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# Geotechnical Report Astoria Cove Development

Buildings 3A/3B, 4 & 5 Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909 Lot 35 Astoria, Queens County, New York

#### Submitted to:

Slim Astoria 2468 LLC c/o CAD3 Construction and Development 571 McDonald Avenue, 3rd Floor Brooklyn, NY 11218

#### Submitted by:

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## **Recommended Material Specifications**

- Structural Fill
- Ordinary Fill
- Crushed Stone
- Peas Stone
- Controlled Low Strength Material
- Geotextile

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# **Executive Summary**

GEI Consultants, Inc., P.C. prepared this report to present the results of a subsurface exploration program and recommendations for the design of foundation supports for the design of Phase 1 of a redevelopment project in the Astoria neighborhood of Queens, New York. Phase 1 of the redevelopment project consists of four new mixed-use buildings between 6 and 26 stories high with cellar/basement footprints between 14,630 ft<sup>2</sup> and 37,700 ft<sup>2</sup>, designated as Building 3A/3B, Building 4, and Building 5.

The top of the cellar/basement slab elevations for Buildings 3A/3B, Building 4, and Building 5 are at El. 6.00, El. 8.46, and El. 22.50, respectively. Localized top of mat for elevator and mechanical pits are called out as El. 2.46 in Building 4 and either El. 22.00 or El. 12.50 in Building 5. General excavations between 11 to 16 feet, 12 to 35 feet, and 2 to 28 feet will be required for Building 3A/3B, Building 4, and Building 5, respectively.

The geotechnical exploration program consisted of 27 geotechnical test borings advanced between about 55 and 102 feet below the ground surface. Subsurface conditions encountered in the borings were generally medium dense to dense miscellaneous granular fill, over discontinuous loose sand, over discontinuous stiff silt/clay, over medium dense sand, over very dense glacial till, over very dense soil-like weathered/decomposed rock, over metamorphic gneiss bedrock. Till soils were generally between 49 and 64 feet thick.

We recommend that the proposed buildings be supported on either a mat foundation or spread and continuous wall footings bearing on the Stratum III dense glacial till soils (Class 3a) or the overlying Stratum IIc sandy soils (Class 3b) or compacted Crushed Stone extending to Stratum III soils or compacted Structural Fill extending to Stratum IIc soils. The maximum allowable design bearing pressures that are recommended range from 3 to 8 tons/ft² as discussed in greater detail in this report. We recommend a unit value for the modulus of vertical subgrade reaction (for a 1-foot loaded area) of 300 tons/ft³ and 130 tons/ft³ for mat foundations bearing in Stratum III and Stratum IIc soils, respectively.

Alternatively, drilled pressure-grouted micropiles also can be considered to support the tower or podium cores for Building 3A/3B deriving support capacity in the dense glacial till soils. Micropiles can be designed to achieve 200-ton compression and 100-ton tension capacity and could also be used as tiedown anchors for the tower core for tension capacity if shallow foundations are used for the entire building footprint.

Groundwater levels were generally observed between El. 1.1 and El. 4 for wells located near Building 3A/3B, between El. 2.9 and El. 4.8 for wells located near Building 4, and between

El. 3.5 and El. 6.2 for wells located near Building 5. Therefore, for the design of the floor slabs and foundation walls, we recommend design groundwater elevations for Building 3A/3B, Building 4, and Building 5 as El. 5, El. 6, and El. 7, respectively. Additionally, well point or sump pit dewatering may be required during construction if excavations are performed to reach bearing strata.

We recommend using Site Class D (Stiff Soil Profile ) for seismic design purposes, in accordance with Building Code § 1613.5.2. Based on the site geology, topography and subsurface investigations reported herein, it is our opinion that the proposed buildings are not susceptible to damage from liquefaction, slope instability, lateral spreading, or surface rupture due to faulting.

## 1. Introduction

GEI Consultants, Inc., P.C. prepared this report to present the results of a subsurface exploration program and foundation recommendations for the design of Phase 1 of a redevelopment project in the Astoria neighborhood of Queens, New York (Fig. 1). Phase 1 of the redevelopment project consists of four new mixed-use buildings, designated Building 3A/3B, Building 4, and Building 5. Slim Astoria 2468 LLC of Brookyln, New York engaged GEI to provide geotechnical services for this project.

## 1.1 Scope of Services

GEI completed the following scope of services for this report. These services were performed to investigate the subsurface conditions at the site.

- Reviewed site plans and preliminary building layout drawings.
- Engaged a test boring contractor to drill 27 geotechnical test borings and install six observation wells in the support of Building 3A/3B, Building 4, Building 5 for geotechnical design.
- Performed split spoon sampling and cumulatively collected about 68 feet of rock cores.
- Evaluated soil and rock samples recovered from the test borings and prepared test boring logs.
- Performed geotechnical index testing on soil samples from the test borings.
- Evaluated the data obtained and prepared this Geotechnical Report.

We performed these services in general accordance with the 2014 edition of the New York City Building Code (Building Code), which is an adaptation of the 2009 International Building Code.

#### 1.2 Authorization

Our work was performed in general accordance with our revised proposal dated November 6, 2023, executed and authorized by Mr. Dov Strohli of Slim Astoria 2468 LLC.

## 1.3 Elevation Datum and Horizontal Coordinate System

Elevations provided in this report are in feet and are referenced to the North American Vertical Datum of 1988 (NAVD88). Horizontal coordinates (northing and easting) are U.S. State Plane coordinates in U.S. survey feet and are referenced to the North American Datum of 1983 (NAD83), New York State Plane Long Island Zone (3104).

## 1.4 Site Description

The site is currently used by various contractors to store equipment and materials and is irregularly shaped. Two vacant 1-story buildings are present on the northern portion of the site. The site is bounded to the north by the East River, to the west by one open lot and three buildings between 1- and 13-stories high (13-story building under construction), to the south by a 4-story building with three cellar levels and 27<sup>th</sup> Avenue beyond, and to the east by 9<sup>th</sup> Street, and eight buildings between 3- and 6-stories high.

Existing ground surface elevations vary between about El. -3 near the East River and up to El. 55 near the south portions of the site. An approximately 3H:2V slope exists from the East River to the vacant 1-story buildings and another approximately 2H:1V slope existing from south of 26<sup>th</sup> Avenue to north end of 8<sup>th</sup> Street. Fig. 2 depicts the general elevations at the site.

A Phase I Environmental Site Assessment (ESA) for the site indicates that the two vacant buildings were constructed in 1935 and 1943. Historical Sanborn Fire Insurance Maps for the site included in the Phase I ESA indicate that in 1898, 1915, and 1928, as many as 20 structures were once located at the site. It is likely that the foundations for many of those structures remain buried at the site after the demolition of the buildings.

# 1.5 FEMA Flood Mapping

The Federal Emergency Management Agency's (FEMA) Effective Flood Insurance Rate Map (FIRM), number 3404970093F dated September 5, 2007, shows that the site is predominantly outside the Special Flood Hazard Area (SFHA) and the Area of Minimal Flood Hazard (Zone X). There is a portion of the site closer to the East River that is identified as the 1 percent annual chance flood Zone AE (base flood elevation for inundation at El. 12). Properties to the west of the site are also in the SFHA designated as Zone AE (base flood elevation for inundation at El. 12) and Zone X.

From the New York City Department of City Planning updated flood maps (based on preliminary updated FIRMs, number 3604970093G, from FEMA in 2013), the site is in an Area of Minimal Flood Hazard (Prelim Shaded Zone X) with a 0.2 percent annual chance of

flooding (moderate flood hazard). There is a portion of the site closer to the East River that is identified as the 1 percent annual chance flood Zone AE (base flood elevation for inundation at El. 14) with a limited area for Moderate Waver Action. Properties to the west of the site are also in the SFHA designated as Zone AE (base flood elevation for inundation at El. 12) and Zone X.

The site is classified as Flood Design Class 2 per ASCE 24. However, none of the proposed structures are in the flood zone and not subject to Appendix G of the 2014 Building Code. Regardless, if ownership decides to design buildings for design flood elevations, any structures located below the base flood elevation should be floodproofed in accordance with the Building Code, New York State Building Code, ASCE 24, and all other agency requirements having jurisdiction. At a minimum, 1 foot of free board must be provided above the controlling base flood elevation.

## 1.6 Project Description

The proposed Phase 1 redevelopment involves construction of four new mixed-use buildings, designated Building 3A/3B, Building 4, and Building 5. Buildings 3A/3B will be located on Block 906 and Lots 1 (08-01 26<sup>th</sup> Avenue) and 5 (08-51 26<sup>th</sup> Avenue); Building 4 will be located on Block 909 and Lot 35 (4-34 26<sup>th</sup> Avenue); and Building 5 is location on Block 908 and Lot 12 (26-10 9<sup>th</sup> Avenue).

<u>Buildings 3A/3B</u> – Buildings 3A/3B will include a 26-story tower in the northwest corner of the building and an 8- to 9-story building podium with a full cellar. Buildings 3A/3B cellar footprint is approximately 37,700 ft<sup>2</sup> in area. Top of slab elevation for proposed the basement/cellar is at El. 6.00. The cellar will be used for parking, possible residential amenities, and mechanical rooms; the 1<sup>st</sup> floor will be used for parking, commercial retail space, and residential space; and floors 4 through 26 will be used for residential space and amenities.

The excavation for the cellar will extend down to approximately El. 6, which is approximately 7 to 15 feet below the existing ground surface.

The project Structural Engineer, Desimone Consulting Engineers, has provided the following preliminary service loads:

- Building 3A
  - o Podium West Core: 3,000 kips Dead / 600 kips Live
  - o Podium East Core: 3,000 kips Dead / 600 kips Live
  - o Typical Podium Column: 600 kips Dead / 220 kips Live

o East Wall: 1,600 kips Dead / 400 kips Live

#### Building 3B:

- o Tower Core: 13,000 kips Dead / 600 kips Live
- o Typical Tower Interior Column: 1,150 kips Dead / 450 kips Live
- o Podium East Core: 2,100 kips Dead / 450 kips Live
- o Typical Podium Column: 600 kips Dead / 220 kips Live
- o West Wall: 3,800 kips Dead / 800 kips Live

Building 4 — Building 4 will consist of an 8-story building with a full cellar (about 21,192 ft<sup>2</sup> in area) below most of the building footprint. Building 4 has a footprint area of approximately 23,650 ft<sup>2</sup>. Top of mat elevation for the proposed cellar is at El. 8.46 with a localized elevator pit top of mat at El. 2.46. The cellar will be used for parking and mechanical rooms; the 1<sup>st</sup> floor will be used for mechanical rooms, commercial retail space, and residential space; and floors 2 through 8 will be used for residential space.

The excavation for the cellar will extend down to approximately El. 8.5, which is approximately 12.5 to 39 feet below the existing ground surface.

Desimone Consulting Engineers has provided the following preliminary service loads:

#### • Building 4:

- o Tower Core: 3,600 kips Dead / 450 kips Live
- o Typical Tower Interior Column: 550 kips Dead / 110 kips Live
- o Podium Core: 800 kips Dead / 100 kips
- o Typical Tower Perimeter Column: 250 kips Dead / 50 kips Live
- o Typical Interior Column: 400 kips Dead / 800 kips Live
- o Typical Podium Perimeter Column: 200 kips Dead / 30 kips Live

Building 5 — Building 5 will consist of a 6-story building (with penthouse) with a walk-out basement at grade in the northern portion of the building and extending to the southern portion of the building as a full cellar approximately 28.5 feet below the existing ground surface, due to the rise in elevation on the southern portion of the site. Building 5 will have a footprint of approximately 14,630 ft<sup>2</sup>. Top of slab elevation for the proposed basement/cellar is at El. 22.50 with a localized elevator pit top of mat at El. 22.00 and a mechanical pit top of mat at El. 12.50. There will be a mechanical pit in the northeast corner of the building with an approximate footprint of 820 ft<sup>2</sup>. The cellar/basement will be used for mechanical rooms

and residential space, and floors one through six and the penthouse will be used for residential space.

The excavation for the basement/cellar will extend to approximately El. 21.5 for the majority of the building except for the mechanical pit where an excavation to about El. 11.5 will be required, which is approximately 3 to 27.5 feet below the existing ground surface.

Desimone Consulting Engineers has provided the following preliminary services reactions:

#### • Building 5:

- o Tower Core: 2,000 kips Dead / 250 kips Live
- o Typical Tower Interior Column: 250 kips Dead / 50 kips Live
- o Podium Core: 600 kips Dead / 50 kips
- o Typical Tower Perimeter Column: 200 kips Dead / 25 kips Live
- o Typical Interior Column: 550 kips Dead / 100 kips Live
- o Typical Podium Perimeter Column: 120 kips Dead / 25 kips Live

The anticipated foundation supports for Buildings 3A/3B, Building 4, and Building 5 are shallow foundations under most of the footprint with deep foundation element supports under the 26-story tower at Building 3B.

# 2. Subsurface Explorations

## 2.1 Exploration Program

The geotechnical exploration program consisted of 27 test boring that were performed in two mobilizations. Geotechnical test borings B1 through B23, B29, and B53 (B000-series borings) were drilled by Craig Geotechnical Drilling Co., Inc. of Mays Landing, New Jersey between March 2 and June 12, 2020. Geotechnical test borings B101 and B102 (B100-series borings) also were drilled by Craig Drilling on November 23 and 24, 2021.

The borings were drilled to depths between 55 and 102 feet below the existing ground surface, which translates to between El. -53.2 to El. -81.9. Approximate exploration locations are depicted in Fig. 2 and the exploration program is summarized in Table 1. Test boring logs as well as GEI's visual-manual description guide are included in Appendix A.

The test borings were advanced using either a CME 75 truck-mounted drilling rig or a CME 55LC track-mounted drilling rig with mud rotary drilling (bentonite-based), and 4-inch I.D. casing used to support the borehole.

A GEI geotechnical field representative observed and documented the geotechnical drilling and sample collection.

Standard Penetration Testing (SPT) and split-spoon sampling, in accordance with ASTM D1586, were generally performed continuously in the upper 12 feet of each boring and at 5-foot intervals, thereafter, using an automatic hammer.

Representative samples of the soils obtained from the borings were classified in accordance with ASTM D2488. Rock coring was performed using NX barrels in boreholes B8, B10, B11, B14, B15, B20, B23, and B101. Split-spoon samples were placed in appropriately identified sealed glass jars and rock cores were placed in wood core boxes. All samples were stored on-site.

Six groundwater observation wells were installed in completed borings B5, B8, B16, B29, B48 (off site well), and B53. Well construction details are presented in Appendix B.

An additional eight monitoring wells were installed in subsequent environmental investigations conducted in 2020 and are shown in Fig. 2. Well construction details are presented in Appendix B.

As-drilled boring locations were documented based on taped off measurements from existing site features. Approximate ground surface elevations at the exploration locations were estimated from the contours provided on Survey Number 44888-6 titled, "Survey of City of New York, County Queens, Tax Blocks as Shown," prepared by Montrose Surveying Co., LLP and revised October 27, 2023.

The boreholes were backfilled with drill cuttings and grout, with excess cuttings spread at the surface in the vicinity of the borehole.

# 3. Subsurface Conditions

#### 3.1 Subsurface Conditions

Seven generalized strata were encountered in the test borings we performed and are described below, starting from the ground surface, and summarized in Table 1. Subsurface profiles based on the exploration information are presented in Figs. 3 through 6.

The drilling contractor used either a safety hammer with rope and cathead lifting system or an automatic hammer to perform the SPTs to measure the N-values in the field (termed  $N_{\rm m}$ ), which resulted in a variation in the efficiency of delivering the theoretical energy of a 140-pound weight freefalling 30 inches.

To use the data set in design, the  $N_m$  values for the automatic hammer were normalized to 60 percent of the theoretical energy delivered in the SPT (termed  $N_{60}$ ), assuming that the automatic hammer was conservatively 80 percent efficient. The corrected "design" N-values are provided in Table 2. Since the  $N_m$  values obtained from a safety hammer with rope and cathead lifting system are typically 60 percent of the theoretical energy delivered in the SPT, no corrections were made on those SPTs.

The soil conditions are known only at the exploration locations. Conditions between exploration locations may vary significantly from the descriptions given below. The Building Code Material Classifications are given for each layer in paratheses.

- Stratum I Fill (Class 7)
- Stratum IIa Loose Sand (Class 6)
- Stratum IIb Silt/Clay (Class 4b/5b)
- Stratum IIc Sand (Class 3b)
- Stratum III Till (Class 3a)
- Stratum IV Decomposed/Weathered Rock (Class 1d)
- Stratum V Bedrock (Class 1b/1c)

Stratum I – Fill (Class 7) – Fill soils were encountered at all exploration location below up to 8 inches of asphalt or 12 inches of concrete. The thickness of the fill ranged from about 6 inches to 23.5 feet, but was generally 2 to 10 feet thick. The fill generally consisted of brown silty sand with varying amounts of glass, wood, bricks, and other construction debris.

SPT  $N_{60}$ -values ranged from 3 blows per foot to split spoon refusal, but were generally between 16 and 41 blows per foot, and averaged approximately 34 blows per foot, indicating generally dense soil conditions.

Stratum IIa – Loose Sand (Class 6) – Loose sand deposits were encountered in 10 of the 25 test borings performed below the fill and were observed to be between about 4 and 11.5 feet thick at the boring locations. The loose sand deposits consist primarily of narrowly and widely graded sand with up to about 10 percent nonplastic fines and up to 10 percent fine gravel. In B21, we observed that the loose sand layer contained up to 20 percent low plasticity clayey fines and up to about 30 percent gravel.

SPT N<sub>60</sub>-values ranged from 3 to 24 blows per foot, but were generally between 7 and 11 blows per foot, and averaged approximately 9 blows per foot, indicating generally loose soil conditions.

Stratum IIb – Silt/Clay (Class 4b/5b) – Silt and clay deposits were encountered in B5, B9, and B21 below the fill and loose sand and were observed to be 9.5, 2, and 5 feet thick at the respective boring locations. The silt deposit observed at B5 consists of light brown nonplastic fines with up to 40 percent fine sand and included about a 3-foot-thick silty sand interlayer containing up to 40 percent nonplastic fines. The clay deposit observed at B21 consists of medium plasticity black fat clay with up to 10 percent gravel and up to 5 percent fine sand.

This stratum is likely discontinuous since the silt deposit encountered in B5 at about El. 21 and the clay deposit was encountered in B21 at about El. -14.5.

SPT  $N_{60}$ -values ranged from 9 to 25 blows per foot, but were generally between 13 and 19 blows per foot, and averaged approximately 16 blows per foot, indicating generally stiff to very stiff soil conditions.

<u>Stratum IIc – Sand (Class 3b)</u> – Sand deposits were encountered in all but four test borings performed below the fill and were observed to be between about 4 and 11.5 feet thick at the boring locations. Stratum IIc generally consists of orange-brown to tan narrowly graded sand with up to 40 percent nonplastic fines and up to 30 percent fine gravel. We observed the sand grade towards a silt before transitioning to Till in B3 and B5.

The top of sand contour plan presented in Fig. 7 depicts the estimated sand stratum surface as the top of sand drops in elevation towards the north portions of the site.

SPT  $N_{60}$ -values ranged from 7 blows per foot to split spoon refusal, but were generally between 14 and 21 blows per foot, and averaged approximately 20 blows per foot, indicating generally medium dense soil conditions.

Stratum III – Till (Class 3a) – Below the Stratum II deposit, a layer of dense glacial till was encountered at depths between 8 and 33.5 feet below grade corresponding to between El. -19.5 and El. 37.4. The top of till contour plan presented in Fig. 8 depicts the estimated till stratum surface as the top of till drops in elevation towards the north and northeast portions of the site.

Stratum III generally consists of brown silty sand, narrowly graded sand, widely graded sand, and narrowly graded gravel with up to 45 percent nonplastic to low plasticity fines and up to 60 percent gravel. The till was observed to be between 39 and 87 feet thick, but was generally between 49 and 64 feet thick and averaged 60 feet thick. Eleven test borings (B1, B3, B6, B7, B14, B17, B19 through B22, and B102) were terminated in the till.

Cobbles and boulders were encountered in the till in all test boring locations observed through rock core runs or observation of drilling rig reaction.

Light brown nonplastic silt was encountered in B3, B6, B16 with up to 25 percent sand.

We encountered spilt spoon refusal on a majority of the SPTs performed and assumed that this was due to the cobbles and boulders present in the till. Excluding the SPTs that met split-spoon refusal, SPT  $N_{60}$ -values ranged from 17 to over 100 blows per foot, but were generally between 49 and over 100 blows per foot, and averaged approximately 77 blows per foot, indicating very dense soil conditions.

<u>Stratum IV – Decomposed/Weathered Rock (Class 1d)</u> – Decomposed/weathered rock underlies the till layer and was encountered in 14 test borings at depths between 53 and 98 feet below grade corresponding to between El. -30.7 and El. -68.3. The decomposed/weathered rock was observed to be between 8.5 and 30 feet thick. Ten test borings (B2, B4, B5, B8, B9, B12, B13, B16, B18, and B101) were terminated in the decomposed/weathered rock.

The weathered rock primarily consists of rock that has decomposed to silt, sandy silt, silty sand, and narrowly graded sand with silt, all of varying colors (black, white, tan, blue, green, red, orange, brown, and yellow). Some samples contain friable rock, gravel, and clay.

SPT  $N_{60}$ -values ranged from 27 blows per foot to split spoon refusal. Most  $N_{60}$ -values were over 50 blows per foot, indicating very dense soil-like material. The weathered rock transitions to a more competent gneiss with depth but is defined as rock with Rock Quality

Designations (RQD) values less than or equal to 35 percent. In B15, we collected a 5-foot-long NQ core run with a RQD value of 0 percent, indicating very poor-quality rock.

Stratum V – Bedrock (Class 1b/1c) – Metamorphic gneiss bedrock was encountered in five borings (B8, B10, B11, B15, and B23) at 75 to 85 feet below the ground surface, corresponding to between El. -53.1 and El. -70.1. The gneiss was gray, black, and white, banded, medium to fine grained, hard with slight to moderate weathering. We collected eight 5-foot-long NQ cores, with recoveries that ranged from 63 to 100 percent, and averaged about 91 percent. RQD values ranged from 38 to 100 percent and averaged 75 percent, indicating fair to good-quality rock.

#### 3.2 Groundwater Conditions

Groundwater levels were gauged in six observation wells installed in geotechnical test borings in March 2020 as identified in Fig. 2 as B5(OW), B8(OW), B16(OW), B29(OW), and B53(OW), with one of the wells, B48(OW), located about 400 feet northwest of B29(OW).

On a November 15, 2023 site visit, we could not find B16(OW), B29(OW), and B53(OW), and are assumed destroyed; B5(OW) and B8(OW) were found damaged and without surface casing; B48(OW) was found undamaged and was gauged. Historical groundwater data from these six observation wells are presented in Table 3.

Groundwater levels were also gauged on February 1 and February 2, 2022 at eight temporary monitoring wells installed on January 25 and January 26, 2022, as identified in Fig. 2 as MW-1 through MW-6, MW-11, and MW-12. Groundwater data from these eight monitoring wells are presented in Table 4.

Recommended design groundwater elevations are provided in the table below:

Building	Observed Elevation Range in Wells	Top of Floor Slab Elevation	Recommended Design Groundwater Elevation
Building 3A/3B	1.1 to 4	6.00	5
Building 4	2.9 to 4.8	8.46	6
Building 5	3.5 to 6.2	22.50	7

Groundwater levels are expected to fluctuate with season, precipitation, temperature, construction activity in the area, and other factors. Groundwater at the site also appears to be influenced by tidal fluctuations and we understand that the tidal range in the East River is about 3.6 to 4.7 feet. Groundwater level measurements represent conditions at the times and

locations when the measurements were made. Different groundwater elevations will occur at other times and locations.

## 3.3 Geotechnical Laboratory Testing

Our laboratory in Woburn Massachusetts performed the following tests on soil samples obtained from the test borings to confirm field classifications and to estimate engineering properties:

- 23 Grain size distribution tests. [ASTM D6913 and D7928]
- 2 Atterberg limits tests. [ASTM D4318]
- 25 moisture content tests. [ASTM 2216]

The results of laboratory testing are provided in Appendix C and have been incorporated into the soil descriptions on the boring logs in Appendix A.

## 3.4 Sample Storage and Disposition

Representative samples of the soils and cored rock samples were classified by the on-site GEI Field Inspector. Soil and rock samples collected as a result of our explorations are being temporarily stored at the on-site warehouse in Astoria, New York. The samples should be retained by the project until 1 year after the issuance of this report or after installed foundations have been approved by the NYC Department of Buildings, whichever is longer.

Building Code §1802.5.3 requires that:

"Soil and rock samples shall be maintained in an accessible location by the permit holder or owner and made available to the engineer responsible for the geotechnical investigation and to the department, until the foundation work has been completed and accepted, or until 1 year after the investigation is complete, whichever is longer."

# 4. Geotechnical Recommendations

The following sections of this report present the recommendations for the design of the foundation support for the proposed buildings for Phase 1 of the Astoria Cove development project. We understand that spread footings or mat foundations will be used under the majority of the building footprints, with the possible exception of deep foundations to support the tower at Building 3B.

## 4.1 Soil Properties

Recommended values of soil properties and design parameters are presented in Table 5. The properties were estimated using data from correlations with SPT  $N_{60}$ -values, laboratory testing results, published empirical correlations, and our engineering judgement.

# 4.2 Foundation Design Selection Approach

Underground program space is proposed for each building and there will be as much as 35 feet of overburden soil removed to reach floor slab subgrades. Based on the subsurface conditions encountered at the boring locations, the planned floor slab subgrades are generally within 2 feet, vertically, of Stratum III (Class 3a) soil deposits for most of the building footprint areas. The northern portions of Buildings 3A/3B and Building 5 would need up to about 11 feet and 18 feet, respectively, of overexcavation below the floor slab subgrades to reach the Stratum III deposits. For the northern portions of Building 3A/3B and Building 5, Stratum IIc soils are approximately 4 feet and 11 feet below the proposed floor slab subgrades, respectively.

The proposed buildings can be supported on either a mat foundation or spread and continuous wall footings bearing on the Stratum III soils or Stratum IIc soils or compacted Crushed Stone (refer to Recommended Material Specifications section of this report) extending to Stratum III soils or compacted Structural Fill extending to Stratum IIc soils.

Building Code Table 1804.1 provides the maximum allowable bearing pressure for material class. As stipulated in the Building Code, the allowable bearing pressure may be increased due to foundation embedment greater than 4 feet. As indicated above, construction of cellars/basements at each proposed building will require the removal of overburden soils and the modified allowable bearing pressures are presented for each building below.

#### 4.2.1 Buildings 3A/3B

The foundations for Buildings 3A/3B will require the removal of at least 11 feet and up to approximately 16 feet of overburden soil across the footprint areas of the buildings. Based on the existing topography at the site and approximate cellar floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.7 and 1 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated two allowable bearing pressures zones (Zones 1 and 2) in Fig. 9 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 6.9 and 7 tons/ft², as depicted for each zone.

We also approximated two allowable bearing pressures zones (Zones 3 and 4) in Fig. 9 for support in the Stratum IIc soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 3.9 and 3.7 tons/ft², as depicted for each approximate zone.

Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations are to derive geotechnical capacity from either the Stratum IIc soils (overexcavating up to about 5 feet) or Stratum III soils (overexcavating up to about 11 feet). In the overexcavation areas identified in Fig. 9 compacted Structural Fill extending to Stratum IIc soils or compacted Crushed Stone extending to Stratum III soils would require allowable bearing pressures of 3 tons/ft² and 6 tons/ft², respectively. No overburden credit can be applied to those foundations bearing on Structural Fill or Crushed Stone (Zone 5).

Alternatively, drilled pressure-grouted micropiles can be used to support the tower or podium cores deriving their load carrying capacity in the till soils. Refer to micropile recommendations section below for additional details.

# 4.2.2 **Building 4**

The foundations for Building 4 will require the removal of at least 12.5 feet and up to approximately 35 feet of overburden soil across the footprint area of the building. Based on the existing topography at the site and approximate cellar floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.8 and 2 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated four allowable bearing pressures zones in Fig. 10 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure between 6.8 and 8 tons/ft², as depicted for each zone.

#### **4.2.3 Building 5**

The foundations for Building 5 will require the removal of at least 2.5 feet and up to approximately 27.5 feet of overburden soil across the footprint area of the building. Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations will derive geotechnical capacity from either the Stratum IIc soils (11 feet) or Stratum III soils (22 feet).

Based on the existing topography at the site and approximate cellar/basement floor subgrades, we estimate that the maximum allowable bearing pressure may be increased between 0.3 and 1.5 ton/ft², due to the removal of overburden soils to reach foundation subgrades.

We approximated one allowable bearing pressures zone (Zone 1) in Fig. 11 for support in the Stratum III soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 7.5 tons/ft², as depicted.

We also approximated two allowable bearing pressures zones (Zones 2 and 3) in Fig. 11 for support in the Stratum IIc soils and, accordingly, we recommend designing the foundations with a maximum allowable bearing pressure of 4.3 and 3.5 tons/ft², as depicted, for each zone.

Overexcavation below the anticipated floor slab subgrades will be required if shallow foundations are to derive geotechnical capacity from either the Stratum IIc soils (overexcavating up to about 11 feet) or Stratum III soils (overexcavating up to about 22 feet). In the overexcavation areas approximated in Fig. 11, compacted Structural Fill extending to Stratum IIc soils or compacted Crushed Stone extending to Stratum III soils would offer allowable bearing pressures of 3 tons/ft² and 6 tons/ft², respectively. No overburden credit can be applied to those foundations bearing on Structural Fill or Crushed Stone (Zone 4).

#### 4.3 Settlement

From our experience with similar structures/foundation systems, and assuming subgrades are prepared and inspected as recommended in this report, we estimate total settlement of the structure will be less than 1 inch, with differential settlements less than ½ inch. Most of the expected settlements should occur during construction.

To minimize settlement after construction, we recommend the site grades be excavated to (or raised to) the finished floor elevations and then excavated to build foundations, as necessary. If grades need to be raised, the fill should be left in place 30 to 60 days prior to the construction of the building foundation.

Foundations should be designed and constructed in accordance with Building Code §1805. Shallow foundations should also satisfy the requirements contained in Building Code §1809.

We recommend that all fill placed below buildings meet the gradation and compaction requirements for Crushed Stone or Structural Fill provided in the Recommended Material Specifications Section.

## 4.4 Modulus of Subgrade Reaction for Foundation Mats

Terzaghi (1955) presents the following recommended values for modulus of vertical subgrade reaction, in tons/ft<sup>3</sup>, for a 1-foot loaded area (1-ft<sup>2</sup> or 1-foot-wide strip) bearing on sand.

Relative Density of Sand	Loose	Medium	Dense
Dry or moist sand	40	130	500
Submerged sand	25	80	300

The values for dense submerged sand are appropriate for Stratum III soils, so the recommended unit value for modulus of vertical subgrade reaction (for a 1-foot loaded area) is 300 tons/ft<sup>3</sup>.

The values for medium dense dry or moist sand are appropriate for Stratum IIc soils, so the recommended unit value for modulus of vertical subgrade reaction (for a 1-foot loaded area) is 130 tons/ft<sup>3</sup>.

The modulus of vertical subgrade reaction decreases as the size of the loaded area increases. The above unit value for a 1-foot loaded area must be adjusted based on the actual size of the loaded area. For a loaded area of width B (in feet), the unit value should be multiplied by a reduction factor of  $[(B+1)/2B]^2$  per Terzaghi (1955). For a loaded area 10 feet wide, the reduction value is 0.30. For a loaded area of infinite width, the reduction factor is 0.25.

For a mat foundation, the effective loaded area is a function of the area of the loading applied to the mat and the mat stiffness (which distributes the load to the soil over a larger area). Foundation mats are typically designed as a relatively stiff reinforced concrete slab. For a

conservative value, we recommend using a reduction factor of 0.25. This gives a conservative area-adjusted modulus of vertical subgrade reaction of:

 $0.25 \times 300 \text{ tons/ft}^3 = 75 \text{ tons/ft}^3 = 87 \text{ lb/in}^3$ .

## 4.5 Footings and Floor Slab

We understand that portions of the cellar/basement floor may be designed as a framed slab or slab-on-grade over spread and continuous footings.

Footings on soil should be at least 3 feet wide. Foundations should bear at least 48 inches below the adjacent grade for frost protection, in accordance with Building Code §1809.3.1. Interior footings bearing on the Stratum III deposit or Structural Fill should bear at least 18 inches below the bottom of the floor slab.

We recommend that contraction joints be incorporated between the slab-on-grade and the columns and perimeter walls of the proposed buildings.

The floor slab should not rest directly on boulders. Protruding boulders should be removed and backfilled as needed to allow a minimum 9-inch-thick compacted Structural Fill or Crushed Stone "cushion" to be placed below the slab. Eliminating protruding boulders will reduce cracking of the slab that may result from stress concentrations.

We recommend that, at a minimum, a vapor retarder be installed below the slab-on-grade and to consult with the manufacturer of the proposed flooring system regarding vapor transmission and vapor retarder/waterproofing requirements.

# 4.6 Micropile Foundation Recommendations

Driven piles may prove difficult to install in the dense Stratum III (Class 3a) soil deposits and it may be difficult to achieve 200-ton compression and 100-ton tension capacity with driven piles. Therefore, we recommend that the northern portions of the Building 3B area, inclusive of the tower footprint, be supported by pressure-grouted micropiles bearing in Stratum III soils.

During installation, these micropiles are drilled with steel-casing to the design tip-elevation. The soil socket is achieved by filling the entire casing with grout, by a tremie pipe, and extracting the amount of casing needed for the length of the soil socket as measured from the bottom of the pile. As casing is extracted, the grout is pressurized to about 50 lb/in², or as determined by the pile designer. Micropiles should be designed by a professional engineer licensed in the State of New York with experience on similar projects.

We recommend a preliminary micropile design consisting of:

- 50 kips/in<sup>2</sup> (ksi) steel casing with the dimensions of 13.375 inches outer diameter and ½ inch in thickness.
- Two 75 ksi #18 steel-reinforcing-bars for the whole length of the pile.
- 5,000 lb/in<sup>2</sup> compressive strength grout.
- Minimum soil socket of 35 feet in length installed entirely in Stratum III.
- 3 feet minimum center-to-center spacing of piles.

Drilled pressure-grouted micropiles can be used to achieve the 200-ton compression and 100-ton tension capacity and could also be used as tiedown anchors for the towner core for tension capacity if shallow foundations are used for the entire building footprint.

A minimum of two compression load tests will be required for the micropiles. To substantiate loads larger than the allowable loads of deep foundations listed in the Building Code, the piles will either need to be instrumented using strain gauges or be subject to cyclical loading per ASTM D1143. Additionally, the final load increment will need to be held for a minimum period of 24 hours. A minimum of two uplift load tests are recommended. The number of uplift load tests will be determined by the professional engineer of record. Lateral load testing is required per the Building Code if the design lateral load exceeds 1 ton.

Micropiles should be installed under full-time Special Inspection in accordance with the Building Code. Use of air hammers should be prohibited for drilling through the overburden soils. Internal flushing of drilling fluids should be required and a positive head shall be always maintained within the casing. Observation of the soil cutting quantity in the wash water should be closely monitored during installation.

# 4.7 Seismic Design

We initially recommend using Site Class D (Stiff Soil Profile) for seismic design purposes, in accordance with Building Code  $\S1613.5.2$ , based on the N<sub>60</sub>-values and the subsurface profile encountered at the site. The site is borderline Site Class C (Very Dense Soil and Soft Rock Profile), which can be further refined and proven out by performing geophysical shear wave velocity studies of the subsurface to evaluate whether Site Class C can be assigned to this site. Using the seismic design values associated with Site Class C, the project design tea

m may be able to find cost savings relative to seismic reinforcement requirements compared with Site Class D recommendations.

Corresponding spectral response design parameters are as follows:

Building Code			
Seismic Design Values			
Site Class	С	D	
Risk Category	II	=	
S <sub>S</sub>	0.279 g	0.279 g	
S <sub>1</sub>	0.072 g	0.072 g	
S <sub>DS</sub>	0.223 g	0.293 g	
S <sub>D1</sub>	0.082 g	0.115 g	
PGA <sub>M</sub>	0.20 g	0.24 g	
Seismic Design Category	В	В	

We calculated the spectral response parameters for the Site using general procedures outlined in Building Code §1613.5. Peak ground acceleration (PGA<sub>M</sub>) is adjusted for Site Class effects, per ASCE 7-10 §11.4.7, and were selected from Building Code Table 1813.2.1.

Based on a Site Class D and Risk Category II for the proposed building, and in accordance with Building Code Tables 1613.5.6(1) and (2), Seismic Design Category B should be assigned to the structure for design.

Based on the site geology, topography and subsurface investigations reported herein, it is our opinion that the proposed building is not susceptible to damage from liquefaction, slope instability, lateral spreading, or surface rupture due to faulting.

# 4.8 Waterproofing and Dampproofing

Groundwater levels observed in wells were in close proximity to the proposed cellar floor slab elevation in Building 3A/3B. In addition, groundwater levels could temporarily rise during flood conditions. Therefore, we recommend that all floors and walls of the cellar should be waterproofed in accordance with Building Code §1807.

A vapor retarder consisting of polyethylene membrane should also be installed below all moisture sensitive slabs (i.e. occupied interior spaces). The installed vapor retarder should be sealed at the foundation walls, columns, and utility penetrations, and panels should be overlapped and joints sealed. We also recommend that waterstops be provided at all joints in the concrete basement walls and floor slab, and that the cellar/basement walls be waterproofed/dampproofed.

We recommend that isolated structures that extend below the slab and near or below the design groundwater elevation in any of the four buildings, such as elevator pits and sump pits, be waterproofed in accordance with Building Code §1807 and designed for hydrostatic uplift pressures.

#### 4.9 Permanent Below Grade Walls

We recommend that cellar/basement walls be designed for the lateral pressures shown in Fig. 12.

Below grade walls used for this project should be designed for at-rest earth pressures of 60 lb/ft² per foot of depth above the groundwater level. Based on the groundwater conditions observed at the site, we do not expect hydrostatic pressures will act on cellar/basement wall. Although hydrostatic pressures are not expected, the cellar/basement walls should be waterproofed. Rainwater and water falling on impermeable surfaces should be tied to subgrade stormwater sewers and directed away from cellar/basement walls.

An earthquake pressure of 10.8 lb/ft<sup>2</sup> per foot of depth should be applied to the walls and be distributed as an inverse triangle over the height of the wall.

In addition to the lateral loads exerted by the soil against the walls, allowance should be included for lateral stresses imposed by any temporary or long-term surcharge loads, such as cars or trucks adjacent to the walls. The wall design shall also include any surcharge loads that may develop within a 45° slope from the toe of the wall using 50 percent of the adjacent surcharge load over the height of the wall. Design of cellar walls should not incorporate the lateral support provided by temporary shoring works that are used to facilitate excavation to the cellar level.

# 4.10 Temporary Support of Excavation (SOE)

Permanent cellar/basement walls and temporary Support of Excavation (SOE) walls will be required to construct the proposed below-grade space at all four building sites. Temporary SOE might be required to perform the undercut to Statum III soils within the footprint of the buildings.

Temporary SOE is required for vertical excavations deeper than 4 feet to support the cut face. Temporary walls should be designed to withstand lateral earth pressures and surcharge loads from construction staging, equipment, stockpiles, and adjacent traffic loads.

Based on the existing grades and the proposed lowest level of the structure, cut heights up to 35 feet are expected. An SOE system consisting of cantilevered soldier piles and lagging is

therefore feasible. Alternatively, a braced SOE system consisting of soldier piles and rakers with heel blocks can be used..

## 4.11 Site Retaining Wall Designs

All permanent earth retaining structures used on the project across the site should be designed using the following earth pressure parameters:

Parameter	Value
Total Unit Weight of Granular Backfill (γ)	125 lb/ft <sup>3</sup>
Angle of Internal Friction (φ)	32°
Cohesion (c)	0 lb/ft²
At-Rest Earth Pressure Coefficient, (K <sub>o</sub> )	0.47 <sup>(1)</sup>
Active Earth Pressure Coefficient, (K <sub>a</sub> )	0.31(1)
Passive Earth Pressure Coefficient, (K <sub>p</sub> )	3.0(1)
Allowable coefficient of friction between rough concrete footing and granular bearing soil	0.50(1)

(1) = Ultimate values, with no Factor of Safety applied

Retaining walls free to rotate at the top should be designed for active earth pressures. In addition to the lateral loads exerted by the soil against the walls, allowance should be included for lateral stresses imposed by any temporary or long-term surcharge loads, such as cars or trucks adjacent to the walls or adjacent footing loads.

The recommended wall design parameters do not consider the development of hydrostatic pressure behind the walls. As such, positive wall drainage must be provided for all earth retaining structures. These drainage systems can be constructed of open-graded washed stone isolated from the soil backfill with a geosynthetic filter fabric and drained by perforated pipe, or several wall drainage products made specifically for this application. Where backfill soils are not drained using an appropriately designed drainage system, the lateral soil pressure on proposed retaining walls must consider hydrostatic forces and submerged soil unit weight.

The earth pressure parameters given in the above table assume placement and compaction of the backfill in accordance with recommendations elsewhere in this report. Compact backfill directly behind walls with light, hand-operated compactors. Heavy compactors and grading equipment should not be allowed to operate within 10 feet of the walls during backfilling to avoid developing excessive temporary or long-term lateral soil pressures.

The soil parameters used in the design of the walls should be based on tests performed on the actual soil materials that will be used to construct the walls.

# 5. Construction Considerations

## 5.1 Excavation, Shoring, and Dewatering

Excavations should be made in accordance with Occupational Safety and Health Administration (OSHA) standards and the Building Code pertaining to the protection of property and worker safety. Any excavation greater than 4 feet in depth should be inclined for safety unless shoring is used. Excavations adjacent to the existing structure foundations should not extend below the bottom of the existing foundations without prior underpinning of the existing structure foundations, if required.

Because of the depth of excavation, we recommend that temporary excavation support systems be used for the cellar/basement excavations and foundation construction at each building site. A Professional Engineer licensed in the State of New York and experienced in the design of excavation support systems should design the excavation support.

The excavation support system should be designed to minimize settlement and lateral movement of surrounding structures including buildings and utilities. Vibrations to nearby structures should be limited during installation of the excavation support elements. We recommend monitoring adjacent structures for vibration during installation of excavation support.

Design groundwater elevations for Building 3A/3B, Building 4, and Building 5 are El. 5, El. 6, and El. 7, respectively. We understand that, in general, proposed cellar/basement levels and mechanical pits will be above groundwater levels. In addition, overexcavation to reach target bearing stratum will likely extend below the groundwater table. The contractor will need to design and install groundwater dewatering systems appropriate to lower groundwater levels to maintain stable subgrades.

Localized dewatering of subgrades can be achieved through a temporary dewatering wellpoint system. Groundwater levels shall be a minimum of 2 feet below final subgrade elevations prior to excavation for final subgrade.

The designer of the dewatering systems should consider the effects of drawdown outside the limits of excavation. The temporary dewatering system shall be designed by a Professional Engineer from the State of New York and in compliance with NYCDEP water discharge guidelines.

Site runoff may infiltrate the excavations. Removal of runoff may be accomplished with filtered sumps and pumps. The sumps should consist of a perforated pipe wrapped in Geotextile and surrounded by Pea Stone. The site should be graded to route stormwater runoff away from excavations.

## 5.2 Subgrade Preparation

Foundations may bear directly on the Stratum IIc or Stratum III soils or compacted Structural Fill extending to Stratum IIc or Stratum III soils. Lean concrete, Controlled Low Strength Material (CLSM), or tamped Crushed Stone wrapped in Geotextile may be substituted for Structural Fill. Any loose or disturbed soil should be removed from the bottom of the excavation, and the subgrade should be proof compacted with a vibratory compactor.

We recommend the following for preparing subgrades below foundations:

- Remove any loose or disturbed soil from the bottom of the excavation and compact the subgrade with a vibrator compactor weighing at least 200 pounds and imparting an impact load of at least 2.5 tons. Any observed soft or 'pumping' zones in the subgrades should be overexcavated and replaced with either compacted Structural Fill or with Crushed Stone wrapped in Geotextile.
- 2. If encountered, uncontrolled fill is not suitable for structural support and should be excavated from below all foundations and replaced with compacted Structural Fill.
- 3. Concrete for foundations may be placed directly on the soil subgrade. Bearing surfaces should be free of standing water, frost, loose soil, and soil softened by water intrusion before placement of reinforcing steel and concrete. Areas of the subgrade disturbed by traffic or surface water should be re-compacted.
- 4. A 6-inch layer of compacted crushed stone over geotextile fabric or a 3- to 4-inch-thick lean concrete mud slab may be required to stabilize foundation subgrades, protect subgrades against weather, and provide a working platform.

We recommend that a GEI engineer observe the final preparation of subgrades prior to foundation construction. The subgrade compaction should be observed continuously by a qualified Special Inspector, a service that GEI can provide.

# 5.3 Freezing Conditions

The soils at the site are frost susceptible. Therefore, if construction is performed during freezing weather, special precautions will be required to prevent the subgrade soils from freezing.

All subgrades should be free of frost before placement of concrete. Frost-susceptible soils that have frozen should be removed and replaced with compacted Structural Fill.

Soil placed as fill should be free of frost, as should the ground on which it is placed.

If mud slabs or the foundation mat are built and left exposed during the winter, precautions should be taken to prevent freezing of the underlying soil. Freezing of the soil beneath the foundation during construction may result in subsequent settlement of the structure.

## 5.4 Backfilling and Compaction

Any fill placed within the building limits should meet the gradation and compaction requirements for Structural Fill. Lean concrete, CLSM, or Crushed Stone wrapped in Geotextile may be substituted for Structural Fill. Fill outside the structure limits should meet the gradation and compaction requirements for Ordinary Fill. Ordinary Fill may be susceptible to frost heave. The potential for frost heave can be reduced by grading outside areas for proper drainage and by using Structural Fill rather than Ordinary Fill for the top 1 to 2 feet.

The lateral pressures given in Fig. 12 assume placement and compaction of the backfill in accordance with recommendations in this report. Compact backfill directly behind walls with light, hand-operated compactors. Heavy compactors and grading equipment should not be allowed to operate within 10 feet of the walls during backfilling to avoid developing excessive temporary or long-term lateral soil pressures.

# 5.5 Adjacent Structures

In accordance with Building Code §107.7.1, we recommend indicating foundation elevations of adjacent structures on any structural foundation plans. We understand that underpinning of adjacent structures will not be required on this project.

We recommend that the ownership team retain an instrumentation consultant, independent of the construction contractor, to perform optical survey monitoring of the support of excavation systems and any existing adjacent structures that might be affected by the excavations. GEI can provide these services, if requested.

#### 5.6 Site Disturbance

Project specifications should require the contractor to maintain stable site conditions at all times. Specific measures to stabilize equipment traffic areas could include compacted aggregate placed over a Geotextile, temporary drainage, or other appropriate measures.

#### 5.7 Wet Site Conditions

The contractor should be prepared to handle excessively over-optimum (wet) soil moisture conditions during earthwork activities in the winter or spring as well as during or following periods of rain. Site soils could easily become disturbed, and the ground surface could become excessively muddy and unstable due to traffic by construction equipment. Colder months of the year generally provide fewer opportunities to dry soils that have become wet. Wet soil subgrades or fill can make proper compaction difficult or impossible. Possible mitigating measures for wet soil conditions are as follows:

- 1. Frequent spreading and mixing during warm dry weather.
- 2. Mixing with drier materials.
- 3. Mixing with a lime, lime-fly ash, or cement product.
- 4. Stabilizing with aggregate, geotextile stabilization fabric, or both.

Options 3 and 4 should be evaluated and approved by the Geotechnical Engineer prior to implementation.

## 6. Limitations

This report was prepared for the exclusive use of Slim Astoria 2468 LLC and the design team. Our recommendations are based on the project information provided to us at the time of this report and may require modification if there are any changes in the nature, design, or location of the proposed structure. We cannot accept responsibility for designs based on our recommendations unless we are engaged to review the final plans and specifications to assess whether any changes in the project affect the validity of our recommendations and whether our recommendations have been properly implemented in the design.

The recommendations in this report are based in part on the data obtained from the subsurface explorations. The nature and extent of variations between explorations may not become evident until construction. If variations from the anticipated conditions are encountered, it may be necessary to revise the recommendations in this report. We, therefore, recommend that GEI be engaged to make site visits during construction to: a) check that the subsurface conditions exposed during construction are in general conformance with our design assumptions, and b) ascertain that, in general, the work is being performed in compliance with the contract documents.

Our professional services for this project have been performed in accordance with generally accepted engineering practices; no warranty, express or implied, is made. This report should be, per Building Code §1802.6, submitted to the building official by any permit applicant at the time of permit application.

# **Recommended Material Specifications**

Structural Fill and Ordinary Fill shall consist of hard, durable sand and gravel, free of clay, organic matter, surface coatings, and other deleterious materials. Soil finer than the No. 200 sieve (the "fines") should be nonplastic. On-site material can be re-used as Structural Fill or Ordinary Fill, provided they can meet the appropriate gradation and compaction requirements indicated below and do not contain deleterious materials. Soils to be used as fill imported from off site should also meet the gradation requirements given below.

#### **Structural Fill**

Structural Fill shall meet the following gradation requirements:

Sieve Size	Percent Passing by Weight
3 inches	100
0.5 inch	50 to 100
No. 4	35 to 85
No. 16	20 to 65
No. 50	5 to 40
No. 200 (fines)	0 to 8

Structural Fill shall be compacted in maximum 9-inch-thick, loose lifts to at least 95 percent of the maximum dry density determined in accordance with ASTM D1557 (Modified AASHTO Compaction). The moisture content should be held to within ±3 percent of optimum moisture content (as determined by ASTM D1557).

#### Ordinary Fill

Ordinary Fill shall meet the following gradation requirements:

Sieve Size	Percent Passing by Weight
6 inches	100
3 inches	80 to 100
No. 4	20 to 100
No. 200 (fines)	0 to 20

Ordinary Fill shall be compacted in maximum 12-inch-thick, loose lifts to at least 92 percent of the maximum dry density determined in accordance with ASTM D1557 (Modified AASHTO Compaction). The moisture content should be held to within  $\pm 3$  percent of optimum moisture content (as determined by ASTM D1557).

#### **Crushed Stone**

Crushed Stone shall consist of clean, durable, sharp-angled fragments of gravel free from surface coatings and shall conform to the requirements of material designation 703-0201 No. 1 Crushed Stone in the May 1, 2020 Standard Specifications prepared by New York State Department of Transportation (NYSDOT). Materials that break up when alternatively, frozen and thawed or wetted and dried shall not be used. Crushed Stone shall meet the following gradation requirements or approved equal:

Sieve Size	Percent Passing by Weight
1 inch	100
0.5 inch	90 to 100
0.25 inch	0 to 15
No. 200 (fines)	0 to 1.0

Crushed Stone should be compacted with at least four passes of a static vibratory compactor.

#### Pea Stone

Pea Stone shall be screened gravel conforming to the requirements of ASTM D 448 for stone size No. 8 and shall consist of durable, inert rounded stone, free from loam or clay, surface coatings, and deleterious materials. Pea Stone shall meet the following gradation requirements:

Sieve Size	Percent Passing by Weight
0.5 inch	100
0.375 inch	85 to 100
No. 4	10 to 30
No. 8	0 to 10
No. 16	0 to 5

#### **Controlled Low Strength Material**

Controlled Low Strength Material (CLSM) shall conform to material designation 733-01 Type 1/2 in the May 1, 2020 Standard Specifications prepared by the NYSDOT.

#### Geotextile

Geotextile should be a non-woven fabric, consisting of Mirafi 140N or an approved equal product.

Geotechnical Report
Astoria Cove Development
Buildings 3A/3B, 4 & 5
Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 |
Block 909 Lot 35
Astoria, Queens County, New York
December 2023

# **Tables**

Table 1. Subsurface Exploration Data Geotechnical Report Astoria Cove Development Astoria, Queens, New York

							Approximate Top of Layer <sup>(2,3,4)</sup> (feet)													
Exploration		Northing <sup>(1)</sup>	Easting <sup>(1)</sup>	Ground Surface Elevation <sup>(2)</sup>	Total Depth of Exploration	Bottom of Exploration Elevation	ı	atum I Fill ass 7)	Loose	um IIa e Sand ss 6)	Silt	tum IIb :/Clay s 4b/5b)	Sa	um IIc and ss 3b)	Т	um III 'ill ss 3a)	athere	osed/We	Bed	itum V drock s 1b/1c)
ID	Building ID	(feet)	(feet)	(feet)	(feet)	(feet)	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.
B1	Bldg. 5	221,653	1,003,263	48.8	102.0	-53.2	0.0	48.8	2.0	46.8			8.0	40.8	28.5	20.3				
B2	Bldg. 5	221,695	1,003,280	48.4	102.0	-53.6	0.0	48.4					7.0	41.4	11.0	37.4	97.7	-49.3		
В3	Bldg. 5	221,718	1,003,305	46.0	102.0	-56.0	0.0	46.0					10.0	36.0	28.5	17.5				
B4	Bldg. 5	221,775	1,003,310	24.7	102.0	-77.3	0.0	24.7					2.0	22.7	10.0	14.7	93.0	-68.3		
B5(OW)	Bldg. 5	221,770	1,003,343	24.8	102.0	-77.2	0.0	24.8			4.0	20.8	13.5	11.3	24.0	8.0	85.6	-60.8		
B6	Bldg. 4	221,715	1,003,183	46.2	102.0	-55.8	0.0	46.2	10.0	36.2			16.0	30.2	28.5	17.7				
B7	Bldg. 4	221,766	1,003,203	40.1	102.0	-61.9	0.0	40.1					18.5	21.6	33.5	6.6				
B8(OW)	Bldg. 4	221,825	1,003,207	23.6	85.0	-61.4	0.0	23.6					8.0	15.6	17.0	6.6	76.1	-52.5	80.0	-56.4
B9	Bldg. 4	221,855	1,003,277	22.3	102.0	-79.7	0.0	22.3	0.7	21.7	6.0	16.3	8.0	14.3	10.5	11.8	88.5	-66.2		
B10	Bldg. 4	221,877	1,003,222	21.9	85.0	-63.1	0.0	21.9	2.0	19.9					13.5	8.4	62.7	-40.8	75.0	-53.1
B11	Bldg. 4	221,898	1,003,171	21.7	90.0	-68.3	0.0	21.7	4.0	17.7			10.0	11.7	13.5	8.2	71.5	-49.8	80.0	-58.3
B12	Bldg. 3A/3B	222,043	1,003,593	14.8	77.0	-62.2	0.0	14.8					13.0	1.8	23.5	-8.7	67.7	-52.9		
B13	Bldg. 3A/3B	221,884	1,003,427	23.5	102.0	-78.5	0.0	23.5	6.0	17.5			10.0	13.5	18.5	5.0	82.7	-59.2		
B14	Bldg. 3A/3B	221,930	1,003,368	21.8	77.0	-55.2	0.0	21.8	4.0	17.8			10.0	11.8	13.5	8.3				
B15	Bldg. 3A/3B	221,889	1,003,534	21.9	93.0	-71.1	0.0	21.9	2.0	19.9			6.0	15.9	13.5	8.4	52.6	-30.7	83.0	-61.1
B16(OW)	Bldg. 3A/3B	221,903	1,003,499	21.9	102.0	-80.1	0.0	21.9					4.0	17.9	10.0	11.9	73.5	-51.6		
B17	Bldg. 3A/3B	221,948	1,003,379	21.5	75.3	-53.8	0.0	21.5					1.0	20.5	13.5	8.0				
B18	Bldg. 3A/3B	221,937	1,003,535	20.6	100.2	-79.6	0.0	20.6					2.0	18.6	23.5	-2.9	68.5	-47.9		
B19	Bldg. 3A/3B	221,997	1,003,398	21.4	75.4	-54.0	0.0	21.4					8.0	13.4	13.5	7.9				
B20	Bldg. 3A/3B	222,066	1,003,426	20.2	75.1	-54.9	0.0	20.2							8.0	12.2				
B21	Bldg. 3A/3B	222,090	1,003,573	14.0	72.0	-58.0	0.0	14.0	18.5	-4.5	28.5	-14.5			33.5	-19.5				
B22	Bldg. 3A/3B	222,064	1,003,531	17.5	75.1	-57.6	0.0	17.5					10.0	7.5	18.5	-1.0				
B23	Bldg. 3A/3B	222,133	1,003,480	15.0	90.1	-75.1	0.0	15.0							23.5	-8.5	73.5	-58.5	85.1	-70.1
B29(OW)	N/A	222,066	1,003,320	21.9	85.0	-63.1	0.0	21.9					8.0	13.9	23.5	-1.6	67.0	-45.1	80.0	-58.1
B53(OW)	N/A	221,759	1,003,421	29.4	55.0	-25.6	1.0	28.4					4.0	25.4	13.5	15.9	38.5	-9.1	45.0	-15.6
B101	Bldg. 3A/3B	222,057	1,003,490	20.1	102.0	-81.9	0.0	20.1	8.0	12.1			19.0	1.1	23.5	-3.4	87.0	-66.9		
B102	Bldg. 3A/3B	221,943	1,003,439	21.4	86.8	-65.4	0.0	21.4					13.5	7.9	18.5	2.9		-		

### Footnotes:

- 1. Horizontal Coordinates are U.S. State Plane Coordinates in U.S. Survey feet and are referenced to the North American Datum of 1983 (NAD83).
- 2. Elevations are referenced to the North American Vertical Datum of 1988 (NAVD88) and estimated from topographic plan prepared by Montrose Surveying.
- 3. "--" indicates that layer was not encountered.

