

Geotechnical & Foundations Land Planning Geo-Structural Environmental Water Resources

Principals:

January 22, 2015 N-8949

Mr. Joseph Hogan

Armonk, NY 10504

Saber Chauncey WP, LLC

via email: jhogan@saberfund.com

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Consultant:

RE: Geotechnical Investigation Report

Proposed Mixed Use Development - The Collection

80 Westchester Avenue

80 Business Park Drive - Suite 100

City of White Plains, New York

SESI Project No. 8949

Dear Mr. Hogan:

In accordance with our Professional Services Agreement dated October 20, 2014, we have completed our geotechnical engineering evaluation of the subsurface soil and groundwater conditions for the proposed development to be constructed at the above referenced site. Enclosed are five (5) copies of the report.

If you have any questions, please call.

Sincerely,

SESI CONSULTING ENGINEERS, PC

John M. Nederfield, P.E.

Project Engineer

Cc: Richard DeCola via email: rideco@live.com

Michael St. Pierre, P.E.

Vice President

N:/Projects/8949/Reports/jn8949georpt.doc



GEOTECHNICAL INVESTIGATION AND REPORT FOR

The Collection
80 Westchester Avenue
City of White Plains, Westchester County, New York

PREPARED FOR:

SABER CHAUNCEY WP, LLC 80 Business Park Drive – Suite 100 Armonk, NY 10504

PREPARED BY:

SESI CONSULTING ENGINEERS, PC 12A Maple Avenue Pine Brook, NJ 07058

Job No.: N-8949

DATE: January 22, 2015

John M. Nederfield, P.E.

Michael St. Pierre, P.E. NY Lic. No. 802711

SITE DESCRIPTION AND PROPOSED CONSTRUCTION

We have completed our geotechnical investigation for the proposed development to be known as "The Collection" to be located at 80 Westchester Avenue in White Plains, New York. The site is bounded to the south by Westchester Avenue, to the north by Franklin Avenue, to the east by Avis Rent a Car and a baseball field, and to the west by a diner and a multi-residential building. The site is currently being utilized as a municipal parking lot, an auto dealership with service garage, an abandoned auto dealership, retail stores, and a restaurant, with associated parking lots and roadways.

Based on the Site Plans prepared by Antunovich Associates - Consultants dated August 28, 2014, we understand that the proposed construction will consist of a 14,370 sf restaurant, a hotel, a multi-story mixed-use building with restaurants, retail space, and parking below, several retail stores, an auto dealership, two auto dealership service centers, and two below grade parking levels, and a parking deck. The multi-story residential building will consist of 261 units that will occupy the fourth to eleventh floors. The second and third floors will be occupied by residential parking. The first floor will be occupied by the residential lobby, retail space, and a restaurant. Additionally, one below grade parking level will be provided beneath the restaurant/retail/lobby footprint. A covered pedestrian access path will be provided along the eastern side of the property connecting Westchester Avenue to Franklin Avenue.

We understand that the proposed construction will be completed in two phases. The first phase will consist of the parking deck, two auto dealership service centers, an auto dealership, partial hotel, and retail space all located on the eastern half of the site. The second phase will consist of parking decks, retail space, a restaurant, partial hotel, and multi-story residential on the western half of the site.

Based on the existing grades shown on the Composite Survey of Property prepared by Ward Carpenter Engineers, Inc. dated July 3, 2012, the site grades slope and/or step gently to moderately downward from a high elevation (EL) of EL 176 at the northwestern corner of the site down to a low elevation of EL 153 at the northeastern corner of the site. Typically, the site slopes downward from west to east and south to north. Several concrete walls and one masonry block wall were observed on the property. We also understand that a storm sewer may traverse the eastern side of the site.

Based on a review of the topographic information shown on the survey and proposed site grades, it appears that cuts of up to 26± feet will be required to reach proposed lower floor grades for the proposed development.

We have not been provided with the proposed column, footing or floor loads at this time. Once the loads are known, we should be provided the opportunity to review these loads to confirm our recommendations.

FIELD AND LABORATORY INVESTIGATIONS

Our engineering study consisted of a site reconnaissance, a review of existing soils and geologic data, a review of borings completed by Soil Testing, Inc., and a field investigation consisting of the drilling of fifteen (15) soil borings. The borings were drilled from October 3 through October 20, 2014 to depths of 32 to 70± feet below the ground surface using a truck-mounted drill rig and an ATV-mounted drill rig. A groundwater monitoring well was installed in boring SB-2. Ground surface elevations were interpolated from the contours shown on the Ward Carpenter Survey.

The locations of the borings are shown on the *Boring Location Plan*, which is included as *Figure 1*. Individual soil boring logs, which describe the materials encountered, are presented as *Figures 2 through 16*. A key to soil terminology is included as *Figure 17*.

Soil samples suitable for identification purposes were extracted from the borings at closely spaced intervals in accordance with the procedures of the Standard Penetration Test (ASTM D1586). For this test, a standard split-spoon sampler (2 inches outside diameter; one and three-eighths inches inside diameter) is driven into the soil by a 140-pound weight falling 30 inches. After discounting the initial six inches of penetration due to possible disturbance of the material resulting from the drilling operation, the number of blows required to drive the sampler a distance of 12 inches is recorded and designated as the standard penetration resistance or "N value". The "N value" provides an indication of the relative compactness of the soil in-situ.

All fieldwork was performed under the full time technical observation of an engineer from SESI Consulting Engineers, PC. Our representative located the borings in the field, maintained continuous logs of the explorations as work proceeded, and coordinated the soil sampling operations in order to develop the required subsurface information.

All soil samples were taken to our soils laboratory for classification and appropriate geotechnical testing. Laboratory testing consisted of two (2) water content determinations, and two (2) mechanical grain size analyses. The results of the water contents are presented on the individual boring logs. The results of the mechanical grain size analyses are presented in graphical form as *Figures 18 and 19*.

Six (6) borings that were completed between November 2012 and December 2012 and twelve (12) borings that were completed March 2014, by Soil Testing, Inc., are attached in the Appendix.

SUBSURFACE CONDITIONS

Based on our investigation and the information provided on the soil borings performed by others, the subsurface conditions vary considerably throughout the site. The subsurface conditions encountered at the site generally consist of:

<u>Surficial Materials:</u> A 1 to 4-inch thick layer of asphalt was encountered at the surface in most of the borings. Borings B-3, B-4, B-5, B-6, B-8, B-9, and B-10 were drilled through a concrete slab with thicknesses ranging from 5 to 8± inches. (Borings B-6, B-8, and B-10 did not indicate the thickness of the concrete slab on the boring logs.)

<u>Fill:</u> A miscellaneous fill was encountered below the asphalt in the majority of the borings consisting of varying percentages of sand, silt, gravel, and brick, asphalt fragments, wood, concrete, plastic, etc., ranging in depth from approximately 3 to 16± feet below the ground surface, where encountered. It should be anticipated that fill may be encountered at other areas from previous site development.

Natural Soils: Beneath the fill, where encountered, the site contains sand, with varying amounts of silt, and gravel with boulders and fractured/weathered rock. This stratum extends to depths ranging from 13 to 56± feet below existing grade. Based on the blow counts obtained during the soil sampling operation, this layer is in a loose to dense condition.

Bedrock: Bedrock was encountered in all the borings, except SB-3, at depths ranging from 15 to 66± feet below the ground surface. In many areas, the upper portion of the bedrock was weathered. Refusal to advance the casing through a possible boulder from 45 to 50 feet below the ground surface was encountered in boring SB-3. Based on the information provided on the boring logs, the bedrock has been identified as schist and gneiss. The bedrock has also been classified as very poor to excellent, based on the Rock Quality Designation (RQD) calculated from the measurements obtained from each rock core. The top of the bedrock generally slopes downward from west to east based on the information provided on the boring logs. Specifically, the bedrock slopes downward from the west, north, and south to the middle of the site along the eastern end. The bedrock was encountered at a high EL 156± in borings B-12 and at a low elevation of EL 88± in borings SB-9.

Rock cores were taken on the assumed bedrock at borings SB-3, SB-5, SB-7, B-7, B-8, B-9, B-10, B-11, B-12, B-13, B-14, and B-15 at various depths below the ground surface. The borings had percent core recoveries ranging from 17 to 100, and RQD values (Rock Quality Designation) ranging from 0 to 100 percent indicating a very poor to excellent rock quality as shown in the Table below. It should be noted that the upper portions of the rock are generally more weathered and fractured and the rock generally will become more sound with depth.

RELATIONSHIP OF RQD AND ROCK QUALITY: ROCK QUALITY DESIGNATION (RQD)⁽¹⁾ DESCRIPTION OF ROCK QUALITY 0 - 25 VERY POOR 25 - 50 POOR 50 - 75 FAIR 75 - 90 GOOD 90 - 100 EXCELLENT

Groundwater: Groundwater was encountered in the majority of the borings at depths ranging from 5 to 20± feet below the ground surface. Borings B-6, B-12, B-14, SB-3, and SB-15 did not encounter groundwater during the short period of time that the hole were left open. Groundwater observation wells were installed by Soil Testing Inc. in borings B-7, B-9, B-10, B-15, PW-1, and SB-2 at depths ranging from approximately 5½ to 15 feet below the ground surface. Fluctuations in the groundwater level of several feet should be anticipated based on the time of year and amount of recent precipitation. Groundwater flow into the proposed deep excavations will likely be an issue unless it is cutoff and/or controlled with sumps/well points. It should also be anticipated that groundwater will be encountered at the soil-rock interface. Additional groundwater level observations and evaluations will be done as part of a separate investigation.

EVALUATION AND RECOMMENDATIONS

The existing subsurface conditions are considered poor to fair from a building foundation support standpoint. The existing uncontrolled fills and underlying loose sands would consolidate significantly under the building loads, resulting in unacceptable total and differential settlements.

We have considered various site improvement techniques to densify the existing loose sands; however, many of the improvement methods create vibrations in order to densify the soils. These vibrations could create off-site settlements and/or damage to existing structures, therefore we did not consider them further.

We recommend that the proposed foundations bear directly on the rock when rock is within an economical depth of excavation, otherwise a deep foundation system is required for the support of the proposed building, deriving its load-carrying capacity from the underlying weathered rock or bedrock. In those areas of the site where the bedrock is less than 10 feet from the lowest floor elevation, it may be economical to over-excavate to the bedrock and construct a footing on the

^{(1) &}quot;Rock Quality Designation" is defined as the cumulative amount of pieces of the core that are at least 4 inches long divided by the total length of the rock core run. Obvious fractures caused by drilling are ignored in this system.

bedrock and extend a pier to the slab elevation. All lower floors should be designed as structural floor slabs to resist uplift pressures. Alternately, drilled caissons socketed into the bedrock could also be used to support the heavier column loads.

