Hubicki, Matthew S (DEC)

From: Hubicki, Matthew S (DEC) Sent: Friday, May 18, 2018 10:31 AM

To: 'Patrick McHugh'

Cc: Crosby, David (DEC); Deyette, Scott (DEC); Marc Godick; Barry White; Christopher Capece; Stephen

Grens; Stephen Schmid

RE: C360099 - Polychrome West Submittal #15 - ISS Mix Design Summary Subject:

Good Morning Patrick – I've reviewed this design mix summary for ISS Units and auger mixing (pile modifications). Looks like design criteria meets or exceeds for 1 X 10-6 (permeability) and 50 psi (compressive strength) for both.

I have no comments further on this ISS Mix Design Summary.

Thanks

Matt

Matthew Hubicki

Project Manager, Remedial Bureau C Division of Environmental Remediation

New York State Department of Environmental Conservation

625 Broadway, Albany, NY 12233-7014

P: 518-402-9605 | F: 518-402-9679 | matthew.hubicki@dec.ny.gov

www.dec.ny.gov | f | E





From: Patrick McHugh [mailto:pmchugh@akrf.com]

Sent: Thursday, May 17, 2018 11:14 AM

To: Hubicki, Matthew S (DEC) <matthew.hubicki@dec.ny.gov>

Cc: Crosby, David (DEC) <david.crosby@dec.ny.gov>; Deyette, Scott (DEC) <scott.deyette@dec.ny.gov>; Marc Godick <mgodick@akrf.com>; Barry White <barry white@avalonbay.com>; Christopher Capece

<Christopher Capece@avalonbay.com>; Stephen Grens <sgrens@akrf.com>; Stephen Schmid <sschmid@akrf.com>

Subject: C360099 - Polychrome West Submittal #15 - ISS Mix Design Summary

ATTENTION: This email came from an external source. Do not open attachments or click on links from unknown senders or unexpected emails

Good morning Matt,

Attached and for your records and/or review is a short write-up regarding the ISS Mix Design for the remedial activities at the Polychrome West Site prepared by Posillico Environmental's subcontractor Geo-Solutions.

The grout mixture will be the same for the slurry wall, ISS Units and auger mixing (pile modifications); however, the auger mixing will be a 15% addition by weight whereas the ISS Units and slurry wall will be a 20% addition by weight.

Please let us know if you have any questions, comments or concerns.

Thanks,

Pat

Patrick McHugh, P.E.

Environmental Engineer

.....

AKRF, INC.

Environmental, Planning, and Engineering Consultants

34 South Broadway , Suite 401 $\,\mid\,\,$ White Plains , NY 10601 P) 914.922.2387 $\,\mid\,\,$ C) 907.378.8737 $\,\mid\,\,$ F) 914.949.7559

www.akrf.com





Thursday, May 10, 2018

17-140

Submitted via e-mail: agomez@posillicoinc.com

Posillico Environmental, Inc. 131-36 20th Avenue College Point, NY 11356 Tel: 631-390-5745

Attn: Alex Gomez, P.E.

RE: Preliminary Design Mix Study Summary
Soil and Groundwater Remediation - In-Situ Soil Solidification
Polychrome East & West Sites
Yonkers, New York

Dear Mr. Gomez,

The following further describes preliminary data collected through the pre-construction laboratory study performed in support of the design for in-situ soil mixing of impacted soils associated with the Polychrome West and Polychrome East sites located in Yonkers, New York.

The overall objective of this study was to evaluate the results obtained, compare the data to the specified dosage of cementitious material and refine the reagent distribution and grout parameters to be used during full-production. This study specifically evaluated the specified performance criteria of strength and permeability of soil mixed material.

GSI's design mix study was developed to test and optimize addition rates of Blast Furnace Slag and Portland cement with site soil composites to create a mixture that is capable of being constructed, is able to achieve the performance requirements and provides an economical solution for solidification and stabilization of impacted soils. Our design mix study can be broken down into three phases;

- 1) Phase 1 Soil Classification
- 2) Phase 2 Composite Sample Proportioning
- 3) Phase 3 Laboratory Testing and Evaluation

Phase 1 – Soil Classification

Soil classification tests will be performed on samples collected from the designed treatment area(s). The following geotechnical properties are to be evaluated to assist GSI in classifying the materials;

- Moisture content (ASTM D 2216)
- Grainsize Distribution (ASTM D 422)
- Soil pH (EPA 600)
- Organic Content (ASTM D 2974)

Laboratory data sheets can be found in Appendix A.