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

Boring   Building   Package   ID   Revalue, N <sub>m</sub>   (blows/foot)   N-value, N <sub>m</sub>   (blows/foot)   N-value, N <sub>m</sub>   (blows/foot)		1	ı	Eiold	
Boring   Building   Package   ID   (blows/foot)				Field	Carrieral and
Bidg. 5					-
Bidg. 5 S1 33 44  Bidg. 5 S2 12 16  Bidg. 5 S3 7 9  Bidg. 5 S4 7 9  Bidg. 5 S6 11 15  Bidg. 5 S6 11 15  Bidg. 5 S6 11 15  Bidg. 5 S8 13 17  Bidg. 5 S8 13 17  Bidg. 5 S8 13 17  Bidg. 5 S9 25 33  Bidg. 5 S9 25 33  Bidg. 5 S10 33 44  Bidg. 5 S10 33 44  Bidg. 5 S11 48 64  Bidg. 5 S12 160/9" 60  Bidg. 5 S13 25 33  Bidg. 5 S14 106/3" 100  Bidg. 5 S15 117 100  Bidg. 5 S16 100/5" 100  Bidg. 5 S17 101/5" 100  Bidg. 5 S18 100/5" 100  Bidg. 5 S18 100/5" 100  Bidg. 5 S19 102/2" 100  Bidg. 5 S20 101/4" 100  Bidg. 5 S21 178/10" 100  Bidg. 5 S21 178/10" 100  Bidg. 5 S21 111 100  Bidg. 5 S22 102/5" 100  Bidg. 5 S23 104/2" 100  Bidg. 5 S24 111 15  Bidg. 5 S2 11 178/10" 100  Bidg. 5 S3 7 9  Bidg. 5 S4 11 15  Bidg. 5 S6 45 60  Bidg. 5 S6 45 60  Bidg. 5 S7 13 17  Bidg. 5 S8 31 141  Bidg. 5 S8 31 141  Bidg. 5 S9 41 55  Bidg. 5 S1 15 100  Bidg. 5 S1 10 115 100  Bidg. 5 S1 10 100  Bidg. 5 S1 100  Bidg.	_	_	· -		
Bidg. 5   S2   12   16	<u>ID</u>	<u> </u>		,	,
Bidg. 5 S3 7 9 Bidg. 5 S4 7 9 Bidg. 5 S5 13 17 Bidg. 5 S6 11 15 Bidg. 5 S7 12 16 Bidg. 5 S8 13 17 Bidg. 5 S8 13 17 Bidg. 5 S8 13 17 Bidg. 5 S9 25 33 Bidg. 5 S10 33 44 Bidg. 5 S11 48 64 Bidg. 5 S12 16009" 60 Bidg. 5 S13 25 33 Bidg. 5 S13 25 33 Bidg. 5 S14 106/3" 100 Bidg. 5 S15 117 100 Bidg. 5 S16 100/5" 100 Bidg. 5 S17 101/5" 100 Bidg. 5 S18 100/5" 100 Bidg. 5 S19 102/2" 100 Bidg. 5 S20 101/4" 100 Bidg. 5 S21 178/10" 100 Bidg. 5 S22 102/5" 100 Bidg. 5 S24 111 100 Bidg. 5 S24 111 100 Bidg. 5 S2 9 12 Bidg. 5 S3 17 15 Bidg. 5 S2 9 12 Bidg. 5 S3 7 13 17 Bidg. 5 S6 45 60 Bidg. 5 S6 45 60 Bidg. 5 S7 13 17 Bidg. 5 S6 45 60 Bidg. 5 S9 41 55 Bidg. 5 S9 41 50 Bidg. 5 S9 510 50 Bidg. 5 S9 41 50 Bidg. 5 S9 510 50 Bidg. 5					
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Bldg. 5         S21         178/10"         100           Bldg. 5         S22         102/5"         100           Bldg. 5         S23         104/2"         100           Bldg. 5         S24         111         100           Bldg. 5         S1         19         25           Bldg. 5         S2         9         12           Bldg. 5         S3         7         9           Bldg. 5         S4         11         15           Bldg. 5         S5         20         27           Bldg. 5         S6         45         60           Bldg. 5         S6         45         60           Bldg. 5         S8         31         41           Bldg. 5         S8         31         41           Bldg. 5         S9         41         55           Bldg. 5         S10         115         100           Bldg. 5         S11         150         100           Bldg. 5         S12         76         100           Bldg. 5         S13         86         100           Bldg. 5         S14         103/5"         100           Bldg. 5					
Bldg. 5 S22 102/5" 100 Bldg. 5 S23 104/2" 100 Bldg. 5 S24 111 100  Bldg. 5 S2 9 112 Bldg. 5 S2 9 12 Bldg. 5 S3 7 9 Bldg. 5 S4 111 155 Bldg. 5 S5 S2 9 12 Bldg. 5 S5 S4 11 155 Bldg. 5 S5 S6 S6 S7 S7 S7 S8					
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Bldg. 5 S1 19 25 Bldg. 5 S2 9 12 Bldg. 5 S3 7 9 Bldg. 5 S4 11 15 Bldg. 5 S5 20 27 Bldg. 5 S5 20 27 Bldg. 5 S6 45 60 Bldg. 5 S7 13 17 Bldg. 5 S8 31 41 Bldg. 5 S8 31 41 Bldg. 5 S9 41 55 Bldg. 5 S9 41 55 Bldg. 5 S10 115 100 Bldg. 5 S11 150 100 Bldg. 5 S11 150 100 Bldg. 5 S12 76 100 Bldg. 5 S13 86 100 Bldg. 5 S13 86 100 Bldg. 5 S14 103/5" 100 Bldg. 5 S15 103/5" 100 Bldg. 5 S16 104/5" 100 Bldg. 5 S17 62/0" 100 Bldg. 5 S18 179/10" 100 Bldg. 5 S19 101/2" 100 Bldg. 5 S20 104/5" 100 Bldg. 5 S21 101/3" 100 Bldg. 5 S22 189/11" 100					
Bldg. 5 S2 9 12  Bldg. 5 S3 7 9  Bldg. 5 S4 11 15  Bldg. 5 S5 20 27  Bldg. 5 S6 45 60  Bldg. 5 S7 13 17  Bldg. 5 S8 31 41  Bldg. 5 S8 31 41  Bldg. 5 S9 41 55  Bldg. 5 S9 41 55  Bldg. 5 S10 115 100  Bldg. 5 S11 150 100  Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100					
Bidg. 5 S3 7 9 Bidg. 5 S4 11 15 Bidg. 5 S5 20 27 Bidg. 5 S6 45 60 Bidg. 5 S7 13 17 Bidg. 5 S8 31 41 Bidg. 5 S8 31 41 Bidg. 5 S9 41 55 Bidg. 5 S10 115 100 Bidg. 5 S11 150 100 Bidg. 5 S12 76 100 Bidg. 5 S12 76 100 Bidg. 5 S13 86 100 Bidg. 5 S14 103/5" 100 Bidg. 5 S15 103/5" 100 Bidg. 5 S16 104/5" 100 Bidg. 5 S18 179/10" 100 Bidg. 5 S18 179/10" 100 Bidg. 5 S19 101/2" 100 Bidg. 5 S20 104/5" 100 Bidg. 5 S21 101/3" 100 Bidg. 5 S22 189/11" 100 Bidg. 5 S23 102/5" 100					
Bldg. 5 S4 11 15 Bldg. 5 S5 20 27 Bldg. 5 S6 45 60 Bldg. 5 S7 13 17 Bldg. 5 S8 31 41 Bldg. 5 S9 41 55 Bldg. 5 S10 115 100 Bldg. 5 S11 150 100 Bldg. 5 S12 76 100 Bldg. 5 S13 86 100 Bldg. 5 S13 86 100 Bldg. 5 S14 103/5" 100 Bldg. 5 S15 103/5" 100 Bldg. 5 S16 104/5" 100 Bldg. 5 S17 62/0" 100 Bldg. 5 S18 179/10" 100 Bldg. 5 S19 101/2" 100 Bldg. 5 S19 101/2" 100 Bldg. 5 S20 104/5" 100 Bldg. 5 S21 101/3" 100 Bldg. 5 S22 189/11" 100 Bldg. 5 S22 189/11" 100 Bldg. 5 S23 102/5" 100					
Bidg. 5 S5 20 27  Bidg. 5 S6 45 60  Bidg. 5 S7 13 17  Bidg. 5 S8 31 41  Bidg. 5 S9 41 55  Bidg. 5 S10 115 100  Bidg. 5 S11 150 100  Bidg. 5 S12 76 100  Bidg. 5 S13 86 100  Bidg. 5 S14 103/5" 100  Bidg. 5 S15 103/5" 100  Bidg. 5 S16 104/5" 100  Bidg. 5 S17 62/0" 100  Bidg. 5 S18 179/10" 100  Bidg. 5 S19 101/2" 100  Bidg. 5 S20 104/5" 100  Bidg. 5 S21 101/3" 100  Bidg. 5 S22 189/11" 100  Bidg. 5 S22 189/11" 100  Bidg. 5 S23 102/5" 100					
Bidg. 5 S6 45 60  Bldg. 5 S7 13 17  Bldg. 5 S8 31 41  Bldg. 5 S9 41 55  Bldg. 5 S10 115 100  Bldg. 5 S11 150 100  Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100					
Bldg. 5 S7 13 17  Bldg. 5 S8 31 41  Bldg. 5 S9 41 55  Bldg. 5 S10 115 100  Bldg. 5 S11 150 100  Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100					27
Bidg. 5 S8 31 41 55 Bidg. 5 S9 41 55 Bidg. 5 S10 115 100 Bidg. 5 S11 150 100 Bidg. 5 S12 76 100 Bidg. 5 S13 86 100 Bidg. 5 S14 103/5" 100 Bidg. 5 S15 103/5" 100 Bidg. 5 S16 104/5" 100 Bidg. 5 S17 62/0" 100 Bidg. 5 S18 179/10" 100 Bidg. 5 S19 101/2" 100 Bidg. 5 S20 104/5" 100 Bidg. 5 S21 101/3" 100 Bidg. 5 S21 101/3" 100 Bidg. 5 S22 189/11" 100 Bidg. 5 S23 102/5" 100					
Bldg. 5 S9 41 55 Bldg. 5 S10 115 100 Bldg. 5 S11 150 100 Bldg. 5 S12 76 100 Bldg. 5 S13 86 100 Bldg. 5 S14 103/5" 100 Bldg. 5 S15 103/5" 100 Bldg. 5 S16 104/5" 100 Bldg. 5 S17 62/0" 100 Bldg. 5 S18 179/10" 100 Bldg. 5 S19 101/2" 100 Bldg. 5 S20 104/5" 100 Bldg. 5 S21 101/3" 100 Bldg. 5 S22 189/11" 100 Bldg. 5 S23 102/5" 100					
Bldg. 5 S10 115 100  Bldg. 5 S11 150 100  Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S17 62/0" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100			S8		41
Bldg. 5 S11 150 100  Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S17 62/0" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100					
Bldg. 5 S12 76 100  Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S17 62/0" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100					
Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S17 62/0" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100		Bldg. 5			
Bldg. 5 S13 86 100  Bldg. 5 S14 103/5" 100  Bldg. 5 S15 103/5" 100  Bldg. 5 S16 104/5" 100  Bldg. 5 S17 62/0" 100  Bldg. 5 S18 179/10" 100  Bldg. 5 S19 101/2" 100  Bldg. 5 S20 104/5" 100  Bldg. 5 S21 101/3" 100  Bldg. 5 S22 189/11" 100  Bldg. 5 S23 102/5" 100	R2	Bldg. 5			100
Bldg. 5       S15       103/5"       100         Bldg. 5       S16       104/5"       100         Bldg. 5       S17       62/0"       100         Bldg. 5       S18       179/10"       100         Bldg. 5       S19       101/2"       100         Bldg. 5       S20       104/5"       100         Bldg. 5       S21       101/3"       100         Bldg. 5       S22       189/11"       100         Bldg. 5       S23       102/5"       100	DZ	Bldg. 5	S13	86	100
Bldg. 5         S16         104/5"         100           Bldg. 5         S17         62/0"         100           Bldg. 5         S18         179/10"         100           Bldg. 5         S19         101/2"         100           Bldg. 5         S20         104/5"         100           Bldg. 5         S21         101/3"         100           Bldg. 5         S22         189/11"         100           Bldg. 5         S23         102/5"         100		Bldg. 5	S14	103/5"	100
Bldg. 5       S17       62/0"       100         Bldg. 5       S18       179/10"       100         Bldg. 5       S19       101/2"       100         Bldg. 5       S20       104/5"       100         Bldg. 5       S21       101/3"       100         Bldg. 5       S22       189/11"       100         Bldg. 5       S23       102/5"       100		Bldg. 5	S15	103/5"	100
Bldg. 5       S18       179/10"       100         Bldg. 5       S19       101/2"       100         Bldg. 5       S20       104/5"       100         Bldg. 5       S21       101/3"       100         Bldg. 5       S22       189/11"       100         Bldg. 5       S23       102/5"       100		Bldg. 5	S16	104/5"	100
Bldg. 5       S18       179/10"       100         Bldg. 5       S19       101/2"       100         Bldg. 5       S20       104/5"       100         Bldg. 5       S21       101/3"       100         Bldg. 5       S22       189/11"       100         Bldg. 5       S23       102/5"       100			S17		100
Bldg. 5     S19     101/2"     100       Bldg. 5     S20     104/5"     100       Bldg. 5     S21     101/3"     100       Bldg. 5     S22     189/11"     100       Bldg. 5     S23     102/5"     100			S18		100
Bldg. 5     S20     104/5"     100       Bldg. 5     S21     101/3"     100       Bldg. 5     S22     189/11"     100       Bldg. 5     S23     102/5"     100			S19	101/2"	100
Bldg. 5     S21     101/3"     100       Bldg. 5     S22     189/11"     100       Bldg. 5     S23     102/5"     100					
Bldg. 5         S22         189/11"         100           Bldg. 5         S23         102/5"         100					
Bldg. 5 S23 102/5" 100					
		Bldg. 5			

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

	T	1	F'	
			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 5	S1	17	23
	Bldg. 5	S2	11	15
	Bldg. 5	S3	4	5
	Bldg. 5	S4	2	3
	Bldg. 5	S5	14	19
	Bldg. 5	S6	13	17
	Bldg. 5	S7	7	9
	Bldg. 5	S8	18	24
	Bldg. 5	S9	14	19
	Bldg. 5	S10	27	36
	Bldg. 5	S11	80	100
В3	Bldg. 5	S12	111	100
	Bldg. 5	S13	78	100
	Bldg. 5	S14	101/3"	100
	Bldg. 5	S15	100/3"	100
	Bldg. 5	S16	100/3"	100
	Bldg. 5	S17	124/5"	100
	Bldg. 5	S18	102/5"	100
	Bldg. 5	S19	103/5"	100
	Bldg. 5	S20	55/0"	100
	Bldg. 5	S21	178	100
	Bldg. 5	S22	100/4"	100
	Bldg. 5	S23	100/2"	100
	Bldg. 5	S24	173	100
	Bldg. 5	S1	21	28
	Bldg. 5	S2	9	12
	Bldg. 5	S3	9	12
	Bldg. 5	S4	8	11
	Bldg. 5	S5	9	12
	Bldg. 5	S6	47	63
	Bldg. 5	S7	107/3"	100
	Bldg. 5	S8	101/3"	100
	Bldg. 5	S9	39	52
	Bldg. 5	S10	100/5"	100
	Bldg. 5	S11	125	100
B4	Bldg. 5	S12	108	100
	Bldg. 5	S13	106/5"	100
	Bldg. 5	S14	97	100
	Bldg. 5	S15	101/2"	100
	Bldg. 5	S16	92	100
	Bldg. 5	S17	100/5"	100
	Bldg. 5	S18	107/4"	100
	Bldg. 5	S19	100/2"	100
	Bldg. 5	S20	126/3"	100
	Bldg. 5	S21	100/2"	100
	Bldg. 5	S22	102/5"	100
	Bldg. 5	S23	41	55
	Bldg. 5	S24	23	31

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

	1	1	l Piala	
			Field	E accidentate
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 5	S1	24	32
	Bldg. 5	S2	14	19
	Bldg. 5	S3	15	20
	Bldg. 5	S4	19	25
	Bldg. 5	S5	13	17
	Bldg. 5	S6	10	13
	Bldg. 5	S7	7	9
	Bldg. 5	S8	18	24
	Bldg. 5	S9	13	17
	Bldg. 5	S10	32	43
	Bldg. 5	S11	75	100
	Bldg. 5	S12	100/4"	100
B5	Bldg. 5	S13	77	100
	Bldg. 5	S14	102	100
	Bldg. 5	S15	101	100
	Bldg. 5	S16	101/1"	100
	Bldg. 5	S17	106/1"	100
	Bldg. 5	S18	72	96
	Bldg. 5	S19	154/10"	100
	Bldg. 5	S20	107/5"	100
	Bldg. 5	S21	108/3"	100
	Bldg. 5	S22	53	100
	Bldg. 5	S23	34	45
	Bldg. 5	S24	34	45
	Bldg. 5	S25	30	40
	Bldg. 4	S1	18	24
	Bldg. 4	S2	9	12
	Bldg. 4	S3	15	20
	Bldg. 4	S4	7	9
	Bldg. 4	S5	5	7
	Bldg. 4	S6	5	7
	Bldg. 4	S7	14	19
	Bldg. 4	S8	15	20
	Bldg. 4	S9	15	20
	Bldg. 4	S10	23	31
	Bldg. 4	S11	28	37
	Bldg. 4	S12	27	36
B6	Bldg. 4	S13	51	68
	Bldg. 4	S13	81	100
		S15	100/4"	100
	Bldg. 4			
	Bldg. 4	S16	101/4" 102/5"	100
	Bldg. 4	S17		100
	Bldg. 4	S18	100/3"	100
	Bldg. 4	S19	107/4"	100
	Bldg. 4	S20	108/4"	100
	Bldg. 4	S21	100/2"	100
	Bldg. 4	S22	100/3"	100
	Bldg. 4	S23	100/4"	100
Consultants	Bldg. 4	S24	103/5"	100

Consultants, Inc., P.C.

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Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

		1	Field	
			Measured	Equivalent
	B 11.8	0 1 -		
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 4	S1	45	60
	Bldg. 4	S2	15	20
	Bldg. 4	S3	27	36
	Bldg. 4	S4	18	24
	Bldg. 4	S5	22	29
	Bldg. 4	S6	18	24
	Bldg. 4	S7	77	100
	Bldg. 4	S8	11	15
	Bldg. 4	S9	15	20
	Bldg. 4	S10	12	16
	Bldg. 4	S11	32	43
В7	Bldg. 4	S12	38	51
D,	Bldg. 4	S13	101/5"	60
	Bldg. 4	S14	30	40
	Bldg. 4	S15	100/3"	100
	Bldg. 4	S16	57/0"	100
	Bldg. 4	S17	193/10"	100
	Bldg. 4	S18	138	100
	Bldg. 4	S19	102/1"	100
	Bldg. 4	S20	100/5"	100
	Bldg. 4	S21	101/4"	100
	Bldg. 4	S22	57/0"	100
	Bldg. 4	S23	100/5"	100
	Bldg. 4	S24	100/3"	100
	Bldg. 4	S1	24	32
	Bldg. 4	S2	55	73
	Bldg. 4	S3	24	32
	Bldg. 4	S4	24	32
	Bldg. 4	S5	8	11
	Bldg. 4	S6	9	12
	Bldg. 4	S7	11	15
	Bldg. 4	S8	61	81
	Bldg. 4	S9	69	92
DO		S10	64	85
БО		S11	105/5"	100
		S12	143	100
		S13	102/3"	100
		S14		100
		S15		100
В8		\$8 \$9 \$10 \$11 \$12 \$13 \$14	61 69 64 105/5" 143	81 92 85 100 100 100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

		ı	Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 4	S1	13	17
	Bldg. 4	S2	6	8
	Bldg. 4	S3	8	11
	Bldg. 4	S4	7	9
	Bldg. 4	S5	9	12
	Bldg. 4	S6	56	75
	Bldg. 4	S7	38	51
	Bldg. 4	S8	114/4"	100
	Bldg. 4	S9	144	100
	Bldg. 4	S10	127	100
	Bldg. 4	S11	165/9"	100
В9	Bldg. 4	S12	139	100
20	Bldg. 4	S13	216/9"	100
	Bldg. 4	S14	103/3"	100
	Bldg. 4	S15	115/3"	100
	Bldg. 4	S16	111/4"	100
	Bldg. 4	S17	121/4"	100
	Bldg. 4	S18	120/5"	100
	Bldg. 4	S19	100/5"	100
	Bldg. 4	S20 S21	191/11"	100
	Bldg. 4	S21	82 21	100 28
	Bldg. 4 Bldg. 4	S23	20	27
	Bldg. 4	S24	40	53
-	Bldg. 4	S1	26	35
	Bldg. 4	S2	13	17
	Bldg. 4	S3	8	11
	Bldg. 4	S4	5	7
	Bldg. 4	S5	7	9
	Bldg. 4	S6	8	11
	Bldg. 4	S7	44	59
	Bldg. 4	S8	50/0"	60
B10	Bldg. 4	S9	67	89
510	Bldg. 4	S10	128	100
	Bldg. 4	S11	100/3"	100
	Bldg. 4	S12	110/5"	100
	Bldg. 4	S13	100/3"	100
	Bldg. 4	S14	100/4"	100
	Bldg. 4	S15	110/3"	100
	Bldg. 4	S16	125/4"	100
	Bldg. 4	S17	111/3"	100
	Bldg. 4	S18	102/1"	100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
<u> </u>	•		`	,
	Bldg. 4	S1	14	19
	Bldg. 4	S2	72	96
	Bldg. 4	S3	8	11
	Bldg. 4	S4	5	7
	Bldg. 4	S5	6	8
	Bldg. 4	S6	12	16
	Bldg. 4	S7	37	49
	Bldg. 4	S8	42	56
	Bldg. 4	S9	61	81
B11	Bldg. 4	S10	100/3"	100
	Bldg. 4	S11	102/5"	100
	Bldg. 4	S12	107/5"	100
	Bldg. 4	S13	102/4"	100
	Bldg. 4	S14	145/10"	100
	Bldg. 4	S15	62/0"	100
	Bldg. 4	S16	100/5"	100
	Bldg. 4	S17	100/3"	100
	Bldg. 4	S18	122	100
	Bldg. 4	S19	100/1"	100
	Bldg. 3A/3B	S1	10	13
	Bldg. 3A/3B	S2	50/3"	60
	Bldg. 3A/3B	S3	40	53
	Bldg. 3A/3B	S4	14	19
	Bldg. 3A/3B	S5	4	5
	Bldg. 3A/3B	S6	8	11
	Bldg. 3A/3B	S7	20	27
	Bldg. 3A/3B	S8	43	57
B12	Bldg. 3A/3B	S9	83	100
DIZ	Bldg. 3A/3B	S10	86	100
	Bldg. 3A/3B	S11	100/2"	100
	Bldg. 3A/3B	S12	100	100
	Bldg. 3A/3B	S13	171/11"	100
	Bldg. 3A/3B	S14	100/5"	100
	Bldg. 3A/3B	S15	120	100
	Bldg. 3A/3B	S16	101/5"	100
	Bldg. 3A/3B	S17	103/3"	100
	Bldg. 3A/3B	S18	58	77

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

		I	Field	
			Measured	Equivalent
Poring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
Boring ID	Package	ID	(blows/foot)	(blows/foot)
			,	
	Bldg. 3A/3B	S1	15 7	20
	Bldg. 3A/3B	S2		9
	Bldg. 3A/3B	S3	12	16
	Bldg. 3A/3B	S4	5	5 7
	Bldg. 3A/3B	S5 S6		
	Bldg. 3A/3B	S7	10	13
	Bldg. 3A/3B		16	21
	Bldg. 3A/3B	S8	60	80
	Bldg. 3A/3B	S9	36	48
	Bldg. 3A/3B	S10	37	49
	Bldg. 3A/3B	S11	116	100
B13	Bldg. 3A/3B	S12	47	63
	Bldg. 3A/3B	S13	196	100
	Bldg. 3A/3B	S14	105	100
	Bldg. 3A/3B	S15	170/11"	100
	Bldg. 3A/3B	S16	101/4"	100
	Bldg. 3A/3B	S17	108/4"	100
	Bldg. 3A/3B	S18	104/3"	100
	Bldg. 3A/3B	S19	107/5"	100
	Bldg. 3A/3B	S20	100/4"	100
	Bldg. 3A/3B	S21	36	48
	Bldg. 3A/3B	S22	27	36
	Bldg. 3A/3B	S23	30	40
	Bldg. 3A/3B	S24	31	41
	Bldg. 3A/3B	S1	32	43
	Bldg. 3A/3B	S2	17	23
	Bldg. 3A/3B	S3	7	9
	Bldg. 3A/3B	S4	13	17
	Bldg. 3A/3B	S5	4	5
	Bldg. 3A/3B	S6	15	20
	Bldg. 3A/3B	S7	24	32
	Bldg. 3A/3B	S8	35	47
	Bldg. 3A/3B	S9	85	100
B14	Bldg. 3A/3B	S10	20/1"	100
	Bldg. 3A/3B	S11	100/4"	100
	Bldg. 3A/3B	S12	50/2"	100
	Bldg. 3A/3B	S13	100/5"	100
	Bldg. 3A/3B	S14	50/4"	100
	Bldg. 3A/3B	S15	150/9"	100
	Bldg. 3A/3B	S16	100/5"	100
	Bldg. 3A/3B	S17	100/3"	100
	Bldg. 3A/3B	S18	169/9"	100
	Bldg. 3A/3B	S19	100/2"	100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

	<u> </u>		Field	
			Measured	Equivalent
				Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	19	25
	Bldg. 3A/3B	S2	9	12
	Bldg. 3A/3B	S3	5	7
	Bldg. 3A/3B	S4	16	21
	Bldg. 3A/3B	S5	13	17
	Bldg. 3A/3B	S6	10	13
	Bldg. 3A/3B	S7	32	43
	Bldg. 3A/3B	S8	27	36
	Bldg. 3A/3B	S9	36	48
B15	Bldg. 3A/3B	S10	129	100
	Bldg. 3A/3B	S11	80	100
	Bldg. 3A/3B	S12	120/5"	100
	Bldg. 3A/3B	S13	100/1"	100
	Bldg. 3A/3B	S14	100/2"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	49	65
	Bldg. 3A/3B	S17	100/1"	100
	Bldg. 3A/3B	S18	103/0"	100
	Bldg. 3A/3B	S19	36/0"	100
	Bldg. 3A/3B	S1	44/3"	60
	Bldg. 3A/3B	S2	23	31
	Bldg. 3A/3B	S3	8	11
	Bldg. 3A/3B	S4	11	15
	Bldg. 3A/3B	S5	13	17
	Bldg. 3A/3B	S6	69	92
	Bldg. 3A/3B	S7	54	72
	Bldg. 3A/3B	S8	28	37
	Bldg. 3A/3B	S9	32	43
	Bldg. 3A/3B	S10	134	100
	Bldg. 3A/3B	S11	150/11"	100
D40	Bldg. 3A/3B	S12	106/5"	100
B16	Bldg. 3A/3B	S13	134	100
	Bldg. 3A/3B	S14	111	100
	Bldg. 3A/3B	S15	117	100
	Bldg. 3A/3B	S16	104/5"	100
	Bldg. 3A/3B	S17	79	100
	Bldg. 3A/3B	S18	113	100
	Bldg. 3A/3B	S19	87	100
	Bldg. 3A/3B	S20	62	83
	Bldg. 3A/3B	S21	48	64
	Bldg. 3A/3B	S22	35	47
	Bldg. 3A/3B	S23	56	75
	Bldg. 3A/3B	S24	40	53
_	Diag. JA/JD	L 024	70	55

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

Boring   Building   Package   ID   Measured   N-value, N <sub>m</sub>   (blows/foot)   N-value, N <sub>m</sub>   (blows/foot)		T	1	F'	
Boring   Building   Package   ID   N-value, N <sub>m</sub>   (blows/foot)   (blows/foot)				Field	Fauritralant
Bidg. 3A/3B		B 11.11	0 1 -		-
Bidg. 3A/3B   S1   16/6"   60	_	_			
Bidg. 3A/3B   S2   16   21	ID	Раскаде	ID	(blows/foot)	(blows/foot)
Bldg. 3A/3B   S3   12   16		•			
Bidg. 3A/3B					
Bidg. 3A/3B					
Bidg. 3A/3B		Bldg. 3A/3B			37
Bidg. 3A/3B S7 72 96 Bidg. 3A/3B S8 32 43 Bidg. 3A/3B S9 30 40 Bidg. 3A/3B S10 50/3" 100 Bidg. 3A/3B S11 100/5" 100 Bidg. 3A/3B S13 50/3" 100 Bidg. 3A/3B S13 50/3" 100 Bidg. 3A/3B S13 50/3" 100 Bidg. 3A/3B S15 100/5" 100 Bidg. 3A/3B S15 100/5" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S16 100/3" 100 Bidg. 3A/3B S16 100/3" 100 Bidg. 3A/3B S18 1100/4" 100 Bidg. 3A/3B S18 100/4" 100 Bidg. 3A/3B S1 57 7 Bidg. 3A/3B S2 12 12 16 Bidg. 3A/3B S3 15 20 Bidg. 3A/3B S3 15 20 Bidg. 3A/3B S5 20 27 Bidg. 3A/3B S6 20 27 Bidg. 3A/3B S8 13 17 Bidg. 3A/3B S8 11 65 87 Bidg. 3A/3B S1 100/1" 100 Bidg. 3A/3B S1 100 100/1" 100 Bidg. 3A/3B S11 65 87 Bidg. 3A/3B S11 65 87 Bidg. 3A/3B S11 100 100/1" 100 Bidg. 3A/3B S11 165 87 Bidg. 3A/3B S11 100 100/1" 100 Bidg. 3A/3B S11 100 100/5" 100 Bidg. 3A/3B S15 111/4" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S18 143 100 Bidg. 3A/3B S19 69 92 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S22 101/2" 100		Bldg. 3A/3B		11	
Bidg. 3A/3B		Bldg. 3A/3B	S6	20	27
BIdg. 3A/3B   S9   30   40		Bldg. 3A/3B	S7	72	96
Bidg. 3A/3B		Bldg. 3A/3B	S8	32	43
Bidg. 3A/3B   S11   100/5"   100	B17	Bldg. 3A/3B	S9	30	40
Bidg. 3A/3B   S13   50/3"   100		Bldg. 3A/3B	S10	50/3"	100
Bidg. 3A/3B		Bldg. 3A/3B	S11	100/5"	100
Bidg. 3A/3B		Bldg. 3A/3B	S13	50/3"	100
Bidg. 3A/3B		Bldg. 3A/3B	S14	47/4"	100
Bldg. 3A/3B		Bldg. 3A/3B	S15	100/5"	100
Bldg. 3A/3B			S16		100
Bidg. 3A/3B         \$18         \$100/4"         \$100           Bidg. 3A/3B         \$1         \$5         \$7           Bidg. 3A/3B         \$2         \$12         \$16           Bidg. 3A/3B         \$3         \$15         \$20           Bidg. 3A/3B         \$4         \$25         \$33           Bidg. 3A/3B         \$5         \$20         \$27           Bidg. 3A/3B         \$6         \$20         \$27           Bidg. 3A/3B         \$8         \$13         \$17           Bidg. 3A/3B         \$8         \$13         \$17           Bidg. 3A/3B         \$9         \$94         \$100           Bidg. 3A/3B         \$10         \$100/1"         \$100           Bidg. 3A/3B         \$11         \$65         \$87           Bidg. 3A/3B         \$12         \$100/2"         \$100           Bidg. 3A/3B         \$13         \$100         \$100           Bidg. 3A/3B         \$13         \$100         \$100           Bidg. 3A/3B         \$14         \$177/8"         \$100           Bidg. 3A/3B         \$15         \$111/4"         \$100           Bidg. 3A/3B         \$16         \$100/5"         \$100           B			S17	100/5"	100
BIdg. 3A/3B S1 5 7 Bidg. 3A/3B S2 12 16 Bidg. 3A/3B S3 15 20 Bidg. 3A/3B S4 25 33 Bidg. 3A/3B S5 20 27 Bidg. 3A/3B S5 20 27 Bidg. 3A/3B S6 20 27 Bidg. 3A/3B S6 20 27 Bidg. 3A/3B S7 100/4" 60 Bidg. 3A/3B S8 13 17 Bidg. 3A/3B S9 94 100 Bidg. 3A/3B S9 94 100 Bidg. 3A/3B S10 100/1" 100 Bidg. 3A/3B S11 65 87 Bidg. 3A/3B S11 65 87 Bidg. 3A/3B S12 100/2" 100 Bidg. 3A/3B S13 100 100 Bidg. 3A/3B S13 100 100 Bidg. 3A/3B S14 177/8" 100 Bidg. 3A/3B S15 111/4" 100 Bidg. 3A/3B S15 111/4" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S17 100 100 Bidg. 3A/3B S18 143 100 Bidg. 3A/3B S19 69 92 Bidg. 3A/3B S19 69 92 Bidg. 3A/3B S20 100/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S22 101/2" 100 Bidg. 3A/3B S22 101/2" 100 Bidg. 3A/3B S22 101/2" 100			S18	100/4"	100
Bldg. 3A/3B					7
Bidg. 3A/3B S3 15 20 Bidg. 3A/3B S4 25 33 Bidg. 3A/3B S5 20 27 Bidg. 3A/3B S6 20 27 Bidg. 3A/3B S7 100/4" 60 Bidg. 3A/3B S8 13 17 Bidg. 3A/3B S9 94 100 Bidg. 3A/3B S10 100/1" 100 Bidg. 3A/3B S10 100/1" 100 Bidg. 3A/3B S11 65 87 Bidg. 3A/3B S12 100/2" 100 Bidg. 3A/3B S12 100/2" 100 Bidg. 3A/3B S13 100 100 Bidg. 3A/3B S14 177/8" 100 Bidg. 3A/3B S15 111/4" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S16 100/5" 100 Bidg. 3A/3B S17 100 100 Bidg. 3A/3B S18 143 100 Bidg. 3A/3B S19 69 92 Bidg. 3A/3B S19 69 92 Bidg. 3A/3B S20 100/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S21 114/3" 100 Bidg. 3A/3B S22 101/2" 100 Bidg. 3A/3B S22 101/2" 100 Bidg. 3A/3B S23 100/2" 100			S2	12	16
Bldg. 3A/3B		Bldg. 3A/3B	S3	15	20
Bidg. 3A/3B			S4	25	33
Bidg. 3A/3B			S5	20	27
BIdg. 3A/3B			S6	20	27
BIdg. 3A/3B			S7	100/4"	60
BIdg. 3A/3B S9 94 100 Bldg. 3A/3B S10 100/1" 100 Bldg. 3A/3B S11 65 87 Bldg. 3A/3B S12 100/2" 100 Bldg. 3A/3B S13 100 100 Bldg. 3A/3B S14 177/8" 100 Bldg. 3A/3B S15 111/4" 100 Bldg. 3A/3B S16 100/5" 100 Bldg. 3A/3B S16 100/5" 100 Bldg. 3A/3B S17 100 100 Bldg. 3A/3B S18 143 100 Bldg. 3A/3B S18 143 100 Bldg. 3A/3B S19 69 92 Bldg. 3A/3B S20 100/3" 100 Bldg. 3A/3B S21 114/3" 100 Bldg. 3A/3B S21 114/3" 100 Bldg. 3A/3B S22 101/2" 100 Bldg. 3A/3B S23 100/2" 100			S8		17
BIdg. 3A/3B S10 100/1" 100 Bldg. 3A/3B S11 65 87 Bldg. 3A/3B S12 100/2" 100 Bldg. 3A/3B S13 100 100 Bldg. 3A/3B S14 177/8" 100 Bldg. 3A/3B S15 111/4" 100 Bldg. 3A/3B S16 100/5" 100 Bldg. 3A/3B S16 100/5" 100 Bldg. 3A/3B S17 100 100 Bldg. 3A/3B S18 143 100 Bldg. 3A/3B S18 143 100 Bldg. 3A/3B S19 69 92 Bldg. 3A/3B S20 100/3" 100 Bldg. 3A/3B S21 114/3" 100 Bldg. 3A/3B S21 114/3" 100 Bldg. 3A/3B S22 101/2" 100 Bldg. 3A/3B S23 100/2" 100			S9	94	100
BIdg. 3A/3B S11 65 87  Bldg. 3A/3B S12 100/2" 100  Bldg. 3A/3B S13 100 100  Bldg. 3A/3B S14 177/8" 100  Bldg. 3A/3B S15 111/4" 100  Bldg. 3A/3B S16 100/5" 100  Bldg. 3A/3B S17 100 100  Bldg. 3A/3B S17 100 100  Bldg. 3A/3B S18 143 100  Bldg. 3A/3B S19 69 92  Bldg. 3A/3B S20 100/3" 100  Bldg. 3A/3B S21 114/3" 100  Bldg. 3A/3B S22 101/2" 100  Bldg. 3A/3B S23 100/2" 100			S10	100/1"	100
BIdg. 3A/3B S12 100/2" 100  Bldg. 3A/3B S13 100 100  Bldg. 3A/3B S14 177/8" 100  Bldg. 3A/3B S15 111/4" 100  Bldg. 3A/3B S16 100/5" 100  Bldg. 3A/3B S17 100 100  Bldg. 3A/3B S18 143 100  Bldg. 3A/3B S18 143 100  Bldg. 3A/3B S19 69 92  Bldg. 3A/3B S20 100/3" 100  Bldg. 3A/3B S21 114/3" 100  Bldg. 3A/3B S21 114/3" 100  Bldg. 3A/3B S22 101/2" 100  Bldg. 3A/3B S23 100/2" 100			S11		87
BIdg. 3A/3B S13 100 100  Bldg. 3A/3B S14 177/8" 100  Bldg. 3A/3B S15 111/4" 100  Bldg. 3A/3B S16 100/5" 100  Bldg. 3A/3B S17 100 100  Bldg. 3A/3B S18 143 100  Bldg. 3A/3B S18 143 100  Bldg. 3A/3B S19 69 92  Bldg. 3A/3B S20 100/3" 100  Bldg. 3A/3B S21 114/3" 100  Bldg. 3A/3B S21 114/3" 100  Bldg. 3A/3B S22 101/2" 100  Bldg. 3A/3B S23 100/2" 100	D40	•	S12	100/2"	100
Bldg. 3A/3B         S14         177/8"         100           Bldg. 3A/3B         S15         111/4"         100           Bldg. 3A/3B         S16         100/5"         100           Bldg. 3A/3B         S17         100         100           Bldg. 3A/3B         S18         143         100           Bldg. 3A/3B         S19         69         92           Bldg. 3A/3B         S20         100/3"         100           Bldg. 3A/3B         S21         114/3"         100           Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100	B18	•			
Bldg. 3A/3B         S15         111/4"         100           Bldg. 3A/3B         S16         100/5"         100           Bldg. 3A/3B         S17         100         100           Bldg. 3A/3B         S18         143         100           Bldg. 3A/3B         S19         69         92           Bldg. 3A/3B         S20         100/3"         100           Bldg. 3A/3B         S21         114/3"         100           Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100			S14		
Bldg. 3A/3B         S16         100/5"         100           Bldg. 3A/3B         S17         100         100           Bldg. 3A/3B         S18         143         100           Bldg. 3A/3B         S19         69         92           Bldg. 3A/3B         S20         100/3"         100           Bldg. 3A/3B         S21         114/3"         100           Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100					
Bldg. 3A/3B         S17         100         100           Bldg. 3A/3B         S18         143         100           Bldg. 3A/3B         S19         69         92           Bldg. 3A/3B         S20         100/3"         100           Bldg. 3A/3B         S21         114/3"         100           Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100		-			
Bldg. 3A/3B         S18         143         100           Bldg. 3A/3B         S19         69         92           Bldg. 3A/3B         S20         100/3"         100           Bldg. 3A/3B         S21         114/3"         100           Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100					
BIdg. 3A/3B       S19       69       92         BIdg. 3A/3B       S20       100/3"       100         BIdg. 3A/3B       S21       114/3"       100         BIdg. 3A/3B       S22       101/2"       100         BIdg. 3A/3B       S23       100/2"       100					
Bldg. 3A/3B     S20     100/3"     100       Bldg. 3A/3B     S21     114/3"     100       Bldg. 3A/3B     S22     101/2"     100       Bldg. 3A/3B     S23     100/2"     100					
Bldg. 3A/3B     S21     114/3"     100       Bldg. 3A/3B     S22     101/2"     100       Bldg. 3A/3B     S23     100/2"     100					
Bldg. 3A/3B         S22         101/2"         100           Bldg. 3A/3B         S23         100/2"         100					
Bldg. 3A/3B S23 100/2" 100					
		•			

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
	Bldg. 3A/3B	S1	50/3"	60
	Bldg. 3A/3B	S2	100/5"	60
	Bldg. 3A/3B	S3	10	13
	Bldg. 3A/3B	S4	16	21
	Bldg. 3A/3B	S5	20	27
	Bldg. 3A/3B	S6	28	37
	Bldg. 3A/3B	S7	67	89
	Bldg. 3A/3B	S8	47	63
B19	Bldg. 3A/3B	S9	100/5"	100
	Bldg. 3A/3B	S10	50/1"	100
	Bldg. 3A/3B	S11	50/3"	100
	Bldg. 3A/3B	S12	100/4"	100
	Bldg. 3A/3B	S13	100/4"	100
	Bldg. 3A/3B	S14	100/5"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	50/2"	100
	Bldg. 3A/3B	S17	100/5"	100
	Bldg. 3A/3B	S1	35	47
	Bldg. 3A/3B	S2	57	76
	Bldg. 3A/3B	S3	32	43
	Bldg. 3A/3B	S4	18	24
	Bldg. 3A/3B	S5	42	56
	Bldg. 3A/3B	S6	29	39
	Bldg. 3A/3B	S7	37	49
	Bldg. 3A/3B	S8	71	95
B20	Bldg. 3A/3B	S9	42	56
D2U	Bldg. 3A/3B	S10	51/4"	100
	Bldg. 3A/3B	S11	100/4"	100
	Bldg. 3A/3B	S12	100/5"	100
	Bldg. 3A/3B	S13	100/3"	100
	Bldg. 3A/3B	S14	100/4"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	100/4"	100
	Bldg. 3A/3B	S17	50/3"	100
	Bldg. 3A/3B	S18	100/1"	100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

-			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
U	<u> </u>		,	` ,
	Bldg. 3A/3B	S1	25	33
	Bldg. 3A/3B	S2	32	43
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	23	31
	Bldg. 3A/3B	S5	54	72
	Bldg. 3A/3B	S6	100/5"	60
	Bldg. 3A/3B	S7	13	17
	Bldg. 3A/3B	S8	2	3
B21	Bldg. 3A/3B	S9	18	24
DZ I	Bldg. 3A/3B	S10	10	13
	Bldg. 3A/3B	S11	32	43
	Bldg. 3A/3B	S12	45	60
	Bldg. 3A/3B	S13	52	69
	Bldg. 3A/3B	S14	192/8"	100
	Bldg. 3A/3B	S15	100/5"	100
	Bldg. 3A/3B	S16	134/11"	100
	Bldg. 3A/3B	S17	100/3"	100
	Bldg. 3A/3B	S18	165/11"	100
	Bldg. 3A/3B	S1	21	28
	Bldg. 3A/3B	S2	25	33
	Bldg. 3A/3B	S3	25	33
	Bldg. 3A/3B	S4	6	8
	Bldg. 3A/3B	S5	22	29
	Bldg. 3A/3B	S6	5	7
	Bldg. 3A/3B	S7	7	9
	Bldg. 3A/3B	S8	100/5"	100
	Bldg. 3A/3B	S9	89	100
B22	Bldg. 3A/3B	S10	156/11"	100
	Bldg. 3A/3B	S11	164/10"	100
	Bldg. 3A/3B	S12	113/5"	100
	Bldg. 3A/3B	S13	102/4"	100
	Bldg. 3A/3B	S14	101/4"	100
	Bldg. 3A/3B	S15	100/3"	100
	Bldg. 3A/3B	S16	100/5"	100
	Bldg. 3A/3B	S17	100/1"	100
	Bldg. 3A/3B	S18	102/3"	100
	Bldg. 3A/3B	S19	100/1"	100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

			Field	
			Measured	Equivalent
Boring	Building	Sample	N-value, N <sub>m</sub>	N-value, N <sub>60</sub>
ID	Package	ID	(blows/foot)	(blows/foot)
1	Bldg. 3A/3B	S1	63	84
	Bldg. 3A/3B	S2	70	93
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	7	9
	Bldg. 3A/3B	S5	14	19
	Bldg. 3A/3B	S6	8	11
	Bldg. 3A/3B	S7	13	17
	Bldg. 3A/3B	S8	5	7
	Bldg. 3A/3B	S9	27	36
B23	Bldg. 3A/3B	S10	14	19
	Bldg. 3A/3B	S11	69	92
	Bldg. 3A/3B	S12 S13	102/1"	100
	Bldg. 3A/3B Bldg. 3A/3B	S13	127 125	100 100
	Bldg. 3A/3B	S15	102/3"	100
	Bldg. 3A/3B	S16	104/2"	100
	Bldg. 3A/3B	S17	79	100
	Bldg. 3A/3B	S18	145	100
	Bldg. 3A/3B	S19	50	67
	Bldg. 3A/3B	S20	105/1"	100
	N/A	S1	9	12
	N/A	S2	30	40
	N/A	S3	39	52
	N/A	S4	17	23
	N/A	S5	34	45
	N/A	S6	16	21
	N/A	S7	9	12
	N/A	S8	17	23
	N/A	S9	42	56
B29	N/A	S10	104/3"	100
	N/A	S11	101/3"	100
	N/A	S12	148	100
	N/A	S13	102/2"	100
	N/A	S14	199/10"	100
	N/A	S15	110/5"	100
	N/A	S16	102/2"	100
	N/A	S17	105/1"	100
	N/A	S18	100/4"	100
	N/A	S19	45/0"	100

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

		ı	Field	
Boring ID	Building Package	Sample ID	Measured N-value, N <sub>m</sub> (blows/foot)	Equivalent N-value, N <sub>60</sub> (blows/foot)
	N/A	S1	11	15
	N/A	S2	13	17
	N/A	S3	11	15
	N/A	S4	6	8
	N/A	S5	10	13
	N/A	S6	14	19
B53	N/A	S7	25	33
	N/A	S8	93	100
	N/A	S9	73	97
	N/A	S10	48	64
	N/A	S11	105	100
	N/A	S12	100/4"	100
	N/A	S13	53/0"	100
	Bldg. 3A/3B	S1	13	17
	Bldg. 3A/3B	S2	24	32
	Bldg. 3A/3B	S3	30	40
	Bldg. 3A/3B	S4	13	17
	Bldg. 3A/3B	S5	5	7
	Bldg. 3A/3B	S6	3	4
	Bldg. 3A/3B	S7	2	3
	Bldg. 3A/3B	S8	2	3
	Bldg. 3A/3B	S9	51	68
	Bldg. 3A/3B	S10	29	39
	Bldg. 3A/3B	S11	65	87
B101	Bldg. 3A/3B	S12	123	100
БПП	Bldg. 3A/3B	S13	100/5"	100
	Bldg. 3A/3B	S14	122	100
	Bldg. 3A/3B	S15	101/4"	100
	Bldg. 3A/3B	S16	100/4"	100
	Bldg. 3A/3B	S17	100/5"	100
	Bldg. 3A/3B	S18	50/0"	100
	Bldg. 3A/3B	S19	51/4"	100
	Bldg. 3A/3B	S20	69	92
	Bldg. 3A/3B	S21	69	92
	Bldg. 3A/3B	S22	41	55
	Bldg. 3A/3B	S23	22	29
	Bldg. 3A/3B	S24	27	36

Table 2. Equivalent SPT N-Values Geotechnical Report Astoria Cove Development Astoria, Queens, New York

Boring ID	Building Package	Sample ID	Field Measured N-value, N <sub>m</sub> (blows/foot)	Equivalent N-value, N <sub>60</sub> (blows/foot)
	Bldg. 3A/3B	S1	12	16
	Bldg. 3A/3B	S2	10	13
	Bldg. 3A/3B	S3	7	9
	Bldg. 3A/3B	S4	7	9
	Bldg. 3A/3B	S5	84	100
	Bldg. 3A/3B	S6	19	25
	Bldg. 3A/3B	S7	35	47
	Bldg. 3A/3B	S8	36	48
	Bldg. 3A/3B	S9	109	100
B102	Bldg. 3A/3B	S10	93	100
D102	Bldg. 3A/3B	S11	100/5"	100
	Bldg. 3A/3B	S12	50/0"	100
	Bldg. 3A/3B	S13	184/9"	100
	Bldg. 3A/3B	S14	173/11"	100
	Bldg. 3A/3B	S15	100/3"	100
	Bldg. 3A/3B	S16	100/2"	100
	Bldg. 3A/3B	S17	101/5"	100
	Bldg. 3A/3B	S18	100/3"	100
	Bldg. 3A/3B	S19	100/3"	100
	Bldg. 3A/3B	S20	165	100

Table 3. Groundwater Level Data in Observation Wells Geotechnical Report Astoria Cove Development Astoria, Queens, New York

		Loca	ition1	Approximate	Depth	Top of	Screen	Interval	Depth	Date of	Water Level Reading #1			Water Level Reading #2				Water Level Reading #3			<del>‡</del> 3	
Well ID	Building ID			Ground Surface		PVC	Тор	Bottom		Well												
		Northing	Easting	Elevation <sup>2</sup>	of PVC		Depth		' 5	Development	Date			Notes	Date	_	Elevation	Notes	Date		Elevation	Notes
				(feet)	(feet)	(feet)	(feet)	(feet)	(feet)			(feet)	(feet)			(feet)	(feet)			(feet)	(feet)	
B5(OW)	Bldg. 5	221,770	1,003,343	25.0	0.00	25.0	15.0	25.0	102.0	Note 3	03/10/20	18.8	6.2	-	-	-	-	-	11/14/23	Dry	Dry	5
B8(OW)	Bldg. 4	221,825	1,003,207	23.2	0.30	22.9	20.0	30.0	85.0	Note 3	03/05/20	18.3	4.6	-	03/09/20	18.1	4.8	-	11/14/23	17.0	Uncertain	5
B16(OW)	Bldg. 3A/3B	221,903	1,003,499	21.9	0.61	21.3	3.0	13.0	14.0	Note 3	03/05/20	17.3	4.0	-	03/10/20	17.3	4.0	-	-	-	-	6
B29(OW)	N/A	222,066	1,003,320	21.9	0.35	21.6	3.0	13.0	21.0	Note 3	03/18/20	16.9	5.0	-	-	-	-	-	-	-	-	6
B48(OW)	N/A	222,448	1,003,170	12.9	0.33	12.5	4.0	13.7	14.0	Note 3	05/21/20	11.1	1.4	-	-	-	-	-	11/14/23	10.8	2.1	-
B53(OW)	N/A	221,759	1,003,421	29.4	0.33	29.1	4.0	13.7	28.1	Note 3	05/15/20	23.5	5.6	-	-	-	-	-	-	-	-	6

### Notes:

- 1. Coordinates for boring locations are taken from taped off distances frm survey site features and are referenced to NAD83.
- 2. Ground surface elevations are estimated from topographic data and are referenced to NAVD88.
- 3. Well development not required in observation wells.
- 4. Water level measured in casing at the completion of drilling.
- 5. Surface casing damaged/missing with PVC riser exposed; debris had fallen into well. Measurements made from exposed PVC casing are likely erroneous.
- 6. Well not located.

Table 4. Groundwater Level Data in Monitoring Wells Geotechnical Report Astoria Cove Development Astoria, Queens, New York

Monitoring Wells	Building ID	Date	Approx. Screen Interval Depth, (feet)	Top of Casing Elevation, (feet)	Depth to Water, (feet)	Depth to Bottom, (feet)	Groundwater Elevation, (feet)	
MW-1	Bldg. 3A/3B	2/1/2022	15 to 25	19.1	17.2	23.1	2.0	
MW-2	Bldg. 3A/3B	2/2/2022	10 to 20	12.4	11.3	19.4	1.1	
MW-3	Bldg. 3A/3B	2/2/2022	10 to 20	14.1	12.8	19.6	1.4	
MW-4	Bldg. 3A/3B	2/1/2022	20 to 30	19.6	17.6	28.6	2.0	
MW-5	Bldg. 4	2/2/2022	15 to 25	21.9	18.8	24.4	3.1	
MW-6	Bldg. 4	2/2/2022	15 to 25	23.4	18.9	29.7	4.5	
MW-11	Bldg. 5	2/1/2022	15 to 25	25.5	21.6	28.5	3.9	
MW-12	Bldg. 5	2/1/2022	20 to 30	24.6	21.7	30.1	2.9	

### Notes:

1. Ground surface elevations are estimated from topographic data and are referenced to NAVD88.

Table 5. Recommended Geotechnical Design Parameters Geotechnical Report Astoria Cove Development Astoria, Queens, New York

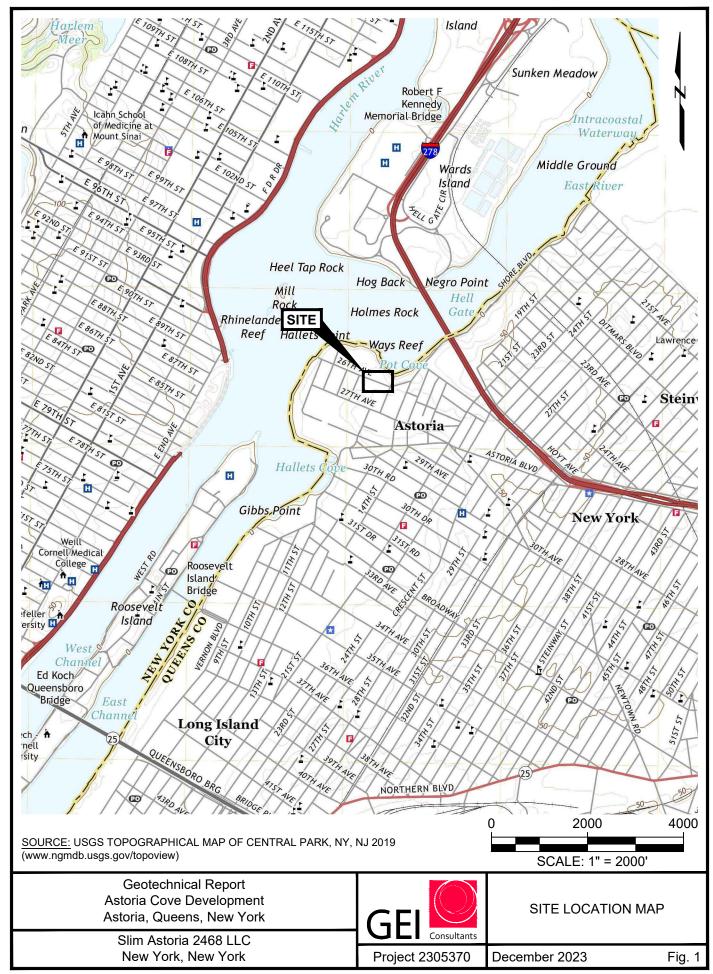
	Total U	nit Weight	Drained Parameters						
Soil Material <sup>(1)</sup>	Above Water Table Below Water Table		Drained Friction Angle	Effective Cohesion	Earth Pressure Coefficients <sup>(2)</sup>				
	γ <sub>t</sub> (pcf)	γ <sub>sat</sub> (pcf)	φ' (degrees)	C' (ksf)	K <sub>o</sub>	K <sub>a</sub>	K <sub>p</sub>		
Stratum I -Fill (Class 7)	120	122	34	0	0.44	0.28	3.5		
Stratum IIa - Loose Sand (Class 6)	115	118	30	0	0.50	0.33	3.0		
Stratum IIb - Silt/Clay (Class 4b/5b)	115	118	30	1	0.50	0.33	3.0		
Stratum IIc - Sand (Class 3b)	120	122	34	0	0.44	0.28	3.5		
Stratum III - Till (Class 3a)	130	135	40	0	0.36	0.22	4.6		
Stratum IV - Decomposed/Weathered Rock (Class 1d)	130	135	40	0	0.36	0.22	4.6		
Stratum V - Bedrock (Class 1b/1c)	150	150							
Ordinary Fill (92% Compaction) <sup>(3)</sup>	120	125	32	0	0.47	0.31	3.3		
Structural Fill (95% Compaction) <sup>(4)</sup>	120	125	35	0	0.43	0.27	3.7		

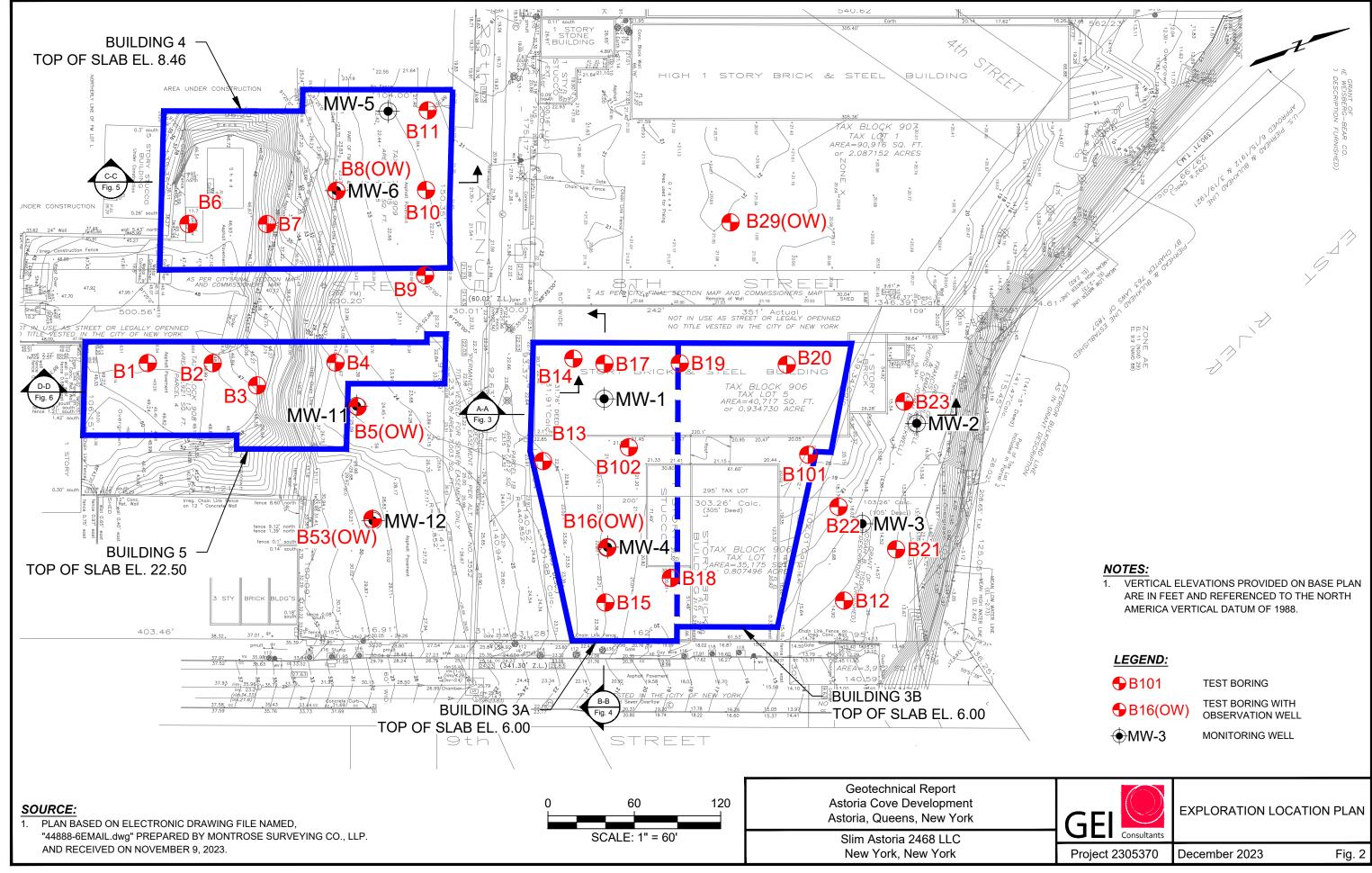
#### Footnotes:

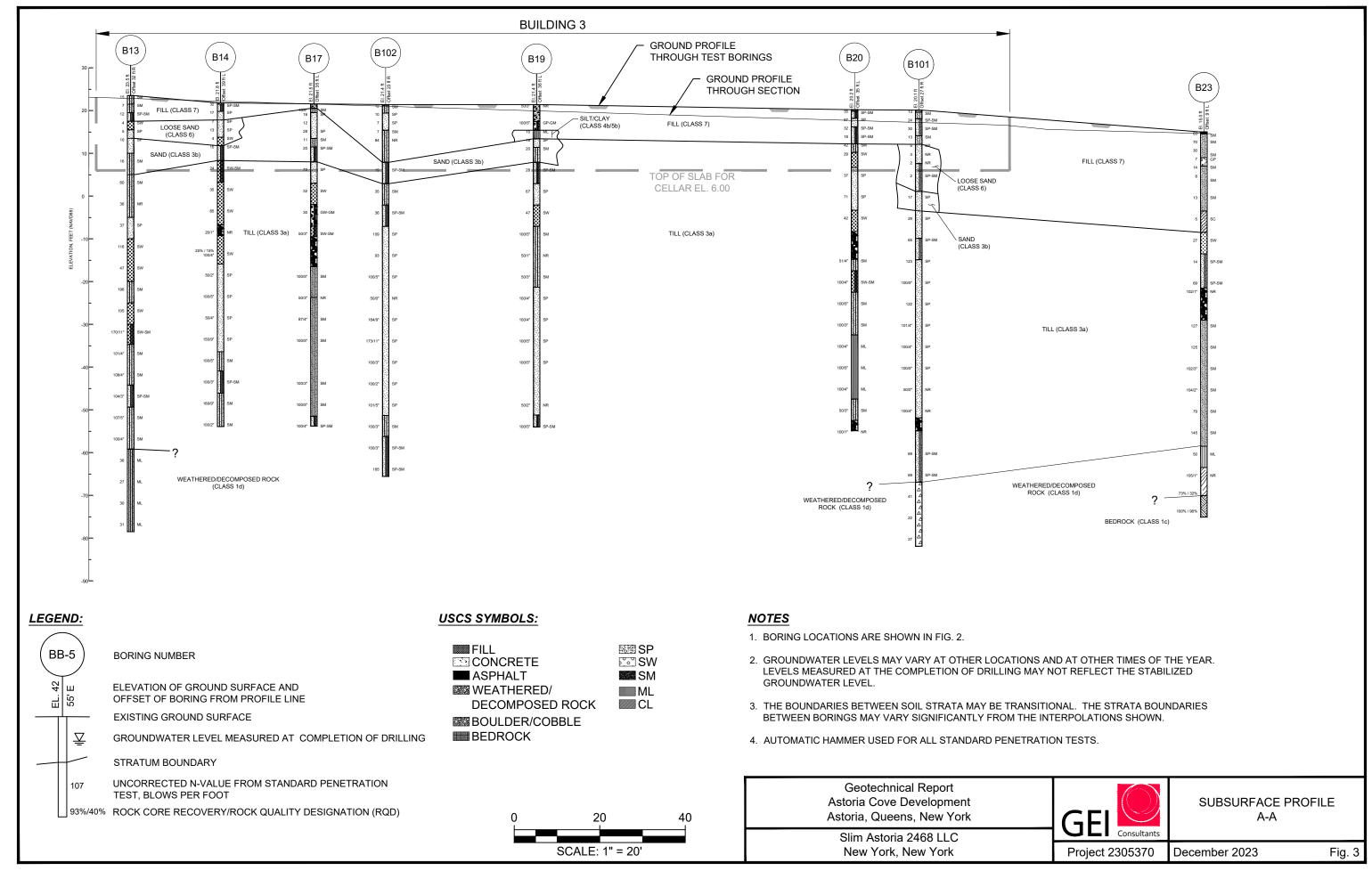
- 1. The values of soil/rock properties in this table are based on site-specific field test results (where available), empirical correlations using the results of standard penetration tests and laboratory index tests, and engineering judgement.
- 2. Parameters to be used for footings and structures:
  - K<sub>o</sub> = Coefficient of Earth Pressure at Rest (Rankine Method)
  - K<sub>a</sub> = Active Earth Pressure Coefficient (Rankine Method)
  - K<sub>p</sub> = Passive Earth Pressure Coefficient (Rankine)
- 3. For material compacted to ~92% of Modified Proctor maximum dry density in accordance with ASTM D1557.
- 4. For material compacted to ~95% of Modified Proctor maximum dry density in accordance with ASTM D1557.