We have not considered any environmental issues with regards to our geotechnical recommendations. We recommend that your environmental consultant review our recommendations prior to the start of construction.

Based on the groundwater levels observed during our investigation and indicated on the boring logs by others, groundwater will likely be encountered during the mass excavation to attain the proposed lowest level grades. Dewatering of the excavation during construction will likely be required and could consist of installing well-points or deeper wells to control the groundwater. It may be necessary to construct a watertight (bathtub like) foundation, or install a permanent drainage system which could consist of drywells with permanent sump pumps.

Driven Pile Foundation

In order to eliminate both the potential total and differential settlements, the building superstructure and structural floor slab should be supported on piles (or footings on rock). We recommend that the pile types consist of end bearing piles that would derive their support on the underlying bedrock (e.g. H-piles or pipe piles filled with concrete). The total settlement for a pile-supported structure would be negligible.

Based on the deep soils data, we estimate that end-bearing pile lengths would be on the order of 10 to 44± feet below the proposed lower floor elevation and would achieve capacities on the order of 70 to 120 tons per pile driven into the weathered bedrock or bedrock. The actual pile capacity will be determined based on a test pile program.

Our preliminary recommendations for pile types include:

- HP 12x53 steel H-pile, Grade 50 allowable capacity 90 tons
- HP14x73 steel H-pile, Grade 50 allowable capacity 120 tons
- 10.75" x .375" wall thickness steel pipe pile with a minimum yield stress of 42ksi filled with 4,000 psi concrete – allowable capacity 70 tons
- 14" x 0.5" wall thickness steel pipe pile with a minimum yield stress of 42ksi filled with 4,000 psi concrete allowable capacity 120 tons

Selection of the pile type will be based on the actual column loads and costs associated with each pile type. Since the piles will be driven to bedrock, negative skin friction (downdrag) need not be considered in the pile design.

Regardless of the pile capacity or pile type chosen, a Wave Equation Analysis Program (WEAP) should be performed by the pile contractor using the proposed hammer type, cushion type, etc. to determine the appropriate driving criteria. The selection of the hammer is the responsibility of the pile contractor and should be of sufficient size to achieve the required capacity while not over-stressing the pile.

Selection of the pile driving system should consider driving stresses from difficult driving and hammer energy. For preliminary planning purposes, we recommend that the piles be driven using a single-acting driving hammer with a rated energy of at least 19,500 foot-pounds. Prior to construction, the contractor should perform, and submit for review, a wave equation analysis (WEAP) for the pile installation, demonstrating that the selected driving system, including hammer type, hammer weight, delivered hammer energy, cushion type and thickness, is capable of transferring the appropriate energy to the pile tip without overstressing the pile.

Final pile type, hammer size, and driving criteria for the piles will depend on the selected pile driving system and should be provisionally determined by the geotechnical engineer following review of the contractor's Wave Equation Analysis. The driving resistance should be maintained for at least 12 inches, or to refusal to prevent damage to the piles. Refusal may be taken as twice the required driving resistance, or twenty blows per inch, whichever is less.

The contractor should provide a method to splice piles if additional length is required. Splices shall be constructed so as to provide and maintain true alignment and position of the component parts of the pier or pile during installation and subsequent thereto, and shall be of adequate strength to transmit the vertical and lateral loads and moments occurring at the location of the splice during driving and under service loading. Splices shall develop not less than 50 percent of the least capacity of the pier or pile in bending. In addition, splices occurring in the upper 10 feet of the embedded portion of the pier or pile shall be capable of resisting at allowable working stresses the moment and shear that would result from an assumed eccentricity of the pier or pile load of 3 inches, or the pier or pile shall be braced to other piers or piles that do not have splices in the upper 10 feet of embedment.

It should be noted that driven piles are subject to damage when obstructions are encountered. It should be anticipated that such obstructions may be encountered within the existing fill materials or in the natural soils beneath the fill. The use of a pile point will assist the pile in penetrating through rubble in the existing fill, and protect the pile tip during hard driving on boulders or obstructions. Due to the presence of obstructions within the existing fill materials, spudding each pile location through the fill, pre-drilling, or pre-excavation of the individual pile cap may be warranted, in order to minimize the damage to the piles during driving through the fill.

The lateral load capacity of a pier, pile, or pile group shall be determined by an approved method of analysis or by lateral load tests to at least twice the proposed design working load. The resulting allowable load shall not be more than one-half of that test load that produces a gross lateral movement of 1 inch at the ground surface.

Drilled Caisson Pile Foundation Alternate

In lieu of driven piles, it may be more economical to install drilled caisson piles to support the proposed structure due to the obstructions (boulders) encountered during our investigation and in the boring drilled by Soil Testing Inc. Additionally,

the installation of drilled piles would create less ground vibrations than the driven piles. The caisson should consist of a concrete (min. 4,000 psi) filled pipe extending to bedrock with an uncased rock socket. The caisson pile should have a minimum outside diameter of 18 inches and a minimum wall thickness of %-inch and be fitted with a steel driving shoe welded to the bottom of the pipe. The diameter of the rock socket shall be approximately equal to the inside diameter of the pile. The depth of the rock socket shall not be less than the outside diameter of the pile and should be sufficient to develop the full load capacity of the caisson pile with a minimum factor of safety of two. Higher loads may be obtained by increasing the rock socket depth. The rock socket and pile should be thoroughly cleaned of foreign materials and water should be removed prior filling with concrete. The caisson shall have a full depth structural core or a stub core installed into the rock socket and extending in to the pie portion an equal distance to the socket depth. The steel core shall not exceed 25 percent of the gross area of the caisson and have a minimum clearance of 2 inches between the structural core and the pipe. The steel core shall be bedded in cement grout at the base of the rock socket. If water is present within the pile and rock socket and cannot be removed prior to placing the concrete, the concrete shall be placed by tremie methods or an approved method. The means and methods of the caisson installation should be left to the discretion of the contractor.

A net end bearing capacity of approximately 8 tons per square foot can be used in the caisson design. A friction capacity, along the sides of the rock socket, of 50 pounds per square inch can be used in the design of the caissons. Based on a combination of the end bearing capacity and the skin friction, we estimate that a 2 foot diameter caisson would provide in excess of 100 kip ultimate shaft capacity. Increasing the socket into the rock will greatly increase the caissons ultimate capacity.

Load Tests

Load tests are required for any pile with design loads over 40 tons/pile in accordance with the 2010 Building Code of New York State. For this site, we recommend that a minimum of 2 load tests per uniform subsoil conditions be conducted at a location selected by the geotechnical and structural engineer. The load test should be done in accordance with ASTM D1143, Standard Test Method for Deep Foundations Under Static Axial Compressive Load or ASTM D 4945 Standard Test Method for High-Strain Dynamic Testing of Piles and the requirements of the 2010 Building Code of New York State. The test load should be applied in increments and be held at 2 times the ultimate load on the pile for a minimum of 4 hours. A lateral test and uplift test may also be done, if needed. We also recommend that 5 to10 percent of the total number of piles be drive test piles (spaced throughout the building) and be driven and monitored using a PDA to determine the energy being delivered to the pile, measure the driving stresses and determine the theoretical capacity of the pile.

Shallow Foundations

It is anticipated that the bedrock will be encountered or will be at a relatively shallow depth in the northwest corner of the site. A conventional shallow spread footing could be constructed at grade or at a lower elevation, founded on the

competent bedrock with an allowable bearing capacity of 8 tsf and a reinforced concrete pier extended to grade if required. In areas where the bedrock is within approximately 10 feet of the proposed lower floor elevation, it may be more economical to drill caissons, bearing on or socketed into the bedrock, for support of the proposed building. Regardless of the method chosen, removal of the subgrade soil to the bedrock would be required for either of these options. Excavation support or the stepping/sloping back of the excavation would be required for the construction of the spread footing and pier construction. Slopes and Excavations are discussed later in this report.

Excavation Support

In order to make the required mass excavation to achieve the lowest proposed level, an excavation support/groundwater cut-off wall will be required that must be installed prior to excavating the site soils.

We have been informed that it is your intention to install sheeting for the support of excavation which will also be used as the finished wall in the below grade garage. Due to the depth of the excavation and the presence of groundwater, it is most likely that the sheeting will require some form of bracing or tiebacks under the existing roadways. Bracing of the sheeting may consist of tiebacks or raker beams. Tiebacks are installed by rotary or auger-drilling techniques, drilled through the earth retention wall line into the retained soil behind the sheeting at specified elevations as the mass excavation proceeds. A steel tendon is then inserted into the drilled hole and injected with grout under pressure. After allowing the grout to harden, the steel tendon is stressed to a specific tension and locked into the support wall to restrain it from moving. Temporary easements for the tiebacks that encroach into the roadway and adjacent properties will be required. Testing of all the tiebacks will be required to verify that they meet the design load capacity. The tiebacks should be detensioned once the permanent structural slabs are constructed and the walls can be backfilled.

Raker beams can also provide the necessary support to support the sheeting. Steel beams are supported at the base of the excavation with concrete heel blocks below the proposed floor slab and secured to the sidewalls of the system. Using this system, the raker beams are left in place until the foundation walls are backfilled and the first floor system is in place. Penetrations through the building walls and floors will require closure once the rakers are removed.

Slopes and Excavations

Permanent soil cut and fill slopes should be limited to a maximum of 2 horizontal to 1 vertical for slopes up to 15 feet high and 2.5 horizontal to 1 vertical for slopes greater than 15 feet high and be evaluated by a qualified geotechnical engineer.

All temporary excavations greater than 4 feet in depth should have the sides sloped back or be appropriately sheeted and braced in accordance with all applicable codes. All excavations should be performed in accordance with OSHA requirements, including but not limited to, temporary shoring, trench boxes and benching and be evaluated by a qualified Geotechnical Engineer.

All excavations should be performed in accordance with OSHA requirements, including but not limited to, temporary shoring, trench boxes and benching and be evaluated by a qualified person.

Utility Lines

The site soils will provide suitable support for utility lines. Cobbles greater than 4 inches in diameter should be removed from the utility line subgrade or a minimum 4-inch thick sand layer placed beneath the utility lines. If the bottom of the excavation for any utility lines falls within soft soils, the excavation should be extended an additional 12-inches and replaced with 3/4-inch clean crushed stone or clean sand and gravel. In any areas where the utility lines are excavated into rock, a minimum of 6 inches of ¾-inch clean crushed stone or a clean sand and gravel layer should be placed beneath the pipe.