Phase 2 – Composite Sample Preparation

Samples obtained through means selected by others were transported in sealed buckets from the site to GSI's office located in New Kensington, Pennsylvania. Based on visual classification during sample collection, soils were segregated into separate containers by soil type. After GSI is able to evaluate the material, "average" and "worst" case scenario composites were generated. These composites were be based on soil type, review of the boring logs provided in the bid documents, and the potential impact each soil type has on the cement's ability to achieve strength and permeability.

Proportioning of the composite samples was based on the percent of a soil type compared to the total weight of the composite sample. For example, assume an average case composite is made up of 45% silts/clays/organic soils and 65% of the sample is sands. A mix of 2,000 grams of a composite would result in the addition of 900 grams of silts/clays/organic soils from the bag samples and 1,100 grams of sands. The Blast Furnace Slag and Portland cement was then added based on the desired water to cement ratio in the form of a wet grout. No dry addition of cement is anticipated.

Phase 3 - Laboratory Testing and Evaluation

Composite samples for design mix evaluation were be molded into 2-inch by 4-inch plastic molds per ASTM D 1632 with variable dosages of Blast Furnace Slag and Portland cement to achieve the design strength and permeability. Samples were temporarily stored in coolers inside of GSI's laboratory with free water in the bottom of the cooler to control moisture and remain at room temperature. Samples were then tested at ages of 1-day, 3-day, 5-days with a pocket penetrometer to evaluate compressive strength while others were transported to an outside laboratory to be tested for unconfined compressive strength per ASTM D 2166 and hydraulic conductivity per ASTM D 5084 starting at an age of 14-days.

Design Mix Study Objectives

The primary objectives which GSI looks to accomplish from the pre-construction design mix study are;

- Create a mixture that is able to be cost effectively implemented in the field that achieves the specified post-treatment objectives.
- Refine reagent dosage and water to solids ratios to minimize cost and volume of spoils.
- Reduce the likelihood of re-work and change orders based on varying field conditions once full production commences.
- Create a soil-cement material with sufficient strength at an early age to support loading of GSI's equipment, and also accommodate future site use.
- Estimate the potential increase in volume (spoils) resulting from the ISS operation.
- Develop baseline quality control parameters that can be used by GSI throughout full-scale production to ensure a successful installation.

Specific performance criteria have been established for this project, which are presented in the following table. These allow for ideal laboratory conditions, and therefore, GSI will be targeting 1.5 to 2.0 times more than the standards required for UCS and an order of magnitude, or greater, for permeability during full-scale production.



Table 1: Performance Criteria

Parameter	Method	Requirement
Unconfined Compressive Strength	ASTM D 1633	50 psi (minimum)
Hydraulic Conductivity	ASTM D 5084	1x10 ⁻⁷ cm/sec

For the purposes of this preliminary design mix study summary, GSI is presenting the data currently available for the specified cement dosage of 20% by weight of soil. A summary of the laboratory testing results is provided below. Laboratory data sheets are included in **Appendix B**.

Table 2: 14-day Sample Results

Mix ID	BFS/PC Ratio	Total Cement (%)	Total Bentonite (%)	Cylinders 2"x4"	Cylinders 3"x6"	UCS 14-days (psi)	Permeability 14-days (cm/sec)
1	50/50	20%	0%	4	2	417.4	2.4x10 ⁻⁸
2	50/50	20%	1%	3	3	261.5	1.3x10 ⁻⁷
3	70/30	20%	0%	4	2	42.8	9.9x10 ⁻⁷

Note: Reagent dosage is based off of weight of soil.

This bench scale study demonstrated that a properly performed soil mixing operation can be used on the Polychrome East and West sites to stabilize the near surface soils in the designated improvement areas. The following conclusions may be drawn from the results:

- The sample materials consist of sands and silts with a fines content of approximately 38%.
- The soil pH is neutral (~7.5) and the organic content is elevated at approximately 4.33%.
- The target strength of 50 psi and permeability of 1x10⁻⁷ cm/sec, along with the GSI's factor for laboratory conditions, can be achieved by mixing the site soils with <u>20% cement (50/50 blend BFS to PC)</u>, added via water and cement grout.
- Mix 1 displayed in the table above is GSI's selected mix to take into the field for full production work.
- Careful planning and execution of the work in conjunction with a comprehensive quality control program is always recommended for soil mixing.

Conclusion

If you have any questions or comments regarding the information presented within this document, please do not hesitate to contact me directly. I will be available to discuss any questions you may have.

Sincerely.

Geo-Solutions, Inc.