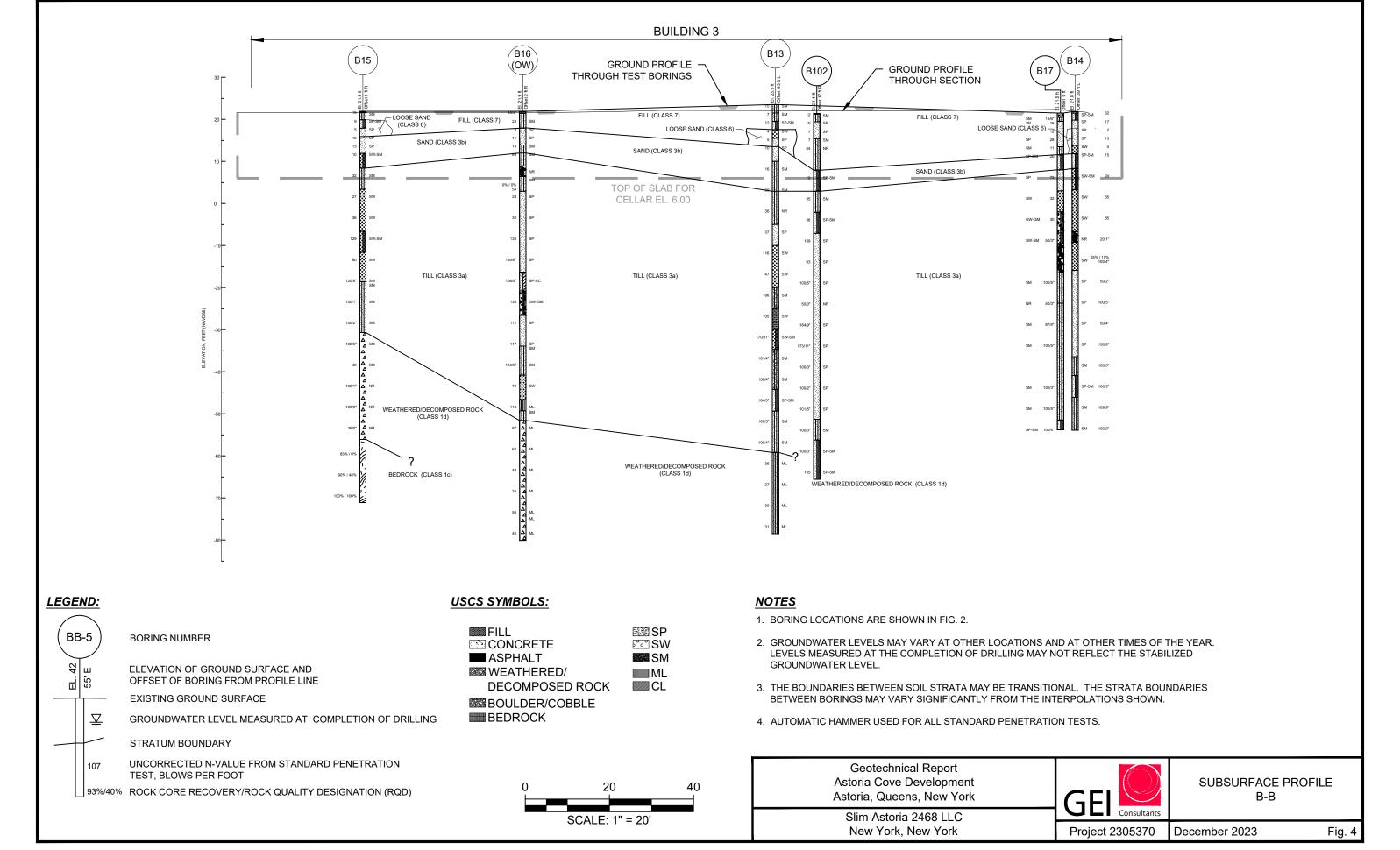
Geotechnical Report
Astoria Cove Development
Buildings 3A/3B, 4 & 5
Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 |
Block 909 Lot 35
Astoria, Queens County, New York
December 2023

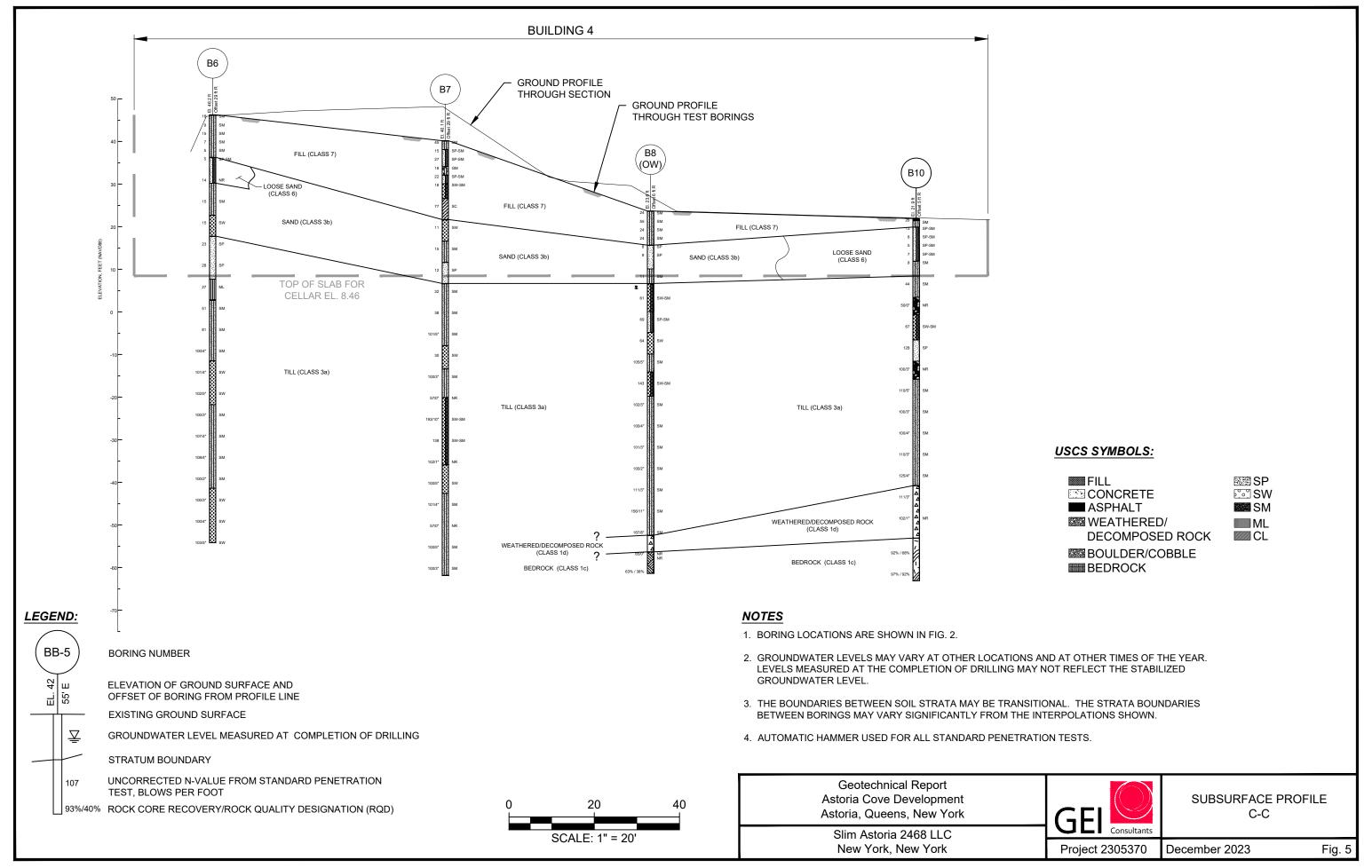
# **Figures**

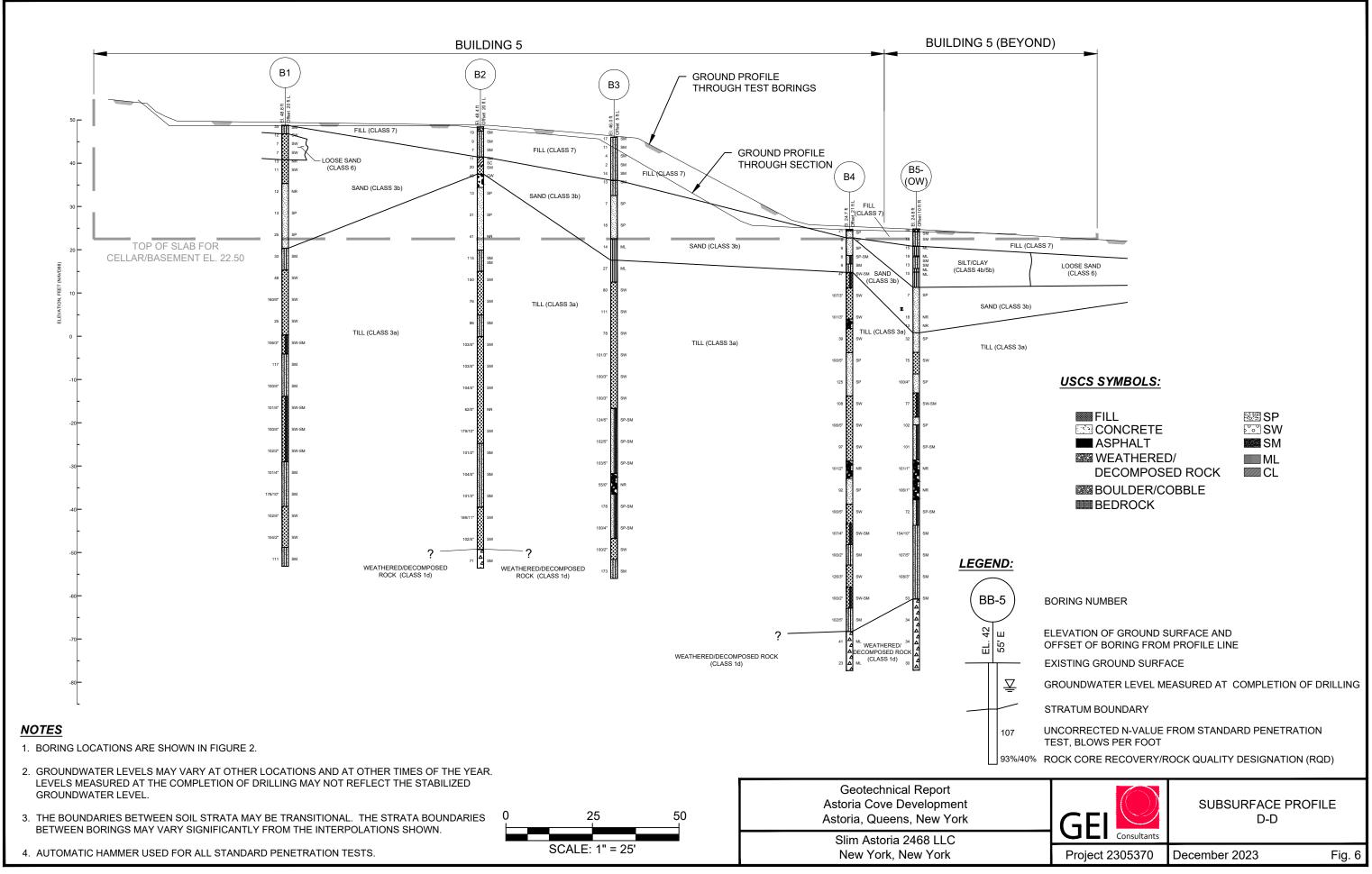


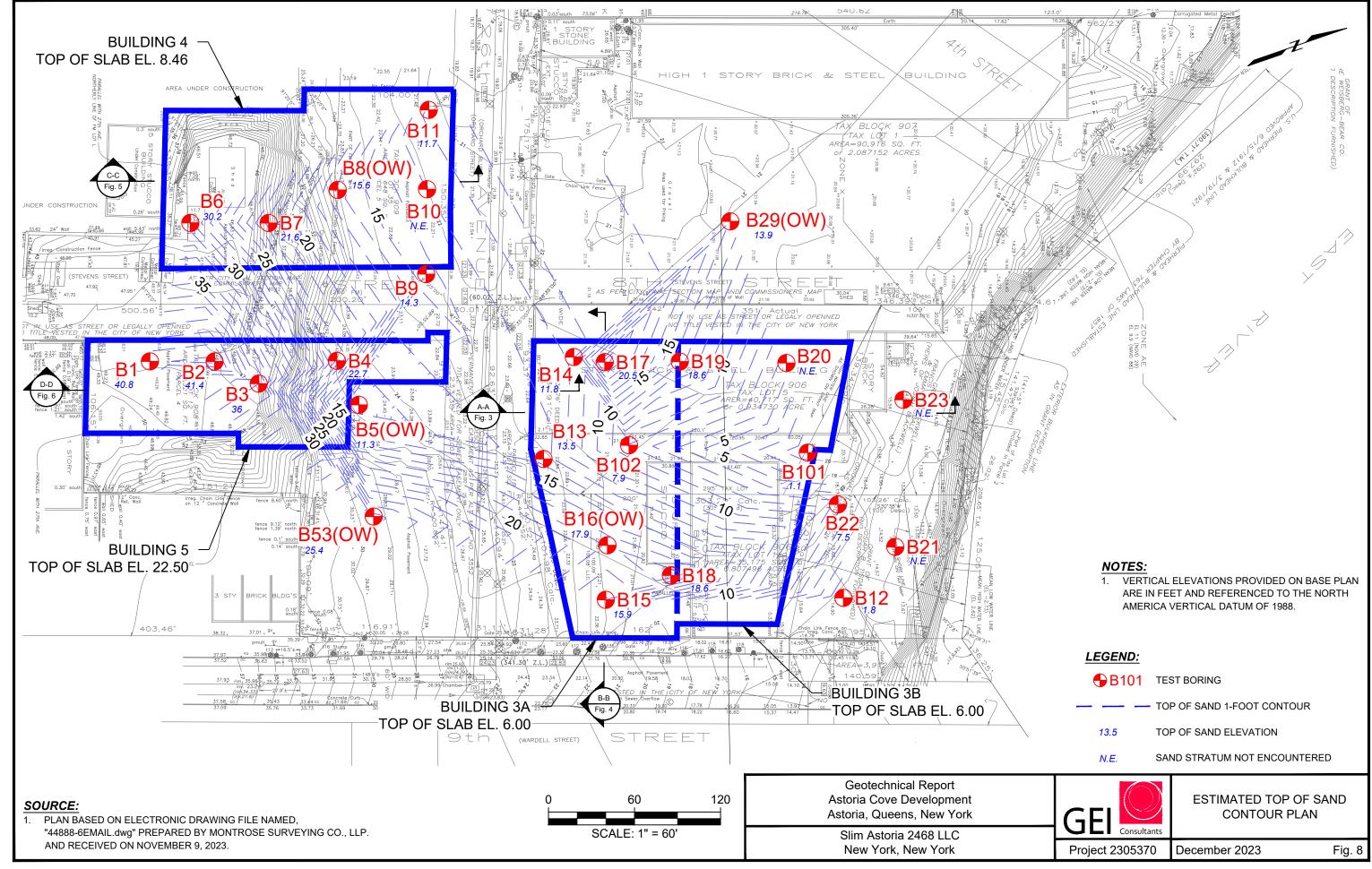


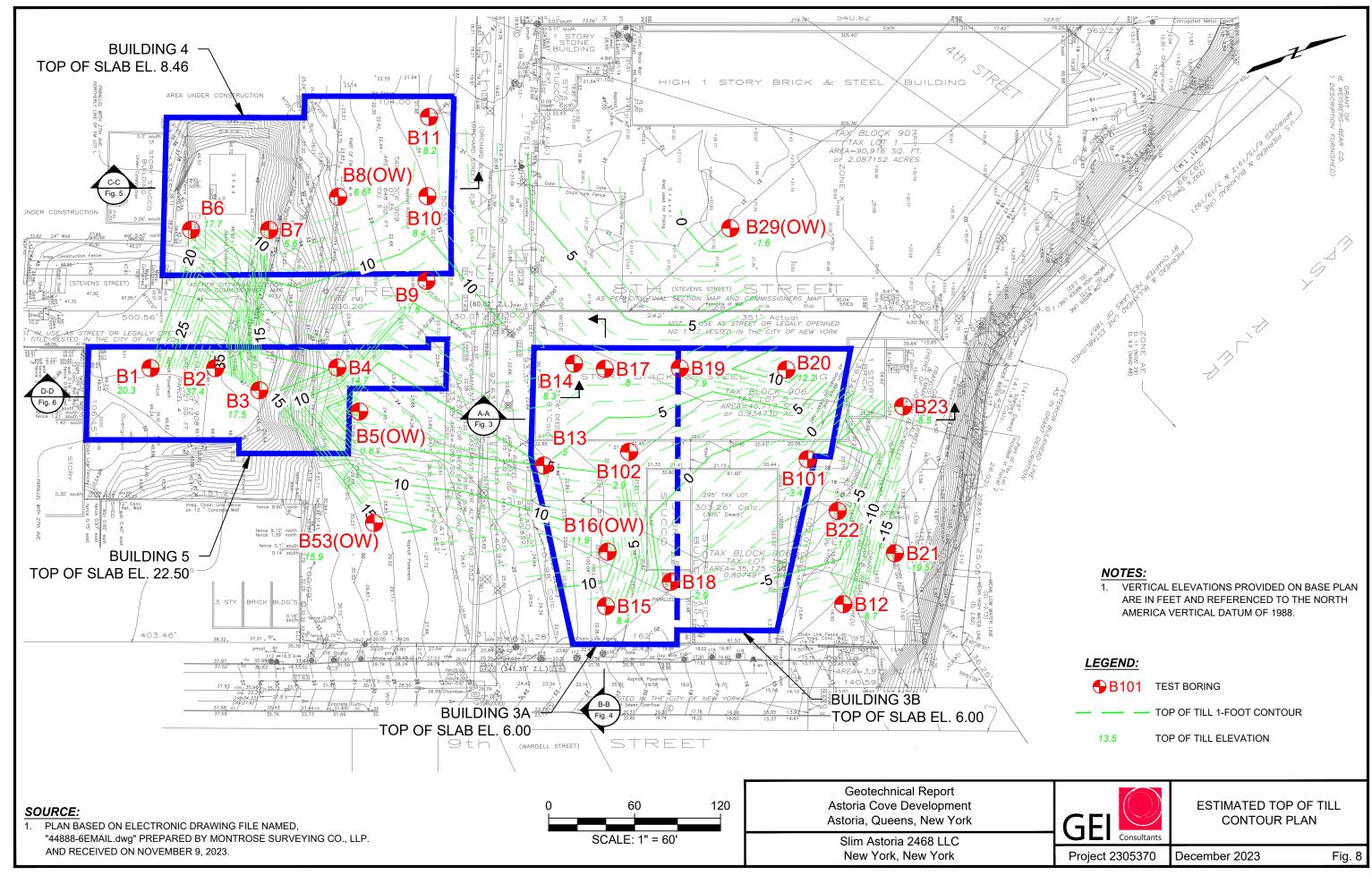


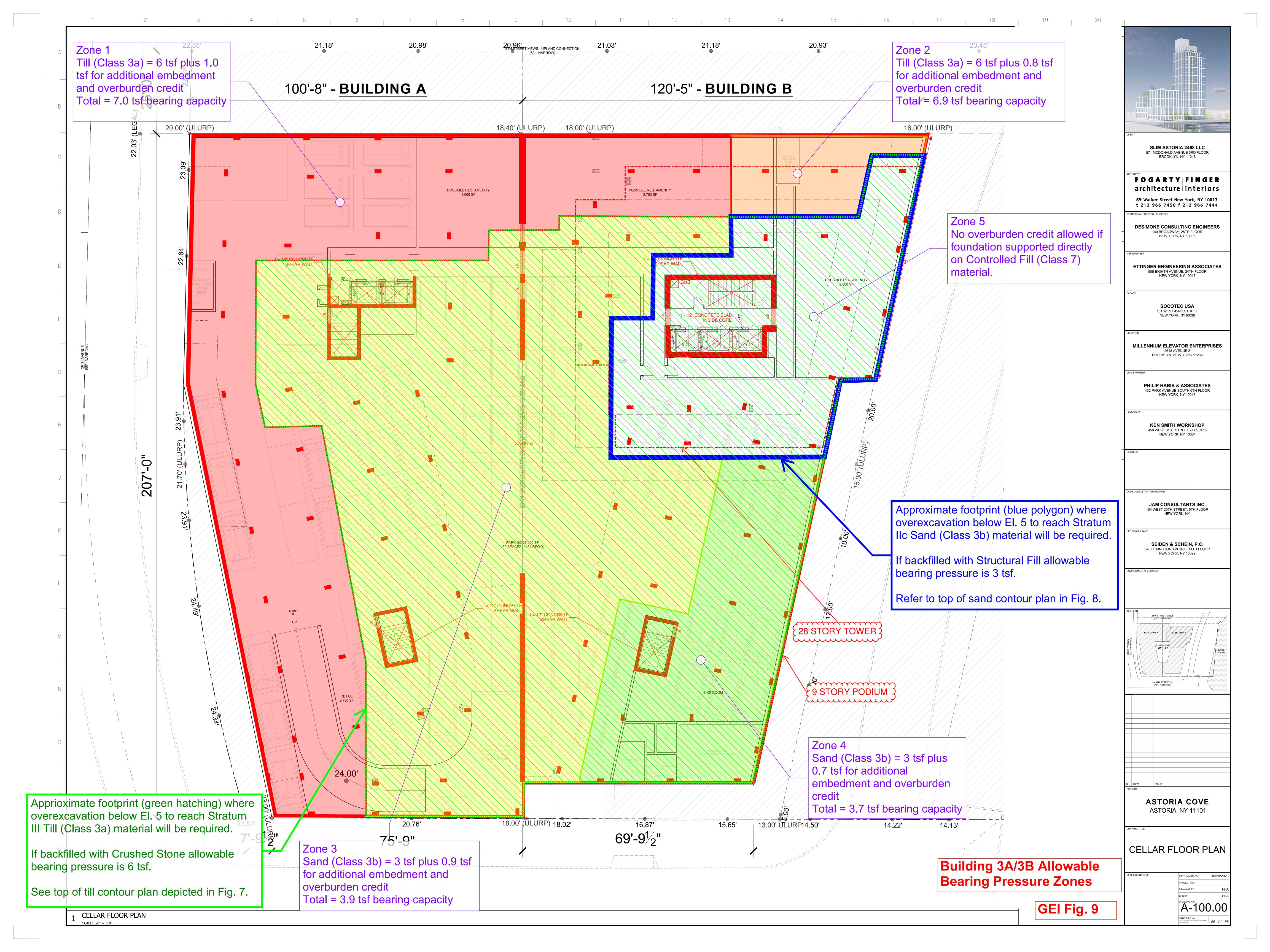


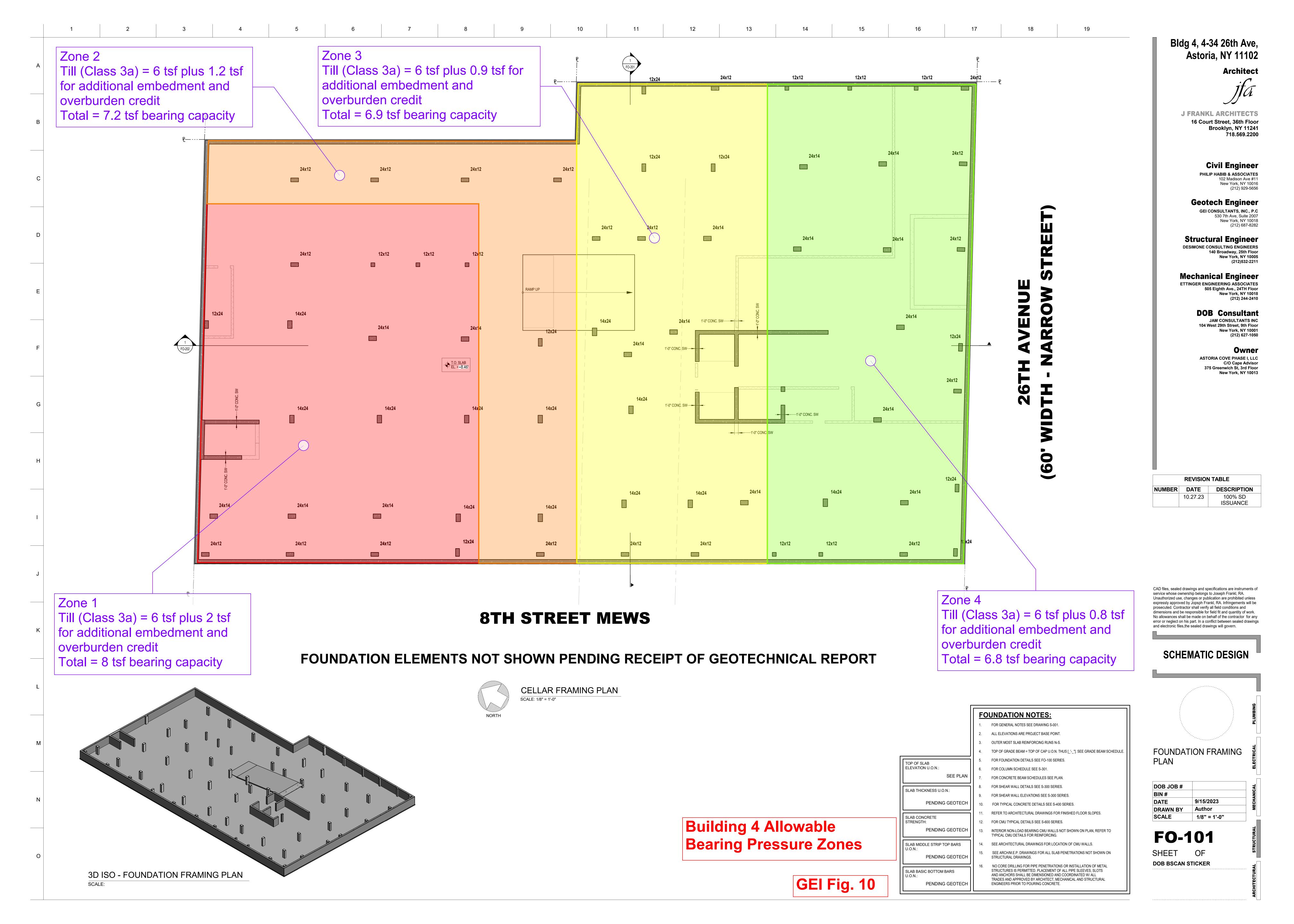


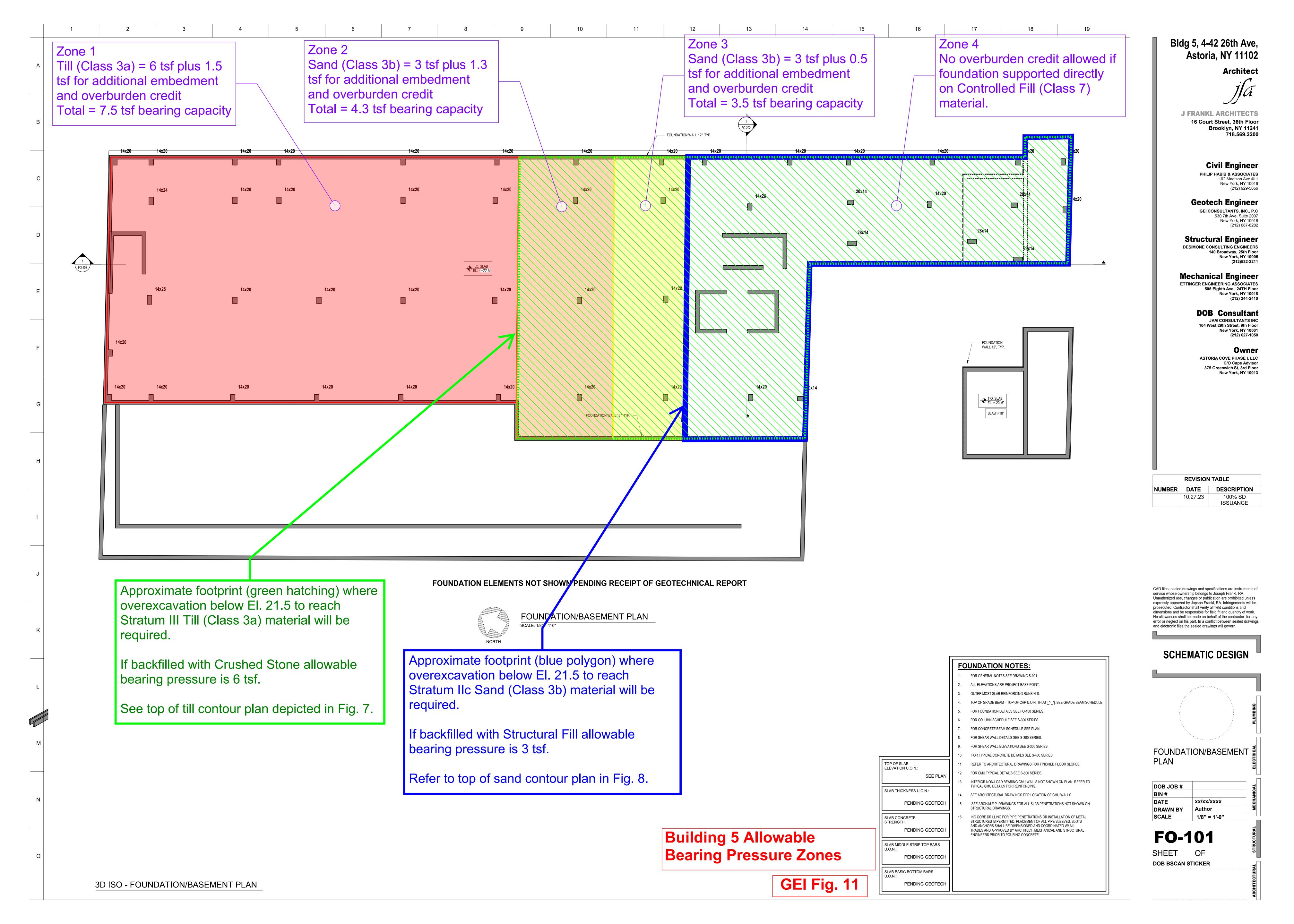


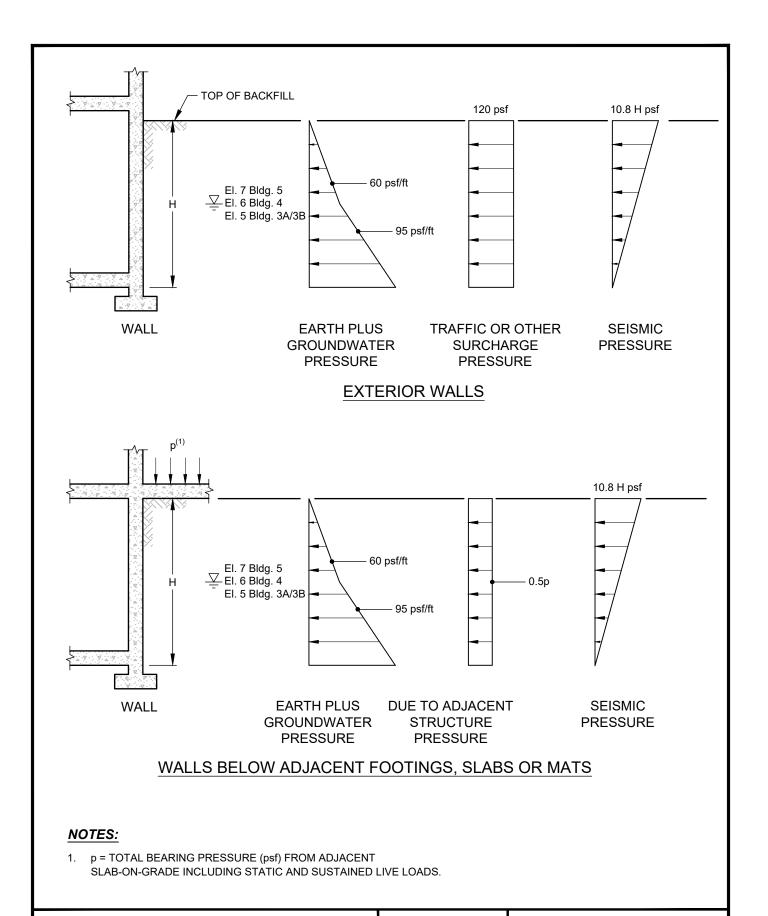












Geotechnical Report Astoria Cove Development Astoria, Queens, New York

Slim Astoria 2468 LLC New York, New York



RECOMMENDED LATERAL EARTH PRESSURES

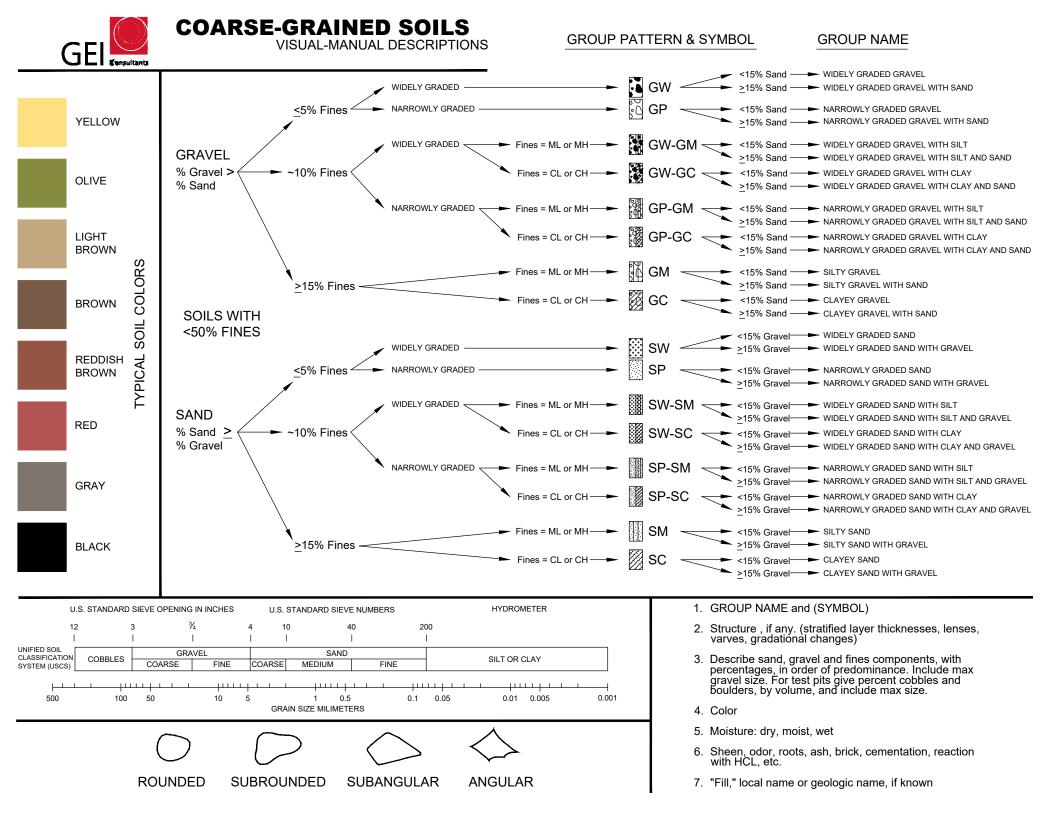
05370 December 2023

Fig. 12

Geotechnical Report
Astoria Cove Development
Buildings 3A/3B, 4 & 5
Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909
Lot 35
Astoria, Queens County, New York
December 2023

# Appendix A

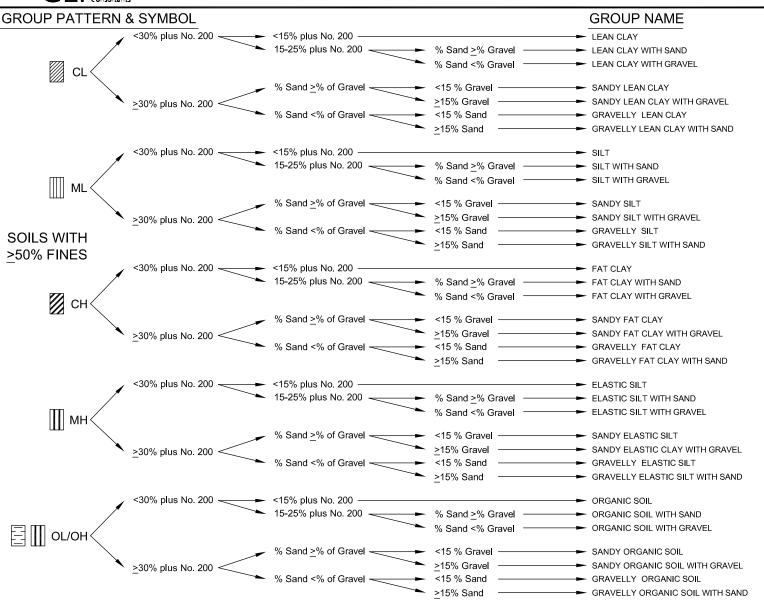
## **Exploration Logs**





## **FINE-GRAINED SOILS**

VISUAL-MANUAL DESCRIPTIONS



## ID OF INORGANIC FINE SOILS FROM MANUAL TESTS

Symbol	Name	Dry Strength	Dilatancy	Toughness*
ML	Silt	None to low	Slow to rapid	Low or thread cannot be formed
CL	Lean Clay	Medium to high	None to slow	Medium
МН	Elastic Silt	Low to medium	None to slow	Low to medium
СН	Fat Clay	High to very high	None	High

- 1. GROUP NAME and (SYMBOL)
- Describe fines, sand, and gravel components, in order of predominance. Include plasticity of fines. Include percentages of sand and gravel.
- 3. Color
- 4. Moisture: dry, moist, wet
- Sheen, odor, roots, ash, brick, cementation, torvane and penetrometer results, etc.
- 6. "Fill," local name or geologic name, if known



Peat refers to a sample composed primarily of vegetable matter in varying stages of decomposition. The description should begin: PEAT (PT) and need not include percentages of sand, gravel or fines.

#### CRITERIA FOR DESCRIBING PLASTICITY

Description	Criteria
Nonplastic ML	A 1/8-in. (3 -mm) thread cannot be rolled at any water content
Low Plasticity ML, MH	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit *
Medium Plasticity MH, CL	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit
High Plasticity CH	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit

\* Toughness refers to the strength of the thread near plastic limit. The lump refers to a lump of soil drier than the plastic, similar to dry strength.

NORT	HING (1	t): _	221,653 CE EL. (		3		EASTING (ft): 1,003,2 DATE START/END:		20 - 5/1/2020	BORING
			ATUMS: t): <u>102</u>		8/NAD83 N	IY Zone 310	DRILLING COMPANY: DRILLER NAME: Kei		B1	
LOGG	SED BY:	_	G. Holme	es			RIG TYPE: CME 75			PAGE 1 of 5
HAMM AUGE DRILL	MER TY ER I.D./C LING MI	PE: ).D.: ETH	Auton NA / OD: _M DEPTHS	natic NA lud Rotary	wash (		CASING I.D./O.D.: 4 DRILL ROD O.D.: NN			RREL TYPE: NA RREL I.D./O.D.: NA / NA
ABBR	REVIATIO	ONS	Rec. RQD WOI	= Length of R = Weight	/ Length lality Design f Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
			S	ample Inf	formation	1		go		
Elev. (ft)	Depti (ft)		Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description (CLASS 7) at ~0 ft / El. 48.8
	-	X	S1	0 to 2	24/7	23-16- 17-8	PID = 0.0ppm @S1		medium sand, ~20% nonplas 1"; brown; dry. Brick, asphali STRATUM IIA - LOOSE S	H GRAVEL (SM); ~60% fine to stic fines, ~20% angular gravel up to t, red brick and concrete fragments.
		$\bigvee$	S2	2 to 4	24/6	5-6-6-5	PID = 0.0ppm @S2		S2: WIDELY GRADED SANI subangular gravel up to 1.5";	
			S3	4 to 6	24/10	5-4-3-2	PID = 0.0ppm @S3		S3: WIDELY GRADED SANI	O (SW): Similar to S2.
.GDT 12/22/23	- - -		S4	6 to 8	24/12	5-3-4-8	PID = 0.0ppm @S4		S4: WIDELY GRADED SANI	,
2013	+		S5	8	24/0	5-7-6-6	PID = 0.0ppm @S5		STRATUM IIC - SAND	) (CLASS 3B) at ~8 ft / El. 40.8
TEMPLATE			35	to 10	24/0	5-7-0-0				
SPJ GEI DATA	- 10  -  -		S6	10 to 12	24/10	5-4-7-13	PID = 0.0ppm @S6		S6: WIDELY GRADED SANI	O (SW): Similar to S2.
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT 12/22/23  A		<u>/</u>	V				Drive casing to 14 feet below grade. Roller bit to 15 feet.	<i>'</i>		
ST-GRAPHIC LOG 23(	- 15 - - -		S7	15 to 17	24/0	10-6-6-5			S7: No Recovery. Rock stud	k in shoe.
3TD 6-NORTH-EAS	- - - -						Roller bit to 20 feet.			
NOTE	S:							PROJ	JECT NAME: Astoria Cove	
GEI WO									STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 48.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,263

**DATE START/END:** 5/1/2020 - 5/1/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B1** 

PAGE 2 of 5

	Sample Information				1		g			
	epth (ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and R	ock Description
-		$\bigvee$	S8	20 to 22	24/17	5-5-8-7	PID = 0.0ppm @S8		S8: NARROWLY GRADED SA sand, ~5% nonplastic fines; tar	ND (SP); ~95% fine to medium n-brown; dry.
- -							Advance casing to 25 feet below grade. Roller bit to 25 feet.			
-	25	V	S9	25 to 27	24/19	11-11- 14-12	PID = 0.0ppm @S9		S9: NARROWLY GRADED SA	ND (SP); Similar to S8.
1		/ \					Roller bit to 30 feet.		STRATUM III - TILL (CL	ASS 3A) at ~28.5 ft / El. 20.3
20 —	30	M	S10	30 to 32	24/13	9-15-18- 18	PID = 0.0ppm @S10		S10: SILTY SAND (SM); ~60% tan; dry.	ofine sand, ~40% nonplastic fines;
-							Roller bit to 35 feet.			
-	35	M	S11	35 to 37	24/19	22-22- 26-20	PID = 0.0ppm @S11		S11: WIDELY GRADED SAND fines, ~5% subrounded gravel	0 (SW); ~90% sand, ~5% nonplas up to 1.5"; brown; wet.
10 —							Roller bit to 40 feet. Intermittent rig chatter from 35 to 40 feet.			
<u></u>	40	X	S12	40 to 41.3	15/11	29-58- 102/3"	PID = 0.0ppm @S12		S12: WIDELY GRADED SAND	) (SW); Similar to S11. Trace mica
-							Roller bit to 45 feet. Rig chatter from 41 to 42 feet.			
+	45	X	S13	45 to 47	24/8	12-11- 14-14	PID = 0.0ppm @S13		S13: WIDELY GRADED SAND	0 (SW); Similar to S11.



GROUND SURFACE EL. (ft): 48.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

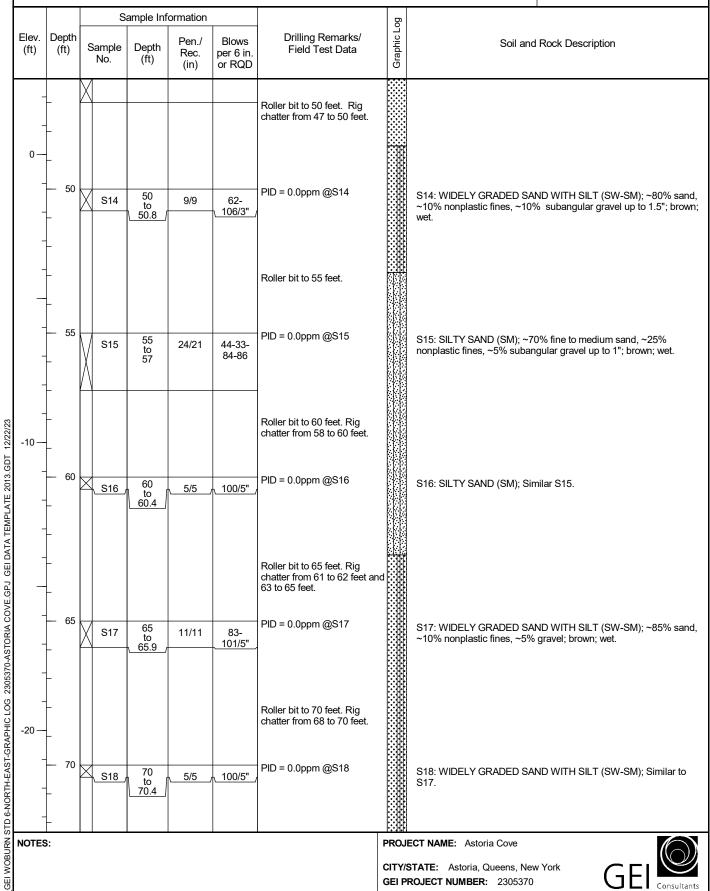
EASTING (ft): 1,003,263

5/1/2020 - 5/1/2020 DATE START/END:

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B1** 

PAGE 3 of 5





GROUND SURFACE EL. (ft): 48.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

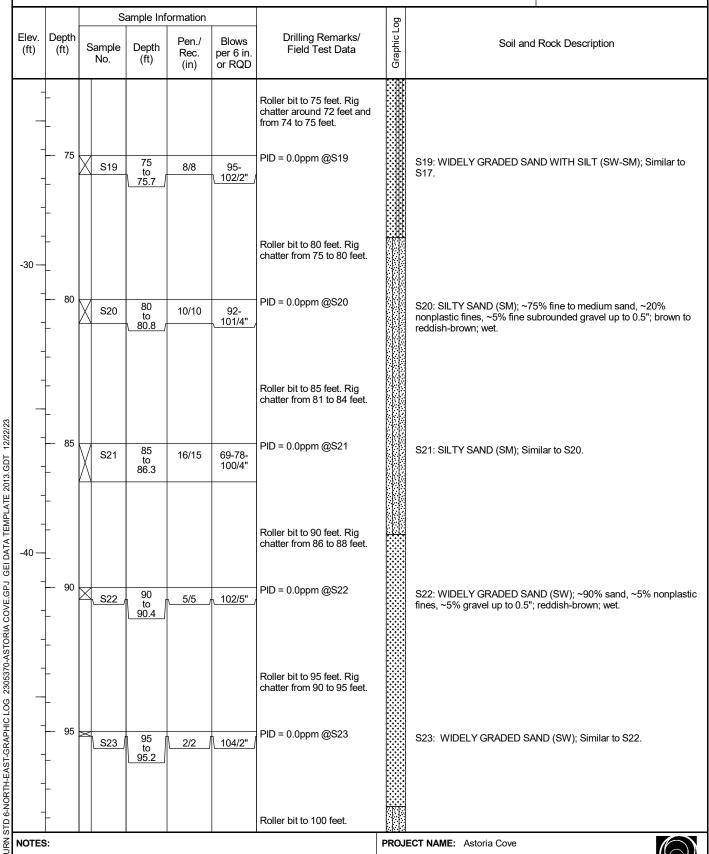
**EASTING (ft):** 1,003,263

**DATE START/END:** 5/1/2020 - 5/1/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B1

PAGE 4 of 5



NOTE:



NORTHING (ft): 221,653 **EASTING (ft):** 1,003,263 GROUND SURFACE EL. (ft): \_ 48.8

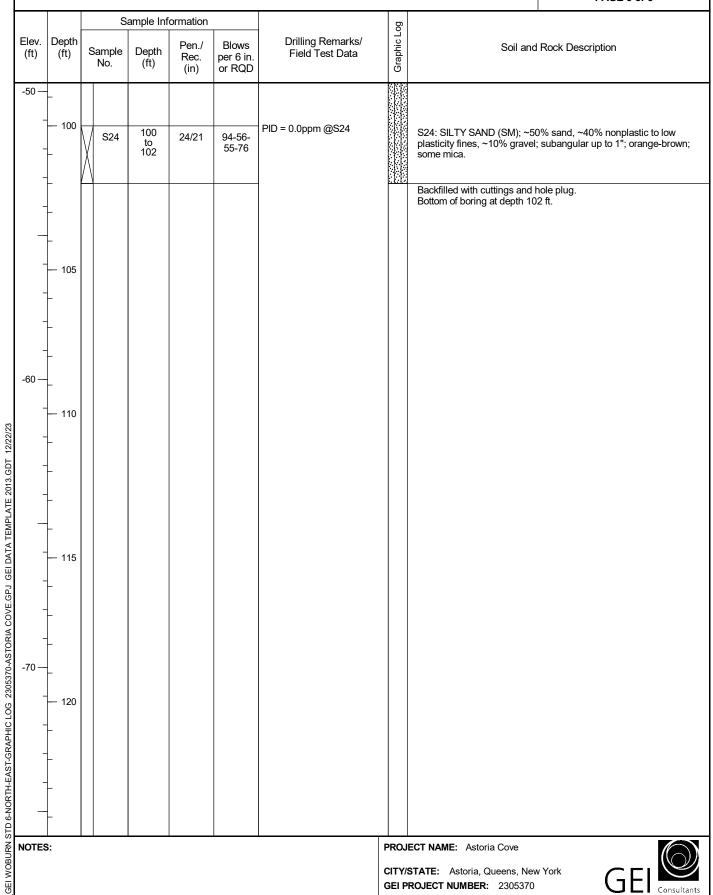
**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

**DATE START/END:** 5/1/2020 - 5/1/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B1** 

PAGE 5 of 5





	NORTH	HING ND S	(ft) URI	= _2 =AC		<b>t):</b> 48.4		N 7 04	_	EASTING (ft): _1,003,20	/28/20		BORING
	TOTAL	.DE	PTH	(ft)	T <b>UMS</b> : . :102 . Holme	.0	3/NAD83 N	Y Zone 310	<u>04</u> —	DRILLING COMPANY: DRILLER NAME: Kei RIG TYPE: CME 75			<b>B2</b> PAGE 1 of 5
	HAMM AUGER DRILLI	ER 1 R I.D NG I	TYPI ./O.I MET	≣: D.: 'HO	Autom NA / I D: M	natic NA lud Rotary	Wash			CASING I.D./O.D.: 4 i			RREL TYPE: NA RREL I.D./O.D.: NA / NA
	ABBR	EVIA	TIOI	NS:	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	.,%	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
	Elev. (ft)	De <sub>l</sub>			ample	Depth	Pen./ Rec.	Blows per 6 in.		Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
					No.	(ft)	(in)	or RQD			G		(CLASS 7) at ~0 ft / El. 48.4
	-	  -    -		M	S1	1 to 3	24/15	13-11-8- 10	PID	= 0.0ppm @S1		S1 (0-1'): Concrete.  S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% subar Brick, asphalt; trace mica. As	ngular gravel up to 1.5": brown: drv.
	_	_		M	S2	3 to 5	24/8	3-6-3-4	PID	= 0.0ppm @S2		S2: SILTY SAND (SM); Simil	ar to S1.
12/22/23	-	_	5	$\bigvee$	S3	5 to 7	24/7	3-3-4-12	PID	= 0.0ppm @S3		S3: SILTY SAND (SM); Simil	ar to S1. D (CLASS 3B) at ~7 ft / El. 41.4
EMPLATE 2013.GDT 12/22/23	40 —	_		X	S4	7 to 9	24/8	19-6-5-6	PID	= 0.0ppm @S4			0% sand, ~20% low plasticity fines,
DATA TEMPLA	-	_ _ 	10	M	S5	9 to 11	24/7	17-9-11- 34	PID	= 0.0ppm @S5		~20% subangular gravel up t dry; some mica. S5: WIDELY GRADED SANI	D WITH GRAVEL (SW); ~75% sand, to 1.25", ~5% nonplastic fines; brown;
COVE.GPJ GEI	-	_		M	S6	11 to 13	24/6	19-21- 24-12	Driv	= 0.0ppm @S6 e casing to 14 feet below le. Rollerbit to 15 feet.		dry; some mica.  STRATUM III - TILL ( S6: WIDELY GRADED GRA	o 1.25", ~5% nonplastic fines; brown;  CLASS 3A) at ~11 ft / El. 37.4  VEL WITH SAND (GW); ~75%  ~25% sad; brown to black; dry.
2305370-ASTORIA COVE.GPJ GEI DATA TI	_	_		' \									
	-	_	15	$\bigvee$	S7	15 to 17	24/9	6-5-8-7				S7: NARROWLY GRADED S nonplastic fines; tan; dry.	SAND (SP); ~95% fine sand, ~5%
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG	30 —	-							Roll	er bit to 20 feet.			
BURN S	NOTES	<b>S</b> :						ı			PROJ	JECT NAME: Astoria Cove	
GEI WOE												STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 48.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

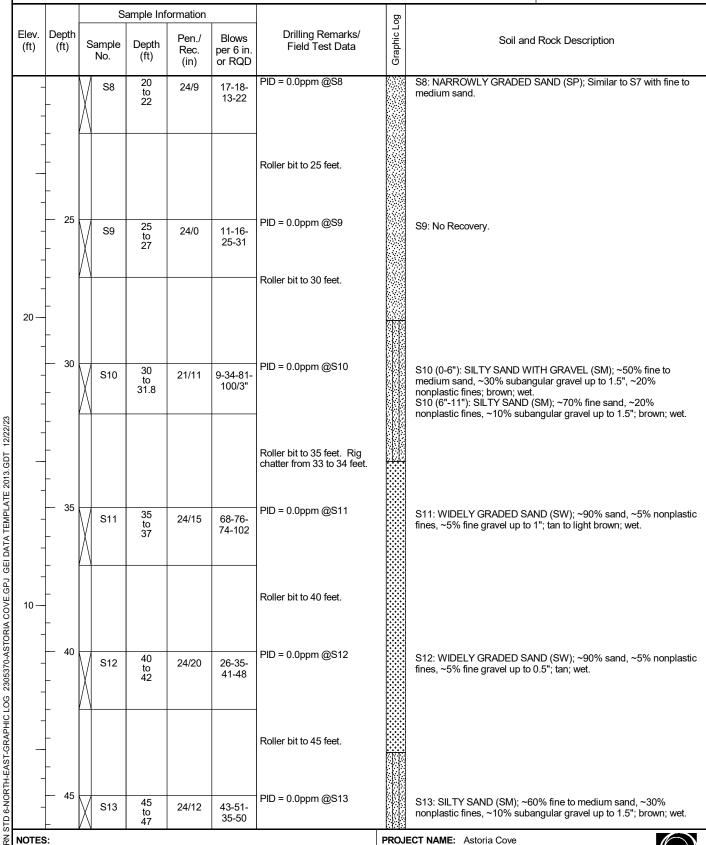
**EASTING (ft):** 1,003,280

**DATE START/END:** 4/28/2020 - 4/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B2** 

PAGE 2 of 5



GEI WOBURN

PROJECT NAME: Astoria Cove



NORTHING (ft): \_221,695 GROUND SURFACE EL. (ft): 48.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