Backfill material placed around utility lines to 6 inches above the utility lines should have a maximum particle size of 1.5 inches. Backfill of utility trenches that fall within load-bearing areas should be placed in maximum 6-inch thick lifts and compacted to a minimum of 92 percent and average of 95 percent of Modified Proctor density (ASTM D 1557). Trench backfill in non-load bearing areas should be compacted to 90 percent of Modified Proctor density (ASTM D 1557).

Permanent Walls

Permanent below grade walls should be designed to resist lateral loadings from static earth pressure, water pressure (if present), and vertical surcharges. Backfill should not be placed against below-grade walls until the concrete has reached its 28-day compressive strength and after adequate lateral bracing has been provided to prevent rotation of the wall. We recommend the following design parameters:

- For braced walls (no rotation) a triangular earth pressure distribution with an equivalent fluid pressure of 60 pounds per square foot per foot of depth for unsaturated soil.
- For cantilevered walls a triangular earth pressure distribution with an equivalent fluid pressure of 40 pounds per square foot per foot of depth for unsaturated soil.
- Lateral pressures due to surface surcharges should have a uniform distribution based on a pressure equal to 0.5 times the vertical pressure for the entire depth of the wall. We recommend using a minimum surcharge load of 250 pounds per square foot to account for fire truck loading scenarios.

All retaining walls should be provided with positive drainage behind the wall to preclude hydrostatic pressures from developing.

Seismic Design

The site soils have been classified as Site Class C for seismic design purposes in accordance with the 2010 Building Code of New York State. For any buildings with foundations founded within 10 feet of bedrock Site Class B may be used.

The following seismic design criteria for Class "C" soils should be used for this project:

Mapped Spectral Response Acceleration for Short Periods	SS = 0.270g
Mapped Spectral Response Acceleration for 1-Second Period	S1 = 0.072g
Site Coefficient	Fa = 1.200
Site Coefficient	Fv = 1.700
Spectral Response for short periods	SMS = 0.324g
Spectral Response for 1 second period	SM1 = 0.122g
Design Spectral Response Acceleration for Short Periods	SDS = 0.216g
Design Spectral Response Accelerations for 1-Second Period	SD1 = 0.081g

A summary of recommended soil design parameters is included in Table 1.

Control of Groundwater

Deep foundations (including the excavation for pile caps and/or grade beams) and deep utilities will likely encounter the groundwater. If stormwater/groundwater seepage is encountered during construction, gravel filled sumps with pumps should be installed below the subgrade elevation to allow for dewatering of the excavation. If any significant groundwater inflow is encountered or if any excavations extend greater than two feet below the observed groundwater levels, a more comprehensive dewatering system may be required. This system would consist of installing several well-points to control the groundwater. Design of this system will be based on the depth and size of the excavation and the groundwater levels encountered. NYDEC permits for construction dewatering will also be required.

Foundation drains are required for any retaining walls below grade and should be tied to the storm sewer system or to a sump with pump.

Waterproofing

All foundation walls should be waterproofed. We recommend the Preprufe/Bituthene waterproofing system by W.R. Grace and Co. Depending on the floor finishes in the lowest level, waterproofing may be required under the slab. A drainage board (MiraDRAIN) or stone backfill should be placed along the foundation wall to allow any groundwater to flow down to the footing drains.

It may be necessary to construct a watertight (bathtub like) foundation, or install a permanent dewtering system which could consist of drywells with permanent sump pumps.

INSPECTION

The recommendations presented in the previous sections of this report are based on the assumption that the site preparation procedures will be done under engineering inspection by a representative of SESI Consulting Engineers, PC. We should inspect the support of excavation installation, the pile driving, the over-excavation of the existing fill (where required), the placement of the compacted fill (where required), and the bottom of the footing excavations prior to the placement

of concrete. Visual observations and in-place density testing should be done throughout fill construction to determine that the work is done in accordance with our recommendations.

LIMITATIONS

The subsurface data reviewed identifies the subsurface conditions only at the locations of the test holes and at the depths where the samples were taken by others. SESI Consulting Engineers, PC reviews the published geologic data and the field data and uses their professional judgment and experience to render an opinion on the subsurface conditions throughout the site. Because the actual subsurface conditions may differ, we recommend that SESI be retained to provide construction inspection in order to minimize the risks associated with unanticipated conditions.

This report should not be used:

- When the nature of the proposed development is changed;
- When the size or configuration of the proposed development is altered;
- When the location or orientation of the proposed development is modified;
- When there is a change in ownership; or
- For application to an adjacent or any other site.

SESI shall not accept any responsibility for problems which may occur if SESI is not consulted when there are changes to the factors considered in this report's development.

The soil logs should not be separated from the Engineering Report in order to minimize the likelihood of soil log misinterpretation.

DISCLAIMER

This Report was prepared by SESI for the sole and exclusive use of Saber Chauncey WP, LLC. Nothing under the Professional Services Agreement between SESI and its client, Saber Chauncey WP, LLC shall be constructed to give any rights or benefits to anyone other than Client and SESI, and all duties and responsibilities undertaken pursuant to the Agreement will be for the sole and exclusive benefit of Client and SESI and not for the benefit of any other party. This Report has been prepared and issued subject to the express conditions that same is not to be disseminated to anyone other than Client, without the advance written consent of SESI (which SESI, in its sole discretion, is free to grant or withhold). Use of the Report by any other person is unauthorized and such use is at the sole risk of the user.

TABLE 1

SUMMARY OF SOIL DESIGN PARAMETERS

	ARAMETER Allowable Bearing Cap	acity (net)	VALUE		
•••		Compacted Fill	2 tsf 8 tsf		
2.	Total Unit Weight	(Onsite/Imported Soil)	120 pcf		
3.	Angle of Internal Frictio Backfill Agai	32 degrees			
4.	Earth Pressure Coeffici Active Earth Pressu Earth Pressure @ R Passive Earth Press	re (Ka) Rest (Ko)	0.31 0.47 3.25		
5.	Coefficient of Sliding (c	oncrete over soil)	0.40		
6.	Subgrade Modulus for F (Granular Fill)	Floor Slab Design	175pci		
7.	Slopes (Above Grounds Maximum Cut Slope Maximum Fill Slope	in Soil	2.5H:1V 2.5H:1V		
8.	Footing Depth for Frost	Protection (exterior)	3.5 ft		
9.	Seismic Design Criteria If within	- Site Class n 10 feet of bedrock	C B		

Notes:

- 1. A drainage medium should be installed along all retaining walls to avoid hydrostatic pressures from developing.
- 2. Compaction equipment used within 5± feet permanent walls should not weigh more than 5,000 pounds.

SB-1

SB-7 7' F R 50'

10' F R 38'

N:\ACAD\8949\8949 BORING LOCATION PLAN.dwg, Layout1, 11/25/2014 3:44:41 PM, \\sesi2007\KONICA MINOLTA C360 PCL

BORING LOCATION PLAN

11/25/14

date:

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chk by:

dwg by:

SOILS / FOUNDATIONS

ENVIRONMENTAL

SITE DESIGN

PROPOSED MIXED USE DEVELOPMEN THE COLLECTION 80 WESTCHESTER AVENUE CITY OF WHITE PLAINS. NEW YORK

drawing ti

job no: 8949 drawing no:

FIG. 1

1 of

								8949_SB1_BORING								
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		INE			-		ETHOD:			ow Stem Auger GROUND ELEVATION:		157±				
_	NG BY:	Gener	al Borin	gs Inc.			ARTED:		6/2014	GROUNDWATER TABLE DEPTH						
	ECTOR:		JN		DATE	COMP	LETED:	10/16	6/2014	0 Hr. 10± Date 10/16/14 24 Hr.	Date					
DEPTH (ft)	SAMPLE No.	REC	FROM	PTH TO		Blows	on Spoor	n N		SOIL DESCRIPTION AND STRATIFICATION		Symbo				
0		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)			USCS				
										2" Asphalt, 4.5" Subbase, 6" Brick		1				
	1	18	1	-	8	12			25	FILL: Gray-brown fine SAND and Silt, little coarse to fine (Gravel					
	\vdash			3		<u> </u>	13	11								
_	2	20	3	├—	8	6	<u> </u>		12	FILL: Gray-brown fine SAND, some Silt, trace Gravel						
5				5	<u> </u>	<u></u>	6	4								
	3	22	5	<u> </u>	WOI	H/ 12"			3	with Brick fragment	_					
				7			3	3		Dark brown CLAY						
	4	24	7		WOH	3			7	Gray-brown CLAY with fine Sand Lenses						
				9	<u> </u>		4	5								
10				ļ						Gray/orange CLAY, with fine Sand lenses						
	5	16	10		5	6			12	Brown medium to fine SAND, trace Gravel, trace Silt		_				
				12			6	_ 5								
15																
	6	24	15		3	6			24		_					
		\rightarrow		17			18	34		Brown fine SAND, with Silt lenses						
20																
	7	24	20		WOH	/ 12"			6	Brown coarse to fine SAND, trace Gravel, trace Silt						
- 1				22			6	6		Brown fine SAND, little Silt						
- 1																
}																
25					\longrightarrow											
ŀ	8	24	25		2	3			7	Same						
- }		\longrightarrow		27			4	4								
-		\longrightarrow														
<u> </u>	-		-													
30								ightharpoonup								
ŀ	9	20	30		10	10			22	Brown medium to fine SAND, trace Silt, trace Gravel						
-		\rightarrow	\rightarrow	32			12	17								
 		-+	\rightarrow													
<u>. </u>						$-\!\!\perp$										
35		\longrightarrow	\rightarrow	\dashv			$-\downarrow$									
-										Weathered Rock						
L		\longrightarrow									1					
L																
								T		BORING COMPLETED AT 38 FEET						
40 F																

0.41		Approximate Change in Strata: Inferred Change in Strata:
Core Size	in	
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.