Imant A Spllan

Vincent A. Spillane Project Manager



APPENDIX A



Geotechnical, Geosynthetic and Materials Testing and Research

938 South Central Avenue
Canonsburg, Pennsylvania, 15317
Tel: 724-746-4441 Fax: 724-745-4261
e-mail: jboschuk@jitlabs.com
www.jitlabs.com

April 16, 2018 18LS3660.01

GeoSolutions 1250 Fifth Avenue New Kensington, PA 15068

Attn: Josh Bonetto

RE: GEOTECHNICAL TEST RESULTS

YONKERS, NY 17-140

Dear Mr. Bonetto:

Submitted herein are the results of Sieve, pH and Organic Content performed on one (1) sample identified as 17-140-S1 for the above referenced project. All testing was performed per ASTM Standards while subject to JLT's internal QA / QC and data validation procedures.

We appreciate the opportunity of being of service to you and look forward to working with you again. Should you have any questions, comments or require additional information, please do not hesitate to call. Thank you.

Sincerely,

JLT LABORATORIES, INC.

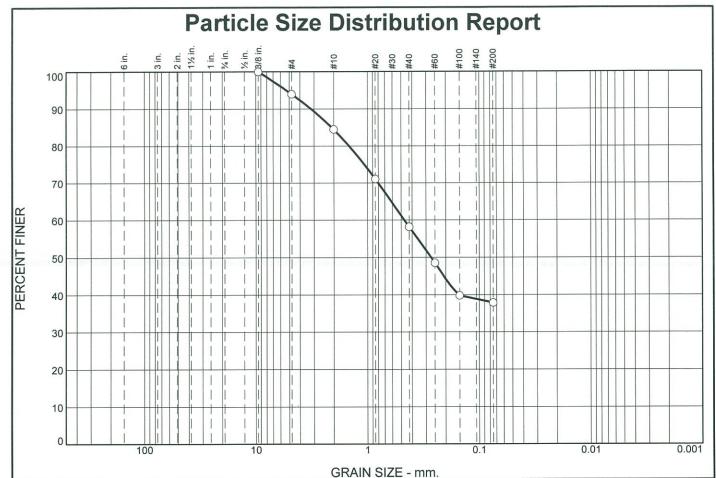
John Boschuk, Jr., P.E., C.F.E.

President

cc:

Colm & AP

Enclosures
JB\mlb
\MSWord\letter\1896
Inv# 6912



% Fines % Gravel % Sand % +3" Silt Clay Fine Coarse Fine Coarse Medium 20.3 37.9 0.0 0.0 6.0 9.5 26.3

	SIEVE	PERCENT	SPEC.*	PASS?
	SIZE	FINER	PERCENT	(X=NO)
-[0.375	100.0		
	#4	94.0		
	#10	84.5		
	#20	71.1		
	#40	58.2		
	#60	48.6		
	#100	39.8		
	#200	37.9		
		5-2" (00)012990		
L				

Material Description						
	Atterberg Limits					
PL=	LL=	PI=				
D ₉₀ = 3.1782 D ₅₀ = 0.2696 D ₁₀ =	Coefficients D ₈₅ = 2.0768 D ₃₀ = C _u =	D ₆₀ = 0.4680 D ₁₅ = C _c =				
USCS=	Classification AASH1	го=				
	Remarks					

(no specification provided)

Location: Yonkers (17-140) Sample Number: 17-140-S1

Date: 04/16/2018

JLT Laboratories, Inc.

Client: GeoSolutions

Project: Yonkers (17-140)

Canonsburg, PA

Project No: 18LS3660.01

Figure

Tested By: AE

Checked By: JB



MOISTURE, ASH AND ORGANIC CONTENT and pH

Organic Content per ASTM D 2974 Methods A and D and pH per ASTM D-4972

Client: GeoSolutions
Project: 17-140 Yonkers NY
Imple ID: Sample 17-140-S1

Job No. : 18LS3660.01 Date : 10/16/2018 Perf'd By : MLB / AE Chk'd By : JBJr

Bulk Sample Moisture Content = 19.46 %

MOISTURE CONTENT @ 105 Deg "C'

		Rep 1	RFep 2	Rep 3	
Tare ID	Units	2	3	4	
Wet Soil + Tare	grs				
Dry Soil + Tare	grs	121.2629	118.7262	110.6127	
Tare	grs	53.7331	50.5762	56.4965	
Water Loss	grs				
Dry Soil	grs	67.5298	68.1500	54.1162	
Moisture Content	%	0.0000	0.0000	0.0000	

ASH and ORGANIC CONTENT @ 750 Deg "C'

		Rep 1	RFep 2	Rep 3	
Tare ID		2	3	4	
Oven Dry Soil + Tare	grs	121.2629	118.7262	110.6127	
Furnace Dry Soil + Tare	grs	118.4300	115.5692	108.3323	
Tare	grs	53.7331	50.5762	56.4965	
Oven Dry Soil	grs	67.5298	68.1500	54.1162	
Furnace Dry Soil (Ash)	grs	64.6969	64.9930	51.8358	
Ash Content	%	95.8050	95.3676	95.79	
Organic Content	%	4.20	4.63	4.21	

pH Test Data

Due to the limited quantity of material in the split-spoon jars, only one replicate could be performed.



