITH SAND
	<30% plus No. 200	🗕 <15% plus No. 200 ———		
		15-25% plus No. 200	───── % Sand ≥% Gravel ────	ORGANIC SOIL WITH SAND
⊟ Ш ог∕он <			% Sand <% Gravel	ORGANIC SOIL WITH GRAVEL
		✓ % Sand ≥% of Gravel	► <15 % Gravel	SANDY ORGANIC SOIL
×	>30% plus No. 200 <		>15% Gravel	SANDY ORGANIC SOIL WITH GRAVEL
		➤ % Sand <% of Gravel	← <15 % Sand ←	GRAVELLY ORGANIC SOIL
			>15% Sand	GRAVELLY ORGANIC SOIL WITH SAND

ID OF INORGANIC FINE SOILS FROM MANUAL TESTS

Symbol	Name	Dry Strength	Dilatancy	Toughness*	
ML	Silt	None to low	Slow to rapid	Low or thread cannot be formed	
CL Lean Clay		Medium to high	None to slow	Medium	
MH	Elastic Silt	Low to medium	None to slow	Low to medium	
СН	Fat Clay	High to very high	None	High	

- 1. GROUP NAME and (SYMBOL)
- Describe fines, sand, and gravel components, in order of predominance. Include plasticity of fines. Include percentages of sand and gravel. 2.
- 3. Color
- 4 Moisture dry, moist, wet
- Sheen, odor, roots, ash, brick, cementation, torvane and penetrometer results, etc. 5.
- 6. "Fill," local name or geologic name, if known

PEAT 24 2

Peat refers to a sample composed primarily of vegetable matter in varying stages of decomposition. The description should begin: PEAT (PT) and need not include percentages of sand, gravel or fines.

CRITERIA FOR DESCRIBING PLASTICITY

Description	Criteria
Nonplastic ML	A 1/8-in. (3 -mm) thread cannot be rolled at any water content
Low Plasticity ML, MH	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit *
Medium Plasticity MH, CL	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit
High Plasticity CH	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit

* Toughness refers to the strength of the thread near plastic limit. The lump refers to a lump of soil drier than the plastic, similar to dry strength.

B		IG IN IING	IFC (fft)·	RN	1ATION	1			EASTING (ff): 1 003 2	63		BORING	
G	ROUI		URF	AC	E EL. (f	t): 48.8	3		DATE START/END:	5/1/2020 - 5/1/2020			
v	ERT./	HOR	IZ. I	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Craig Geotechnical Drilling B1			
T	OTAL	DEP	тн	(ft):	102	.0			DRILLER NAME: Kei	th Par	ent		
	LOGGED BY: <u>G. Holmes</u> RIG TYPE: <u>CME 75</u>								RIG TYPE: CME 75			PAGE 1 of 5	
D	RILL	ING	INF	OF	RMATIC	N							
н	AMM	ER T	YPE		Autom	atic			CASING I.D./O.D.: 4	nch/ 4	.5 inch CORE BAR	REL TYPE: NA	
A	UGEF	RI.D./	/O.E).: uo			Wash		DRILL ROD O.D.:N	1	CORE BAR	REL I.D./O.D.: <u>NA / NA</u>	
w	ATE	R LEV	/EL	DE	:PTHS (1	<u>uu Rotary</u> ft):	Wash						
A	ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% WOR = Weight of Rods WOH = Weight of Hammer								S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample % SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. liameter	
					Sa	ample Inf	ormation			bc			
E	lev.	Dep	th	_			Pen /	Blows	Drilling Remarks/	ic Lo	Soil and	Pack Description	
((ft)	(ft))	Sa	ample No	Depth (ft)	Rec.	per 6 in.	Field Test Data	raph		Nock Description	
						()	(in)	or RQD		U	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 48.8	
			N	Λ	S1	0 to	24/7	23-16-	PID = 0.0ppm @S1		S1 (0-2"): Asphalt. S1 (2"-7"): SILTY SAND WIT	H GRAVEL (SM); ~60% fine to	
	_	-		XI		2		σ. 17-8			medium sand, ~20% nonplas	tic fines, ~20% angular gravel up to	
	-			$^{\prime}$							STRATUM IIA - LOOSE S	AND (CLASS 6) at ~2 ft / El. 46.8	
				Λ	S2	2 to	24/6	5-6-6-5	PID = 0.0ppm @S2		S2: WIDELY GRADED SANE) (SW); ~90% sand, ~10% orange-brown to brown: dry	
	_	_		XI		4					cubungular graver up to 1.0,		
				$^{\prime}$									
				$\langle \rangle$	S3	4 to	24/10	5-4-3-2	PID = 0.0ppm @S3		S3: WIDELY GRADED SAND	0 (SW): Similar to S2.	
	-		5	5	5 X 6 6								
33	_			\mathbb{N}									
2/22/2		_		Ţ	S4	6	24/12	5-3-4-8	PID = 0.0ppm @S4		S4: WIDELY GRADED SAND	0 (SW): Similar to S2.	
1 1	_	_		χI		8							
13.GI	_			\mathbb{N}						STRATUM IIC - SAND (CLASS 3B) at ~8 ft / El. 40.8			
Е 20		-		Ì	S5	8	24/0	5-7-6-6	PID = 0.0ppm @S5		S5: No Recovery.		
PLAT	40 —	_		Υ		to 10							
TEM				\mathbb{N}									
ATA	_	- 1	10		56	10	24/10	5-1-7-13	PID = 0.0ppm @S6		S6: WIDELY GRADED SAND	0 (SW): Similar to S2.	
E E	_	_		VI	00	to 12	24/10	0-4-7-10					
PJ 0				\mathbb{N}									
OVE.0	-		ľ	+					Drive casing to 14 feet below	/			
ACC	-								grade. Roller bit to 15 feet.				
TORI		-											
0-AS		_											
0537	_												
G 23		<u> </u>	15		S7	15 to	24/0	10-6-6-5			S7: No Recovery. Rock stuck	(in shoe.	
СГО	_	_		χl		17							
APHI	_			\mathbb{N}									
T-GR		-	f	╡									
EAS'	_	_							Roller hit to 20 feet				
RTH-	30												
0N-9	30	-											
STD	_												
N N	OTES	:								PROJ	ECT NAME: Astoria Cove		
WOB										CITY	STATE: Astoria, Queens, New		
GEI										GEI F	ROJECT NUMBER: 2305370		
NORT GROU	HINC IND :	g (ft) Suri	: _2 FAC	221,653 E EL. (1	i t): _ 48.8	}		EASTING (ft): _1,003,2 DATE START/END:	63 5/1/202	0 - 5/1/2020	BORING B1		
---------------	---------------	----------------	-------------	---------------------	----------------------------	-----------------------------------	--------------------	---	-------------------------	---	--		
VERT.	/HO	riz.	DA		NAVD 88	3/NAD83 N	Y Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 5		
Elev. (ft)	De (1	epth ft)	Si	Sa ample No.	ample Inf Depth (ft)	ormation Pen./ Rec. (in)	Blows per 6 in.	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description		
	_		M	S8	20 to 22	24/17	5-5-8-7	PID = 0.0ppm @S8		S8: NARROWLY GRADED S sand, ~5% nonplastic fines; t	AND (SP); ~95% fine to medium an-brown; dry.		
-								Advance casing to 25 feet below grade. Roller bit to 25 feet.	5				
-		25	X	S9	25 to 27	24/19	11-11- 14-12	PID = 0.0ppm @S9		S9: NARROWLY GRADED S	AND (SP); Similar to S8.		
-								Roller bit to 30 feet.		STRATUM III - TILL (C	:LASS 3A) at ~28.5 ft / El. 20.3		
20		30	X	S10	30 to 32	24/13	9-15-18- 18	PID = 0.0ppm @S10		S10: SILTY SAND (SM); ~60 tan; dry.	% fine sand, ~40% nonplastic fines;		
-	-							Roller bit to 35 feet.					
-		35	X	S11	35 to 37	24/19	22-22- 26-20	PID = 0.0ppm @S11		S11: WIDELY GRADED SAN fines, ~5% subrounded grave	ID (SW); ~90% sand, ~5% nonplastic al up to 1.5"; brown; wet.		
- 10 —								Roller bit to 40 feet. Intermittent rig chatter from 35 to 40 feet.					
-		40	X	S12	40 to 41.3	15/11	29-58- 102/3"	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN	ID (SW); Similar to S11. Trace mica.		
-	-							Roller bit to 45 feet. Rig chatter from 41 to 42 feet.					
-		45	X	S13	45 to 47	24/8	12-11- 14-14	PID = 0.0ppm @S13		S13: WIDELY GRADED SAN	ID (SW); Similar to S11.		
NOTES	S:								PROJ CITY/: GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants		







	BORIN	IG I	NFC) RN	ATION	1			EASTING (fft) 1 003 29	30		BORING		
	GROU	ND S	SUR	· _2 FAC	E EL. (f	it): 48.4	<u> </u>		DATE START/END: 4	/ <u>28</u> /20	020 - 4/28/2020			
	VERT.	HOF	riz.	DAT	rums:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B2		
		DE	PTH	l (ft)	:	.0			DRILLER NAME: Keit	h Par	ent			
	LOGGI	EDE	SY:	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 5		
,	DRILL HAMM	.ING ER	i IN Typ	Fof E:	Autom	<u>DN</u> natic			CASING I.D./O.D.: _4 i	nch/ 4	.5 inch CORE BARF	REL TYPE: NA		
		R I.D	./0.	D.:	<u>NA / I</u>	NA NA Deterry	Weeh		DRILL ROD O.D.:NM		CORE BARF	REL I.D./O.D.: NA / NA		
	WATE	RLE			D. <u>IVI</u> PTHS (<u>uu Rotary</u> ft):	Wash							
	ABBR	evia	TIO	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight	on Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample % SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Di	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter		
					S	ample Inf	ormation			Бc				
	Elev. (ft)	De (f	pth t)	Si	ample No	Depth	Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	raphic Lo	Soil and F	Rock Description		
					110.	(14)	(in)	or RQD		G	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 48.4		
ſ	_										S1 (0-1'): Concrete.			
	-			X	S1	1 to 3	24/15	13-11-8- 10	PID = 0.0ppm @S1		S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% subang Brick, asphalt; trace mica. Asp	fine to medium sand, ~20% Jular gravel up to 1.5"; brown; dry. halt/concrete fragments.		
		$- \begin{bmatrix} & & & & & & & \\ & & & & & & \\ & & & &$						3-6-3-4	PID = 0.0ppm @S2	D = 0.0ppm @S2 S2: SILTY SAND (SM); Similar				
/22/23	-		5	$\left \right\rangle$	S3	5 to 7	24/7	3-3-4-12	PID = 0.0ppm @S3		S3: SILTY SAND (SM); Simila	r to S1.		
3DT 12		_		$\left(\right)$	S4	7	24/8	19656	PID = 0.0ppm @S4		STRATUM IIC - SAND S4: CLAYEY SAND (SC); ~70	(CLASS 3B) at ~7 ft / El. 41.4 % sand, ~20% low plasticity fines,		
ATE 2013.0	40 —	_		X	04	to 9	24/0	10-0-0-0			~10% subangular gravel up to	1.5"; brown; dry.		
I DATA TEMPI	-	_	10	X	S5	9 to 11	24/7	17-9-11- 34	PID = 0.0ppm @S5		S5: WIDELY GRADED SAND ~20% subangular gravel up to dry; some mica. S5: WIDELY GRADED SAND ~20% subangular gravel up to	WITH GRAVEL (SW); ~75% sand, 1.25", ~5% nonplastic fines; brown; WITH GRAVEL (SW); ~75% sand, 1.25", ~5% populatic fines; brown;		
J GE	_	-		$\left(\right)$	<u>S6</u>	11	24/6	19-21-	PID = 0.0ppm @S6		dry; some mica.	(1.25, ~5%) nonplastic lines, brown,		
A COVE.GF	-	_		\mathbb{N}		to 13	2 0	24-12	Drive casing to 14 feet below grade. Rollerbit to 15 feet.		S6: WIDELY GRADED GRAV subangular gravel up to 1.5", ~	EL WITH SAND (GW); ~75% -25% sad; brown to black; dry.		
2305370-ASTORI		_	15			15								
GRAPHIC LOG	-	-		X	S7	15 to 17	24/9	6-5-8-7			ארגעטעעבי GRADED S/ nonplastic fines; tan; dry.	עזער (סד), ~90% tine sand, ~5%		
TD 6-NORTH-EAS I-	- 30—	-							Roller bit to 20 feet.					
GEI WOBUKN S	NOTES	5:		. I				·		PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ^N ROJECT NUMBER: 2305370	York GEL Consultants		

NORT	HING	(ft):	2	21,695				EASTING (ft): 1,003,2	80		BORING
GROU VERT.	nd s /Hor	URI RIZ.	FAC	E EL. (1 'UMS:	ft): <u>48.4</u> NAVD 88	3/NAD83 N	IY Zone 31(DATE START/END: 04 DRILLING COMPANY:	4/28/20 Crai	20 - 4/28/2020 g Geotechnical Drilling	B2
							Lone on	<u> </u>		<u>g ecoloon noar 2000 g</u>	PAGE 2 of 5
				S	ample Inf	ormation	I	-	bo-		
Elev. (ft)	Dep (ft	oth t)	Sa	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and	Rock Description
-	-			S8	20 to 22	24/9	17-18- 13-22	PID = 0.0ppm @S8		S8: NARROWLY GRADED S medium sand.	SAND (SP); Similar to S7 with fine to
-	-	25						Roller bit to 25 feet.			
-		20	X	S9	25 to 27	24/0	11-16- 25-31	PID = 0.0ppm @S9 Roller bit to 30 feet.		S9: No Recovery.	
- 20	-	30									
12/22/23	-		X	S10	30 to 31.8	21/11	9-34-81- 100/3"	но – 0.0ppн @310		S10 (0-6"): SILTY SAND WI medium sand, ~30% subang nonplastic fines; brown; wet. S10 (6"-11"): SILTY SAND (\$ nonplastic fines, ~10% subar	TH GRAVEL (SM); ~50% fine to ular gravel up to 1.5", ~20% SM); ~70% fine sand, ~20% ngular gravel up to 1.5"; brown; wet.
		35						Roller bit to 35 feet. Rig chatter from 33 to 34 feet.			
	-		X	S11	35 to 37	24/15	68-76- 74-102	0.0ppin @011		S11: WIDELY GRADED SAN fines, ~5% fine gravel up to 1	ID (SW); ~90% sand, ~5% nonplastic "; tan to light brown; wet.
10 — 10 —	-							Roller bit to 40 feet.			
		40		S12	40 to 42	24/20	26-35- 41-48	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN fines, ~5% fine gravel up to C	ID (SW); ~90% sand, ~5% nonplastic .5"; tan; wet.
	- - -							Roller bit to 45 feet.			
		45	$\left \right $	S13	45 to 47	24/12	43-51- 35-50	PID = 0.0ppm @S13		S13: SILTY SAND (SM); ~60 nonplastic fines, ~10% subar	% fine to medium sand, ~30% ngular gravel up to 1.5"; brown; wet.
NOTES: PRO CIT GE									PROJ CITY/S GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants





STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT



		IG IN		RN	1ATION	1			FASTING (ff). 1 002 2	05		BORING
	GROUI	ND S	URI		E EL. (f	t): 46.0)		DATE START/END: 4	1/27/20	020 - 4/27/2020	-
1	/ERT./	HOR	IZ.	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B3
1	TOTAL	DEF	тΗ	(ft):	102	.0			DRILLER NAME: Kei	th Par	ent	
	LOGGI	ED B	Y:	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 5
!	DRILL HAMM	.ING ER T	inf Ypi	of :	RMATIC Autom	<u>DN</u> natic			CASING I.D./O.D.: 4	inch/ 4	.5 inch CORE BARF	REL TYPE: NA
4	AUGEF	R I.D.	/0.[) .:	NA / N	NA			DRILL ROD O.D.: NN	1	CORE BARF	REL I.D./O.D.: NA / NA
				HO	D: <u>M</u>	ud Rotary	Wash					
			VEL		PT 115 (ity:						
	ABBRE	EVIA	ΓΙΟΙ	NS:	Pen. Rec. RQD WOR WOH	= Penetrati = Recovery = Rock Qu = Length of R = Weight of I = Weight of	on Length / Length ality Designa ⁵ Sound Core of Rods of Hammer	ation es>4 in / Pen.	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Dia	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter
					Sa	ample Inf	ormation			bc		
	Elev. (ft)	Dep (ft	oth)	Sa	ample No.	Depth (ft)	Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	raphic Lo	Soil and F	Rock Description
					110.	(11)	(in)	or RQD		U	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 46
		_		$\left \right $	S1	0 to 2	24/17	20-9-8- 11	PID = 0.0ppm @S1		S1: SILTY SAND WITH GRAV nonplastic fines, ~20% subang Brick, asphalt, and concrete fra Slight petroleum-like odor.	/EL (SM); ~60% sand, ~20% Jular gravel up to 1.5"; brown; dry. agments.
	-	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							PID = 0.0ppm @S2		S2: SILTY SAND WITH GRA petroleum odor.	/EL (SM); Similar to S1. No
~	+		5	X	S3	4 to 6	24/4	3-2-2-3	PID = 0.0ppm @S3		S3: SILTY SAND WITH GRAN petroleum odor.	/EL (SM); Similar to S1. No
3.601 12/22/2	40 —	_		$\left \right $	S4	6 to 8	24/8	2-1-1-4	PID = 0.0ppm @S4		S4: SILTY SAND WITH GRAN petroleum odor.	/EL (SM); Similar to S1. No
EMPLATE 201	_	_		X	S5	8 to 10	24/3	2-6-8-5	PID = 0.0ppm @S5		S5: SILTY SAND WITH GRA	/EL (SM); Similar to S1. No
IAI	_	_	10	/ \							STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 36
eru Gelua		-		X	S6	10 to 12	24/10	5-7-6-7	ты – 0.0ppm @30		S6: SILTY SAND (SM); ~70% nonplastic fines, ~10% subang yellow-brown; dry; trace mica.	fine to medium sand, ~20% jular gravel up to 1"; brown to
23053/0-AS IUKIA CUVE.	-	_	15						Drive casing to 14 feet below grade. Rollerbit to 15 feet.			
	30 —	_		$\left \right $	S7	15 to 17	24/8	2-3-4-3			S7: NARROWLY GRADED S/ sand, ~5% nonplastic fines; br	AND (SP); ~95% fine to medium own to tan; dry.
STD 6-NUKIH-EASI	-	-							Roller bit to 20 feet.			
	NOTES): 								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New Y ROJECT NUMBER: 2305370	fork GEI Consultants

NORT	HING (ft) ND SUR	: _2 FAC	21,718 E EL. (f	it): 46.0			EASTING (ft): _1,003,3	305 4/27/20	20 - 4/27/2020	BORING B3
VERT.	/Horiz.	DAT	UMS:	NAVD 88	3/NAD83 N	IY Zone 310	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 5
Elev. (ft)	Depth (ft)	Sa	Sample No.	ample Inf Depth (ft)	ormation Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-	-	X	S8	20 to 22	24/7	3-6-12- 15	PID = 0.0ppm @S8		S8: NARROWLY GRADED S	GAND (SP); Similar to S8.
	- 25						Roller bit to 25 feet. PID = 0.0ppm @S9		SO: SII T (MII): 040/ complete	is finan 6% fina condulight brown:
20 —	-	X	S9	25 to 27	24/13	2-4-10- 15	Roller bit to 30 feet.		wet. [GRAIN SIZE TEST PEF	ic fines, 6% fine sand; light brown; FORMED].
-	-								STRATUM III - TILL (C	CLASS 3A) at ~28.5 ft / El. 17.5
_	- 30 -	X	S10	30 to 32	24/13	11-13- 14-27	PID = 0.0ppm @S10		S10: SILT WITH SAND (ML) sand; light brown; moist. [GR	; 77.6% nonplastic fines, 22.4% fine AIN SIZE TEST PERFORMED].
-	-						Roller bit to 35 feet. Rig chatter ~32 feet.			
10 —	35	\mathbb{N}	S11	35 to 37	24/15	20-34- 46-56	PID = 0.0ppm @S11		S11: WIDELY GRADED SAN subrounded gravel up to 1.25 some mica.	ID (SW); ~85% sand, ~10% ", ~5% nonplastic fines; brown; wet;
-	-						Roller bit to 40 feet.			
_	- 40 -	\mathbb{N}	S12	40 to 42	24/15	29-61- 50-47	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN fines, ~5% fine gravel; brown	ID (SW); ~90% sand, ~5% nonplastic to tan; wet.
-	+						Roller bit to 45 feet. Rig chatter ~44 feet.			
-	45	X	S13	45 to 47	24/10	14-10- 68-67	PID = 0.0ppm @S13		S13: WIDELY GRADED SAN subrounded gravel up to 1.5"	ID (SW); ~85% sand, ~10% , ~5% nonplastic fines; brown; wet.
0	+ S:	VN		41	<u> </u>		1	PROJ	ECT NAME: Astoria Cove	
								CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370		



	NORTH	IING (ft): _221,718	3			EASTING (ft): _1,003,30)5		BORING
			FACE EL.	(ft): 46.0) R/NAD83 N	V 70ne 31	DATE START/END: 4	/27/20	20 - 4/27/2020	B3
	VERI.	nuriz.	DATONIS.		D/INADOS IN	TT ZONE ST	DRILLING COMPANY.	_ Ulai		PAGE 4 of 5
ľ	Elev. (ft)	Depth (ft)	Sample No.	Sample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
	- -30	- 75 -	X <u></u> 519_	75 to 75.4	5/4	103/5" _	Roller bit to 75 feet. Rig chatter from 70 to 75 feet. PID = 0.0ppm @S19 Roller bit to 80 feet.		S19: NARROWLY GRADED S17.	SAND WITH SILT (SP-SM); Similar
	-	80 	S20	80 to 80	0/0	55/0"	Intermittent rig chatter from 75 to 80 feet. PID = 0.0ppm @S20		S20: No Recovery. (80'-82'): BOULDER / COBBI	LE.
EMPLATE 2013.GDT 12/22/23	- -40	- 85 -	S21	85 to 86.8	22/17	66-70- 108- 100/4"	Roller bit to 85 feet. Intermittent rig chatter from 80 to 85 feet. PID = 0.0ppm @S21		S21: NARROWLY GRADED fine to medium sand, ~10% n gravel up to 1"; brown to tan-t	SAND WITH SILT (SP-SM); ~80% ionplastic fines, ~10% subangular prown; wet.
ASTORIA COVE.GPJ GEI DATA TE	-	- 90 -	× <u>s22</u>	90 to 90.3	4/4	<u>100/4"</u>	Roller bit to 90 feet. Intermittent rig chatter from 85 to 90 feet. PID = 0.0ppm @S22		S22: NARROWLY GRADED to S21.	9 SAND WITH SILT (SP-SM); Similar
0 6-NORTH-EAST-GRAPHIC LOG 2305370-	- -50 —	_ 95 _ _	× <u>\$23</u>	95 to 95.2	2/2	100/2"	Roller bit to 95 feet. Rig chatter between 90 to 92 feet and 94 to 95 feet. PID = 0.0ppm @S23		S23: WIDELY GRADED SAt sand, ~20% subangular grav	ND WITH GRAVEL (SW); ~80% el up to 1"; brown; wet.
N STL										
GEI WOBUKN	NOTES	:						PROJI CITY/S GEI PI	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants



B N	orin Orth	ig in IING	FOF ft):	221,775	<u>N</u>			EASTING (ft): 1,003,31	10		BORING	
G				CE EL. (ft): 24.7		V Zone 244		6/4/202	20 - 5/4/2020	R/	
Т	ERT./ OTAL	DEP	Ζ. <i>D/</i> ΓΗ (f	ATUMIS: t): 102	88 2.0	3/NAD83 N	Y Zone 310	DRILLING COMPANY: DRILLER NAME: Keit	th Par	ent	D4	
L	OGGE	ED BY	': _	G. Holm	es			RIG TYPE: CME 75			PAGE 1 of 5	
	RII I	ING		RMATI								
н Н		ER T	PE:	Autor	natic			CASING I.D./O.D.: 4 i	nch/ 4	.5 inch CORE BAR	REL TYPE: NA	
A	UGEF	R I.D./	0.D.	NA /	NA			DRILL ROD O.D.: NM		CORE BAR	REL I.D./O.D.: NA / NA	
D			ETH El r		/lud Rotary (#)·	Wash						
					(11).							
A	BBRE	EVIAT	ONS	E Pen Rec RQI WO WO	. = Penetrati . = Recovery) = Rock Qu = Length of R = Weight H = Weight	ion Length / Length ality Designa f Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter	
				S	Sample Inf	ormation			D			
E	lev.	Dep	h			Pen /	Blows	Drilling Remarks/	lic Lo	Soil and I	Pack Description	
((ft)	(ft)		Sample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	Sraph			
				,		(in)			0	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 24.7	
	-	AND (SP); ~90% fine to medium 5% coarse subangular gravel up to										
	-	_	L	¥				PID = 0.0 nnm @S2		STRATUM IIC - SAND	(CLASS 3B) at ~2 ft / El. 22.7	
	-	_	X	S2	to 4	24/16	5-4-5-6			S2: NARROWLY GRADED S nonplastic fines; tan; dry.	AND (SP); ~95% fine sand, ~5%	
		-	+		4	04/40	5540	PID = 0.0ppm @S3		S3 [.] NARROWLY GRADED S	AND (SP) [.] Similar to S3	
:	20 —	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				24/10	5-5-4-6					
GDT 12/22/23	_	_		S4	6 to 8	24/8	4-3-5-5	PID = 0.0ppm @S4		S4: NARROWLY GRADED S sand, ~10% nonplastic fines;	AND WITH SILT (SP-SM); ~90% fine light brown; dry.	
MPLATE 2013.	-	-		S5	8 to 10	24/15	5-3-6-6	PID = 0.0ppm @S5	= 0.0ppm @S5 S5: SILTY SAND (SM); ~80% fine sand, ~2 light brown; dry.			
A TEI	_	_ 1	Ľ							STRATUM III - TILL (0	CLASS 3A) at ~10 ft / El. 14.7	
PJ GEI DAT	_	-		S6	10 to 12	24/10	15-19- 28-63	PID = 0.0ppm @S6		S6: WIDELY GRADED SAND ~10% nonplastic fines, ~10% to brown; dry.	WITH SILT (SW-SM); ~80% sand, subangular gravel up to 1.5"; white	
5370-ASTORIA COVE.G	- - -	_		V				Drive casing to 14 feet below grade. Rollerbit to 15 feet.				
OG 230	-U-	— 1	5	S7	15 to	9/9	24-			S7: WIDELY GRADED SAND ~25% subangular gravel up to	WITH GRAVEL (SW); ~75% sand, o 1"; brown; dry.	
ID 6-NORTH-EAST-GRAPHIC L	Roller bit to 20 feet. Rig chatter from 15 to 18 feet.											
Na Na	OTES	:		1	1	I	I		PROJ	ECT NAME: Astoria Cove		
		-							CITY/ GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants	