			=		П	30 IE03	E ALABAE.	_	-	0.11.41	BORING NO.	ODZ_BURING I	
			3		F-		NAME:			ne Collection		B-2	
			120	9	<u> </u>		CATION:	_		nite Plains, NY	JOB NO. 894		
BOBII		INE			_		ETHOD:			w Stem Auger GROUND ELEVATION:			57±
	NG BY: ECTOR:	Genera	al Boring	gs inc.		DATE STARTED: DATE COMPLETED:			0/2014		DUNDWATER TABLE DEP		
DEPTH			RG	PTH	DATE	COMP	LETED:	10/20	0/2014	0 Hr. 11'± Date	10/20/14 24 Hr.	Date	
	SAMPLE	REC	FROM		-	Blows o	n Spoor	1	N				Symbol
(ft) 0	No.	(!-)		-	0.10	2442	Lious			-	PTION AND STRATIFICAT	ION	
-		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)				USCS
			 	┝			-			4" Asphalt			
	1	22	1	-	12	14	-		29	FILL: Dark brown fine SA	AND, little medium to fine Gra	vel, little Silt	
1		10	_	3			15	12		4			
_	2	19	3	 _	16	14			28	Same			
5		40		5			14	12		1		_	
	3	18	5		7	13			26	with Wood, Asphalt fr	agments		
				7			13	10					
	4	18	7		10	11			22	Gray-brown Clayey SILT,	some fine Sand		
_				9			11	11		ļ			
10										 			
	5	14	10		10	12			25	Gray-brown coarse to fine S	AND, some medium to fine Gra	vel, trace Silt	
1 1				12			13	11					
15										l			
1 1	6	20	15		7	18			43	Gray-brown fine SAND, s	ome Silt		
1 [17			2.5	38					
il													
[
20													
	7	24	20		5	7			14	trace Gravel		_	
[22			7	11					
ΙГ													
[
25													\vdash
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						$\neg \neg$	$\neg \uparrow$						
													 -
							$\overline{}$	\dashv					
30	- 												
\Box	8	24	30	\neg	7	6		-+	13	Same		\dashv	
	- +			32	- 	Ť	7	11	13	name			
						\dashv	' +	- 11	\dashv				
-			\dashv	$\overline{}$	-+								
35	+	-	-+	\dashv	-	-		\dashv				ļ	
-55			\rightarrow						\dashv			_	
				-									
 	-+		\dashv	\rightarrow	\dashv			\rightarrow					
 -		\rightarrow	-+			\rightarrow		-+					
10 F			-+				-+	\dashv			COMPLETED AT 38 FEET	Į	
40										RE	FUSAL ON ROCK		

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
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		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	_
		Approximate Change in Strata: Inferred Change in Strata:

					T	_				8949_SB3_BORING L	_OG
					PF		NAME:	_			B-3
	CONI			3	<u> </u>		ATION:				949
	ENG	INE	ERS			_	ETHOD:		low Ste		58±
_	NG BY:	Gener	al Boring	gs Inc.			ARTED:	10/20	0/2014	GROUNDWATER TABLE DEPTH	
	CTOR:		RG		DATE	COMP	LETED:	10/20)/2014	0 Hr. 10.5± Date 10/20/14 24 Hr. Date	
DEPTH	SAMPLE	REC		PTH	Blows on Spoon		1	N		Symbo	
(ft)	No.		FROM							SOIL DESCRIPTION AND STRATIFICATION	Symbo
0		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)		USCS
										4" Asphalt	
	1	21	1		6	7			14	FILL: Dark brown coarse to fine SAND, some coarse to fine	
				3			7	8		Gravel, little Silt	
	2	20	3		7	8			19	with Asphalt fragments	
5				5			11	10			
	3	24	5		4	4			12	Gray-brown Clayey SILT, little fine Sand	
				7			8	12			
	4	24	7		8	11			25	Gray-brown fine SAND, little Silt	
				9			14	14		Light brown fine SAND, little coarse to fine Gravel, trace Silt	
10										.,	
	5	19	10		5	4			8	_	\vdash
				12			4	4			
										Brown coarse to fine SAND, some coarse to fine Gravel, trace Silt	
										, see of the see of th	
15											
	6	10	15		2	2			4	Same	
				17			2	3			<u> </u>
											_
20											
	7	19	20		4	5			11	Gray-brown fine SAND and Silt	
Ì				22			6	5		oral me oral ma one	<u> </u>
1											
Ì									$\neg \neg$		
25											
										\dashv	
- 1								$\neg \neg$			
ı		- 									
ı	$\overline{}$							\neg			
30							$\overline{}$				
	8	15	30		5	4	-		8	Brown coarse to fine SAND and coarse to fine Gravel, trace Silt, with	
ı				32			4	6		Cobbles	
ŀ			\dashv		$\overline{}$		- -	-		ryonnies	
ŀ			\dashv		-+	\dashv	$\overline{}$				
35				-	-	-+		\dashv			
				\dashv	$\overline{}$	-				-	
H		+	-+	$\overline{}$	$\overline{}$		\dashv	\rightarrow		ļ	
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 	$\overline{}$		-		-+		-+	-+		ļ	
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		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

	PROJECT NAME:								The Collection BORING NO.								
\$					_	CATION:	_			NU.		3949					
1	CONSULTING ENGINEERS					ETHOD:			ite Plains, NY								
BOR	ING BY:	Gener	al Boring	as Inc	DATE	START			0/2014	m Auger/Mud Rotary		ELEVATION:		157±			
-	ECTOR:	3011011	RG	50 mio.		COMP	_		0/2014	0 Hr. 10.5± Date	10/20/14	ER TABLE DEPTH 24 Hr.					
DEPTH				PTH						VIII. IO.JI Date	10/20/14	27 FII.	Date	Т-			
(ft)	SAMPLE No.	REC	FROM		1	Blows	on Spoor	ו	N	SOIL DESCRI	PTION ANI	STRATIFICATIO	N	Symbo			
40	No.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)				•	USCS			
	9	13	40		57	27			43	Weathered Rock: Appears	as Grav/wh	ite/brown coarse to fi		0303			
				42			16	20		SAND, and coarse to fine			ще				
											010103, 240						
	\Box																
45														-			
	RC-1	0	45						2.5	Rock Core Run 1: (45'-50'))						
									1.5	Boulder from 45-50ft							
		_							1.5								
									1.5								
50				50	<u> </u>				1.5								
							lacksquare					ED AT 50 FEET					
				-			<u> </u>			COULD NOT ADVA	NCE CASI	NG BEYOND BOU	LDER				
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55	 																
		_															
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60				\vdash										<u> </u>			
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		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

													8949_SB4_BORIN	G LOC	<i>j</i>	
SES PRO							PROJECT NAME: The Collection BORING NO.								SB-4	
			TING				ATION:			ite Plains, NY			JOB NO.	894	9	
		INE			 _ _		ETHOD:			ow Sten	Auger		GROUND ELEVATION:	156	±	
	NG BY:	Gener	al Boring	gs Inc.	_		ARTED:		6/2014	-			DUNDWATER TABLE DEPTH			
DEPTH	CTOR:		JN T DEI	PTH	DATE	: COMP	LETED:	10/16	3/2014	0 Hr.	8.5'±	Date	10/16/14 24 Hr. Date			
(ft)	SAMPLE	REC	FROM		1	Blows o	n Spoor	1	N		0011			18	Syml	
(11)	No.	(in)	_	_	O/C	640	40/40	40/04	(1.10)	-	SOIL	DESCRI	PTION AND STRATIFICATION	L		
	├	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	_				4	USC	
	1	20	1		11	13				-	sphalt, 2.5			\perp		
		20	1	3	111	13	9		22	FILL: I	Dark gray	fine SAN	ND and Silt, trace Gravel, with Wood	-		
	2	4	3	-	2	50/3"			_	٠.,		TO		\vdash		
5	 	-		5	+-	30/3			_	with	Concrete	, Plastic		-		
	3	3	5	<u> </u>	3	4	\vdash		11	FIT 1 . T				+		
	-		 	7	-	-	7	5	-11	FILL: E	srown coar	rse to fine	e SAND, some Silt, little coarse to fine Gra	vel_		
	4	8	7	'	6	6		,	15	EII 1 . /	loor for s	AND -	.d 0.04 American .dd .vvd		_	
	 		<u> </u>	9	Ť	ا ا	9	9	13	True:	nay ime S	MIND an	d Silt, trace Gravel, with Wood	-	_	
10				-			-			1				\vdash	_	
	5	24	10		WOH	6	\vdash		16	Brown	coarse to	fine S A N	ID, trace Silt	+		
				12			10	13	10	DIOWII	coarse to	IIIIC SAIN	in, trace sin	\vdash	_	
										1				-	_	
										1				\vdash	_	
15								-		1				\vdash	_	
	6	24	15		5	7			22	Brown	fine SANI	D. trace S	Silt	╁		
- 1				17			15	13				,		-	_	
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[\vdash	_	
20																
	7	18	20		4	6			13	Same				╅	_	
[22			7	10						\vdash		
L														\vdash	_	
L																
25																
	8	24	25		8	8	\Box		19	Same			•	1		
Ţ				27			11	11								
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30	\rightarrow	\longrightarrow														
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-																
-																
<u>. </u>			\rightarrow				$-\downarrow$									
35				\rightarrow			\rightarrow									
ŀ	9	24	35		20	23		\rightarrow				ne SANI	D, trace Silt, trace Gravel			
-				37			22	23		Weather	ed Rock					
F			\longrightarrow				$-\downarrow$									
<u>"</u>	\longrightarrow				$-\!$	\rightarrow					BO	ORING (COMPLETED AT 38 FEET			
40												RE	FUSAL ON ROCK			

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
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Weight/type of Hammer on Split Barrel		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