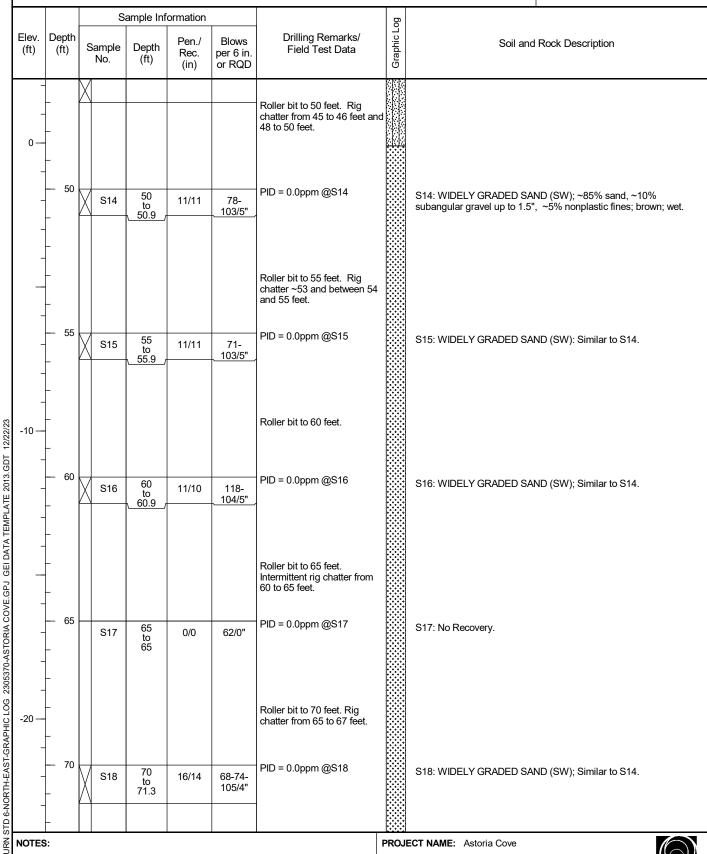
**EASTING (ft):** 1,003,280

**DATE START/END:** 4/28/2020 - 4/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B2

PAGE 3 of 5



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN S

NORTHING (ft): 221,695 GROUND SURFACE EL. (ft):

48.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

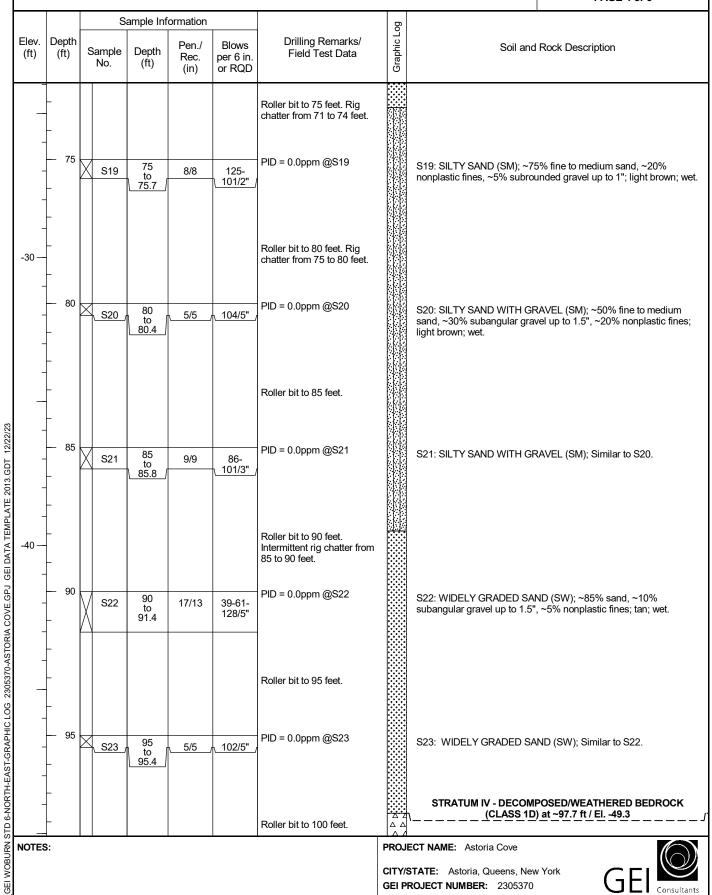
**EASTING (ft):** 1,003,280

4/28/2020 - 4/28/2020 DATE START/END:

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B2** 

PAGE 4 of 5



NORTHING (ft): 221,695 **EASTING (ft):** 1,003,280 GROUND SURFACE EL. (ft): \_ 48.4

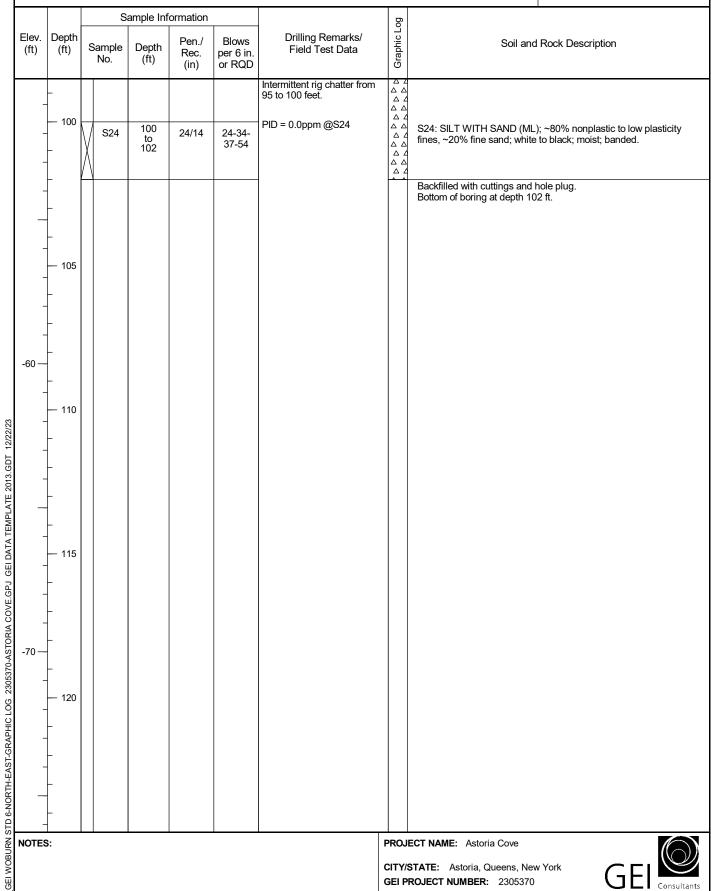
**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

DATE START/END: 4/28/2020 - 4/28/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B2** 

PAGE 5 of 5



_				MATIO	_						BORING
				221,718 SEEL #	ft): 46.0	<u> </u>		EASTING (ft): _1,003,3  DATE START/END: _4		020 4/27/2020	DOMING
				•	· —		IY Zone 31				B3
				: 102				DRILLER NAME: Kei			
LOGG	SED	BY:	_G	i. Holme	es			RIG TYPE: CME 75		_	PAGE 1 of 5
HAMI	IER R I.I	TYP D./O.	E: D.:	Autom NA /	natic	, Wash		CASING I.D./O.D.: 4			REL TYPE: NA REL I.D./O.D.: NA / NA
				EPTHS (		wasii					
ABBR	REVIA	ATIO	NS:	Rec. RQD WOF	= Length of R = Weight	/ Length lality Design f Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter
				S	ample Inf	formation			go		
Elev. (ft)		epth ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description (CLASS 7) at ~0 ft / El. 46
_			$\bigvee$	S1	0 to 2	24/17	20-9-8- 11	PID = 0.0ppm @S1		S1: SILTY SAND WITH GRA' nonplastic fines, ~20% suban Brick, asphalt, and concrete fr Slight petroleum-like odor.	gular gravel up to 1.5"; brown; dry.
	† †		M	S2	2 to 4	24/3	12-6-5-3	PID = 0.0ppm @S2		S2: SILTY SAND WITH GRA petroleum odor.	VEL (SM); Similar to S1. No
	+	5	$\bigvee$	S3	4 to 6	24/4	3-2-2-3	PID = 0.0ppm @S3		S3: SILTY SAND WITH GRA petroleum odor.	VEL (SM); Similar to S1. No
40 —	+		$\bigvee$	S4	6 to 8	24/8	2-1-1-4	PID = 0.0ppm @S4		S4: SILTY SAND WITH GRA petroleum odor.	VEL (SM); Similar to S1. No
	<u> </u>		$\bigvee$	S5	8 to 10	24/3	2-6-8-5	PID = 0.0ppm @S5		S5: SILTY SAND WITH GRA petroleum odor.	VEL (SM); Similar to S1. No
		10	$/ \setminus$					DID 000		STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 36
_	<u> </u>	10	M	S6	10 to 12	24/10	5-7-6-7	PID = 0.0ppm @S6		S6: SILTY SAND (SM); ~70% nonplastic fines, ~10% suban yellow-brown; dry; trace mica.	gular gravel up to 1"; brown to
30 –	<u></u>		/ \					Drive casing to 14 feet below grade. Rollerbit to 15 feet.	/ <u>                                    </u>		
30 —	<u></u>	15	M	S7	15 to 17	24/8	2-3-4-3			S7: NARROWLY GRADED S sand, ~5% nonplastic fines; b	AND (SP); ~95% fine to medium rown to tan; dry.
	  -  -							Roller bit to 20 feet.			
NOTE	S:				<u> </u>	1			PRO.	JECT NAME: Astoria Cove	
									CITY	STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEL Consultants



GROUND SURFACE EL. (ft): 46.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

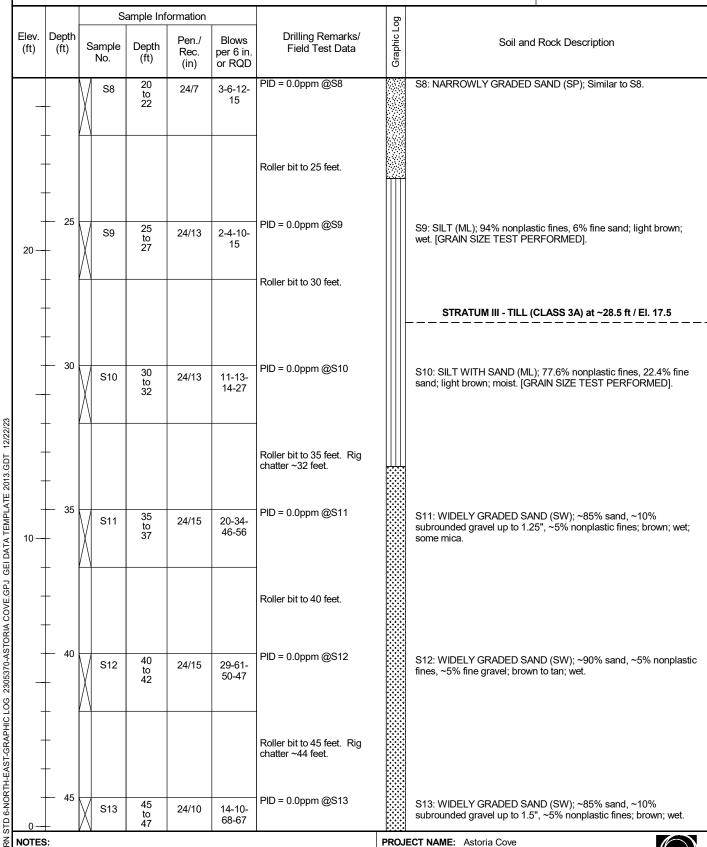
**EASTING (ft):** 1,003,305

**DATE START/END:** 4/27/2020 - 4/27/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B3

PAGE 2 of 5



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN 8

GROUND SURFACE EL. (ft): 46.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

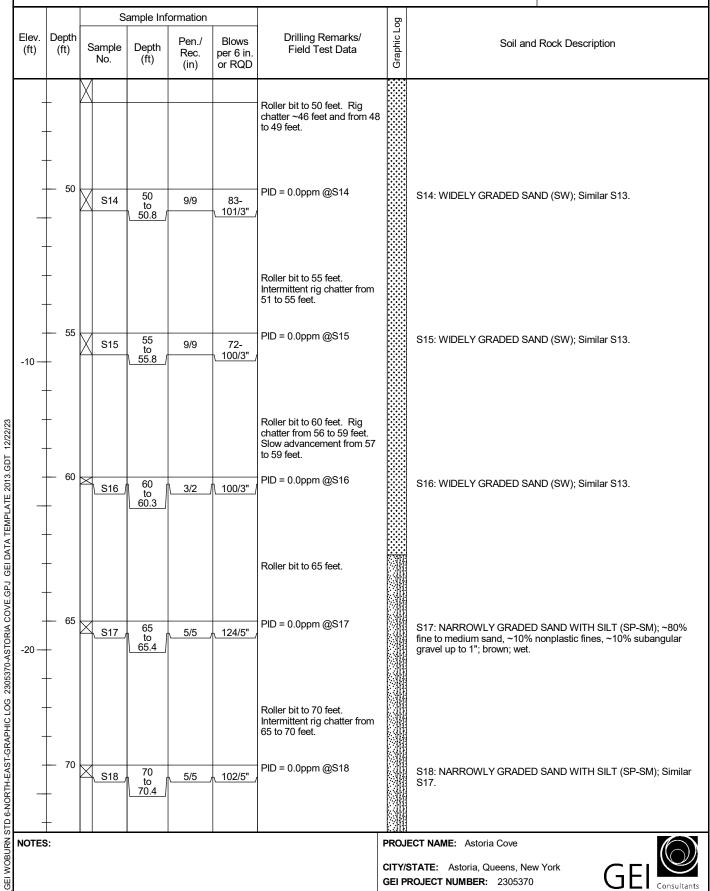
**EASTING (ft):** 1,003,305

DATE START/END: 4/27/2020 - 4/27/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B3** 

PAGE 3 of 5





GROUND SURFACE EL. (ft): 46.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

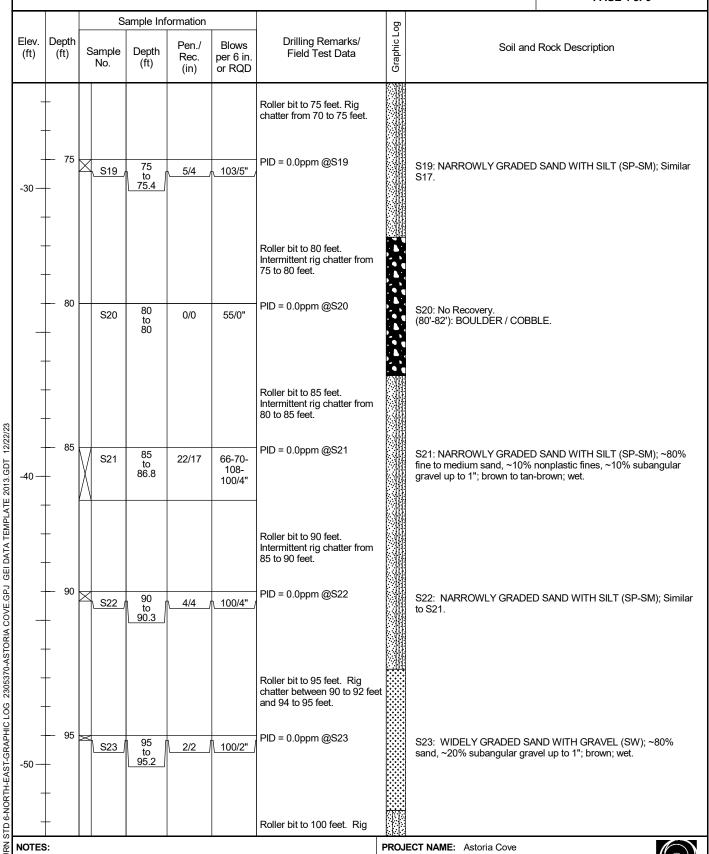
**EASTING (ft):** 1,003,305

**DATE START/END:** 4/27/2020 - 4/27/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B3

PAGE 4 of 5



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN

NORTHING (ft): 221,718 GROUND SURFACE EL. (ft): 46.0

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

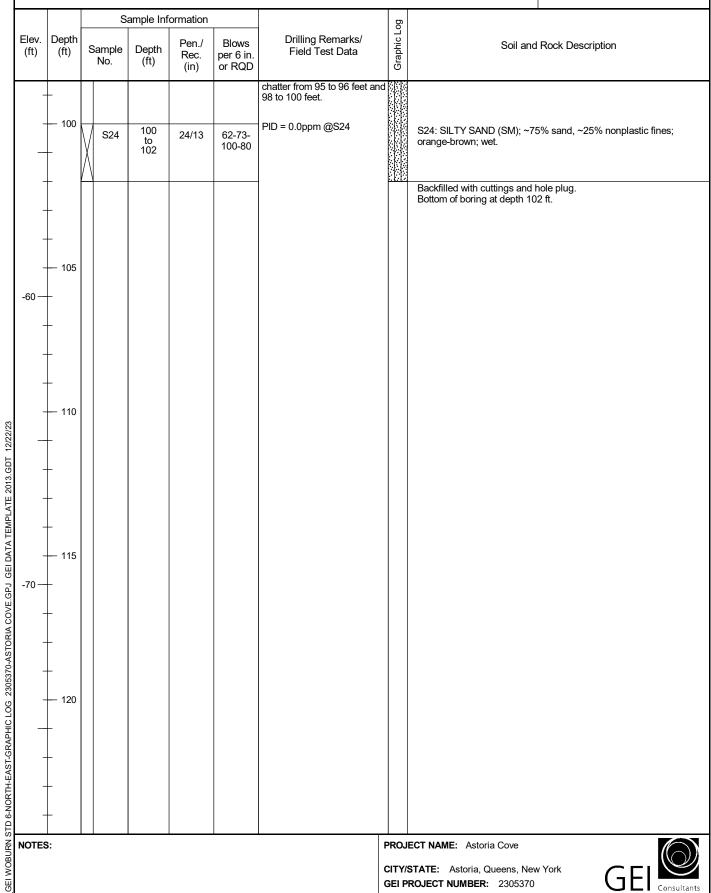
**EASTING (ft):** 1,003,305

**DATE START/END:** 4/27/2020 - 4/27/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B3** 

PAGE 5 of 5





	NORTH	IING	(ft):	221,775	_	7		EASTING (ft): _1,003,		20 - 5/4/2020	BORING
					• • —		IY Zone 310			g Geotechnical Drilling	<b>B</b> 4
				( <b>ft)</b> :10					eith Par	ent	
	LOGGI	ED BY	<b>/</b> : .	G. Holm	ies			RIG TYPE: CME 75			PAGE 1 of 5
				ORMAT : Auto				CASING I.D./O.D.:	1 inch/ 4	5 inch CORE BAI	RREL TYPE: NA
				: <u>NA</u>				DRILL ROD O.D.: N			RREL I.D./O.D.: NA / NA
				_	Mud Rotary	/ Wash					
	WATE	R LE\	ÆL.	DEPTHS	(ft):						
	ABBRE	EVIAT	ION	Red RQI WC	= Length o R = Weight	y Length uality Designa f Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auge		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside	Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
				5	Sample In	formation	_		DG C		
	Elev. (ft)	Dep		Sample	Depth	Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
	( )	, ,		No.	(ft)	(in)	or RQD		Gra	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 24.7
	-	_	\	S1	0 to 2	24/10	14-13-8- 8	PID = 0.0ppm @S1			SAND (SP); ~90% fine to medium ~5% coarse subangular gravel up to
	_	_	(	S2	2 to	24/16	5-4-5-6	PID = 0.0ppm @S2			D (CLASS 3B) at ~2 ft / El. 22.7 SAND (SP); ~95% fine sand, ~5%
	_		ľ		4					,,	
	20 —	_	5	S3	4 to 6	24/10	5-5-4-6	PID = 0.0ppm @S3		S3: NARROWLY GRADED	SAND (SP): Similar to S3.
GDT 12/22/23	_	- -	\	S4	6 to 8	24/8	4-3-5-5	PID = 0.0ppm @S4		S4: NARROWLY GRADED sand, ~10% nonplastic fines	SAND WITH SILT (SP-SM); ~90% fine ; light brown; dry.
EMPLATE 2013.GDT 12/22/23	-	- -	\	S5	8 to 10	24/15	5-3-6-6	PID = 0.0ppm @S5		S5: SILTY SAND (SM); ~80° light brown; dry.	% fine sand, ~20% nonplastic fines;
		L 1	٥					DID 00 COO		STRATUM III - TILL	(CLASS 3A) at ~10 ft / El. 14.7
J GEI DAT	-			S6	10 to 12	24/10	15-19- 28-63	PID = 0.0ppm @S6			D WITH SILT (SW-SM); ~80% sand, % subangular gravel up to 1.5"; white
ASTORIA COVE.GI	-	-	1	\				Drive casing to 14 feet belograde. Rollerbit to 15 feet.	w		
2305370-	10 —	_ 1	5		15	0.10				S7: WIDELY GRADED SAN	D WITH GRAVEL (SW); ~75% sand,
PHIC LOG	-	<u> </u>  -	Z	S7	to 15.8	9/9	24- 107/3"			~25% subangular gravel up	
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA T	-	_ _ _						Roller bit to 20 feet. Rig chatter from 15 to 18 feet.			
TD 6-1											
RN S	NOTES	: ::		1	1	I	I	1	PROJ	ECT NAME: Astoria Cove	
GEI WOBU									CITY	STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	v York GEI Consultants

GROUND SURFACE EL. (ft): 24.7

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

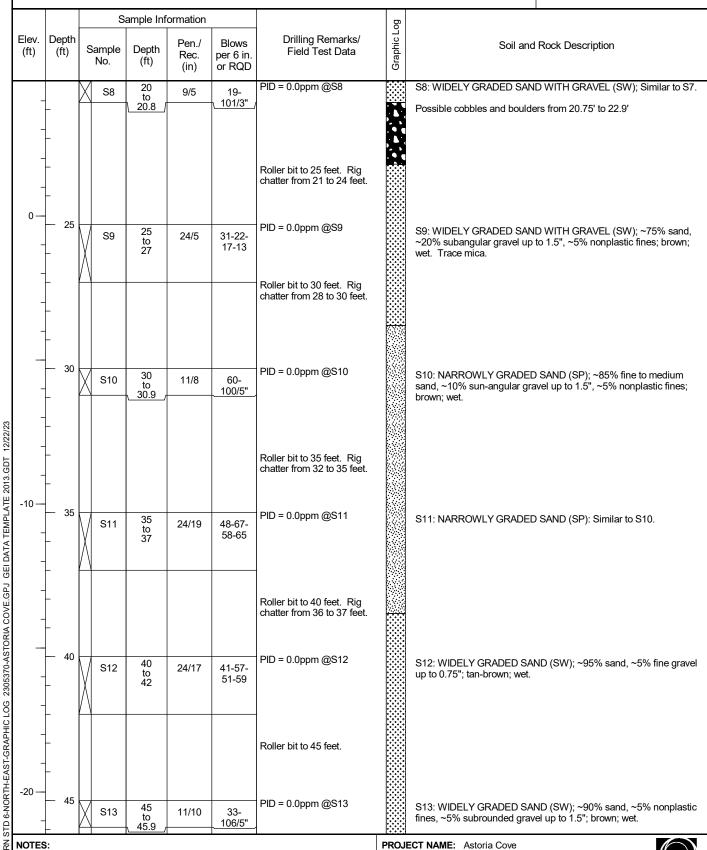
EASTING (ft): 1,003,310

**DATE START/END:** 5/4/2020 - 5/4/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B4** 

PAGE 2 of 5



WOBURN GEI



GROUND SURFACE EL. (ft): 24.7

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

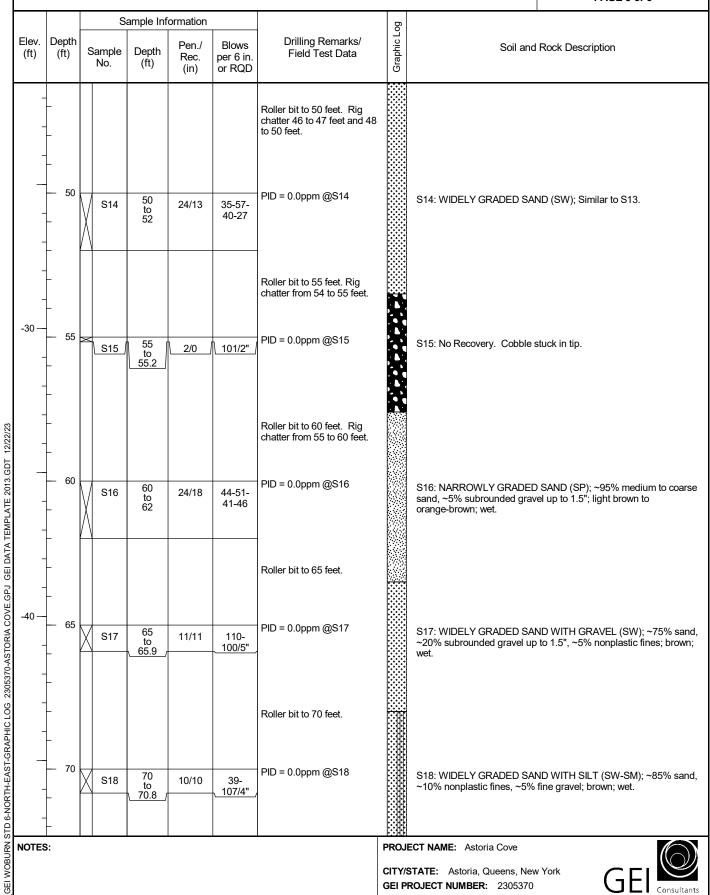
EASTING (ft): 1,003,310

**DATE START/END:** 5/4/2020 - 5/4/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B4** 

PAGE 3 of 5





NORTHING (ft): 221,775

GROUND SURFACE EL. (ft): 24.7

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

GEI

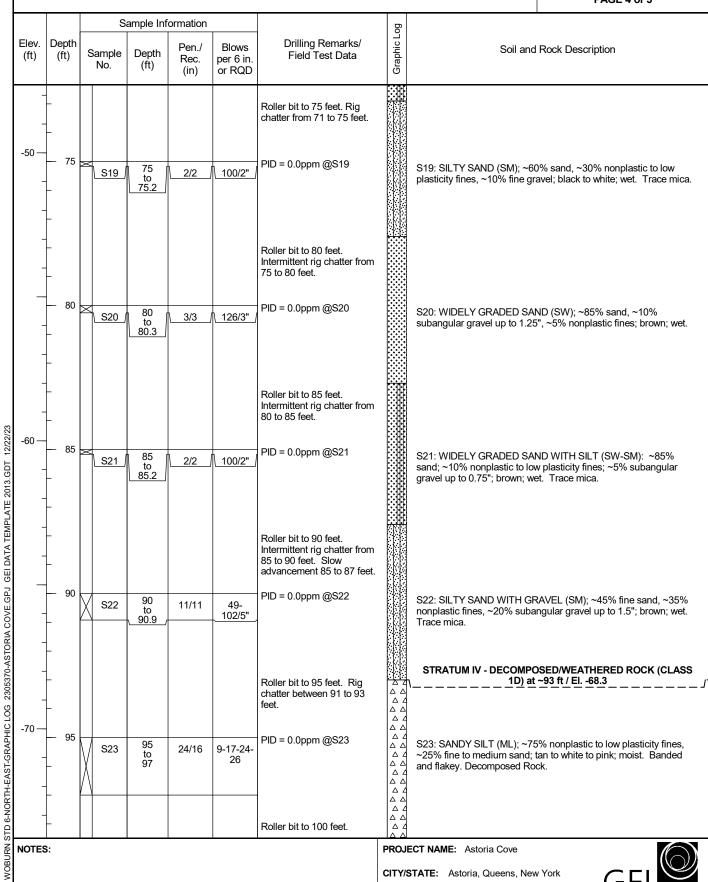
**EASTING (ft):** 1,003,310

**DATE START/END:** 5/4/2020 - 5/4/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B4

PAGE 4 of 5



GEI PROJECT NUMBER: 2305370

**NORTHING (ft):** 221,775 **EASTING (ft):** 1,003,310 GROUND SURFACE EL. (ft): 24.7

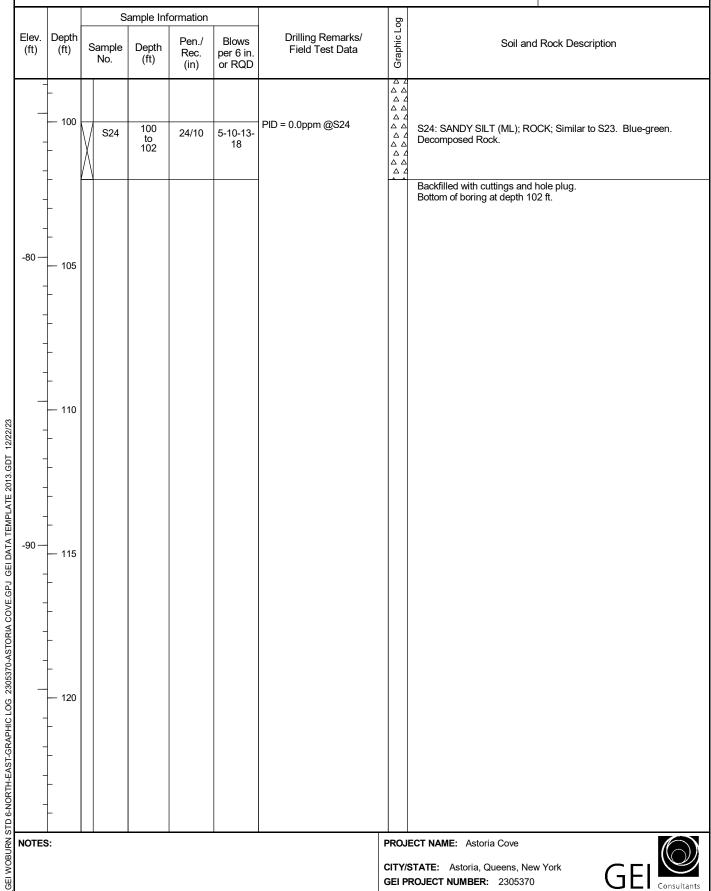
VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**DATE START/END:** 5/4/2020 - 5/4/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B4** 

PAGE 5 of 5



BORING INFORMATION **BORING** NORTHING (ft): 221,770 **EASTING (ft):** 1,003,343 GROUND SURFACE EL. (ft): **DATE START/END:** 3/9/2020 - 3/9/2020 24.8 **B5(OW)** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 102.0 DRILLER NAME: Joe Schuster LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 5 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NX Wireline AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: 2.125 inch / 3 inch DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): 

▼ 18.8 3/10/2020 7:00 am in observation well ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 24.8 PID = 0.0ppm @S1 S1 (0-6"): ASPHALT AND BASE MATERIAL. 0 S1 24/17 17-14to 2 10-11 S1 (6"-17"): SILTY SAND (SM); ~75% fine sand, ~25% nonplastic fines; tan; dry. PID = 0.0ppm @S2 S2: SILTY SAND (SM); ~70% low plasticity clay, ~30% fine to S2 24/17 6-5-9-11 to medium sand: brown: moist. STRATUM IIB - SILT/CLAY (CLASS 4B/5B) at ~4 ft / El. 20.8 PID = 0.0ppm @S3 S3: SILT WITH SAND (ML); ~60% nonplastic fines, ~40% fine S3 9-7-8-7 24/12 to sand; light brown; dry. 20 5 PID = 0.0ppm @S4 6 S4 (0-3"): SILT WITH SAND (ML); Similar to S3. S4 24/13 8-9-10-4 to 8 S4 (3"-13"): SILTY SAND (SM); ~60% fine sand, ~40% nonplastic fines; light brown; dry. PID = 0.0ppm @S5 S5 (0-8"): SILTY SAND (SM); Similar to S4. 8 S5 24/14 10-7-6-8 to 10 Drive casing to 8 feet below S5 (8"-14"): SILT WITH SAND (ML); ~60% nonplastic fines, ~40% grade. fine sand; light brown; dry. 10 PID = 0.0ppm @S6 S6: SILT WITH SAND (ML); 80.6% nonplastic fines; 19.4% fine sand; light brown; dry. [GRAIN SIZE TEST PERFORMED]. 10 24/14 4-4-6-8 S6 to 12 STRATUM IIC - SAND (CLASS 3B) at ~13.5 ft / El. 11.3 10 15 PID = 0.0ppm @S7 S7: NARROWLY GRADED SAND WITH GRAVEL (SP); ~75% S7 24/5 9-4-3-3 to 17 fine to medium sand; ~20% subangular gravel up to 1.5"; ~5% nonplastic fines; brown; moist. Trace mica.

NOTES:

LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT

STD 6-NORTH-EAST-GRAPHIC

R

WOBL

핑

PROJECT NAME: Astoria Cove



GROUND SURFACE EL. (ft): 24.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,343

**DATE START/END:** 3/9/2020 - 3/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B5(OW)** 

PAGE 2 of 5

s s s s s s s s s s s s s s s s s s s	Sample No. S8	Depth (ft)  20 to 22  22 to 24	Pen./ Rec. (in) 24/0	Blows per 6 in. or RQD 6-9-9-14	Drilling Remarks/ Field Test Data  PID = 0.0ppm @S8	Graphic Log	Soil and Rock Description  S8: No Recovery.	
	No. \$8	20 to 22 to	Rec. (in) 24/0	per 6 in. or RQD 6-9-9-14	Field Test Data	Graphic		
25	S9	22 to			PID = 0.0ppm @S8		S8: No Recovery.	
25	-	to	24/0	4-5-8-11				
25	S10						S9: No Recovery.	
25	S10				Rig chatter from 24 to 25 feet.		STRATUM III - TILL (CLASS 3A) at ~24 ft / E	il. 0.8 - — — — -
- I/ \I		25 to 27	24/14	15-11- 21-33	PID = 0.0ppm @S10		S10: NARROWLY GRADED SAND WITH GRAVEL (string to medium sand, ~10% subangular gravel up to 1 nonplastic fines; brown; wet.	
<u>/ \</u>					Rig chatter from 27 to 30 feet.			
30	S11	30 to 32	24/18	93-27- 48-88	PID = 0.0ppm @S11		S11: WIDELY GRADED SAND WITH GRAVEL (SW): ~20% subangular gravel up to 1.5"; ~5% low plasticity brown-black; wet. Some mica.	; ~75% sa / fines;
35		25			Intermittent rig chatter from 30 to 35 feet.		S12: NADDOMI V CDADED SAND WITH CDAVEL (	SD). ~75%
X	S12	55 to ∖_35.8_∫	10/10	86- 100/4" <i>,</i>			fine to medium sand, ~20% subangular gravel up to 1 plasticity fines; brown-black; wet. Some mica.	.5"; ~5% l
					and 38 to 40 feet.	1		
40	S13	40 to 42	24/19	23-35- 42-96	PID = 0.0ppm @S13		\$13: WIDELY GRADED SAND WITH SILT (SW-SM); ~10% nonplastic fines, ~10% fine subangular gravel ubrown; wet.	~80% sa .p to 0.5";
					Rig chatter from 40 to 41 fee and ~43 feet.	et in the second		
45	S14	45 to 47	24/20	31-50- 52-63	PID = 0.0ppm @S14		S14 (0-3"): NARROWLY GRADED SAND WITH GRA ~75% fine to medium sand, ~25% subangular gravel ~5% low plasticity fines; brown-black; wet.	VEL (SP); up to 1.5";
4(	55	S11  S12  S13	S11 30 to 32 35 to 35.8 35.8 35.8 35.8 35.8 35.8 35.8 35.8	S11 30 24/18 32 24/18 32 35 to 35.8 10/10 35.8 50 24/19 42 24/19	S11 30 24/18 93-27-48-88  S12 35 to 35.8 10/10 86-100/4"  S13 40 24/19 23-35-42-96	S11	S11   30   24/18   93-27- 48-88   Drive casing to 28 feet. Intermittent rig chatter from 30 to 35 feet.   PID = 0.0ppm @S12   Rig chatter from 36 to 37 feet and 38 to 40 feet.   Rig chatter from 40 to 41 feet and ~43 feet.   PID = 0.0ppm @S14   PROJECTIVES	S11   30   24/18   93-27-   48-88   24/18   93-27-   48-88   25/18   20/19   23-35-   42   24/19   23-35-   42   45   24/20   31-50-   59-63   57.5



GROUND SURFACE EL. (ft): 24.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,343

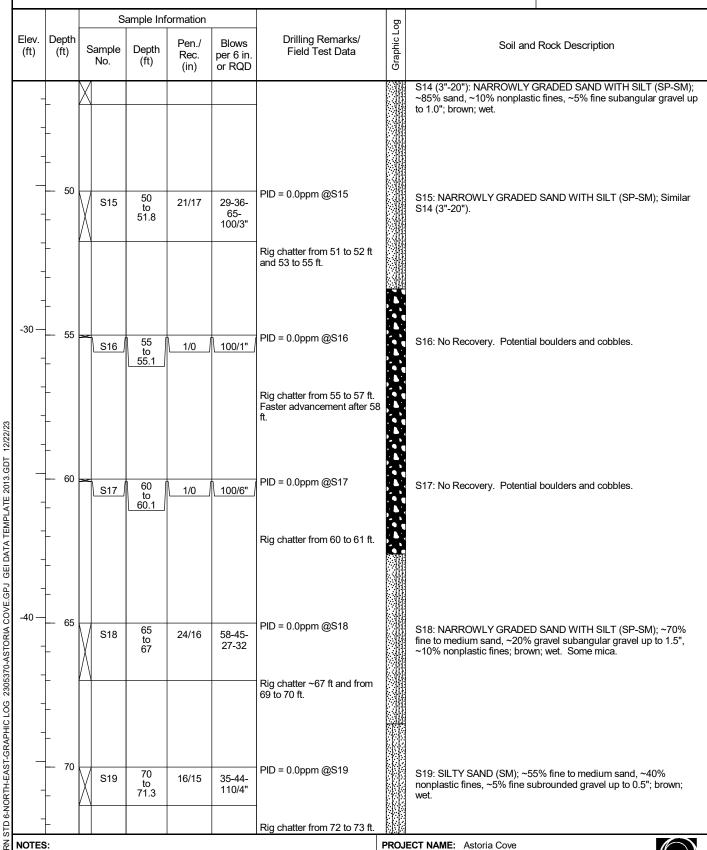
**DATE START/END:** 3/9/2020 - 3/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

BORING B5(OW)

PAGE 3 of 5



GEI WOBURN

GROUND SURFACE EL. (ft): 24.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

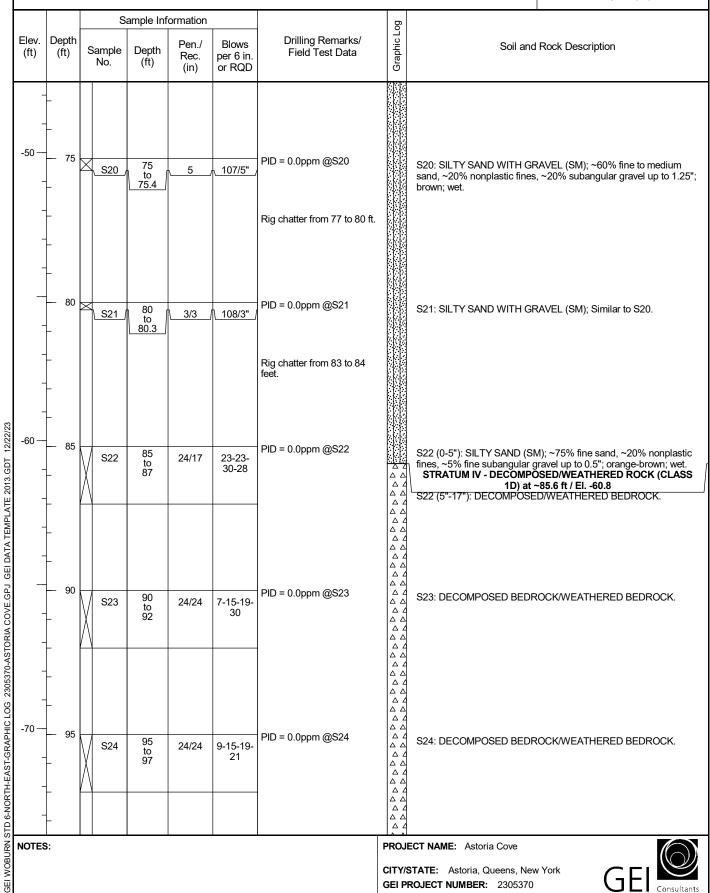
**EASTING (ft):** 1,003,343

**DATE START/END:** 3/9/2020 - 3/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B5(OW)** 

PAGE 4 of 5



GEI PROJECT NUMBER: 2305370

NORTHING (ft): 221,770 GROUND SURFACE EL. (ft):

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

24.8

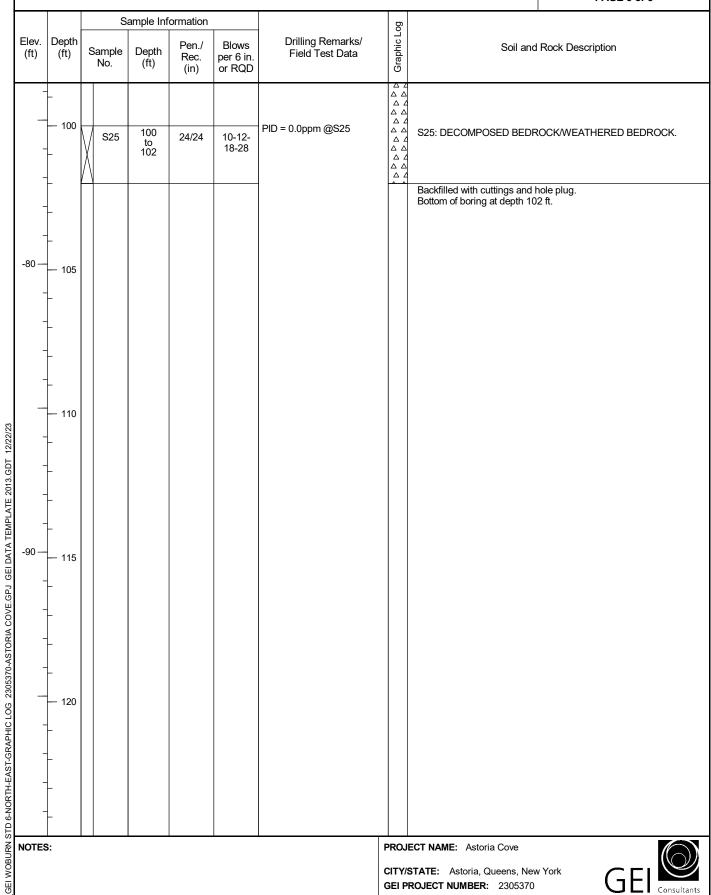
**EASTING (ft):** 1,003,343

**DATE START/END:** 3/9/2020 - 3/9/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B5(OW)** 

PAGE 5 of 5





	NORTH GROUN VERT.	HING ( ND SU (HORIZ DEP)	ft):  RF/ Z. D [H (	221,715 ACE EL. (' ATUMS: ft):100 G. Holme	ft): 46.2 NAVD 88		Y Zone 310	<u>)4</u>	EASTING (ft): _1,003,1 DATE START/END: _4 DRILLING COMPANY: DRILLER NAME: _Kei RIG TYPE: _CME 75	1/30/20 Crai	g Geotechnical Drilling	BORING  B6  PAGE 1 of 5
	HAMM AUGER DRILLI	ER TY R I.D./( NG M	PE: D.D. ETH	Auton : NA / HOD: M DEPTHS	natic NA lud Rotary	v Wash		_	CASING I.D./O.D.: 4 DRILL ROD O.D.: NN			REL TYPE: NA REL I.D./O.D.: NA / NA
	ABBR	EVIATI	ON	Rec. RQD WOF	= Length of R = Weight	/ Length ality Designa f Sound Core	ation es>4 in / Pen	.,%	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. itameter
	Elev. (ft)	Dept (ft)		Sample No.	ample Inf Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	-	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description
	-	-		S1	0 to 2	24/9	19-11-7- 10		= 1.3ppm @S1		S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% angula Brick, asphalt, and concrete fi S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% angula	ar gravel up to 1"; dark brown; dry. cagments. Slight petroleum-like odor. ofine to medium sand, ~20% ar gravel up to 1"; dark brown; dry.
	_	-		S2	2 to 4	24/9	5-6-3-1		= 0.9ppm @S2		Brick, asphalt, and concrete to S2: SILTY SAND (SM); Simil	ragments. Slight petroleum-like odor. ar to S1. No petroleum odor.
1/23	-	- ; -	5	S3	4 to 6	24/7	2-9-6-8		= 30.2ppm @S3		S3: SILTY SAND (SM); Simil	·
MPLATE 2013.GDT 12/22/23	40 —	-		S4	6 to 8	24/7	3-3-4-3		= 1.3ppm @S4		S4: SILTY SAND (SM); Simil	·
IA TEMPLATE 2	_	- - 10		S5	8 to 10	24/10	5-4-1-4		= 0.8ppm @S5			AND (CLASS 6) at ~10 ft / El. 36.2
E.GPJ GEI DAT	_	-		S6	10 to 12	24/12	4-3-2-6		= 2.3ppm @S6			AND WITH SILT (SP-SM); ~85% fine astic fines, ~5% subangular gravel
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TE	-	- - - 1:	5						re casing to 14 feet below de. Rollerbit to 15 feet.	<i>,</i>		
-GRAPHIC LOG 2	30 —	-		S7	15 to 17	24/0	6-8-6-7	-				to sample with 3" spoon; no recovery.  (CLASS 3B) at ~16 ft / El. 30.2
STD 6-NORTH-EAST	-	-						Roll	er bit to 20 feet.			
GEI WOBURN &	NOTES	S:								CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 46.2

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,183

**DATE START/END:** 4/30/2020 - 4/30/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B6** 

PAGE 2 of 5

								Т	
			Sa	ample Inf	ormation			Log	
	epth (ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description
-	<u>,</u>	$\bigvee$	S8	20 to 22	24/6	11-4-11- 10	PID = 0.0ppm @S8		S8: SILTY SAND WITH GRAVEL (SM); ~60% fine to medium sand, ~20% nonplastic fines, ~20% subangular gravel up to 1.5" brown to orange-brown; dry.
							Roller bit to 25 feet.		
20 —	25		S9	25 to 27	24/2	10-8-7-9	PID = 0.0ppm @S9		S9: WIDELY GRADED SAND WITH GRAVEL (SW); ~70% sand ~30% subrounded gravel up to 1.5"; brown; dry.
-	<u>/</u>						Drive casing to 25 feet below grade. Roller bit to 30 feet.		STRATUM III - TILL (CLASS 3A) at ~28.5 ft / El. 17.7
-	30 -	$\sqrt{}$	S10	30 to 32	24/13	10-11- 12-14	PID = 0.0ppm @S10		S10: NARROWLY GRADED SAND (SP): ~95% fine to medium sand, ~5% nonplastic fines; tan; dry.
-	-						Roller bit to 35 feet. Rig chatter from 32 to 35 feet.		
10 —	35		S11	35 to 37	24/24	11-14- 14-12	PID = 0.0ppm @S11		S11: NARROWLY GRADED SAND (SP); Similar to S10.
							Roller bit to 40 feet. Rig chatter from 36 to 37 feet.		
-	40		S12	40 to 42	24/15	9-12-15- 17	PID = 0.0ppm @S12		S12: SILT WITH SAND (ML); 82.9% nonplastic fines, 17.1% salight brown; dry. [GRAIN SIZE TEST PERFORMED].
1							Roller bit to 45 feet.		
	45	X	S13	45 to 47	24/18	15-19- 32-40	PID = 0.0ppm @S13		S13: SILTY SAND (SM); ~60% sand, ~30% nonplastic fines, ~10% subrounded gravel up to 1"; brown; wet.



GROUND SURFACE EL. (ft): 46.2

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

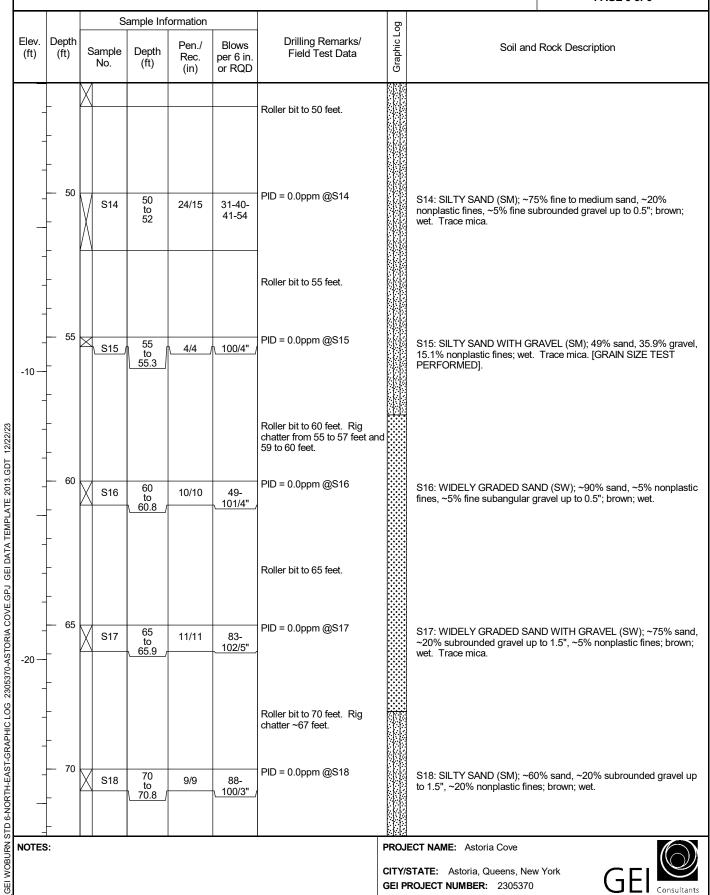
**EASTING (ft):** 1,003,183

DATE START/END: 4/30/2020 - 4/30/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B6** 

PAGE 3 of 5





NORTHING (ft): 221,715 GROUND SURFACE EL. (ft): 46.2

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

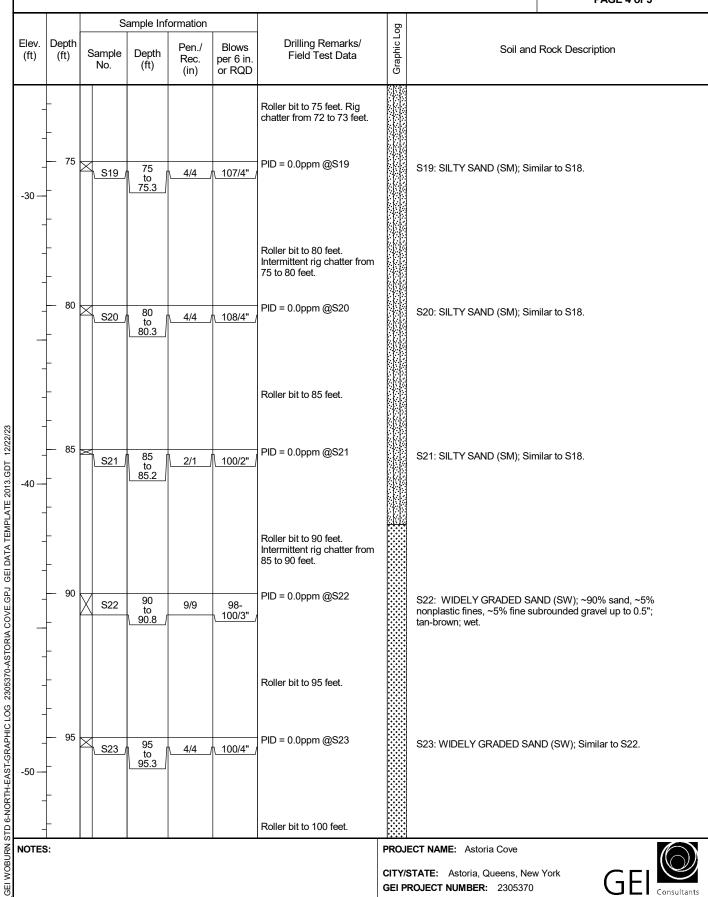
**EASTING (ft):** 1,003,183

DATE START/END: 4/30/2020 - 4/30/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B6** 

PAGE 4 of 5





**EASTING (ft):** 1,003,183 NORTHING (ft): 221,715 GROUND SURFACE EL. (ft): 46.2

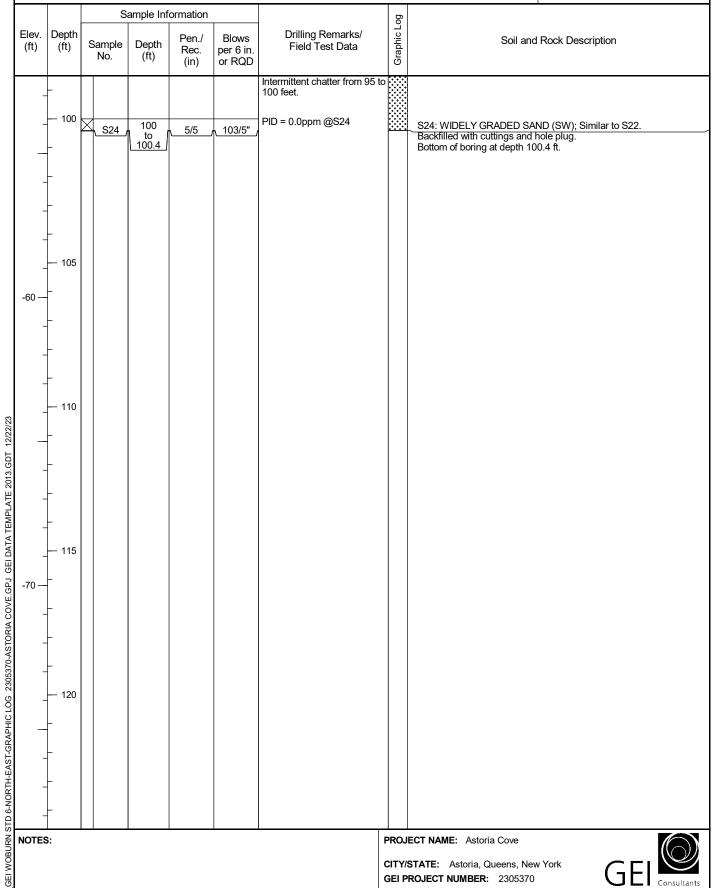
**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

**DATE START/END:** 4/30/2020 - 4/30/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B6** 

PAGE 5 of 5





	NORTH	IINC	(ft)	: _2	MATION 221,766 EE EL. (f	_	1			EASTING (ft): 1,003,20 DATE START/END: 4		020 4/20/2020	BORING
					•	· —		Y Zone 310	04	DRILLING COMPANY:			B7
				٠,	: 102					DRILLER NAME: Kei	th Par	ent	
	LOGGI	ED E	3Y:	_G	. Holme	es			_	RIG TYPE: CME 75			PAGE 1 of 5
	HAMM	ER R I.D	TYP )./O.	E: D.:	Autom NA / I	natic NA	.\A/ l-		_	CASING I.D./O.D.: 4 i			REL TYPE: NA REL I.D./O.D.: NA / NA
					PTHS (	ud Rotary ft):	wasn						_
						<u> </u>							
	ABBRE	EVIA	ПО	NS:	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	.,%	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
					Sa	ample Inf	ormation				go		
	Elev. (ft)		pth t)	ı	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD		Drilling Remarks/ Field Test Data	Graphic Log		Rock Description (CLASS 7) at ~0 ft / El. 40.1
	-	_		M	S1	0 to 2	24/13	38-25- 20-12	PID	= 0.0ppm @S1		S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% angulary. Brick, asphalt, wood, and petroleum-like odor.	ar gravel up to 1.25"; gray to black;
	-	_		M	S2	2 to 4	24/6	10-7-8- 10	PID	= 0.0ppm @S2			SAND WITH SILT (SP-SM); ~85% fine ingular gravel up to 1.5", ~10% dry. Concrete fragments.
3	_	_	5	M	S3	4 to 6	24/10	10-10- 17-12	PID	= 0.0ppm @S3		S3: NARROWLY GRADED S S2.	SAND WITH SILT (SP-SM); Similar to
MPLATE 2013.GDT 12/22/23	-	_		M	S4	6 to 8	24/8	16-12-6- 8	PID	= 0.0ppm @S4		up to 1.5", 34.7% sand, 17.6	AND (GM); 47.7% subangular gravel % nonplastic fines; dark brown; dry. nd concrete fragments. [GRAIN SIZE
TEMPLATE 201	-	_		M	S5	8 to 10	24/9	22-13-9- 8	PID	= 0.0ppm @S5	<u>.VIC</u>		SAND WITH SILT AND GRAVEL im sand, ~20% subangular gravel up s; dark brown; dry. Concrete
SPJ GEI DATA	30 —	_	10	M	S6	10 to 12	24/13	16-10-8- 29	PID	= 0.0ppm @S6			subangular gravel up to 1.5", ~10% dry. Wood fragments and black
2305370-ASTORIA COVE.GPJ GEI DATA TE	-	_		<u> </u>						e casing to 14 feet below le. Rollerbit to 15 feet.			
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 230	-	-	15	M	S7	15 to 17	24/13	8-7-70- 17	-				0% fine to medium sand, ~20% low subangular up to 1"; dark brown; dry.
-EAS	_	Ļ							Roll	er bit to 20 feet.		STRATUM IIC - SAND (	CLASS 3B) at ~18.5 ft / El. 21.6
TD 6-NORTH	_	_							. (011		<i>[///</i>	CHARION NO - SAND (	02.00 00) at ≈ 10.0 117 El. 21.0
JRN S	NOTES	S:									PROJ	ECT NAME: Astoria Cove	
GEI WOBL												STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 40.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,203

**DATE START/END:** 4/29/2020 - 4/29/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B7** 

PAGE 2 of 5

	Sample Information				1		g g		
Elev. (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description
-	-	$\bigvee$	S8	20 to 22	24/10	8-6-5-6	PID = 0.0ppm @S8		S8: WIDELY GRADED SAND (SW); ~90% sand, ~5% nonplifines, ~5% subangular gravel up to 1.5"; brown; dry.
-	-						Roller bit to 25 feet.		
-	— 25 -	V	S9	25 to 27	24/10	6-6-9-16	PID = 0.0ppm @S9		S9: SILTY SAND (SM); ~60% fine sand, ~40% nonplastic fine brown; dry.
_	_	/\					Roller bit to 30 feet.		
10 —	- 30 -		S10	30 to 32	24/11	6-6-6-6	PID = 0.0ppm @S10		S10: NARROWLY GRADED SAND (SP); ~95% fine to mediu sand, ~5% nonplastic fines; tan; dry.
	- -						Roller bit to 35 feet.		STRATUM III - TILL (CLASS 3A) at ~33.5 ft / El. 6.6
	— 35 –	M	S11	35 to 37	24/12	13-17- 15-17	PID = 0.0ppm @S11		S11: SILTY SAND (SM); ~75% fine to medium sand, ~25% nonplastic fines; tan; dry.
	-						Roller bit to 40 feet.		
0	— 40 –	M	S12	40 to 42	24/16	17-20- 18-19	PID = 0.0ppm @S12		S12: SILTY SAND WITH GRAVEL (SM); 56.7% sand, 23.2% nonplastic fines, 20.1% subangular gravel; brown; dry. [GRAII SIZE TEST PERFORMED].
	-						Roller bit to 45 feet. Rig chatter from 44 to 45 feet.		
+	<b>—</b> 45	M	S13	45 to 45.9	11/5	40- 101/5"	PID = 0.0ppm @S13		S13: SILTY SAND WITH GRAVEL (SM); Similar to S12.