NORT	HING	(ft):	221,775	5			EASTING (ft): 1,003,	310		BORING
GROU	ND S	URF	FACE EL. ((ft):24.7	7		DATE START/END:	5/4/202	0 - 5/4/2020	B4
VERT.	/HOR	IZ. I	DATUMS:	NAVD 88	3/NAD83 N	IY Zone 31	DA DRILLING COMPANY:	Craiç	g Geotechnical Drilling	PAGE 2 of 5
			5	Sample Inf	ormation			D		
Elev. (ft)	Dep (ft	oth)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and I	Rock Description
			X 58	20	9/5	19-	PID = 0.0ppm @S8		S8: WIDELY GRADED SAND	WITH GRAVEL (SW); Similar to S
-		ć		20.8		- <u>101/3"</u>			Possible cobbles and boulder	s from 20.75' to 22.9'
- 0	-						Roller bit to 25 feet. Rig chatter from 21 to 24 feet.			
-		25	S9	25 to 27	24/5	31-22- 17-13	PID = 0.0ppm @S9		S9: WIDELY GRADED SAND ~20% subangular gravel up to wet. Trace mica.	9 WITH GRAVEL (SW); ~75% san o 1.5", ~5% nonplastic fines; brow
-							Roller bit to 30 feet. Rig chatter from 28 to 30 feet.			
-		30	S10	30 to 30.9	11/8	60- 100/5"	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~10% sun-angular grav brown; wet.	SAND (SP); ~85% fine to medium el up to 1.5", ~5% nonplastic fines
-	-						Roller bit to 35 feet. Rig chatter from 32 to 35 feet.			
- 10		35	S11	35 to 37	24/19	48-67- 58-65	PID = 0.0ppm @S11		S11: NARROWLY GRADED	SAND (SP): Similar to S10.
-							Roller bit to 40 feet. Rig chatter from 36 to 37 feet.			
-		40	S12	40 to 42	24/17	41-57- 51-59	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN up to 0.75"; tan-brown; wet.	D (SW); ~95% sand, ~5% fine gra
-							Roller bit to 45 feet.			
-20 —		45	S13	45 to 45.9	11/10	33- 106/5"	PID = 0.0ppm @S13		S13: WIDELY GRADED SAN fines, ~5% subrounded grave	D (SW); ~90% sand, ~5% nonpla I up to 1.5"; brown; wet.
IOTES	S:							PROJE	ECT NAME: Astoria Cove	
								CITY/S GEI PI	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	

NORT	HING (ft): _:	221,775 CE EL 4	ft) ∙ 0/17	7		EASTING (ft): 1,003,3	5/4/202	20 - 5/4/2020	
VERT.	/HORIZ	DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 31	DATE OTAKINEND. DATE OTAKINEND. DATE OTAKINEND.	Crai	g Geotechnical Drilling	
										FAGE 3 01 5
Elev. (ft)	Depth (ft)	s	ample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and F	Rock Description
-	-						Roller bit to 50 feet. Rig chatter 46 to 47 feet and 48 to 50 feet.			
-	50 	\mathbb{N}	S14	50 to 52	24/13	35-57- 40-27	PID = 0.0ppm @S14		S14: WIDELY GRADED SANI	D (SW); Similar to S13.
-30 —							Roller bit to 55 feet. Rig chatter from 54 to 55 feet.			
-	- 55 - - -	M	S15	55 to 55.2	2/0	101/2"	PID = 0.0ppm @S15		S15: No Recovery. Cobble stu	ıck in tip.
_	- - - - 60	V	S16	60 to 62	24/18	44-51- 41-46	Roller bit to 60 feet. Rig chatter from 55 to 60 feet. PID = 0.0ppm @S16		S16: NARROWLY GRADED S sand, ~5% subrounded gravel orange-brown; wet.	SAND (SP); ~95% medium to coarse up to 1.5"; light brown to
-	- - - -	Δ					Roller bit to 65 feet.			
-40 — -	- 65 - -	X	S17	65 to <u>65.9</u>	11/11	110- 100/5"	PID = 0.0ppm @S17		S17: WIDELY GRADED SANE ~20% subrounded gravel up to wet.	D WITH GRAVEL (SW); ~75% sand, o 1.5", ~5% nonplastic fines; brown;
-	- - - - - - - - - - - - - - - - - - -			70			Roller bit to 70 feet. PID = 0.0ppm @S18			
-	-	X	S18	to 10, 10, 10, 10, 10, 10, 10, 10,	10/10	39- <u>107/4"</u>			~10% nonplastic fines, ~5% fi	ne gravel; brown; wet.
NOTES	S:							PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ^N ROJECT NUMBER: 2305370	York GEI Consultants

NORT	HING (ft): <u>221,775</u>	S	-		EASTING (ft): 1,003,3	10	0.5440000	BORING
VERT.	ND SUR /HORIZ.	DATUMS:	n): 24. NAVD 8	/ 8/NAD83 N	Y Zone 310	DATE START/END: D4_ DRILLING COMPANY:	Crai	g Geotechnical Drilling	B4
									PAGE 4 of 5
Elev. (ft)	Depth (ft)	Sample No.	ample In Depth (ft)	formation Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-50 — -50 — -	- - - - - - - -	× <u>S19</u>	75 to 75.2	2/2	100/2"	Roller bit to 75 feet. Rig chatter from 71 to 75 feet. PID = 0.0ppm @S19		S19: SILTY SAND (SM); ~60 plasticity fines, ~10% fine gra	% sand, ~30% nonplastic to low ivel; black to white; wet. Trace mica.
-	- - - - - - - - -	× <u>s20</u>	80 to 80.3	3/3	126/3"	Roller bit to 80 feet. Intermittent rig chatter from 75 to 80 feet. PID = 0.0ppm @S20		S20: WIDELY GRADED SAN subangular gravel up to 1.25'	ID (SW); ~85% sand, ~10% ', ~5% nonplastic fines; brown; wet.
-60	- - - - - - - -	× <u>S21</u>	85 to 85.2	2/2	100/2"	Roller bit to 85 feet. Intermittent rig chatter from 80 to 85 feet. PID = 0.0ppm @S21		S21: WIDELY GRADED SAN sand; ~10% nonplastic to low gravel up to 0.75"; brown; we	ID WITH SILT (SW-SM): ~85% / plasticity fines; ~5% subangular t. Trace mica.
	- - - - - - - - - -	S22	90 to 90.9	11/11	49- 102/5"	Roller bit to 90 feet. Intermittent rig chatter from 85 to 90 feet. Slow advancement 85 to 87 feet. PID = 0.0ppm @S22		S22: SILTY SAND WITH GR. nonplastic fines, ~20% subar Trace mica.	AVEL (SM); ~45% fine sand, ~35% igular gravel up to 1.5"; brown; wet.
-70-	 95 	S23	95 to 97	24/16	9-17-24- 26	Roller bit to 95 feet. Rig chatter between 91 to 93 feet. PID = 0.0ppm @S23		STRATUM IV - DECOMPO	SED/WEATHERED ROCK (CLASS -93 ft / El68.3 % nonplastic to low plasticity fines, an to white to pink; moist. Banded k.
	 - -					Roller bit to 100 feet.			
	York GEI Consultants								



				MATIO	N			EASTING (#): 1 003 3	13		BORING			
	GROU		RFA	CE EL. (ft): 24.8	3		DATE START/END:	43 3/9/202	20 - 3/9/2020				
	VERT.	HORE	z. D/	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	ig Geotechnical Drilling	B5(OW)			
	TOTAL	DEP	ΓΗ (f	t): <u>102</u>	2.0			DRILLER NAME:	e Schu	ster				
	LUGGI		• -		5						PAGE 1 of 5			
	DRILL	ING I	NFC	RMATI	<u>NC</u>									
			PE:	Auton	natic			CASING I.D./O.D.: _4	inch/ 4	.5 inch CORE BAR	REL TYPE: NX Wireline			
	DRILLI	NG M	ETH	OD: N	lud Rotary	Wash			1		REL I.D./O.D. . 2.125 INCH / 5 INCH			
	WATE	R LEV	EL C	EPTHS	(ft): ⊻ 1	8.8 3/10/2	2020 7:00 a	m in observation well						
	ABBR	EVIATI	ONS	: Pen. Rec. RQD WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight	on Length / Length ality Designa ^c Sound Core of Rods	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auror		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector LD /O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.			
╞				S	ample Inf	ormation			5					
	Elev.	Dept	h			Don /	Ploye	Drilling Remarks/	c Log					
	(ft)	(ft)		Sample	Depth	Rec.	per 6 in.	Field Test Data	raphi	Soil and	Rock Description			
				110.	(14)	(in)	or RQD		U	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 24.8			
			$\left \right\rangle$	S1	0 to	24/17	17-14-	PID = 0.0ppm @S1		S1 (0-6"): ASPHALT AND BA	ASE MATERIAL.			
	_	_	X		2		10-11			S1 (6"-17"): SILTY SAND (SI	M); ~75% fine sand, ~25% nonplastic			
	-	_	\square											
			Λ	S2	2 to	24/17	6-5-9-11	- 0.0ppm @32		S2: SILTY SAND (SM); ~70% medium sand; brown; moist.	6 low plasticity clay, ~30% fine to			
	_	-	IX		4									
	-	_	Ľ					PID = 0.0ppm @S3		STRATUM IIB - SILT/CLA	Y (CLASS 4B/5B) at ~4 ft / El. 20.8			
	20	-		S3	to	24/12	9-7-8-7	n Brotoppin @cc		sand; light brown; dry.	~60% nonplastic fines, ~40% fine			
	20		5		o									
2/23	-	_	Ĺ		6			PID = 0.0ppm @S4			(ML): Similar to S2			
12/2	_		N	S4	to	24/13	8-9-10-4			S4 (3"-13"): SILTY SAND (SI	M); ~60% fine sand, ~40% nonplastic			
GDT		-	Ň		0					fines; light brown; dry.				
2013	-	_	\vdash		8			PID = 0.0ppm @S5		S5 (0-8"): SILTY SAND (SM)	· Similar to S4			
LATE	_		N	S5	to 10	24/14	10-7-6-8		casing to 8 feet below					
TEMP		-			10			Drive casing to 8 feet below grade.		S5 (8"-14"): SILT WITH SAN	D (ML); ~60% nonplastic fines, ~40%			
ATA 1		- 1	┝	00	10	04/44	4460	PID = 0.0ppm @S6		fine sand; light brown; dry.	80.6% nonplastic fines: 19.4% fine			
SEI D	_		N	50	to 12	24/14	4-4-0-8			sand; light brown; dry. [GRAI	N SIZE TEST PERFORMED].			
SPJ 0														
OVE.0	-	╞	\vdash	Y										
IA CC	-	L												
STOR											ULASS 3B) at ~13.5 π / El. 11.3 			
(70-A)	-	╞												
23053	10 —	L 1	5		·-									
90			\backslash	S7	15 to	24/5	9-4-3-3			57: NARROWLY GRADED S fine to medium sand; ~20% s	SAND WITH GRAVEL (SP); ~75% subangular gravel up to 1.5"; ~5%			
HICL	_	F	X		17					nonplastic fines; brown; mois	t. Trace mica.			
RAP	-	Ļ	Ľ											
AST-C	_													
μĤ		F												
NOR	-	F												
TD 6-														
JRN S	NOTES	6:					•	·	PROJ	ECT NAME: Astoria Cove				
GEI WOBU									CITY/ GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants			



NORT GROU	HING (fi IND SUF	i): RFA	221,770 CE EL. (1	f t): <u>24</u> .8	3		EASTING (ft): <u>1,003,3</u> DATE START/END:	43 3/9/202	20 - 3/9/2020	BORING B5(OW)
VERT.	/HORIZ	. DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Cra	ig Geotechnical Drilling	PAGE 3 of 5
		Т	S	ample Inf	ormation					
Elev. (ft)	Depth (ft)		Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Loo	Soil and	Rock Description
-	-	X							S14 (3"-20"): NARROWLY G ~85% sand, ~10% nonplastic to 1.0"; brown; wet.	RADED SAND WITH SILT (SP-SM); c fines, ~5% fine subangular gravel up
-	50 	X	S15	50 to 51.8	21/17	29-36- 65- 100/3"	PID = 0.0ppm @S15		S15: NARROWLY GRADED S14 (3"-20").	SAND WITH SILT (SP-SM); Similar
-30-)55 ;						Rig chatter from 51 to 52 ft and 53 to 55 ft.			
-	- 55 - - -	W	<u>S16</u>	55 to 55.1	1/0	100/1"	PID = 0.0ppm @S16 Rig chatter from 55 to 57 ft.		S16: No Recovery. Potential	boulders and cobbles.
-		W	<u>S17</u>	60 to 60.1	1/0	100/6"	Faster advancement after so ft. PID = 0.0ppm @S17 Rig chatter from 60 to 61 ft.		S17: No Recovery. Potential	boulders and cobbles.
- -40 -	- - - - - - - - - - - - - - -	X	S18	65 to 67	24/16	58-45- 27-32	PID = 0.0ppm @S18 Rig chatter ~67 ft and from 69 to 70 ft.		S18: NARROWLY GRADED fine to medium sand, ~20% <u>c</u> ~10% nonplastic fines; browr	SAND WITH SILT (SP-SM); ~70% gravel subangular gravel up to 1.5", n; wet. Some mica.
-	1_ - - - - - - - - - - -	X	S19	70 to 71.3	16/15	35-44- 110/4"	PID = 0.0ppm @S19 Rig chatter from 72 to 73 ft		S19: SILTY SAND (SM); ~55 nonplastic fines, ~5% fine su wet.	% fine to medium sand, ~40% brounded gravel up to 0.5"; brown;
NOTES	S:							PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

NORTI GROU VERT.	HING (ft) ND SUR /HORIZ.	: _221,770 FACE EL. (DATUMS:) ft):24.8 NAVD 88	3 3/NAD83 N	Y Zone 310	EASTING (ft): 1,003,3 DATE START/END: 2 DATE START/END: 2	43 3/9/202 Crai	20 - 3/9/2020 g Geotechnical Drilling	BORING B5(OW)	
	1							<u> </u>	PAGE 4 of 5	
Elev. (ft)	ev. Depth (ft) Sample No.		Sample Information Depth (ft) Pen. Rec. (in)		Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	d Rock Description	
-50 — -50 — -	- - - - - - - -	X <u>s20</u>	75 to 75.4	<u>5</u>	107/5"	PID = 0.0ppm @S20 Rig chatter from 77 to 80 ft.		S20: SILTY SAND WITH GR sand, ~20% nonplastic fines brown; wet.	RAVEL (SM); ~60% fine to medium , ~20% subangular gravel up to 1.25";	
	- 80 - 80	× <u>S21</u>	80 to 80.3	3/3	108/3"	PID = 0.0ppm @S21 Rig chatter from 83 to 84 feet.		S21: SILTY SAND WITH GR	RAVEL (SM); Similar to S20.	
A TEMPLATE 2013.GDT 12/22/23	- 85 	S22	85 to 87	24/17	23-23- 30-28	PID = 0.0ppm @S22		S22 (0-5"): SILTY SAND (SM fines, ~5% fine subangular g STRATUM IV - DECOMPC 1D) at ~ S22 (5"-17"): DECOMPOSE	M); ~75% fine sand, ~20% nonplastic ravel up to 0.5"; orange-brown; wet. SED/WEATHERED ROCK (CLASS -85.6 ft / El60.8 D/WEATHERED BEDROCK.	
	90 90 	S23	90 to 92	24/24	7-15-19- 30	PID = 0.0ppm @S23		S23: DECOMPOSED BEDR	OCK/WEATHERED BEDROCK.	
	- - - - - - - - - - - - - - - - - - -	S24	95 to 97	24/24	9-15-19- 21	PID = 0.0ppm @S24		S24: DECOMPOSED BEDR	OCKWEATHERED BEDROCK.	
	 5:						PROJ CITY/S GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants	



BOR NOR	XIN(THI	g in Ing	IFC (ft)	DRN : 2	1ATION 221,715	<u>1</u>			EASTING (ft): 1,003,1	83		BORING			
GRO	UN	DS	URI	FAC	E EL. (f	it): _ 46.2	2		DATE START/END:	4/30/2	020 - 4/30/2020	50			
VER	Г./H	IOR	IZ.	DA		NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B6			
LOG	GEI	DEP DB	чн Ү:	(π) G	: <u>100</u> . Holme	.4 S			DRILLER NAME: Kei RIG TYPE: CME 75	ith Par	PAGE 1 of 5				
						-						PAGE 1 01 5			
DRIL	LI	NG		=0F -	RMATIC	<u>NC</u>									
AUG	ME ER	:R I I.D.	199 /0.1	E: D.:	Autom NA / I	NA			CASING I.D./O.D.: 4	inch/ 4 /		RELINPE: NA RELID./O.D.: NA/NA			
DRIL	LIN	IG N	/ET	но	D: <u>M</u>	ud Rotary	Wash								
WAT	ER	LE\	/EL	DE	PTHS (ft):									
ABBI	RE\	VIAT	101	NS:	Pen. Rec. RQD WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight of I = Weight of	on Length / Length ality Designa / Sound Core of Rods of Hammer	ation ss>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Di	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter			
					S	ample Inf	ormation			bc					
Elev	. (Dep	th	6	amala	Donth	Pen./	Blows	Drilling Remarks/	hic Lo	Soil and F	Rock Description			
(ft)		(ft)	2	ampie No.	(ft)	Rec.	per 6 in.	Field Test Data	Grapl					
	_					0	(11)	OFTOE	PID - 1 3ppm @\$1		STRATUM I - FILL (CLASS 7) at ~0 ft / El. 46.2			
-				X	S1	to 2	24/9	19-11-7- 10	PID = 0.0mm @\$2		 SILTY SAND (SIN), ~70% nonplastic fines, ~10% angula Brick, asphalt, and concrete fra SILTY SAND (SM); ~70% nonplastic fines, ~10% angula Prior conclusion 	rg ravel up to 1"; dark brown; dry. agments. Slight petroleum-like odor. fine to medium sand, ~20% r gravel up to 1"; dark brown; dry.			
			5	5	X	S2	2 to 4	24/9	5-6-3-1	ни – 0.9ppm @32		S2: SILTY SAND (SM); Simila	ar to S1. No petroleum odor.		
0		- !			5	5	X	S3	4 to 6	24/7	2-9-6-8	PID = 30.2ppm @S3		S3: SILTY SAND (SM); Simila	ar to S1. No petroleum odor.
- 04 - 12/22/2	-			X	S4	6 to 8	24/7	3-3-4-3	[–] PID = 1.3ppm @S4 [–] PID = 0.8ppm @S5		S4: SILTY SAND (SM); Simila	ar to S1. No petroleum odor.			
EMPLATE 20				X	S5	8 to 10	24/10	5-4-1-4		S5: SILTY SAND (SM); Similar to S1. No petroleum odor.					
I V		_ `	10	$\langle \rangle$		40			PID = 2.3 ppm @S6		STRATUM IIA - LOOSE SAND (CLASS 6) at ~10 ft /				
	-			X	S6	10 to 12	24/12	4-3-2-6	Tib 2.oppin @co		S6: NARROWLY GRADED S/ to medium sand, ~10% nonpla up to 1"; dark brown; dry.	AND WITH SILT (SP-SM); ~85% tine astic fines, ~5% subangular gravel			
			15						Drive casing to 14 feet below grade. Rollerbit to 15 feet.	V					
90 ЭОТ ЭНСТОР 30 -	-		15	X	S7	15 to 17	24/0	6-8-6-7			S7: No Recovery. Attempted t	o sample with 3" spoon; no recovery. CLASS 3B) at ~16 ft / El. 30.2			
יאט-ו אטאט-ו אטאט-ו טאטאט				<u>/ \</u>					Roller bit to 20 feet.						
	ES:									PRO. CITY GEI F	JECT NAME: Astoria Cove /STATE: Astoria, Queens, New ` PROJECT NUMBER: 2305370	fork GEI Consultants			

NORT	HING	6 (ft)	: _2	221,715	6). 40.0	<u></u>		EASTING (ft): 1,003,11	83	200 4/20/0000	BORING
VERT.	/HOF	RIZ.	DAI	EEL. (1 TUMS:	NAVD 88	2 3/NAD83 N	IY Zone 310	DATE START/END: _2 04 DRILLING COMPANY:	<u>Crai</u>	g Geotechnical Drilling	Вр
								I	_		PAGE 2 of 5
Elev. (ft)	Dej (f	pth t)	Sample No.		ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description	
-	-		X	S8	20 to 22	24/6	11-4-11- 10	PID = 0.0ppm @S8 Roller bit to 25 feet.		S8: SILTY SAND WITH GRA sand, ~20% nonplastic fines, brown to orange-brown; dry.	VEL (SM); ~60% fine to medium ~20% subangular gravel up to 1.5";
- 20-		25	X	S9	25 to 27	24/2	10-8-7-9	PID = 0.0ppm @S9	/	S9: WIDELY GRADED SANI ~30% subrounded gravel up	D WITH GRAVEL (SW); ~70% sand, to 1.5"; brown; dry.
-	-							grade. Roller bit to 30 feet.		STRATUM III - TILL (C	CLASS 3A) at ~28.5 ft / El. 17.7
-		30	X	S10	30 to 32	24/13	10-11- 12-14	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~5% nonplastic fines; t	SAND (SP): ~95% fine to medium an; dry.
								Roller bit to 35 feet. Rig chatter from 32 to 35 feet.			
- 10		35	X	S11	35 to 37	24/24	11-14- 14-12	PID = 0.0ppm @S11		S11: NARROWLY GRADED	SAND (SP); Similar to S10.
-	-							Roller bit to 40 feet. Rig chatter from 36 to 37 feet.			
-		40		S12	40 to 42	24/15	9-12-15- 17	PID = 0.0ppm @S12		S12: SILT WITH SAND (ML) light brown; dry. [GRAIN SIZE	; 82.9% nonplastic fines, 17.1% sand; E TEST PERFORMED].
-								Roller bit to 45 feet.			
		45	X	S13	45 to 47	24/18	15-19- 32-40	PID = 0.0ppm @S13		S13: SILTY SAND (SM); ~60 ~10% subrounded gravel up	% sand, ~30% nonplastic fines, to 1"; brown; wet.
NOTES	S:								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants







	RIN RTH	IG IN IINC		RN	1ATION	1			FASTING (ff). 1 002 2	03		BORING		
GR		ND S	UR		E EL. (f	t): 40.1			DATE START/END: 4	4/29/2	020 - 4/29/2020	B7		
VEF	RT./	HOR	IZ.	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Cra	ig Geotechnical Drilling			
TOT	TAL	DEF	тΗ	(ft):	102	.0			DRILLER NAME: Kei	ith Par				
LOC	GGE	ED B	Y:	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 5		
DR	ILL	ING	INF	OF	RMATIC	N								
HAI	ммі	ER T	YPI	:	Autom	atic			CASING I.D./O.D.: 4	inch/ 4	L5 inch CORE BAR	REL TYPE: NA		
	GER	RI.D.	/0.[/=T).: יוורי	<u>NA/1</u>	VA ud Potony	Wash		DRILL ROD O.D.:N	Λ	CORE BAR	REL I.D./O.D.: <u>NA / NA</u>		
WA	TEF	R LEV		. DE	:PTHS (1	ft):	Wash							
AB	BRE	:VIA I	nor	NQ:	Pen. Rec. RQD WOR	= Penetrati = Recovery = Rock Qu = Length of & = Weight	on Length Length ality Designa Sound Core of Rods	ation es>4 in / Pen	S = Spiit-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample		QP = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.		
					WOH	l = Weight	of Hammer		HSA = Hollow-Stem Auger		I.D./O.D. = Inside Diameter/Outside Di	ameter		
					58	arripie int	ormation			Log				
Ele (ft	:V. :)	Dep (ft))	Sa	ample	Depth	Pen./	Blows	Field Test Data	tphic	Soil and F	Rock Description		
				No.		(ft)	(in)	or RQD		Gra	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 40.1		
	-			1	S1	0	24/13	38-25-	PID = 0.0ppm @S1		S1: SILTY SAND (SM); ~70%	fine to medium sand, ~20%		
	-	_		XI		το 2		20-12			nonplastic fines, ~10% angula dry. Brick, asphalt, wood, and	ar gravel up to 1.25"; gray to black; concrete fragments. Slight		
		_		/										
	-			\T	S2 2 24/6 10-7-8- 4 10 PID = 0.0ppm @S2		S2: NARROWLY GRADED SAND WITH SILT (SP-SM); ~85% fi							
	-	_		XI		4		10			nonplastic fines; dark brown; c	dry. Concrete fragments.		
				$\langle \rangle$										
	ſ	_			S3	4 to	24/10	10-10-	PID = 0.0ppm @S3		S3: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to S2.			
				XI		6		17-12			. 52.			
S				$\langle \rangle$										
12.2.12	ſ	_			S4	6 to	24/8	16-12-6-	PID = 0.0ppm @S4		S4: SILTY GRAVEL WITH SA	ND (GM); 47.7% subangular gravel		
10	-	_		XI		8		8		000	Brick, asphalt, wood, glass, an	nd concrete fragments. [GRAIN SIZE		
13.6				\mathbb{N}						Pap	TEST PERFORMED].			
15.2	f	_			S5	8 to	24/9	22-13-9-	PID = 0.0ppm @S5		S5: NARROWLY GRADED S			
IPLA	_	_		XI		10		8			to 1.5", ~10% nonplastic fines	es; dark brown; dry. Concrete		
A I EV				\mathbb{N}							tragments.			
1 30	⊢י		10		S6	10	24/13	16-10-8-	PID = 0.0ppm @S6		S6: WIDELY GRADED SAND	D WITH SILT AND GRAVEL		
ee C	_	_		χI		12		29			ovv-Sivi); ~60% sand, ~30% nonplastic fines; dark brown; c	supangular gravel up to 1.5", ~10% dry. Wood fragments and black		
GPJ				\mathbb{N}							staining on bottom 6" of sampl	le.		
OVE	-	-							Drive casing to 14 feet below	v				
A C		_							grade. Noierbil 10 13 1881.					
S C	1													
Y0-A	-	-												
3053			15											
50				$\langle $	S7	15 to	24/13	8-7-70-			S7: CLAYEY SAND (SC); ~70)% fine to medium sand, ~20% low		
IIC LC	-	-		XI		17		1/			Some organics in top 6".			
XAPF		_		/										
1-6-	ſ	-		T										
-EAS	_	_							Roller bit to 20 feet.		STRATUM IIC - SAND (0	CLASS 3B) at ~18.5 ft / El. 21.6		
± XC														
9 C-9	ł	-												
	15	•								rkU.	ECTINAIVIE: ASIONA COVE			
										CITY	STATE: Astoria, Queens, New	York GEI		
5										GEIH	RUJEUI NUMBER: 23053/0			

NORTI	HING ND S	G (ft): SURI	: _2 FAC	21,766 E EL. (f	it): 40.1		N/7 011	EASTING (ft): 1,003,2 DATE START/END:	203 4/29/20	20 - 4/29/2020	BORING B7
VERT.	HOF	≺ı∠ .	UAI	UMS:	NAVD 88	s/NAD83 N	ir ∠one 310	DATE DRILLING COMPANY:	Crai	g Geotecnnical Drilling	PAGE 2 of 5
Elev. (ft)	Dej (f	pth t)	Sa	Sa ample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
	-		X	S8	20 to 22	24/10	8-6-5-6	PID = 0.0ppm @S8		S8: WIDELY GRADED SANE fines, ~5% subangular grave	0 (SW); ~90% sand, ~5% nonplastic up to 1.5"; brown; dry.
-								Roller bit to 25 feet.			
		25	\mathbb{N}	S9	25 to 27	24/10	6-6-9-16	PID = 0.0ppm @S9		S9: SILTY SAND (SM); ~60% brown; dry.	o fine sand, ~40% nonplastic fines;
-	- - -							Roller bit to 30 feet.			
10 — - -	-	30	X	S10	30 to 32	24/11	6-6-6-6	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~5% nonplastic fines; t	SAND (SP); ~95% fine to medium an; dry.
-	-							Roller bit to 35 feet.		STRATUM III - TILL ((CLASS 3A) at ~33.5 ft / El. 6.6
-		35	\mathbb{X}	S11	35 to 37	24/12	13-17- 15-17	PID = 0.0ppm @S11		S11: SILTY SAND (SM); ~75 nonplastic fines; tan; dry.	% fine to medium sand, ~25%
-	-							Roller bit to 40 feet.			
0		40		S12	40 to 42	24/16	17-20- 18-19	PID = 0.0ppm @S12		S12: SILTY SAND WITH GR nonplastic fines, 20.1% subar SIZE TEST PERFORMED].	AVEL (SM); 56.7% sand, 23.2% ngular gravel; brown; dry. [GRAIN
-	 							Roller bit to 45 feet. Rig chatter from 44 to 45 feet.			
		45	X	S13	45 to 45.9	11/5	40- 101/5"	PID = 0.0ppm @S13		S13: SILTY SAND WITH GR	AVEL (SM); Similar to S12.
IOTES	5:								PROJ	ECT NAME: Astoria Cove	