938 S. Central Ave, Canonsburg, Pa. 15317 Tel: 724-746-4441 Fax: 724-745-4261



APPENDIX B



UNCONFINED COMPRESSIVE STRENGTH

ASTM D2166-16 / AASHTO T208-10 (Modified-Peak Load Only) (SOP S-30)

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Reference:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M1

 Project No.:
 2018-248-001
 Sample No.: 15 Day

Lab ID: 2018-248-001-001 Visual Description: Gray Stabilized Material

INITIAL SAMPLE DIMENSIONS				
Length 1 (in):	3.764	Top Dia. (in):	2.008	
Length 2 (in):	3.764	Mid. Dia. (in):	2.007	
Length 3 (in):	3.763	Bot. Dia. (in):	2.005	
Avg. Length (in):	3.764	Area (in ²):	3.163	

WATER CONTENT	
(AFTER TEST)	
Tare No.:	3164
Weight of Tare & Wet Sample (g):	376.32
Weight of Tare & Dry Sample (g):	297.72
Weight of Tare (g):	8.11
% Moisture:	27.14

UNIT WEIGHT					
Weight of Tube & Wet Sample (g):	368.7	Sample Volume (cm ³):	195.1		
Weight of Tube (g):	0.00	Unit Wet Weight (g/cm ³):	1.89		
Weight of Wet Sample (g):	368.7	Unit Wet Weight (pcf):	117.95		
Avg. Diameter (in):	2.01	Moisture Content (%):	27.14		
Avg. Length (in):	3.76	Unit Dry Weight (pcf):	92.77		
Avg. Length (cm):	9.56				

ELECTRONIC DEVICE LOAD (Ib)

STRESS (psi)

1320 417.38

Tested By JAC Date 4/24/18 Input Checked By KC Date 4/25/18

page 1 of 1

DCN: CT-S30A Date: 11/09/09 Revision: 4



UNCONFINED COMPRESSIVE STRENGTH

ASTM D2166-16 / AASHTO T208-10 (Modified-Peak Load Only) (SOP S-30)

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Reference:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M2

 Project No.:
 2018-248-001
 Sample No.: 15 Day

Lab ID: 2018-248-001-002 Visual Description: Gray Stabilized Material

INITIAL SAMPLE DIMENSIONS				
Length 1 (in):	3.754	Top Dia. (in):	2.007	
Length 2 (in):	3.754	Mid. Dia. (in):	2.004	
Length 3 (in):	3.754	Bot. Dia. (in):	2.002	
Avg. Length (in):	3.754	Area (in ²):	3.155	

WATER CONTENT	
(AFTER TEST)	
Tare No.:	3095
Weight of Tare & Wet Sample (g):	370.39
Weight of Tare & Dry Sample (g):	290.35
Weight of Tare (g):	8.16
% Moisture:	28.36

UNIT WEIGHT					
Weight of Tube & Wet Sample (g):	362.6	Sample Volume (cm ³):	194.1		
Weight of Tube (g):	0.00	Unit Wet Weight (g/cm ³):	1.87		
Weight of Wet Sample (g):	362.6	Unit Wet Weight (pcf):	116.57		
Avg. Diameter (in):	2.00	Moisture Content (%):	28.36		
Avg. Length (in):	3.75	Unit Dry Weight (pcf):	90.81		
Avg. Length (cm):	9.54				

ELECTRONIC DEVICE LOAD (lb)

STRESS (psi)

825 261.47

Tested By JAC Date 4/24/18 Input Checked By KC Date 4/25/18

page 1 of 1

DCN: CT-S30A Date: 11/09/09 Revision: 4



UNCONFINED COMPRESSIVE STRENGTH

ASTM D2166-16 / AASHTO T208-10 (Modified-Peak Load Only) (SOP S-30)

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Reference:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M3

 Project No.:
 2018-248-001
 Sample No.: 15 Day

Lab ID: 2018-248-001-003 Visual Description: Gray Stabilized Material

INITIAL SAMPLE DIMENSIONS						
Length 1 (in):	3.748	Top Dia. (in):	2.008			
Length 2 (in):	3.746	Mid. Dia. (in):	2.002			
Length 3 (in):	3.746	Bot. Dia. (in):	2.000			
Avg. Length (in):	3.747	Area (in ²):	3.152			