GROUND SURFACE EL. (ft): 40.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,203

**DATE START/END:** 4/29/2020 - 4/29/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B7** 

PAGE 3 of 5

			S	ample Inf	ormation			go		
Elev. (ft)	Depth (ft)	1 5	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
-	<u>-</u>						Roller bit to 50 feet. Rig chatter from 46 to 48 feet.			
-10 — -	50 	, \	S14	50 to 52	24/9	14-11- 19-13	PID = 0.0ppm @S14		S14: WIDELY GRADED SAN ~25% subrounded gravel up to reddish-brown; wet.	ND WITH GRAVEL (SW); ~70% sand to 1.5", ~5% nonplastic fines; brown
-	 55						Roller bit to 55 feet. Rig chatter from 51 to 52 feet an 53 to 55 feet.	d <del></del>		
-	_	' ×	<u>S15</u> ∫	55 to 55.3	3/3	100/3"	PID = 0.0ppm @S15		S15: SILTY SAND WITH GR subangular gravel up to 1", 1 [GRAIN SIZE TEST PERFOR	AVEL (SM); 51.8% sand, 28.9% 9.3% nonplastic fines; brown; wet. RMED].
-	<u>-</u>						Roller bit to 60 feet. Rig chatter from 58 to 60 feet.			
-20 — -	.— 60 .–	)	S16	60 to 60	0/0	57/0"	PID = 0.0ppm @S16		S16: NO RECOVERY. No sp	poon penetration.
-	- -						Roller bit to 65 feet. Rig chatter from 60 to 62 feet.			
-	.— 65  		S17	65 to 66.3	16/13	71-93- 100/4"	PID = 0.0ppm @S17		S17: WIDELY GRADED SAN ~10% nonplastic fines, ~5% brown; wet.	ND WITH SILT (SW-SM); ~85% sand fine subrounded gravel up to 0.5";
-							Roller bit to 70 feet. Intermittent rig chatter from 65 to 70 feet.			
-30 — -	- 70 - -		S18	70 to 72	24/19	46-44- 94-106	PID = 0.0ppm @S18		S18: WIDELY GRADED SAN S17.	ND WITH SILT (SW-SM); Similar to
NOTES	<b>S</b> :							CITY/S	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New  ROJECT NUMBER: 2305370	YYork GEL CONSULTAN

NORTHING (ft): 221,766 GROUND SURFACE EL. (ft):

40.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

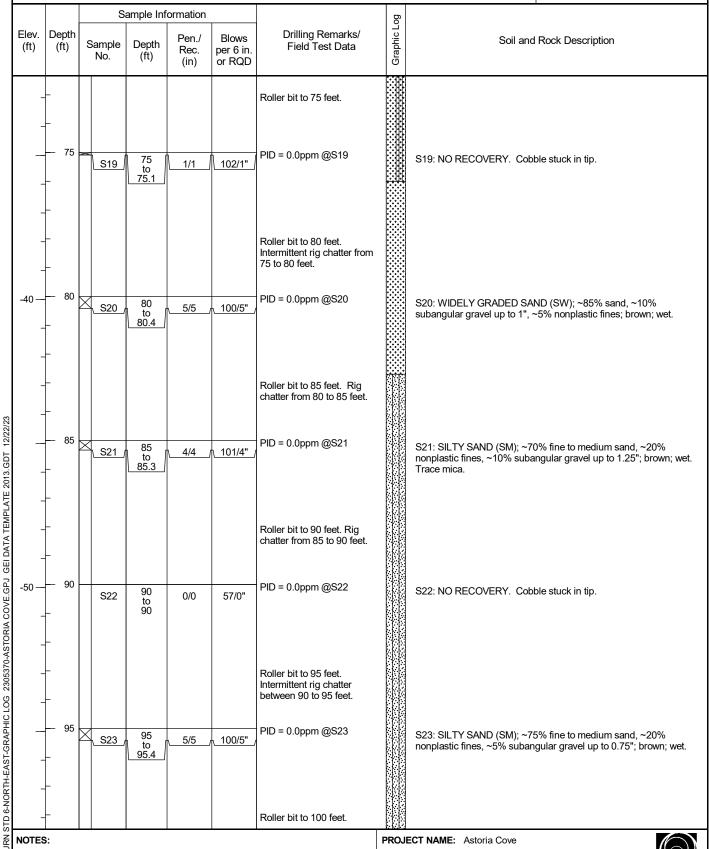
**EASTING (ft):** 1,003,203

**DATE START/END:** 4/29/2020 - 4/29/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B7** 

PAGE 4 of 5



CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370

GEI WOBURN

NORTHING (ft): 221,766 GROUND SURFACE EL. (ft): 40.1

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

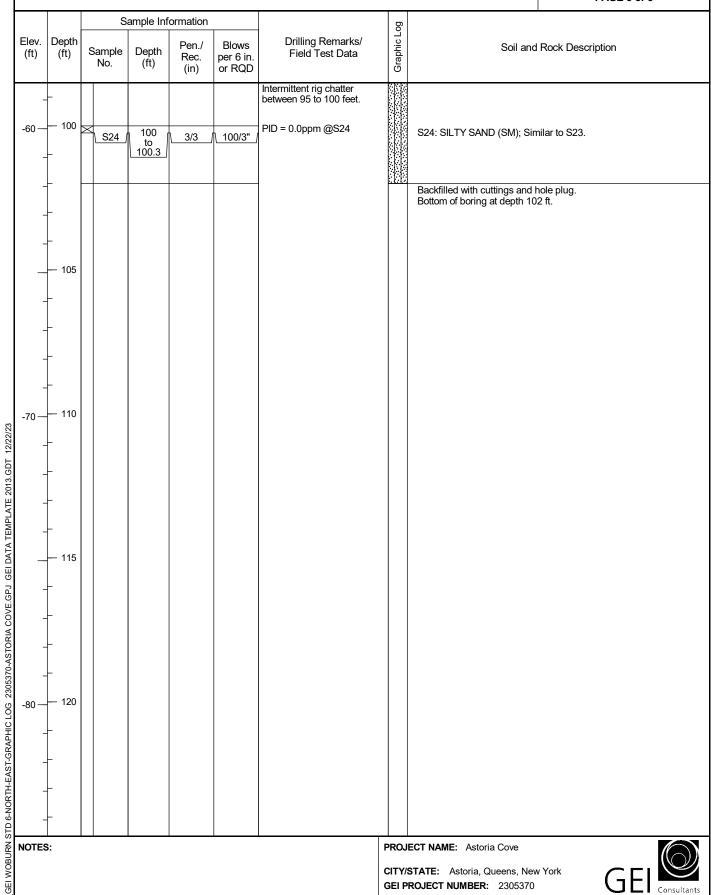
**EASTING (ft):** 1,003,203

**DATE START/END:** 4/29/2020 - 4/29/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B7** 

PAGE 5 of 5



NOTES:

PROJECT NAME: Astoria Cove



**BORING INFORMATION BORING** NORTHING (ft): 221,825 **EASTING (ft):** 1,003,207 GROUND SURFACE EL. (ft): **DATE START/END:** 3/3/2020 - 3/5/2020 23.6 **B8(OW)** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 85.0 DRILLER NAME: Joe Schuster LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: \_Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NX Wireline CORE BARREL I.D./O.D.: 2.125 inch / 3 inch AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): 
▼ 18.1 3/9/2020 6:40 am in observation well ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) (in) or RQD STRATUM I - FILL (CLASS 7) at ~0 ft / El. 23.6 S1: SILTY SAND WITH GRAVEL (SM); ~50% fine sand, ~30% PID = 0.0ppm @S1 0 4-8-16-S1 24/10 to 2 nonplastic fines, ~20% angular gravel up to 1.5"; dark brown; dry. 77 Brick fragments. PID = 0.0ppm @S2 S2: SILTY SAND WITH GRAVEL (SM); Similar to S1; with metal S2 24/4 6-9-46to fragments. 16 20 PID = 0.0ppm @S3 S3: SILTY SAND (SM); ~80% fine sand; ~20% nonplastic fines; S3 24/11 8-12-12to asphalt fragments. 5 PID = 0.0ppm @S4 6 S4: SILTY SAND (SM); similar to S3. S4 24/9 6-12-12to 8 STRATUM IIC - SAND (CLASS 3B) at ~8 ft / El. 15.6 PID = 0.0ppm @S5 S5: NARROWLY GRADED SAND (SP); ~100% fine to medium 8 24/10 S5 3-3-5-5 to 10 sand; light brown and tan; dry. 10 PID = 0.0ppm @S6 S6: NARROWLY GRADED SAND (SP); similar to S5. 10 4-4-5-5 S6 24/9 to 12 Drive casing 12 feet below 10 15 PID = 0.0ppm @S7 S7: SILTY SAND (SM);  $\sim$ 60% fine to medium sand,  $\sim$ 30% S7 24/14 2-3-8-29 to 17 nonplastic fines, ~10% subangular gravel up to 0.75"; light brown; moist. STRATUM III - TILL (CLASS 3A) at ~17 ft / El. 6.6 Rig chatter from 17 to 20 ft. NOTES: PROJECT NAME: Astoria Cove

STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT

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GROUND SURFACE EL. (ft): 23.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

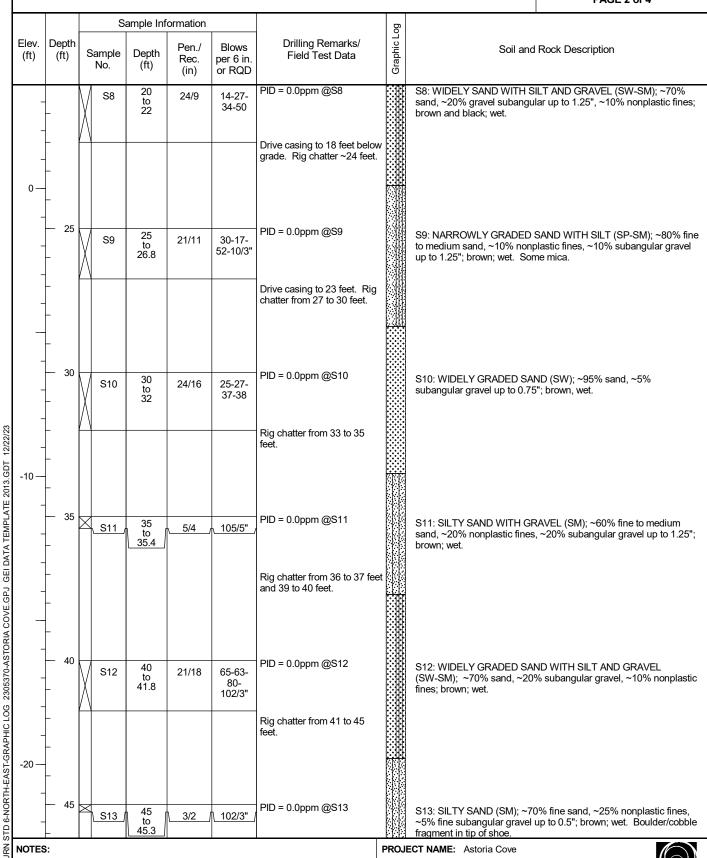
**EASTING (ft):** 1,003,207

**DATE START/END:** 3/3/2020 - 3/5/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B8(OW)

PAGE 2 of 4



CITY/STATE: Astoria, Queens, New York

GEI PROJECT NUMBER: 2305370

GEI WOBURN

GROUND SURFACE EL. (ft): 23.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

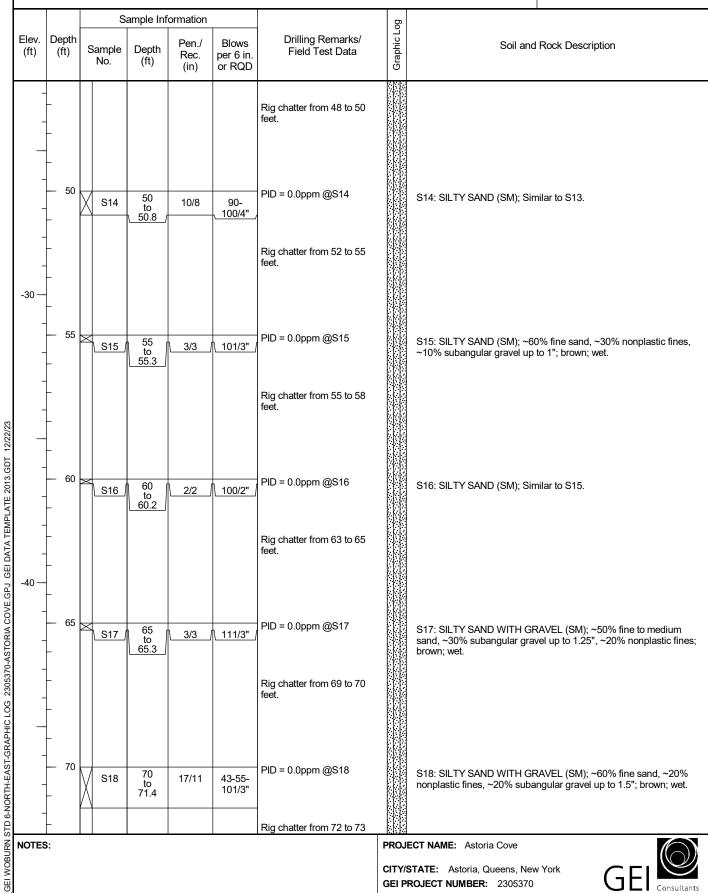
EASTING (ft): 1,003,207

DATE START/END: 3/3/2020 - 3/5/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B8(OW)** 

PAGE 3 of 4





NORTHING (ft): 221,825 GROUND SURFACE EL. (ft): 23.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

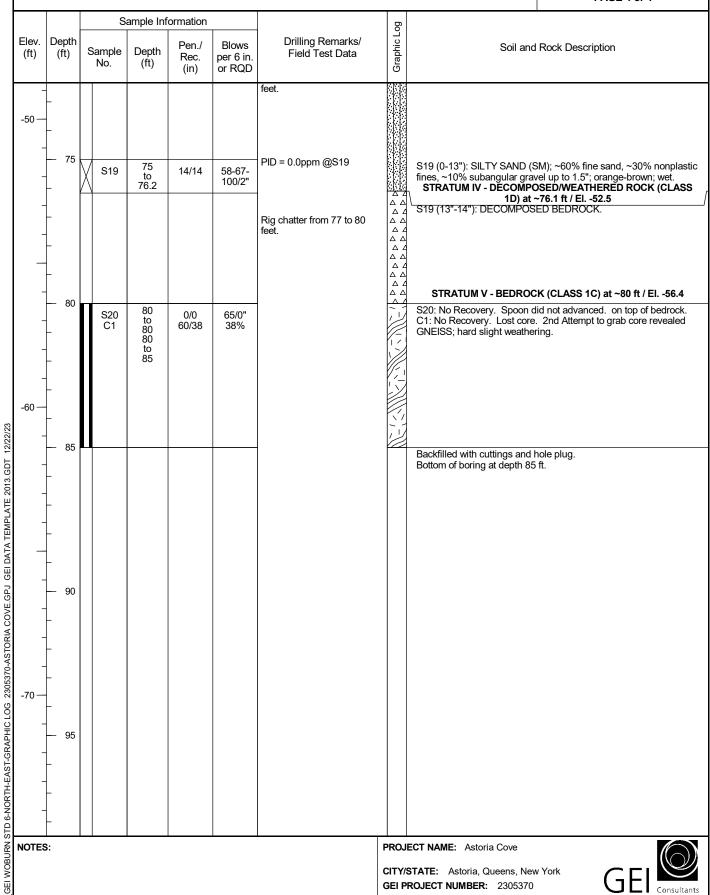
EASTING (ft): 1,003,207

**DATE START/END:** 3/3/2020 - 3/5/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B8(OW)** 

PAGE 4 of 4



	NORTI	HING (	ft):	221,855 CE EL. (	_	3			EASTING (ft): 1,003,2 DATE START/END:		020 - 4/22/2020	BORING
							IY Zone 310		DRILLING COMPANY:			B9
				<b>t):</b> 102 G. Holme					<b>DRILLER NAME</b> : Ke RIG TYPE: CME 75	ith Par	ent	D.O
	LOGGI	ום ט		G. HOIIII	=5			_	RIG TIPE. CIVIE 75			PAGE 1 of 5
	НАММ	ER TY	PE:	Auton	natic				CASING I.D./O.D.: 4 DRILL ROD O.D.: NI			REL TYPE: NA
					lud Rotary	Wash						
	WATE	R LEV	EL C	DEPTHS	(ft):							
	ABBRE	EVIATI	ONS	Rec. RQD WOI	= Length of R = Weight	y Length Iality Designa f Sound Core	ation es>4 in / Pen		S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
				S	ample Inf	formation				go		
	Elev. (ft)	Dept (ft)		Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD		Drilling Remarks/ Field Test Data	Graphic Log		Rock Description (CLASS 7) at ~0 ft / El. 22.3
			+		0	04/00	0.7.0.7	PID	= 0.0ppm @S1		(0-8"): Asphalt.	(CLASS 7) at -0 It / Li. 22.3
	_	_		S1	to 2	24/23	9-7-6-7		•		STRATUM IIA - LOOSE SA S1: NARROWLY GRADED S	AND (CLASS 6) at ~0.7 ft / El. 21.6 SAND WITH SILT (SP-SM); ~85% fine plastic fines, ~5% subangular gravel
	20 —	_		S2	2 to 4	24/8	2-3-3-4	PID	= 0.0ppm @S2		S2: NARROWLY GRADED S nonplastic fines; tan; dry.	SAND (SP); ~95% fine sand, ~5%
	-	<u> </u>	5	S3	4 to 6	24/12	2-5-3-4	PID	= 0.0ppm @S3		S3: NARROWLY GRADED S	SAND (SP); Similar to S2.
8	-		]/\	V							STRATUM IIB - SILT/CLA	Y (CLASS 4B/5B) at ~6 ft / El. 16.3
EMPLATE 2013.GDT 12/22/23	-	<u>-</u>	X	S4	6 to 8	24/17	5-4-3-5	PID	= 0.0ppm @S4		S4: LEAN CLAY (CL); ~89.99	% low plasticity fines, 10.1% fine to . [GRAIN SIZE TEST PERFORMED].
013.0			Ľ						0.0			(CLASS 3B) at ~8 ft / El. 14.3
	-	-	$\backslash$	S5	8 to 10	24/14	4-4-5-18	PID	= 0.0ppm @S5		S5: SILTY SAND (SM); ~80% light brown; dry.	6 fine sand, ~20% nonplastic fines;
J GEI DATA	-	— 1·		S6	10 to 12	24/13	20-26- 30-86	PID	= 0.0ppm @S6		S6 (3"-13"): SILTY SAND (SI	; Similar S5. <b>CLASS 3A) at ~10.5 ft / El. 11.8</b> VI): ~70% fine to medium sand, ~20% Ingular gravel up to 1.25"; brown; dry.
RIA COVE.GP.	10 —	_ 	<u> </u>						e casing to 14 feet below e. Rollerbit to 15 feet.	v	nonpiastic lines, ~ 10 /6 Subar	igulai gravei up to 1.23 , biowii, ury.
2305370-ASTORIA COVE.GPJ GEI DATA T	_	- - 1:	5 —									
RAPHIC LOG 2	-	_ -		S7	15 to 17	24/13	22-19- 19-23					SAND (SP); ~85% fine to medium el up to 1", ~5% nonplastic fines;
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG	- -	-						Inter	er bit to 20 feet. mittent rig chatter from o 18 feet. Rig chatter / advancement from 18 eet.	0		
JRN S	NOTES	S:		·						PROJ	ECT NAME: Astoria Cove	
GEI WOBL											STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GEI

GROUND SURFACE EL. (ft): 22.3

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

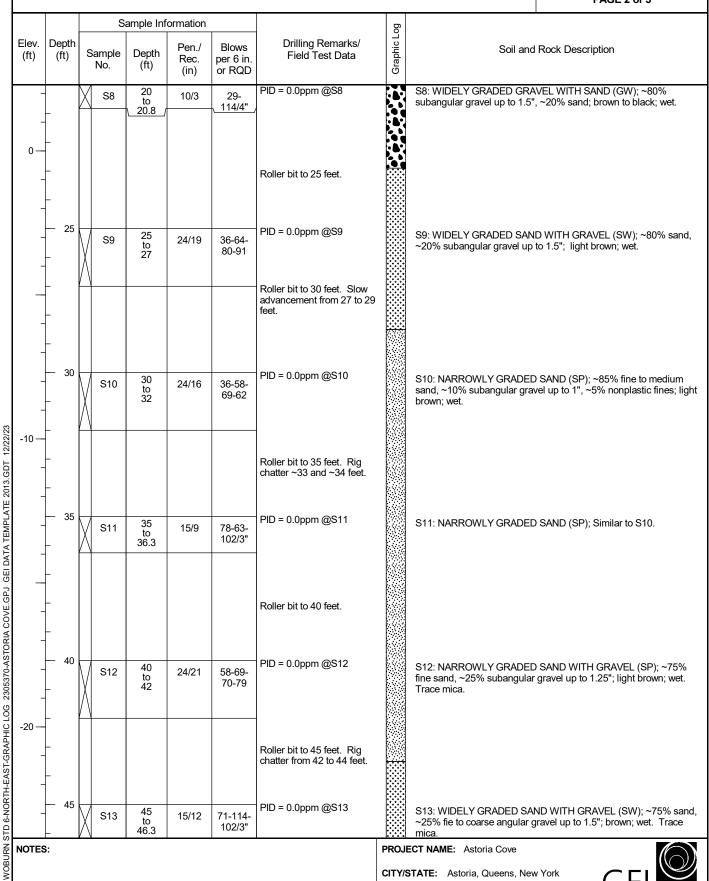
**EASTING (ft):** 1,003,277

**DATE START/END:** 4/20/2020 - 4/22/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B9

PAGE 2 of 5



GROUND SURFACE EL. (ft): 22.3

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

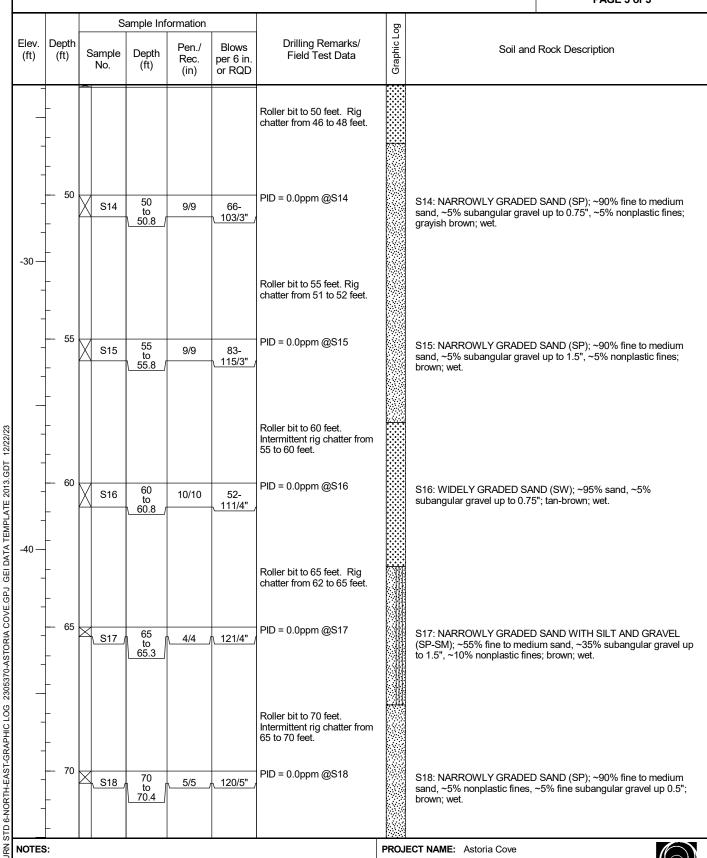
EASTING (ft): 1,003,277

**DATE START/END:** 4/20/2020 - 4/22/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B9

PAGE 3 of 5



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN

NORTHING (ft): 221,855 GROUND SURFACE EL. (ft): 22.3

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,277

**DATE START/END:** 4/20/2020 - 4/22/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B9** 

PAGE 4 of 5

		5	Sample Inf	ormation			go	,
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description
-	-					Roller bit to 75 feet. Rig chatter from 72 to 75 feet.		
- - -	— 75 –	S19	75 to 75.4	5/5	100/5"	PID = 0.0ppm @S19		S19: SILTY SAND WITH GRAVEL (SM); ~50% fine sand, ~30% nonplastic to low plasticity fines, ~20% fine subrounded gravel up to 1.5"; grayish brown; wet. Trace mica.
	-					Roller bit to 80 feet. Intermittent rig chatter from 75 to 80 feet.		
- - -	— 80 -	S20	80 to 81.4	17/17	32-89- 102/5"	PID = 0.0ppm @S20		S20: SILTY SAND WITH GRAVEL (SM); Similar to S19.
-60 —	-					Roller bit to 85 feet. Intermittent rig chatter from 80 to 85 feet.		
- - -	— 85 –	S21	85 to 87	24/19	42-41- 41-27	PID = 0.0ppm @S21		S21: SILTY SAND WITH GRAVEL (SM); Similar to S19. Orange-reddish brown.
-	_					Roller bit to 90 feet.	Δ 2 Δ Δ	STRATUM IV - DECOMPOSED/WEATHERED ROCK (CLASS
- - -	— 90 - -	\$22	90 to 92	24/22	6-9-12- 14	PID = 0.0ppm @S22		S22: SILT (ML); ~90% nonplastic to low plasticity fines, ~5% san ~5% gravel up to 1"; blue to blue-green; wet. Trace mica. Decomposed Rock.
-70 <del>-</del>	- -					Roller bit to 95 feet.		
- - - -	— 95 –	\$23	95 to 97	24/17	5-10-10- 9	PID = 0.0ppm @S23		S23: SILT (ML); Similar to S21; except white. Decomposed Rock
_	-	y V				Roller bit to 100 feet.		
NOTES	<u> </u>						PROJ	ECT NAME: Astoria Cove STATE: Astoria, Queens, New York PROJECT NUMBER: 2305370  GEI Const

NORTHING (ft): 221,855 **EASTING (ft):** 1,003,277 GROUND SURFACE EL. (ft): 22.3

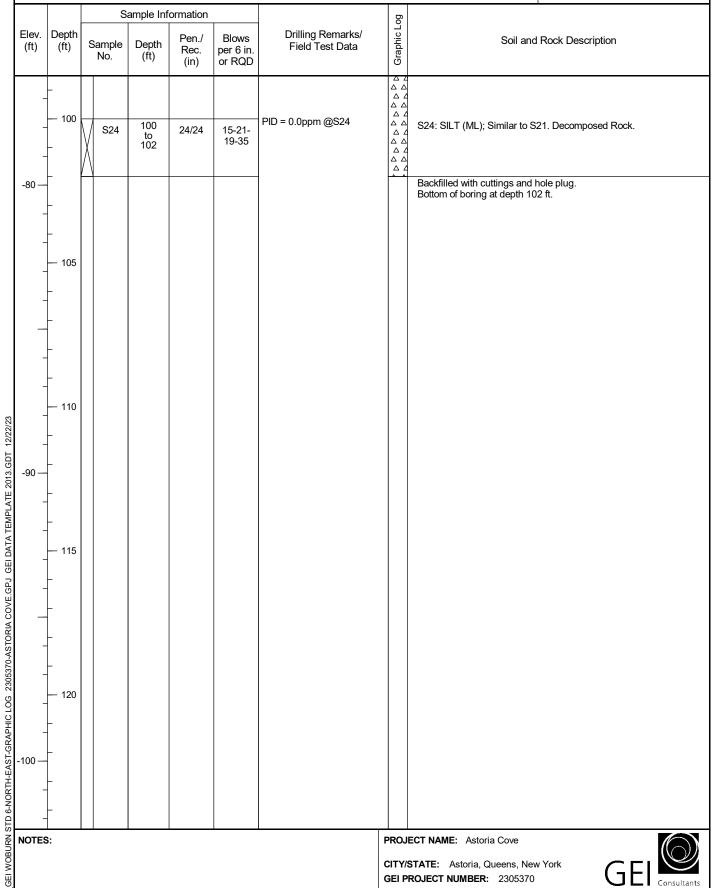
VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**DATE START/END:** 4/20/2020 - 4/22/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B9** 

PAGE 5 of 5



### BORING INFORMATION **BORING** NORTHING (ft): 221,877 **EASTING (ft):** 1,003,222 GROUND SURFACE EL. (ft): 21.9 **DATE START/END:** 4/16/2020 - 4/20/2020 **B10** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 85.0 DRILLER NAME: Keith Parent LOGGED BY: A. Erb / G. Holmes RIG TYPE: CME 75 PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NA CORE BARREL I.D./O.D.: NA / NA AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) Rec. per 6 in. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.9 0 S1 (0-6"): Asphalt. S1 24/13 11-16to 2 S1 (6"-13"):SILTY SAND (SM); ~85% fine to medium sand, ~15% 10-9 nonplastic fines; light brown; dry. STRATUM IIA - LOOSE SAND (CLASS 6) at ~2 ft / El. 19.9 20 S2: NARROWLY GRADED SAND WITH SILT (SP-SM); ~90% fine S2 24/15 9-6-7-7 to to medium sand, ~10% nonplastic fines; tannish brown; dry. Trace mica. S3: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to S3 24/17 4-4-4-2 to 6 S4: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to 24/15 4-2-3-4 to 8 8 S5: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to S5 24/11 4-4-3-3 to 10 Drive casing to 9 feet below grade. Rollerbit to 10 feet. 10 S6: NARROWLY GRADED SAND (SM); ~95% fine to medium 10 S6 24/8 3-4-4-6 to 12 sand, ~5% nonplastic fines; brown; dry. 10 Roller bit to 15 feet. STRATUM III - TILL (CLASS 3A) at ~13.5 ft / El. 8.4 15 S7: SILTY SAND WITH GRAVEL (SM); ~60% fine to medium S7 24/11 10-26to 17 sand, ~25% subangular gravel up to 0.75", ~15% nonplastic fines; 18-38 dark gray; moist. Roller bit to 20 feet. Rig chatter ~16 feet and from 18.5 to 20 feet. NOTES: PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York

GEI PROJECT NUMBER: 2305370

**GEI DATA TEMPLATE 2013.GDT** 

LOG 2305370-ASTORIA COVE.GPJ

STD 6-NORTH-EAST-GRAPHIC

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GEI Consultants

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

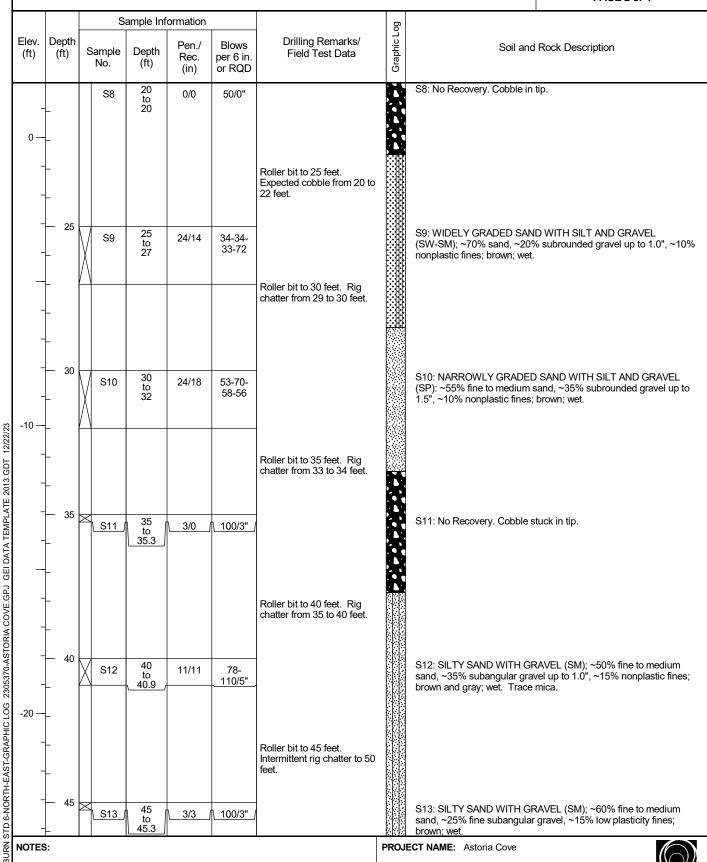
EASTING (ft): 1,003,222

**DATE START/END:** 4/16/2020 - 4/20/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B10

PAGE 2 of 4



**CITY/STATE:** Astoria, Queens, New York **GEI PROJECT NUMBER:** 2305370

GEI WOBURN

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

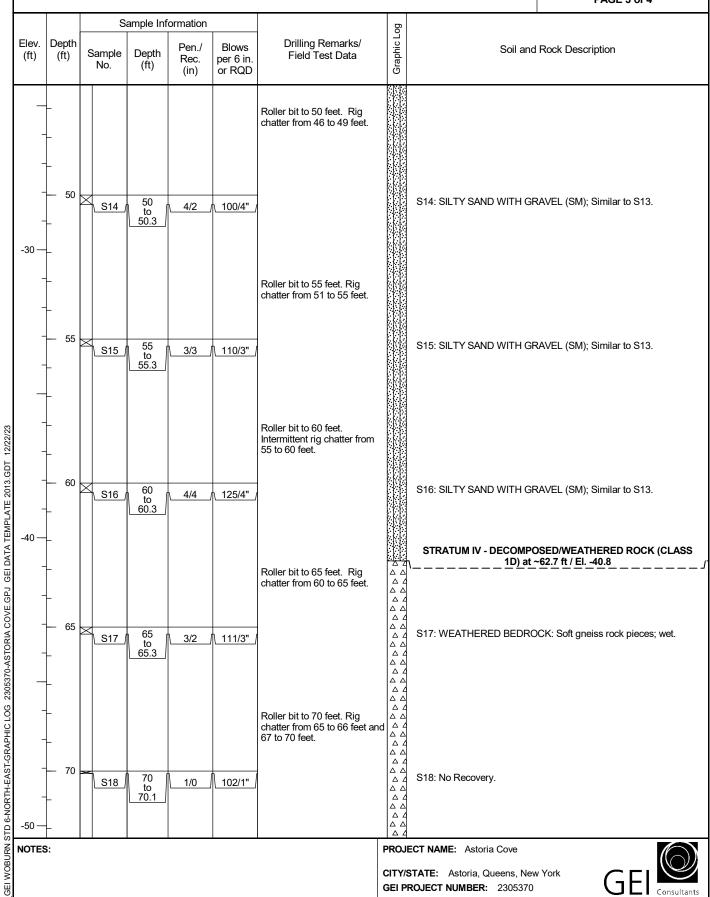
**EASTING (ft):** 1,003,222

**DATE START/END:** 4/16/2020 - 4/20/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B10

PAGE 3 of 4



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

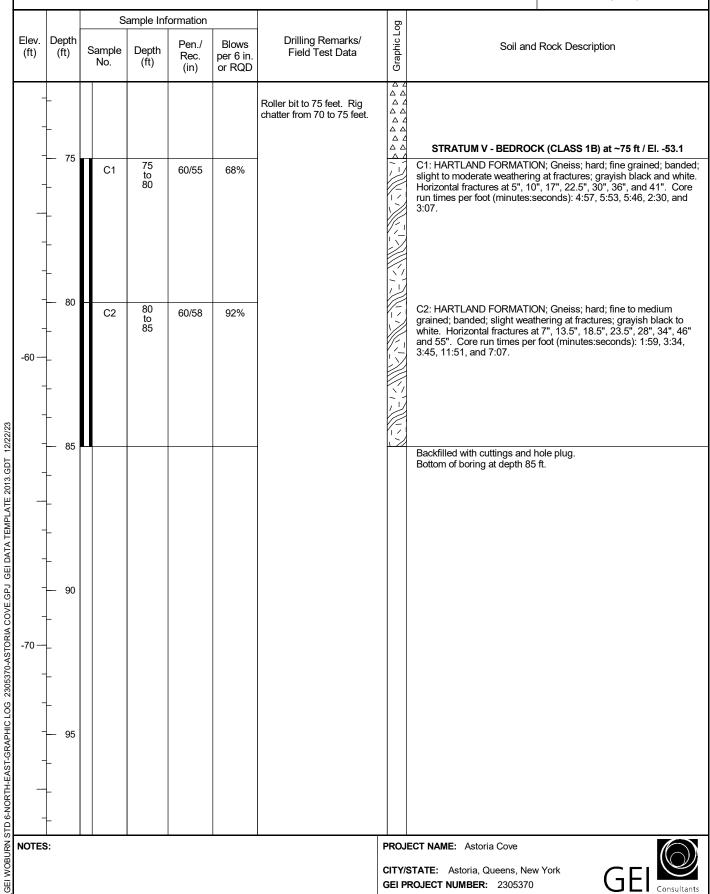
**EASTING (ft):** 1,003,222

**DATE START/END:** 4/16/2020 - 4/20/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B10** 

PAGE 4 of 4



				221,89				<b>EASTING (ft):</b> 1,003,1	71		BORING
			٠,		o (ft): 21.1	7		DATE START/END: 5		20 - 5/7/2020	
	VERT.	/HOR	Z. D	ATUMS:	NAVD 8	8/NAD83 N	IY Zone 310	DRILLING COMPANY:	Crai	g Geotechnical Drilling	B11
				<b>ft):</b> <u>90</u> G. Holm				DRILLER NAME: Kei	th Par	ent	
	LUGG	בט פו	-	G. HOIII	les			RIG TYPE: CME 75		_	PAGE 1 of 4
	DRILL	ING	NF	ORMAT	<u>ION</u>						
				: <u>Auto</u> .: NA				CASING I.D./O.D.: 4 i			RREL TYPE: NA RREL I.D./O.D.: NA / NA
					Mud Rotary	/ Wash		DRILL ROD O.D NIV		CORE BAI	TREE I.D.JO.D NAT NA
	WATE	R LE\	ΈL	DEPTHS	(ft):						
	ABBRI	EVIAT	ION	Red RQ WC		y Length Jality Designa f Sound Core of Rods	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside	Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
				;	Sample In	formation			g		
	Elev.	Dep	th			Pen./	Blows	Drilling Remarks/	Graphic Log	Soil and	Rock Description
	(ft)	(ft)		Sample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	iraph	Soli aliu	Nock Description
L					. ,	(in)	or RQD	DID 00 CO1	Θ		(CLASS 7) at ~0 ft / El. 21.7
	20 —	_		S1	to 2	24/6	41-12-2- 5	PID = 0.0ppm @S1			6 fine to medium sand, ~20% o medium angular gravel up to 1"; dark te fragments.
	-			S2	2 to 4	24/17	3-54-18- 5	PID = 0.0ppm @S2		~10% nonplastic fines, ~10% brown; dry. Concrete fragme	D WITH SILT (SW-SM); ~80% sand, 6 subangular gravel up to 1" tannish ents in bottom 7" of spoon. Fill.
	-	-	-	}	4			PID = 0.0ppm @S3			SAND (CLASS 6) at ~4 ft / El. 17.7
23	-		5	S3	to 6	24/11	3-4-4-3	9		53: WIDELY GRADED SAIN	D (SW); ~100% sand; tan-brown; dry.
EMPLATE 2013.GDT 12/22/23		_ _ _		S4	6 to 8	24/5	2-2-3-3	PID = 0.0ppm @S4		S4: WIDELY GRADED SAN fines, ~5% subangular up to	D (SW); ~90% sand, ~5% nonplastic 1"; brown; dry. Some mica.
EMPLATE 201	-	_ _		S5	8 to 10	24/3	2-3-3-7	PID = 0.0ppm @S5		S5: WIDELY GRADED SAN	, ,
ATA T	-	<u> </u>	o (	)	10	0.444.4		PID = 0.0ppm @S6	••••		D (CLASS 3B) at ~10 ft / El. 11.7  D WITH GRAVEL (SW); ~70% sand,
E.GPJ GEI DA	10 —	_	/	S6	to 12	24/11	9-6-6-5	Drive casing to 9 feet below			to 1.5"; brown; dry. Trace mica.
TORIA COV	-	_						grade. Roller bit to 15 feet. Rig chatter from 12 to 14 feet.		STRATUM III - TILL (	CLASS 3A) at ~13.5 ft / El. 8.2
, 2305370-As	_	-   1	5	/ S7	15	24/3	16-21-	PID = 0.0ppm @S7		S7: WIDELY GRADED SAN	D WITH GRAVEL (SW); ~75% sand,
STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA	_	- - -	\ <u>/</u>	51	to 17	24/3	16-21- 16-20	Roller bit to 20 feet. Rig			to 1.25", ~5% nonplastic fines;
	- -	_						chatter from 16 to 20 feet.			
OBURN	NOTES	<b>S</b> :								ECT NAME: Astoria Cove	
GEI WOBURN										STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	YYORK GEI Consultants

GROUND SURFACE EL. (ft): 21.7

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,171

**DATE START/END:** 5/7/2020 - 5/7/2020

**BORING B11** DRILLING COMPANY: Craig Geotechnical Drilling

PAGE 2 of 4

							T			PAGE 2 01 4
	D = 1=4l=		S	ample Inf			Duillin or Dama anka/	. Log		
Elev. (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description
0-	-	$\bigvee$	S8	20 to 22	24/10	13-22- 20-14			S8: WIDELY GRADED SANI	O WITH GRAVEL (SW); Similar to Si
-	-						Roller bit to 25 feet. Intermittent rig chatter from 20 to 25 feet.			
_	— 25 - -	$\bigvee$	S9	25 to 27	24/11	25-38- 23-36	Roller bit to 30 feet. Rig		S9: NARROWLY GRADED S to medium sand, ~10% nonp up to 1.5"; brown; wet. Trace	SAND WITH SILT (SP-SM); ~80% fil lastic fines, ~10% subangular grave · mica.
-	_						chattert from 26 to 28 feet.			
-10 —	— 30 –	X	S10	30 to \_30.8/	9/9	81- 100/3"			S10: NARROWLY GRADED to S9.	SAND WITH SILT (SP-SM); Simila
-	- -						Roller bit to 35 feet.			
-	— 35 -	X	S11 )	35 to 35.4	√ 5/5	102/5"			S11: NARROWLY GRADED to S9.	SAND WITH SILT (SP-SM); Simila
-	-						Roller bit to 40 feet. Rig chatter from 39 to 40 feet.			
-	— 40 –		S12	40 to 40.4	5/5	<u>, 107/5"</u>			S12: SILTY SAND (SM); ~70 nonplastic fines, ~10% subar wet.	% fine to medium sand, ~20% ngular gravel up to 1.0"; gray-browr
-20 —	_						Roller bit to 45 feet. Intermittent rig chatter from 40 to 45 feet.			
_	— 45 –	X	S13	45 to 45.3	\ 4/4	/ 102/4"			S13: SILTY SAND (SM); Sim	ilar to S12.
NOTES	:							CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New  ROJECT NUMBER: 2305370	York GEI Consulta



GROUND SURFACE EL. (ft): 21.7

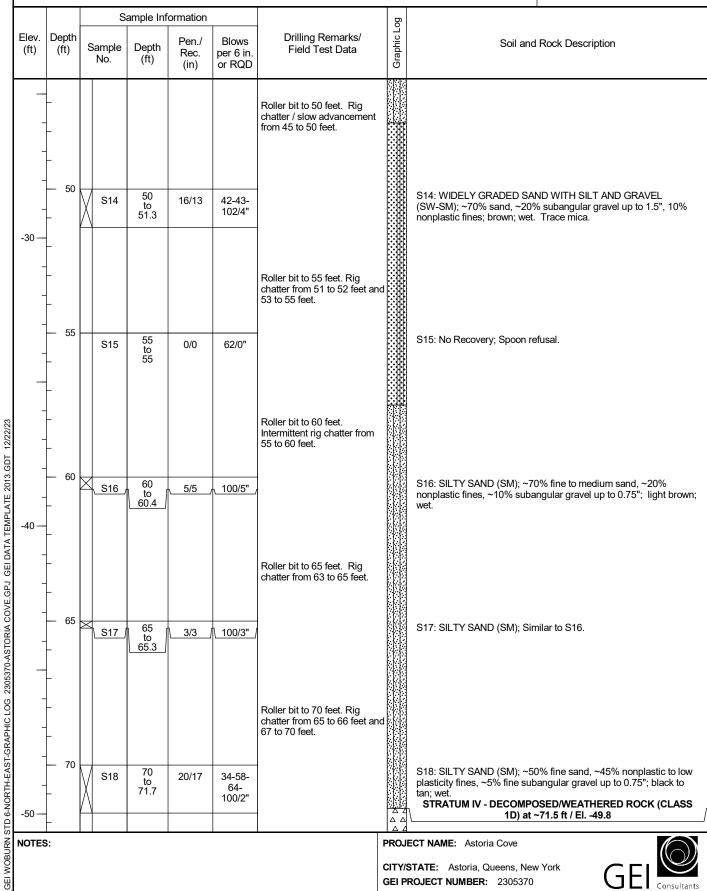
VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,171

**DATE START/END:** 5/7/2020 - 5/7/2020

**BORING B11** DRILLING COMPANY: Craig Geotechnical Drilling

PAGE 3 of 4





NORTHING (ft): 221,898 GROUND SURFACE EL. (ft): 21.7

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

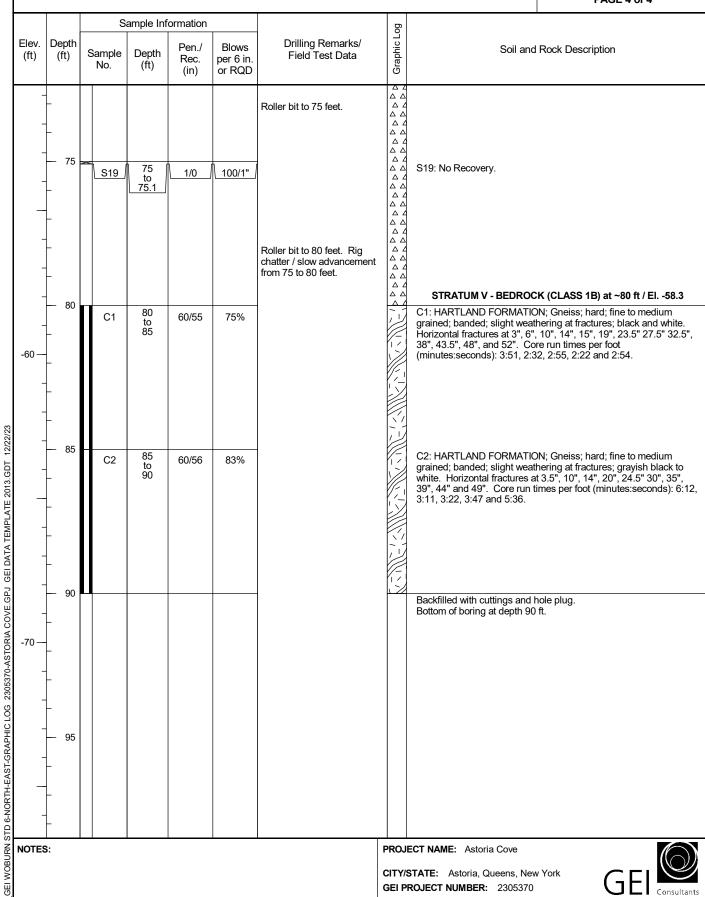
EASTING (ft): 1,003,171

**DATE START/END:** 5/7/2020 - 5/7/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B11** 

PAGE 4 of 4



Ī				<b>MATION</b> 222,043	_			<b>EASTING (ft):</b> 1,003,55	93		BORING
	GROU	ND SÚI	RFA	CE EL. (1	ft):14.8			DATE START/END: 5	/27/20		D40
							Y Zone 310				B12
			-	): <u>77.0</u> G. Holme				DRILLER NAME: <u>Ed</u> RIG TYPE: CME 75	гіапа	gan	PAGE 1 of 4
ŀ											FAGE 1 01 4
				RMATION Autom				CASING I.D./O.D.: 4 i	nah/ 1	E inch CORE BAE	DEL TYPE. NA
				NA /				DRILL ROD O.D.: NN			RREL TYPE: NA RREL I.D./O.D.: NA / NA
	DRILLI	NG ME	ETHO	OD: M	lud Rotary	Wash					
	WATE	R LEVE	EL D	EPTHS (	ft):						
	ABBRI	EVIATIO	ONS	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside [	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
				S	ample Inf	ormation			)g		
	Elev.	Depth				Pen./	Blows	Drilling Remarks/	Graphic Log	Soil and	Rock Description
	(ft)	(ft)	١	Sample No.	Depth (ft)	Rec. (in)	per 6 in. or RQD	Field Test Data	raph		·
					. ,	(111)	OI KQD				(CLASS 7) at ~0 ft / El. 14.8
	-	_		S1	0 to 2	24/9	9-4-6-3				GRAVEL WITH SAND (GP); ~75% , ~25% sand; brown and black; dry. sand.
			X	S2	2 to	9/7	6-50/3"				SAND WITH GRAVEL (SP); ~80%
	-	_			2.8					tine to medium sand, ~20% to brown; dry. Asphalt.	ine angular gravel up to 1"; black and
	_										
	10 —	- - 5	5					Roller bit to 5.5 feet with stabilizer bit.			
12/22/23	-	_	$\mathbb{N}$	S3	5.5 to 7	18/10	24-22-18			(SP-SM); ~70% fine to mediu	SAND WITH SILT AND GRAVEL Im sand, ~15% nonplastic fines, to 1"; brown and white; wet. Concrete
	-	_		S4	7	24/0	15-10-4-			fragments. S4: No Recovery.	
EMPLATE 2013.GDT	-	_		34	to 9	24/0	15-10-4-			,	
PLATE	-							No recovery Podrive with			
	_	<u> </u>	$\mathbb{N}$	S5	9 to 11	24/0	4-3-1-1	No recovery. Redrive with 3-inch split spoon. @S5		(SP-SM); ~70% fine to mediu	SAND WITH SILT AND GRAVEL Im sand, ~15% nonplastic fines, '; brown and black; wet. Contains
GEI DATA T	_		$\backslash$							glass and roots.	, -
J GE		_									
Æ.GF	-	_						Advance casing to 15 feet			
4 CO	_							below grade. Roller bit to 15 feet.		STRATUM IIC - SAND	(CLASS 3B) at ~13 ft / El. 1.8
2305370-ASTORIA COVE.GPJ								loct.			
0-AS	-	-									
30537	0 —	4.5									
)G 23		<u> </u>	`\	S6	15 to	24/6	4-4-4-3			S6: WIDELY GRADED SANI ~15% fine angular gravel up	D WITH GRAVEL (SW); ~85% sand,
IIC LC	-	_	IX.		17					1070 mile angalar graver ap	,,
RAP	-		$\langle \rangle$								
ST-G											
TH-EA		<u> </u>						Advance casing to 20 feet below grade. Roller bit to 20			
NOR.	-	<u> </u>						feet.			
STD 6-NORTH-EAST-GRAPHIC LOG	_										
	NOTES	):		1			1		PROJ	ECT NAME: Astoria Cove	
GEI WOBURN										STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 14.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

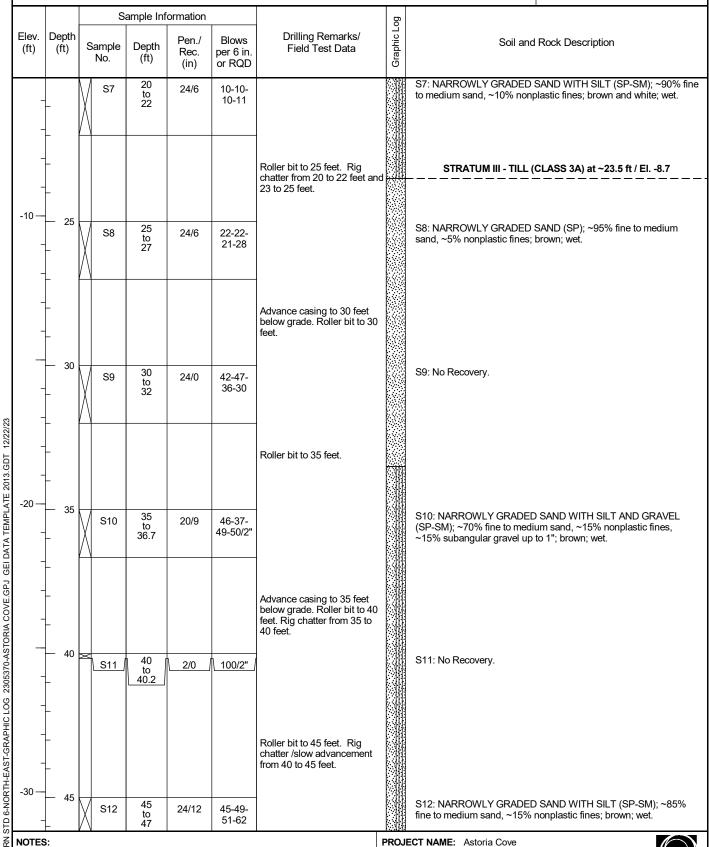
**EASTING (ft):** 1,003,593

**DATE START/END:** 5/27/2020 - 5/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B12

PAGE 2 of 4



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN

GROUND SURFACE EL. (ft): 14.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,593

**DATE START/END:** 5/27/2020 - 5/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B12

PAGE 3 of 4

Elev. De		0				1			
Flev Da			ample Inf	ormation	T		Fog		
(ft) (f		Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
	50	4 212	50	47/44	50.74	Roller bit to 50 feet.		S13: SILTY SAND (SM): ~7(	√% fine to medium sand, ~10%
		/ S13	51.4	17/11	52-71- 100/5"	Roller bit to 55 feet. Rig chatter from 53 to 55 feet.		nonplastic fines, ~10% fine s wet.	ubangular gravel up to 3/4"; brown;
-40	55	S14_	55 to 55.4	<u>5/5</u>	100/5"	PID = 0.0ppm @S14  Roller bit to 60 feet. Slight rig chatter.		S14: NARROWLY GRADED fine to medium sand, ~10% i to subrounded gravel; brown	SAND WITH SILT (SP-SM); ~85% nonplastic fines, ~5% fine subangula ; wet.
	60	S15	60 to 61.8	21/9	49-48- 72- 100/3"	PID = 0.0ppm @S15  Roller bit to 65 feet.		S15: SILTY SAND (SM); ~75 nonplastic fines, ~5% subant tan-brown; wet.	% fine to medium sand, ~20% gular to subrounded gravel up to 1";
-50 —	65	S16 /	65 to 65.4	5/5	101/5"	PID = 0.0ppm @S16		S16: SILTY SAND (SM); Sin orange-brown to yellow-brow	illar to S15, except sand and n.  SED/WEATHERED ROCK (CLASS 67.7 ft / El52.9
	70 >	S17 J	70 to 70.3	3/3	103/3" )	Roller bit to 70 feet.  PID = 0.0ppm @S17		S17: SILTY SAND (SM); ~65	% micaceous sand, ~30% nonplastine gravel; black, white, reddish-brow