	HING (1	t): RF^	221,766	ft): 40 1			EASTING (ft): 1,003,2	203	020 - 4/29/2020	BORING
VERT.	/HORIZ	. D/	TUMS:	NAVD 88	3/NAD83 N	Y Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	
										FAGE 5 01 5
Elev. (ft)	Deptl (ft)	n s	Sample No.	ample Information Depth (ft) Pen./ Rec. (in)		Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description	
-							Roller bit to 50 feet. Rig chatter from 46 to 48 feet.			
- -10 — -	50 50 	,	S14	50 to 52	24/9	14-11- 19-13	PID = 0.0ppm @S14		S14: WIDELY GRADED SAND ~25% subrounded gravel up to to reddish-brown; wet.	0 WITH GRAVEL (SW); ~70% sand, o 1.5", ~5% nonplastic fines; brown
-	_ 5	i ×	S15	55 j	3/3	100/3"	Roller bit to 55 feet. Rig chatter from 51 to 52 feet ar 53 to 55 feet. PID = 0.0ppm @S15	nd	S15: SILTY SAND WITH GRA	VEL (SM); 51.8% sand, 28.9%
-				55.3			Roller bit to 60 feet. Rig chatter from 58 to 60 feet.		Subangular gravel up to 1°, 19. [GRAIN SIZE TEST PERFORN	.3% nonplastic tines; brown; wet. /IED].
-20 — 	60)	S16	60 to 60	0/0	57/0"	PID = 0.0ppm @S16		S16: NO RECOVERY. No spo	oon penetration.
-	65 65	; X	S17	65 to 66.3	16/13	71-93- 100/4"	Roller bit to 65 feet. Rig chatter from 60 to 62 feet. PID = 0.0ppm @S17		S17: WIDELY GRADED SAND ~10% nonplastic fines, ~5% fir brown; wet.	0 WITH SILT (SW-SM); ∼85% sand, he subrounded gravel up to 0.5";
- -30 —	- 7(,	S18	70 to 72	24/19	46-44- 94-106	Roller bit to 70 feet. Intermittent rig chatter from 65 to 70 feet. PID = 0.0ppm @S18		S18: WIDELY GRADED SANE S17.) WITH SILT (SW-SM); Similar to
NOTES	- 5 :	<u> </u>	<u> </u>					PROJ CITY/	ECT NAME: Astoria Cove	

NORT	HING (ft)	: _221,766				EASTING (ft): 1,003,2	03		BORING
VERT.	ND SUR /HORIZ.	DATUMS:	NAVD 8	i 8/NAD83 N	IY Zone 310	DATE START/END: D4_ DRILLING COMPANY:	4/29/20 Crai	g Geotechnical Drilling	B/
		1				I			PAGE 4 of 5
Elev. (ft)	Depth (ft)	Sample No.	ample Int Depth (ft)	formation Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-	 75 	S19	75 to 75.1	1/1	102/1"	Roller bit to 75 feet. PID = 0.0ppm @S19		S19: NO RECOVERY. Cobb	le stuck in tip.
- -40 -	80 	× <u>s20</u>	80 to 80.4	5/5	100/5"	Roller bit to 80 feet. Intermittent rig chatter from 75 to 80 feet. PID = 0.0ppm @S20		S20: WIDELY GRADED SAN subangular gravel up to 1", ~	ID (SW); ~85% sand, ~10% 5% nonplastic fines; brown; wet.
-	85 85 	× <u>s21</u>	85 to 85.3	4/4	<u>م 101/4"</u>	Roller bit to 85 feet. Rig chatter from 80 to 85 feet. PID = 0.0ppm @S21 Roller bit to 90 feet. Rig		S21: SILTY SAND (SM); ~70 nonplastic fines, ~10% subar Trace mica.	% fine to medium sand, ~20% ngular gravel up to 1.25"; brown; wet.
-50 —	90 90	S22	90 to 90	0/0	57/0"	PID = 0.0ppm @S22		S22: NO RECOVERY. Cobb	le stuck in tip.
-	 95 	× <u>s23</u>	95 to 95.4	5/5	<u>100/5"</u>	Roller bit to 95 feet. Intermittent rig chatter between 90 to 95 feet. PID = 0.0ppm @S23		S23: SILTY SAND (SM); ~75 nonplastic fines, ~5% subang	% fine to medium sand, ∼20% gular gravel up to 0.75"; brown; wet.
NOTES	<u>.</u> 5:		<u> </u>	<u> </u>		INVITED DIE LO TUO TEEL.	PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants



-		NG ING		RN		1			EASTING (#): 1 003 2	07		BORING						
	GROUI	ND S	URI		E EL. (f	t): 23.6	6		DATE START/END:	3/3/202								
,	VERT./	/HOR	NZ.	DAT	rums: Ì	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Cra	B8(OW)							
	TOTAL	DEF	РΤΗ	(ft)	85.0)			DRILLER NAME:	e Schu	, , , , , , , , , , , , , , , , , , ,							
	LOGGI	ED B	Y:	G	. Holme	S			RIG TYPE : CME 75		PAGE 1 of 4							
	DRILL	.ING	INF	FOF	RMATIC	ON												
	намм	ER T	YP	Ξ:	Autom	natic			CASING I.D./O.D.: _4	inch/ 4	.5 inch CORE BAR	REL TYPE: NX Wireline						
		R I.D.	/0.1).:	NA / I	NA			DRILL ROD O.D.: N	M	CORE BAR	REL I.D./O.D.: 2.125 inch / 3 inch						
	WATE	RLE	VEL	HU . DE	D: <u>M</u> PTHS (ud Rotary ft): ⊻ 1	vvasn 18.1 3/9/20)20 6:40 am	in observation well									
	ABBRE	EVIA	τιοι	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight I = Weight	ion Length / Length lality Designa f Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Diameter						
F					Sa	ample Inf	ormation			b								
	Elev.	Dep	oth				Pen /	Blows	Drilling Remarks/	ic Lo	Coil and	Deals Deceministics						
	(ft)	(ft	:)	S	ample No	Depth (ft)	Rec.	per 6 in.	Field Test Data	raph	Soli and	Rock Description						
				110.		(11)	(in)	or RQD		0	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 23.6						
	_			\/	S1	0 to	24/10	4-8-16-	PID = 0.0ppm @S1		S1: SILTY SAND WITH GRA	VEL (SM); ~50% fine sand, ~30% ar gravel up to 1.5": dark brown: drv						
		╞		XI		2		//			Brick fragments.							
	_			/														
	_			$\langle \rangle$	S2	2 to	24/4	6-9-46-	PID = 0.0ppm @S2		S2: SILTY SAND WITH GRAVEL (SM); Similar to S1; with meta	VEL (SM); Similar to S1; with metal						
		F		IXI -		4 16 16	nagmono.											
	20 —			$\langle \rangle$														
		F		\backslash	S3	4 to	24/11	8-12-12-	PID = 0.0ppm @S3		S3: SILTY SAND (SM); ~80%	6 fine sand; ~20% nonplastic fines;						
	_	Ļ	5	XI		6		9			asphait fragments.							
ŝ	-			\mathbb{N}														
2/22/2		-			S4	6	24/9	6-12-12-	PID = 0.0ppm @S4		S4: SILTY SAND (SM); simila	ar to S3.						
1	_	Ļ		χI		8		10										
13.GI	-			\mathbb{N}							STRATUM IIC - SAND) (CLASS 3B) at ~8 ft / El. 15.6						
E 20		-			S5	8	24/10	3-3-5-5	PID = 0.0ppm @S5		S5: NARROWLY GRADED S	SAND (SP); ~100% fine to medium						
PLAT		Ļ		Υ		to 10					sand; light brown and tan; dry	/.						
TEM	-																	
ATA		-	10	10	10	10	10	10	10		56	10	24/9	4-4-5-5	PID = 0.0ppm @S6		S6: NARROWLY GRADED S	SAND (SP); similar to S5.
GEL	-	L		Υ	00	to 12	2.00	1100										
- Ldg	_			\mathbb{N}														
OVE.(╞		+					Drive casing 12 feet below									
IA CC	-	L							grade.									
STOR	10 —																	
70-AS		╞																
30537	_		15															
G 23	_		ıb	$\langle $	S7	15 to	24/14	2-3-8-29	PID = 0.0ppm @S7		S7: SILTY SAND (SM); ~60%	6 fine to medium sand, ~30%						
IC LC	_	F		XI		17					moist.	iguiai gravei up to 0.75 , light brown,						
APH	_			\mathbb{N}							STRATUM III - TILL	(CLASS 3A) at ~17 ft / El. 6.6						
T-GR									Rig chatter from 17 to 20 ft.									
-EAS	_	\vdash																
RTH	_																	
6-NC		F																
I STD	_	1																
	NOTES	5:								PROJ	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370							
0 I										9211		Consultants						



STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT

WOBURN




	BORIN NORTH	NG HING	INF(G (ft)	DRN : 2	/ATION 221,855	<u>N</u>			EASTING (ft): 1,003,27	7		BORING	
	GROUI	ND	SUR	FAC	E EL. (f	i t): 22.3			DATE START/END: 4/	20/20	020 - 4/22/2020	50	
	VERT.	/HO	RIZ.	DA		NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B9	
	LOGG	ED I	EPTE BY:	ι (π) G	: <u>102</u> . Holme	.u s			DRILLER NAME: <u>Keiti</u> RIG TYPE: CME 75	n Par		PAGE 1 of 5	
											_	PAGE 1 01 5	
	DRILL	<u>INC</u>	<u>g in</u>	FOI	RMATIC	<u>NC</u>							
		EK RIT	ייזי ס/ר	Е: П·	Autom	NA			DRILL ROD O D · NM	nch/4	.5 inch CORE BARI	RELITIPE: NA	
	DRILLI	NG	ME	гно	D: <u>M</u>	ud Rotary	Wash		<u></u>				
	WATE	R LI	EVEI	L DE	EPTHS (ft):							
	ABBRE	EVIA	ΑΤΙΟ	NS:	Pen. Rec. RQD WOF WOF	= Penetration = Recovery = Rock Quant = Length of R = Weight of I = Weight of	on Length Length ality Designa Sound Core of Rods of Hammer	ation ss>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Di	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter	
					S	ample Inf	ormation			og			
	Elev. (ft)	De (epth ft)	s	ample	Depth	Pen./ Rec	Blows per 6 in	Drilling Remarks/ Field Test Data	aphic L	Soil and Rock Description		
					No.	(ft)	(in)	or RQD		Gr	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 22.3		
Ī	-	_		M	S1	0 to 2	24/23	9-7-6-7	PID = 0.0ppm @S1		(0-8"): Asphalt. STRATUM IIA - LOOSE SA S1: NARROWLY GRADED S. to medium sand. ~10% nonpl	ND (CLASS 6) at ~0.7 ft / El. 21.6 AND WITH SILT (SP-SM); ~85% fine astic fines. ~5% subangular gravel	
	20 —	-		M	S2	2 to 4	24/8	2-3-3-4	PID = 0.0ppm @S2		up to 1.5"; tan-brown; dry. S2: NARROWLY GRADED S, nonplastic fines; tan; dry.	AND (SP); ~95% fine sand, ~5%	
	-	_	5	M	S3	4 to 6	24/12	2-5-3-4	PID = 0.0ppm @S3		S3: NARROWLY GRADED S	9 SAND (SP); Similar to S2.	
23	_		J	Δ							STRATUM IIB - SILT/CLAY	/ (CLASS 4B/5B) at ~6 ft / El. 16.3	
DT 12/22	_	_		М	S4	6 to 8	24/17	5-4-3-5	ны – 0.0ppm @34		S4: LEAN CLAY (CL); ~89.9% medium sand; tan-brown; dry.	6 low plasticity fines, 10.1% fine to [GRAIN SIZE TEST PERFORMED].	
2013.G		Ļ		A	0.5	8			STRATUM IIC - SAND (CLASS 3B) at ~8 ft / PID = 0.0ppm @S5 \$5: SILTY SAND (SM); ~80% fine sand, ~20% nonpl		(CLASS 3B) at ~8 ft / El. 14.3		
A TEMPLATE	_	_	40	X	55	to 10	24/14	4-4-5-18			light brown; dry.		
sPJ GEI DAT/	-	_	10	X	S6	10 to 12	24/13	20-26- 30-86	PID = 0.0ppm @S6		S6 (0-3"): SILTY SAND (SM); STRATUM III - TILL (CI S6 (3"-13"): SILTY SAND (SM nonplastic fines, ~10% subary	Similar S5. L ASS 3A) at ~10.5 ft / El. 11.8 J): ~70% fine to medium sand, ~20% gular gravel up to 1.25"; brown; dry.	
5370-ASTORIA COVE.C	10 —	 							Drive casing to 14 feet below grade. Rollerbit to 15 feet.				
T-GRAPHIC LOG 230:			15		S7	15 to 17	24/13	22-19- 19-23			S7: NARROWLY GRADED S, sand, ~10% subangular grave brown; moist. Trace mica.	AND (SP); ~85% fine to medium I up to 1", ~5% nonplastic fines;	
STD 6-NORTH-EAS		Roller bit to 20 feet. Intermittent rig chatter from 15 to 18 feet. Rig chatter / slow advancement from 18 to 20 feet.											
GEI WOBURN (NOTES	S:								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants	

NORT	ORTHING (ft): _221,855 EASTING (ft ROUND SURFACE EL. (ft): _22.3 DATE STAR ERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING C							EASTING (ft):1,003,2	277		BORING
GROU VERT.	ND SL /HORI	jrf Z. i	FACE DATI	: EL. (1 JMS:	ft): <u>22.3</u> NAVD 88	3/NAD83 N	IY Zone 31	DATE START/END: 04 DRILLING COMPANY:	4/20/20 Crai	20 - 4/22/2020 g Geotechnical Drilling	B 9
	//iera				10100 00		IT Zono o n			g ecologinilogi Dhining	PAGE 2 of 5
				S	ample Inf	ormation	-	-	bo		
Elev. (ft)	Dept (ft)	th	Sai N	mple lo.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and F	Rock Description
-	-	4	X	S8	20 to ∖_20.8_ſ	10/3	29- 114/4"	PID = 0.0ppm @S8		S8: WIDELY GRADED GRAV subangular gravel up to 1.5", 4	EL WITH SAND (GW); ~80% ~20% sand; brown to black; wet.
0	-							Roller bit to 25 feet.			
-	2	25	X	S9	25 to 27	24/19	36-64- 80-91	PID = 0.0ppm @S9		S9: WIDELY GRADED SAND ~20% subangular gravel up to	WITH GRAVEL (SW); ~80% sand, 9 1.5"; light brown; wet.
-		/						Roller bit to 30 feet. Slow advancement from 27 to 29 feet.			
-	3 3	0		S10	30 to 32	24/16	36-58- 69-62	PID = 0.0ppm @S10		S10: NARROWLY GRADED s sand, ~10% subangular grave brown; wet.	SAND (SP); ~85% fine to medium el up to 1", ~5% nonplastic fines; light
-10								Roller bit to 35 feet. Rig chatter ~33 and ~34 feet.			
-	- 3 -	5		S11	35 to 36.3	15/9	78-63- 102/3"	PID = 0.0ppm @S11		S11: NARROWLY GRADED \$	SAND (SP); Similar to S10.
	-							Roller bit to 40 feet.			
_	- 4 - -	.0		S12	40 to 42	24/21	58-69- 70-79	PID = 0.0ppm @S12		S12: NARROWLY GRADED fine sand, ~25% subangular g Trace mica.	SAND WITH GRAVEL (SP); ~75% ravel up to 1.25"; light brown; wet.
-20	- - -							Roller bit to 45 feet. Rig chatter from 42 to 44 feet.			
_	- 4	5	$\left \right $	S13	45 to 46.3	15/12	71-114- 102/3"	PID = 0.0ppm @S13		S13: WIDELY GRADED SANI ~25% fie to coarse angular gra mica.	D WITH GRAVEL (SW); ~75% sand, avel up to 1.5"; brown; wet. Trace
NOTES	:								PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370		

NORT	HING (ft)	: _221,85	5			EASTING (ft): 1,003,277 DATE START/END: 4/20/2020 - 4/22/2020			BORING	
		FACE EL.	(ft): 22.3	8/NIAD83 N	IV Zono 31(DATE START/END:	1/20/20	20 - 4/22/2020	B9	
VERI.	/HURIZ.	DATUMS	. NAVD o	D/NADOS N	IT ZONE ST	DRILLING COMPANT:	Crai	g Geolechnical Dhiling	PAGE 3 of 5	
			Sample Inf	formation						
Flev	Denth					Drilling Remarks/	, Log			
(ft)	(ft)	Sample	Depth	Pen./	Blows per 6 in.	Field Test Data	aphic	Soil and	Rock Description	
		No.	(ft)	(in)	or RQD		U U U			
-		~				-				
l	-					Roller bit to 50 feet. Rig				
						chatter from 46 to 48 feet.				
-	F									
	-									
-	1									
-	- 50	X \$14	50	9/9	66-	PID = 0.0ppm @S14		S14: NARROWLY GRADED	SAND (SP); ~90% fine to medium	
	L		to 50.8		103/3"			sand, ~5% subangular grave grayish brown; wet.	l up to 0.75", ~5% nonplastic fines;	
-										
-30 —	-									
	L					Poller hit to 55 feet. Pig				
-	-					chatter from 51 to 52 feet.				
_	-									
	55									
-		X S15	55 to	9/9	83-	PID = 0.0ppm @S15		S15: NARROWLY GRADED	SAND (SP); ~90% fine to medium	
	-		55.8		<u>115/3"</u>	n		brown; wet.		
_										
	+									
	F					Roller bit to 60 feet.				
-						Intermittent rig chatter from				
-	-					55 to 00 leet.				
	- 60					PID = 0.0ppm @S16				
-	-	X S16	60 to	10/10	52- 111/4"			subangular gravel up to 0.75	ND (SW); ~95% sand, ~5% "; tan-brown; wet.	
_	-		60.8							
	L									
-40 —	-									
_	ŀ					Roller bit to 65 feet. Rig				
	L					chatter from 62 to 65 feet.				
-	-									
-	- 65		65		104/4	PID = 0.0ppm @S17		S17: NARROWLY GRADED	SAND WITH SILT AND GRAVEL	
		<u> </u>	to 65.3	4/4	<u>121/4"</u>	1		(SP-SM); ~55% fine to mediu	um sand, ~35% subangular gravel up	
-	F							ເບົາ.ວຸ~າບ% nonplastic fine	s, DIOWII, WEL	
	F									
	1									
-	ŀ					Roller bit to 70 feet.				
	Ļ					65 to 70 feet.				
-	1									
_	- 70	Q 010	70	5/5	120/5"	PID = 0.0ppm @S18		S18: NARROWLY GRADED	SAND (SP); ~90% fine to medium	
	L	310	- to 70.4					sand, ~5% nonplastic fines, [,] brown: wet	~5% fine subangular gravel up 0.5";	
-	-							, 		
	-									
NOTES	S:						PROJECT NAME: Astoria Cove			
							CITY/STATE: Astoria, Queens, New York			
							CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370			

NORT	HING (ft): <u>221,85</u>	5			EASTING (ft): 1,003,2	77		BORING
VERT.	HORIZ.	DATUMS	(π):22.3 NAVD 88	3 8/NAD83 N	Y Zone 31	DATE START/END: 04 DRILLING COMPANY:	Crai	g Geotechnical Drilling	By
	1								PAGE 4 01 5
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	- Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
- - -	- - - - - - - -	× <u>s19</u>	75 to 75.4	5/5	<u>100/5"</u>	Roller bit to 75 feet. Rig chatter from 72 to 75 feet. PID = 0.0ppm @S19		S19: SILTY SAND WITH GR nonplastic to low plasticity fin to 1.5"; grayish brown; wet. 1	AVEL (SM); ~50% fine sand, ~30% es, ~20% fine subrounded gravel up Frace mica.
- - -60 —	- - - - - - - - - - -	S20	80 to 81.4	17/17	32-89- 102/5"	Roller bit to 80 feet. Intermittent rig chatter from 75 to 80 feet. PID = 0.0ppm @S20		S20: SILTY SAND WITH GR	AVEL (SM); Similar to S19.
	- 85	S21	85 to 87	24/19	42-41- 41-27	Roller bit to 85 feet. Intermittent rig chatter from 80 to 85 feet. PID = 0.0ppm @S21		S21: SILTY SAND WITH GR Orange-reddish brown.	AVEL (SM); Similar to S19.
	-					Roller bit to 90 feet.		STRATUM IV - DECOMPO	SED/WEATHERED ROCK (CLASS 88.5 ft / El66.2
	90	S22	90 to 92	24/22	6-9-12- 14	PID = 0.0ppm @S22		S22: SILT (ML); ~90% nonpl ~5% gravel up to 1"; blue to t Decomposed Rock.	astic to low plasticity fines, ~5% sand, olue-green; wet. Trace mica.
-70						Roller bit to 95 feet.			
	- 95 - -	S23	95 to 97	24/17	5-10-10- 9	PID = 0.0ppm @S23		S23: SILT (ML); Similar to S2	21; except white. Decomposed Rock.
	-					Roller bit to 100 feet.			
NOTES:								ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants



BO	RIN	ig in	NFC	RN	IATION	1						BORING	
NO	RTH	IING	(ft):	2	21,877	w. c.			EASTING (ft): 1,003,2	22		DOIVING	
GR		ND S HOP		-AC ראח	с EL. (f 'IMQ·	דן: <u>21.9</u> NA\/D 89	י אראשטאא	V Zone 31(16/20 Cro	J20 - 4/20/2020	B10	
TO			IZ. PTH	(ft)	01113. 85 (MADOS N		DRILLER NAME: Kei	th Par	ent	Вю	
LO	GGE	ED B	Y:	<u>A</u>	Erb / G	6. Holmes			RIG TYPE: <u>CME 75</u>	u		PAGE 1 of 4	
		ING		-OF 	Autom	<u>NC</u>				noh/ (
	GER	⊑R I ₹ I.D.	7P1 /0.[=:).:	NA / I	NA			CASING 1.D./O.D.: _4 DRILL ROD O.D.: NN	<u>ncn/ 4</u> 1		RELIPE: NA RELID/O.D.: NA/NA	
DR	ILLI	NGN	ИЕТ	но	D: M	ud Rotary	Wash						
WA	ATEF	R LE	VEL	DE	PTHS (ft):							
AB	BRE	VIAT	ΓΙΟΙ	NS:	Pen. Rec. RQD WOF	= Penetration = Recovery = Rock Quarts = Length of R = Weight of H = Weight of	on Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stern Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Diameter	
					Sa	ample Inf	ormation			6			
Fle	ΞV	Den	oth				Den (Disus	Drilling Remarks/	c Lo			
(ft	t)	(ft)	Sa	ample	Depth	Rec.	per 6 in.	Field Test Data	aphi	Soil and	Rock Description	
					INO.	(11)	(in)	or RQD		Q	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 21.9	
				$\langle $	S1	0 to	24/13	11-16-		8.8.9	S1 (0-6"): Asphalt.	A) 050/ 5 / 1 / 1 / 1 / 1 / 1	
	4	-		XI		2		10-9			S1 (6"-13"):SILTY SAND (SN nonplastic fines: light brown:	<i>I</i>); ~85% fine to medium sand, ~15% drv.	
20				\mathbb{N}							STRATUM IIA - LOOSE S	SAND (CLASS 6) at ~2 ft / El. 19.9	
20		_		Ń	S2	2	24/15	9-6-7-7			S2: NARROWLY GRADED	SAND WITH SILT (SP-SM); ~90% fine	
	-	_		χI		4					Trace mica.		
	_			\mathbb{N}									
		-		1	S3	4	24/17	4-4-4-2	1-4-4-2		S3: NARROWLY GRADED S	SAND WITH SILT (SP-SM); Similar to	
			5	Υ		to 6				S2.			
~			-	\wedge									
22/23	-	-			S4	6	24/15	4224			S4: NARROWLY GRADED S	SAND WITH SILT (SP-SM); Similar to	
Γ 12/	_	_		VI	54	to 8	24/15	4-2-3-4			S2.		
3.GD				AL		-							
2013	+	-				8			_		S5' NARROWLY GRADED S	SAND WITH SILT (SP-SM). Similar to	
LATE				VI	55	to 10	24/11	4-4-3-3			S5: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar S2.		
EMP		_		\mathbb{N}		10			Drive casing to 9 feet below grade. Rollerbit to 10 feet.				
TA T	-		10			10						SAND (SM): -05% find to modium	
EIDA				VI	S6	to	24/8	3-4-4-6			sand, ~5% nonplastic fines; t	prown; dry.	
0 0]	_		λI		١Z							
10 U	0-	_		/ \					Roller bit to 15 feet				
CO													
ORIA	-	-									STRATUM III - TILL (CLASS 3A) at ~13.5 ft / El. 8.4	
-AST	4	_											
5370													
230	+		15		97	15	21/11	10.26	-		S7: SILTY SAND WITH GRA	VEL (SM); ~60% fine to medium	
FOG	_	_		γl	31	to 17	24/11	18-38			sand, ~25% subangular grav	el up to 0.75", ~15% nonplastic fines;	
PHIC	Ī			\mathbb{N}							aan yay, musi.		
GRA	-	_		' \					-				
AST-													
프	Ì	-							Roller bit to 20 feet. Rig chatter ~16 feet and from				
NOR	4	-							18.5 to 20 feet.				
TD 6-										Ŕ			
NO.	TES	:						1	·	PRO.	ECT NAME: Astoria Cove		
GEI WOBUF											STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants	
I													