WATER CONTENT				
(AFTER TEST)				
Tare No.:	3242			
Weight of Tare & Wet Sample (g):	367.22			
Weight of Tare & Dry Sample (g):	283.09			
Weight of Tare (g):	8.46			
% Moisture:	30.63			

UNIT WEIGHT					
Weight of Tube & Wet Sample (g):	361.0	Sample Volume (cm ³):	193.5		
Weight of Tube (g):	0.00	Unit Wet Weight (g/cm ³):	1.87		
Weight of Wet Sample (g):	360.98	Unit Wet Weight (pcf):	116.39		
Avg. Diameter (in):	2.00	Moisture Content (%):	30.63		
Avg. Length (in):	3.75	Unit Dry Weight (pcf):	89.10		
Avg. Length (cm):	9.52				

ELECTRONIC DEVICE LOAD (lb)

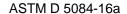
STRESS (psi)

135 42.83

Tested By JAC Date 4/24/18 Input Checked By KC Date 4/25/18

page 1 of 1

DCN: CT-S30A Date: 11/09/09 Revision: 4





 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

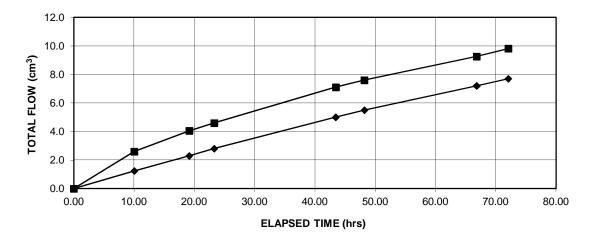
 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M1

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-001
 Avg. Conf. Pressure (psi): 6.25

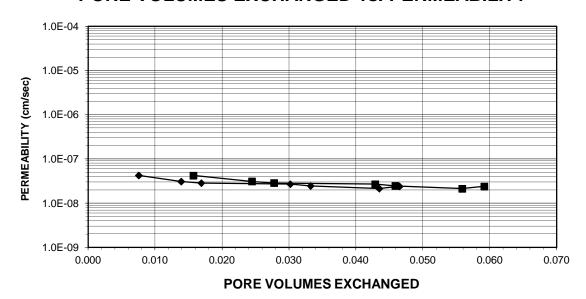
AVERAGE PERMEABILITY = 2.4E-08 cm/sec @ 20°C AVERAGE PERMEABILITY = 2.4E-10 m/sec @ 20°C

TOTAL FLOW vs. ELAPSED TIME



→ INFLOW — OUTFLOW

PORE VOLUMES EXCHANGED vs. PERMEABILITY



Tested By: RPE Date: 4/24/18 Checked By: KC Date: 5/7/18



ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M1

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-001
 Avg. Conf. Pressure (psi): 6.25

Specific Gravity: 2.70 Assumed Sample Condition: Previously Remolded

Visual Description: Dark Gray Stabilized Material

Permeant Type: Deaired Water

MOISTURE CONTENT:	BEFORE TEST	AFTER TEST
Tare Number	565	1728
Weight of Tare & Wet Sample (g)	381.56	765.57
Weight of Tare & Dry Sample (g)	315.06	611.93
Weight of Tare (g)	82.40	81.38
Weight of Water (g)	66.50	153.64
Weight of Dry Sample (g)	232.66	530.55
Moisture Content (%)	28.6	29.0

SPECIMEN:	BEFORE TEST	AFTER TEST
Mainte of T. has 0 Mat Onwale (a)	202 72	NIA
Weight of Tube & Wet Sample (g)	680.76	NA
Weight of Tube (g)	0.00	NA
Weight of Wet Sample (g)	680.76	682.75
Length 1 (in)	3.058	3.035
Length 2 (in)	3.035	3.046
Length 3 (in)	3.040	3.050
Top Diameter (in)	3.024	3.041
Middle Diameter (in)	3.039	3.039
Bottom Diameter (in)	3.038	3.035
Average Length (in)	3.04	3.04
Average Length (in)		
Average Area (in ²)	7.23	7.25
Sample Volume (cm ³)	360.59	361.63
Unit Wet Weight (g/cm ³)	1.89	1.89
Unit Wet Weight (pcf)	117.8	117.9
Unit Dry Weight (pcf)	91.6	91.4
Unit Dry Weight (g/cm³)	1.47	1.46
Void Ratio, e	0.84	0.84
Porosity, n	0.46	0.46
Pore Volume (cm ³)	164.5	165.5
Total Weight of Sample After Test (g)		691.10

Page 2 of 3 DCN: CT-22 DATE: 1/1/17 REVISION: 11

RPE

Tested By:

Date:

5/7/18

Checked By:

KC

4/24/18

Date:



ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M1

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-001
 Avg. Conf. Pressure (psi): 6.25

Pressure Heads (Co	onstant)	Final Sample Dimensions			
Top Cap (psi)	67.5	Sample Length (cm), L	7.73		
Bottom Cap (psi)	70.0	Sample Diameter (cm)	7.72		
Cell (psi)	75.0	Sample Area (cm ²), A	46.78		
Total Pressure Head (cm)	175.8	Inflow Burette Area (cm ²), a-in	0.918		
Hydraulic Gradient	22.73	Outflow Burette Area (cm ²), a-out	0.904		
		B Parameter (%)	97		

AVERAGE PERMEABILITY = 2.4E-08 cm/sec @ 20°C AVERAGE PERMEABILITY = 2.4E-10 m/sec @ 20°C

DATE	TIN	ИE	ELAPSED TIME	TOTAL INFLOW	TOTAL OUTFLOW	TOTAL HEAD	FLOW	TEMP.	INCREMENTAL PERMEABILITY
			t			h	(0 flow)		@ 20°C
(mm/dd/yy)	(hr)	(min)	(hr)	(cm ³)	(cm ³)	(cm)	(1 stop)	(°C)	(cm/sec)
5/01/18	12	44	0.000	0.0	0.0	202.0	0	21.5	NA
5/01/18	22	46	10.033	1.3	2.6	197.7	0	21.5	4.2E-08
5/02/18	7	53	19.150	2.3	4.1	195.0	0	21.3	3.1E-08
5/02/18	12	1	23.283	2.8	4.6	193.9	0	21.7	2.9E-08
5/03/18	8	11	43.450	5.0	7.1	188.7	0	21.4	2.7E-08
5/03/18	12	56	48.200	5.5	7.6	187.6	0	21.5	2.5E-08
5/04/18	7	34	66.833	7.2	9.3	184.0	0	21.3	2.1E-08
5/04/18	12	49	72.083	7.7	9.8	182.8	1	21.6	2.4E-08

Tested By: RPE Date: 4/24/18 Checked By: KC Date: 5/7/18

Page 3 of 3





 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

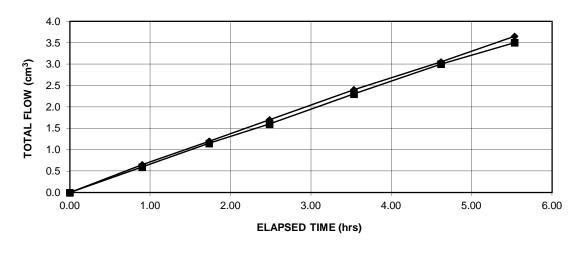
 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M2

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-002
 Avg. Conf. Pressure (psi): 6.25

AVERAGE PERMEABILITY = 1.3E-07 cm/sec @ 20°C AVERAGE PERMEABILITY = 1.3E-09 m/sec @ 20°C

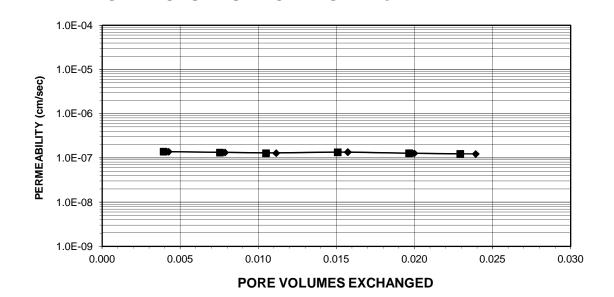
TOTAL FLOW vs. ELAPSED TIME



PORE VOLUMES EXCHANGED vs. PERMEABILITY

- OUTFLOW

- INFLOW



Tested By: RPE Date: 4/24/18 Checked By: KC Date: 4/30/18

Page 1 of 3 DCN: CT-22 DATE: 1/1/17 REVISION: 11



ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M2

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-002
 Avg. Conf. Pressure (psi): 6.25

Specific Gravity: 2.70 Assumed Sample Condition: Previously Remolded

Visual Description: Dark Gray Stabilized Material

Permeant Type: Deaired Water

MOISTURE CONTENT:	BEFORE TEST	AFTER TEST
Tare Number	1125	901
Weight of Tare & Wet Sample (g)	285.01	725.18
Weight of Tare & Dry Sample (g)	239.63	580.08
Weight of Tare (g)	83.60	110.35
Weight of Water (g)	45.38	145.10
Weight of Dry Sample (g)	156.03	469.73
Moisture Content (%)	29.1	30.9