NORTHING (ft): 222,043 GROUND SURFACE EL. (ft): 14.8

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

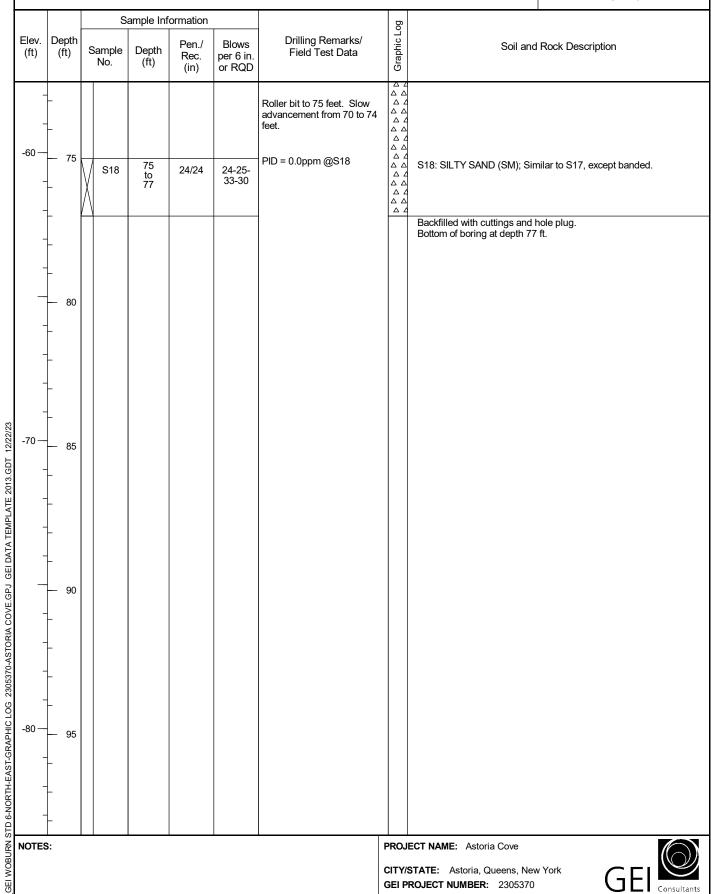
**EASTING (ft):** 1,003,593

**DATE START/END:** 5/27/2020 - 5/28/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B12** 

PAGE 4 of 4



				<b>MATION</b> 221,884	_			<b>EASTING (ft):</b> 1,003,4	27		BORING
	GROU	ND SUR	FA	CE EL. (1	ft):23.5			DATE START/END:	5/6/20		D40
				TUMS: ): 102		B/NAD83 N	Y Zone 310			ig Geotechnical Drilling	B13
			•	): <u>102</u> 3. Holme				DRILLER NAME: Ke RIG TYPE: CME 75	ılıı Pai	ent	PAGE 1 of 5
											TAGETOTO
				RMATION Autom				CASING I.D./O.D.: 4	inch/	1.5 inch COPE BAS	RREL TYPE: NA
				NA /				DRILL ROD O.D.: NA			RREL I.D./O.D.: NA / NA
					lud Rotary	Wash					
	WATE	R LEVE	LD	EPTHS (	(π):						
	ABBRI	EVIATIO	NS	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample .,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	30 inches to drive a 2-inch-O.D. split spoon sampler.
				S	ample Inf	ormation			gc		
	Elev.	Depth	Ĺ		D41	Pen./	Blows	Drilling Remarks/	Graphic Log	Soil and	Rock Description
	(ft)	(ft)	١٤	Sample No.	Depth (ft)	Rec. (in)	per 6 in. or RQD	Field Test Data	Sraph		·
						(111)	OI NQD	PID = 0.0ppm @S1	SIM:E		(CLASS 7) at ~0 ft / El. 23.5
	-	-	$\mathbb{N}$	S1	to 2	24/9	7-7-8-12			nonplastic fines, ~10% angul Coal fragments.	6 fine to medium sand, ~30% ar gravel up to 1"; dark brown; dry.
	20 —	<b>-</b> -	V	S2	2 to 4	24/10	2-4-3-6	PID = 0.0ppm @S2			% sand, 37.6% nonplastic fines, 7.6% e subangular to subrounded gravel up FEST PERFORMED].
	-	- 5	$\bigvee$	S3	4 to 6	24/24	8-9-3-9	PID = 0.0ppm @S3		to medium sand, ~10% nonp	SAND WITH SILT (SP-SM); ~90% fine lastic fines; orange-brown; dry.
2/23	_		$\triangle$					PID = 0.0ppm @S4		4	SAND (CLASS 6) at ~6 ft / El. 17.5
13.GDT 12/22/23	-	-	$\bigvee$	S4	6 to 8	24/13	6-2-2-2	0		S4: WIDELY GRADED SANI fines, ~5% subrounded gravi	O (SW); ~90% sand, ~5% nonplastic al up to 0.75"; reddish-brown; dry.
EMPLATE 2013.GDT		<del>-</del> -	V	S5	8 to 10	24/16	5-3-2-2	PID = 0.0ppm @S5		sand, ~5% nonplastic fines; I	, <b>,</b>
TAT		10	(-)		10			PID = 0.0ppm @S6		<u> </u>	(CLASS 3B) at ~10 ft / El. 13.5 SAND (SP); ~95% fine to medium
GPJ GEI DATAT	-	-	$\mathbb{N}$	S6	to 12	24/24	4-3-7-8	11 0		sand, ~5% nonplastic fines; I	
OVE	_							Drive casing to 10 feet below grade. Rollerbit to 15 feet.	v		
05370-ASTORIA	10 —	-									
STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ	-	— 15 - -	$\bigvee$	S7	15 to 17	24/8	10-8-8- 10				% sand, 35.6% nonplastic fines, 5.8% 1"; tan-brown; dry. [GRAIN SIZE
RTH-EAST.	_	<u>-</u>						Drive casing to 20 feet below grade. Rollerbit to 20 feet.	v	STRATUM III - TILL	(CLASS 3A) at ~18.5 ft / El. 5
STD 6-NO	_	_									
URN	NOTES	<b>3</b> :							PRO	JECT NAME: Astoria Cove	
GEI WOBURN										/STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 23.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

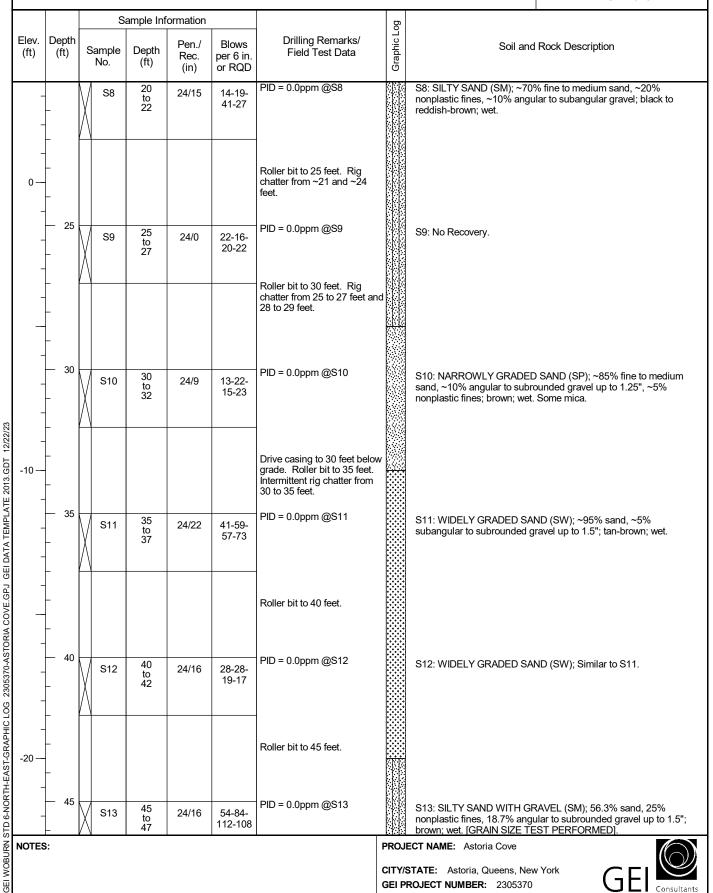
**EASTING (ft):** 1,003,427

**DATE START/END:** 5/6/2020 - 5/6/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B13** 

PAGE 2 of 5



GROUND SURFACE EL. (ft): 23.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

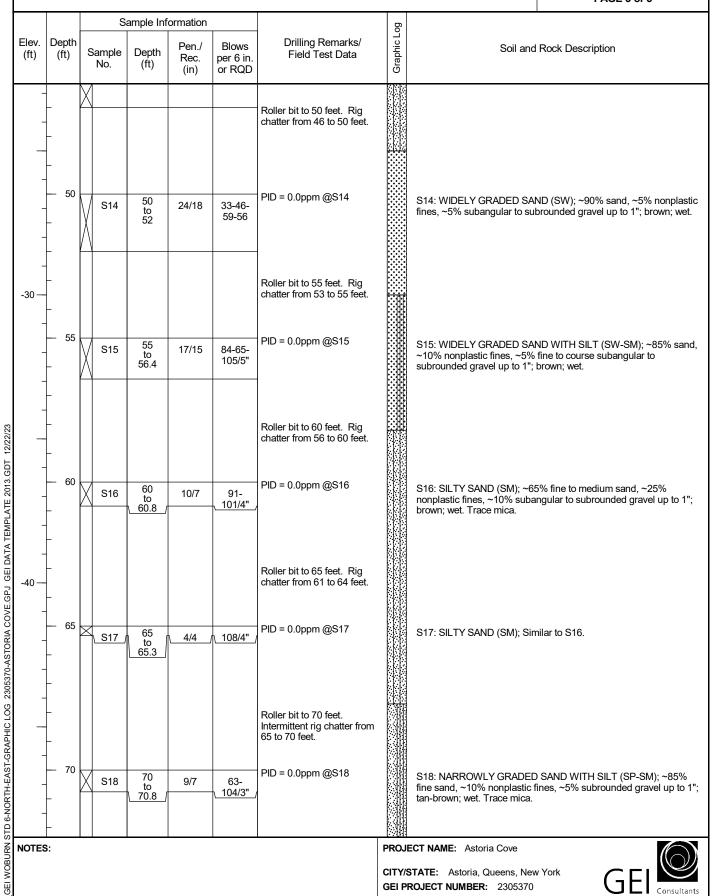
EASTING (ft): 1,003,427

**DATE START/END:** 5/6/2020 - 5/6/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B13** 

PAGE 3 of 5



NORTHING (ft): 221,884 GROUND SURFACE EL. (ft): 23.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

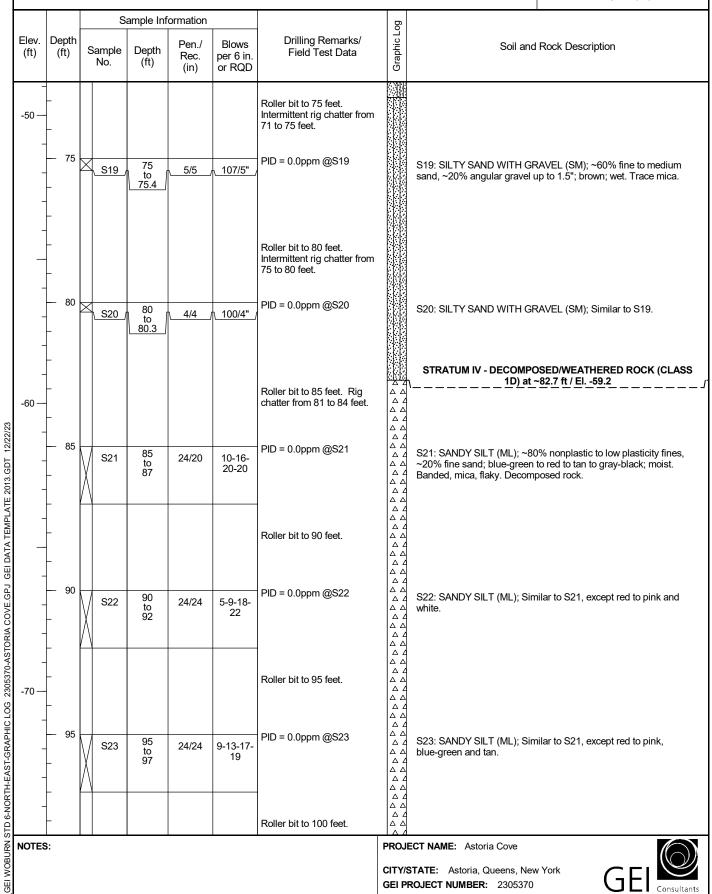
**EASTING (ft):** 1,003,427

**DATE START/END:** 5/6/2020 - 5/6/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B13** 

PAGE 4 of 5



VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

23.5

GROUND SURFACE EL. (ft): \_

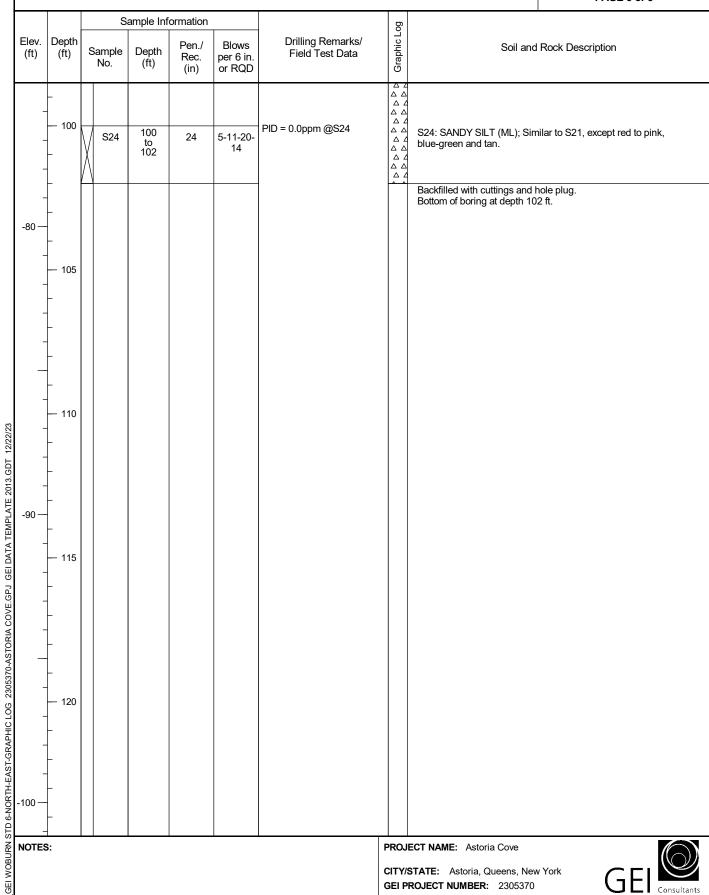
**EASTING (ft):** 1,003,427

**DATE START/END:** 5/6/2020 - 5/6/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B13** 

PAGE 5 of 5



				MATIOI	_			EASTING (E) 4 000 0	20		BORING
		-	_	221,930 CE EL. (1	ft): 21.8	3		EASTING (ft): _1,003,31 DATE START/END: 6		20 - 6/9/2020	20.4.10
				•			Y Zone 310				B14
				):75.				DRILLER NAME: Nic	k Beel	nlaer	
	LOGGI	ED BY:	_/	A. Erb / C	6. Holmes			RIG TYPE: CME 55LC			PAGE 1 of 4
İ	DRILL	ING IN	IFO	RMATI	ON						
	НАММ	ER TYF	E:	Auton	natic			CASING I.D./O.D.: _4.	125 inc		RREL TYPE: NX Wireline
				NA /				DRILL ROD O.D.: NN	1	CORE BAI	RREL I.D./O.D.: 2.125 inch / 3 inch
				טט: <u>№</u> EPTHS (	lud Rotary (ft):	vvasn					
ļ											
	ABBR	EVIATIO	ONS	Rec. RQD WOF	= Length of R = Weight	/ Length ality Designate Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside	Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
Ī				S	ample Inf	ormation			ō		
	Elev.	Depth				Pen./	Blows	Drilling Remarks/	Graphic Log	Cail and	Real Description
	(ft)	(ft)	8	Sample No.	Depth (ft)	Rec.	per 6 in.	Field Test Data	raph	Soil and	Rock Description
L				110.	(11)	(in)	or RQD		Ö	STRATUM I - FILL	(CLASS 7) at ~0 ft / El. 21.8
	20 —	_	$\mathbb{N}$	S1	0 to 2	24/18	14-18- 14-15			(0-4"):Concrete. S1: NARROWLY GRADED sand, ~30% nonplastic fines tan-brown; dry.	SAND WITH SILT (SP-SM): ~85% fine , ~5% subangular gravel up to 1";
		_		S2	2	24/20	12-8-9-6				SAND (SP); ~95% fine to medium
	-	_	ΙV		to 4			Roller bit to 4 feet.		sand, ~5% nonplastic fines,	tan-brown; moist.
	_		$ \rangle$					Troiler bit to 4 leet.		STRATUM IIA - LOOSE	SAND (CLASS 6) at ~4 ft / El. 17.8
		_		S3	4	24/16	5-4-3-4			S3: NARROWLY GRADED	SAND (SP); Similar to S2.
	-	<u> </u>	ΙV		to 6						
က	_										
EMPLATE 2013.GDT 12/22/23		_		S4	6	24/7	4-5-8-5				SAND (SP); ~95% sand, ~5%
T 12	_	_	ΙV	•	to 8					nonplastic fines; brown; mois	st.
3.6□											
E 20		_		S5	8	24/13	2-2-2-2			S5: WIDELY GRADED SAN	D (SW): ~90% sand, ~5% fine
PLAT	-	_	ΙV		to 10			Drive casing to 4 feet below		subangular gravel, ~5% non mica.	plastic fines; brown; moist. Trace
⊢								grade. Roller bit to 10 feet.		STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 11.8
ATA		— 10		S6	10	24/14	5-5-10-			S6: NARROWLY GRADED	SAND WITH SILT (SP-SM): ~90% fine
GEL	_		Ŋ	00	to 12	2-7/1-7	10			to medium sand, ~10% nonp	plastic fines; brown; moist. Trace mica.
3PJ	40		$\mathbb{N}$								
OVE.(	10 —	_	$\vdash$					Roller bit to 15 feet. Add EZ			
ΑCC	-							mud.			
STOR										STRATUM III - TILL (	CLASS 3A) at ~13.5 ft / El. 8.3
70-AS	_	_									
3053	-	— 15									
JG 2		13	$\mathbb{N}$	S7	15 to	24/12	17-12-			S7: WIDELY GRADED SAN ~20% gravel up to 0.5". ~10	D WITH SILT (SW-SM): ~70% sand, % nonplastic fines; brown; wet. Trace
¶C L(	_	_	Įχ.		17		12-9			mica.	, , , ,
RAPH	_		$\mathbb{Z}$								
ST-G											
H-EA:	_	_						Roller bit to 20 feet.			
ORT	_								147		
STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA											
	NOTES		1						DDO !	ECT NAME: Astoria Cove	
BUR	NOTES										
GEI WOBURN										STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	YYORK GEI Consultants

GROUND SURFACE EL. (ft): 21.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

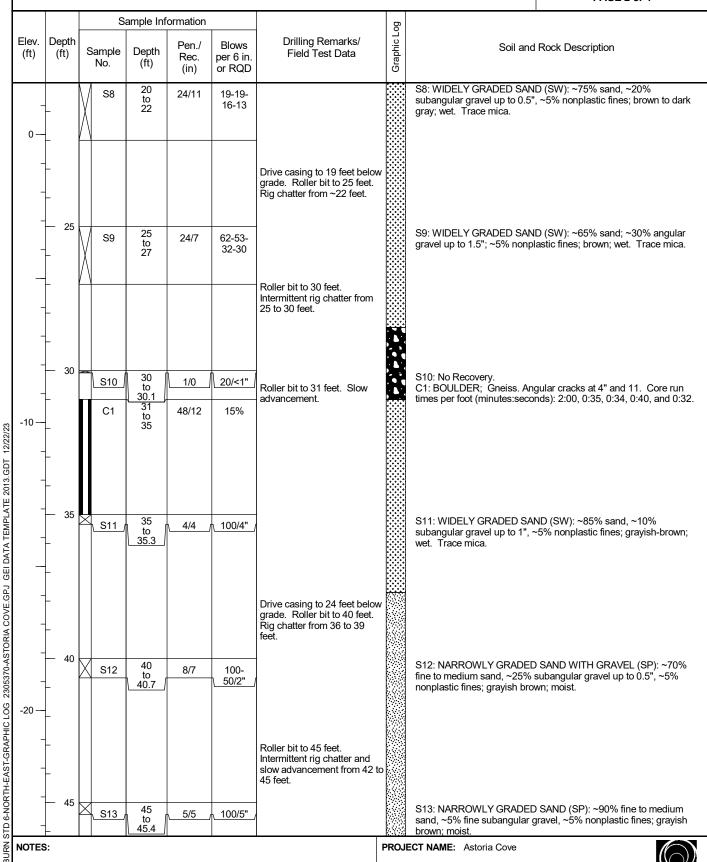
**EASTING (ft):** 1,003,368

**DATE START/END:** 6/8/2020 - 6/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

### BORING B14

PAGE 2 of 4



GEI WOBURN



GROUND SURFACE EL. (ft): 21.8

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,368

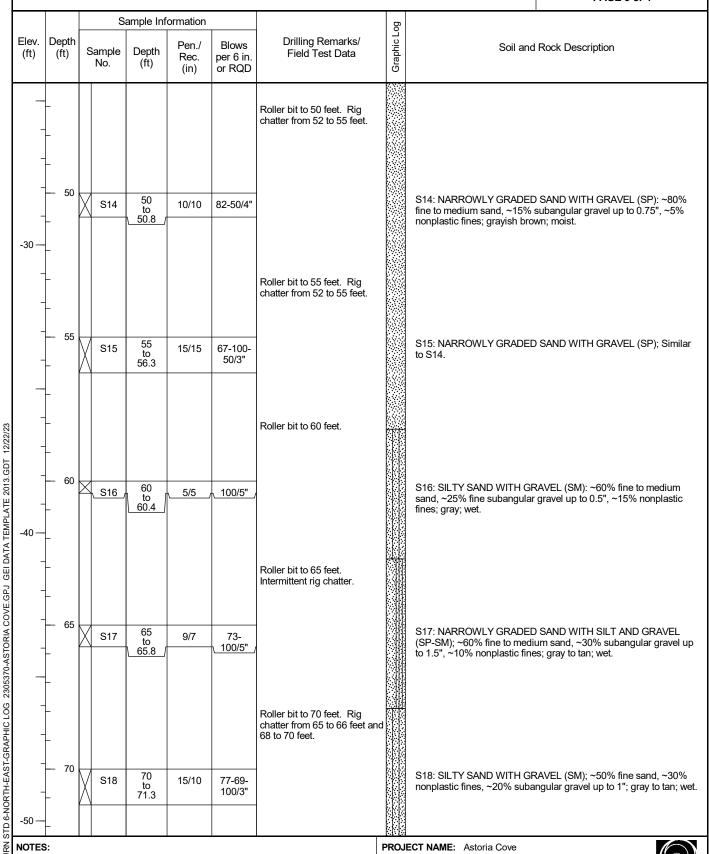
**DATE START/END:** 6/8/2020 - 6/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

BORING B14

PAGE 3 of 4



NOTE MOBURN

NORTHING (ft): 221,930 GROUND SURFACE EL. (ft): \_ 21.8

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

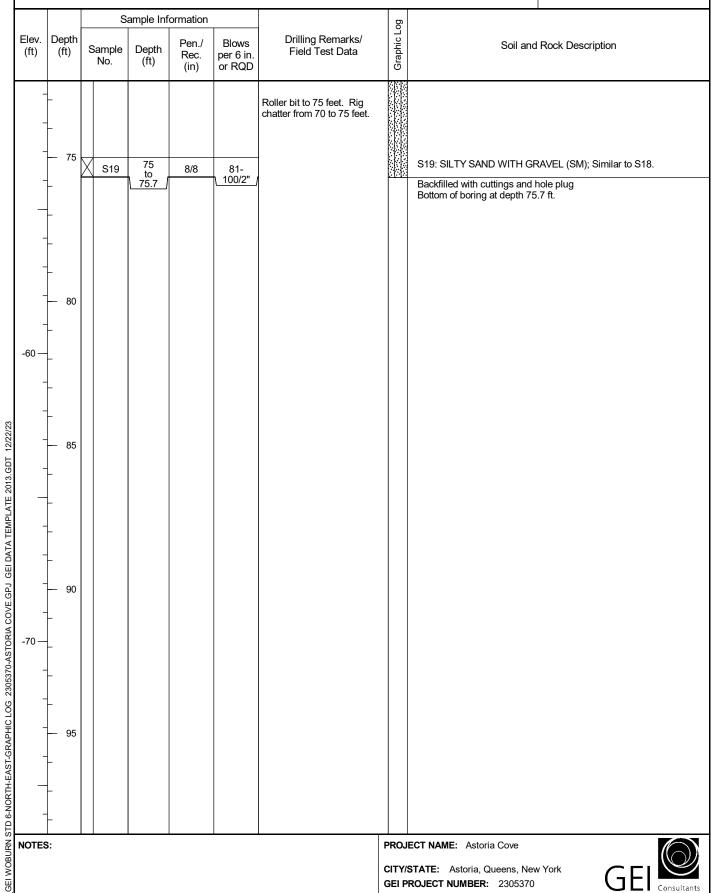
**EASTING (ft):** 1,003,368

**DATE START/END:** 6/8/2020 - 6/9/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B14** 

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					<b>MATION</b> 221,889	_				<b>EASTING (ft):</b> 1,003,5	34		BORING
						ft):21.9				DATE START/END:			D45
					<b>TUMS</b> : . : 93.0		3/NAD83 N	Y Zone 310	04_	DRILLING COMPANY: DRILLER NAME: Ke			B15
					. <u> </u>				_	RIG TYPE: CME 75	illi Fai	enii	PAGE 1 of 4
				_									PAGE 1 01 4
					RMATIO								
					Autor NA / I				_	DRILL ROD O.D.: N			RREL TYPE: NX Wireline RREL I.D./O.D.: 2.125 inch / 3 inch
						lud Rotary	Wash		_	THE ROD C.D		OONE DAI	2.120 mon / 0 mon
	WATE	R LE	VE	L DE	PTHS (	ft):							
	ABBRI	EVIA	TIO	NS:	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	.,%	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
					S	ample Inf	ormation				g		
	Elev.	De	pth				Pen./	Blows		Drilling Remarks/	Graphic Log	Soil and	Rock Description
	(ft)	(f	t)		ample No.	Depth (ft)	Rec.	per 6 in.		Field Test Data	raph	Soli and	Nock Description
						. ,	(in)	or RQD			٥		(CLASS 7) at ~0 ft / El. 21.9
	20 —	_		$\bigvee$	S1	0 to 2	24/13	14-10-9- 4		= 0.0ppm @S1		nonplastic fines, ~10% subar reddish-brown; dry. Brick fra STRATUM IIA - LOOSE \$	SAND (CLASS 6) at ~2 ft / El. 19.9
	_	_		$\bigvee$	S2	2 to 4	24/19	7-5-4-4	PID	= 0.0ppm @S2			SAND WITH SILT (SP-SM); ~90% fine lastic fines; yellow-brown; dry.
	-	-		$\prod$	S3	4 to	24/8	2-2-3-5	PID	= 0.0ppm @S3	***	S3: NARROWLY GRADED S sand, ~5% nonplastic fines; t	SAND (SP); ~95% fine to medium
	-	-	5	IXI		6						Sana, 670 nonplastic lines, t	an brown, ary.
23	_			$/ \setminus$								STRATUM IIC - SAND	) (CLASS 3B) at ~6 ft / El. 15.9
EMPLATE 2013.GDT 12/22/23	_	_		$\bigvee$	S4	6 to 8	24/16	11-6-10- 6	PID	= 0.0ppm @S4		S4: NARROWLY GRADED S	SAND (SP); Similar to S3.
EMPLATE 20	_	-		M	S5	8 to 10	24/12	10-7-6-3	PID	= 0.0ppm @S5		S5: NARROWLY GRADED S sand, ~10% subrounded gra yellow-brown; dry.	SAND (SP); ~85% fine to medium vel up to 1 inch, ~5% nonplastic fines;
J GEI DATA ا	_	_	10	$\bigvee$	S6	10 to 12	24/16	4-4-6-3	PID	= 0.0ppm @S6			D WITH SILT (SW-SM); ~90% sand, v-brown to orange-brown; dry.
2305370-ASTORIA COVE.GPJ GEI DATA T	10 —	  -  -		/ \						e casing to 15 feet below le. Roller bit to 15 feet.	v	STRATUM III - TILL (	CLASS 3A) at ~13.5 ft / El. 8.4
2305370-AST	-	<u>-</u>	15		67	15	24/10	10.12	-			\$7: SILTY SAND (SM); 77.3	% sand, 35.6% nonplastic fines, 5.8%
-GRAPHIC LOG	_	  -  -		X	S7 	to 17	24/10	18-12- 20-16	_				1"; brown to black; wet. [GRAIN SIZE
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG	-	  -  -							Rolle	er bit to 20 feet.			
RN ST	NOTES	: S:				<u> </u>	<u> </u>	1			PRO	ECT NAME: Astoria Cove	
GEI WOBUF		-									CITY	STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,534

**DATE START/END:** 5/11/2020 - 5/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B15** 

PAGE 2 of 4

				Sa	ample Inf	ormation			бc	·
Ele (ff	ev. De	epth ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description
	-		$\bigvee$	S8	20 to 22	24/3	18-12- 15-14	PID = 0.0ppm @S8		S8: WIDELY GRADED SAND WITH GRAVEL (SW); ~70% sand, ~30% subangular gravel up to 1.5"; brown; wet.
								Roller bit to 25 feet. Rig chatter from 23 to 25 feet.		
	+	25	M	S9	25 to 27	24/3	20-20- 16-11	PID = 0.0ppm @S9		S9: Gravel stuck in shoe. subangular up to 1.5".
	- - -							Drive casing to 25 feet below grade. Roller bit to 30 feet. Intermittent rig chatter from 25 to 30 feet.	<b>/</b>	
-10	- - -	30	M	S10	30 to 32	24/13	24-44- 85-92	PID = 0.0ppm @S10		S10: WIDELY GRADED SAND WITH SILT AND GRAVEL (SW-SM); ~60% sand, ~30% subangular gravel up to 1.5", ~10% nonplastic fines; brown; wet.
TE 2013.GDT 12,	<u></u>							Roller bit to 35 feet. Rig chatter from 31 to 35 feet.		
GEI DATA TEMPLATE 2013.GDT 12/22/23	† †	35	M	S11	35 to 37	24/14	33-35- 45-61	PID = 0.0ppm @S11		S11: WIDELY GRADED SAND (SW); ~95% sand, ~5% subangular gravel up to 0.75"; tan-brown; wet.
2305370-ASTORIA COVE.GPJ	- -							Roller bit to 40 feet. Rig chatter from 38 to 40 feet.		
	- - -	40	X	S12	40 to 40.9	11/10	53- 120/5"	PID = 0.0ppm @S12		S12 (0-5"): WIDELY GRADED SAND (SW); Similar to S11. S12 (5"-10"): SILTY SAND (SM); ~70% fine to medium sand, ~20% nonplastic fines, ~10% subangular gravel up to 1.5"; brown; wet.
STD 6-NORTH-EAST-GRAPHIC LOG	_ _ _ _							Roller bit to 45 feet. Intermittent rig chatter and slow advancement from 41 t 45 feet.	0	
TD 6-NOR	+	45		S13	45 to 45.1	1/1	100/1"	PID = 0.0ppm @S13		S13: SILTY SAND (SM); Similar to S12 (5"-10"); black and white; micaceous sand.
	TES:		ı						CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New York  ROJECT NUMBER: 2305370  GEI  Consultants



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,534

**DATE START/END:** 5/11/2020 - 5/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B15** 

PAGE 3 of 4

			S	ample Inf	ormation			ğ		
Elev. (ft)	Depth (ft)	Sa	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
	<del>-</del>						Roller bit to 50 feet.			
-	- 50 -	· ×	S14 J	50 to 50.2	2/2	100/2"	PID = 0.0ppm @S14		S14: SILTY SAND WITH GR sand, ~30% nonplastic fines, orange-brown; wet.	AVEL (SM); ~50% fine to medium ~20% subangular gravel up to 0.75
-30 —	-						Roller bit to 55 feet. Rig chatter from 51 to 52 feet.		STRATUM IV - DECOMPO	SED/WEATHERED ROCK (CLASS 52.6 ft / El30.7
- - -	55 55	X	S15	55 to \_55.9/	11/11	54- 100/5"	PID = 0.0ppm @S15		S15: SILTY SAND (SM); ~60 ~35% nonplastic to low plasti 1"; brown; wet.	% fine to medium micaceous sand, city fines, ~5% angular gravel up to
-	<del>-</del> -						Roller bit to 60 feet.			
-40	— 60 −		S16	60 to 62	24/16	23-22- 27-56	PID = 0.0ppm @S16		S16: SILTY SAND (SM); Sim 1.5"; yellow-brown to brown.	ilar to S15. subangular gravel up to
-	<del>-</del>						Roller bit to 65 feet. Rig chatter from 62 to 65 feet.			
-	— 65 –		S17	65 to 65.1	1/0	100/1"	PID = 0.0ppm @S17		S17: No Recovery.	
=	-  -						Roller bit to 70 feet. Rig chatter / slow advancement from 65 to 70 feet.			
-	70 -		S18	70 to 70	0/0	103/0"	PID = 0.0ppm @S18		S18: No Recovery.	
-50 —	<u> </u>							A A A	TOT MANE. A	
NOTES	):							CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultar



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,534

**DATE START/END:** 5/11/2020 - 5/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

## **BORING B15**

PAGE 4 of 4

									PAGE 4 of 4
		S	ample Inf	ormation			go		
Elev. (ft)	Depth (ft)	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
1	- - - 75 -					Rollerbit to 70 feet. Rig chatter / slow advancement from 70 to 75 feet.  PID = 0.0ppm @S19			
-	-	S19	75 to 75	0/0	36/0"	Rollerbit to 78 feet. Slow advancement from 75 to 78 feet.		S19: No Recovery.	
-60	- - 80 -	C1	78 to 83	60/50	0%	leet.		grained; banded; completely Horizontal fractures at 3", 4", 21"-24", 27", 29", 31"-33", 3 Core run times per foot (minu and 4:16.	6", 9", 10", 11-15", 17.5", 19", 5", 36", 39", 41"-44", 45.5" and 48". Ites:seconds): 5:05, 5:17, 4:15, 4:52
	- - 85 -	C2	83 to 88	60/54	48%			C2: HARRISON FORMATIO grained; banded;slight to mo Horizontal fractures at 0 to 1.	(CLASS 1B/1C) at ~83 ft / El61. N; Gneiss; hard; fine to medium derate weathering; black and white. 5", 5.5", 8", 9", 12", 17.5", 20.5", 43.5", and 48.5". Core run times p 7:33, 6:56, 6:56 and 6:28.
-70	- - 90 -	C3	88 to 93	60/60	100%			9", 15.5", 20.5", 27.5", 32", 3	N; Similar C1. Horizontal fractures 6", 42.5", 50.5", and 56.5". Core ru nds): 5:02, 6:47, 6:45, 6:27 and 4:44
    	- - 95 - -							Backfilled with cuttings and h Bottom of boring at depth 93	ole plug. ft.
NOTES:	:						CITY/	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GFI

#### BORING INFORMATION **BORING** NORTHING (ft): 221,904 **EASTING (ft):** 1,003,499 GROUND SURFACE EL. (ft): **DATE START/END:** 3/2/2020 - 3/3/2020 21.9 **B16(OW)** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 102.0 DRILLER NAME: Joe Schuster LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 5 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NX Wireline AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: 2.125 inch / 3 inch DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ▼ 17.3 3/10/2020 7:10 am in observation well ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) or RQD (in) STRATUM I - FILL at ~0 ft / El. 21.9 S1 (0"-2"): Asphalt. S1 (2"-12"): Concrete. PID = 0.0ppm @S1 0 S1 24-44/3' to 0.8 20 PID = 0.0ppm @S2 S2: SILTY SAND WITH GRAVEL (SM); $\sim$ 50% fine to medium sand, $\sim$ 30% subangular gravel up to 1", $\sim$ 20% nonplastic fines; brown; dry. Brick, asphalt and concrete fragments. 19-10-S2 24/7 to 13-8 STRATUM IIC - SAND (CLASS 3B) at ~4 ft / El. 17.9 PID = 0.0ppm @S3S3: NARROWLY GRADED SAND (SP); ~85% fine to medium S3 24/12 4-4-4-5 to sand, ~10% low-plasticity fines, ~5% fine angular gravel up to 0.5"; 5 light brown to reddish-brown; dry. PID = 0.0ppm @S4 6 S4: NARROWLY GRADED SAND (SP); ~95% fine to medium S4 24/10 5-7-4-5 to 8 sand, ~5% low-plasticity fines; light brown to reddish-brown; dry. PID = 0.0ppm @S5 8 S5: SILTY SAND (SM); ~75% fine to medium sand, ~25% 5-4-9-12 S5 24/8 to 10 nonplastic fines; brown; dry. Drive 8LF casing. Roller bit to 8 feet. STRATUM III - TILL (CLASS 3A) at ~10 ft / El. 11.9 10 PID = 0.0ppm @S6 S6: SILTY SAND (SM); ~65% fine to medium sand, ~25% 10 11-30-S6 24/15 to 12 nonplastic fines, ~10% angular gravel up to 1"; brown; dry. 19-26 10 Rig chatter from 13 to 14 feet. Rollerbit PID = 0.0ppm @C1 C1: No Recovery. Roller bit was grinding and not advancing. Begin C1 12/0 0% coring. After 1' core barrel advanced quickly. Likely cobble. Core to 15 run times per foot (minutes:seconds): 2:04. 15 Rig chatter from 18 to 20 S7: SILTY SAND WITH GRAVEL (SM); ~60% fine to medium S7 24/12 8-21-33feet. to 18 sand, ~30% nonplastic fines, ~10% subangular gravel up to 1.5"; 22 Crushed rock/cobble @11"-12"; brown; moist.

NOTES:

GEI DATA TEMPLATE 2013.GDT

LOG 2305370-ASTORIA COVE.GPJ

STD 6-NORTH-EAST-GRAPHIC

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PROJECT NAME: Astoria Cove



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,499

**DATE START/END:** 3/2/2020 - 3/3/2020

DRILLING COMPANY: Craig Geotechnical Drilling

# **BORING B16(OW)**

PAGE 2 of 5

										PAGE 2 of 5
		Sample Information						go		
Elev. (ft)	Depth (ft)	Sample No.		Depth F	Pen./ Rec. (in)	c. per 6 in.	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description	
0-	_		S8	20 to 22	24/8	12-13- 15-14	PID = 0.0ppm @S8		fine to medium sand with mid	SAND WITH GRAVEL (SP); ~75% a flakes, ~20% angular to ", ~5% nonplastic fines; brown to
=	<u>-</u>						Rollerbit to 25 feet. Rig chatter from 22 to 25 feet.			
-	25 	M	S9	25 to 27	24/14	30-16- 16-15	PID = 0.0ppm @S9		S9: NARROWLY GRADED S8; except crushed rock pow	SAND WITH GRAVEL (SP); SImilar der and fragments @0-6".
-	<u>-</u> - -	/ \					Drive casing to 18 feet due t fall in. Rollerbit to 30 feet. Rig chatter from 25 to 26 feet.	О		
-10 —	— 30 - -	M	S10	30 to 32	24/22	28-63- 69-101	PID = 0.0ppm @S10		S10: NARROWLY GRADED sand, ~5% angular to subrounonplastic fines; brown; wet.	SAND (SP); ~90% fine to medium inded gravel up to 0.75", ~5%
-	- - -						Roller bit to 35 feet.			
<del>-</del>	— 35 —	X	S11	35 to 36.4	17/13	64-47- 103/3"	PID = 0.0ppm @S11		S11: NARROWLY GRADED	SAND (SP); Similar to S10.
-	- - -						Roller bit to 40 feet.			
- -	— 40 —	X	S12	40 to 40.4	5/4	106/5"	PID = 0.0ppm @S12		S12: NARROWLY GRADED fine to medium sand, ~10% I angular to subrounded grave	SAND WITH CLAY (SP-SC); ~85% ow plasticity clayey fines, ~5% I up to 0.75"; brown; wet.
-20 — -	<u>-</u> - -						Roller bit to 45 feet. Rig chatter at 41 feet.			
<del>-</del>	— 45 -	X	S13	45 to 47	24/19	60-64- 70-31	PID = 0.0ppm @S13		(GW-GM); 47.7% angular to	AVEL WITH SILT AND SAND subrounded gravel up to 2", 45.2% brown; wet. [GRAIN SIZE TEST
NOTES	<b>S</b> :	v V						CITY/S	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

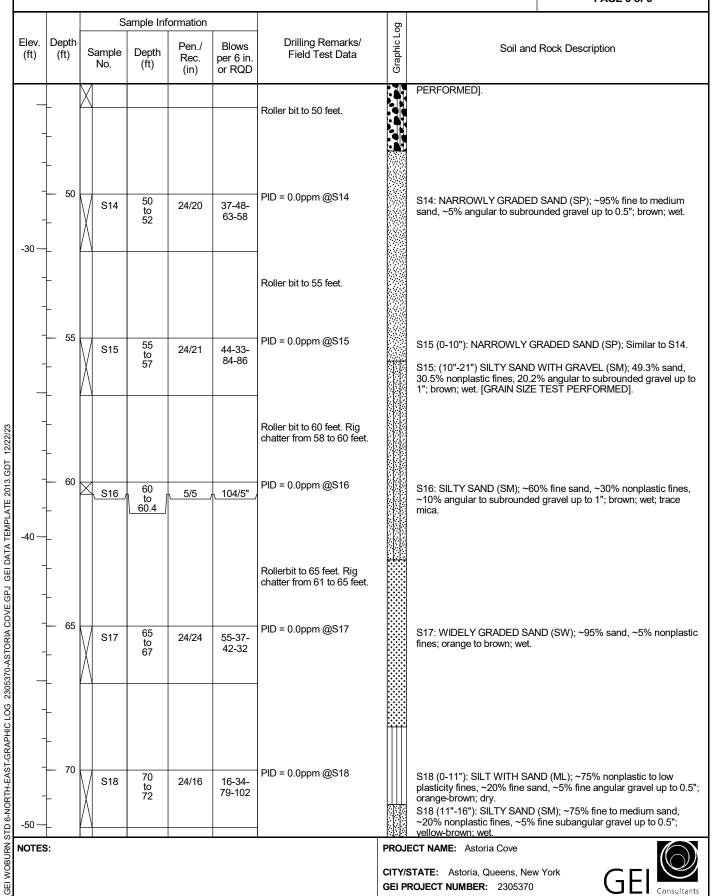
**EASTING (ft):** 1,003,499

**DATE START/END:** 3/2/2020 - 3/3/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B16(OW)

PAGE 3 of 5



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

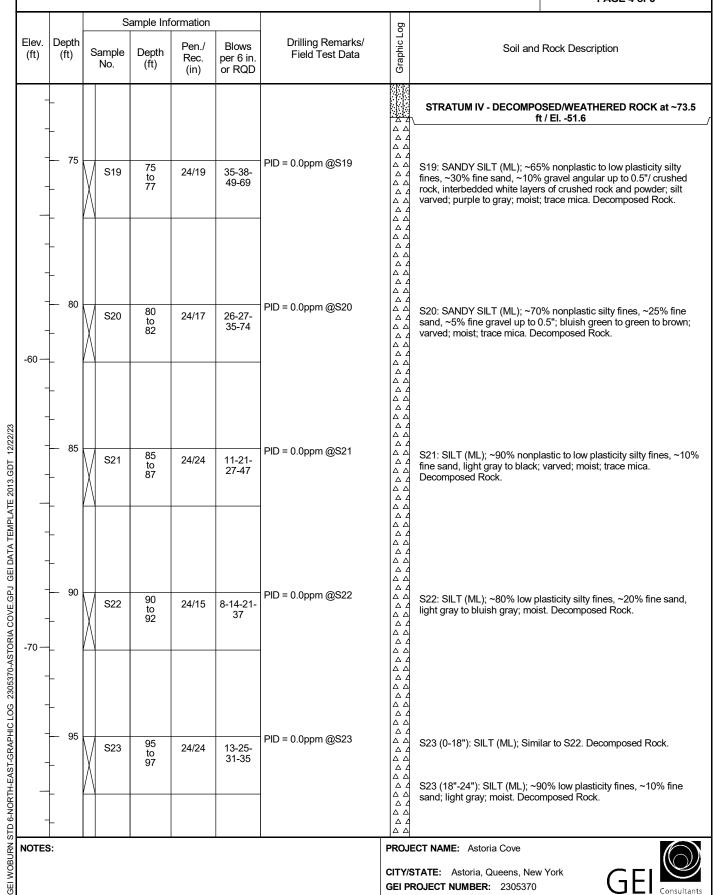
**EASTING (ft):** 1,003,499

**DATE START/END:** 3/2/2020 - 3/3/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B16(OW)

PAGE 4 of 5



GROUND SURFACE EL. (ft): 21.9

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

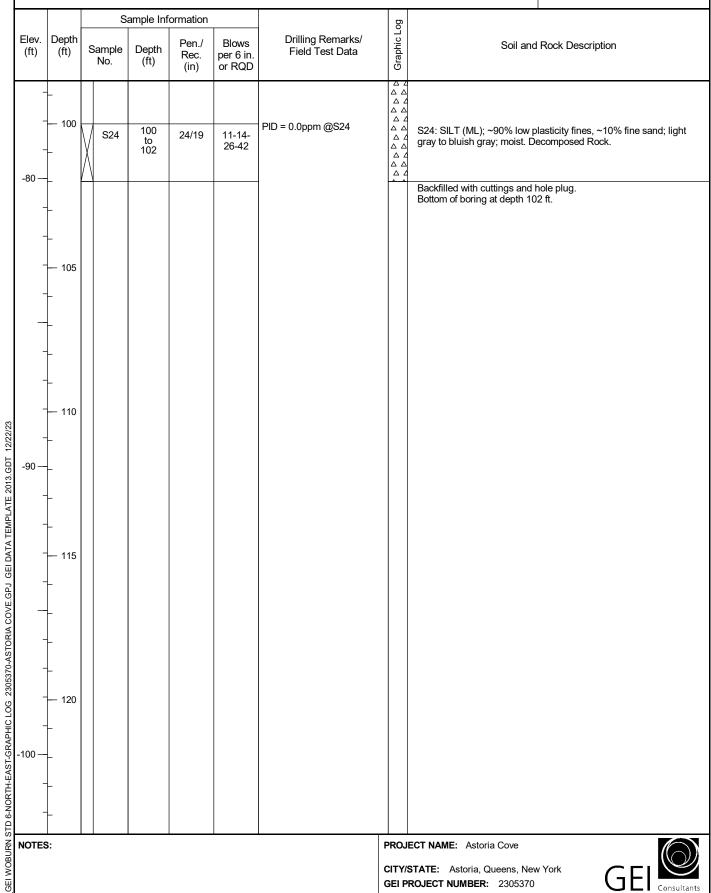
**EASTING (ft):** 1,003,499

**DATE START/END:** 3/2/2020 - 3/3/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

## **BORING B16(OW)**

PAGE 5 of 5



BORING INFORMATION **BORING** NORTHING (ft): 221,948 **EASTING (ft):** 1,003,379 GROUND SURFACE EL. (ft): **DATE START/END:** 6/11/2020 - 6/12/2020 21.5 **B17** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 75.3 DRILLER NAME: Nick Beehlaer LOGGED BY: A. Erb / G. Holmes RIG TYPE: CME 55LC PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: Automatic **CASING I.D./O.D.:** 4.125 inch/ 4.375 inch CORE BARREL TYPE: NA AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: NA / NA DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.5 (0-1'): Concrete. STRATUM IIC - SAND (CLASS 3B) at ~1 ft / El. 20.5 PID = 0.0ppm @S1 S1: SILTY SAND WITH GRAVEL (SM); ~60% fine sand, ~20% S1 12/10 23-16 20 to nonplastic fines, ~20% angular gravel up to 1.5"; brown; dry. Concrete and brick fragments. PID = 0.0ppm @S2 S2: NARROWLY GRADED SAND (SP); ~90% fine to medium S2 24/24 11-9-7to sand, ~5% nonplastic fines, ~5% fine gravel; brown to 13 Roller bit to 4 feet below orange-brown; dry. arade. PID = 0.0ppm @S3S3(0-7"): NARROWLY GRADED SAND (SP); Similar to S2. S3(7"-13"): NARROWLY GRADED SAND (SP); ~95% fine to 5-5-7-9 S3 24/13 to 5 medium sand, ~5% fine gravel; white to tan; dry. PID = 0.0ppm @S4 6 S4: NARROWLY GRADED SAND (SP); Similar to S3 (7"-13"). 24/8 11-14to 8 14-17 Roller bit to 8 feet below grade. PID = 0.0ppm @S5 8 S5: SILTY SAND (SM); ~70% fine sand, ~30% nonplastic fines; 24/10 5-4-7-8 S5 to 10 tan; dry. Trace mica. Drive casing to 10 feet below grade. Roller bit to 10 feet. 10 PID = 0.0ppm @S6 S6: NARROWLY GRADED SAND WITH SILT (SP-SM); ~85% fine 10 S6 24/10 13-9-11to 12 to medium sand, ~10% nonplastic fines, ~5% fine gravel; tan to brown; dry. Trace mica. 10 Roller bit to 15 feet. STRATUM III - TILL (CLASS 3A) at ~13.5 ft / El. 8 15 PID = 0.0ppm @S7 S7: NARROWLY GRADED SAND (SP); ~95% fine to medium S7 24/15 31-47to 17 sand, ~5% fine gravel; brown to black; dry. Black micaceous sand 25-20 @7"-14". Drive casing to 15 feet below grade. Roller bit to 20 feet. PROJECT NAME: Astoria Cove

NOTES: Sample ID S12 was skipped during drilling.

GEI DATA TEMPLATE 2013.GDT

STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ

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GROUND SURFACE EL. (ft): 21.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

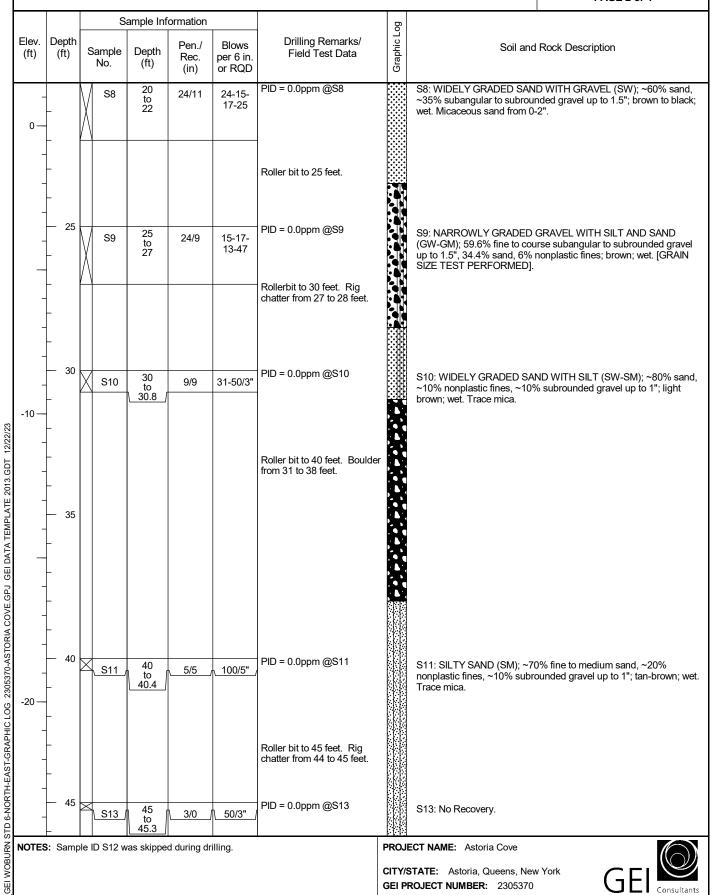
**EASTING (ft):** 1,003,379

**DATE START/END:** 6/11/2020 - 6/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

### **BORING B17**

PAGE 2 of 4



NOTES: Sample ID S12 was skipped during drilling.

PROJECT NAME: Astoria Cove



GROUND SURFACE EL. (ft): 21.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

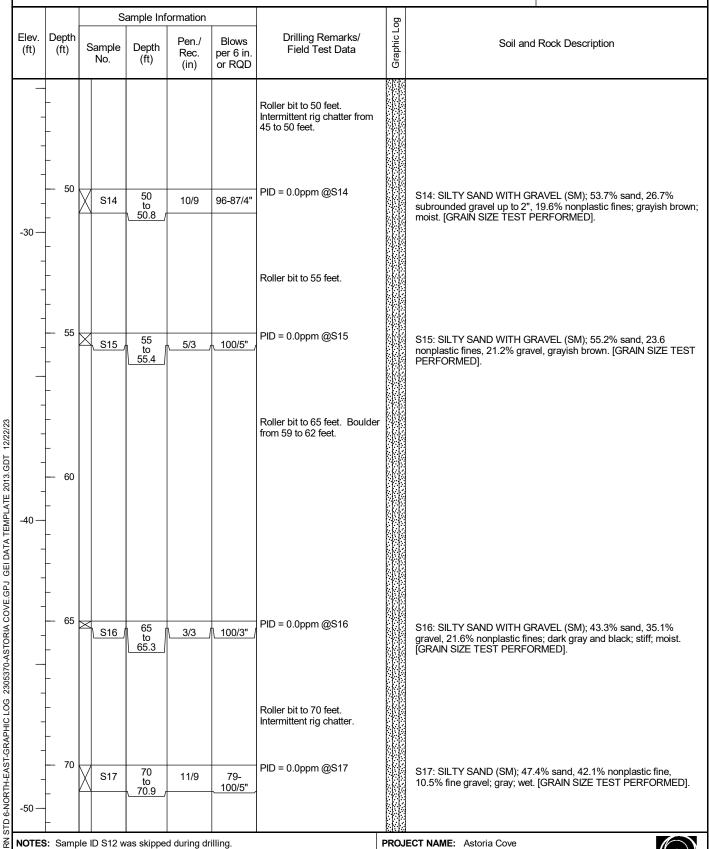
**EASTING (ft):** 1,003,379

**DATE START/END:** 6/11/2020 - 6/12/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B17

PAGE 3 of 4



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

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GROUND SURFACE EL. (ft): 21.5

**VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

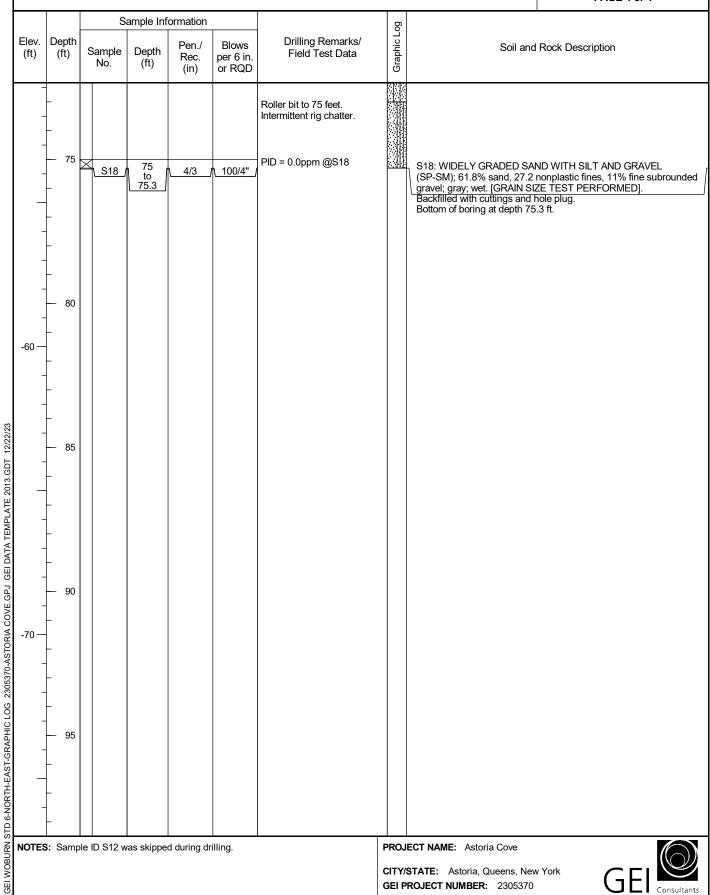
**EASTING (ft):** 1,003,379

**DATE START/END:** 6/11/2020 - 6/12/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

**BORING B17** 

PAGE 4 of 4



NOTES: Sample ID S12 was skipped during drilling.