					PF	ROJEC	ΓNAME:		TI	ne Collection	BORING NO.	S	B-5
7	CON	SULT		3			CATION:		Wh	ite Plains, NY	JOB NO.		949
		INE					ETHOD:	Hol	low Ste	m Auger/Mud Rotary	GROUND ELEVATION:	1:	56±
-	NG BY: ECTOR:	Gener	al Borin	gs inc.					7/2014		DUNDWATER TABLE DEPTH		
DEPTH			RG I DE	PTH	DATE	DATE COMPLETED:		10/17	7/2014	0 Hr. 11'± Date	10/17/14 24 Нг.	Date	
(ft)	SAMPLE	REC	FROM	_	1	Blows	on Spoor	1	N	SOIL DESCRI	DTION AND OTDATION	_	Symbol
0	No.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	SOIL DESCRI	PTION AND STRATIFICATION	1	L
			\ \ \ \ \ \	1,7	-		12710	10,24	(Dirit)	2" Asphalt, 2.5" Subbase			USCS
	1	22	1		10	10			20		SAND, little coarse to fine Gravel	1241- 026	<u> </u>
				3			10	9			orand, mine comse to mie gravei	, mue sin	
	2	16	3		6	11			24	FILL: Black/brown coarse	to fine SAND, some coarse to fin	e Gravel	_
5				5			13	12		some Silt	, , , , , , , , , , , , , , , , , , , ,	o ciuroi,	
	3	24	5		2	2			4	Black Silty CLAY, little fi	ne Sand, trace Gravel		
				7			2	4					
	4	23	7		4	5	\vdash		10	Gray-brown Clayey SILT,	and fine Sand		
10				9			5	- 8					
10	5	21	10		2							_	
		21	10	12		7	7	-	14	Same			
				12			'-	8			e SAND, little medium to fine Gra	vel, little	
										Silt			
15													
	6	16	15		9	9			21	Brown fine SAND, trace S			
				17			12	10		Diown imo Braid, dace 5	ш		
20]								
-	7	19	20		6	8		I	15	Gray-brown fine SAND, so	me Silt		
				22			7	8					
						-							
25		-										[
20	-		-										
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ı					-+		$\overline{}$	-+					
30												-	$-\!\!-\!\!\!-$
	8	16	30		9	11		-+	26	Gray-brown coarse to fine S	AND, little Silt, trace Gravel		
				32			15	16		y Tombo w Alife D		H	
												ŀ	
L												ŀ	
35	\longrightarrow					\Box						ŀ	
-				\rightarrow								7	$\neg \neg$
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-	-		$-\!$		- +	$-\!$		$-\!$				ſ	
40 F		-+	-+	-+	\dashv								
40													

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Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:
Soil descriptions represent a Cald id-atic-at-	-0. D	16 D

					T pr	O IEC	T NIANE.	_		0-11- 11		_	T = = = = = =		949_SB5_BORIN	
-	SI	= {	S		-		NAME:		The Collection White Plains, NY				BORING	NO.		SB-5
		BUL	FING	G	<u> </u>		CATION:				_		JOB NO.			8949
BOB				an lan	-		ETHOD:			m Auger/Mud Rotary			ELEVATION		156±	
	ING BY: ECTOR:	Gerier	al Borin	gs inc.		STAR			7/2014	GROUNDWATER TABLE DEPT						
DEPTH			RG	PTH	DATE	COMP	LETED	10/1	7/2014	0 Hr. 11	1'±	Date	10/17/14	24 Hr.	Date	
	SAMPLE	REC			-	Blows	on Spoor	1	N							Symb
(ft)	No.	(:)	FROM			I	1.5			J S	OIL E	DESCRI	PTION ANI	D STRATIFICA	ATION	
40		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	-						USC
	9	14	40		53	24	-		66	Gray-brown	fine	SAND, I	ittle Silt, trac	ce Gravel		
	\vdash		-	42	-	<u> </u>	42	55		4						
			-	-	_		-			4						
	\longrightarrow			-	<u> </u>	_	_									
45	\vdash		-	 	<u> </u>	_	_									
			<u> </u>	├						1						
			-						<u> </u>	1						
										1						
		_		<u> </u>						1						
50	\vdash				<u> </u>					ļ						
	10	2	50		100/2"					Weathered F	Rock:	appears	as White/bla	ack/ brown coar	rse to	
	\longrightarrow			52						fine SAND a	and co	oarse to i	ine Gravel,	with Cobbles		
										1						
55										ľ						
										1						
60																
	RC-1	48	60						2 min	Rock Core: I	Run 1	(60'-65')				_
										Schist						
									1 min	Recovery: 48	3"/60"	= 80%				+
										RQD: 35"/60						-
65				65					3 min							-
\neg	RC-2	60	65							Rock Core: R	Run 2	(65'-70')				+
- 1								$\overline{}$	1 min			,				-
Ì					$\neg \neg$			$\neg \neg$		Recovery: 60)"/60"	= 100%				_
ľ										RQD: 54"/60						-
70				70	$\neg \uparrow$				1 min							-
								-			P()RING ('OMDI ETE	D AT 70 FEET		+
ı	-+						-+	-			DC	VITAG (~INTLETE	DAI WEEL		—
- 1					\neg		$\overline{}$	-								—
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	$\overline{}$	- 		-+	\rightarrow			- +							-	
- 1	+	- 					-+	\dashv								<u> </u>
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Weight/type of Hammer on Split Barrel		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

SESI					PI		T NAME							SB-6	
					-		CATION		Wh	ite Plains, NY		JOB NO.		8949	
	ENG				┿		ETHOD			ow Stem Auger		GROUND ELEVATION:		156±	
	NG BY:	Gene	ral Boring	gs Inc.	_		ARTED:		0/2014			DUNDWATER TABLE DEPTH			
DEPTH	ECTOR:	Τ	RG	PTH	DATE	DATE COMPLETED:		10/12	0/2014	0 Hr. 11.5'±	Date	10/20/14 24 Hr.	Date		
(ft)	SAMPLE No.		FROM	то	_		on Spoo	n	N	SOIL D	ESCRI	PTION AND STRATIFICATIO	N	Symbo	
0		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)					USCS	
				<u> </u>		<u> </u>				4" Asphalt					
			+	-		-				ļ					
			+		-	-				1					
5					-		-			1					
										1			-	+	
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			\vdash	<u> </u>											
10	1	24	9		1	3			6	Brown fine SAND	, trace S	Silt			
			-	11			3	6					_		
			┼──┤							Drilled to Bedrock					
			 												
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
Sail descriptions represent a field identification	often D	Approximate Change in Strata: Inferred Change in Strata:

\$	SI		3		PF		NAME			e Collection	BORING NO.	S	B-6
	CONS	3 U L	FIN	3)	<u></u>		CATION			ite Plains, NY	JOB NO.		949
	ENG				-		ETHOD	_		w Stem Auger	GROUND ELEVATION:	1	57±
	NG BY:	Gener	al Boring	gs Inc.			ARTED	_	0/2014		GROUNDWATER TABLE DEPTH		
	ECTOR:		RG		DATE	COMF	LETED:	10/12	0/2014	0 Hr. 11.5'± Date	Date 10/20/14 24 Hr. Da		
DEPTH (ft)	SAMPLE No.	REC	FROM	PTH TO	1	Blows	on Spoo	n	N	SOIL DESCRI	PTION AND STRATIFICATION		Symbo
40	10.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	1			USCS
										Drilled to Bedrock			0000
45													
												_	
50													<u> </u>
												-	_
													
55													
												_	_
										BORING	COMPLETED AT 56 FEET		
											EFUSAL ON ROCK		
60													_
									-			_	
- 1													
65									$\overline{}$				
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler		It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe		to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	-
		Approximate Change in Strata: Inferred Change in Strata:

					_			_		8949_SB7_BORING LO	G	
1.	S I				P		NAME			he Collection BORING NO. SB	-7	
	CON			G	<u> </u>		CATION			hite Plains, NY JOB NO. 894	8949	
POF		O			+		ETHOD	_	_	em Auger/Mud Rotary GROUND ELEVATION: 154		
	ING BY: PECTOR:	Gene		ngs Inc.			ARTED		5/2014	GROUNDWATER TABLE DEPTH		
DEPT		Т	JN	EDTIL	DATE	COMP	LETED:	10/1	5/2014	0 Hr. 8'± Date 10/15/14 24 Hr. Date		
(ft)	SAMPLE No.	REC	FROM	EPTH M TO		Blows	n Spoo	n	N	SOIL DESCRIPTION AND STRATIFICATION	Symbol	
0	-	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	J	USCS	
	<u> </u>									1.5" Asphalt, 2.5" Subbase	-	
1	1	21	1	╄	11	20			34	FILL: Gray medium to fine SAND, little Silt, trace Gravel, with Wood		
	<u> </u>		₩	3			14	24				
_	2	7	3	┼	30	16			36	FILL: Wood fragments		
5			↓	5		<u> </u>	20	8				
	3	2	5		2	3			7	FILL: Gray medium to fine SAND, trace Gravel, trace Clay		
1		-	-	7	 		4	7				
1	4	20	7	 	6	9			18	Gray-brown medium to fine SAND, trace Silt with Organic fibers		
40	-		-	9			9	14]		
10	-		-	┼								
	5	24	10	-	3	4			13	Gray-brown CLAY, little fine Sand		
				12			9	_11		Brown coarse to fine SAND, little coarse to fine Gravel, trace Silt		
			-	┼								
15				-						l – – – – – – – – – – – – – – – – – – –		
15		10	15	┼								
	6	12	15		3	10			22	Brown medium fine SAND, little Silt, trace Gravel		
	 			17			12	10				
	\vdash		_							Į		
20				-						ļ		
20	7	18	20									
	'	18	20	22	5	7		-+	_17	Brown fine SAND, little Silt		
	-			22			10	12				
25				-								
	8	12	25		_	_						
	-	12	23	27	2	6	_		_14	Same		
	-			27			8	10				
30					-+			-+				
 -	9	14	30		4							
- 1	 -	- 17	20	32	-4	5	10	. 	15	Brown SILT and fine Sand		
ŀ	-		-	34			10	15				
ŀ	- 							+				
35					+	-+			\dashv			
-	10	18	35		-,-	-	+					
H	10	10	33	27	3	7		26	18	Same		
ŀ	- 			37	-+		11	26				
	- 			\dashv	-+	-+	-+		<u> </u>	Weathered Rock		
40		 +		-+	-+	-+	-+	-+				
.,,												

Nominal I.D. of Hole	In	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1 % in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

			_	_	PI	ROJECT	Γ NAME:		Th	e Collection BORING NO.	
-	5		5		<u> </u>		CATION:	+			SB-7
		6 7 E	FING	c ì			ETHOD:				154±
BOR	ING BY:	Gen	eral Bor	ings	DATE	START		_	5/2014	GROUNDWATER TABLE DEPTH	J41
	ECTOR:		JN		_	COMP		-	5/2014	0 Hr. 8± Date 10/15/14 24 Hr. Date	
DEPTH		REC		PTH						Date	T
(ft)	SAMPLE No.	NEC	FROM	то		DIOWS (on Spoor	1	N	SOIL DESCRIPTION AND STRATIFICATION	Symbo
40	140.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)		USCS
										Weathered Rock	
45											
				-						_	
	11	3	46	-	35	50/3"				Weathered rock: Appears as Brown coarse to fine SAND,	
			<u> </u>	48	 -					some coarse to fine Gravel, trace Silt	
F 0	\vdash			-	<u> </u>		\vdash				
50	DC 1		- 50			_					↓
	RC-1	60	50			 	-		2 min	Rock Core: Run 1 (50'-55')	
				-		_			2 min	Gneiss	
							\vdash			Recovery: 60"/60" = 100%	
55				55		-	 			RQD: 60"/60" = 100%	-
-55	RC-2	56	55	33					2.5 min	Rock Core: Run 2 (55'-60')	
	RC-2		- 33			_			2.5 min	Gneiss —	├──
		-						-+		Recovery: 56"/60" = 93%	-
								-		RQD: 56"/60" = 93%	+
60				60					3 min	_	-
									J 2000	BORING COMPLETED AT 60 FEET	
										Zotato cominina in outila	
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler		It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe		to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	-
		Approximate Change in Strata: Inferred Change in Strata:

					-			_		8949_SB8_BORING L	UG
1 3	S I	<u> </u>			⊢ Pi		NAME				B-8
`						_	CATION:	_	W	nite Plains, NY JOB NO. 89	249
	BORING BY: General Borings Inc. NSPECTOR: RG DEPTH SAMPLE REC DEPTH						ETHOD:	_		ow Stem Auger GROUND ELEVATION: 15	54±
		Gener		igs inc.			ARTED:		0/2014	GROUNDWATER TABLE DEPTH	
-			_		DATE	COMP	LETED:	10/20	0/2014	0 Hr. 11.5'± Date 10/20/14 24 Hr. Date	
	SAMPLE	REC			4	Blows o	n Spooi	า	N		Combal
(ft) 0	No.	(i)	FROM	_	-					SOIL DESCRIPTION AND STRATIFICATION	Symbol
1	 	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)		USCS
	1	17	 	-						2" Asphalt, 2" Subbase	
	-	17	1	+-	15	19			46	FILL: Dark brown coarse to fine SAND, some coarse to fine Gravel,	
1	2	22	 _	3	2.5		27	24		little Silt	
5		22	3	+-	35	25			55	FILL: Dark brown coarse to fine SAND, little fine Gravel,	
۳	3	8	5	5	1/	50/00	30	25		little Silt, with Cobbles	
	-	· · ·	- 3	- 7	16	50/3"				Same	
			-	7	-					(Boulder from 6'-8')	
	 		-		-						
10	 		 	+-	 	-				1	
10	4	18	10	-	10						
	 	10	10	10	18	29			50	Gray-brown fine SAND, some Silt	
	-			12			21	24		Brown coarse to fine SAND, trace Silt	
	 			-							
15				-]
-13	5	18	15	-							
	- 	10	13	17	6	6	-10		16	Brown fine SAND, some Silt	
	-			17			10	20			
	+									<u> </u>	
20	- +			\vdash						<u> </u>	
	6	24	20		11	11		\rightarrow	- 40		
	Ť		20	22	- 11	-11	7		18	Gray-brown fine SAND, some Silt	
				22		$\overline{}$		5		<u> </u>	
- 1					-				-	L	
25						-		-+		L	
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	7	14	30	\dashv	5	13	+	-+		_	
ŀ			-	32	- +	13	13	1.5	26	Same	
ŀ		-+	\neg	- 32		- 	13	15	—	L	
ŀ		- 		-+		-	-+	\rightarrow	-	L	
35	- -		-+	-+	-+	-+		-		L	
		+	\dashv	-+	$\overline{}$				-		
-		- +	\dashv	-+				-+			
<u> </u>	_			- +		-+	-+	\rightarrow			
H	+			-+	-+	-+					
40	- -	-	-+	$\overline{}$		\dashv					
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1¾ in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted,
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification		Approximate Change in Strata: Inferred Change in Strata:

					T pr	20 150	T NICO CO				8949_SB8_BOI	_	_	
3	SI		5				NAME	_		e Collection	BORING NO.	SB-		
	OON	8 Z E	FING	G.	-		CATION		_	ite Plains, NY	JOB NO.	894 155		
BOB	ING BY:				DATE		ETHOD	_						
		Gener	al Borin	gs inc.	_	STAR			0/2014		DUNDWATER TABLE DEPTH			
DEPTH	ECTOR:		RG	PTH	DATE	COMP	LETED	10/20	0/2014	0 Hr. 11.5'± Date	10/20/14 24 Hr. Da	te		
	SAMPLE	REC			-	Blows	on Spoo	n	N				Symbo	
(ft)	No.	(!-)	FROM	-	0/0	1 2/12	Linus			SOIL DESCRÎ	PTION AND STRATIFICATION	L		
40		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)				USCS	
	8	20	40		9	17	<u> </u>		52		as Gray-brown coarse to fine	L		
	 			42			35	34		SAND, some coarse to fin	e Gravel, little Silt	L		
	\vdash	-	-		-			<u> </u>				L		
	\vdash		ļ —	-	-		-							
45			-	-	_	<u> </u>	├							
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	\vdash			-	<u> </u>							Г		
	$\vdash \!$		<u> </u>									r		
50			<u> </u>							Weathered Rock: Appears	as Brown/white/black coarse to fine SA	ND,		
	9	4	50		50/4"					some coarse to fine Grav				
				50.3"										
			<u></u>							BORING C	OMPLETED AT 50.3 FEET	_		
											FUSAL ON ROCK		_	
55		_										_		
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		Approximate Change in Strata: Inferred Change in Strata:
Core Size	in	
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Weight/type of Hammer on Split Barrel		or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Weight/type of Hammer on Drive Pipe		to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.

					_						8949_SB9_BORING	LOG		
			3		PF		NAME:			e Collection BORING NO.		SB-9		
	CON	SULT		■ 3•	<u></u>		CATION:			ite Plains, NY JOB NO.		8949		
	ENG	INE	E R S		—		ETHOD:		Holk	w Stem Auger GROUND ELEVAT	ION:	154±		
	NG BY:	Gener	al Boring	gs Inc.	D.	ATE ST	ARTED:	10/1	5/2014_	GROUNDWATER TABI	E DEPTH			
	ECTOR:		JN		DATE	COMP	LETED:	10/18	5/2014	0 Hr. 10'± Date 10/15/14 24 Hr.	Date			
DEPTH	SAMPLE	REC	_	PTH	Blows on Spoor			1	N					
(ft)	No.		FROM	_	↓					SOIL DESCRIPTION AND STRAT	TEICATION	Symb		
0		(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)			USC		
										1.5" Asphalt, 2.5" Subbase				
	1	20	1		13	13			24	FILL: Gray medium to fine SAND, some coarse	to fine Gravel			
			<u> </u>	3			11	8		little Silt, with Brick				
	2	10	3		9	10			22	Same				
5				5			12	8				\vdash		
	3	24	5	<u> </u>	4	5			9	Brown medium to fine SAND, little coarse to fin	ne Gravel, trace Silt	_		
	 	_		7			4	2						
	4	18	7_		2	2			6					
				9			4	4		Gray CLAY, little fine Sand, with Organic fibers		_		
10														
	5	24	10		6	7			14	Gray-brown fine SAND, some Silt				
				12			7	8		Brown fine SAND, little Silt				
15														
	6	24	15		7	8			18	Same	-	_		
				17			10	14						
												<u> </u>		
20														
	7	24	20		4	5			12	Same	-			
				22			7	10						
25														
	8	12	25		6	6			14	Brown coarse to fine SAND, some coarse to fine	Gravel, trace Silt	 		
L		I	I	27			8	7			,			
		I												
]										
30														
I	9	24	30		6	10			30	Same	_	+		
				32			20	30						
			I							Brown fine SAND, little Silt				
35									$\neg \neg$					
I	10	10 18 35 14 27 54 Brown coarse to fine SAND, little coarse to fine Gravel, trace Silt			-									
				37			27	33		, , , , , , , , , , , , , , , , , , ,	uno ont	_		
Γ							$\neg \neg$	_				—		
Γ								$\neg \uparrow$	$\neg \neg$					
40 F												<u> </u>		

	_
Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1% in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate	Change in Strata:	 Inferred Change in Strata:	

1 3	91				P		T NAME:		TI	ne Collection BORING NO.	SB-9
'	CON	SUL	TING	3	<u> </u>		CATION:			ite Plains, NY JOB NO.	8949
BOD		NE			 _		ETHOD:			ow Stem Auger GROUND ELEVATION:	156±
_	ING BY: ECTOR:	Gene	ral Boring	gs Inc.			ARTED:	_	5/2014	GROUNDWATER TABLE DEPTH	
DEPTI			JN DE	PTH	DATE	= COMF	LETED:	10/15	5/2014	0 Hr. 10'± Date 10/15/14 24 Hr. Da	te
(ft)	SAMPLE No.	REC	FROM		1	Blows	on Spoor	1	N	SOIL DESCRIPTION AND STRATIFICATION	Symbo
40	INO.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	SOL BESONII HON AND STRATIFICATION	USCS
	11	20	40		11	20			47	Brown coarse to fine SAND, little coarse to fine Gravel, trace	0303
				42			27	36		Silt	
	<u> </u>	 	-	L	-	<u> </u>					
4-	<u> </u>		┼	<u> </u>		_				Boulder at 43 feet	
45	12	24	15		100						
	12	24	45	47	40	44	- 50		97	Brown medium to fine SAND, trace Gravel, trace Silt	
			+	47	┼	├	53	80			
				_	 		-				
50					\vdash						
	13	8	50		50/2"					Weathered Rock	
				52							
											-
			<u> </u>								
55											
	14	3	55		100/3"					Same	
			-	57				_			
			\vdash								
60			\vdash					-			<u> </u>
											-
65											
	15	2	65		50/2"					Same	
				67		-				BORING COMPLETED AT 66 FEET	
1							-+	-+		REFUSAL ON ROCK	
70		_					_	-	-		
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								_	-		
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30 1											

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Orop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Informal Change in Strata:

				_	-	100:55	T 11								8949_SB10	BORING	LOG
	SI		5				TNAME	_		he Collec			BORING			S	B-10
	CON	G	-		CATION	_		hite Plain			JOB NO				3949		
BOB		Cons			+		METHOD:	1 —		low Stem	Auger			D ELEVA		1	63' ±
	ING BY: ECTOR:		ral Borin RG	ıys ınc.			TARTED:		3/2014	+		GRO			LE DEPTH		
DEPTH		т —		PTH	DAT	E COM	PLETED:	10/3	/2014	0 Hr.	11'±	Date	10/3/14	24 Нг.	N/A	Date N	I/A
(ft)	SAMPLE	REC	FROM		-	Blows	on Spoor	า	N								Symbo
0	No.	(in)	(ft)	_	_	6/40	140/40	10/04	0.100	_	SOIL	DESCRI	PTION AI	ND STRA	TIFICATION	1	Cymbo
ا ا		(111)	(10)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)								USCS
		18	1	-	1 9	+-	-		<u> </u>	_		" Gravel					
	<u> </u>	10	+ -	3	 	8	+	_	11	FILL: D	ark brow	n coarse	to fine SA	ND, trace (Gravel, trace	Silt	
	2	4	3	+	6	-	3	3	<u> </u>	-							
5			-	5	 	6	+ -	-	10	with	Cobbles						
Ľ	3	23	5	+	3	3	4	4		┿							
			+ -	7	+ -	1 3	+		6	Brown n	nedium t	o fine SA	ND, little	Silt			
	4	0	7	+-	10	14	3	4		4							
	 		 '	9	10	14	10		32	-							
10			-	+ -		+	18	24		4							
	5	16	10	 	10	-	+-1										
	 	10	10	12	10	9	-		15					rse to fine	Gravel, trace	Silt	
			-	12	+	├	6	6		Gray/bro	wn fine	SAND, li	ttle Silt				
			_	-	-		╀──┤			4							
15					-	 	┝╼╼┩			-							
	6	13	15		5	11	 							- -		. 	
	– 	- 15	-13	17	-	 	10	10	21	Brown co	oarse fine	e SAND,	little coars	e to fine G	ravel, trace S	ilt	
				\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	 	-	10	10		1							
				_		 	-			┨							
20						_				1							
	7	18	20		3	7	┞──┤	\rightarrow	18	Same							
				22			11	27	10	Same							
										1							
							 	-+		1							
25								-		1							<u> </u>
	8	16	25		5	15		\neg	40	with C	obblo					_	
				27			25	50/4"	-10	with C	ODDIC						
								007.1									
1								-+									\vdash
30							 	$\overline{}$									
\neg	9	12	30		16	33				Gray/brow	m cooms	to fine 9	AND 4		ine Gravel,		
ſ				32			50/4"	_		little Silt,				coarse to 1	me Gravel,		
Ī								$\overline{}$		July Citty	m 1140	ran cu tuc	n.				
ļ							$\neg \dashv$	$\overline{}$	-								
35								-+	-							- 1	
\neg	10	3	35		50/3"		-+	$\overline{}$		Weatehred	Rock - e	nneers co	Recover for 1		fine GRAV		
Γ				37			$\overline{}$		<u> </u>	1 some o	narse to	Sand two	or Cilt	te coarse to	nne GKAV	LL, fine	
								$\overline{}$		зоше с				AT 35.25			
									\dashv		DOR		USAL ON		ree!		
40				$\neg \neg$								REF	OBAL UN	RUUK		-	

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

					_					8949_SB11_BORING LOG	11_BORING LOG			
1 3	SESI						TNAME			he Collection BORING NO. SB-11				
`							CATION	_	Wi	hite Plains, NY JOB NO. 8949	8949			
					-		/ETHOD:		Holl	low Stem Auger GROUND ELEVATION: 161' ±	161' ±			
	ING BY:	Gener		igs Inc.	_		TARTED:		2014	GROUNDWATER TABLE DEPTH				
_	ECTOR:		RG		DAT	E COM	PLETED:	10/3	/2014	0 Hr. 15'± Date 10/3/14 24 Hr. N/A Date N/A				
DEPTI	SAMPLE No.	REC	FROM	PTH 1 TO		Blows	on Spooi	n	N		mbo			
0	1.1	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	<u>-</u>	SCS			
										1" Asphalt and 3" Gravel	303			
	1	10	1		13	13			23	FILL: Dark brown coarse to fine SAND, and coarse to fine	_			
				3			10	7		Gravel, trace Silt, with Cobbles and Asphalt fragments	_			
	2	0	3		10	2			3		_			
5				5			1	2		1 ⊢				
	. 3	18	5		2	2			5	Same	_			
				7			3	3						
	4	18	7		11	15			35	Brown SILT, and medium to fine Sand, trace Gravel	_			
				9			20	35		W.C.=19.3% (-200)=52.7%				
10				Γ						W.S15.576 (-200)32.776				
	5	13	10		3	6			15	Brown coarse to fine SAND, little medium to fine Gravel,				
				12			9	9		trace Silt W.C.=4.6% (-200)=2.8%				
										W.C4.0% (-200)=2.8%				
										1				
15										1 –				
	6	24	15		9	9			16	Gray-brown fine SAND, little Silt				
				17			7	12	10	oray-brown rine SAND, fittle Stit	_			
										1				
							 			 	_			
20							 	-	_	 				
	7	24	20		3	4		\neg	10	Same				
				22			6	7			_			
										<u> </u>	_			
										<u> </u>				
25									$\neg \neg$	<u> </u>				
	8	22	25		7	14			30	Brown coarse to fine SAND, trace Gravel, trace Silt	-			
				27			16	19		brown compe to line Britab, trace Graver, trace Suit	\dashv			
										<u> </u>	\dashv			
ſ										ļ <u> </u>	\dashv			
30									$\neg \neg$	<u> </u>				
	9	14	30		20	38				Weathered Rock - appears as Brown/white coarse to fine				
ľ				32			50/3"	\dashv		SAND, some fine Gravel, trace Silt				
- 1				-				-	 	BORING COMPLETED AT 32 FEET	-			
ı		一 †						_		<u> </u>	_			
35								\dashv		REFUSAL ON ROCK				
				_			-+	$\overline{}$		-	_			
ı		$\overline{}$						-+			_			
		$\overline{}$							-		_			
					$\overline{}$			-+	-		_			
40		$\overline{}$	$\neg \uparrow$	\dashv	\dashv		-+				_			

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	140 lb	or judgment of such authorized years. Information at the last of such authorized years.
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
		engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

					_					8949_SB12_BORING L	CG
≤		PROJECT NAME:						_	_		3-12
CONSULTING							CATION:				949
DOD!		BY: General Borings Inc.					ETHOD:				0' ±
_	NG BY: ECTOR:	Gener		gs inc.		_	ARTED:		/2014	GROUNDWATER TABLE DEPTH	
DEPTH	7		RG	PTH	DATE	COMP	PLETED:	10/3	/2014	0 Hr. 16.5'± Date 10/3/14 24 Hr. N/A Date N/A	Α
(ft)	SAMPLE	REC	FROM		-	Blows	on Spoor	3	N		Symbo
0	No.	(in)	(ft)	(ft)	0/6	6/12	140/40	40/04	(1.1/0)	SOIL DESCRIPTION AND STRATIFICATION	
-		(11)	(11)	(11)	0/6	6/12	12/18	18/24	(bl/ft)		USCS
	1	12	1	+	33	26	 		-	2" Asphalt and 4" Gravel	
	 	12	 - 	3	33	20	24	20	50	FILL: Black/gray coarse to fine SAND, and coarse to fine	<u> </u>
	2	7	3	 	3	3	24	20	-	Gravel, trace Silt, with Asphalt fragments	
5		<u> </u>	 	5	-	-	2	3	5	Brown coarse to fine SAND, some coarse to fine Gravel,	
	3	24	5	 	2	6		3	13	little Silt, with Cobble	
	<u> </u>		Ť	7	-	<u> </u>	7	11	13	1	
	4	17	7	 	18	34		11		Design (see a see a C C C C C C C C C C C C C C C C C C	
			-	9	10	34	50/5"			Brown/gray coarse to fine SAND, little medium to fine	
10				Ť			30/3			Gravel, little Silt	
	5	14	10		5	9			18	Decree occurs to fire CAND . 1 th	
				12	-	<u> </u>	9	8	10	Brown coarse to fine SAND, and medium to fine Gravel,	
							-	-		uace Snt	
										•	
15		-								1	
	6	19	15		3	4			10	Brown/gray/black medium to fine SAND, little Silt (odor)	
				17			6	7	10	Drown gray/olack medium to time SAND, fittle Stit (odor)	
- 1										 	
20										}	
$\neg \neg$	7	24	20		4	4			9	Same	
				22			5	5			
									\neg	Brown medium to fine SAND, trace Gravel, trace Silt (odor)	
[$\neg \uparrow$		and the state of the country, that country	
25									$\neg \neg$	 	
	8	19	25		4	6			13	Brown coarse to fine SAND, some coarse to fine Gravel, trace Silt	
				27			7	20		- The Glaver, trace Site	
										ŀ	
										ŀ	
30										ŀ	
	9	24	30		8	8			32	with Fractured Rock	
			\Box	32			24	19		}	
										ŀ	
										<u> </u>	
35										ŀ	
	10	17	35		11	50/5"				Same	
				37						BORING COMPLETED AT 36 FEET	
										REFUSAL ON ROCK	
40											

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Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