SPECIMEN:	BEFORE TEST	AFTER TEST
Weight of Tube & Wet Sample (g)	612.09	NA
Weight of Tube (g)	0.00	NA
Weight of Wet Sample (g)	612.09	620.65
Length 1 (in)	2.755	2.768
Length 2 (in)	2.783	2.772
Length 3 (in)	2.773	2.756
Top Diameter (in)	3.040	3.032
Middle Diameter (in)	3.042	3.039
Bottom Diameter (in)	3.031	3.040
Average Length (in)	2.77	2.77
Average Area (in ²)	7.25	7.24
Sample Volume (cm ³)	329.01	328.27
Unit Wet Weight (g/cm ³)	1.86	1.89
Unit Wet Weight (pcf)	116.1	118.0
Unit Dry Weight (pcf)	90.0	90.2
Unit Dry Weight (g/cm³)	1.44	1.44
Void Ratio, e	0.87	0.87
Porosity, n	0.47	0.47
Pore Volume (cm ³)	153.4	152.6
Total Weight of Sample After Test (g)		620.76

 Tested By:
 RPE
 Date:
 4/24/18
 Checked By:
 KC
 Date:
 4/30/18

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 DCN: CT-22 DATE: 1/1/17 REVISION: 11
 permflow.xls



ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M2

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-002
 Avg. Conf. Pressure (psi): 6.25

Pressure Heads (Co	onstant)	Final Sample Dimensions			
Top Cap (psi)	67.5	Sample Length (cm), L	7.02		
Bottom Cap (psi)	70.0	Sample Diameter (cm)	7.71		
Cell (psi)	75.0	Sample Area (cm ²), A	46.74		
Total Pressure Head (cm)	175.8	Inflow Burette Area (cm ²), a-in	0.897		
Hydraulic Gradient	25.02	Outflow Burette Area (cm ²), a-out	0.878		
•		B Parameter (%)	96		

AVERAGE PERMEABILITY = 1.3E-07 cm/sec @ 20°C AVERAGE PERMEABILITY = 1.3E-09 m/sec @ 20°C

DATE	TIN	ΛE	ELAPSED	TOTAL	TOTAL	TOTAL	FLOW	TEMP.	INCREMENTAL
			TIME	INFLOW	OUTFLOW	HEAD			PERMEABILITY
			t			h	(0 flow)		@ 20°C
(mm/dd/yy)	(hr)	(min)	(hr)	(cm ³)	(cm ³)	(cm)	(1 stop)	(°C)	(cm/sec)
4/26/18	10	23	0.000	0.0	0.0	202.7	0	20.8	NA
4/26/18	11	17	0.900	0.7	0.6	201.3	0	20.9	1.4E-07
4/26/18	12	7	1.733	1.2	1.2	200.0	0	20.9	1.3E-07
4/26/18	12	52	2.483	1.7	1.6	199.0	0	20.9	1.3E-07
4/26/18	13	55	3.533	2.4	2.3	197.4	0	21.0	1.4E-07
4/26/18	15	0	4.617	3.1	3.0	195.9	0	21.0	1.3E-07
4/26/18	15	55	5.533	3.7	3.5	194.7	1	21.1	1.2E-07

Tested By: RPE Date: 4/24/18 Checked By: KC Date: 4/30/18

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 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

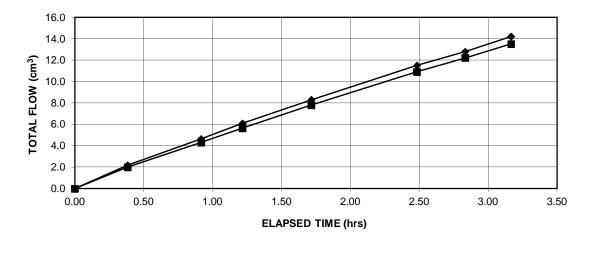
 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M3

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-003
 Avg. Conf. Pressure (psi): 6.25

AVERAGE PERMEABILITY = 9.9E-07 cm/sec @ 20°C AVERAGE PERMEABILITY = 9.9E-09 m/sec @ 20°C

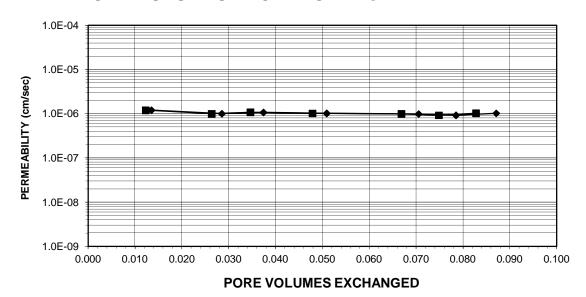
TOTAL FLOW vs. ELAPSED TIME



- INFLOW

PORE VOLUMES EXCHANGED vs. PERMEABILITY

- OUTFLOW



Tested By: RPE Date: 4/24/18 Checked By: KC Date: 4/27/18

Page 1 of 3 DCN: CT-22 DATE: 1/1/17 REVISION: 11



ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M3

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-003
 Avg. Conf. Pressure (psi): 6.25