PROJECT NAME: Astoria Cove



#### BORING INFORMATION **BORING** NORTHING (ft): 221,937 **EASTING (ft):** 1,003,535 GROUND SURFACE EL. (ft): **DATE START/END:** 5/8/2020 - 5/8/2020 20.6 **B18** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 100.2 DRILLER NAME: Keith Parent LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 5 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NA CORE BARREL I.D./O.D.: NA / NA AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 20.6 S1: WIDELY GRADED SAND WITH GRAVEL (SW); ~65% sand, PID = 0.0ppm @S1 0 6-3-2-15 S1 24/12 20 to 2 ~30% angular to subangular gravel up to 1.5", ~5% nonplastic fines; brown to orange-brown; dry. STRATUM IIC - SAND (CLASS 3B) at ~2 ft / El. 18.6 PID = 0.0ppm @S2 S2: NARROWLY GRADED SAND (SP); ~85% fine to medium S2 24/11 7-5-7-8 to sand, ~10% angular to subangular gravel up to 1", ~5% nonplastic fines; brown to dark brown; dry. PID = 0.0ppm @S3S3: NARROWLY GRADED SAND (SP); ~100% fine to medium S3 24/6 4-8-7-9 to sand; tan-brown; dry. 5 PID = 0.0ppm @S4 6 S4: NARROWLY GRADED SAND (SP); ~95% fine to medium 24/8 16-16-9to 8 sand, ~5% nonplastic fines; tan-brown; dry. PID = 0.0ppm @S5 S5: NARROWLY GRADED SAND WITH SILT (SP-SM); ~80% fine 8 S5 24/5 14-12-8to medium sand, ~10% nonplastic fines, ~10% subangular to to 10 8 subrounded gravel up to 1.5"; brown to reddish-brown; dry. Drive casing to 10 feet below grade. Roller bit to 10 feet. 10 PID = 0.0ppm @S6 S6: CLAYEY SAND (SC); ~70% fine to medium sand, ~20% low 10 12-10-S6 24/2 to 12 plasticity fines, ~10% subangular to subrounded gravel up to 1"; 10 10-24 tan to black; moist. Some micaceous sand. Roller bit to 15 feet. 15 S7: No Recovery. S7 100/4" to 15.3 Drive casing to 15 feet below grade. Roller bit to 20 feet. NOTES: PROJECT NAME: Astoria Cove

DATA TEMPLATE 2013.GDT

STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI

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GEI Consultants

GROUND SURFACE EL. (ft): 20.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,535

**DATE START/END:** 5/8/2020 - 5/8/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B18** 

PAGE 2 of 5

										PAGE 2 of 5
			S	ample Inf	ormation			go		
Elev. (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
0	_	$\bigvee$	S8	20 to 22	24/14	5-7-6-5	PID = 0.0ppm @S8		S8: SILTY SAND (SM); ~70% ~20% nonplastic fines, ~10% to 1"; black; wet.	6 fine to medium micaceous sand, 5 subangular to subrounded gravel u
-	_						Drive casing to 20 feet below grade. Roller bit to 25 feet. Rig chatter from 22 to 24 feet.	,	STRATUM III - TILL (0	CLASS 3A) at ~23.5 ft / El2.9
_	25 	V	S9	25 to 27	24/6	88-42- 52-23	PID = 0.0ppm @S9			D WITH GRAVEL (SW); ~70% sand ided gravel up to 1.5"; brown; wet.
-	- - - -	/\					Rollerbit to 30 feet. Rig chatter from 25 to 30 feet.			
-10 — -	30 		S10	30 to 30.1	1/0	100/1"	PID = 0.0ppm @S10		S10: No Recovery.	
-	- - - - - 35						Drive casing to 25 feet below grade. Roller bit to 35 feet. Rig chatter from 30 to 35 feet.	,		
-	_ 33	$\bigvee$	S11	35 to 37	24/12	34-34- 31-37	PID = 0.0ppm @S11		S11: WIDELY GRADED SAN ~20% subangular to subrour	ND WITH GRAVEL (SW); ~80% sar ided gravel up to 1"; light brown; we
-	- -						Roller bit to 40 feet.			
-20 — -	— 40 —	×	S12	40 to 40.2	2/2	100/2"	PID = 0.0ppm @S12		S12: SILTY SAND WITH GR subangular to subrounded gr fines; brown; wet. [GRAIN SIZ	AVEL (SM); 52.9% sand, 32% avel up to 1.5"; 15.1% nonplastic ZE TEST PERFORMED].
-	_						Roller bit to 45 feet. Rig chatter from 42 to 45 feet.			
	45	M	S13	45 to 47	24/16	37-44- 56-53	PID = 0.0ppm @S13		S13: WIDELY GRADED SAN subrounded gravel up to 1"; t	ND (SW); ~95% sand, ~5% can-brown; wet.
NOTES	 S:							CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consulta



GROUND SURFACE EL. (ft): 20.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

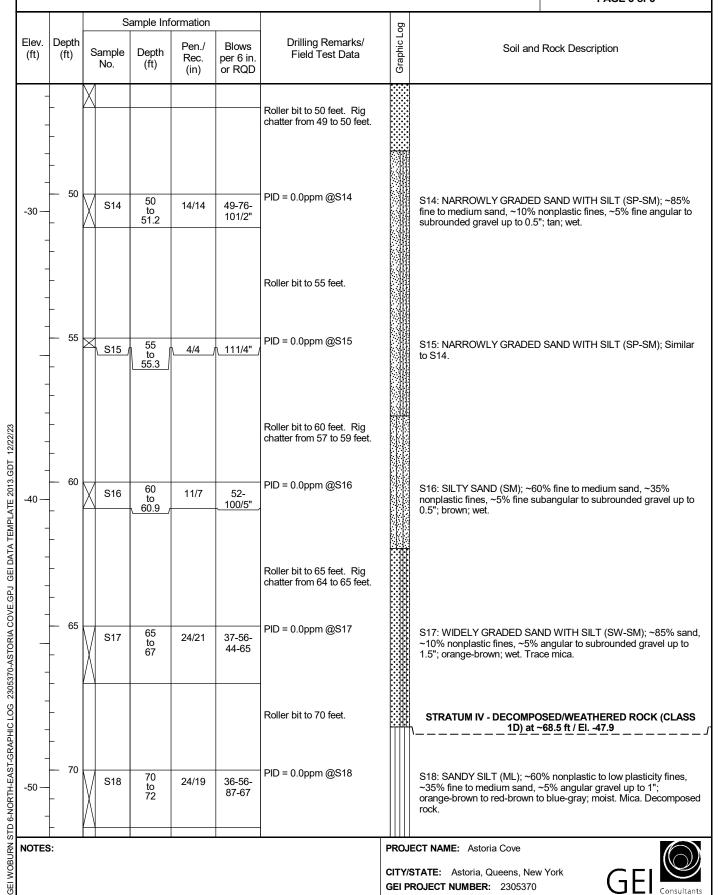
**EASTING (ft):** 1,003,535

**DATE START/END:** 5/8/2020 - 5/8/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B18

PAGE 3 of 5



GROUND SURFACE EL. (ft): 20.6

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

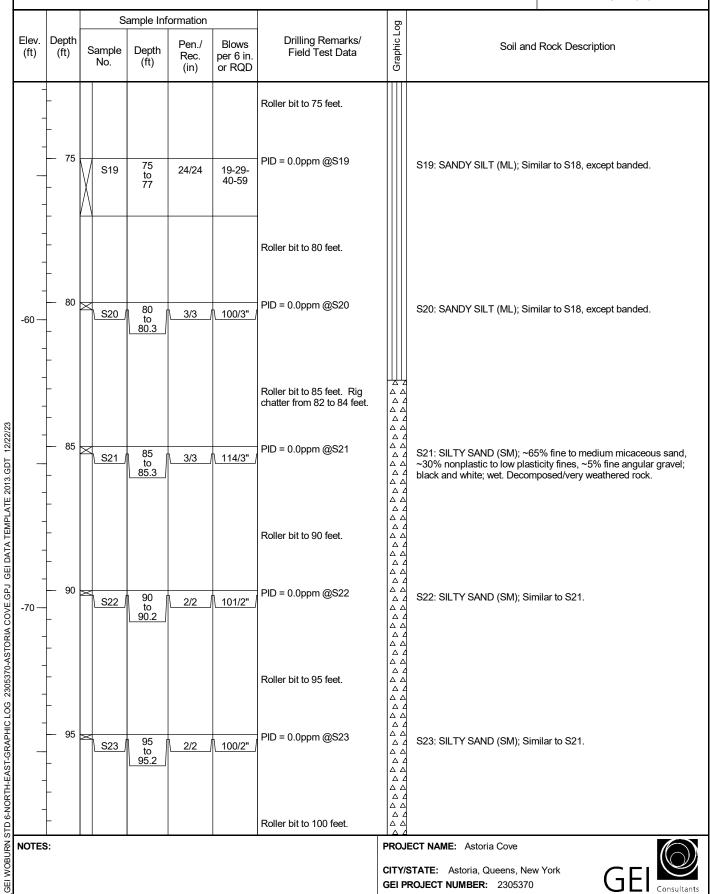
EASTING (ft): 1,003,535

DATE START/END: 5/8/2020 - 5/8/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B18** 

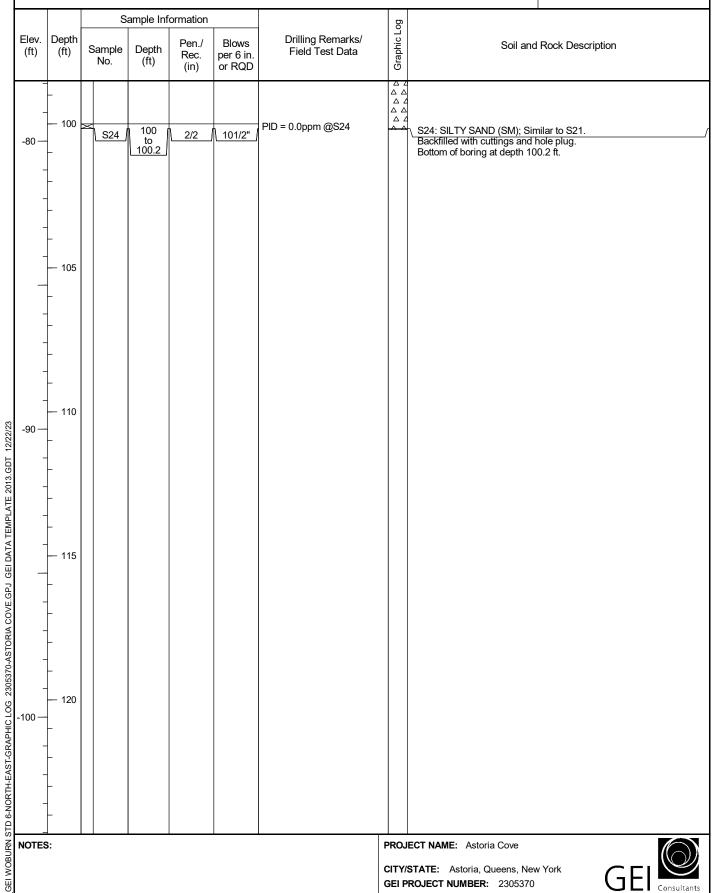
PAGE 4 of 5



NORTHING (ft): 221,937 **EASTING (ft):** 1,003,535 GROUND SURFACE EL. (ft): \_ 20.6 **DATE START/END:** 5/8/2020 - 5/8/2020

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling **BORING B18** 

PAGE 5 of 5



				<b>MATIOI</b> 221,997	_			<b>EASTING (ft):</b> 1,003,3	98		BORING
				•	ft): 21.4			DATE START/END:			D40
				.1UMS: ): 75.		B/NAD83 N	Y Zone 310	DRILLING COMPANY:  DRILLER NAME: Nic		<u> </u>	B19
			•		o / G. Holr	nes		RIG TYPE: CME 55LC			PAGE 1 of 4
	DRILL	ING IN	IFO	RMATIO	ON						
				Auton				CASING I.D./O.D.: _4		ch/ 4.375 inch CORE BAR	REL TYPE: NA
				NA /	NA lud Rotary	/Wash		DRILL ROD O.D.: N	И	CORE BAR	RREL I.D./O.D.: NA / NA
				EPTHS (		VVGSII					
	ABBRI	EVIATIO	ONS	Rec. RQD WOF	= Length of R = Weight	/ Length ality Designa f Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample ,,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.
				S	ample Inf	ormation			og		
	Elev. (ft)	Depth (ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description (CLASS 7) at ~0 ft / El. 21.4
				S1	0	0/0	50/3"	Roller bit through asphalt.			cated second concrete slab, offset
	20 —	- -		31	to 0	0/0	30/3	Roller bit through concrete slab at 3 feet. @S1		hole 4 feet to thé south.	,
	-	_						Roller bit through slab to 4 feet.			
	-	<u> </u>	X	∖_S2_,	4 to 4.4	5/5	100/5"_			(GP-GM); ~75% fine angular ~10% nonplastic fines; black	<b>5 7</b> ·
2/23			L					Roller bit to 6 feet. Drill	900		Y (CLASS 4B/5B) at ~6 ft / El. 15.4
3DT 12/2;	_	_	$\mathbb{N}$	S3	6 to 8	24/14	5-4-6-10	chatter from 4 to 6 feet.		sand; tan; dry. 3" seam of nar	~85% nonplastic fines, ~15% fine тоwly graded sand above silt.
2013.0			$\square$						2222		(CLASS 3B) at ~8 ft / El. 13.4
EMPLATE	-	-	$\bigvee$	S4	8 to 10	24	7-6-10- 19				SAND WITH GRAVEL (SP); ~85% ne angular gravel up to 1.5"; tan; dry.
.GPJ GEI DATA TEMPLATE 2013.GDT 12/22/23	10 —	— 10 -		S5	10 to 12	24/12	20-10- 10-10	Drive casing to 10 feet. Roller bit to 10 feet.		S5: SILTY SAND (SM); ~85% tan; moist.	6 fine sand, ~15% nonplastic fines;
STORIA COVE	-	_								STRATUM III - TILL (	CLASS 3A) at ~13.5 ft / El. 7.9
305370-A	-	- 15									
-GRAPHIC LOG 2	_	-		S6	15 to 17	24/12	17-14- 14-13	Roller bit to 15 feet. Drive casing to 15 feet.		S6: NARROWLY GRADED S to medium sand, ~10% nonp	SAND WITH SILT (SP-SM); ~90% fine lastic fines; brown; wet.
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ	- -	_									
URN 8	NOTES	3:							PROJ	ECT NAME: Astoria Cove	
GEI WOBI										STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consultants

GEI Consultants

GROUND SURFACE EL. (ft): 21.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

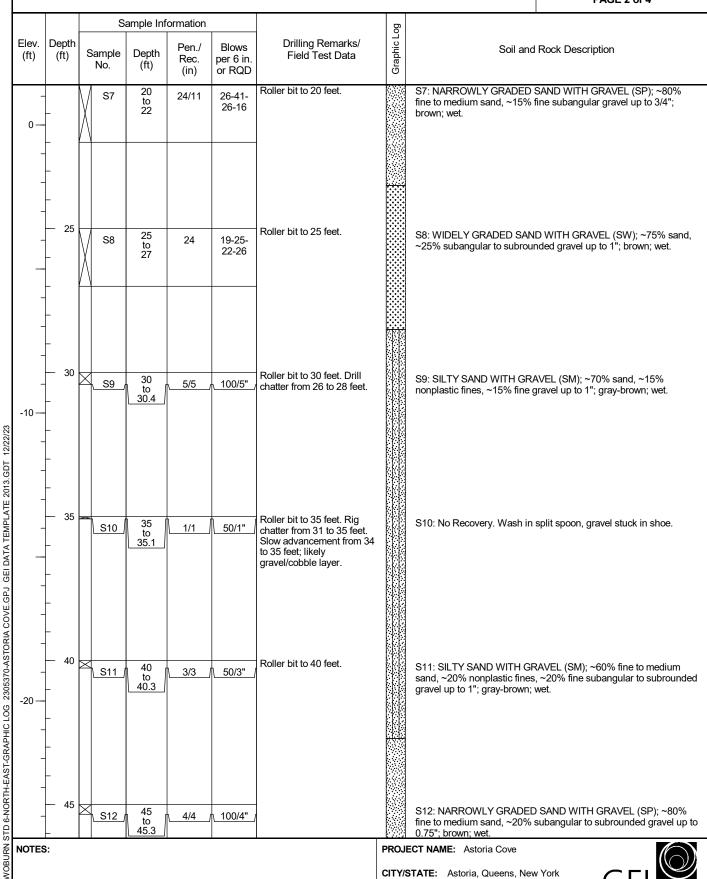
**EASTING (ft):** 1,003,398

**DATE START/END:** 6/10/2020 - 6/11/2020

DRILLING COMPANY: Craig Geotechnical Drilling

### **BORING B19**

PAGE 2 of 4



GEI PROJECT NUMBER: 2305370

GEI WOBURN

GROUND SURFACE EL. (ft): 21.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

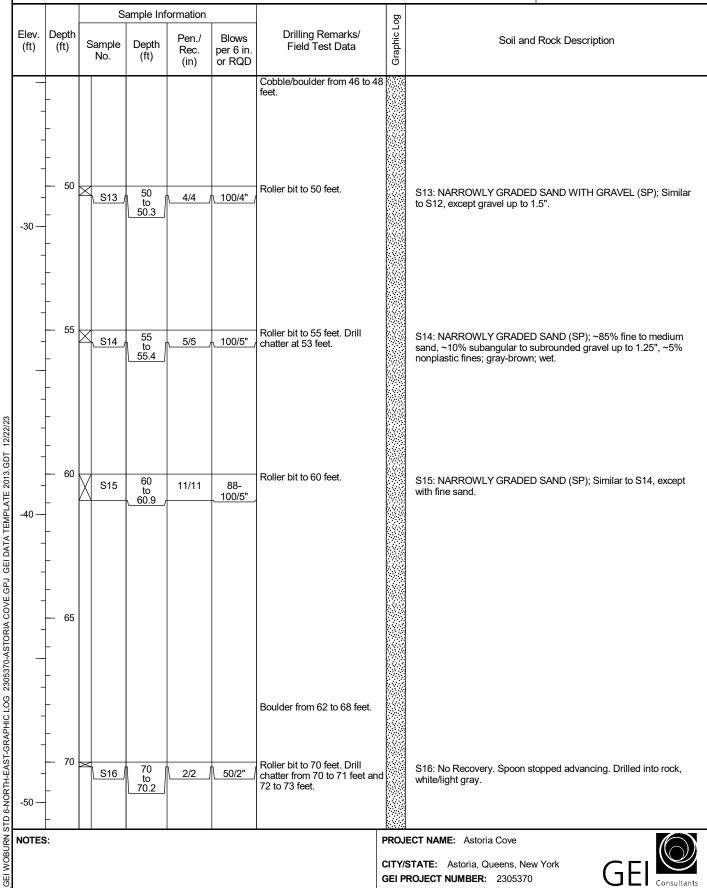
**EASTING (ft):** 1,003,398

**DATE START/END:** 6/10/2020 - 6/11/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B19** 

PAGE 3 of 4





NORTHING (ft): 221,997 **EASTING (ft):** 1,003,398 **BORING B19** 

PAGE 4 of 4



Sample Information Graphic Log Drilling Remarks/ Elev. Depth Pen./ Blows Soil and Rock Description Sample Depth (ft) (ft) Field Test Data per 6 in. Rec. No. (ft) or RQD (in) 75 Roller bit to 75 feet. 75 to 75.4 S17: NARROWLY GRADED SAND WITH SILT (SP-SM); ~80% S17 100/5" 5/5 fine to medium sand, ~10% nonplastic fines, ~10% subangular to subrounded gravel up to 1.5"; brown to yellow; wet.

Backfilled with cuttings and hole plug.
Bottom of boring at depth 75.4 ft. 80 -60 GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT 12/22/23 85 90 -70 95 PROJECT NAME: Astoria Cove NOTES:



	NORTH GROU VERT TOTAL LOGGI DRILL HAMM AUGEI DRILL	HING ND S /HOF DE ED E LING ERD	G (ft) GUR RIZ. PTH BY: G INI GINE	E: _2 FAC DAT I (ft): _G FOF E: CHO	RMATIC Autom NA / I D: M Pen. Rec. RQD	This 20.2 NAVD 88 In the ses In t	B/NAD83 N		DRILLING COMPANY: DRILLER NAME: Nic RIG TYPE: CME 55LC  CASING I.D./O.D.: 4. DRILL ROD O.D.: NM  S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample	6/9/202 Crai k Beel ;	ch/ 4.375 inch CORE BARI	BORING  B20  PAGE 1 of 4  REL TYPE: NA REL I.D./O.D.: NA / NA  NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D.
		ı			WOF	R = Weight		es>4 in / Pen	.,% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Diameter/	split spoon sampler.
	Elev. (ft)	De (f			Sample No.	Depth	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description  CLASS 7) at ~0 ft / El. 20.2
	20 —	_			S1	0 to 2	24/16	9-14-21- 32	PID = 1.1 ppm @S1 PID = 0.1 ppm @S2		(0-4"): Concrete. S1: NARROWLY GRADED S (SP-SM); ~70% fine to mediul gravel up to 1.25", ~10% nonpetroleum-like odor.	AND WITH SILT AND GRAVEL m sand, ~20% angular to subangular olastic fines; brown to tan; dry. Slight
	_			$\bigvee$	S2	to 4	24/21	47-21- 36-24	Roller bit to 4 feet. Rig	•	sand, ~5% nonplastic fines, ~ up to 0.75"; light to dark brown	
/23	_		5	$\bigvee$	S3	4 to 6	24/10	7-16-16- 10	chatter from 3-4 feet. PID = 0.0 ppm @S3		(SP-SM); ~70% fine to mediui gravel up to 1", ~10% nonplas brick fragments.	AND WITH SILT AND GRAVEL m sand, ~20% angular to subangular stic fines; dark brown; dry. Coal and
13.GDT 12/22	-	_		$\bigvee$	S4	6 to 8	24/8	19-11-7- 12	PID = 0.0 ppm @S4		(SP-SM); Similar to S3.	AND WITH SILT AND GRAVEL  CLASS 3A) at ~8 ft / El. 12.2
GEI DATA TEMPLATE 2013.GDT 12/22/23	-			M	S5	8 to 10	24/9	5-16-26- 14	PID = 0.0 ppm @S5		S5: SILTY SAND WITH GRAN nonplastic fines, ~20% suban 1.5"; tan to brown; dry.	/EL (SM); ~60% sand, ~20% gular to subrounded gravel up to
OVE.GPJ GEI DATA	10 —	_	10		S6	10 to 12	24/10	19-14- 15-10	Drive casing 10 feet below grade. PID = 0.0 ppm @S6			WITH GRAVEL (SW); ~65% sand, ded gravel up to 1.5", ~5% nonplastic y. Some micaceous sand.
JG 2305370-ASTORIA C	-	_	15	\/	S7	15 to	24/6	11-18-	Drive casing to 15 feet below grade. Roller bit to 15 feet.	v		AND (SP); ~85% fine to medium el up to 1", ~5% nonplastic fines;
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ	- - -					17		19-26	PID = 0.0 ppm @S7		light brown; wet.	Constitution
<b>3EI WOBURN S</b>	NOTES	S:		•						CITY/	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consultants



GROUND SURFACE EL. (ft): 20.2

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,426

**DATE START/END:** 6/9/2020 - 6/9/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B20** 

PAGE 2 of 4

			S	ample Inf	ormation			0	l	
Elev. (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
0-	-	M	S8	20 to 22	24/9	27-24- 47-21	Roller bit to 20 feet. Rig chatter from 19 to 20 feet. PID = 0.0 ppm @S8		S8: NARROWLY GRADED S	SAND (SP); Similar to S7.
-	-						Roller bit to 25 feet. Rig chatter at 22 feet and from 2 to 24 feet.	23		
-	25	$\bigvee$	S9	25 to 27	24/12	27-22- 20-16	PID = 0.0 ppm @S9		S9: WIDELY GRADED SAND ~20% subrounded gravel up	O WITH GRAVEL (SW); ~80% san to 1.25"; light brown; wet.
-	_						Roller bit to 30 feet. Rig chatter/slow advance from 2 to 30 feet.	29		
-10 — - -	30		C1	30 to 35	60/17				C1: BOULDER. Core run time 01:16, 01:12, 00:33, 00:26.	es per foot (minutes:seconds): 01:
- - -	35		S10 J	35 to 35.3	\ <u>4/4</u>	51/4"	PID = 0.0 ppm @S10	**************************************	S10: SILTY SAND WITH GR. nonplastic fines, ~20% angulto yellow-brown; wet.	AVEL (SM); ~60% fine sand, ~20% ar to subangular gravel up to 1"; ta
-							Roller bit to 40 feet. Rig chatter from 38 to 40 feet.			
-20 — -	40	X	S11 J	40 to 40.3	\ 4/4	100/4"	PID = 0.0 ppm @S11		S11: WIDELY GRADED SAN micaceous sand, ~10% nonp brown; wet.	ID WITH SILT (SW-SM); ~85% lastic fines, ~5% fine gravel; black
-	_						Roller bit to 45 feet. Rig chatter from 43 to 45 feet.			
	— 45 –	X	S12 )	45 to 45.4	5/5	100/5"	PID = 0.0 ppm @S12		S12: SILTY SAND WITH GRA subangular to subrounded gra fines; gray-brown; wet.	AVEL (SM); ~50% fine sand, ~30% avel up to 1.5", ~20% nonplastic
NOTES	S:							CITY/S	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEL Consult



GROUND SURFACE EL. (ft): 20.2

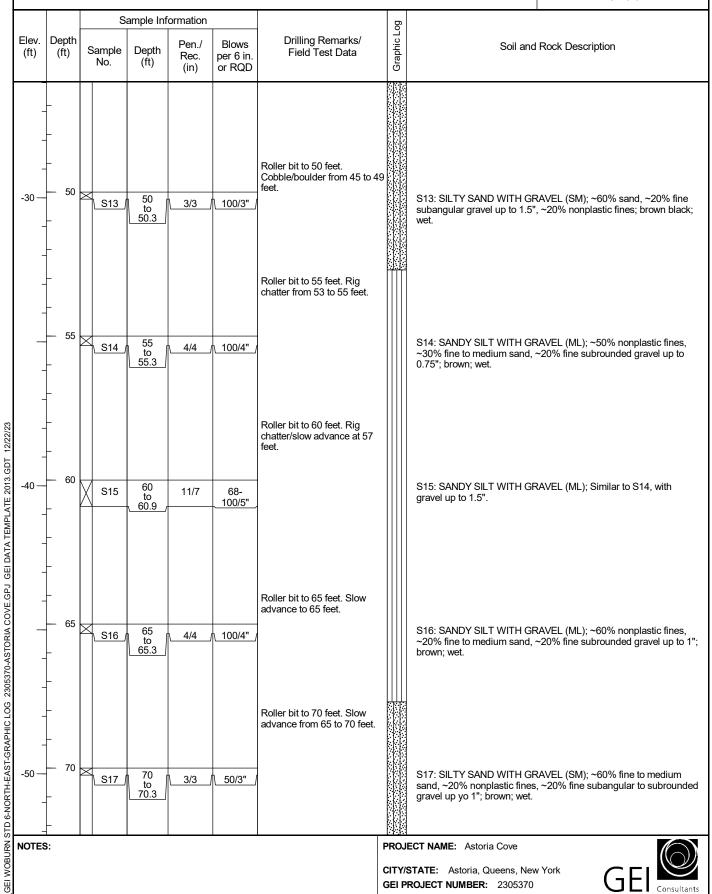
VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,426

**DATE START/END:** 6/9/2020 - 6/9/2020

**BORING B20** DRILLING COMPANY: Craig Geotechnical Drilling

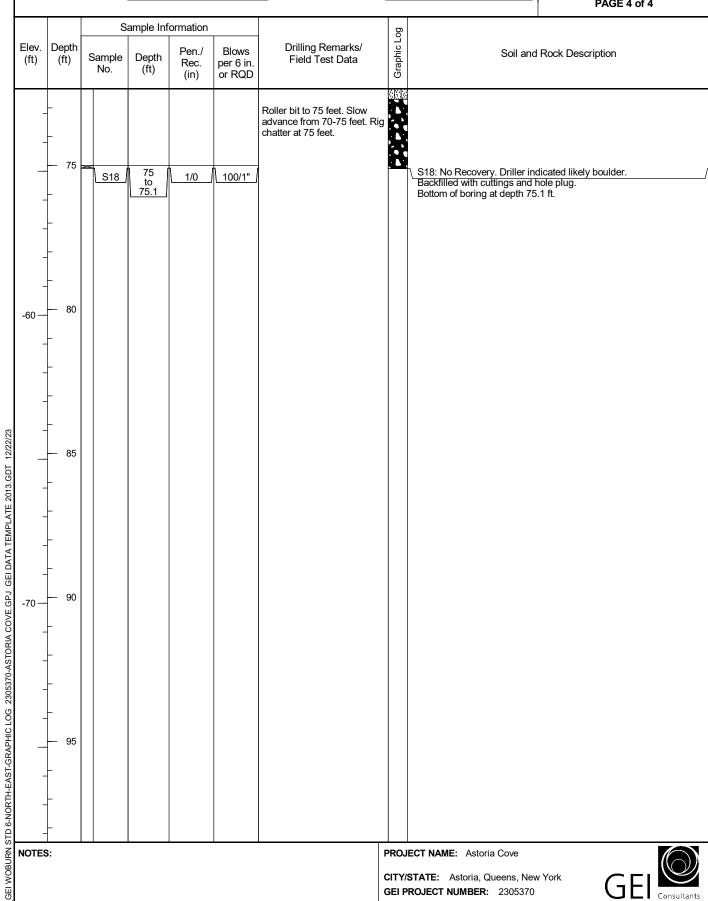
PAGE 3 of 4



NORTHING (ft): 222,066 **EASTING (ft):** 1,003,426 GROUND SURFACE EL. (ft): 20.2

**DATE START/END:** 6/9/2020 - 6/9/2020 **VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104 **DRILLING COMPANY:** Craig Geotechnical Drilling **BORING B20** 

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					MATION	_							BORING
			٠,	_	22,090 EFFL (1	ft): 14.0	)			DATE START/END:		020 - 5/13/2020	DOMINO
								IY Zone 310	04	-		aig Geotechnical Drilling	B21
T	OTAL	DEF	тн	(ft)	:71.4	4				DRILLER NAME: Ed	l Flana	ngan	
L	OGGI	ED B	<b>/</b> :	G	. Holme	es			_	RIG TYPE: CME 75			PAGE 1 of 3
D	RILL	ING	INF	OF	RMATIO	ON							
					Autom					CASING I.D./O.D.: 4	inch/	4.5 inch CORE BAF	RREL TYPE: NA
					NA / I					DRILL ROD O.D.: N	М	CORE BAR	RREL I.D./O.D.: NA / NA
					D: <u>M</u> PTHS (	lud Rotary	Wash						
L				_									
A	BBRI	EVIAT	'IOI	IS:	Rec. RQD WOF	= Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	.,%	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside I	30 inches to drive a 2-inch-O.D. split spoon sampler.
					S	ample Inf	ormation				g		
E	lev.	Dep	th				Pen./	Blows		Drilling Remarks/	Graphic Log	Soil and	Pools Description
(	ft)	(ft	)		ample No.	Depth (ft)	Rec.	per 6 in.		Field Test Data	raph	Soli aliu	Rock Description
						( )	(in)	or RQD			Ö	STRATUMIT-FILL	_ (CLASS 7) at ~0 ft / El. 14
	-	_		$\sqrt{}$	S1	0 to 2	24/12	16-13- 12-15	PID	= 0.0ppm @S1		S1: SILTY SAND WITH GRAsand, ~30% nonplastic fines dark brown to black; dry.	NEL (SM); ~50% fine to medium , ~20% subangular gravel up to 1.5";
	-	_		$\sqrt{}$	S2	2 to 4	24/10	27-19- 13-11	PID	= 0.0ppm @S2		S2: SILTY SAND WITH GRA	AVEL (SM); Similar to S1.
	10 — -	_	5	$\langle$	S3	4 to 5.6	19/4	7-17-13- 55/1"	PID	= 0.0ppm @S3		S3: SILTY SAND (SM); ~70% subangular gravel up to 1"; ta	% sand, ~20% nonplastic fines, ~10% an; dry.
EMPLATE 2013.GDT 12/22/23	-	_	N	$\sqrt{}$	S4	6 to 8	24/6	16-10- 13-62	PID	= 0.0ppm @S4		sand, ~20% nonplastic fines	AVEL (SM); ~60% fine to medium , ~20% subangular gravel up to 1.5"; ick fragments. Slight organic-like
	_	_		$\bigvee$	S5	8 to 10	24/13	33-27- 27-28	PID	= 0.0ppm @S5		subangular gravel up to 1.5 i	AVEL (SM); ~50% sand, ~30% nch, ~20% nonplastic fines; brown to oncrete fragments; moist; slight
ATA	-	<u></u>	0	$\frac{1}{2}$	S6 ,	10	5/0	100/5"				S6: No Recovery.	
GEI WOBURN STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA T ▼	-	_				to 10.4		,100/3,			DEDICATE OF		
:305370-ASTOR	0 —		15						grad Rig feet. feet.	e casing to 15 feet belo le. Roller bit to 15 feet. chatter from 10 to 15 Cobble from 10 to 14 = 0.0ppm @S7	w		
GRAPHIC LOG 2	-	_	<u> </u>	$\bigvee$	S7	15 to 17	24/4	16-8-5-3		– 0.0µµп (дот		S7: WIDELY GRADED GRA subangular gravel up to 1.5"	VEL WITH SAND (GW); ~80% ~20% sand; brown; wet.
ORTH-EAST.	_									e casing to 20 feet belo le. Roller bit to 20 feet.	w W	STRATUM IIA - LOOSE S	AND (CLASS 6) at ~18.5 ft / El4.5
TD 6-N													
N S	OTES	S:				I	ı	1			PRO	ಸ JECT NAME: Astoria Cove	
GEI WOBU												//STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	YYORK GEI Consultants

GROUND SURFACE EL. (ft): 14.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,573

**DATE START/END:** 5/12/2020 - 5/13/2020

DRILLING COMPANY: Craig Geotechnical Drilling

## **BORING B21**

PAGE 2 of 3

		_					Т		PAGE 2 of 3	
			S	ample Inf	ormation	1		l go		
Elev. (ft)	Depth (ft)	s	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description	
-		$\bigvee$	S8	20 to 22	24/14	5-2- WOH-1	PID = 0.0ppm @S8		S8: CLAYEY SAND (SC); ~75% sand, ~20% low plasticity fi ~5% fine gravel; dark brown and black; wet. Slight organic-lodor.	nes, ike
-10 —							Drive casing to 25 feet below grade. Roller bit to 25 feet.			
-	25	$\bigvee$	S9	25 to 27	24/7	5-7-11- 15	PID = 0.0ppm @S9		S9: CLAYEY SAND WITH GRAVEL (SC); ~50% sand, ~30% subangular gravel, ~20% low plasticity fines; black-white; we Some organic fibers. Organic-like odor.	
-	_						Drive casing to 30 feet below grade. Roller bit to 30 feet.		STRATUM IIB - SILT/CLAY (CLASS 4B/5B) at ~28.5 ft / E	1 <u>4</u>
-	30	$\bigvee$	S10	30 to 32	24/17	2-5-5-5	PID = 0.0ppm @S10		S10: FAT CLAY WITH GRAVEL (CH); ~85% medium plastic fines, ~10% subangular gravel up to 1.5", ~5% fine sand; blawet. Organic-like odor.	
-20 —	_						Roller bit to 35 feet. Rig chatter from 32 to 34 feet (Cobble/boulder).		STRATUM III - TILL (CLASS 3A) at ~33.5 ft / EI19.	5 — –
-	35	$\bigvee$	S11	35 to 37	24/2	22-11- 21-10	PID = 0.0ppm @S11		S11: WIDELY GRADED GRAVEL WITH SAND (GW); ~70% subangular gravel up to 1.5", ~30% sand; brown; wet.	6
-	40		S12	40 to	24/8	12-21-	Roller bit to 40 feet. Rig chatter from 39 to 40 feet.  PID = 0.0ppm @S12		S12: NARROWLY GRADED SAND (SP); ~85% fine to med sand, ~10% subangular gravel up to 1.5", ~5% nonplastic fir	ium
-	<u>-</u>	X 		42		24-26	Drive casing to 40 feet below	,	yellow-brown; wet.	.50,
-30 — -	45	\/	S13	45	24/6	29-23-	grade. Roller bit to 45 feet.  PID = 0.0ppm @S13		S13: WIDELY GRADED SAND WITH GRAVEL (SW); ~75%	sar
		$\bigwedge$	2.0	to 47		29-30			~25% subangular gravel up to 1.5"; light brown; wet.	
NOTES	<b>S</b> :							CITY/S	ECT NAME: Astoria Cove  STATE: Astoria, Queens, New York  ROJECT NUMBER: 2305370  GEI  Corr	nsulta

GROUND SURFACE EL. (ft): 14.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,573

**DATE START/END:** 5/12/2020 - 5/13/2020

DRILLING COMPANY: Craig Geotechnical Drilling

## **BORING B21**

PAGE 3 of 3

		S	ample Inf	ormation	1		g.	
Elev. Dept (ft) (ft)		Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description
	0 X	S14	50 to 51.2	14/8	39-90- 102/2"	Roller bit to 50 feet. Rig chatter ~47 and from 49 to 50 feet.  PID = 0.0ppm @S14		S14: SILTY SAND WITH GRAVEL (SM): ~50% fine to medium sand; ~30% nonplastic fines; ~20% subangular gravel up to 1. brown; wet. Trace mica. (Class 3a)
-40	5	S15	55 to 55.9	11/0	96- 100/5"	Drive casing to 50 feet below grade. Roller bit to 55 feet. Rig chatter ~51 feet and from 52 to 54 feet.  PID = 0.0ppm @S15		S15: NO RECOVERY.
— 6I —	0	S16	60 to 61.4	17/9	22-34- 100/5"	Roller bit to 60 feet. Rig chatter ~56 feet.  PID = 0.0ppm @S16		S16: WIDELY GRADED SAND WITH SILT (SW-SM); ~80% si ~10% nonplastic fines, ~10% subangular gravel up to 1"; brow wet. Some micaceous sand.
-50	5	S17	65 to 65.8	9/9	37- 100/3"	Roller bit to 65 feet. Rig chatter ~61 feet and from 63 to 65 feet. Losing return drilling fluid.		S17: WIDELY GRADED SAND WITH GRAVEL (SW); ~75% s ~20% subangular gravel up to 1.5", ~5% nonplastic fines; browwet.
	0 \	S18	70 to 71.4	17/7	66-65- 100/5"	Roller bit to 70 feet. Rig chatter from 66 to 70 feet. Cannot drive casing and losing return drilling fluid.		S18: SILTY SAND WITH GRAVEL (SM); ~50% fine to medium sand, ~30% nonplastic fines, ~20% subangular gravel up to 1. orange-brown; wet. Trace mica.



				<b>MATION</b> 222,064	_			<b>EASTING (ft):</b> 1,003,5	31		BORING		
			_		ft):17.5	5		DATE START/END:		020 - 5/28/2020			
						8/NAD83 N	IY Zone 310				B22		
				: <u>75.</u> i. Holme				DRILLER NAME: <u>Ed</u> RIG TYPE: CME 75	Flana	gan			
LOGG		ы.	_	i. I lollile	75			KIG TIFE. CIVIL 73			PAGE 1 of 4		
DRILI	_IN	G IN	FOI	RMATIO	<u>NC</u>								
HAMN				Auton							REL TYPE: NA		
				NA /	NA lud Rotary	· Mach		DRILL ROD O.D.: NN	1	CORE BAR	REL I.D./O.D.: NA / NA		
				EPTHS (		vv don							
		. =											
ABBR	EVI	AIIC	ino:	Rec. RQD WOF	= Length of R = Weight	/ Length ality Designate Sound Core	ation es>4 in / Pen	S = Split-Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PL = Plasticity Limit PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside D	NA, NM = Not Applicable, Not Measure Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler. iameter		
				S	ample Inf	ormation			g				
Elev. (ft)		epth ft)		ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log		Rock Description		
			Ļ,		` ′	(111)	OI RQD	DID 000 0	2000	· ·	CLASS 7) at ~0 ft / El. 17.5		
-	  - 		$\bigvee$	S1	to 2	24/13	4-14-7-5	PID =363.0ppm @S1		sand, ~20% nonplastic fines,	VEL (SM); ~60% fine to medium ~20% subangular gravel up to 1.5"; troleum odor, black staining. Red tal fragments.		
_				S2	2 to 4	24/11	9-13-12- 7	PID = 154.9ppm @S2		S2: NARROWLY GRADED S to medium sand, ~10% nonpl subangular gravel up to 1"; bi	AND WITH SILT (SP-SM); ~80% fine astic fines, ~10% fie to coarse own; dry.		
-	-	5		S3	4 to 6	24/9	17-13- 12-9	PID = 121.4ppm @S3		S3: NARROWLY GRADED S S2. Some coal fragments.	3: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to 2. Some coal fragments.		
-	_		$\left\langle \cdot \right\rangle$	S4	6 to 8	24/14	3-2-4-10	PID = 58.9ppm @S4		S4: NARROWLY GRADED S S3.	AND WITH SILT (SP-SM); Similar to		
10 —	]  -  -		$\left\langle \cdot \right\rangle$	S5	8 to 10	24/10	28-13-9- 9	PID = 31.2ppm @S5		S5: NARROWLY GRADED S S3. Concrete fragments.	AND WITH SILT (SP-SM); Similar to		
-	1	40	$\mathbb{N}$							STRATUM IIC - SAND	(CLASS 3B) at ~10 ft / El. 7.5		
-	-	10	$\bigvee$	S6	10 to 12	24/6	2-3-2-5	PID = 24.7ppm @S6			0 WITH SILT (SW-SM); ~85% sand, ine subrounded gravel up to 0.5";		
-	  -  -							Advance casing to 15 feet below grade. Roller bit to 15 feet.					
-	-	15		S7	15 to 17	24/8	3-3-4-2	PID = 13.6ppm @S7		S7: WIDELY GRADED SAND	WITH SILT (SW-SM); Similar to S6.		
10 —								Advance casing to 20 feet below grade. Roller bit to 20		STRATUM III - TILL (	CLASS 3A) at ~18.5 ft / El1		
-	_							feet.					
NOTES	3:		_						PROJ	JECT NAME: Astoria Cove			
										STATE: Astoria, Queens, New PROJECT NUMBER: 2305370	York GEI Consultants		

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GROUND SURFACE EL. (ft): 17.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

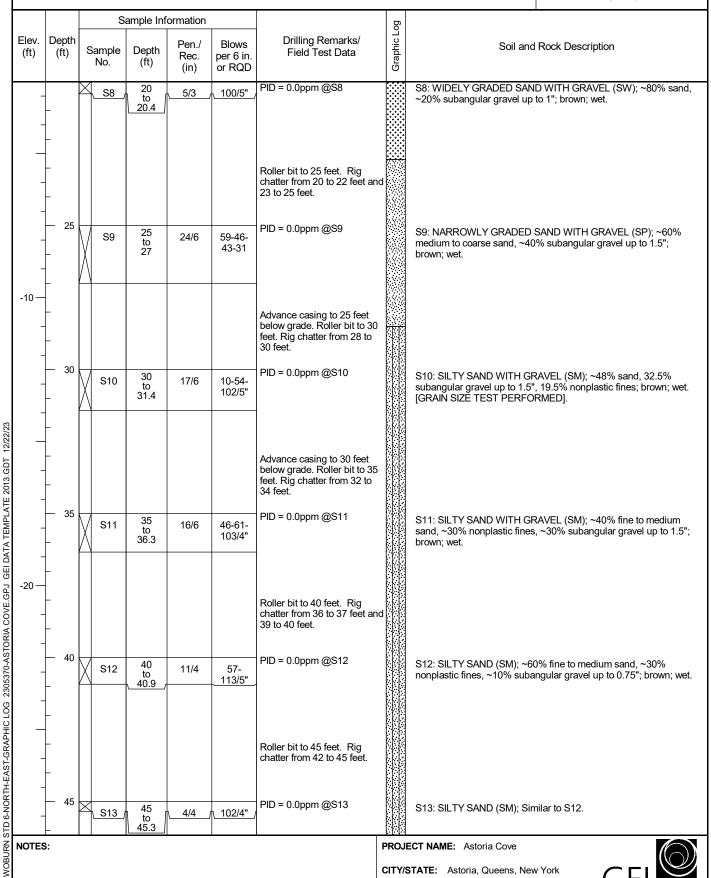
**EASTING (ft):** 1,003,531

**DATE START/END:** 5/28/2020 - 5/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B22** 

PAGE 2 of 4



CITY/STATE: Astoria, Queens, New York

GROUND SURFACE EL. (ft): 17.5

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

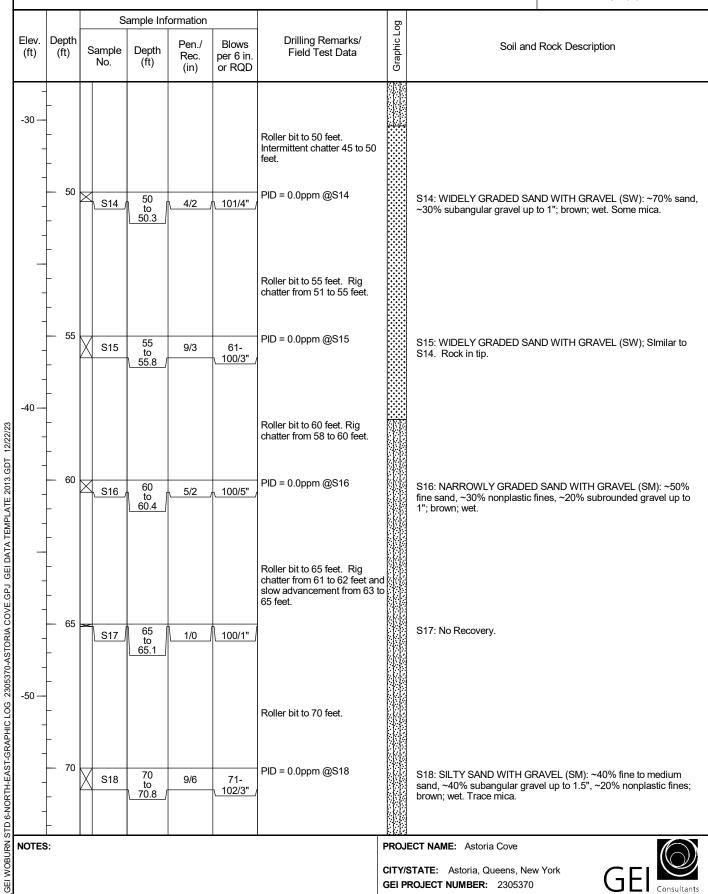
EASTING (ft): 1,003,531

**DATE START/END:** 5/28/2020 - 5/28/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B22** 

PAGE 3 of 4

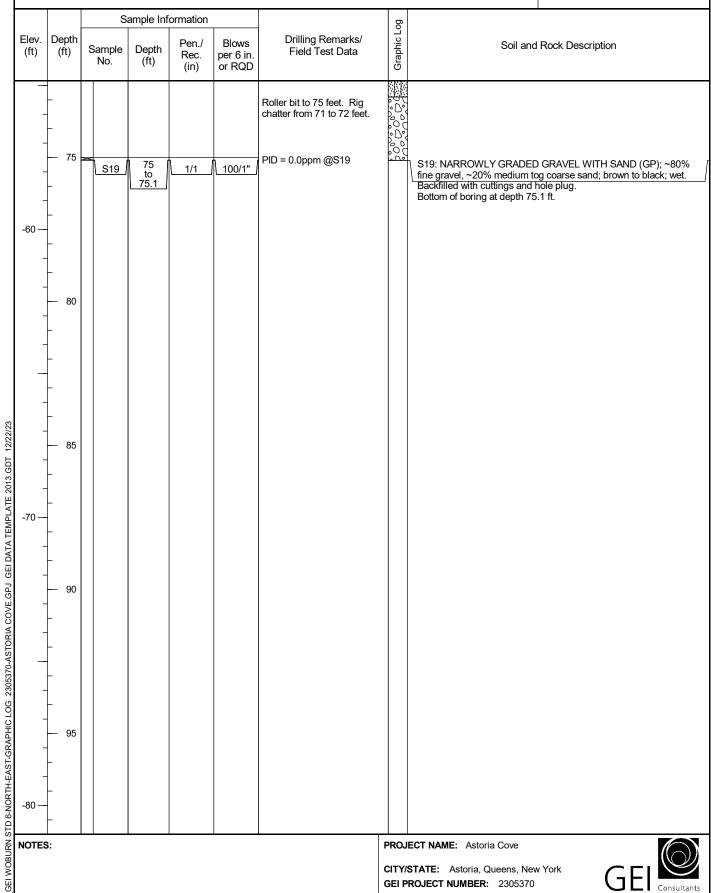


NORTHING (ft): 222,064 **EASTING (ft):** 1,003,531

GROUND SURFACE EL. (ft): 17.5 **VERT./HORIZ. DATUMS:** NAVD 88/NAD83 NY Zone 3104

**DATE START/END:** 5/28/2020 - 5/28/2020 **DRILLING COMPANY:** Craig Geotechnical Drilling **BORING B22** 

PAGE 4 of 4



#### BORING INFORMATION **BORING** NORTHING (ft): 222,133 **EASTING (ft):** 1,003,480 GROUND SURFACE EL. (ft): **DATE START/END:** 3/10/2020 - 3/11/2020 15.0 **B23** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 90.1 DRILLER NAME: Joe Schuster LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NX Wireline AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: 2.125 inch / 3 inch DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) per 6 in. Rec. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 15 PID = 0.9ppm @S1 (0-6"): Asphalt. 0 S1 24/12 13-26to 2 S1 (6"-12"): SILTY SAND WITH GRAVEL (SM); ~60% fine sand, 37-25 ~20% nonplastic fines, ~20% angular gravel up to 1.25"; dark brown; dry. Brick fragments. PID = 0.4ppm @S2 S2: SILTY SAND (SM); ~70% fine to medium sand, ~20% S2 24/3 90-37nonplastic fines, $\sim\!10\%$ angular gravel up to 0.75"; brown; dry. Brick, asphalt and glass fragments. to 33-41 PID = 0.0ppm @S3 S3 29-19-24/15 to 11-11 10 5 S3: SILTY SAND (SM); Similar to S2. PID = 0.0ppm @S4 6 S4: GRAVEL WITH SAND (GP); ~70% subangular gravel, ~30% 24/3 7-5-2-4 to 8 fine t coarse sand; brown; dry. Asphalt, brick, concrete fragments and plastic debris. PID = 0.0ppm @S5 S5: SILTY SAND WITH GRAVEL (SM); ~50% fine to medium 8 S5 24/7 6-9-5-4 sand, ~30% nonplastic fines, ~20% subangular gravel up to 1"; to 10 brown; moist. Brick fragments and plastic debris. Drive casing to 8 feet below grade. 10 PID = 0.0ppm @S6 10 5-5-3-7 S6 24/7 to 12 S6:SILTY SAND WITH GRAVEL (SM); Similar to S5. Drive casing to 13 feet below arade. 0 15 PID = 0.0ppm @S7 S7:SILTY SAND WITH GRAVEL (SM); Similar to S5. Brown to S7 24/7 9-7-6-5 to 17 black. Trace mica. Faint petroleum odor at bottom of spoon. Black staining. Drive casing to 18 feet below arade.

NOTES:

2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT

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STD 6-NORTH-EAST-GRAPHIC

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**PROJECT NAME:** Astoria Cove



GROUND SURFACE EL. (ft): 15.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,480

**DATE START/END:** 3/10/2020 - 3/11/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B23** 

PAGE 2 of 4

		S	ample Inf	ormation			g		
Depth (ft)			Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Lo	Soil and Rock Description	
<u>,</u>		S8	20 to 22	24/11	7-3-2-3	PID = 0.0ppm @S8		S8: CLAYEY SAND (SC); ~70% sand, ~20% low plasticit ~10% gravel subangular up to 1"; brown and black; wet. staining; slight petroleum odor. Retained for environment analysis.	Black
						Drive casing to 23 feet below grade.		STRATUM III - TILL (CLASS 3A) at ~23.5 ft / El	8.5
- 25	\ \/	S9	25 to	24/6	21-17- 10-12	PID = 0.0ppm @S9		S9: WIDELY GRADED SAND WITH GRAVEL (SW); ~75 ~25% subangular gravel up to 1.25"; grayish-brown; wet.	5% sand
<u> </u>	<u> </u>		21			Drive casing to 28 feet below grade. Rig chatter 28 to 30 feet.			
- 30 ;		S10	30 to 31.8	21/10	3-5-9- 105/3"	PID = 0.0ppm @S10		S10: NARROWLY GRADED SAND WITH SILT AND GR (SP-SM); ~70% fine to medium sand, ~20% subangular to 1.5", ~10% nonplastic fines; grayish-brown to brown, w	gravel ι
						Rollerbit to 35 feet. Rig chatter from 34 to 35 feet.			
- 35	$\bigvee$	S11	35 to 36.7	20/17	18-24- 45- 102/2"	PID = 0.0ppm @S11		S11: NARROWLY GRADED SAND WITH SILT AND GR. (SP-SM); Similar to S10.	AVEL
=		S12	37 to	1/0	102/1"		13	S12: No Recovery.	
		C1	37.1 38 to 43	60/22			XXXX	C1: BOULDER/COBBLE. Core run times: 7:31, 2:25, 1:4 0:20.	15, 0:54
- 40									
- 45 (	<u> </u>	S13	45 to	24/17	67-63-	PID = 0.0ppm @S13		S13: SILTY SAND WITH GRAVEL (SM); 47.1% sand, 32 subangular gravel up to 1.5", 20.8% nonplastic fines; brow	2.1% wn; wet
	(ft) 25; 30; 40	(ft) Sa (ft) S	Sample No.  Sample No.  S8  S8  S9  S10  C1  40	Sample No. Depth (ft)  S8 20 to 22  S9 25 to 27  S10 30 31.8  S11 35 to 36.7  S12 37 to 37.1  C1 33 to 37.1  C1 43  45 V S13 45 to	(ft) Sample No.   Depth (ft)   Rec. (in)	Penth (ft) Sample No. Depth (ft) Rec. (in) Pen./ Rec. (in) or RQD  S8 20 24/11 7-3-2-3  S9 25 to 24/6 21-17-10-12  S10 30 to 31.8 21/10 3-5-9-105/3"  S11 35 to 36.7 20/17 18-24-45-102/2"  S12 37 to 37.1 1/0 102/1" 37.1  C1 38 60/22 443  40 45 45 24/17 67-63-63-6437	Pepth (ft) Sample   Depth   Pen. / Rec. (in)   Pen.	Pepth   No.   Depth   Pen.   Rec.   (fit)   Rec.	Pent   Rec.

GROUND SURFACE EL. (ft): 15.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,480

**DATE START/END:** 3/10/2020 - 3/11/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B23** 

PAGE 3 of 4

epth (ft)	Sample No.	Depth (ft)	Pen./ Rec.	Blows	Drilling Remarks/	lic L	0.31	
X			(in)	per 6 in. or RQD	Field Test Data	Graphic Log	Soli and	Rock Description
					Rig chatter from 47 to 49 feet.			
50	S14	50 to 51.8	21/15	31-52- 73- 102/3"	PID = 0.0ppm @S14		S14: SILTY SAND (SM); Sim	nilar to S13.
55 🔀	\ S15 ]	55 to 55.3	3/2	102/3"	Intermittent rig chatter from 52 to 55 feet.  PID = 0.0ppm @S15		S15: SILTY SAND (SM); ~70 ~5% subangular gravel up to	)% fine sand, ~25% nonplastic fines 1"; brown; wet.
60	S16	60 to 60.7	8/8	70- \ 104/2" )	Rig chatter at 56 to 57 feet.  PID = 0.0ppm @S16  Rig chatter at 63 to 65 feet.		S16: SILTY SAND (SM); Sim	illar to S15.
65	S17	65 to 67	24/17	41-47- 32-79	PID = 0.0ppm @S17  Rig chatter at 67 to 69 feet.		S17: SILTY SAND (SM); ~70 nonplastic fines, ~10% subar orange-brown; wet.	9% fine to medium sand, ~20% ngular gravel up to 1.5"; brown to
70	S18	70 to 72	24/20	88-84- 61-116	PID = 0.0ppm @S18  Rig chatter at ~72 feet.		S18: SILTY SAND (SM); Sim	nilar to S17. Orange-brown.
	60 \( \times \)	55 S15  60 S16  65 S17	S14 50 51.8  55 S15 55 to 55.3  60 S16 60 to 60.7  65 S17 65 to 67  70 S18 70 to	S14 50 51.8 21/15  55 S15 55 to 55.3 3/2  60 S16 60 8/8  60.7  65 S17 65 to 24/17  67  70 S18 70 24/20	S14	S14	S14	S14   S16   S18   S17   S18   S17   S18   S17   S18   S18   S17   S18   S18



NORTHING (ft): 222,133 GROUND SURFACE EL. (ft): 15.0

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,480

**DATE START/END:** 3/10/2020 - 3/11/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B23** 

PAGE 4 of 4

			S	ample Inf	ormation			g		
Elev. (ft)	Depth (ft)	S	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and F	Rock Description
-60 — -	- - 75 ; - -		S19	75 to 77	24/20	11-21- 29-39	PID = 0.0ppm @S19 Rig chatter at 79 to 80 feet.		S19: SILT (ML); ~85% nonpla: sand, ~5% angular gravel up t Decomposed bedrock.	sED/WEATHERED ROCK (CLASS 3.5 ft / El58.5 stic to low plasticity fines, ~10% o 0.75"; greenish-blue.
-	- 80 : - -		S20 C2	80 to 80.1 80.1 to 85.1	1/0 60/44	105/1" 32%			C2: HARTLAND FORMATION from 16" to base, moderate we banded, fine to medium graine sand. Horizontal fractures at 4'	didn't advanced. On top of bedroc l; Gneiss, hard, slight weathering athering above 16", black and whit d. (4-10.5") is broken apart with ', 10.5", 16", 22.5", 26.5", 29.5", 33 er foot (minutes:seconds): 2:19, 1:5
-70 — - -	- 85 - -		C3	85.1 to 90.1	60/60	96%			C3: HARTLAND FORMATION 19.5" and 38.5". Gneiss, hard weathering at fracture at 19.5".	, black and white, banded, fine to es per foot (minutes:seconds): 03:0
-	- 90   -								Backfilled with cuttings and ho Bottom of boring at depth 90.1	le plug. ft.
-80 —	_ 95 _									
NOTES	- 3:							PROJ	ECT NAME: Astoria Cove	6

#### BORING INFORMATION **BORING** NORTHING (ft): 222,066 **EASTING (ft):** 1,003,320 GROUND SURFACE EL. (ft): **DATE START/END:** 3/16/2020 - 3/17/2020 21.9 **B29(OW)** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 85.0 DRILLER NAME: Joe Schuster LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: \_Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NA AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: NA / NA DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ▼ 16.9 3/18/2020 7:55 am in observation well ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) Rec. per 6 in. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.9 PID = 0.0ppm @S1 S1: WIDELY GRADED SAND WITH SILT AND GRAVEL 0 S1 24/3 17-4-5to 2 (SW-SM); ~80% sand, ~10% gravel angular to subrounded up to 47 1.25", ~10% nonplastic fines; brown; dry. 20 PID = 0.0ppm @S2 S2: WIDELY GRADED SAND WITH SILT AND GRAVEL S2 24/14 52-21-9to (SW-SM); Similar to S1. Drilled through cobble boulder 4 to 14 5 inches. PID = 0.0ppm @S3S3: WIDELY GRADED SAND (SW); ~95% sand, ~5% angular to S3 24/3 32-22to subrounded gravel up to 0.75"; yellow-brown; dry. 17-11 5 PID = 0.0ppm @S4 6 S4: WIDELY GRADED SAND WITH GRAVEL (SW); ~70% sand, S4 24/7 11-11-6to 8 ~30% subangular to subrounded gravel up to 1.5"; orange-brown; STRATUM IIC - SAND (CLASS 3B) at ~8 ft / El. 13.9 Advancing casing to 8feet; S5: NARROWLY GRADED SAND (SP); ~90% fine to medium 8 S5 24/9 12-19-Roller bit to 10feet after sand, ~5% subangular to subrounded gravel up to 1.25", ~5% to 10 15-16 sample. nonplastic fines; brown to reddish brown; dry. PID = 0.0ppm. @S5 10 PID = 0.0ppm @S6 S6 (0-12"): SILTY SAND (SM); ~75% fine to medium sand, ~20% 10 12-8-8-S6 24/8 to 12 nonplastic fines, ~5% subangular gravel up to 0.5"; brown; dry. S6 (12"-18"): NARROWLY GRADED SAND (SP); ~95% fine to 10 medium sand, ~5% nonplastic fines; tan; dry. Roller bit to 15 feet. 15 PID = 0.0ppm @S7 S7 (0-7"): NARROWLY GRADED SAND (SP); Similar to S6 but S7 24/17 7-4-5-26 to 17 brown and moist. S7 (7"-17"): SILT WITH SAND (ML); ~90% nonplastic fines, ~10% fine sand; brown; moist. Trace mica. Roller bit to 20 feet. Rig chatter 16 to 18 feet.