ſ	BORIN		NFO	DRN		1				74		BORING
	GROU	IING ND S	i (ft) SUR	: _2 FAC	21,898 E EL. (f	t): 217	,		EASTING (ft): DATE START/END:	/1 5/7/202	20 - 5/7/2020	2011110
	VERT./	HOF	RIZ.	DA		<u>NAVD</u> 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	_ <u>C</u> rai	g Geotechnical Drilling	B11
	TOTAL	.DE	РТН	(ft)	90.0)			DRILLER NAME: Kei	th Par	ent	
	LOGGE	ED E	BY:	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 4
ŀ	DRILL	ING	IN	FOF	RMATIC	ON						
	НАММ	ER '	ГҮР	E:	Autom	atic			CASING I.D./O.D.: 4	inch/ 4	.5 inch CORE BAR	REL TYPE: NA
	AUGEF	R I.D	./0.	D.:	NA / N	NA			DRILL ROD O.D.: NN	1	CORE BAR	REL I.D./O.D.: <u>NA / NA</u>
	DRILLI	NG	ME		D: <u>M</u>	ud Rotary	Wash					
	WATE		VEL		PTH5 (π):						
	ABBRE	evia	TIO	NS:	Pen. Rec. RQD WOR WOH	= Penetrati = Recovery = Rock Qu = Length of R = Weight of I = Weight	on Length / Length ality Designa ⁵ Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter
					Sa	ample Inf	ormation			bc		
	Elev.	De	pth			D ''	Pen./	Blows	Drilling Remarks/	lic Lo	Soil and I	Rock Description
	(ft)	(f	t)	S	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	irapt	Contand I	
						. ,	(in)			0	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.7
	20—	-		M	S1	0 to 2	24/6	41-12-2- 5	PID = 0.0ppm @S1		(0-4"): Asphait. S1:SILTY SAND (SM): ~70% nonplastic fines, ~10% fine to brown; dry. Asphalt / concrete	fine to medium sand, ~20% medium angular gravel up to 1"; dark e fragments.
	_	_		V	S2	2 to 4	24/17	3-54-18- 5	PID = 0.0ppm @S2		S2: WIDELY GRADED SAND ~10% nonplastic fines, ~10%	WITH SILT (SW-SM); ~80% sand, subangular gravel up to 1" tannish
	-	_		Ŵ		7					STRATUM IIA - LOOSE S	AND (CLASS 6) at ~4 ft / El. 17.7
	-	_		\square	S3	4 to	24/11	3-4-4-3	PID = 0.0ppm @S3		S3: WIDELY GRADED SAND) (SW); ~100% sand; tan-brown; dry.
-			5	Ň		6						
GDT 12/22/23				$\left \right\rangle$	S4	6 to 8	24/5	2-2-3-3	PID = 0.0ppm @S4		S4: WIDELY GRADED SAND fines, ~5% subangular up to 1	9 (SW); ~90% sand, ~5% nonplastic "; brown; dry. Some mica.
LATE 2013.	-	_			S5	8 to 10	24/3	2-3-3-7	PID = 0.0ppm @S5		S5: WIDELY GRADED SAND	9 (SW); Similar to S4.
A TEMP	_	_	10	Ŵ		10					STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 11.7
GEI DAT	_	_	10	M	S6	10 to 12	24/11	9-6-6-5	PID = 0.0ppm @S6		S6: WIDELY GRADED SAND ~30% subangular gravel up to	WITH GRAVEL (SW); ~70% sand, o 1.5"; brown; dry. Trace mica.
VE.GPJ	10 —	_		Δ					Drive casing to 9 feet below			
ORIA CC	-	_							grade. Roller bit to 15 feet. Rig chatter from 12 to 14 feet.		STRATUM III - TILL (C	CLASS 3A) at ~13.5 ft / El. 8.2
305370-AST	-	_	15						PID = 0.0ppm @27			
APHIC LOG 2	_			M	S7	15 to 17	24/3	16-21- 16-20	н — о.оррт @S7		S7: WIDELY GRADED SAND ~20% subangular gravel up to tan-brown; dry. Trace mica.	9 WITH GRAVEL (SW); ~75% sand, 5 1.25", ~5% nonplastic fines;
FD 6-NORTH-EAST-GR/	-	-		<u>, </u>					Roller bit to 20 feet. Rig chatter from 16 to 20 feet.			
										PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 230537	York GEI Consultants







Γ	BORIN	IG II	NFC	DRN		1				00		BORING
	NORTH	וואG אח פ	(ft)	: _2 FAC	222,043	†) ∙ 1/1 ⊆	3		EASTING (ft): 1,003,5	93 5/27/2/	120 - 5/28/2020	
,	VERT.		RIZ.	DAI	, L L L. (I TUMS:	NAVD 88	, B/NAD83 N	IY Zone 310	DATE STARTIEND.	Crai	a Geotechnical Drilling	B12
	TOTAL	DEF	РΤΗ	(ft)	: 77.0)			DRILLER NAME: Ed	Flana	gan	212
	LOGGI	ED B	Y:	G	. Holme	s			RIG TYPE: CME 75			PAGE 1 of 4
ŀ		ING	INF	FOF	ΜΔΤΙά	אר						
	HAMM	ER T	YP	E:	Autom	natic			CASING I.D./O.D.: 4	inch/ 4	.5 inch CORE BARI	REL TYPE: NA
	AUGE	R I.D.	./0.1	D.:	NA / I	NA			DRILL ROD O.D.: NN	1	CORE BAR	REL I.D./O.D.: NA / NA
			MET	но	D: <u>M</u>	ud Rotary	Wash					
	WATE	RLE	VEL	. DE	PTH5 (π):						
	ABBR	EVIA ⁻	τιοι	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight of I = Weight of	ion Length / Length ality Designa f Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter	
					S	ample Inf	ormation			bc		
	Elev.	Dep	oth	~			Pen./	Blows	Drilling Remarks/	lic Lo	Soil and F	Rock Description
	(ft)	(ft	t)	S	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	Brapt		
_						. ,	(11)			0	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 14.8
	-	-		X	S1	to 2	24/9	9-4-6-3			(0-2"): Asphait. S1: NARROWLY GRADED G fine angular gravel up to 1.5", Asphalt. Contains 3" seam of s	RAVEL WITH SAND (GP); ~75% ~25% sand; brown and black; dry. sand.
				Х	S2	2 to	9/7	6-50/3"			S2: NARROWLY GRADED S	AND WITH GRAVEL (SP); ~80%
	-	-		\square		2.8			-		brown; dry. Asphalt.	le aliguiai gravei up to 1, black aliu
	_											
		-							Bollor bit to 5 5 foot with			
	10 —	_	5						stabilizer bit.			
c S	_				62	5.5	10/10	24 22 40			S3: NARROWLY GRADED S	AND WITH SILT AND GRAVEL
212212		-		XI	53	to 7	18/10	24-22-18			(SP-SM); ~70% fine to medium	m sand, ~15% nonplastic fines,
DT 12	-	-		4							fragments.	
13.GI				M	S4	to	24/0	15-10-4-			S4: No Recovery.	
E 20	_	-		ΧI		9						
PLAT	-			$\langle \rangle$		-			No recovery Redrive with			
TEM				M	S5	9 to	24/0	4-3-1-1	3-inch split spoon. @S5		S5: NARROWLY GRADED S (SP-SM); ~70% fine to mediur	AND WITH SILT AND GRAVEL m sand, ~15% nonplastic fines,
ATA		-	10	XI		11					~15% angular gravel up to 1";	brown and black; wet. Contains
E E	-			/							gidos dira roots.	
PJ (
OVE.C	-	╞							Advance casing to 15 feet			
A CC	-								below grade. Roller bit to 15 feet.		STRATUM IIC - SAND	(CLASS 3B) at ~13 ft / El. 1.8
STOR												
20-AS	-	F										
3053.	0 —		15									
G SG			10	\backslash	S6	15 to	24/6	4-4-4-3			S6: WIDELY GRADED SAND	WITH GRAVEL (SW); ~85% sand,
IC LC	-	╞		XI		17						о н.о., можн, жон.
APH.	_			$ \rangle$								
T-GF												
I-EAS	-	F							Advance casing to 20 feet			
RTH	_								below grade. Roller bit to 20			
0-NC		F										
ISTD												
SEI WOBURN	NOTES	5:								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL







BOR	RIN(FO							7		BORING			
GRO	UN	ING (ID SI	π): IRF/	221, ACE F	004 L. (ft): 23.5	;		EASTING (π): <u>1,003,42</u> DATE START/END: 5	./ /6/201	20 - 5/6/2020				
VER	Т./Н	HORI	Z. D		IS: _	,0.0 NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B13			
тот	AL	DEP	TH (ft):	102.	0			DRILLER NAME: Keit	h Par	ent				
LOG	GE	D BY	': _	G. Ho	olmes	3			RIG TYPE: CME 75			PAGE 1 of 5			
DRIL	LLI	NG	NFO	DRM/		N									
HAM	ME	ER T	PE:	A	utoma	atic			CASING I.D./O.D.: _4 in	nch/ 4	L5 inch CORE BAR	REL TYPE: NA			
AUG	ER	1.D./	0.D.	: <u>N</u>	IA / N	A Deterry	Weeh		DRILL ROD O.D.:NM		CORE BAR	REL I.D./O.D.: NA / NA			
WAT	ER.		ELI	DEPT	HS (fl	t):	wasn								
ABB	BBREVIATIONS: Pen. = Penetration Length S = Split-Spc Rec. = Recovery Length C = Core Sa RQD = Rock Quality Designation U = Undistur = Length of Sound Cores>4 in / Pen % SC = Spic									Spitt-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measu Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer fallin Undisturbed Sample LL = Liquid Limit 30 incbes to drive a 2-incb-O					
				,	= WOR	Length of = Weight of	Sound Core	es>4 in / Pen	,% SC = Sonic Core DP = Direct Push Sample	Composition PL = Plasticity Limit So increase of an analysis P = Direct Push Sample PID = Photoionization Detector split spoon sample					
					WOH	= Weight o	of Hammer		HSA = Hollow-Stem Auger	1	I.D./O.D. = Inside Diameter/Outside D	iameter			
					Sa	mple Inf	ormation	1		- Bo					
Elev	'. I	Dept	h	Sami	ole	Depth	Pen./	Blows	Drilling Remarks/	hic L	Soil and F	Rock Description			
(11)		(11)		No		(ft)	Rec. (in)	per 6 in. or RQD	Tield Test Data	Grap	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 23.5 S1: SILTY SAND (SM); ~60% fine to medium sand, ~30%				
	+			1 -		0	()		PID = 0.0 ppm @S1	200					
	+	_	$\left \right $	/ s	1	to 2	24/9	/-/-8-12	· · · · · · · · · · · · · · · · · · ·		nonplastic fines, ~10% angula	ar gravel up to 1"; dark brown; dry.			
		-	/			~					ooa nayments.				
	+	-	H		_	2			PID = 0.0ppm @S2		53 29/ SII TV SΔND (SM): 53 29	sand 37.6% nonplastic fines 7.6%			
	+		$ \begin{vmatrix} 52 & t_0 \\ 4 \\ 4 \end{vmatrix} = 24/10 = 2-4-3-6 $			low plasticity clay, ~2.2% fine	subangular to subrounded gravel up								
20-	20-		1/			7					to 1"; tan; dry. [GRAIN SIZE 1	EST PERFORMEDJ.			
20		-	Ļ)		4			9 PID = 0.0ppm @S3						
	-			s	3	to	24/24	8-9-3-9		to medium sand, ~10% nonpl	astic fines; orange-brown; dry.				
			5)			0									
2/23		-	Ĺ)		6			PID = 0.0ppm @S4			AND (CLASS 6) at ~6 ft / El. 17.5 (SW): 00% cond -5% complexite			
12/2	+			s	4	to	24/13	6-2-2-2		fines, ~5% subrounded grave	l up to 0.75"; reddish-brown; dry.				
GDT		-	/			0									
2013		-	Ļ			0			PID = 0.0ppm @S5			$\Delta ND (SD) = 0.5\%$ find to modium			
. LATE	-			/ s	5	to 10	24/16	5-3-2-2			sand, ~5% nonplastic fines; b	rown; dry.			
EMP		-				10									
VTA T	+	- 1	₀⊬		_	10			PID = 0.0ppm @S6			(CLASS 3B) at ~10 π / El. 13.5			
EI D/	+			/ s	6	to	24/24	4-3-7-8			sand, ~5% nonplastic fines; b	rown; dry.			
PJ G		-				12									
VE.G	╞	-	\vdash	1					Drive casing to 10 feet below						
A CO	+								grade. Rollerbit to 15 feet.						
10- 10-		-													
.SP-0	┝	-													
0537	-														
G 23		- 1	°	/ s	7	15 to	24/8	10-8-8-			S7: SILTY SAND (SM); 58.6%	6 sand, 35.6% nonplastic fines, 5.8%			
СГО	┝	-		[]		17		10			TEST PERFORMED].	r, tan-brown, ary. [GRAIN SIZE			
APH	+		V	V											
T-GR		-	F	1											
-EAS	╞	-							Drive casing to 20 feet below		STRATUM III - TILL (CLASS 3A) at ~18.5 ft / FL 5			
NRTH -	-								grade. Rollerbit to 20 feet.						
D 6-N(ļ	-													
	NOTES:									PRO.	IECT NAME: Astoria Cove				
N II											CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370				
U I												Consultants			

NORT	HING (f ND SUI	t): RFA	221,884 CE EL. (ft): 23.5	5		EASTING (ft): 1,003,42	27 /6/202	20 - 5/6/2020	BORING B13
VERT.	/HORIZ	. D4	TUMS:	NAVD 88	3/NAD83 N	IY Zone 31	04 DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 5
			S	ample Inf	ormation			og		
Elev. (ft)	Depth (ft)	n s	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and	Rock Description
-		X	S8	20 to 22	24/15	14-19- 41-27	PID = 0.0ppm @S8		S8: SILTY SAND (SM); ~70% nonplastic fines, ~10% angul reddish-brown; wet.	o fine to medium sand, ~20% ar to subangular gravel; black to
0-	- 						Roller bit to 25 feet. Rig chatter from ~21 and ~24 feet.			
-	— 25 -		S9	25 to 27	24/0	22-16- 20-22	PID = 0.0ppm @S9		S9: No Recovery.	
-	-						Roller bit to 30 feet. Rig chatter from 25 to 27 feet and 28 to 29 feet.			
-	30 - -		S10	30 to 32	24/9	13-22- 15-23	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~10% angular to subro nonplastic fines; brown; wet. \$	SAND (SP); ~85% fine to medium unded gravel up to 1.25", ~5% Some mica.
-10—	- - -						Drive casing to 30 feet below grade. Roller bit to 35 feet. Intermittent rig chatter from 30 to 35 feet.			
-	- 35 -		S11	35 to 37	24/22	41-59- 57-73	PID = 0.0ppm @S11		S11: WIDELY GRADED SAN subangular to subrounded gra	ID (SW); ~95% sand, ~5% avel up to 1.5"; tan-brown; wet.
_	-						Roller bit to 40 feet.			
-	- 40 - 40		S12	40 to 42	24/16	28-28- 19-17	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN	ID (SW); Similar to S11.
-20—	 						Roller bit to 45 feet.			
-	45 	X	S13	45 to 47	24/16	54-84- 112-108	PID = 0.0ppm @S13		S13: SILTY SAND WITH GR nonplastic fines, 18.7% angul brown; wet. [GRAIN SIZE TES	AVEL (SM); 56.3% sand, 25% lar to subrounded gravel up to 1.5"; ST PERFORMED].
NOTES	B:							PROJ	ECT NAME: Astoria Cove	
								CITY/ GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI

NORT	HING (ft): _	221,884	ft) , 23 5			EASTING (ft):	27	20 5/6/2020	BORING	
VERT.	/HORIZ.	DA	TUMS:	NAVD 88	, 3/NAD83 N	Y Zone 31	04 DRILLING COMPANY:	Crai	g Geotechnical Drilling		
		-					1			FAGE 5 01 5	
Elev. (ft)	Depth (ft)	s	S ample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	- Drilling Remarks/ Field Test Data	Graphic Log	Soil and R	ock Description	
-	-	X					Roller bit to 50 feet. Rig chatter from 46 to 50 feet.				
-	- - 50 -	X	S14	50 to 52	24/18	33-46- 59-56	PID = 0.0ppm @S14		S14: WIDELY GRADED SAND fines, ~5% subangular to subro) (SW); ~90% sand, ~5% nonplastic ounded gravel up to 1"; brown; wet.	
-30 —	-						Roller bit to 55 feet. Rig chatter from 53 to 55 feet.				
-	- 55 - -	X	S15	55 to 56.4	17/15	84-65- 105/5"	PID = 0.0ppm @S15		S15: WIDELY GRADED SAND ~10% nonplastic fines, ~5% fin subrounded gravel up to 1"; bro	9 WITH SILT (SW-SM); ~85% sand, le to course subangular to own; wet.	
							Roller bit to 60 feet. Rig chatter from 56 to 60 feet.				
	- 60 - -	X	S16	60 to 60.8	10/7	91- 101/4"	PID = 0.0ppm @S16		S16: SILTY SAND (SM); ~65% nonplastic fines, ~10% subang brown; wet. Trace mica.	fine to medium sand, ~25% ular to subrounded gravel up to 1";	
-40—							Roller bit to 65 feet. Rig chatter from 61 to 64 feet.				
-	- 65 - -	X	S17	65 to 65.3	4/4	108/4"	PID = 0.0ppm @S17		S17: SILTY SAND (SM); Simila	ar to S16.	
-							Roller bit to 70 feet. Intermittent rig chatter from 65 to 70 feet.	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2			
-	- 70	X	S18	70 to 70.8	9/7	63- 104/3"	1 PID = 0.0ppm @S18		S18: NARROWLY GRADED S. fine sand, ~10% nonplastic fine tan-brown; wet. Trace mica.	AND WITH SILT (SP-SM); ~85% s, ~5% subrounded gravel up to 1";	
NOTES	3 :	_	_					PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New Y ROJECT NUMBER: 2305370	fork GEL	

NORTH	HING	(ft):		4	_		EASTING (ft):	27		BORING
VERT.	ND S /HOR	URI IZ.	-ACE EL. DATUMS:	(π): 23.8 NAVD 8) 8/NAD83 N	Y Zone 31	DATE START/END: 04 DRILLING COMPANY:	Crai	g Geotechnical Drilling	B13
							1			PAGE 4 of 5
Elev.	Dep	oth	Camada	Sample Int	formation Pen./	Blows	Drilling Remarks/	nic Log	Soil and F	Rock Description
(ft)	(ft)	No.	(ft)	Rec. (in)	per 6 in. or RQD	Field Test Data	Grap		-
-50 — -50 —		76					Roller bit to 75 feet. Intermittent rig chatter from 71 to 75 feet.			
-		/ 5	× <u>s19</u>	75 to 75.4	5/5	107/5"	PID = 0.0ppm @S19		S19: SILTY SAND WITH GR4 sand, ~20% angular gravel up	WEL (SM); ~60% fine to medium to 1.5"; brown; wet. Trace mica.
-		80					Roller bit to 80 feet. Intermittent rig chatter from 75 to 80 feet.			
-			× <u>s20</u>	80 to 80.3	4/4	<u>100/4"</u>	PID = 0.0ppm @S20 		S20: SILTY SAND WITH GRA	VEL (SM); Similar to S19.
-									STRATUM IV - DECOMPOS 1D) at ~8	SED/WEATHERED ROCK (CLASS 32.7 ft / El59.2
-60 —	-						Roller bit to 85 feet. Rig chatter from 81 to 84 feet.		\ <i></i>	
-		50	S21	85 to 87	24/20	10-16- 20-20	1 PID = 0.0ppm @S21		S21: SANDY SILT (ML); ~809 ~20% fine sand; blue-green to Banded, mica, flaky. Decompo	6 nonplastic to low plasticity fines, o red to tan to gray-black; moist. osed rock.
		20					Roller bit to 90 feet.			
-		90	S22	90 to 92	24/24	5-9-18- 22	עוץ = 0.uppm @S22 -		S22: SANDY SILT (ML); Simil white.	ar to S21, except red to pink and
-70 —		0 <i>F</i>					Roller bit to 95 feet.			
-		90	S23	95 to 97	24/24	9-13-17- 19	PID = 0.0ppm @S23		S23: SANDY SILT (ML); Simil blue-green and tan.	ar to S21, except red to pink,
-	-						Roller bit to 100 feet.			
NOTES:								PROJI CITY/: GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants



BC		IG I	NFO	DRN		1				20		BORING	
GR		ND S	ν (π) SUR	FAC	E EL. (f	t): 21.8	}		DATE START/END: 6	50 5/8/202	20 - 6/9/2020	_ • • • • • •	
VE	RT./	HOI	RIZ.	DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B14	
то	TAL	DE	PTH	l (ft)	75.7	7			DRILLER NAME: Nic	k Bee	hlaer		
LO	GGE	ED E	BY:	A	. Erb / G	6. Holmes			RIG TYPE: CME 55LC			PAGE 1 of 4	
DF HA	RILL MMI	ING ER	i IN TYP	fof E:	Autom	<u>DN</u> natic			CASING I.D./O.D.: 4.1	125 in	ch/ 4.375 inch CORE BAR	REL TYPE: NX Wireline	
AU	GEF	R I.D	./0.	D.:	NA / I	NA			DRILL ROD O.D.: NM		CORE BAR	REL I.D./O.D.: 2.125 inch / 3 inch	
DR		NG			D: <u>M</u>	ud Rotary	Wash						
VV/						ity							
AB	BRE	evia	TIO	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qua = Length of R = Weight of I = Weight of	on Length / Length ality Designa / Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter	
					S	ample Inf	ormation			bo			
Ele (f	ev. t)	De (f	pth t)	S	ample No.	Depth (ft)	Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	traphic L	Soil and I	Rock Description	
						()	(in)	or RQD		0	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.8	
2	-	_		M	S1	0 to 2	24/18	14-18- 14-15			(0-4"):Concrete. S1: NARROWLY GRADED S sand, ~30% nonplastic fines, tan-brown; dry.	AND WITH SILT (SP-SM): ~85% fine ~5% subangular gravel up to 1";	
	_	_	(M	S2	2 to 4	24/20	12-8-9-6	Roller bit to 4 feet.		S2: NARROWLY GRADED S sand, ~5% nonplastic fines, ta	SAND (SP); ~95% fine to medium an-brown; moist.	
	-	_		\square					i-4 i-5	STRATUM IIA - LOOSE S	AND (CLASS 6) at ~4 ft / El. 17.8		
5	_		5	M	S3	4 to 6	24/16	5-4-3-4		S3: NARROWLY GRADED S	AND (SP); Similar to S2.		
13.GDT 12/22/	-	_		M	S4	6 to 8	24/7	4-5-8-5			S4: NARROWLY GRADED S nonplastic fines; brown; moist	AND (SP); ~95% sand, ~5%	
IEMPLAIE 20	-	_		M	S5	8 to 10	24/13	2-2-2-2	Drive casing to 4 feet below grade. Roller bit to 10 feet.		S5: WIDELY GRADED SANE subangular gravel, ~5% nonp mica. STRATUM IIC - SAND) (SW): ~90% sand, ~5% fine lastic fines; brown; moist. Trace (CLASS 3B) at ~10 ft / El. 11.8	
	-	_		\mathbb{N}	S6	10 to 12	24/14	5-5-10- 10			S6: NARROWLY GRADED S to medium sand, ~10% nonpl	AND WITH SILT (SP-SM): ~90% fine astic fines; brown; moist. Trace mica.	
1 10 0 10	0	-		\square					Roller bit to 15 feet. Add EZ mud.				
ASTORIA		_									STRATUM III - TILL (C	CLASS 3A) at ~13.5 ft / El. 8.3	
SKAPHIC LOG ZJUDJIU-A		-	15	X	S7	15 to 17	24/12	17-12- 12-9			S7: WIDELY GRADED SANE ~20% gravel up to 0.5", ~10% mica.) WITH SILT (SW-SM): ~70% sand, onoplastic fines; brown; wet. Trace	
SID 6-NOK IH-EAS I-G	-	_							Roller bit to 20 feet.				
	NOTES:										ECT NAME: Astoria Cove STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL Consultants	







[BORIN		IFC	RN		1				24		BORING	
	GROUI	ND S	(π): URI		E EL. (f	t): 21.9)		EASTING (π): DATE START/END: 5	54 5/11/20	020 - 5/12/2020		
,	/ERT./	HOR	IZ.	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	B15	
•	TOTAL	DEF	тн	(ft):	93.0)			DRILLER NAME: Kei	th Par	ent		
	LOGGI	ED B	Y:	G	. Holme	S			RIG TYPE: CME 75		PAGE 1 of 4		
	DRILL	ING	INF	OF	MATIC	ON							
1	намм	ER T	YP	:	Autom	atic			CASING I.D./O.D.: 4 i	nch/ 4	.5 inch CORE BAR	REL TYPE: NX Wireline	
4	AUGEF	R I.D.	/0.[) .:	NA / N	NA			DRILL ROD O.D.: NM		CORE BAR	REL I.D./O.D.: 2.125 inch / 3 inch	
					D: <u>M</u>	ud Rotary	Wash						
Ľ					.F1113 (ity							
4	ABBRE	EVIAT	101	NS:	Pen. Rec. RQD WOR WOR	= Penetrations = Recovery = Rock Quants = Length of R = Weight of I = Weight of	on Length Length ality Designa Sound Core of Rods of Hammer	ation s>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector LD/D.D. = Inside Diameter/Outside E	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.	
┢					Sa	ample Inf	ormation			D			
	Elev.	Der	oth				Don /	Blows	Drilling Remarks/	c Lo	•	Daals Daa suinti au	
	(ft)	(ft)	Sample		Depth	oth Rec.	per 6 in.	Field Test Data	raphi	Soil and Rock Description		
					NO.	(11)	(in)	or RQD		อ้	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 21.9	
ſ			7	$\langle $	S1	0 to	24/13	14-10-9-	PID = 0.0ppm @S1		(0-4"): Asphalt. S1: SILTY SAND (SM); ~60%	₀ fine to medium sand, ~30%	
	_	-		ХI			nonplastic fines, ~10% subangular gravel up to 1"; brown to						
	20 —			/ \							STRATUM IIA - LOOSE S	SE SAND (CLASS 6) at ~2 ft / El. 19.9	
			Ν	M	S2 2 24/19 7-5-4-4 PD = 0.0ppm @S2		S2: NARROWLY GRADED S to medium sand. ~10% nonp	SAND WITH SILT (SP-SM); ~90% fine lastic fines: vellow-brown: drv.					
	_	-		XI		4					, - 1		
	_			/									
				Λ	S3	4 to	24/8	2-2-3-5	PID = 0.0ppm @S3		S3: NARROWLY GRADED S	SAND (SP); ~95% fine to medium	
	-	_	5	XI		6						an brown, dry.	
23	_			$\langle \rangle$							STRATUM IIC - SAND) (CLASS 3B) at ~6 ft / El. 15.9	
2/22/				$\langle \rangle$	S4	6 to	24/16	11-6-10-	PID = 0.0ppm @S4		S4: NARROWLY GRADED S	SAND (SP); Similar to S3.	
DT		-		XI		8		6					
113.G				$\langle \rangle$							S5: NARROWLY GRADED SAND (SP); ~85% fine to medium sand, ~10% subrounded gravel up to 1 inch, ~5% nonplastic fines yellow-brown; dry.		
TE 2(-			S5	8 to	24/12	10-7-6-3	PID = 0.0ppm @S5				
IPLA.	_	_		χI		10			[—] PID = 0.0ppm @S6				
TEN				\mathbb{N}									
DATA	_	<u> </u>	10	1	S6	10	24/16	4-4-6-3			S6: WIDELY GRADED SAND	WITH SILT (SW-SM); ~90% sand,	
GELL	-	_		Υ	-	to 12					~10% nonplastic fines; yellow	<i>i</i> -brown to orange-brown; dry.	
GPJ				\mathbb{N}									
OVE	10 —	╞		+					Drive casing to 15 feet below	' 🔛			
RIA CO	_	Ļ											
STOR											SIRAIUM III - IILL (JLADD JA) at ~13.5 ft / El. 8.4	
70-A5	-	╞											
3053.	_		15										
<u> 2</u>			.5	\T	S7	15 to	24/10	18-12-			S7: SILTY SAND (SM); 77.39	% sand, 35.6% nonplastic fines, 5.8% I": brown to black: wet IGRAIN SIZE	
IC LC	-	F		XI		17		20-16			TEST PERFORMED].		
RAPH				/									
T-GF		Γ											
I-EAS	_	F							Roller bit to 20 feet.				
RTH													
6-NC	-	F											
I STD	_								<u> </u>				
	NOTES:									PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York			
٥											100LOT HOMBER. 200070	Consultants	