Specific Gravity: 2.70 Assumed Sample Condition: Previously Remolded

Visual Description: Dark Gray Stabilized Material

Permeant Type: Deaired Water

MOISTURE CONTENT:	BEFORE TEST	AFTER TEST	
Tare Number	1706	606	
Weight of Tare & Wet Sample (g)	402.96	744.56	
Weight of Tare & Dry Sample (g)	327.99	591.40	
Weight of Tare (g)	82.68	85.13	
Weight of Water (g)	74.97	153.16	
Weight of Dry Sample (g)	245.31	506.27	
Moisture Content (%)	30.6	30.3	

SPECIMEN:	BEFORE TEST	AFTER TEST
Weight of Tube & Wet Sample (g)	661.87	NA
Weight of Tube (g)	0.00	NA
Weight of Wet Sample (g)	661.87	660.31
Length 1 (in)	2.968	2.981
Length 2 (in)	2.972	2.972
Length 3 (in)	2.985	2.950
Top Diameter (in)	3.030	3.037
Middle Diameter (in)	3.041	3.030
Bottom Diameter (in)	3.040	3.025
Average Length (in)	2.98	2.97
Average Area (in ²)	7.24	7.21
Sample Volume (cm ³)	353.16	350.82
Unit Wet Weight (g/cm ³)	1.87	1.88
Unit Wet Weight (pcf)	117.0	117.5
Unit Dry Weight (pcf)	89.6	90.2
Unit Dry Weight (g/cm³)	1.44	1.45
Void Ratio, e	0.88	0.87
Porosity, n	0.47	0.46
Pore Volume (cm ³)	165.4	163.1
Total Weight of Sample After Test (g)		663.78

 Tested By:
 RPE
 Date:
 4/24/18
 Checked By:
 KC
 Date:
 4/27/18

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 DCN: CT-22 DATE: 1/1/17 REVISION: 11
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ASTM D 5084-16a

 Client:
 Geo-Solutions, Inc.
 Boring No.: 4/9/18

 Client Project:
 Yonkers, NY 17-140
 Depth (ft): 17-140-M3

 Project No.:
 2018-248-001
 Sample No.: 15 Day

 Lab ID No.:
 2018-248-001-003
 Avg. Conf. Pressure (psi): 6.25

Pressure Heads (Co	onstant)	Final Sample Dimensions		
Top Cap (psi)	67.5	Sample Length (cm), L	7.54	
Bottom Cap (psi)	70.0	Sample Diameter (cm)	7.70	
Cell (psi)	75.0	Sample Area (cm ²), A	46.54	
Total Pressure Head (cm)	175.8	Inflow Burette Area (cm ²), a-in	0.877	
Hydraulic Gradient	23.32	Outflow Burette Area (cm ²), a-out	0.960	
		B Parameter (%)	96	

AVERAGE PERMEABILITY = 9.9E-07 cm/sec @ 20°C AVERAGE PERMEABILITY = 9.9E-09 m/sec @ 20°C

DATE	TIN	ЛE	ELAPSED	TOTAL	TOTAL	TOTAL	FLOW	TEMP.	INCREMENTAL
			TIME	INFLOW	OUTFLOW	HEAD			PERMEABILITY
			t			h	(0 flow)		@ 20°C
(mm/dd/yy)	(hr)	(min)	(hr)	(cm ³)	(cm ³)	(cm)	(1 stop)	(°C)	(cm/sec)
4/26/18	10	22	0.000	0.0	0.0	203.4	0	20.8	NA
4/26/18	10	45	0.383	2.2	2.0	198.7	0	20.9	1.2E-06
4/26/18	11	17	0.917	4.7	4.3	193.5	0	20.9	1.0E-06
4/26/18	11	35	1.217	6.1	5.7	190.5	0	20.9	1.1E-06
4/26/18	12	5	1.717	8.3	7.8	185.7	0	20.9	1.0E-06
4/26/18	12	51	2.483	11.5	10.9	178.8	0	20.9	1.0E-06
4/26/18	13	12	2.833	12.8	12.2	175.9	0	20.9	9.3E-07
4/26/18	13	32	3.167	14.2	13.5	173.0	1	20.9	1.0E-06

Tested By: RPE Date: 4/24/18 Checked By: KC Date: 4/27/18