NOTES:

GEI DATA TEMPLATE 2013.GDT

LOG 2305370-ASTORIA COVE.GPJ

STD 6-NORTH-EAST-GRAPHIC

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PROJECT NAME: Astoria Cove



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,320

**DATE START/END:** 3/16/2020 - 3/17/2020

DRILLING COMPANY: Craig Geotechnical Drilling

# **BORING B29(OW)**

PAGE 2 of 4

										PAGE 2 of 4
			S	ample Inf	ormation	_		og		
Elev. (ft)	Depth (ft)	s	ample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and	Rock Description
0-	_		S8	20 to 22	24	16-7-10- 15	PID = 0.0ppm @S8		S8: NARROWLY GRADED Sand, ~5% subangular to sub nonplastic fines; brown; wet.	SAND (SP); ~90% fine to medium orounded gravel up to 1.25", ~5%
_	_						Roller bit to 25 feet. Rig chatter from 23 to 25 feet.		STRATUM III - TILL (0	CLASS 3A) at ~23.5 ft / El1.6
-	25 	M	S9	25 to 27	24/11	23-21- 21-24	PID = 0.0ppm @S9		S9: WIDELY GRADED SANI ~30% subangular to subroun Mica near the top.	D WITH GRAVEL (SW); ~70% sand ded gravel up to 1.5"; brown; wet.
=	- - -						Roller bit to 30 feet. Rig chatter from 25 to 26 feet and 29 to 30 feet.			
-10 —	30	X	S10	30 to 30.8	9/5	38- 104/3"	PID = 0.0ppm @S10			SAND (SP); ~95% fine to medium gular gravel up to 0.75"; brown to
-	- - - - 35						Roller bit to 35 feet. Rig chatter from 31 to 32 feet, 33 to 34 feet.  PID = 0.0ppm @S11			
_	<u>-</u>	X	S11	35 to 35.8	9	118- 101/3"	т і — О.орріп @ЗТТ		fine to medium sand, ~10% a	OSAND WITH SILT (SP-SM); ~80% angular to subrounded gravel up to brown; wet. Drilled into boulder/cobl
-	<u>-</u>						Roller bit to 40 feet. Rig chatter from 36 to 40 feet.			
-20 —	40		S12	40 to 42	24/22	79-82- 66-78	PID = 0.0ppm @S12		S12: WIDELY GRADED SAN ~30% subangular to subrour	ID WITH GRAVEL (SW): ~70% sanded gravel up to 1.5"; brown; wet.
=	<u>-</u>						Roller bit to 45 feet. Rig chatter from 45 to 46 feet, 47 to 50 feet.			
-	- 45 	×	S13	45 to 45.2	2/2	102/2"	PID = 0.0ppm @S13		S13: SANDY SILT (ML): ~60 brown; wet.	% nonplastic fines, ~40% sand;
NOTES	S:			-3				CITY/	ECT NAME: Astoria Cove STATE: Astoria, Queens, New ROJECT NUMBER: 2305370	York GEI Consult

GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

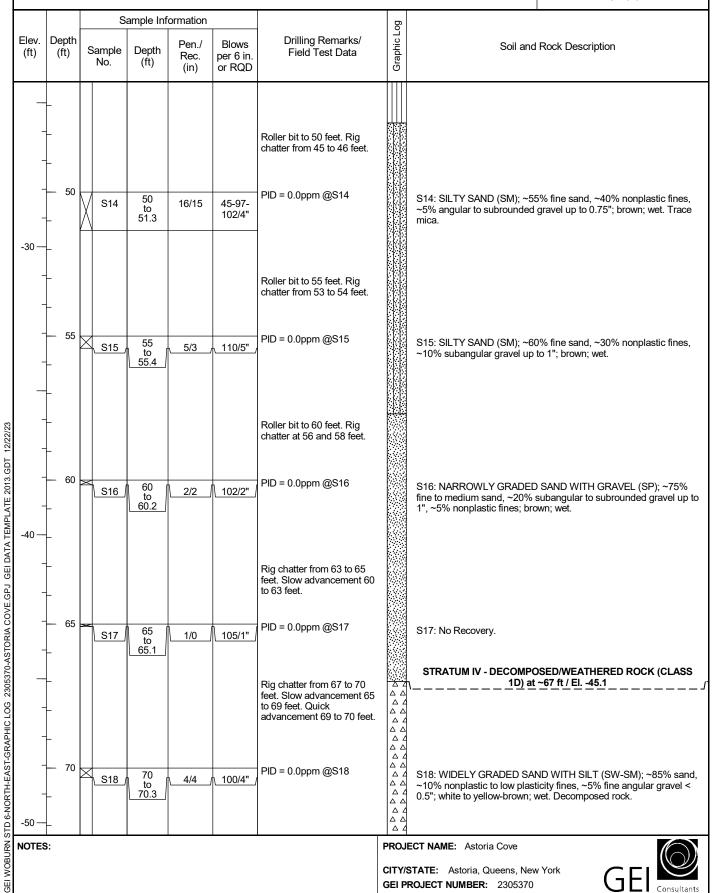
**EASTING (ft):** 1,003,320

**DATE START/END:** 3/16/2020 - 3/17/2020

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B29(OW)

PAGE 3 of 4



GROUND SURFACE EL. (ft): 21.9

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

EASTING (ft): 1,003,320

**DATE START/END:** 3/16/2020 - 3/17/2020

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING** B29(OW)

PAGE 4 of 4

		Sample Information					go		
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Graphic Log	Soil and Rock Description	
	_ 75 \	S19 C1	75 to 75 75	0/0 60/34	45/0" 35%	Roller bit to 75 feet. Driller used low pressure on bit, slow advancement.  No advancement, hard bouncing.		grained; horizontal and vertic fractures; white to grey to blace	I. Gneiss; hard; fine to medium al banding; slight weathering at ck. Horizontal fractures at 1.5", 8.5",
	<del>-</del>		to 80					12.5", 17.5", 21.5", and 26.5". Core run times per foot (minutes:seconds): 1:06, 2:42, 2:38, 3:52, and 3:06.  STRATUM V - BEDROCK (CLASS 1B) at ~80 ft / El58.1	
-60 — -	- 80 - -	C2	80 to 85	60/57	71%			C2: Gneiss; hard; fine to med banding; slight weathering at Horizontal fractures at 4", 7",	dium grained; horizontal and vertical fractures; white to grey to black. 11", 15.5", 19.5", 22.5", 29", 34", 39' s per foot (minutes:seconds): 4:01,
-	- 85 ·							Backfilled with cuttings and h Bottom of boring at depth 85	ole plug. ft.
- - -70 —	- - 90 -								
-	- 95								
NOTES:							PROJECT NAME: Astoria Cove  CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370  GEI Consultants		

#### BORING INFORMATION **BORING** NORTHING (ft): 221,759 **EASTING (ft):** 1,003,421 GROUND SURFACE EL. (ft): **DATE START/END:** 5/13/2020 - 5/14/2020 29.4 **B53(OW)** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 55.0 DRILLER NAME: Keith Parent LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 3 **DRILLING INFORMATION** HAMMER TYPE: \_Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NX Wireline AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: 2.125 inch / 3 inch DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ⊈ 23.5 5/15/2020 9:30 am in observation well ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) Rec. per 6 in. No (ft) or RQD (in) STRATUM I - FILL (CLASS 7) at ~0 ft / El. 29.4 (0-4"): Asphalt. S1: SILTY SAND (SM); ~75% fine sand, ~20% nonplastic fines, PID = 0.0ppm @S1 0 S1 20/3 9-7-4to 9/2" 17 ~5% angular gravel up to 1"; yellow-brown; dry. Red brick PID = 0.0ppm @S2 S2: SILTY SAND (SM); ~75% fine to medium sand, ~25% 24/10 10-6-7-5 S2 to nonplastic fines; orange-brown to brown; dry. STRATUM IIC - SAND (CLASS 3B) at ~4 ft / El. 25.4 PID = 0.0ppm @S3 S3: NARROWLY GRADED SAND WITH SILT (SP-SM); ~90% fine S3 24/10 10-6-5-4 to to medium sand, ~10% nonplastic fines; tan-brown; dry. Trace 5 PID = 0.0ppm @S4 6 S4: NARROWLY GRADED SAND WITH SILT (SP-SM); ~90% fine S4 24/19 6-3-3-4 to 8 sand, ~10% nonplastic fines; tan to yellow-brown; dry. Trace mica. PID = 0.0ppm @S5 8 S5: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to S5 24/14 4-4-6-5 to 10 20 10 PID = 0.0ppm @S6S6: NARROWLY GRADED SAND WITH SILT (SP-SM); Similar to 10 24/15 8-6-8-5 S6 to 12 Advance casing to 15 feet STRATUM III - TILL (CLASS 3A) at ~13.5 ft / El. 15.9 below grade. Roller bit to 15 15 PID = 0.0ppm @S7 S7: SILTY SAND WITH GRAVEL (SM); ~60% fine to medium S7 24/7 9-8-17to sand, ~20% nonplastic fines, ~20% angular to subrounded gravel 19 up to 1.5"; purple-brown; moist. Advance casing to 20 feet below grade. Roller bit to 20 feet. Rig chatter ~16 feet and from 17 to 18 feet. 10

NOTES:

LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT

STD 6-NORTH-EAST-GRAPHIC

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PROJECT NAME: Astoria Cove



GROUND SURFACE EL. (ft): 29.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

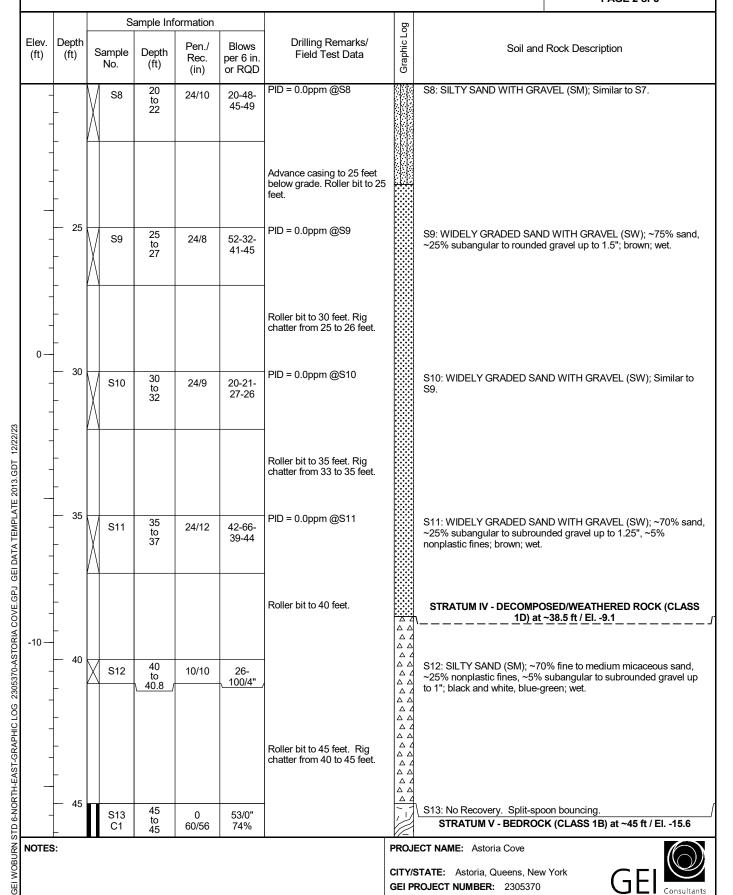
**EASTING (ft):** 1,003,421

**DATE START/END:** 5/13/2020 - 5/14/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

BORING B53(OW)

PAGE 2 of 3



GROUND SURFACE EL. (ft): 29.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

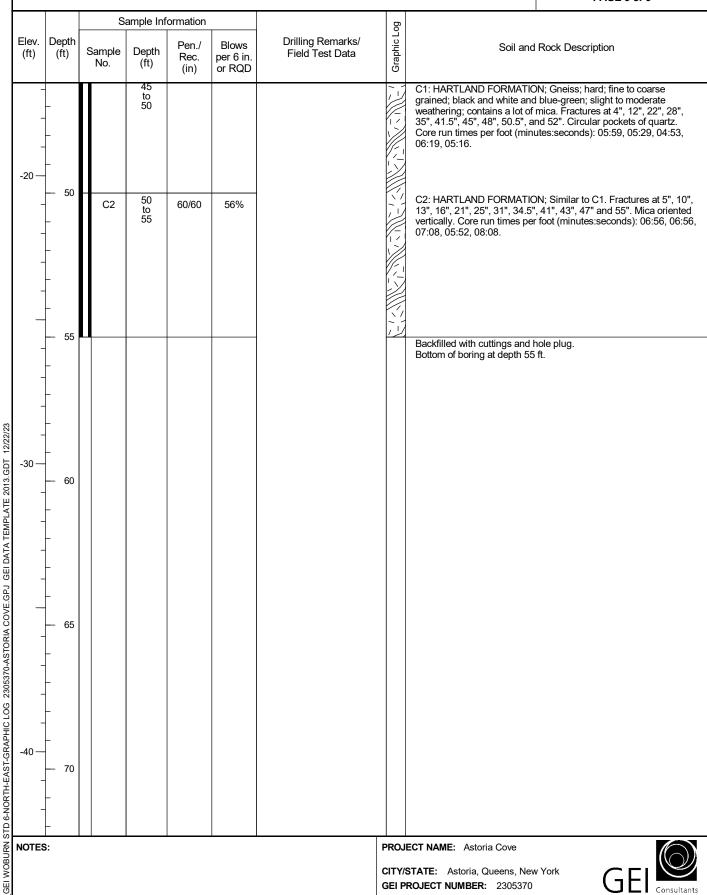
EASTING (ft): 1,003,421

**DATE START/END:** 5/13/2020 - 5/14/2020

**DRILLING COMPANY:** Craig Geotechnical Drilling

### **BORING B53(OW)**

PAGE 3 of 3



				<b>MATION</b> 222,057	_			<b>EASTING (ft):</b> 1,003,4	90	BORING		
	GROU	ND SUR	FA	CE EL. (1	ft):20.1			DATE START/END:	11/23/2		D404	
				TUMS: ): 102		3/NAD83 N	Y Zone 310	<u>DRILLING COMPANY:</u> DRILLER NAME: Ma		B101		
			•	3. Holme				RIG TYPE: CME 75	ic Aqu	PAGE 1 of 5		
ŀ	DRII I	ING IN	FΩ	RMATIC	ON							
				Auton				CASING I.D./O.D.: _4	inch/ 4	RREL TYPE: NX Wireline		
				NA /		Mach		DRILL ROD O.D.: NN	1	CORE BAF	RREL I.D./O.D.: 2.125 inch / 3 inch	
				טכ: <u>וע</u> כ: EPTHS (	lud Rotary ft):	vvasn						
ŀ	ADDD	EVIATIO	NIC.	. Dan	= Penetrati	on I on with		S = Split-Spoon Sample		Qp = Pocket Penetrometer Strength	NA, NM = Not Applicable, Not Measured	
	ADDIN		140	Rec. RQD WOF	= Recovery = Rock Qu = Length of R = Weight	Length ality Designa Sound Core	ation es>4 in / Pen	C = Core Sample U = Undisturbed Sample				
						ormation		j	D <sub>D</sub>			
		Depth	Sample No.		Pen./ Blows		Blows	Drilling Remarks/	lic Lo	Soil and Rock Description		
	(ft)	(ft)			Depth (ft)	Rec. (in)	per 6 in. or RQD	Field Test Data	/ Graphic Log		•	
ŀ			\ /	04	0	, ,				(0-6"): Concrete.	(CLASS 7) at ~0 ft / El. 20.1	
	-	_	$\mathbb{X}$	S1	to 2	24/15	11-7-6-6			S1: SILTY SAND (SM); ~70% nonplastic fines, ~10% subar	of fine to medium sand, ~20% agular to subrounded gravel up to rately tight; concrete / brick fragments	
	-		$\bigvee$	S2	2 to 4	24/11	11-13- 11-5				LY GRADED SAND WITH SILT (SP-SM); ~85% fine d, ~10% nonplastic fines, ~5% subangular to avel up to 0.75"; brown; dry.	
		_ 5		S3	4 to 6	24/9	8-5-25- 19				SAND WITH SILT (SP-SM); ~85% fine s, ~5% fine gravel; brown; dry; loose. b/boulder) from 6 to 9".	
3DT 12/22/23		_		S4	6 to 8	24/7	17-7-6-5				6 fine sand, ~25% nonplastic fines, led gravel up to 0.75"; brown; dry;	
2013.0	_	ļ	$\triangle$					Drive casing to 8 feet below		STRATUM IIA - LOOSE SAND (CLASS 6) at ~8 ft / El. 12.1  S5: NARROWLY GRADED SAND (SP); ~90% fine to medium		
TEMPLATE 2013.GDT	_	_	$\mathbb{N}$	S5	8 to 10	24/7	5-2-3-2	grade. Rollerbit to 8 feet. Added quick-gel to drilling mud. Drive casing to 8 feet below			SAND (SP); ~90% fine to medium astic fines, ~5% fine gravel; brown;	
J GEI DATA T	10 —	<u> </u>		S6	10 to 12	24/0	3-2-1-2	grade. Rollerbit to 8 feet. Added quick-gel to drilling mud. @S5		S6: No Recovery.		
VE.GF	_	-	$\square$		12					S7: No Recovery.		
2305370-ASTORIA COVE.GPJ	-	_	$\mathbb{N}$	S7	to 14	24/0	2-1-1-2					
5370-4	-							Rollerbit to 15 feet.				
	_	15	7	S8	15	24/5	2-1-1-1			S8: NARROWLY GRADED SAND WITH SILT (SP-SM); ~85% f to medium sand, some mica, ~10% nonplastic fines, ~5% fine gravel; brown; wet; loose.		
IIC LO	_	-	IX.		to 17							
RAPH	_		$\triangle$					Drive casing to 15 feet				
\ST-G								Drive casing to 15 feet. Rollerbit to 20 feet. Rig chatter from 19 to 20 feet.				
TH-E/	-	-						Challer Horn 19 to 20 feet.		CTDATUM IIC CANI	) (CLASS 2D) at =40 ft / EL 4.4	
STD 6-NORTH-EAST-GRAPHIC LOG	-	_								JIRATUW IIC - SANL	0 (CLASS 3B) at ~19 ft / El. 1.1	
	NCT-									FOT NAME A		
GEI WOBURN	NOTES	<b>&gt;</b> :							PROJECT NAME: Astoria Cove  CITY/STATE: Astoria, Queens, New York  GEI PROJECT NUMBER: 2305370  GEI Consultants			

GROUND SURFACE EL. (ft): 20.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

**EASTING (ft):** 1,003,490

**DATE START/END:** 11/23/2021 - 11/23/2021

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B101** 

PAGE 2 of 5

		1						1 1		PAGE 2 of 5		
Elev.	Depth		S	ample Inf		Diama	Drilling Remarks/	c Log				
(ft)	(ft)	Sample No.		le Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Field Test Data	Graphic Log	Soil and Rock Description			
_	-	$\bigvee$	S9	20 to 22	24/9	14-9-8-6			S9: NARROWLY GRADED sand, ~5% nonplastic fines, gravel up to 1.25"; brown; we	SAND (SP); ~85% fine to medium ~10% subangular to subrounded t; loose.		
-	- - -						Rollerbit to 25 feet. Rig chatter from 20 to 21 and 23 to 24 feet. Drive casing to 26 feet. Rollerbit to 25 feet. Intermittent rig chatter from 20 to 25 feet.	)	STRATUM III - TILL (0	CLASS 3A) at ~23.5 ft / El3.4		
-	25	M	S10	25 to 27	24/9	12-17- 12-14			sand, ~5% nonplastic fines,	SAND (SP); ~90% fine to medium ~5% subangular gravel up to 1"; 0 to nicaceous sand); wet; moderately tig		
-	_ 	/ \					Drive casing to 25 feet. Rollerbit to 30 feet. Rig chatter from 28 to 30 feet.					
-10 — -	30		S11	30 to 32	24/13	34-29- 36-27			S11: NARROWLY GRADED fine sand, ~10% nonplastic fi gravel up to 1"; brown; wet; ti	SAND WITH SILT (SP-SM); ~80% nes, ~10% subangular to subround ght.		
-							Rollerbit to 35 feet. Intermittent rig chatter from 30 to 35 feet.					
-	35	M	S12	35 to 37	24/12	43-46- 77-62			sand, trace micaceous sand,	SAND (SP); ~90% fine to medium ~5% nonplastic fines, ~5% ravel up to 0.75"; brown; wet; tight.		
-	<del>-</del>  						Rollerbit to 40 feet. Rig chatter from 37 to 39 feet.					
-20 — -	40	X	S13	40 to 40.9/	11/9	53- 100/5"	Rollerbit to 45 feet. Rig chatter from 41 to 42 feet.		S13: NARROWLY GRADED through rock (cobble/boulder	SAND (SP); Similar to S12. Drilled ) from 7 to 9".		
-	1											
_	45	X	S14	45 to 47	24/18	66-49- 73-84			S14: NARROWLY GRADED sand, some micaceous sand to subrounded gravel up to 1	SAND (SP); ~85% fine to medium ~5% nonplastic fines, ~10% angula .5"; brown; wet; tight.		
NOTES	<u></u>	M			2.710			CITY/		.5"; brown; wet; tight.		

GROUND SURFACE EL. (ft): 20.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

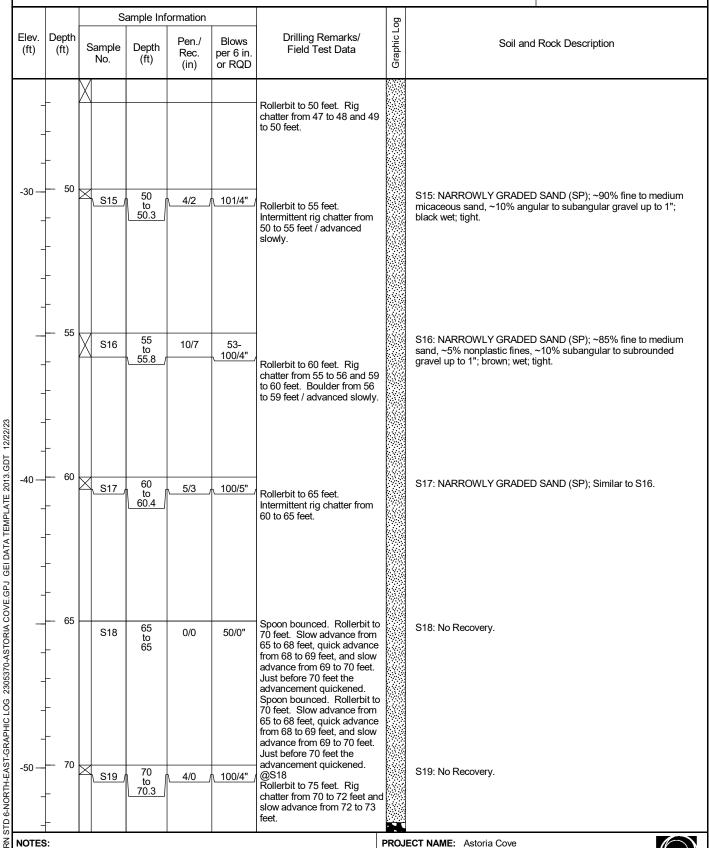
**EASTING (ft):** 1,003,490

**DATE START/END:** 11/23/2021 - 11/23/2021

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B101** 

PAGE 3 of 5



GEI WOBURN

PROJECT NAME: Astoria Cove

CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370



GROUND SURFACE EL. (ft): 20.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

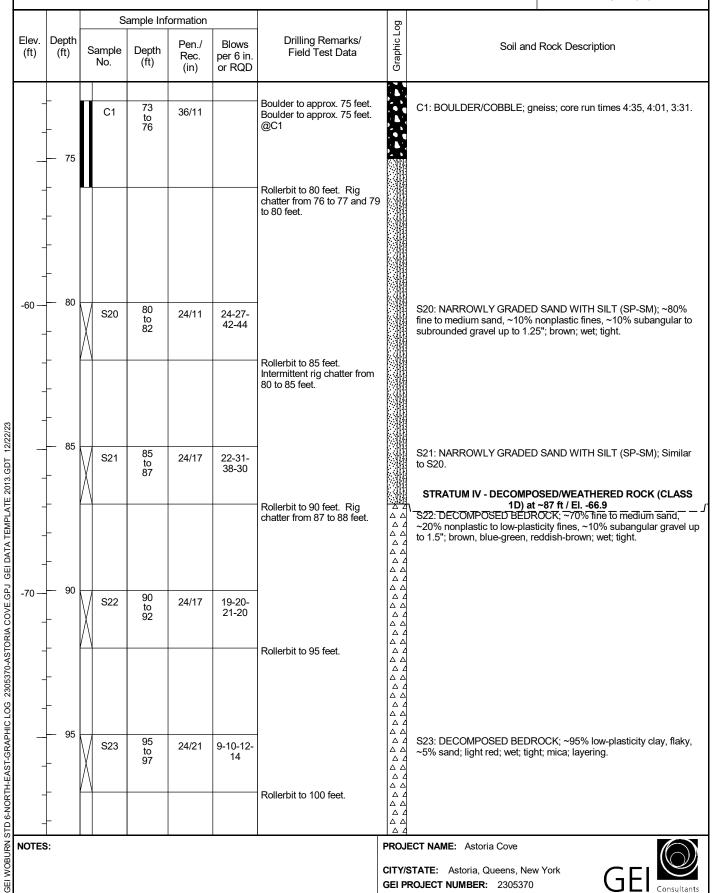
**EASTING (ft):** 1,003,490

**DATE START/END:** 11/23/2021 - 11/23/2021

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B101** 

PAGE 4 of 5



GEI PROJECT NUMBER: 2305370

GROUND SURFACE EL. (ft): \_ 20.1

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

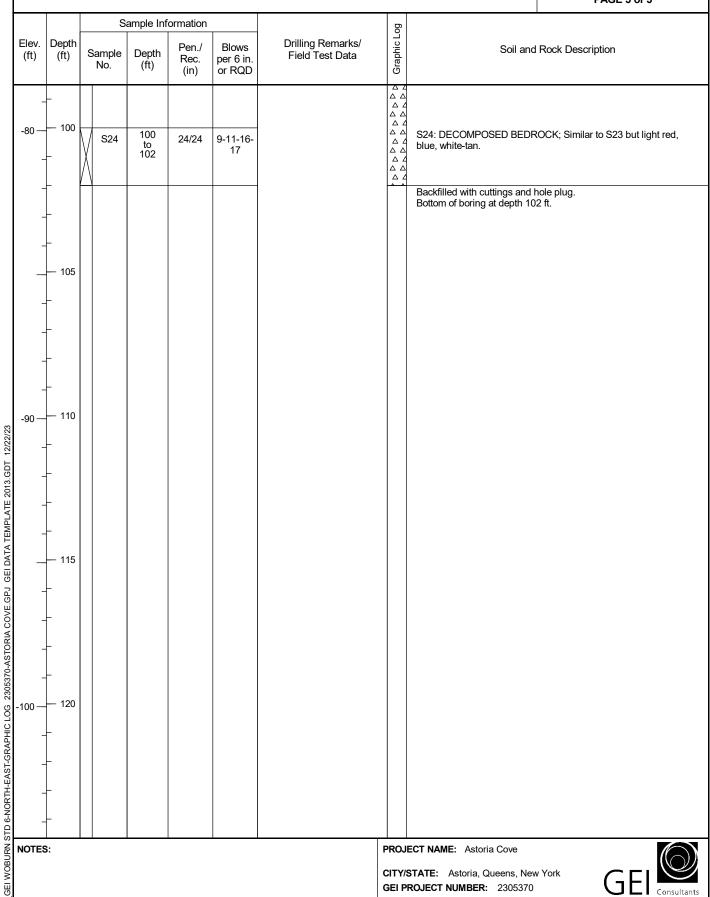
**EASTING (ft):** 1,003,490

**DATE START/END**: 11/23/2021 - 11/23/2021

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B101** 

PAGE 5 of 5



CITY/STATE: Astoria, Queens, New York GEI PROJECT NUMBER: 2305370



#### BORING INFORMATION **BORING** NORTHING (ft): 221,943 **EASTING (ft):** 1,003,439 GROUND SURFACE EL. (ft): **DATE START/END:** 11/24/2021 - 11/24/2021 21.4 **B102** VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 DRILLING COMPANY: Craig Geotechnical Drilling TOTAL DEPTH (ft): 86.8 DRILLER NAME: Marc Aquino LOGGED BY: G. Holmes RIG TYPE: CME 75 PAGE 1 of 4 **DRILLING INFORMATION** HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL TYPE: NA CORE BARREL I.D./O.D.: NA / NA AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: NM DRILLING METHOD: Mud Rotary Wash WATER LEVEL DEPTHS (ft): ABBREVIATIONS: Pen. = Penetration Length S = Split-Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. SC = Sonic Core PL = Plasticity Limit split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter Sample Information Depth Drilling Remarks/ Flev Graphic Pen./ Blows Soil and Rock Description Sample Depth Field Test Data (ft) (ft) Rec. per 6 in. No (ft) (in) or RQD STRATUM I - FILL (CLASS 7) at ~0 ft / El. 21.4 0 0-2": ASPHALT S1 24/11 12-4-8-9 to 2 S1: SILTY SAND (SM); ~70% fine sand, fine to medium at base, ~20% nonplastic fines, ~10% angular to subangular gravel up to 20 1.5"; brown; dry; tight; brick fragments. S2: NARROWLY GRADED SAND (SP); ~95% fine to medium S2 24/12 11-4-6-3 to sand, ~5% nonplastic fines; orange-brown; dry; loose. S3: NARROWLY GRADED SAND (SP); Similar to S2. S3 24/13 7-5-2-2 to 5 6 S4: SILTY SAND (SM); ~60% fine sand, ~40% nonplastic fines; 24/10 4-4-3-2 to 8 light brown; dry; loose. STD 6-NORTH-EAST-GRAPHIC LOG 2305370-ASTORIA COVE.GPJ GEI DATA TEMPLATE 2013.GDT Drive casing to 8 feet below 8 S5: No Recovery. Shoe broke off of spoon. S5 24/0 63-31grade. Rollerbit to 8 feet. to 10 53-42 10 Rollerbit to 15 feet. Rig chatter from 8 to 12 feet. Drive casing to 15 feet. Rollerbit to 15 feet. 10 STRATUM IIC - SAND (CLASS 3B) at ~13.5 ft / El. 7.9 15 S6: NARROWLY GRADED SAND WITH SILT (SP-SM); ~85% fine S6 24/5 10-8-11to 17 to medium sand, ~10% nonplastic fines, ~5% fine gravel; light 10 brown; wet; loose. Rollerbit to 20 feet. Rig chatter from 17 to 18 feet. Drive casing to 20 feet. STRATUM III - TILL (CLASS 3A) at ~18.5 ft / El. 2.9 Rollerbit to 20 feet. NOTES: PROJECT NAME: Astoria Cove CITY/STATE: Astoria, Queens, New York

GEI PROJECT NUMBER: 2305370

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GROUND SURFACE EL. (ft): 21.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104 D

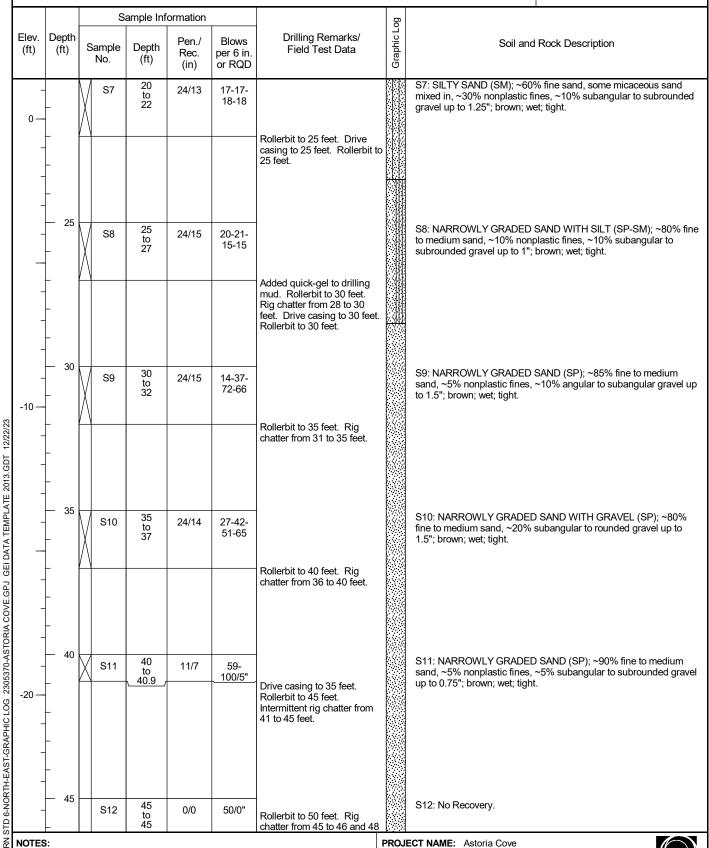
**EASTING (ft):** 1,003,439

**DATE START/END**: 11/24/2021 - 11/24/2021

**DRILLING COMPANY:** Craig Geotechnical Drilling

#### BORING B102

PAGE 2 of 4



CITY/STATE: Astoria, Queens, New York
GEI PROJECT NUMBER: 2305370

GEI WOBURN

GROUND SURFACE EL. (ft): 21.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

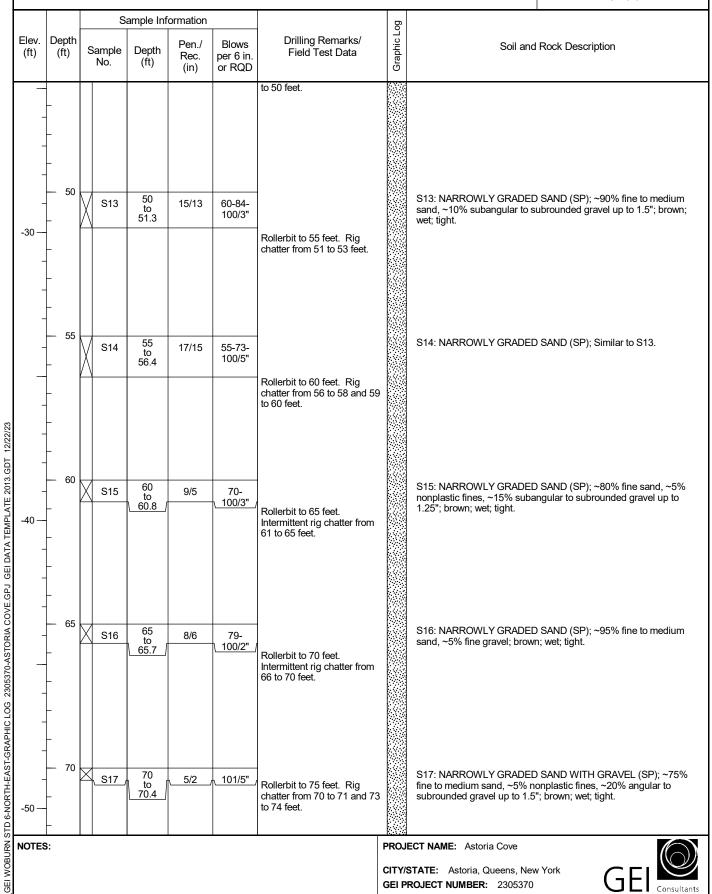
**EASTING (ft):** 1,003,439

**DATE START/END:** 11/24/2021 - 11/24/2021

DRILLING COMPANY: Craig Geotechnical Drilling

**BORING B102** 

PAGE 3 of 4



GEI PROJECT NUMBER: 2305370

GROUND SURFACE EL. (ft): 21.4

VERT./HORIZ. DATUMS: NAVD 88/NAD83 NY Zone 3104

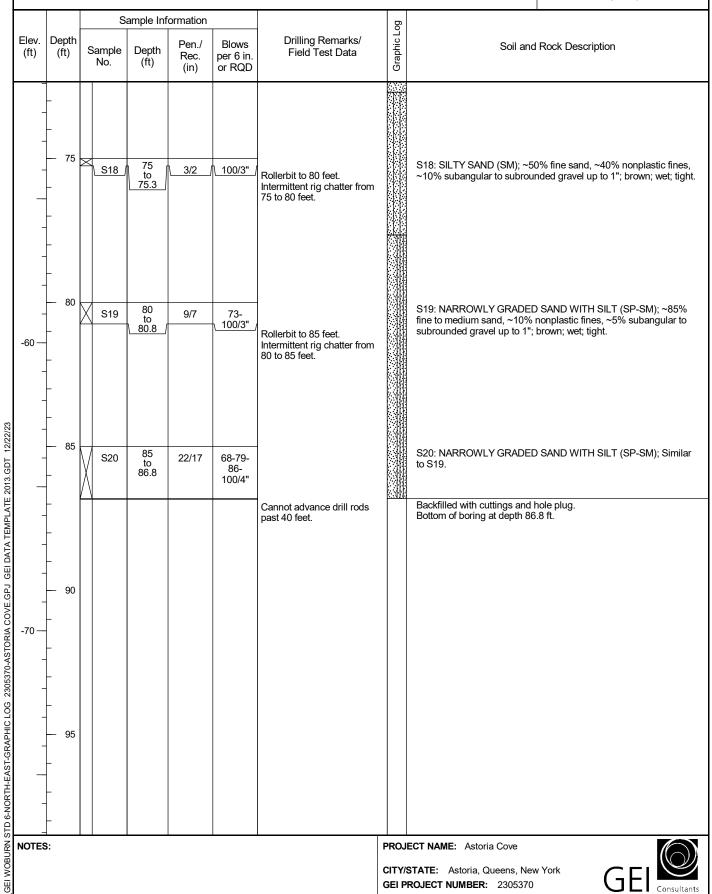
**EASTING (ft):** 1,003,439

**DATE START/END:** 11/24/2021 - 11/24/2021

DRILLING COMPANY: Craig Geotechnical Drilling

BORING B102

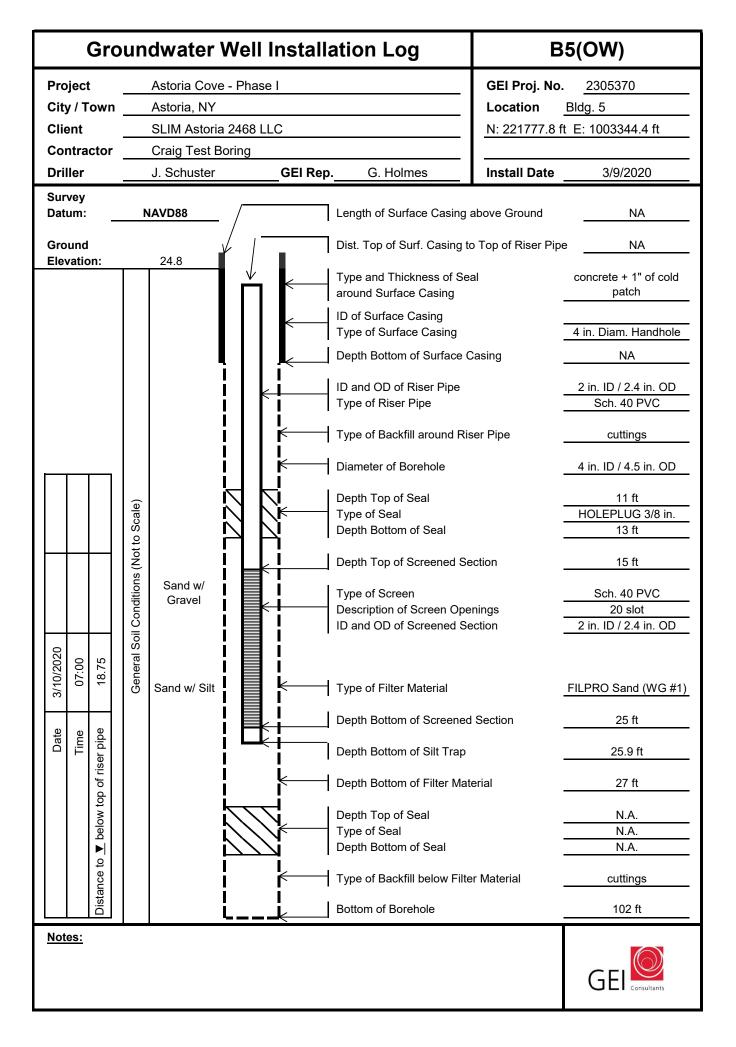
PAGE 4 of 4

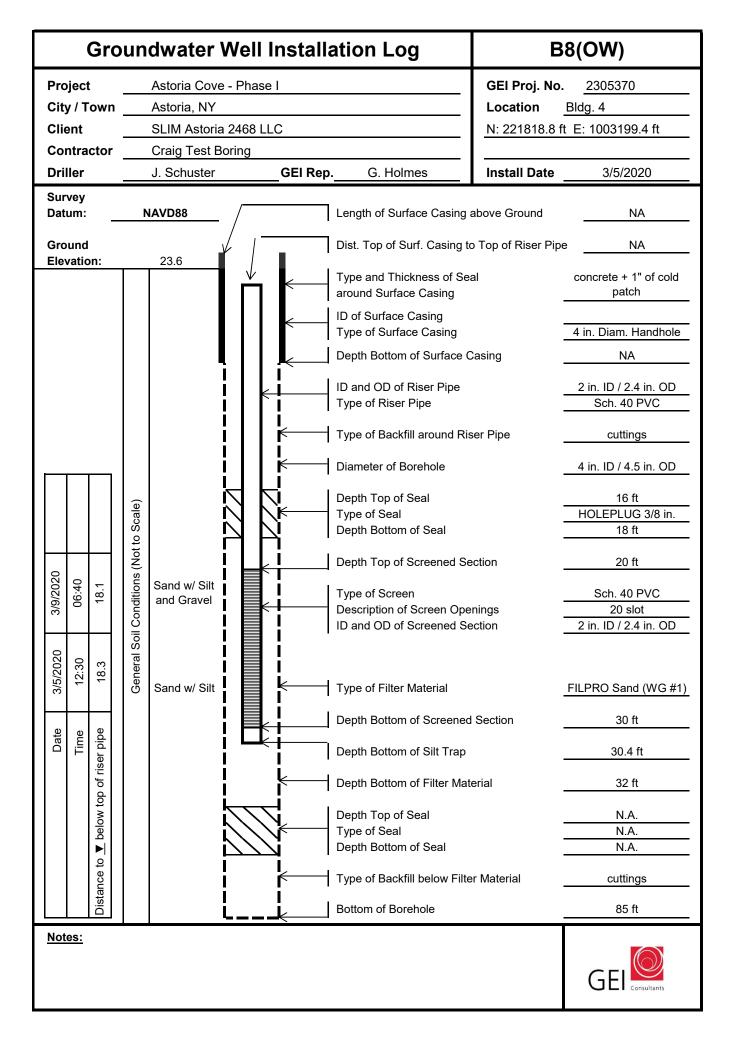


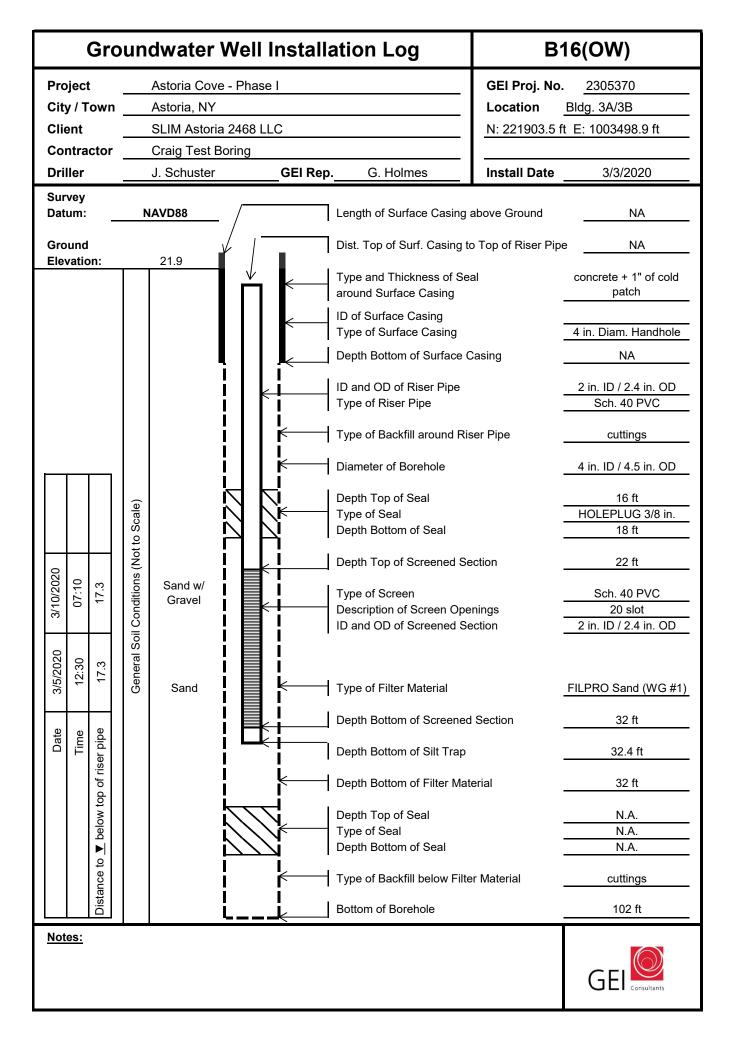
Geotechnical Report
Astoria Cove Development
Buildings 3A/3B, 4 & 5
Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909
Lot 35
Astoria, Queens County, New York
December 2023

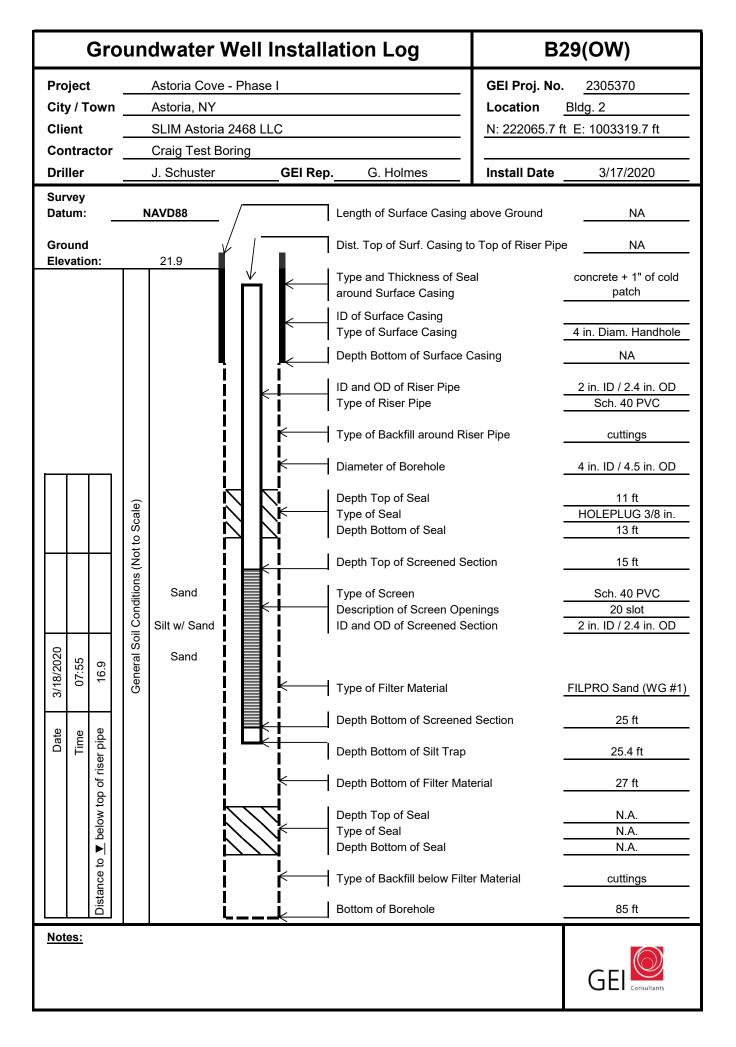
# **Appendix B**

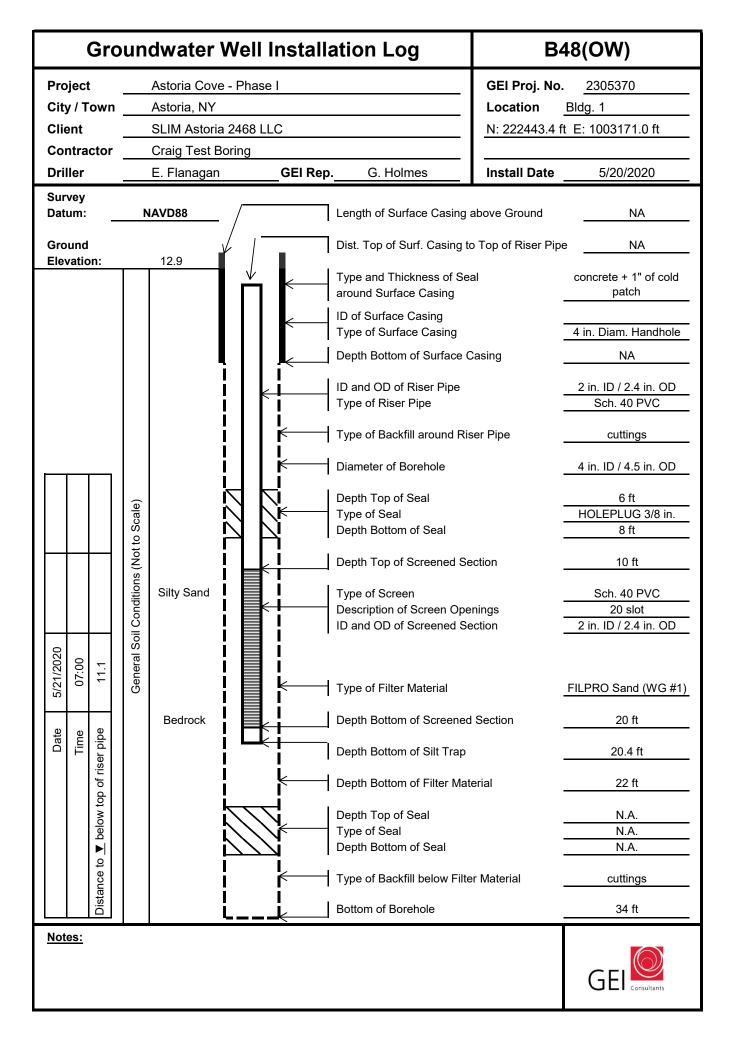
## **Well Installation Logs**

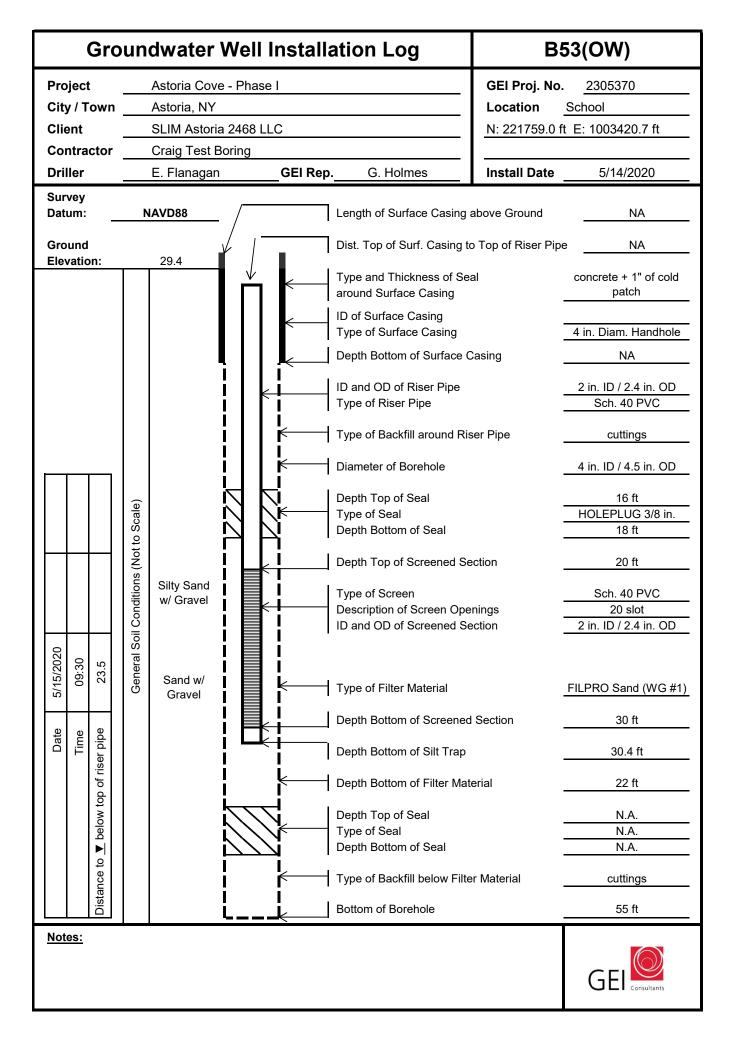








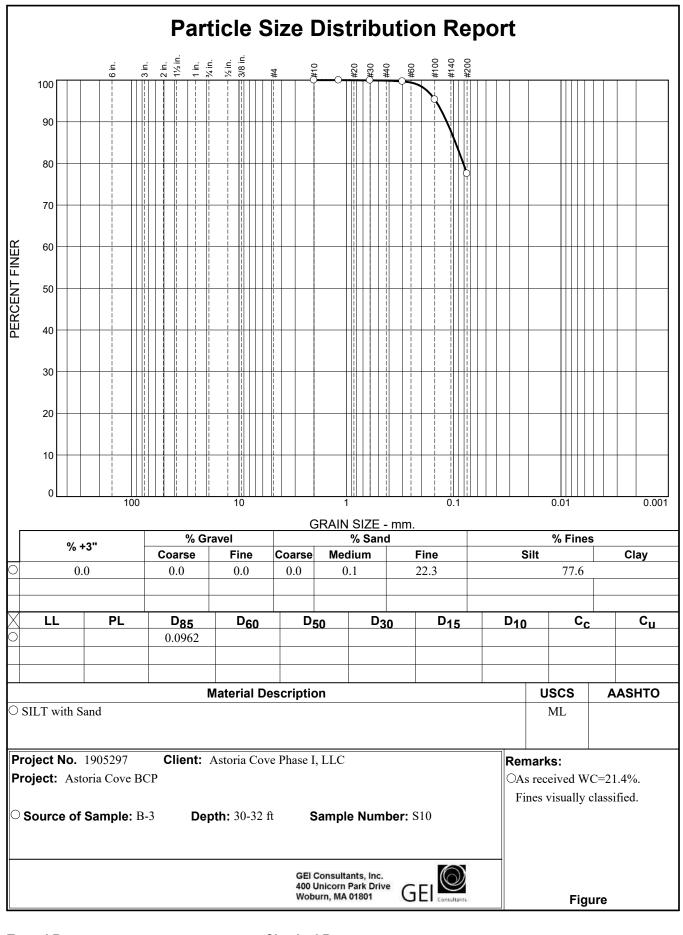


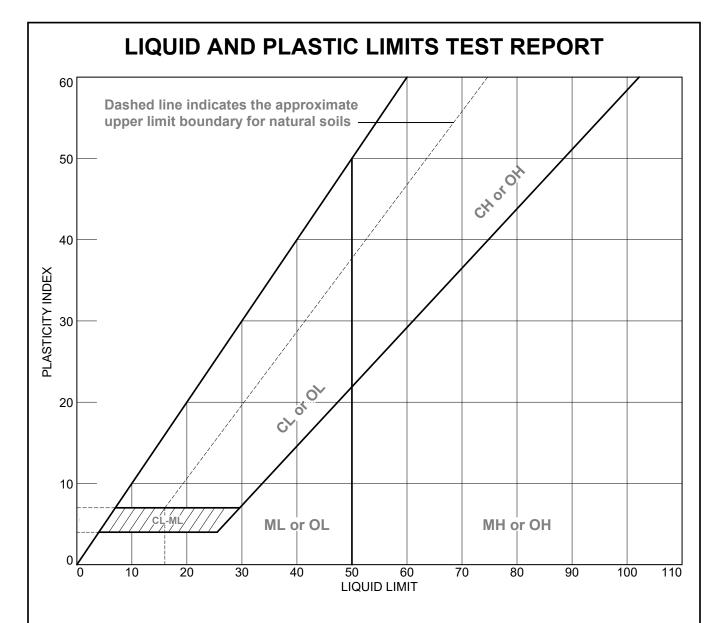


Geotechnical Report
Astoria Cove Development
Buildings 3A/3B, 4 & 5
Queens Block 906, Lots 1 & 5 | Block 908, Lot 12 | Block 909
Lot 35
Astoria, Queens County, New York
December 2023

# **Appendix C**

## **Laboratory Testing Results**





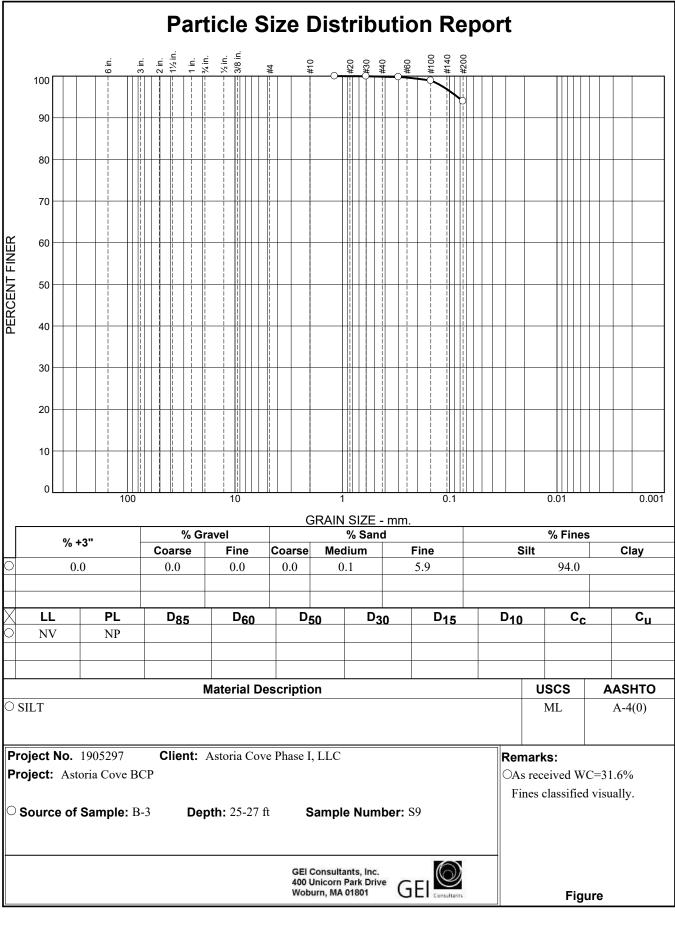
	SOIL DATA											
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	uscs				
•	B-3	S9	25-27 ft	31.4	NP	NV	NP	ML				

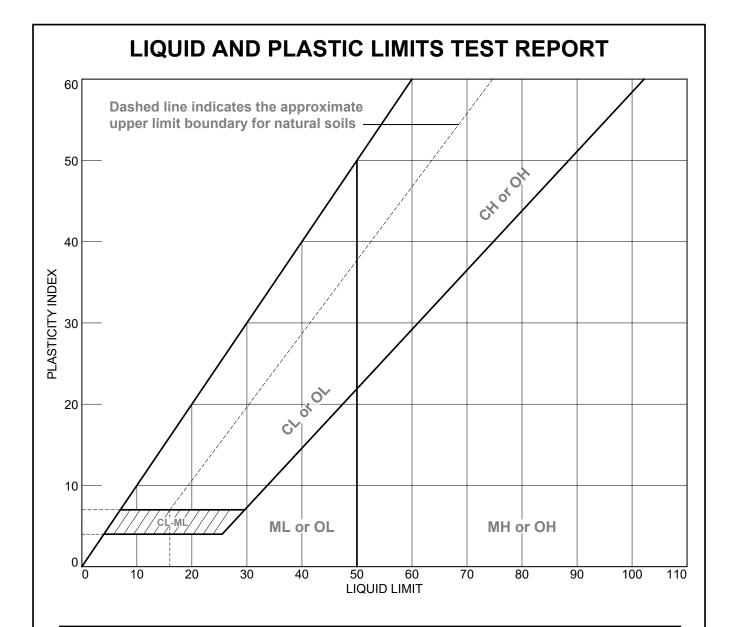
GEI Consultants, Inc.
400 Unicorn Park Drive
Woburn, MA 01801

**Client:** Astoria Cove Phase I, LLC

**Project:** Astoria Cove BCP

Project No.: 1905297 Figure





	SOIL DATA											
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	uscs				
•	B-5	S6	10-12 ft	30.6	NP	NV	NP	ML				

GEI Consultants, Inc.
400 Unicorn Park Drive
Woburn, MA 01801

GEI

**Client:** Astoria Cove Phase I, LLC

**Project:** Astoria Cove BCP

Project No.: 1905297 Figure