NORT	HING (ft)	: _:	221,889 SE EL (1	H) - 21 0			EASTING (ft): 1,003,53	4	20 - 5/12/2020	BORING
VERT.	./HORIZ.	DA		NAVD 88	/NAD83 N	IY Zone 31	DATE STARTIEND:	Craig Geotechnical Drilling PAGE 2 of 4		
Elev. (ft)	Depth (ft)	s	Sample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	- Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
0-		X	S8	20 to 22	24/3	18-12- 15-14	PID = 0.0ppm @S8		S8: WIDELY GRADED SANI ~30% subangular gravel up t	D WITH GRAVEL (SW); ~70% sand, o 1.5"; brown; wet.
-							Roller bit to 25 feet. Rig chatter from 23 to 25 feet.			
-	- 25 -	X	S9	25 to 27	24/3	20-20- 16-11	PID = 0.0ppm @S9		S9: Gravel stuck in shoe. sul	bangular up to 1.5".
-		/ \					Drive casing to 25 feet below grade. Roller bit to 30 feet. Intermittent rig chatter from 25 to 30 feet.			
- - -10 —	- 30 	X	S10	30 to 32	24/13	24-44- 85-92	PID = 0.0ppm @S10		S10: WIDELY GRADED SAN (SW-SM); ~60% sand, ~30% nonplastic fines; brown; wet.	ID WITH SILT AND GRAVEL subangular gravel up to 1.5", ~10%
-							Roller bit to 35 feet. Rig chatter from 31 to 35 feet.			
	- 35 -	X	S11	35 to 37	24/14	33-35- 45-61	PID = 0.0ppm @S11		S11: WIDELY GRADED SAN subangular gravel up to 0.75	ID (SW); ~95% sand, ~5% "; tan-brown; wet.
							Roller bit to 40 feet. Rig chatter from 38 to 40 feet.			
-	- 40 -	Х	S12	40 to 	11/10	53- 120/5"	PID = 0.0ppm @S12		S12 (0-5"): WIDELY GRADE S12 (5"-10"): SILTY SAND (5 ~20% nonplastic fines, ~10% wet.	D SAND (SW); Similar to S11. SM); ~70% fine to medium sand, o subangular gravel up to 1.5"; brown;
-20 —							Roller bit to 45 feet. Intermittent rig chatter and slow advancement from 41 to 45 feet.			
-	45	~	S13	45 to 45.1	1/1	100/1"	PID = 0.0ppm @S13		S13: SILTY SAND (SM); Sim micaceous sand.	ilar to S12 (5"-10"); black and white;
NOTE	S:	<u> </u>				1		PROJ	ECT NAME: Astoria Cove	
								CITY/ GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants

NORT	HING (ft)):	ft): 21 9)		20 - 5/12/2020	BORING B15			
VERT	./HORIZ.	DATUMS:	NAVD 88	3/NAD83 N	IY Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 3 of 4	
Elev. (ft)	Depth (ft)	Sample No.	ample Information Depth Rec. (ft) (in)		Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description	
-						Roller bit to 50 feet.				
-30 —	50 	S14	50 to 50.2	2/2	100/2"	PID = 0.0ppm @S14		S14: SILTY SAND WITH GR/ sand, ~30% nonplastic fines, orange-brown; wet.	AVEL (SM); ~50% fine to medium ~20% subangular gravel up to 0.75";	
-	-					Roller bit to 55 feet. Rig chatter from 51 to 52 feet.		STRATUM IV - DECOMPO: 1D) at ~;	SED/WEATHERED ROCK (CLASS 52.6 ft / El30.7	
-	- 55 -	S15	55 to 55.9	11/11	54- 100/5"	PID = 0.0ppm @S15		S15: SILTY SAND (SM); ~60' ~35% nonplastic to low plastic 1"; brown; wet.	% fine to medium micaceous sand, city fines, ~5% angular gravel up to	
-	- - - - 60		60			Roller bit to 60 feet.				
-40 —		S16	60 to 62	24/16 23-22- 27-56			1.5"; yellow-brown to brown.			
-	- - - 65	S17	65	1/0	▲ 100/1"	Roller bit to 65 feet. Rig chatter from 62 to 65 feet. PID = 0.0ppm @S17		S17: No Recovery.		
-			65.1	65.1	Roller bit to 70 feet Rig					
-	- 70	S18	70	0/0	103/0"	chatter / slow advancement from 65 to 70 feet. PID = 0.0ppm @S18		S18: No Recovery.		
-50 —			to 70							
NOTE	S:						PROJ	ECT NAME: Astoria Cove	\bigcirc	
							CITY/S GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultant	

NORTHING (ft): 221,889

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,534 DATE START/END: 5/11/2020 - 5/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B15

PAGE 4 of 4

Ī			S	ample Inf	ormation			bc	Soil and Rock Description			
	Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo				
	-						Rollerbit to 70 feet. Rig chatter / slow advancement from 70 to 75 feet.					
	-	— 75 _	S19	75 to 75	0/0	36/0"	PID = 0.0ppm @S19		S19: No Recovery.			
		-		79			Rollerbit to 78 feet. Slow advancement from 75 to 78 feet.					
	- - -60 —	80 	C1	to 83	60/50	0%			grained; banded; completely, valuess, naid, line of median Horizontal fractures at 3", 4", 6", 9", 10", 11-15", 17.5", 19", 21"-24", 27", 29", 31"-33", 35", 36", 39", 41"-44", 45.5" and 48". Core run times per foot (minutes:seconds): 5:05, 5:17, 4:15, 4:52 and 4:16.			
	-	_						1-1	STRATUM V - BEDROCK (CLASS 1B/1C) at ~83 ft / El61.1			
MPLATE 2013.GDT 12/22/23	-	85 	C2	83 to 88	60/54	48%			C2: HARRISON FORMATION; Gneiss; hard; fine to medium grained; banded; slight to moderate weathering; black and white. Horizontal fractures at 0 to 1.5", 5.5", 8", 9", 12", 17.5", 20.5", 24.5", 30.5" 32-37", 30", 43", 43.5", and 48.5". Core run times per foot (minutes:seconds): 6:25, 7:33, 6:56, 6:56 and 6:28.			
70-ASTORIA COVE.GPJ GEI DATA TE	- - -70 —	- - - -	СЗ	88 to 93	60/60	100%			C3: HARRISON FORMATION; Similar C1. Horizontal fractures at 9", 15.5", 20.5", 27.5", 32", 36", 42.5", 50.5", and 56.5". Core run times per foot (minutes:seconds): 5:02, 6:47, 6:45, 6:27 and 4:44.			
0 6-NORTH-EAST-GRAPHIC LOG 230537(- - - -	- - 95 -							Backfilled with cuttings and hole plug. Bottom of boring at depth 93 ft.			
GEI WOBURN ST									PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370			

BORI	NG HINY	INF(ORN		1				499 BORING				
GROU	IND :	SUR	FAC	E EL. (f	t): 21.9)		DATE START/END: 3	/2/202	20 - 3/3/2020	_		
VERT	./HO	riz.	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	B16(OW)		
TOTA	LDE	PTH	l (ft)	: 102	.0			DRILLER NAME: Joe	Schu	ster			
LOGG		5Y:	_ <u>G</u>	. Holme	S			KIG TYPE:	PAGE 1 of 5				
DRIL	LINC	g in	FOF	RMATIC	<u>NC</u>								
HAMN	IER	TYP	E:	Autom	atic			CASING I.D./O.D.: _4 i	nch/4	.5 inch CORE BAR	RREL TYPE: NX Wireline		
DRILL	R I.L ING)./O. ME	d.: ГНО	<u>NA/I</u> D: M	<u>vA</u> ud Rotarv	Wash		DRILL ROD O.D.:		CORE BAN	REL I.D./O.D.: 2.125 inch / 3 inch		
WATE	RL	EVEI	L DE	PTHS (ft): _⊻ 1	7.3 3/10/2	2020 7:10 a	m in observation well					
	=\///			Pop	- Popotrati	on Longth		S - Split Spoon Sample		On - Pocket Penetrometer Strength	NA NM - Not Applicable Not Measured		
ABBR		the	NO.	Rec. RQD WOF WOF	= Recovery = Rock Qu = Length of R = Weight	 Length ality Designation Sound Core of Rods of Hammer 	ation es>4 in / Pen	C = Core Sample U = Undisturbed Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		SV = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Diameter		
				S	ample Inf	ormation							
Flev	De	epth				Dan /	Blaura	Drilling Remarks/	c Lo				
(ft)	(ft)	ft)	S	ample No	Depth	Rec.	per 6 in.	Field Test Data	raphi	Soil and Rock Description STRATUM I - FILL at ~0 ft / El. 21.9			
				NO.	(11)	(in)	or RQD		Ō				
			\mathbb{N}	S1	0 to	9/0	24-44/3"	PID = 0.0ppm @S1		S1 (0"-2"): Asphalt. S1 (2"-12"): Concrete.			
-					0.8					(). Sources			
20 —			\square										
			M	S2	2 to	24/7	19-10- 13-8			S2: SILTY SAND WITH GRA sand, ~30% subangular grav	VEL (SM); ~50% fine to medium rel up to 1", ~20% nonplastic fines;		
-	1		XI.		4					brown; dry. Brick, asphalt and	d concrete fragments.		
-	Ļ		\square					PID = 0.000m @S3		STRATUM IIC - SANE	D (CLASS 3B) at ~4 ft / El. 17.9		
			M	S3	4 to	24/12	4-4-4-5			S3: NARROWLY GRADED sand, ~10% low-plasticity fin	SAND (SP); ~85% fine to medium es, ~5% fine angular gravel up to 0.5";		
-	1	5	X		6					light brown to reddish-brown	; dry.		
2/23	Ļ		\square					PID = 0.0 ppm $@S4$					
12/2			M	S4	to	24/10	5-7-4-5	1 10 - 0.0ppin @04		sand, ~5% low-plasticity fine	s; light brown to reddish-brown; dry.		
GDT			M		8								
. 2013	+		()		0			PID = 0.0ppm @S5			/ find to modium cond ~25%		
ATE			M	S5	to	24/8	5-4-9-12			nonplastic fines; brown; dry.			
EMP	F		M		10			Drive 8LF casing. Roller bit to 8 feet.					
- TAT	-	10	$\left(\right)$		10			PID = 0.0ppm @S6		STRATUM III - TILL (CLASS 3A) at ~10 π / El. 11.9		
			M	S6	to	24/15	11-30- 19-26	e coppin Geo		nonplastic fines, ~10% angular g	lar gravel up to 1"; brown; dry.		
PJ G	F		M		12								
0 9 10 –	╞		\mathbb{H}										
A CO													
TOR	ľ							Rig chatter from 13 to 14 feet. Rollerbit					
70-AS	╞			C1	14	10/0	00/	PID = 0.0ppm @C1	Ŕ	C1: No Recovery. Roller bit v	vas grinding and not advancing. Beain		
. 3053		15	Ц	UI.	to 15 .	12/0	0%			coring. After 1' core barrel ad	vanced quickly. Likely cobble. Core econds): 2:04.		
06 2		10			/								
	+		+	<u>87</u>	16	21/12	8-21-22	Rig chatter from 18 to 20		S7: SILTY SAND WITH GRA	VEL (SM); ~60% fine to medium		
RAPI	Ļ		IV.	51	to 18	24/12	22			sand, ~30% nonplastic fines, Crushed rock/cobble @11"-1	~10% subangular gravel up to 1.5"; 2"; brown; moist.		
ST-G											. ,		
- H-E∕	†		H					4					
NOR -	4												
TD 6-													
	⊣ S:					1	1	·	PROJ	ECT NAME: Astoria Cove			
GEI WOBUF									CITY/ GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants		
I													

NORTHING (ft): 221,904 EASTING (ft): 1, GROUND SUBFACE FL (ft): 24.0 DATE START(FN)									9	0 2/2/2020	BORING		
VERT.	/HO	ORIZ.	DA	FUMS:	NAVD 88	, 3/NAD83 N	IY Zone 31	DATE STARTIEND. 3	Crai	g Geotechnical Drilling	BAGE 2 of 5		
				s	ample Inf	ormation			5				
Elev. (ft)	Depth (ft)	epth (ft)	epth (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Loo	Soil and Ro	ock Description
	-	/	M	S8	20 to 22	24/8	12-13- 15-14	PID = 0.0ppm @S8		S8: NARROWLY GRADED SAN fine to medium sand with mica fi subrounded gravel up to 1.25", - black; wet.	ND WITH GRAVEL (SP); ~75% lakes, ~20% angular to ~5% nonplastic fines; brown to		
-	-							Rollerbit to 25 feet. Rig chatter from 22 to 25 feet.					
-		25	S9 25 24/14 30-16- 27 16-15 PID = 0.0ppm @S9	PID = 0.0ppm @S9		S9: NARROWLY GRADED SAND WITH GRAVEL (SP); SIr S8; except crushed rock powder and fragments @0-6".							
-			/ \					Drive casing to 18 feet due to fall in. Rollerbit to 30 feet. Rig chatter from 25 to 26 feet.					
- - -10 —		30	X	S10	30 to 32	24/22	28-63- 69-101	PID = 0.0ppm @S10		S10: NARROWLY GRADED SA sand, ~5% angular to subround nonplastic fines; brown; wet.	AND (SP); ~90% fine to medium ed gravel up to 0.75", ~5%		
-								Roller bit to 35 feet.					
-		35	X	S11	35 to 36.4	17/13	64-47- 103/3"	PID = 0.0ppm @S11		S11: NARROWLY GRADED SA	AND (SP); Similar to S10.		
-	-							Roller bit to 40 feet.					
-20		40	X	S12	40 to 40.4	∖5/4	106/5"	PID = 0.0ppm @S12		S12: NARROWLY GRADED SAND WITH CLAY (SP-SC); ~85 fine to medium sand, ~10% low plasticity clayey fines, ~5% angular to subrounded gravel up to 0.75"; brown; wet.			
-	-							Roller bit to 45 feet. Rig chatter at 41 feet.					
-	 	45		S13	45 to 47	24/19	60-64- 70-31	PID = 0.0ppm @S13		S13: WIDELY GRADED GRAVI (GW-GM); 47.7% angular to sub sand, 7.1% nonplastic fines; bro	EL WITH SILT AND SAND brounded gravel up to 2", 45.2% wn; wet. [GRAIN SIZE TEST		
NOTES	S:	_	_	_					PROJI CITY/S GEI PI	ECT NAME: Astoria Cove STATE: Astoria, Queens, New Yo ROJECT NUMBER: 2305370			

NORT	HING (1	t):	221,904				EASTING (ft): 1,003,4	99		BORING
GROU VERT.	IND SU ./HORIZ	rfa d/	ACE EL. (ATUMS:	π):21.9 NAVD 88) 3/NAD83 N	Y Zone 31	DATE START/END: D4 DRILLING COMPANY:	3/2/202 Crai	g Geotechnical Drilling	B16(OW)
										PAGE 3 of 5
Elev. (ft)	Depti (ft)	ו	Sample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or ROD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-							Roller bit to 50 feet.		PERFORMED].	
- -30 —	50 50 		S14	50 to 52	24/20	37-48- 63-58	PID = 0.0ppm @S14		S14: NARROWLY GRADED sand, ~5% angular to subrou	SAND (SP); ~95% fine to medium inded gravel up to 0.5"; brown; wet.
-	- 55						Roller bit to 55 feet.			
-	0:		S15	55 to 57	24/21	44-33- 84-86	PID = 0.0ppm @S15		S15 (0-10"): NARROWLY G S15: (10"-21") SILTY SAND 30.5% nonplastic fines, 20.2 1"; brown; wet. [GRAIN SIZE	RADED SAND (SP); Similar to S14. WITH GRAVEL (SM); 49.3% sand, % angular to subrounded gravel up to TEST PERFORMED].
-40	60 60		<u>S16</u>	60 to 60.4	<u>5/5</u>	<u>104/5"</u>	Roller bit to 60 feet. Rig chatter from 58 to 60 feet. PID = 0.0ppm @S16		S16: SILTY SAND (SM); ~60 ~10% angular to subrounded mica.)% fine sand, ~30% nonplastic fines, d gravel up to 1"; brown; wet; trace
	 65 65	;	S17	65 to 67	24/24	55-37- 42-32	Rollerbit to 65 feet. Rig chatter from 61 to 65 feet. PID = 0.0ppm @S17		S17: WIDELY GRADED SAN fines; orange to brown; wet.	ND (SW); ~95% sand, ~5% nonplastic
-50	- - - - - - -	,	S18	70 to 72	24/16	16-34- 79-102	PID = 0.0ppm @S18		S18 (0-11"): SILT WITH SAN plasticity fines, ~20% fine sa orange-brown; dry. S18 (11"-16"): SILTY SAND ~20% nonplastic fines, ~5% yellow-brown; wet.	ND (ML); ~75% nonplastic to low nd, ~5% fine angular gravel up to 0.5"; (SM); ~75% fine to medium sand, fine subangular gravel up to 0.5";
NOTES	S:							PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

NORT GROU VERT	HING (ft) ND SUR /HORIZ.	: <u>221,904</u> Face el. (1 Datums:	ft): <u>21.9</u> NAVD 88) 3/NAD83 N	IY Zone 310	EASTING (ft): 1,003,4 DATE START/END:	.99 3/2/202 Craig	0 - 3/3/2020 g Geotechnical Drilling	BORING B16(OW) PAGE 4 of 5
Elev. (ft)	Depth (ft)	Sample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-	 	S19	75 to 77	24/19	35-38- 49-69	PID = 0.0ppm @S19		STRATUM IV - DECOMPO ft S19: SANDY SILT (ML); ~65 fines, ~30% fine sand, ~10% rock, interbedded white layer varved; purple to gray; moist;	SED/WEATHERED ROCK at ~73.5 ; / El51.6 % nonplastic to low plasticity silty gravel angular up to 0.5"/ crushed s of crushed rock and powder; silt trace mica. Decomposed Rock.
- - -60 —	80 	S20	80 to 82	24/17	26-27- 35-74	⁻ PID = 0.0ppm @S20 -		S20: SANDY SILT (ML); ~70 sand, ~5% fine gravel up to 0 varved; moist; trace mica. De	% nonplastic silty fines, ~25% fine .5"; bluish green to green to brown; composed Rock.
-	- 85 - 85	S21	85 to 87	24/24	11-21- 27-47	PID = 0.0ppm @S21		S21: SILT (ML); ~90% nonpl fine sand, light gray to black; Decomposed Rock.	astic to low plasticity silty fines, ~10% varved; moist; trace mica.
- - -70 —	90 90	S22	90 to 92	24/15	8-14-21- 37	PID = 0.0ppm @S22		S22: SILT (ML); ~80% low pl light gray to bluish gray; mois	asticity silty fines, ~20% fine sand, st. Decomposed Rock.
-	95 95 	S23	95 to 97	24/24	13-25- 31-35	PID = 0.0ppm @S23		S23 (0-18"): SILT (ML); Simil S23 (18"-24"): SILT (ML); ~9 sand; light gray; moist. Decor	lar to S22. Decomposed Rock. 0% low plasticity fines, ~10% fine nposed Rock.
NOTE	s:	<u>ı</u>	1	1			PROJI CITY/S	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	


B	ORIN	IG IN	NFC	DRN	ATION	1						BORING
N		ווNG אח פ	(ft) UP	: <u>2</u> FAC	21,948	+) ∙ 01 ⊑	;		EASTING (ft): 1,003,37	(9 (/11/20	120 - 6/12/2020	
	ERT /	AD 2 HOB			, E EL. (T TUMS:	NAVD 88) 3/NAD83 N	Y Zone 310		Crai	g Geotechnical Drilling	B17
Т	OTAL	DEF	" <u>-</u> . РТН	(ft)	: 75.3	3		1 2010 010	DRILLER NAME: Nic	k Beeł	nlaer	ВП
L	OGGE	ED B	Y:	A	Erb / G	. Holmes			RIG TYPE: CME 55LC			PAGE 1 of 4
₽	RILL	ING		= OF -	RMATIC	<u>NC</u>						
		חופ	191 /O I	ב: הי					CASING I.D./O.D.:	125 ind	CORE BAR	
	RILLI	NG N	/0.1 //ET	л ТНО	D: M	ud Rotary	Wash					
v	VATE	R LE	VEL	. DE	PTHS (ft):						
H												
	DDR			NO:	Rec.	= Penetration = Recovery = Rock Quarts = Length of	Length Length ality Designa Sound Core	ation es>4 in / Pen	C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core		QP = Pocket Torvane Shear Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit	Blows per 6 in: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
	WOK – Weight of Rods DF = Direct Push S WOH = Weight of Hammer HSA = Hollow-Ster										I.D./O.D. = Inside Diameter/Outside Di	ameter
	Sample Information									bo		
E	lev.	ev. Depth Blows Drilling Remarks									Soil and F	Rock Description
	(ft)	(ft)	5	ampie No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	irapt		
							(in)	or RQD		0	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.5
	_										(0-1'): Concrete.	(CLASS 3B) at ~1 ft / EL 20 5
		-				1	40//0	00.40	PID = 0.0ppm @S1		S1: SILTY SAND WITH GRAV	/EL (SM): ~60% fine sand ~20%
	20 —			Ж	51	to 2	12/10	23-16	_		nonplastic fines, ~20% angula	r gravel up to 1.5"; brown; dry.
		-		$\overline{)}$	S2	2	24/24	11-9-7-	PID = 0.0ppm @S2		S2: NARROWLY GRADED S/	AND (SP); ~90% fine to medium
				Υ		to 4		13	Poller hit to 4 feet below		sand, ~5% nonplastic fines, ~{	5% fine gravel; brown to
	-	-		\mathbb{N}					grade.		orange-brown, dry.	
		_	_	$\left(\right)$	00	4	04/40	6670	PID = 0.0ppm @S3		S3(0-7"): NARROWLY GRAD	ED SAND (SP): Similar to S2.
	-	- 5		VI	53	to	24/13	5-5-7-9			S3(7"-13"): NARROWLY GRA	DED SAND (SP); ~95% fine to
			5	Å		U					medium sand, ~5% fine grave	r; white to tan; dry.
2123		_		(PID = 0.0 nnm @S1			
7771	_			M	S4	6 to	24/8	11-14-			S4: NARROWLY GRADED S/	AND (SP); Similar to S3 ($7^{-13^{-13}}$).
<u>n</u>		_		XI		8		14-17	Roller bit to 8 feet below			
013.0	-			$\langle \rangle$					grade.			
Ц	_			$\backslash /$	S5	8 to	24/10	5-4-7-8	PID = 0.0ppm @S5		S5: SILTY SAND (SM); ~70%	fine sand, ~30% nonplastic fines;
A LA		_		XI		10			Drive casing to 10 feet below		tan, dry. Trace mica.	
	-			\mathbb{N}					grade. Roller bit to 10 feet.			
AIA			10		S6	10	24/10	13-9-11-	PID = 0.0ppm @S6		S6: NARROWLY GRADED S/	AND WITH SILT (SP-SM); ~85% fine
- - - - -		_		Y		to 12	2.7.0	11			to medium sand, ~10% nonpla brown dry Trace mica	astic fines, ~5% fine gravel; tan to
- L	10 —			\mathbb{N}								
VE.C		-		$^{\prime}$					Roller bit to 15 feet.			
D D D	-											
1 N O N		_									STRATUM III - TILL (0	CLASS 3A) at ~13.5 ft / El. 8
PA-]	_										
1/201	-											
231		_	15		67	15	21/15	31 /7	PID = 0.0ppm @S7		S7: NARROWLY GRADED SA	AND (SP); ~95% fine to medium
PO-	-	_		VI	31	to 17	2 4 /10	25-20			sand, ~5% fine gravel; brown t	o black; dry. Black micaceous sand
	_	_		\mathbb{N}		.,					wi -14 .	
IKA!		_		/ \					-			
)-I.S.	-											
μ́		-							Drive casing to 15 feet below			
NO1	-	_										
4-0	_											
	0								<u> </u>			
N	OTES	: Sa	Imp	ie I[J S12 w	as skippe	a during dr	illing.		PROJ	ECT NAME: Astoria Cove	
0M										CITY/STATE: Astoria, Queens, New York		
IJ.										GEI PROJECT NUMBER: 2305370		







BC		IG IN	FO	RN		1				25		BORING
NC GF	JK [] ROUI	11NG ND SI	(π): JRF	_2 AC	21,937 E EL. (f	t): 20.6	;		EASTING (ft): DATE START/END:	35 5/8/202	20 - 5/8/2020	
VE	RT./	HOR	Z. C	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	B18
тс	DTAL	DEP	ТΗ	(ft):	100	.2			DRILLER NAME: Keit	th Par	ent	
LC	GGE	ED BI	/ :	G.	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 5
DF	RILL	ING	INF	OR	MATIO	ON						
HA	мм	ER T	YPE	:	Autom	atic			CASING I.D./O.D.: 4 i	nch/ 4	.5 inch CORE BARI	REL TYPE: NA
AL	JGEF	R I.D./	O.D	.:	NA / I	NA			DRILL ROD O.D.: NM		CORE BAR	REL I.D./O.D.: NA / NA
	RILLI ATER	NG N RIE\	IETI /EI		D: <u>M</u> PTHS (ud Rotary ft)·	Wash					
AE	BRE	eviat	ION	IS:	Pen. Rec. RQD WOF WOF	= Penetrations = Recovery = Rock Quants = Length of R = Weight of H = Weight of	on Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen.	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Di	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter
					S	ample Inf	ormation		-			
FI									Drilling Remarks/	c Lo		
(1	ft)	(ft)		Sa	ample	Depth	Pen./ Rec.	per 6 in.	Field Test Data	aphi	Soil and F	Rock Description
	NO. (TT) (in) or RQD									Δ	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 20.6
	20		N		S1	0 to	24/12	6-3-2-15	PID = 0.0ppm @S1		S1: WIDELY GRADED SAND	WITH GRAVEL (SW); ~65% sand,
2		_	ľ	XI		2					fines; brown to orange-brown;	dry.
	-		/	$\langle \rangle$							STRATUM IIC - SAND	(CLASS 3B) at ~2 ft / El. 18.6
	_	_	5	$\left(\right)$	S2	2 to 4	24/11	7-5-7-8	PID = 0.0ppm @S2		S2: NARROWLY GRADED S. sand, ~10% angular to suban fines; brown to dark brown; dr	AND (SP); ~85% fine to medium gular gravel up to 1", ~5% nonplastic /.
		-		+	62	4	24/6	4970	PID = 0.0ppm @S3		S3: NARROWLY GRADED S	AND (SP): ~100% fine to medium
23	_				53	to 6	24/6	4-8-7-9			sand; tan-brown; dry.	
3.GDT 12/22/	_	_			S4	6 to 8	24/8	16-16-9- 14	1 PID = 0.0ppm @S4		S4: NARROWLY GRADED S, sand, ~5% nonplastic fines; ta	AND (SP); ~95% fine to medium n-brown; dry.
IEMPLAIE 201	_	_	10	$\left(\right)$	S5	8 to 10	24/5	14-12-8- 8	PID = 0.0ppm @S5 Drive casing to 10 feet below grade. Roller bit to 10 feet.	,	S5: NARROWLY GRADED S, to medium sand, ~10% nonpla subrounded gravel up to 1.5";	AND WITH SILT (SP-SM); ~80% fine astic fines, ~10% subangular to brown to reddish-brown; dry.
	0	1 -		0	\langle	S6	10 to 12	24/2	12-10- 10-24	PID = 0.0ppm @S6		S6: CLAYEY SAND (SC); ~70 plasticity fines, ~10% subangutan to black; moist. Some mice
05370-AS IORIA COVE.6		_							Roller bit to 15 feet.			
EASI-GRAPHIC LUG 20		1 - -	5 4		S7	15 to 15.3	4/0	<u>100/4"</u>	Drive casing to 15 feet below		S7: No Recovery.	
VOBURN STD 6-NOKIH	NOTES:									PROJ CITY/	ECT NAME: Astoria Cove	York
GEI										GEI P	ROJECT NUMBER: 2305370	

NORTH	ORTHING (ft): 221,937 EASTING (ft) COUND SURFACE EL. (ft): 20.6 DATE STAIL RT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING (ft)						EASTING (ft):1,003,53	35		BORING
GROU VERT.	ND SU /HORI/	rf# Z. d.	ACE EL. ATUMS:	(ft): <u>20.6</u> NAVD 88	3 3/NAD83 N	IY Zone 31	DATE START/END: 5 04 DRILLING COMPANY:	/8/202 Crai	g Geotechnical Drilling	B18
		_								PAGE 2 of 5
			5	Sample Inf	ormation	1	-	- Bo		
Elev. (ft)	Dept (ft)	h	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic I	Soil and	Rock Description
0	-		S8	20 to 22	24/14	5-7-6-5	PID = 0.0ppm @S8		S8: SILTY SAND (SM); ~70% ~20% nonplastic fines, ~10% to 1"; black; wet.	6 fine to medium micaceous sand, 5 subangular to subrounded gravel up
-	-						Drive casing to 20 feet below grade. Roller bit to 25 feet. Rig chatter from 22 to 24 feet.	, , , , , , , , , , , , , , , , , , ,	STRATUM III - TILL (0	CLASS 3A) at ~23.5 ft / El2.9
	- 2: -	5	S9	25 to 27	24/6	88-42- 52-23	PID = 0.0ppm @S9		S9: WIDELY GRADED SANI ~30% subangular to subroun Trace mica.	D WITH GRAVEL (SW); ~70% sand, ided gravel up to 1.5"; brown; wet.
-	- 	/					Rollerbit to 30 feet. Rig chatter from 25 to 30 feet.			
- -10 —	3 -) =	<u>S10</u>	30 to 30.1	1/0	100/1"	PID = 0.0ppm @S10		S10: No Recovery.	
-	- - - - 3: -	5	S11	35 to 37	24/12	34-34- 31-37	Drive casing to 25 feet below grade. Roller bit to 35 feet. Rig chatter from 30 to 35 feet. PID = 0.0ppm @S11	/ 	S11: WIDELY GRADED SAN ~20% subangular to subroun	ND WITH GRAVEL (SW); ~80% sand, ided gravel up to 1"; light brown; wet.
-							Roller bit to 40 feet.			AVEL (SM): 52.0% sand 32%
-20 —	- - - - -		<u>S12</u>	40.2	2/2	<u> 100/2" </u>	Roller bit to 45 feet. Rig chatter from 42 to 45 feet.		subangular to subrounded gr fines; brown; wet. [GRAIN SIz	zvel up to 1.5"; 15.1% nonplastic ZE TEST PERFORMED].
	- 4	5	S13	45 to 47	24/16	37-44- 56-53	PID = 0.0ppm @S13		S13: WIDELY GRADED SAN subrounded gravel up to 1"; t	ND (SW); ~95% sand, ~5% an-brown; wet.
NOTES	S:							PROJ CITY/S GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

NORT	NORTHING (ft): 221,937 EASTING (ft) GROUND SURFACE EL. (ft): 20.6 DATE STAF /ERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING (ft)						35		BORING
VERT.	ND SUR /HORIZ.	DATUMS	. (π): <u>20.6</u> : NAVD 8	5 8/NAD83 N	IY Zone 31	DATE START/END: D4 DRILLING COMPANY:	5/8/202 Crai	g Geotechnical Drilling	B18
								<u> </u>	PAGE 3 of 5
Elev. (ft)	Depth (ft)	Sample No.	Sample Inf	formation Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and F	Rock Description
-	-					Roller bit to 50 feet. Rig chatter from 49 to 50 feet.			
- -30 — -	- - - 50 -	S14	50 to 51.2	14/14	49-76- 101/2"	PID = 0.0ppm @S14		S14: NARROWLY GRADED fine to medium sand, ~10% n subrounded gravel up to 0.5";	SAND WITH SILT (SP-SM); ~85% onplastic fines, ~5% fine angular to tan; wet.
-	- - - - - - - - - - - - - - - - - - -					Roller bit to 55 feet.			
	55 - -	× <u>s15</u>	55 to 55.3	4/4	<u>111/4"</u>	PID = 0.0ppm @S15		S15: NARROWLY GRADED to S14.	SAND WITH SILT (SP-SM); Similar
-	- - - - 60					Roller bit to 60 feet. Rig chatter from 57 to 59 feet.			
-40	- - -	S16	60 to 60.9	11/7	52- 100/5"	PID = 0.0ppm @S16		S16: SILTY SAND (SM); ~60 ^c nonplastic fines, ~5% fine sub 0.5"; brown; wet.	% fine to medium sand, ~35% angular to subrounded gravel up to
-	- - - - 65		65			Roller bit to 65 feet. Rig chatter from 64 to 65 feet. PID = 0.0ppm @S17			D W/ITH SII T (SW/_SM): ~85% sand
- -	-	\$17	to 67	24/21	37-56- 44-65			~10% nonplastic fines, ~5% a 1.5"; orange-brown; wet. Trac	ingular to subrounded gravel up to e mica.
-	-					Roller bit to 70 feet.		STRATUM IV - DECOMPOS	SED/WEATHERED ROCK (CLASS 58.5 ft / El47.9
-50 — _	- /0 - -	S18	70 to 72	24/19	36-56- 87-67	PID = 0.0ppm @S18		S18: SANDY SILT (ML); ~60% ~35% fine to medium sand, ~ orange-brown to red-brown to rock.	% nonplastic to low plasticity fines, 5% angular gravel up to 1"; blue-gray; moist. Mica. Decomposed
NOTES:								ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants





	BORIN	IG I	NF	ORN		1						BORING
	NORTH	HINC ND 9	s (ft) SUP): <u>2</u> FAC	21,997 EFI /	it): 21.4			EASTING (ft): DATE START/END.	598 6/10/20	020 - 6/11/2020	
	VERT./	HOI	RIZ.	DA1	(I TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B19
	TOTAL	DE	PTH	l (ft)	75.4	4			DRILLER NAME: Ni	ck Bee	hlaer	
	LOGGI	ED E	BY:	_J.	Passno	o / G. Holn	nes		RIG TYPE: CME 55LC	2		PAGE 1 of 4
┢	DRILI	ING) IN	FOF	RMATIC	ON						
	HAMM	ER	TYP	E:	Autom	natic			CASING I.D./O.D.: 4	.125 in	ch/ 4.375 inch CORE BAR	RREL TYPE: NA
	AUGE	R I.D)./O.	D.:	NA / I	NA			DRILL ROD O.D.: N	M	CORE BAR	RREL I.D./O.D.: NA / NA
	DRILLI	NG	ME	тно	D: <u>M</u>	ud Rotary	Wash					
	WATE		:VE		PTHS (π):						
	ABBRE	EVIA	TIO	NS:	Pen. Rec. RQD WOF WOF	= Penetration = Recovery = Rock Quant = Length of R = Weight of H = Weight of	on Length Length ality Designa Sound Core of Rods of Hammer	ation s>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Diameter
					S	ample Inf	ormation			bc		
	Elev.	De	pth	_			Pen./	Blows	Drilling Remarks/	lic Lo	Soil and	Rock Description
	(ft)	(f	ft)	S	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	iraph		
						()	(in)	or RQD		0	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 21.4
	_ 20 —				S1	0 to 0	0/0	50/3"	Roller bit through asphalt. Roller bit through concrete slab at 3 feet. @S1		S1: No Recovery. Drilled ind hole 4 feet to the south.	icated second concrete slab, offset
	-								Roller bit through slab to 4 feet.	- পা		
	-			A		to to	5/5				(GP-GM); ~75% fine angular	gravel up to 1.5", ~15% fine sand,
	_	-	5			4.4			Roller bit to 6 feet. Drill	~10% nonplastic fines; black	and gray; wet.	
2/23		_		\square		-						Y (CLASS 4B/5B) at ~6 ft / El. 15.4
3.GDT 12/2		_		M	S3	6 to 8	24/14	5-4-6-10	chatter from 4 to 6 feet.		S3: SILT WITH SAND (ML); sand; tan; dry. 3" seam of na	~85% nonplastic tines, ~15% tine rrowly graded sand above silt.
2013		-		$\left(\right)$		8					STRATUM IIC - SAN	
, TEMPLATE	-	_		M	S4	to 10	24	7-6-10- 19			fine to medium sand, ~15% f	ine angular gravel up to 1.5"; tan; dry.
GPJ GEI DATA	- 10		10	M	S5	10 to 12	24/12	20-10- 10-10	Drive casing to 10 feet. Roller bit to 10 feet.		S5: SILTY SAND (SM); ~859 tan; moist.	% fine sand, ∼15% nonplastic fines;
ASTORIA COVE	_	_	15								STRATUM III - TILL (CLASS 3A) at ~13.5 ft / El. 7.9
AST-GRAPHIC LOG 2305370-#				X	S6	15 to 17	24/12	17-14- 14-13	Roller bit to 15 feet. Drive casing to 15 feet.		S6: NARROWLY GRADED to medium sand, ~10% nonp	SAND WITH SILT (SP-SM); ~90% fine lastic fines; brown; wet.
GEI WOBURN STD 6-NORTH-E.										PRO. CITY. GEI F	JECT NAME: Astoria Cove /STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI



STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT





				4			EASTING (#): 1 003 42	6		BORING		
GROU	ND SUR	FACE	E EL. (f	t):20.2			DATE START/END:	.0 /9/202	20 - 6/9/2020			
VERT.	HORIZ.	DAT	UMS:	NAVD 88	8/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B20		
TOTAL		l (ft):	75.1				DRILLER NAME: Nick	Beel	nlaer			
LOGG	ED BY:	G.	Holme	S			RIG TYPE: CME 55LC			PAGE 1 of 4		
DRILL	ING IN	FOR	ΜΑΤΙΟ	<u>NN</u>								
HAMM	IER TYP	E: _	Autom	atic			CASING I.D./O.D.: 4.1	25 in	ch/ 4.375 inch CORE BAR	REL TYPE: NA		
	R I.D./O.	D.:	<u>NA/1</u>	NA ud Potany	Wash		DRILL ROD O.D.:NM		CORE BAR	REL I.D./O.D.: <u>NA / NA</u>		
WATE	R LEVE		7. <u>111</u> PTHS (1	ft):	wasn							
			-									
ABBR	EVIATIO	NS:	Pen. Rec.	= Penetratio = Recovery	on Length Length		S = Split-Spoon Sample C = Core Sample		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling		
			RQD	= Rock Qua = Length of	ality Designa Sound Core	ation es>4 in / Pen	U = Undisturbed Sample .,% SC = Sonic Core		LL = Liquid Limit PL = Plasticity Limit	30 inches to drive a 2-inch-O.D.		
			WOR WOH	t = Weight o I = Weight o	of Rods of Hammer		DP = Direct Push Sample HSA = Hollow-Stem Auger		PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	split spoon sampler. Diameter		
			Sa	ample Infe	ormation			b				
Elev.	Depth				Don /	Blows	Drilling Remarks/	ic Lo	O sil su d	Deale December 1		
(ft)	(ft)	Sa	mple	Depth	Rec.	per 6 in.	Field Test Data	raphi	Soli and	Rock Description		
			NO.	(11)	(in)	or RQD		Ū	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 20.2		
20 —	1	NÆ	S1	0 to	24/16	9-14-21-	PID = 1.1 ppm @S1		(0-4"): Concrete. S1: NARROWI Y GRADED S	SAND WITH SILT AND GRAVE		
-	-	IXI-		2		32			(SP-SM); ~70% fine to mediu	im sand, ~20% angular to subangular		
		$\langle \rangle$							petroleum-like odor.	plastic lines; brown to tan; dry. Slight		
-	F	Λ	S2	2 to	24/21	47-21-	PID = 0.1 ppm @S2		S2: NARROWLY GRADED S	SAND (SP); ~90% fine to medium		
_	-	IXI-		4		36-24			up to 0.75"; light to dark brow	n; dry. Coal fragments.		
		$\langle \rangle$										
-	F		S3	4	24/10	7-16-16-	Roller bit to 4 feet. Rig		S3: NARROWLY GRADED S	SAND WITH SILT AND GRAVEL		
	- 5	IV.		6		10	PID = 0.0 ppm @S3		gravel up to 1", ~10% fine to mediu	im sand, ~20% angular to subangular stic fines; dark brown; dry. Coal and		
	1	$ \rangle$							brick fragments.			
-	-	$\left(\right)$	S/	6	24/8	10-11-7-	PID = 0.0 ppm @S4		S4: NARROWLY GRADED S	SAND WITH SILT AND GRAVEL		
	L	IVI -	04	to 8	24/0	12	-/- PID = 0.0 ppm @S5	(SP-SM); Similar to S3.				
-	1							STRATUM III - TILL	(CLASS 3A) at ~8 ft / El. 12.2			
-	ŀ	$\left(\right)$	S 5	8	2//0	5-16-26-			S5: SILTY SAND WITH GRA	VEL (SM); ~60% sand, ~20%		
	L	IVI -	00	to 10	24/0	14			nonplastic fines, ~20% subar 1.5": tan to brown: drv.	ngular to subrounded gravel up to		
-	1	$ \Lambda $							···• , ···· ·· ···, ··· , ··· ,			
10 —	- 10	$\left(\right)$	86	10	24/10	10.14	Drive casing 10 feet below	S6: WIDELY GRADED SAND WITH GRAVEL (SW); ~65% san				
		IVI -	50	to 12	24/10	15-10	grade. PID = 0.0 ppm @S6	50: WIDELY GRADED SAND WITH GRAVEL (SW), ~05% Sand ~30% subangular to subrounded gravel up to 1.5", ~5% nonplas				
-	-	M						fines; brown to dark brown; dry. Some micaceous sand.				
_	ŀ	+										
-	ſ											
_	ŀ											
	15	Λ	S7	15 to	24/6	11-18-	Drive casing to 15 feet below grade. Roller bit to 15 feet		S7: NARROWLY GRADED S	SAND (SP); ~85% fine to medium		
	F	IXI-		17		19-26	PID = 0.0 ppm @S7		light brown; wet.	vel up to 1°, ~5% nonplastic fines;		
_]	$ \rangle$							-			
-	-	\square										
	Ļ											
-	1											
-	ŀ											
NOTES	S:							PROJ	ECT NAME: Astoria Cove			
								CITY/	STATE: Astoria, Queens, New	York		
								GEI P	ROJECT NUMBER: 2305370			







E	BORIN	n <mark>g in</mark> Hing	NFC (ft):	RN 2	1ATION 22.090	<u>1</u>			EASTING (ft): 1.003 5	73		BORING
C	GROU	ND S	URF	AC	E EL. (f	it): 14.0)		DATE START/END: _5	5/12/20	020 - 5/13/2020	
V	/ERT.	/HOR	IZ. I	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B21
T	OTAL	DEF	тн	(ft):	71.4	4			DRILLER NAME:Ed	Flanaç	gan	
	.OGG	ED B.	Y:	G	. Holme	IS			RIG TYPE: CME 75			PAGE 1 of 3
E F A	DRILL HAMM	<u>ING</u> ER T	INF YPE /0.0	OF :: ::	Autom	<mark>DN</mark> natic NA			CASING I.D./O.D.:4 i DRILL ROD O.D.: NM	nch/ 4	.5 inch CORE BARF	REL TYPE: <u>NA</u> REL I.D./O.D.: NA / NA
6	RILL	ING N	ΛET	но	D : M	ud Rotary	Wash					
V	VATE	R LE	VEL	DE	PTHS (ft):						
A	ABBRI			IS:	Pen. Rec. RQD WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight I = Weight	on Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Di	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. ameter
					S	ample Inf	ormation	1		og.		
E	Elev. (ft)	Dep (ft	oth)	Sa	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and F	Rock Description
F				/	01	0 24/42 16 12 PID = 0.0ppm @S1		PID = 0.0ppm @S1		S1 [·] SILTY SAND WITH GRAV	/FL (SM): ~50% fine to medium	
	-	-		X	S1	to 2	24/12	16-13- 12-15			sand, ~30% nonplastic fines, ~ dark brown to black; dry.	-20% subangular gravel up to 1.5";
	-	_		$\left(\right)$	S2	2 to 4	24/10	27-19- 13-11	PID = 0.0ppm @S2		S2: SILTY SAND WITH GRAV	/EL (SM); Similar to S1.
23	10		5	X	S3	4 to 5.6	19/4	7-17-13- 55/1"	PID = 0.0ppm @S3		S3: SILTY SAND (SM); ~70% subangular gravel up to 1"; tar	sand, ~20% nonplastic fines, ~10% n; dry.
13.GDT 12/22/	-	-		X	S4	6 to 8	24/6	16-10- 13-62	PID = 0.0ppm @S4		S4: SILTY SAND WITH GRAV sand, ~20% nonplastic fines, ~ brown to dark brown; dry. Bric odor.	/EL (SM); ~60% fine to medium ~20% subangular gravel up to 1.5"; k fragments. Slight organic-like
A TEMPLATE 20		-		$\left(\right)$	S5	8 to 10	24/13	33-27- 27-28	PID = 0.0ppm @S5		S5: SILTY SAND WITH GRAV subangular gravel up to 1.5 ind black; dry. Red brick and con organic-like odor.	/EL (SM); ~50% sand, ~30% ch, ~20% nonplastic fines; brown to crete fragments; moist; slight
DVE.GPJ GEI DAT/	-			×,	S6	10 to 10.4	<u>ر 5/0</u>	100/5"			S6: No Recovery.	
05370-ASTORIA CC	0	-		15						Drive casing to 15 feet below grade. Roller bit to 15 feet. Rig chatter from 10 to 15 feet. Cobble from 10 to 14 feet.		
-GRAPHIC LOG 23	-	-	15	X	S7	15 to 17	24/4	16-8-5-3	PID = 0.0ppm @S7		S7: WIDELY GRADED GRAV subangular gravel up to 1.5", ^	EL WITH SAND (GW); ~80% ~20% sand; brown; wet.
TD 6-NORTH-EAST	-	-							Drive casing to 20 feet below grade. Roller bit to 20 feet.		STRATUM IIA - LOOSE SAI	ND (CLASS 6) at ~18.5 ft / El4.5
	NOTES:									PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New NROJECT NUMBER: 2305370	York GEI Consultants

NORT	RTHING (ft): 222,090 EASTING (OUND SURFACE EL. (ft): 14.0 DATE STA RT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING							EASTING (ft): 1,003,57	BORING		
GROU	ND S /HOF	5URI 217	FAC DAT	E EL. (1	ft): <u>14.0</u> NAVD 88) 3/NAD83 N	Y Zone 31	DATE START/END: _5	/12/20 Crai	020 - 5/13/2020 g Geotechnical Drilling	B21
• =			2711		1010000		2010 01			g oooloonnioui Dhinnig	PAGE 2 of 3
				S	ample Inf	ormation			bo		
Elev. (ft)	De (f	oth t)	Sa	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and	Rock Description
-			X	S8	20 to 22	24/14	5-2- WOH-1	PID = 0.0ppm @S8		S8: CLAYEY SAND (SC); ~7 ~5% fine gravel; dark brown odor.	'5% sand, ~20% low plasticity fines, and black; wet. Slight organic-like
- -10—	_							Drive casing to 25 feet below grade. Roller bit to 25 feet.			
-	<u>-</u>	25	X	S9	25 to 27	24/7	5-7-11- 15	PID = 0.0ppm @S9		S9: CLAYEY SAND WITH G subangular gravel, ~20% lov Some organic fibers. Organi	RAVEL (SC); ~50% sand, ~30% v plasticity fines; black-white; wet. c-like odor.
-	_							Drive casing to 30 feet below grade. Roller bit to 30 feet.		STRATUM IIB - SILT/CLAY	(CLASS 4B/5B) at ~28.5 ft / El14.5
-	-	30	X	S10	30 to 32	24/17	2-5-5-5	PID = 0.0ppm @S10		S10: FAT CLAY WITH GRAV fines, ~10% subangular grav wet. Organic-like odor.	/EL (CH); ~85% medium plasticity /el up to 1.5", ~5% fine sand; black;
-20 -	-							Roller bit to 35 feet. Rig chatter from 32 to 34 feet (Cobble/boulder).		STRATUM III - TILL (C	CLASS 3A) at ~33.5 ft / El19.5
	-	35	\mathbb{N}	S11	35 to 37	24/2	22-11- 21-10	PID = 0.0ppm @S11		S11: WIDELY GRADED GR subangular gravel up to 1.5"	AVEL WITH SAND (GW); ~70% , ~30% sand; brown; wet.
	-							Roller bit to 40 feet. Rig chatter from 39 to 40 feet.			
	 - -	40		S12	40 to 42	24/8	12-21- 24-26	PID = 0.0ppm @S12		S12: NARROWLY GRADED sand, ~10% subangular grav yellow-brown; wet.	9 SAND (SP); ~85% fine to medium /el up to 1.5", ~5% nonplastic fines;
-30								Drive casing to 40 feet below grade. Roller bit to 45 feet.			
		45	X	S13	45 to 47	24/6	29-23- 29-30	PID = 0.0ppm @S13		S13: WIDELY GRADED SAI ~25% subangular gravel up	ND WITH GRAVEL (SW); ~75% sand, to 1.5"; light brown; wet.
NOTES:									PROJI CITY/S GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants



		NG I	INF(N			EASTING (ff): 1 003 5	31		BORING
	GROUI	ND	SUR	FAC	:E EL. (1	ft): <u>17</u> .5	5		DATE START/END:	5/28/2	020 - 5/28/2020	
	VERT./	/HO	riz.	DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Cra	ig Geotechnical Drilling	B22
		DE		l (ft)	: 75.	1				Flana	gan	_
	LUGG		BY:		. Holme	es			KIG I YPE: CME 75			PAGE 1 of 4
	DRILL	INC	g in	FO	RMATIC	ON						
	НАММ	ER	TYP	E:	Autom	natic			CASING I.D./O.D.:	inch/ 4	4.5 inch CORE BAR	REL TYPE: NA
		R I.E)./O.	D.:		NA Nd Detern	Weeh		DRILL ROD O.D.:N	1	CORE BAR	REL I.D./O.D.: <u>NA / NA</u>
	WATE	RLE			EPTHS ((ft):	Wash					
	ABBRE	EVIA	ΑΤΙΟ	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight H = Weight	on Length (Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample % SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iiameter
F					S	ample Inf	ormation			b		
	Elev.	De	epth	_			Pen /	Blows	Drilling Remarks/	lic Lo	Soil and	Rock Description
	(ft)	(1	ft)	S	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	raph		Nock Description
L				 ,		. ,	(in)	or RQD			STRATUM I - FILL ((CLASS 7) at ~0 ft / El. 17.5
	_	-		M	S1	0 to 2	24/13	4-14-7-5	PID =363.0ppm @S1		S1: SILTY SAND WITH GRA sand, ~20% nonplastic fines, brown to black; dry. Slight pe brick, concrete, wood and me	VEL (SM); ~60% fine to medium ~20% subangular gravel up to 1.5"; troleum odor, black staining. Red tal fragments.
		-	5	M	S2	2 to 4	24/11	9-13-12- 7	PID = 154.9ppm @S2		S2: NARROWLY GRADED S to medium sand, ~10% nonpl subangular gravel up to 1"; bi	AND WITH SILT (SP-SM); ~80% fine astic fines, ~10% fie to coarse own; dry.
~	_	-		M	S3	4 to 6	24/9	17-13- 12-9	PID = 121.4ppm @S3		S3: NARROWLY GRADED S S2. Some coal fragments.	AND WITH SILT (SP-SM); Similar to
3.GDT 12/22/2:	- 10	-		$\left \right\rangle$	S4	6 to 8	24/14	3-2-4-10	PID = 58.9ppm @S4		S4: NARROWLY GRADED S S3.	AND WITH SILT (SP-SM); Similar to
MPLATE 201	_	-		$\left \right\rangle$	S5	8 to 10	24/10	28-13-9- 9	PID = 31.2ppm @S5		S5: NARROWLY GRADED S S3. Concrete fragments.	AND WITH SILT (SP-SM); Similar to
A TE	-	L	10	/							STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 7.5
VE.GPJ GEI DAT	_	-	10	X	S6	10 to 12	24/6	2-3-2-5	PID = 24.7ppm @S6		S6: WIDELY GRADED SANE ~10% nonplastic fines, ~5% f brown to reddish-brown; dry.	0 WITH SILT (SW-SM); ~85% sand, ine subrounded gravel up to 0.5";
370-ASTORIA CC	-		15						Advance casing to 15 feet below grade. Roller bit to 15 feet.			
RAPHIC LOG 2305	_	-		X	S7	15 to 17	24/8	3-3-4-2	PID = 13.6ppm @S7		S7: WIDELY GRADED SANE) WITH SILT (SW-SM); Similar to S6.
D 6-NORTH-EAST-G	0	-							Advance casing to 20 feet below grade. Roller bit to 20 feet.		STRATUM III - TILL (CLASS 3A) at ~18.5 ft / El1
GEI WOBURN ST	NOTES:										TI JECT NAME: Astoria Cove /STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL Consultants

NORT	RTHING (ft): 222,064 EASTING (ROUND SURFACE EL. (ft): 17.5 DATE STA RT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING							EASTING (ft):	31	120 - 5/28/2020	BORING
VERT.	RT./HORIZ. DATUMS: NAVD 88/N Sample Inform				NAVD 88	3/NAD83 N	IY Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 4
	Γ			S	amnle Inf	ormation					
Elev. (ft)	De (epth ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Roo	k Description
-	-		X	S8	20 to 20.4	5/3	<u>, 100/5"</u>	PID = 0.0ppm @S8		S8: WIDELY GRADED SAND W ~20% subangular gravel up to 1"	ITH GRAVEL (SW); ~80% sand, ; brown; wet.
-	- - -							Roller bit to 25 feet. Rig chatter from 20 to 22 feet and 23 to 25 feet.	d		
-	-	25	X	S9	25 to 27	24/6	59-46- 43-31	PID = 0.0ppm @S9		S9: NARROWLY GRADED SAN medium to coarse sand, ~40% so brown; wet.	D WITH GRAVEL (SP); ~60% lbangular gravel up to 1.5";
-10	 - - 							Advance casing to 25 feet below grade. Roller bit to 30 feet. Rig chatter from 28 to 30 feet.			
-	-	30	X	S10	30 to 31.4	17/6	10-54- 102/5"	PID = 0.0ppm @S10		S10: SILTY SAND WITH GRAVE subangular gravel up to 1.5", 19. [GRAIN SIZE TEST PERFORME	EL (SM); ~48% sand, 32.5% 5% nonplastic fines; brown; wet. D].
-	-							Advance casing to 30 feet below grade. Roller bit to 35 feet. Rig chatter from 32 to 34 feet.			
-	-	35	X	S11	35 to 36.3	16/6	46-61- 103/4"	PID = 0.0ppm @S11		S11: SILTY SAND WITH GRAVE sand, ~30% nonplastic fines, ~30 brown; wet.	:L (SM); ~40% fine to medium)% subangular gravel up to 1.5";
-20 —	-							Roller bit to 40 feet. Rig chatter from 36 to 37 feet and 39 to 40 feet.	đ		
-		40	X	S12	40 to 40.9	11/4	57- 113/5"	PID = 0.0ppm @S12		S12: SILTY SAND (SM); ~60% fi nonplastic fines, ~10% subangul	ne to medium sand, ~30% ar gravel up to 0.75"; brown; wet
-	-							Roller bit to 45 feet. Rig chatter from 42 to 45 feet.			
-		45	R	S13	45 to 45.3	<u>4/4</u>	<u>102/4"</u>	PID = 0.0ppm @S13		S13: SILTY SAND (SM); Similar	to S12.
NOTES	S:								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New Yor ROJECT NUMBER: 2305370	K GEI





	BORIN NORTI	<u>ng in</u> Hing	IFC (ft):	2 2	iation 22,133	<u>1</u>			EASTING (ft): 1,003,48	80		BORING
	GROU	ND SI	JRF	AC	E EL. (f	t): 15.0)	· -	DATE START/END: 3	/10/20	020 - 3/11/2020	D 02
	VERT.	HOR	ΙΖ. Ι ΤΠ		UMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B23
	LOGG	ED B	/:	G	Holme	s			RIG TYPE: CME 75	Schu		PAGE 1 of 4
		<u>ING</u> ER T	INF YPE	<u>OF</u>	Autom	DN atic			CASING I.D./O.D.:4 in	nch/ 4	.5 inch CORE BAR	REL TYPE: NX Wireline
	DRILLI	NG N	IET	, но	D: M	ud Rotary	Wash					REL I.D./O.D <u>2.125 Incl / 5 Incl</u>
	WATE	R LE\	/EL	DE	PTHS (ft):						
ŀ	4000	-\// 6 T	101	10.	Der	- Davidari						
	ADDRI		IUr	10:	WOR	= Penetration = Recovery = Rock Quants = Length of R = Weight of I = Weight of	of Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	C = Core Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		QP = Pocket Perletonneter Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
					Sa	ample Inf	ormation			ō		
	Elev. (ft)	Dep (ft)	th)	Sa	ample	Depth	Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	aphic Lo	Soil and F	Rock Description
					INU.	(11)	(in)	or RQD		Ū	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 15
	_	_		$\left(\right)$	S1	0 to 2	24/12	13-26- 37-25	PID = 0.9ppm @S1		(0-6"): Asphalt. S1 (6"-12"): SILTY SAND WIT ~20% nonplastic fines, ~20% brown; dry. Brick fragments.	TH GRAVEL (SM); ~60% fine sand, angular gravel up to 1.25"; dark
	-	+ + +			S2	2 to 4	24/3	90-37- 33-41	PID = 0.4ppm @S2		S2: SILTY SAND (SM); ~70% nonplastic fines, ~10% angula Brick, asphalt and glass fragm	fine to medium sand, ~20% ar gravel up to 0.75"; brown; dry. nents.
~	- 10 —	_	5	$\left \right $	S3	4 to 6	24/15	29-19- 11-11	PID = 0.0ppm @S3		S3: SILTY SAND (SM); Simila	ar to S2.
3.GDT 12/22/23	-	_		$\left \right\rangle$	S4	6 to 8	24/3	7-5-2-4	PID = 0.0ppm @S4		S4: GRAVEL WITH SAND (G fine t coarse sand; brown; dry. and plastic debris.	P); ~70% subangular gravel, ~30% . Asphalt, brick, concrete fragments
FEMPLATE 201	-	_		$\left \right\rangle$	S5	8 to 10	24/7	6-9-5-4	PID = 0.0ppm @S5 Drive casing to 8 feet below grade.	×	S5: SILTY SAND WITH GRAV sand, ~30% nonplastic fines, brown; moist. Brick fragments	/EL (SM); ~50% fine to medium ~20% subangular gravel up to 1"; s and plastic debris.
PJ GEI DATA		- 1 -	10	X	S6	10 to 12	24/7	5-5-3-7	PID = 0.0ppm @S6		S6:SILTY SAND WITH GRAV	/EL (SM); Similar to S5.
5370-ASTORIA COVE.G	-	+ + + +							Drive casing to 13 feet below grade.	v		
GRAPHIC LOG 230	0	1 - -	15		S7	15 to 17	24/7	9-7-6-5	PID = 0.0ppm @S7		S7:SILTY SAND WITH GRAV black. Trace mica. Faint petro staining.	(EL (SM); Similar to S5. Brown to bleum odor at bottom of spoon. Black
TD 6-NORTH-EAST-	-	_							Drive casing to 18 feet below grade.			
										PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants



NORT	HING (ft)	: _222,13	3	_		EASTING (ft): 1,003,4	80		BORING
GROU VERT.	IND SUR ./HORIZ.	FACE EL.	(ft): 15.0 NAVD 88) 8/NAD83 N	Y Zone 31	DATE START/END: D4 DRILLING COMPANY:	3/10/20 Crai	ig Geotechnical Drilling	B23
								<u> </u>	PAGE 3 of 4
			Sample Inf	formation			bo-		
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic I	Soil and	Rock Description
-		X				Rig chatter from 47 to 49 feet.			
-	- - 50	S14	50 to 51.8	21/15	31-52- 73- 102/3"	PID = 0.0ppm @S14		S14: SILTY SAND (SM); Sin	nilar to S13.
-	-					Intermittent rig chatter from 52 to 55 feet.			
-40 —	- 55	× <u>S15</u>	55 to 55.3	3/2	102/3"	PID = 0.0ppm @S15		S15: SILTY SAND (SM); ~7(~5% subangular gravel up to)% fine sand, ~25% nonplastic fines, p 1"; brown; wet.
	- - 60	X S16	60 to 60.7	8/8	70- _104/2"_	Rig chatter at 56 to 57 feet. PID = 0.0ppm @S16		S16: SILTY SAND (SM); Sin	illar to S15.
	-					Rig chatter at 63 to 65 feet.			
-50	- 65	S17	65 to 67	24/17	41-47- 32-79	PID = 0.0ppm @S17		S17: SILTY SAND (SM); ~7(nonplastic fines, ~10% suba orange-brown; wet.	9% fine to medium sand, ~20% ngular gravel up to 1.5"; brown to
	+ + +	<u>v v</u>				Rig chatter at 67 to 69 feet.			
	70	S18	70 to 72	24/20	88-84- 61-116	PID = 0.0 ppm @S18		S18: SILTY SAND (SM); Sin	nilar to S17. Orange-brown.
	S:	1	1	1	l	The chance at 72 leel.	PROJ	ECT NAME: Astoria Cove	
							PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370		

NORT GROU	HING (ft) IND SUR	: <u>222,133</u> FACE EL. (1	ft):15.0)		EASTING (ft): _1,003,44 DATE START/END: _3	30 3/10/20)20 - 3/11/2020	BORING B23	
VERT.	RT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPAN							Craig Geotechnical Drilling PAGE 4 of		
Elev. (ft)	Depth (ft)	S Sample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description		
-60	(ii) - 75 - 80 - 85 - 85 - 90 - 90 - 90	No. S19	(ft) 75 to 77 77 80.1 80.1 to 85.1 to 90.1	Rec. (in)	per 6 In. or RQD 11-21- 29-39 105/1" 32%	PID = 0.0ppm @S19 Rig chatter at 79 to 80 feet.		STRATUM IV - DECOMPC 1D) at1D) at	Astic to low plasticity fines, ~10% to 0.75"; greenish-blue.	
- - NOTE:							PROJ CITY/	ECT NAME: Astoria Cove STATE: Astoria, Queens, New		

BOR NOR	inc Thi	g inf Ng (1	OR t):	MATIO 222,066	N			EASTING (ft): _1,003,3	20		BORING		
GRO	UNI	d su	RFA	CE EL. (1	ft):21.9)		DATE START/END:	3/16/20	020 - 3/17/2020			
VERT	г./Н	IORIZ	. DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DRILLING COMPANY:	Crai	ig Geotechnical Drilling	B29(OW)		
TOTA		DEPT	Ή (ft):85.	0			DRILLER NAME: Joe	e Schu	ster			
LOGO	GEI	D BY:		G. Holme	es			RIG TYPE: CME 75	RIG TYPE: CME 75 PAGE 1 of 4				
DRII	1 11	NG II	NFO	RMATI	ON								
HAM	ME	RTY	PE:	Auton	natic			CASING I.D./O.D.: 4	inch/ 4	.5 inch CORE BARR	EL TYPE: NA		
AUG	ER	I.D./C).D.:	NA /	NA			DRILL ROD O.D.: NN	И	CORE BARR	EL I.D./O.D.: NA / NA		
DRIL	LIN	IG MI	ЕТНО	DD: N	lud Rotary	Wash							
WAT	ER	LEVI	EL D	EPTHS ((ft): ⊻ 1	6.9 3/18/2	2020 7:55 a	m in observation well					
ABBH	κΕ/		ONS	Pen. Rec.	= Penetrati = Recovery	on Length		S = Split-Spoon Sample C = Core Sample		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength	NA, NM = Not Applicable, Not Measured		
				RQD	= Rock Qu	ality Designa	ation s>4 in / Pen	U = Undisturbed Sample % SC = Sonic Core		LL = Liquid Limit PL = Plasticity Limit	30 inches to drive a 2-inch-O.D.		
				WOF	R = Weight	of Rods		DP = Direct Push Sample		PID = Photoionization Detector	split spoon sampler.		
	_		-	WOH	H = Weight	of Hammer		HSA = Hollow-Stem Auger		I.D./O.D. = Inside Diameter/Outside Dia	meter		
				S	ample Inf	ormation			bo				
Elev.		Deptl	n			Pen /	Blows	Drilling Remarks/	lic L	Soil and P	ock Description		
(ft)		(ft)	15	Sample	Depth	Rec.	per 6 in.	Field Test Data	aph		ock Description		
				INO.		(in)	or RQD		Ū	STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.9			
			1	S1	0	24/3	17-4-5-	PID = 0.0ppm @S1		S1: WIDELY GRADED SAND	WITH SILT AND GRAVEL		
	L		IV	•	to 2		47			(SW-SM); ~80% sand, ~10% g 1 25" ~10% popplastic fines: b	ravel angular to subrounded up to		
			ΙΛ							, u y.			
20 -	╇		\square					PID = 0 0ppm @S2					
			\mathbb{N}	S2	2 to	24/14	52-21-9- 5			S2: WIDELY GRADED SAND WITH SILT AND GRAVEL (SW-SM); Similar to S1. Drilled through cobble boulder 4 to 14 inches.			
	+		IX		4								
			$ \rangle$										
	+		\vdash	00	4	0.4/0	00.00	PID = 0.0ppm @S3		S3 ⁻ WIDELY GRADED SAND (SW): ~95% sand ~5% angular to		
			W	53	to	24/3	17-11			subrounded gravel up to 0.75"; yellow-brown; dry.			
	7	- 5	۶IX		0								
23			$\langle \rangle$										
2/22/2			$\langle \rangle$	S4	6	24/7	11-11-6-	PID = 0.0ppm @S4		S4: WIDELY GRADED SAND	WITH GRAVEL (SW); ~70% sand,		
	4		IY		8		11			 ~30% subangular to subrounde dry. 	ed gravel up to 1.5"; orange-brown;		
3.60									CLASS 3B) at ~8 ft / EL 13 9				
2010	+		\vdash		0			Advancing casing to 8feet;			(CLASS SD) at ~0 it / Ei. 13.9		
ATE			\mathbb{N}	S5	to	24/9	12-19-	Roller bit to 10feet after		sand, ~5% subangular to subro	bunded gravel up to 1.25", ~5%		
APL	+		IX		10		15-10	sample. PID = 0.0ppm. @S5		nonplastic fines; brown to reddi	sh brown; dry.		
V TEI			$ \rangle$										
1 M	7	- 10	1	56	10	24/8	12-8-8-	PID = 0.0ppm @S6		S6 (0-12"): SILTY SAND (SM);	~75% fine to medium sand, ~20%		
3EI L			IV	50	to 12		14			nonplastic fines, ~5% subangular gravel up to 0.5"; brown; dry.			
с Г	Γ										ADED SAND (SD): ~05% find to		
ا ت ا	╇		()							medium sand, ~5% nonplastic	fines; tan; dry.		
COV											•		
RIA	╀							Roller bit to 15 feet.					
0 Is													
A-07	+												
053													
23	7	- 15	° 7	S 7	15	24/17	7-4-5-26	PID = 0.0ppm @S7		S7 (0-7"): NARROWLY GRADI	ED SAND (SP); Similar to S6 but		
ŏ			IV	57	to 17					brown and moist.	(ML): =00% populatio frace 40%		
E	Γ									fine sand; brown; moist. Trace	$(m \perp)$, ~90% nonplastic lines, ~10% mica.		
- RAF	+		\square										
ΈA:	╀							Roller bit to 20 feet. Rig					
τ								chatter 16 to 18 feet.					
-NC	╀												
n													
	s:				•	•			PROJ	IECT NAME: Astoria Cove			
OBU													
Ň									CITY/	SIATE: Astoria, Queens, New Y			
<u></u> נ	l									GEI PROJECT NUMBER: 2305370			

NORTI GROU VERT.	HING (ND SL /HORI	(ft): JRF IZ. [<u>22</u> FACE	22,066 E EL. (f	t): 21.9 NAVD 88) 3/ <u>NAD</u> 83 N	IY Zone 310	EASTING (ft): 1,003,3 DATE START/END: 2 DRILLING COMPANY:	20 3/16/20 _ <u>C</u> rai)20 - 3/17/2020 g Geotechnical Drilling	BORING B29(OW)
	Comple lafe and the										PAGE 2 of 4
Elev. (ft)	Dept (ft)	th	Sa N	Sa mple No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and I	Rock Description
- 0—				S8	20 to 22	24	16-7-10- 15	PID = 0.0ppm @S8		S8: NARROWLY GRADED S sand, ~5% subangular to sub nonplastic fines; brown; wet.	AND (SP); ~90% fine to medium rounded gravel up to 1.25", ~5%
-								Roller bit to 25 feet. Rig chatter from 23 to 25 feet.		STRATUM III - TILL (C	:LASS 3A) at ~23.5 ft / El1.6
-	- 2 -	25		S9	25 to 27	24/11	23-21- 21-24	PID = 0.0ppm @S9		S9: WIDELY GRADED SANE ~30% subangular to subround Mica near the top.) WITH GRAVEL (SW); ~70% sand, ded gravel up to 1.5"; brown; wet.
-								Roller bit to 30 feet. Rig chatter from 25 to 26 feet an 29 to 30 feet.	d		
- -10-	- 3 -	80	X	S10	30 to ∖ 30.8 ∫	9/5	38- 104/3"	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~5% angular to subang reddish brown; wet.	SAND (SP); ~95% fine to medium ular gravel up to 0.75"; brown to
-								Roller bit to 35 feet. Rig chatter from 31 to 32 feet, 33 to 34 feet.	3		
-	- 3 -	85	X	S11	35 to ∖35.8 ∫	9	118- <u>101/3</u> "_	PID = 0.0ppm @S11		S11: NARROWLY GRADED fine to medium sand, ~10% a 1.5", ~10% nonplastic fines; b in shoe.	SAND WITH SILT (SP-SM); ~80% ngular to subrounded gravel up to rown; wet. Drilled into boulder/cobble
-								Roller bit to 40 feet. Rig chatter from 36 to 40 feet.			
-20—	- 4 -	io		S12	40 to 42	24/22	79-82- 66-78	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN ~30% subangular to subroun	D WITH GRAVEL (SW): ~70% sand, ded gravel up to 1.5"; brown; wet.
-	 - - -							Roller bit to 45 feet. Rig chatter from 45 to 46 feet, 47 to 50 feet.	7		
-	- 4	15		<u>S13</u>	45 to 45.2	2/2	102/2"	PID = 0.0ppm @S13		S13: SANDY SILT (ML): ~60° brown; wet.	% nonplastic fines, ~40% sand;
NOTES	3:	_							PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

NORTHING	(ft):	222,066

 GROUND SURFACE EL. (ft):
 21.9

 VERT./HORIZ. DATUMS:
 NAVD 88/NAD83 NY Zone 3104

EASTING (ft): <u>1,003,320</u> DATE START/END: <u>3/16/2020 - 3/17/2020</u>

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B29(OW)

PAGE 3 of 4

ľ			Sa	ample Inf	ormation			p				
	Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and Rock Description			
	 - -30		S14	50 to 51.3	16/15	45-97- 102/4"	Roller bit to 50 feet. Rig chatter from 45 to 46 feet. PID = 0.0ppm @S14 Roller bit to 55 feet. Rig chatter from 53 to 54 feet.		S14: SILTY SAND (SM); ~55% fine sand, ~40% nonplastic fines, ~5% angular to subrounded gravel up to 0.75"; brown; wet. Trace mica.			
	-	- 55 - 55	× <u></u>	55 to 55.4	<i>5/3</i> /	<u>110/5"</u>	PID = 0.0ppm @S15		S15: SILTY SAND (SM); ~60% fine sand, ~30% nonplastic fines, ~10% subangular gravel up to 1"; brown; wet.			
TEMPLATE 2013.GDT 12/22/23	- - -40 —	60 60	× S16	60 to 60.2	2/2	102/2"	Roller bit to 60 feet. Rig chatter at 56 and 58 feet. PID = 0.0ppm @S16		S16: NARROWLY GRADED SAND WITH GRAVEL (SP); ~75% fine to medium sand, ~20% subangular to subrounded gravel up to 1", ~5% nonplastic fines; brown; wet.			
LOG 2305370-ASTORIA COVE.GPJ GEI DATA	-	65 		65 to 65.1	1/0	105/1"	Rig chatter from 63 to 65 feet. Slow advancement 60 to 63 feet. PID = 0.0ppm @S17 Rig chatter from 67 to 70 feet. Slow advancement 65 to 69 feet. Quick advancement 69 to 70 feet		S17: No Recovery. STRATUM IV - DECOMPOSED/WEATHERED ROCK (CLASS 1D) at ~67 ft / El45.1			
I STD 6-NORTH-EAST-GRAPHIC	-50 —	70 	× <u></u> S18_	70 to 70.3	<u>4/4</u>	٢ 100/4" ر	PID = 0.0ppm @S18		S18: WIDELY GRADED SAND WITH SILT (SW-SM); ~85% sand, ~10% nonplastic to low plasticity fines, ~5% fine angular gravel < 0.5"; white to yellow-brown; wet. Decomposed rock.			
GEI WOBURN	NOTES:								PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370			



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,320 DATE START/END: 3/16/2020 - 3/17/2020 DRILLING COMPANY: Craig Geotechnical Drilling BORING B29(OW)

PAGE 4 of 4

Ī				Sa	ample Inf	ormation			bc	
	Elev. (ft)	Depth (ft)	Sar N	nple o.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and Rock Description
	-	I.						Roller bit to 75 feet. Driller used low pressure on bit, slow advancement.		
	-	- 75 - -		S19 C1	75 to 75 75 to 80	0/0 60/34	45/0" 35%	No advancement, hard bouncing.		S19: No Recovery. Spoon bouncing. C1: 1.5 to 8.5" very fractured. Gneiss; hard; fine to medium grained; horizontal and vertical banding; slight weathering at fractures; white to grey to black. Horizontal fractures at 1.5", 8.5", 12.5", 17.5", 21.5", and 26.5". Core run times per foot (minutes:seconds): 1:06, 2:42, 2:38, 3:52, and 3:06.
	_	- 80							1/	STRATUM V - BEDROCK (CLASS 1B) at ~80 ft / El58.1
122/23	-60	- 80 - - -		C2	80 to 85	60/57	71%			C2: Gneiss; hard; fine to medium grained; horizontal and vertical banding; slight weathering at fractures; white to grey to black. Horizontal fractures at 4", 7", 11", 15.5", 19.5", 22.5", 29", 34", 39", 45"and 51.5". Core run times per foot (minutes:seconds): 4:01, 2:47, 2:57, 2:38, and 3:07.
STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT 12/	-70	- 85 - 90 - 90 - 95 - 95								Backfilled with cuttings and hole plug. Bottom of boring at depth 85 ft.
№ F № 0 0 0 0 0										STATE: Astoria, Queens, New York ROJECT NUMBER: 2305370

ſ	BORIN North	ng I Hing	NFC i (ft)	DRN : _2	1ATION 221,759	<u>N</u>			EASTING (ft): _1,003,4	21		BORING							
	GROU	ND S	SUR	FAC	E EL. (f	t): 29.4 NA\/ר פי	ארצארואל/	Y 7000 211		5/13/20 Cra	020 - 5/14/2020	B53(OW)							
		DE	NZ. PTH	l (ft)	: 55.0)	MAD03 N		DRILLER NAME: Kei	th Par	rent	D93(UVV)							
	LOGGI	ED B	SY:	G	. Holme	s			RIG TYPE: CME 75			PAGE 1 of 3							
┢		ING		FOF	ΜΔΤΙά	אר													
	HAMM	ER	ГҮР	E:	Autom	natic			CASING I.D./O.D.: 4	inch/ 4	I.5 inch CORE BAR	REL TYPE: NX Wireline							
	AUGE	R I.D	./0.	D.:	NA / I	NA			DRILL ROD O.D.: NM	1	CORE BAR	REL I.D./O.D.: 2.125 inch / 3 inch							
	DRILLI WATEI	ING I R LE	ME1 VEI	fho _ De	D: <u>M</u> PTHS (ud Rotary ft):	Wash 3.5 5/15/2	2020 9:30 a	m in observation well										
	ABBRI	EVIA	τιο	NS:	Pen. Rec. RQD WOF WOF	= Penetrati = Recovery = Rock Qu = Length of R = Weight I = Weight	on Length Length ality Designa Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter							
					Sa	ample Inf	ormation			bc									
	Elev.	De	pth				Pen./	Blows	Drilling Remarks/	lic Lo	Soil and I	Rock Description							
	(ft)	(f	t)	Sample No.		Depth (ft)	Rec.	per 6 in.	Field Test Data	Brapt									
						0	(11)	UINQD			STRATUM I - FILL (CLASS 7) at ~0 ft / El. 29.4							
	-	-		X	S1	to 1.7	20/3	9-7-4- 9/2"	- 0.0μμπ @31		(U-4): Aspnant. S1: SILTY SAND (SM); ~75% ~5% angular gravel up to 1"; ; fragments.	o fine sand, ∼20% nonplastic fines, yellow-brown; dry. Red brick							
	-	_		X	S2	2 to 4	24/10	10-6-7-5	PID = 0.0ppm @S2		S2: SILTY SAND (SM); ~75% nonplastic fines; orange-brow	fine to medium sand, ~25% n to brown; dry.							
				$\langle \rangle$							STRATUM IIC - SAND	(CLASS 3B) at ~4 ft / El. 25.4							
e			5	M	S3	4 to 6	24/10	10-6-5-4	PiD – 0.0ppin @33		S3: NARROWLY GRADED S to medium sand, ~10% nonpl mica.	AND WITH SILT (SP-SM); ~90% fine astic fines; tan-brown; dry. Trace							
.GDT 12/22/2	-	_		X	S4	6 to 8	24/19	6-3-3-4	PID = 0.0ppm @S4		S4: NARROWLY GRADED S sand, ~10% nonplastic fines;	AND WITH SILT (SP-SM); ~90% fine tan to yellow-brown; dry. Trace mica.							
EMPLATE 2013	- 20	_		X	S5	8 to 10	24/14	4-4-6-5	PID = 0.0ppm @S5		S5: NARROWLY GRADED S S4.	AND WITH SILT (SP-SM); Similar to							
SPJ GEI DATA TI	-	 	10	Ň	S6	10 to 12	24/15	8-6-8-5	PID = 0.0ppm @S6		S6: NARROWLY GRADED S S4.	AND WITH SILT (SP-SM); Similar to							
370-ASTORIA COVE.0	-	_					<u>/</u>	<u>/</u>	<u>/</u>	/	/ \						Advance casing to 15 feet below grade. Roller bit to 15 feet.		STRATUM III - TILL (C
GRAPHIC LOG 2305	-		15	X	S7	15 to 17	24/7	9-8-17- 19	PID = 0.0ppm @S7		S7: SILTY SAND WITH GRA' sand, ~20% nonplastic fines, up to 1.5"; purple-brown; mois	VEL (SM); ~60% fine to medium ~20% angular to subrounded gravel st.							
TD 6-NORTH-EAST-(- - 10	- 							Advance casing to 20 feet below grade. Roller bit to 20 feet. Rig chatter ~16 feet and from 17 to 18 feet.										
GEI WOBURN S	NOTES	S:								PROJ CITY/ GEI F	IECT NAME: Astoria Cove ISTATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL Consultants							




B N	ORIN ORTH	NG IN HING	IFO (ft):		1ATION 22,057	<u>I</u> t): 20.1			EASTING (ft): 1,003,4	90 11/23/	2021 - 11/23/2021	BORING
V	ERT./	HOR	JRF IZ. C	AC DAT	сс. (т UMS:	NAVD 88	3/NAD83 N	IY Zone 310	DATE START/END: DATE START/END: DATE START/END:	Crai	ig Geotechnical Drilling	B101
т	OTAL	DEP	тн	(ft):	102	.0			DRILLER NAME: Ma	rc Aqu	iino	2.0.
LC	oggi	ED B	/ :	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 5
D	RILL	ING	INF	OF		ON						
H/	АММ	ER T	YPE		Autom	atic			CASING I.D./O.D.: 4	inch/ 4	.5 inch CORE BAR	REL TYPE: NX Wireline
AI	UGEF	R I.D./	0.D).:	NA / N	A			DRILL ROD O.D.: N	1	CORE BAR	REL I.D./O.D.: 2.125 inch / 3 inch
DI	RILLING METHOD:Mud Rotary Wash											
vv	AIE	R LEV	/EL	DE	PTHS (π):						
A	BBRE	EVIAT	10N	IS:	Pen. Rec. RQD WOR WOR	= Penetration = Recovery = Rock Quant = Length of t = Weight of I = Weight of	on Length / Length ality Designa / Sound Core of Rods of Hammer	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Jiameter
					Sa	ample Inf	ormation			bo		
E	lev.	Dep	th	~		Durth	Pen./	Blows	Drilling Remarks/	ji L	Soil and	Rock Description
((ft)	(ft))	Sa	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	rapt		
						()	(in)	or RQD		0	STRATUM I - FILL ((CLASS 7) at ~0 ft / El. 20.1
				Λ	S1	0 to	24/15	11-7-6-6			(0-6"): Concrete.	fine to medium sand $\sim 20\%$
	-	╞		XI		2					nonplastic fines, ~10% suban	igular to subrounded gravel up to
			ľ								0.75"; dark brown; dry; moder	rately tight; concrete / brick fragments.
	_			Λ	S2	2 to	24/11	11-13-			S2: NARROWLY GRADED S to medium sand. ~10% nonpl	SAND WITH SILT (SP-SM); ~85% fine lastic fines, ~5% subangular to
	-	-		XI		4		G-11			subrounded gravel up to 0.75	"; brown; dry.
			ľ									
	_		Ν		S3	4 to	24/9	8-5-25-			S3: NARROWLY GRADED S	SAND WITH SILT (SP-SM); ~85% fine
		_	5	XI		6		19			Drilled through rock (cobble/b	boulder) from 6 to 9".
23			/	'								
2/22/2	-		Ī	1	S4	6	24/7	17-7-6-5			S4: SILTY SAND (SM); ~70%	5 fine sand, ~25% nonplastic fines,
T T	_	F		ХI		8					~5% subangular to subround loose.	ed gravel up to 0.75"; brown; dry;
13.GI				\mathbb{N}							STRATUM IIA - LOOSE S	AND (CLASS 6) at ~8 ft / El. 12.1
Е 20.	_	-	Ī	1	S5	8	24/7	5-2-3-2	Drive casing to 8 feet below		S5: NARROWLY GRADED S	SAND (SP); ~90% fine to medium
PLAT		_	ľ	Υ		to 10			Added quick-gel to drilling		sand, some mica, ~5% nonpl dry; loose.	astic fines, ~5% fine gravel; brown;
TEM				\mathbb{A}					mud. Drive casing to 8 feet below			
ATA ,	10 —	- 1	10	+	56	10	24/0	3-2-1-2	grade. Rollerbit to 8 feet.		S6: No Recovery.	
3EI D			ľ	$\langle $	00	to 12	24/0	5-2-1-2	mud. @S5			
Ld:	_			$\langle $								
VE.G	_	F	ł		07	12	0.4/0	0110			S7: No Recovery	
A CC				$\langle $	51	to 14	24/0	2-1-1-2				
TORI	-	_				. 7						
0-AS	_	F	ľ						Rollerbit to 15 feet.			
0537												
G 23	_		15		S8	15	24/5	2-1-1-1			S8: NARROWLY GRADED S	SAND WITH SILT (SP-SM); ~85% fine
CLO	_	F	ľ	χL		17	-				gravel; brown; wet; loose.	~10% nonplastic tines, ~5% fine
APHI			/	$^{\prime}$								
-GR/	_	╞	f	╡					Drive casing to 15 feet.			
EAS1	_ 1	Ļ							chatter from 19 to 20 feet.			
RTH-							STRATUM IIC - SAND) (CLASS 3B) at ~19 ft / El. 1.1				
ION-9												
STD												
WOBURN	OTES	5: 		_						PROJ	IECT NAME: Astoria Cove	
UGEI 19										GEI P	PROJECT NUMBER: 2305370	

NORT GROU	HING IND S	g (ft) Suri	: _2 FAC	222,057 E EL. (, ft): 20.1			EASTING (ft): 1,003,45 DATE START/END: 1	90 1/23/2	2021 - 11/23/2021	BORING B101
VERT.	/HOF	riz.	DAT	rums:	NAVD 88	3/NAD83 N	IY Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 5
				S	ample Inf	ormation			bc		
Elev. (ft)	De (f	pth t)	Si	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and F	Rock Description
-	_		M	S9	20 to 22	24/9	14-9-8-6			S9: NARROWLY GRADED S, sand, ~5% nonplastic fines, ~ gravel up to 1.25"; brown; wet	AND (SP); ~85% fine to medium 10% subangular to subrounded ; loose.
-								Rollerbit to 25 feet. Rig chatter from 20 to 21 and 23 to 24 feet. Drive casing to 20 feet. Rollerbit to 25 feet. Intermittent rig chatter from 20 to 25 feet.		STRATUM III - TILL (C	LASS 3A) at ~23.5 ft / El3.4
		25	X	S10	25 to 27	24/9	12-17- 12-14			S10: NARROWLY GRADED 5 sand, ~5% nonplastic fines, ~ 2" brown and 2 to 9" black (mi	SAND (SP); ~90% fine to medium 5% subangular gravel up to 1"; 0 to (caceous sand); wet; moderately tight
-			/ \					Drive casing to 25 feet. Rollerbit to 30 feet. Rig chatter from 28 to 30 feet.			
-10		30	X	S11	30 to 32	24/13	34-29- 36-27	Rollerbit to 35 feet. Intermittent rig chatter from 30 to 35 feet.		S11: NARROWLY GRADED fine sand, ~10% nonplastic fir gravel up to 1"; brown; wet; tig	SAND WITH SILT (SP-SM); ~80% les, ~10% subangular to subrounded ht.
-		35	X	S12	35 to 37	24/12	43-46- 77-62	Rollerbit to 40 feet. Rig chatter from 37 to 39 feet.		S12: NARROWLY GRADED sand, trace micaceous sand, subangular to subrounded gra	SAND (SP); ~90% fine to medium -5% nonplastic fines, ~5% avel up to 0.75"; brown; wet; tight.
- -20 -		40	X	S13	40 to 40.9	11/9	53- 100/5"	Rollerbit to 45 feet. Rig chatter from 41 to 42 feet.		S13: NARROWLY GRADED through rock (cobble/boulder)	SAND (SP); Similar to S12. Drilled from 7 to 9".
-		45	X	S14	45 to 47	24/18	66-49- 73-84			S14: NARROWLY GRADED sand, some micaceous sand, to subrounded gravel up to 1.5	SAND (SP); ~85% fine to medium ~5% nonplastic fines, ~10% angular ""; brown; wet; tight.
NOTES	S:								PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	

NORT	HING (ft) ND SUR	: _222,057 FACE EL. (1	ft): 20.1			EASTING (ft): DATE START/END:	90 1/23/2	2021 - 11/23/2021	BORING B101
VERT.	/HORIZ.	DATUMS:	NAVD 88	3/NAD83 N	IY Zone 31	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 3 of 5
		S	ample Inf	ormation			0		
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and	Rock Description
-	-					Rollerbit to 50 feet. Rig chatter from 47 to 48 and 49 to 50 feet.			
-30	50 	× <u>s15</u>	50 to 50.3	1/2	<u>, 101/4"</u>	Rollerbit to 55 feet. Intermittent rig chatter from 50 to 55 feet / advanced slowly.		S15: NARROWLY GRADED micaceous sand, ~10% angu black wet; tight.	SAND (SP); ~90% fine to medium lar to subangular gravel up to 1";
-	55 55	S16	55 to ∖ 55.8 /	10/7	53- 100/4"	Rollerbit to 60 feet. Rig chatter from 55 to 56 and 59 to 60 feet. Boulder from 56 to 59 feet / advanced slowly.		S16: NARROWLY GRADED sand, ~5% nonplastic fines, gravel up to 1"; brown; wet; ti	SAND (SP); ~85% fine to medium ~10% subangular to subrounded ght.
- -40 -	- 60 	× <u>s17</u>	60 to 60.4	<u>5/3</u>	<u>100/5"</u>	Rollerbit to 65 feet. Intermittent rig chatter from 60 to 65 feet.		S17: NARROWLY GRADED	SAND (SP); Similar to S16.
-	65 65 	S18	65 to 65	0/0	50/0"	Spoon bounced. Rollerbit to 70 feet. Slow advance from 65 to 68 feet, quick advance from 68 to 69 feet, and slow advance from 69 to 70 feet. Just before 70 feet the advancement quickened. Spoon bounced. Rollerbit to 70 feet. Slow advance from		S18: No Recovery.	
-50 —	70 70	× <u>s19</u>	70 to 70.3	<u>4/0</u>	100/4"	65 to 68 feet, quick advance from 68 to 69 feet, and slow advance from 69 to 70 feet. Just before 70 feet the advancement quickened. @S18 Rollerbit to 75 feet. Rig chatter from 70 to 72 feet and slow advance from 72 to 73 feet.		S19: No Recovery.	
NOTE	5:						PROJ CITY/: GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL

NORT	HING (ft)	: <u>222,057</u>	, (∰)• 20.1	1		EASTING (ft):	90	2021 - 11/23/2021	BORING
VERT.	HORIZ.	DATUMS:	NAVD 88	3/NAD83 N	IY Zone 31	04 DRILLING COMPANY:	Crai	g Geotechnical Drilling	
		_				1			PAGE 4 01 5
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-	- 75	C1	73 to 76	36/11		Boulder to approx. 75 feet. Boulder to approx. 75 feet. @C1		C1: BOULDER/COBBLE; gn	eiss; core run times 4:35, 4:01, 3:31.
-	-					Rollerbit to 80 feet. Rig chatter from 76 to 77 and 79 to 80 feet.			
-60 — - 60 -	80 	S20	80 to 82	24/11	24-27- 42-44	Rollerbit to 85 feet. Intermittent rig chatter from		S20: NARROWLY GRADED fine to medium sand, ~10% r subrounded gravel up to 1.2	SAND WITH SILT (SP-SM); ~80% oonplastic fines, ~10% subangular to 5"; brown; wet; tight.
-	85 85 	S21	85 to 87	24/17	22-31- 38-30	Rollerbit to 90 feet. Rig		S21: NARROWLY GRADED to S20. STRATUM IV - DECOMPO 10) at	SAND WITH SILT (SP-SM); Similar SED/WEATHERED ROCK (CLASS ~87 ft / El66.9
- - -70 —	90 90	S22	90 to 92	24/17	19-20- 21-20	chatter from 87 to 88 feet.		'-S22: DECOMPOSED BEDR ~20% nonplastic to low-plast to 1.5"; brown, blue-green, re	OCK; ~70% fine to medium sand, [—] icity fines, ~10% subangular gravel up ddish-brown; wet; tight.
-	_ 95					Rollerbit to 95 feet.			
-		S23	95 to 97	24/21	9-10-12-14	Rollerbit to 100 feet.		S23: DECOMPOSED BEDR ~5% sand; light red; wet; tigh	OCK; ~95% low-plasticity clay, flaky, t; mica; layering.
NOTES	3:						PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants

NORT	HING (ft)	: _2 FAC	22,057 E EL. (1	it): 201			EASTING (ft): 1,003,45 DATE START/END: 1	90 1/23/2	2021 - 11/23/2021	BORING B101
VERT.	/HORIZ.	DAT	UMS:	NAVD 88	3/NAD83 N	Y Zone 3104	DRILLING COMPANY:	Crai	g Geotechnical Drilling	
	1									PAGE 5 of 5
			S	ample Inf	ormation			bo		
Elev. (ft)	Depth (ft)	Sa	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic L	Soil and	d Rock Description
-80			S24	100 to 102	Rec. (in)	9-11-16- 17			S24: DECOMPOSED BEDI blue, white-tan. Backfilled with cuttings and Bottom of boring at depth 10	ROCK; Similar to S23 but light red, hole plug. 22 ft.
-100 — - - -	120 									
							Ι			
NOTES	S:							PROJ	ECT NAME: Astoria Cove	
								CITY/ GEI P	STATE: Astoria, Queens, Ne ROJECT NUMBER: 2305370	W York GEI Consultant

E	BORIN			DRN		1				20		BORING	
	ROU	ND 9	י (tt) SUR	: <u>2</u> FAC	221,943 E EL. /I	t): 21.4			EASTING (π): 1,003,43 DATE START/END: 1	59 1/24/2	2021 - 11/24/2021	_ • •	
1	/ERT./	HO	RIZ.	DA	TUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	ig Geotechnical Drilling	B102	
1	OTAL	DE	PTH	l (ft)	: 86.8	3			DRILLER NAME: Mai	rc Aqu	ino		
L	.OGGE	ED E	BY:	G	. Holme	S			RIG TYPE: CME 75			PAGE 1 of 4	
	RILL	INC	3 IN	FOF	RMATIC	ON							
H	IAMM	ER	ТҮР	E:	Autom	natic			CASING I.D./O.D.: 4 i	nch/ 4	.5 inch CORE BAR	REL TYPE: NA	
4	UGEF	R I.C	0./0.	D.:	NA / I	NA			DRILL ROD O.D.: NN	AILL ROD O.D.: NM CORE BARREL I.D./O.D.: NA / NA			
[RILLI	NG	ME	THO	D: <u>M</u>	ud Rotary	Wash						
ľ	VAIE		EVEI		PTHS (π):							
4	ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% WOR = Weight of Rods WOH = Weight of Hammer								S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger	S = Split-Spoon Sample Qp = Pocket Penetrometer Strength C = Core Sample Sv = Pocket Torvane Shear Strength U = Undisturbed Sample LL = Liquid Limit SC = Sonic Core PL = Plasticity Limit DP = Direct Push Sample PID = Photoionization Detector HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diame		NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. Jiameter	
	Sample Information									bc			
E	Elev.	De	pth	_			Pen./	Blows	Drilling Remarks/	lic Lo	Soil and	Rock Description	
	(ft)	(1	ft)	S	ample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	iraph			
						()	(in)	or RQD		0	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 21.4	
	- 20			M	S1	0 to 2	24/11	12-4-8-9			0-2": ASPHALT. S1: SILTY SAND (SM); ~70% ~20% nonplastic fines, ~10% 1.5"; brown; dry; tight; brick fr	o fine sand, fine to medium at base, angular to subangular gravel up to agments.	
	_			M	S2	2 to 4	24/12	11-4-6-3			S2: NARROWLY GRADED S sand, ~5% nonplastic fines; c	SAND (SP); ~95% fine to medium range-brown; dry; loose.	
3	-		5	M	S3	4 to 6	24/13	7-5-2-2			S3: NARROWLY GRADED S	ED SAND (SP); Similar to S2.	
3.GDT 12/22/2				\square	S4	6 to 8	24/10	4-4-3-2			S4: SILTY SAND (SM); ~60% light brown; dry; loose.	6 fine sand, ~40% nonplastic fines;	
EMPLATE 201	-	_		M	S5	8 to 10	24/0	63-31- 53-42	Drive casing to 8 feet below grade. Rollerbit to 8 feet.		S5: No Recovery. Shoe brok	e off of spoon.	
VE.GPJ GELDATA I	-		10	/ \					Rollerbit to 15 feet. Rig chatter from 8 to 12 feet. Drive casing to 15 feet. Rollerbit to 15 feet.				
D-ASTORIA CO	-										STRATUM IIC - SAND	(CLASS 3B) at ~13.5 ft / El. 7.9 	
SAPHIC LOG 230537	-		15	X	S6	15 to 17	24/5	10-8-11- 10			S6: NARROWLY GRADED S to medium sand, ~10% nonpl brown; wet; loose.	SAND WITH SILT (SP-SM); ~85% fine lastic fines, ~5% fine gravel; light	
LD 6-NOR TH-EAS I-GF	-	-							Rollerbit to 20 feet. Rig chatter from 17 to 18 feet. Drive casing to 20 feet. Rollerbit to 20 feet.		STRATUM III - TILL (0	CLASS 3A) at ~18.5 ft / El. 2.9	
	IOTES:									PROJ CITY/ GEI P	ECT NAME: Astoria Cove STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL Consultants	

NORTI GROU	HING (ft) ND SUR	: _221,943 FACE EL. (3 (ft): 21.4			EASTING (ft): 1,003,43	39 1/24/2	021 - 11/24/2021	BORING B102
VERT.	/HORIZ.	DATUMS:	NAVD 88	3/NAD83 N	Y Zone 310	DA DRILLING COMPANY:	Crai	g Geotechnical Drilling	PAGE 2 of 4
		5	Sample Inf	ormation			Ď		1
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and	Rock Description
0	-	S7	20 to 22	24/13	17-17- 18-18	Rollerbit to 25 feet. Drive casing to 25 feet. Rollerbit to 25 feet.		S7: SILTY SAND (SM); ~60 [°] mixed in, ~30% nonplastic fil gravel up to 1.25"; brown; we	% fine sand, some micaceous sand nes, ~10% subangular to subrounded at, tight.
	- 25 - 25 	S8	25 to 27	24/15	20-21- 15-15	Added quick-gel to drilling mud. Rollerbit to 30 feet. Rig chatter from 28 to 30 feet. Drive casing to 30 feet. Rollerbit to 30 feet.		S8: NARROWLY GRADED to medium sand, ~10% nonp subrounded gravel up to 1";	SAND WITH SILT (SP-SM); ~80% fine plastic fines, ~10% subangular to brown; wet; tight.
- -10	- 30 - 30 	S9	30 to 32	24/15	14-37- 72-66	Rollerbit to 35 feet. Rig chatter from 31 to 35 feet.		S9: NARROWLY GRADED sand, ~5% nonplastic fines, to 1.5"; brown; wet; tight.	SAND (SP); ~85% fine to medium ~10% angular to subangular gravel up
	_ 35 - 35 	S10	35 to 37	24/14	27-42- 51-65	Rollerbit to 40 feet. Rig chatter from 36 to 40 feet.		S10: NARROWLY GRADED fine to medium sand, ~20% s 1.5"; brown; wet; tight.	9 SAND WITH GRAVEL (SP); ~80% subangular to rounded gravel up to
	- - - - - - - - - - - - -	S11	40 to 40.9	11/7	59- 100/5"	Drive casing to 35 feet. Rollerbit to 45 feet. Intermittent rig chatter from 41 to 45 feet.		S11: NARROWLY GRADED sand, ~5% nonplastic fines, up to 0.75"; brown; wet; tight	9 SAND (SP); ~90% fine to medium ~5% subangular to subrounded gravel
- - NOTES	- 45 	S12	45 to 45	0/0	50/0"	Rollerbit to 50 feet. Rig chatter from 45 to 46 and 48	PROJ	S12: No Recovery.	
							CITY/S GEI P	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	v York GEL Consultants





Geotechnical Report Astoria Cove Development Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York December 2023

Appendix B

Well Installation Logs

Gro	undwater Well	Installation Log	B5(OW)
Project	Astoria Cove - Phas	e l	GEI Proj. No. 2305370
City / Town	Astoria, NY		Location Bldg. 5
Client	SLIM Astoria 2468 L	LC	N: 221777.8 ft E: 1003344.4 ft
Contractor	Craig Test Boring		
Driller	J. Schuster	GEI Rep. G. Holmes	Install Date 3/9/2020
Survey Datum:	NAVD88	Length of Surface Casing	above Ground NA
Ground Elevation:	24.8	Dist. Top of Surf. Casing t	o Top of Riser Pipe NA
		Type and Thickness of Se around Surface Casing	al concrete + 1" of cold patch
		ID of Surface Casing Type of Surface Casing	4 in. Diam. Handhole
	▏▏ ▋▎▎	Depth Bottom of Surface 0	Casing NA
		ID and OD of Riser Pipe Type of Riser Pipe	2 in. ID / 2.4 in. OD Sch. 40 PVC
		← Type of Backfill around Ri	ser Pipe cuttings
		← Diameter of Borehole	4 in. ID / 4.5 in. OD
		Depth Top of Seal	11 ft
	Scale	Type of Seal	HOLEPLUG 3/8 in.
			<u> </u>
	s (No	Depth Top of Screened Se	ection 15 ft
	5 Sand w/	Type of Screen	Sch. 40 PVC
		Description of Screen Ope	enings 20 slot
2020 :00 .75	eral Soil		
3/10/ 07 18	ອີ ຫຼັ Sand w/ Silt	← Type of Filter Material	FILPRO Sand (WG #1)
ate ne ipe		Depth Bottom of Screened	l Section 25 ft
Tir D		Depth Bottom of Silt Trap	25.9 ft
top of I		Depth Bottom of Filter Mat	erial 27 ft
Note		Depth Top of Seal	<u> </u>
d∎		Depth Bottom of Seal	<u> </u>
nce to		Type of Backfill below Filte	er Material cuttings
Dista	 L	Bottom of Borehole	102 ft
Notes:		GEL	

Gro	undwater Well Insta	allation Log	B8(OW)
Project	Astoria Cove - Phase I		GEI Proj. No.	2305370
City / Town	Astoria, NY		Location Bld	g. 4
Client	SLIM Astoria 2468 LLC		N: 221818.8 ft E:	1003199.4 ft
Contractor	Craig Test Boring			
Driller	J. Schuster GEI F	Rep. G. Holmes	Install Date	3/5/2020
Survey Datum:	NAVD88 /	Length of Surface Casing at	bove Ground	NA
Ground		Dist. Top of Surf. Casing to	Top of Riser Pipe	NA
Elevation.		Type and Thickness of Seal around Surface Casing	l co	ncrete + 1" of cold patch
	←	ID of Surface Casing Type of Surface Casing	4 i	n. Diam. Handhole
	│ │	Depth Bottom of Surface Ca	asing	NA
		ID and OD of Riser Pipe	2	in. ID / 2.4 in. OD Sch. 40 PVC
		Type of Backfill around Rise	er Pipe	cuttings
		Diameter of Borehole	4	in. ID / 4.5 in. OD
		Depth Top of Seal		16 ft
		Type of Seal	Н	IOLEPLUG 3/8 in.
	A K I	Depth Bottom of Seal		18 ft
0 20		Depth Top of Screened Sec	tion	20 ft
9/20: 06:4(is and Gravel	Type of Screen		Sch. 40 PVC
3/6		Description of Screen Open	ings	20 slot
6/2020 2:30 18.3	eneral Soil		<u></u>	11. 10 / 2. + 11. OD
3/5	Sand w/ Silt ■ K	Type of Filter Material	FIL	PRO Sand (WG #1)
ate The me		Depth Bottom of Screened S	Section	30 ft
Tir D		Depth Bottom of Silt Trap		30.4 ft
top of r		Depth Bottom of Filter Mater	rial	32 ft
No la		Depth Top of Seal		N.A.
be		Type of Seal Depth Bottom of Seal		<u>N.A.</u> N.A.
ice to _		Type of Backfill below Filter	Material	cuttings
Dista		Bottom of Borehole		85 ft
<u>Notes:</u>				

Gro	undwater Well Installatior	ו Log B ^r	16(OW)
Project	Astoria Cove - Phase I	GEI Proj. No.	. 2305370
City / Town	Astoria, NY	Location	Bldg. 3A/3B
Client	SLIM Astoria 2468 LLC	N: 221903.5 f	t E: 1003498.9 ft
Contractor	Craig Test Boring		
Driller	J. Schuster GEI Rep. (G. Holmes Install Date	3/3/2020
Survey Datum:	NAVD88 / Length	of Surface Casing above Ground	NA
Ground Elevation:	21.9 Dist. To	op of Surf. Casing to Top of Riser Pipe	e <u>NA</u>
	Type a around	nd Thickness of Seal I Surface Casing	concrete + 1" of cold patch
	← ID of S Type o	urface Casing f Surface Casing	4 in. Diam. Handhole
	Depth	Bottom of Surface Casing	NA
	ID and	OD of Riser Pipe	2 in. ID / 2.4 in. OD
	Туре о	f Backfill around Riser Pipe	cuttings
·		ter of Borehole	4 in. ID / 4.5 in. OD
		Top of Seal	16 ft
	Type o	f Seal	HOLEPLUG 3/8 in.
		Bottom of Seal	18 ft
0 20		Top of Screened Section	22 ft
0/20: 7:10 17.3	5 Sand w/ 5 Gravel Type o	f Screen	Sch. 40 PVC
3/1 0	Descrip	ption of Screen Openings	20 slot
/2020 2:30 7.3	neral Soil		2 111. 10 / 2.4 111. 00
3/5 1: 1	ଓଁ Sand I I I Type o	f Filter Material	FILPRO Sand (WG #1)
ate ne ipe	Depth	Bottom of Screened Section	32 ft
Tir D	Depth	Bottom of Silt Trap	32.4 ft
top of	← Depth	Bottom of Filter Material	32 ft
No	Depth	Top of Seal	N.A.
₽	l ype o Depth	f Seal Bottom of Seal	<u> </u>
ice to _		f Backfill below Filter Material	cuttings
Distar	L Bottom	n of Borehole	102 ft
<u>Notes:</u>			

Gro	undwater Well Ins	tallation Log	B	29(OW)
Project	Astoria Cove - Phase I		GEI Proj. No.	2305370
City / Town	Astoria, NY		Location	Bldg. 2
Client	SLIM Astoria 2468 LLC		N: 222065.7 f	t E: 1003319.7 ft
Contractor	Craig Test Boring			
Driller	J. Schuster GE	G. Holmes	Install Date	3/17/2020
Survey Datum:	NAVD88	Length of Surface Casing a	above Ground	NA
Ground Elevation:	219	Dist. Top of Surf. Casing to	o Top of Riser Pip	e <u>NA</u>
		Type and Thickness of Sea around Surface Casing	al	concrete + 1" of cold patch
	←	ID of Surface Casing Type of Surface Casing		4 in. Diam. Handhole
		Depth Bottom of Surface C	asing	NA
		ID and OD of Riser Pipe		2 in. ID / 2.4 in. OD
		Type of Backfill around Ris	er Pipe	cuttings
· · · · · · · · ·		Diameter of Borehole		4 in. ID / 4.5 in. OD
		Depth Top of Seal		11 ft
		Type of Seal		HOLEPLUG 3/8 in.
		Depth Bottom of Seal		13 ft
	Is (Not	Depth Top of Screened Se	ction	15 ft
	Sand	Type of Screen		Sch. 40 PVC
		Description of Screen Ope	nings	20 slot
3/2020 7:55 6.9	Sand Sand			2 111. 107 2.4 111. 00
3/18 0		Type of Filter Material		FILPRO Sand (WG #1)
ate ne		Depth Bottom of Screened	Section	25 ft
Da Tin iser pi		Depth Bottom of Silt Trap		25.4 ft
top of I		Depth Bottom of Filter Mate	erial	27 ft
No		Depth Top of Seal		N.A.
be		i ype of Seal Depth Bottom of Seal		<u>N.A.</u> N.A.
ice to		Type of Backfill below Filte	r Material	cuttings
Distar		Bottom of Borehole		85 ft
<u>Notes:</u>				GEI

Gro	undwater Wel	Installation Log	B48(OW)
Project	Astoria Cove - Pha	sel	GEl Proj. No. 2305370
City / Town	Astoria, NY		Location Bldg. 1
Client	SLIM Astoria 2468	LLC	N: 222443.4 ft E: 1003171.0 ft
Contractor	Craig Test Boring		
Driller	E. Flanagan	GEI Rep. G. Holmes	Install Date 5/20/2020
Survey Datum:	NAVD88 /	Length of Surface Casing	above Ground NA
Ground		Dist. Top of Surf. Casing	o Top of Riser Pipe NA
Elevation:		Type and Thickness of Se around Surface Casing	eal concrete + 1" of cold patch
		Type of Surface Casing	4 in. Diam. Handhole
		Depth Bottom of Surface	Casing NA
		ID and OD of Riser Pipe Type of Riser Pipe	2 in. ID / 2.4 in. OD Sch. 40 PVC
		K──── Type of Backfill around Ri	ser Pipe cuttings
		Contraction Diameter of Borehole	4 in. ID / 4.5 in. OD
		Depth Top of Seal	6 ft
		Type of Seal	HOLEPLUG 3/8 in.
		Depth Bottom of Seal	<u> </u>
	s (Not	Depth Top of Screened S	ection 10 ft
	oit Silty Sand	Type of Screen	Sch. 40 PVC
	, ond	Description of Screen Op	enings 20 slot
/2020 :00	leral Soil C	ID and OD of Screened S	ection2 in. ID / 2.4 in. OD
5/21. 07	Ger	← Type of Filter Material	FILPRO Sand (WG #1)
be e	Bedrock	Depth Bottom of Screened	d Section 20 ft
Da Tim		Depth Bottom of Silt Trap	20.4 ft
op of r		Cepth Bottom of Filter Ma	terial 22 ft
ow t		Depth Top of Seal	N.A.
pel		Type of Seal	<u> </u>
e to _		Deptn Bottom of Seal	N.A.
istanc			er Material cuttings
	⊾	Bottom of Borehole	34 ft
<u>Notes:</u>			GEL

Groundwater Well Installation Log			B53(OW)
Project	Astoria Cove - Phase I		GEI Proj. No. 2305370
City / Town	Astoria, NY		Location School
Client	SLIM Astoria 2468 LLC		N: 221759.0 ft E: 1003420.7 ft
Contractor	r Craig Test Boring		
Driller	E. Flanagan	GEI Rep. G. Holmes	Install Date 5/14/2020
Survey Datum:	NAVD88	Length of Surface Casing	above Ground NA
Ground		Dist. Top of Surf. Casing t	o Top of Riser Pipe NA
Elevation:	29.4	← Type and Thickness of Se around Surface Casing	al concrete + 1" of cold patch
		← ID of Surface Casing Type of Surface Casing	4 in. Diam. Handhole
		Depth Bottom of Surface 0	Casing NA
		ID and OD of Riser Pipe	2 in. ID / 2.4 in. OD Sch. 40 PVC
		Type of Backfill around Ris	ser Pipecuttings
F-T-T-1		Diameter of Borehole	4 in. ID / 4.5 in. OD
		Depth Top of Seal	16 ft
		Type of Seal	HOLEPLUG 3/8 in.
		Depth Bottom of Seal	18 ft
	Is (Not	Depth Top of Screened Se	ection 20 ft
	· · · · · · · · · · · · · · · · · ·	Type of Screen	Sch. 40 PVC
		Description of Screen Ope	enings 20 slot
5/2020 9:30 3:5	neral Soil (
5/15 01 2	Gravel	← Type of Filter Material	FILPRO Sand (WG #1)
late me bipe		Depth Bottom of Screened	Section 30 ft
Ti Tiser p		Depth Bottom of Silt Trap	30.4 ft
/ top of		Depth Bottom of Filter Mat	erial <u>22 ft</u>
elow		Type of Seal	<u> </u>
a ▶		Depth Bottom of Seal	N.A.
ance to		Type of Backfill below Filte	er Material cuttings
Dista	 L	Bottom of Borehole	55 ft
<u>Notes:</u>			GEL

Geotechnical Report Astoria Cove Development Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York December 2023

Appendix C

Laboratory Testing Results





Checked By: EF















Checked By: EF



















Checked By: EF




Checked By: W. Lukas



Checked By: EF





Checked By: W. Lukas