LOCATION: White Plains, NY JOB NO. 8949 METHOD: Hollow Stem Auger/Mud Rotary GROUND ELEVATION: 159± BORING BY: General Borings Inc. DATE STARTED: 10/16/2014 GROUNDWATER TABLE DEPTH INSPECTOR: RG DATE COMPLETED: 10/16/2014 0 Hr. 10½ Date 10/16/14 24 Hr. Date DEPTH (ft) No. (ip) (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft						T =					8949_SB13_BORING	LOG		
METHOD Hollow Stem Augen/Mul Rotary GROUND ELEVATION: 1994] 5	S I		PROJECT NAME:					_			B-13		
BORNICS General Borings Inc. DATE STARTED 101/16/2014 GROUNDWATER TABLE DEPTH			SULTING -						_	White Plains, NY JOB NO.				
INSPECTOR: RG	POPI					+			_					
Description SAMPLE REC DEPTH No. Or Or Or Or Or Or Or O			Gener		igs Inc.									
(B) No. No. Company No. No.			T		DTU	DATE	COMF	LETED:	10/1	6/2014	0 Hr. 10'± Date 10/16/14 24 Hr. Date			
O	1	SAMPLE	REC			-	Blows	on Spoor	ı	N		Symbol		
1		No.	(in)	_	+	O/C	6/40	14040	10/04	4 1100		Cymbo		
1	٣		(111)	(11)	(11)	1 0/6	0/12	12/18	18/24	(bl/ft)		USCS		
18			14	1	-	16	16	-			7			
2				 '	2	13	13	10		25	FILL: Light brown coarse to fine SAND, little coarse to fine Gravel,			
Same		2	18	3	+-	6		10		10	-			
3 9 5 7 3 6 7 12 FILL: Light brown coarse to fine SAND, some coarse to fine Gravel trace Sit, with Cobbles 10 9 7 7 14 Same 10 5 5 10 5 8 14 Brown coarse to fine SAND and coarse to fine Gravel, little Sit (Switched to Mud Rotary at 10 ft.) 15	5	- -		-	-	1 0	l °	1	4	12	Same			
10	<u> </u>	3	9	5	+ –	3	6	4	4	-10		-		
4	1				7	-	-	-	7	12	FILL: Light brown coarse to fine SAND, some coarse to fine Gravel			
10		4	16	7	 ' -		7	 ° 		1.4		<u></u>		
10				-			-	 7 	7	14	Same			
S S 10 S 8	10				 		_				1			
12		5	5	10		5	8			14	Dayway and C. CAND			
15					12	Ť	-	6	-5			<u></u>		
6 6 15 3 3 3 6 Same 20 7 7 20 6 6 6 15 Same 25 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel 36 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel								- -			(Switched to Mild Rotary at 10 ft.)			
6 6 15 3 3 3 6 Same 20 7 7 20 6 6 6 15 Same 25 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel 36 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel								\vdash						
20	15													
20		6	6	15		3	3			6	Some _			
20					17			3	3		baile			
7												\vdash		
7														
22 9 7 25	20													
22 9 7		7	7	20		6	6			15	Same —	╀──┤		
30 8 15 30 10 12 26 Gray-brown fine SAND, some Silt Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel					22			9	7					
30 8 15 30 10 12 26 Gray-brown fine SAND, some Silt Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel												\vdash		
30 8 15 30 10 12 26 Gray-brown fine SAND, some Silt Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel														
8 15 30 10 12 26 Gray-brown fine SAND, some Silt 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel 35 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel	25											\vdash		
8 15 30 10 12 26 Gray-brown fine SAND, some Silt 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel 35 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel	L										-	┝─┤		
8 15 30 10 12 26 Gray-brown fine SAND, some Silt 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel 35 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel	ļ							\Box						
8 15 30 10 12 26 Gray-brown fine SAND, some Silt 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel 35 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel	-											\vdash		
8 15 30 10 12 26 Gray-brown fine SAND, some Silt 32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel 35 30 10 12 26 Gray-brown fine SAND, some Silt, some coarse to fine Gravel	-													
32 14 12 Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel	30	\rightarrow												
35 Chay-brown coarse to fine Gravel	-	8	15	30	\rightarrow	10	12			26	Gray-brown fine SAND, some Silt			
35	-				32			14	12		Gray-brown coarse to fine SAND, some Silt, some coarse to fine Gravel			
	- 1	-				- +								
	_ -		\rightarrow	$- \downarrow$	\dashv	$-\downarrow$								
40	35													
40	-	\longrightarrow						\rightarrow	\rightarrow		_			
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40	\vdash			\longrightarrow		\rightarrow	$-\downarrow$							
	40 F	-	-+	-+	-+	+	$-\!\!\!\!+$	-+						
	70													

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Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
Call days 1 dr		Approximate Change in Strata: Inferred Change in Strata:

					Т	O IEC	T NI A NATA		Th			Inonius i		13_BORING	
	51	= {	5		PROJECT NAME:					e Collection					SB-13
	CON	BULT		LOCATION:				_		ite Plains, NY		JOB NO.			8949
BORI	NG BY:		al Borin		+ -	METHOD: DATE STARTED:				tem Auger/Mud Rotary GROUND ELEVATION:				158±	
	ECTOR:	Gener	RG	ys inc.			LETED:		6/2014	011 40			ER TABLE DEPTH		
DEPTH				PTH	DATE	COMP	LETED:	10/16	5/2014	0 Hr. 10'±	Date	10/16/14	24 Hr.	Date	
(ft)	SAMPLE	REC	FROM		1	Blows	on Spoor	1	N		D=00B				Symbo
40	No.	(in)	(ft)	(ft)	0/6	6/12	12/18	40/04	(1-1/6)	SOIL	DESCR	IPTION ANI	D STRATIFICATIO	N	
H-0	9	11	40	(11)	+		12/18	18/24	(bl/ft)						USCS
		11	40	42	26	16	11	1.4			arse to fine	e SAND, son	ne coarse to fine Gra	vel	
	 			42			11	14		little Silt					
				┼──	-		 								
45	\vdash		├─	-		-									
45	 			+		_	-								
	\vdash		-	+											
	 														
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50	 		 	-											
50	10	0	60	 	50.13.0										
	10	0	50	-	50/1"					E	BORING (COMPLETE	D AT 50'1" FEET	_	
				52							R	EFUSAL ON	ROCK		
			 												
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Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
		Approximate Change in Strata: Inferred Change in Strata:

					_			_		8949_SB14_BORING LOG	G
1 5	SI			PROJECT NAME:				_	_	he Collection BORING NO. SB-1	14
	OON				-		CATION:	† 		nite Plains, NY JOB NO. 894	9
BOB!	NG BY:	Conc	_		+	METHOD: DATE STARTED:				em Auger/Mud Rotary GROUND ELEVATION: 159)±
$\overline{}$	ECTOR:	Gener	al Borin RG	ys Inc.		_			6/2014	GROUNDWATER TABLE DEPTH	
DEPTI		· ·		PTH	DATI	E COMP	PLETED:	10/1	7/2014	0 Hr. 16'± Date 10/16/14 24 Hr. 16'± Date 10/	/17/14
(ft)	SAMPLE	REC	FROM		┥	Blows	on Spooi	1	N	SOIL DESCRIPTION AND OTHER PROPERTY.	Symbol
0	No.	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	- COLE DECORDE TION AND STRATIFICATION	
		(4.7)	(17)	1.3	1 0,0	07.12	1270	10/24	(DI/IL)	1.5" Asphalt	USCS
	1	13	1		7	11			21	7 L	
				3			10	6	21	Light brown coarse to fine SAND, little coarse to fine Gravel, trace Silt	
	2	6	3		10	11			20	Same	
_ 5				5			9	9		-	
	3	12	5		6	8			17	with Cobble	
				7			9	9		1	
	4	3	7		15	10			20	Brown medium to fine SAND, trace Silt	
1				9			10	9		j – – – – – – – – – – – – – – – – – – –	
10										<u>]</u>	
	5	16	10		3	3			7	Brown fine SAND, trace Silt	
				12			4	5]	
				<u> </u>	<u> </u>]	
					-		\sqcup			l – – – – – – – – – – – – – – – – – – –	
15					-		\vdash				
	6	8	15		3	50/2"	 			Gray-brown fine SAND, some Silt, with Cobble	
				17	<u> </u>					[
			_		 	<u> </u>	\vdash				
20		_		-			\vdash				
								-		Boulder at 19-21 ft.	
	7	3	21		50/3"						
l				23	30/3		- 			Gray-brown coarse to fine GRAVEL, trace Sand, trace Silt	
- 1								-		_	
25	$\overline{}$				\vdash			\dashv	-	<u> </u>	
	8	8	25		20	40				Gray coarse to fine SAND and coarse to fine Gravel, little Silt,	
ľ				27			50/2"			with weathered Rock	
[$\overline{}$		White World Floor	
									\neg		\dashv
30								$\neg \uparrow$		Weathered Rock	\dashv
								\neg		+	
										 -	\dashv
]							<u> </u>	
35	9	0	34		50/1"					BORING COMPLETED AT 34 FEET	$\overline{}$
				36						REFUSAL ON ROCK	
L										F	_
		\longrightarrow									
		\longrightarrow			\rightarrow			\bot			
40											

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	
0.71		Approximate Change in Strata: Inferred Change in Strata:

			_	_	PI	ROJECT	Γ NAME:	NAME: The Collection BORING NO.					B-15
LOCATIO								201101.					
1	CONS			3			ETHOD:	Holl		m Auger/ Mud Rotary	GROUND ELEVATION:	_	949
BOR	NG BY:	Gener	al Borin	gs Inc.	D		ARTED:		7/2014		DUNDWATER TABLE DEPTH	10	59±
INSP	ECTOR:		RG				LETED:	_	/2014	0 Hr. NE Date	10/17/14 24 Hr. Date		
DEPTH	1 CAMBLE	REC	DE	PTH							Total Date		1
(ft)	SAMPLE No.	REC	FROM	то	7	Blows	on Spoor	l	N	SOIL DESCRI	PTION AND STRATIFICATION		Symbo
0	7.01	(in)	(ft)	(ft)	0/6	6/12	12/18	18/24	(bl/ft)	1			USCS
										1.5" Asphalt			-
	1	3	1		5	7			19	FILL: Brown fine SAND,	trace Silt, with Asphalt fragments		
				3		<u> </u>	12	9					
	2	20	3		4	4			8	Brown fine Sand, trace Si			
5				5		<u> </u>	4	4					
	3	17	5	_	3	5			11	Same			
			<u> </u>	7			6	5					
	4	18	7	<u> </u>	5	5			9	Same			
40				9	-		4	3					
10	-			-	+								
	5	20	10	10	2	4			8	Same			
	\vdash			12	-	-	4	5		(Switched to Mud Rotary a	at 12 ft.)		
				 	-							ļ	
15	\vdash	_		_	-							J	
15	6	10	15	-	8	20	-					_	
	\vdash	10	13	17	+ °	20	40	-	60	Brown coarse to fine SAN	D, some coarse to fine Gravel, little Silt	- [
				1/		_	40	20				ı	
				-			 	-				ı	
20					 		-					ŀ	
	7	7	20		20	50/3"		-	-	Description of Co. CAND	D 1941	4	
				22		50/2		-	\dashv	Diowii coaise to line SAN	D, little coarse to fine Gravel, little Silt	-	
					1				\dashv			ŀ	
								_	$\neg \neg$			ŀ	
25									$\neg \neg$			ŀ	
								\neg	$\neg \neg$			\dashv	
								$\neg \uparrow$	$\neg \neg$			ŀ	
								\neg				ŀ	
									$\neg \neg$			ŀ	
30												H	
	8	3	30		100/3"				,	Weathered Rock: Appears	as black/white/brown coarse to fine SAN	ᆎ	
				32						some medium to fine Grave		_; -	
l		\Box										ŀ	
]									ļ	
35					$oxed{oxed}$							ŀ	
ļ												7	
ļ												r	
ļ								$\perp I$				r	
										BORING C	COMPLETED AT 40 FEET		
40 I	9	0	40		100/1"			- 1	- 1	DEL	USAL ON ROCK	-	

		Approximate Change in Strata: Inferred Change in Strata:
Core Size	in	
Drop of Hammer on Drive Pipe		engineers recommendations contained in the report from which these logs were extracted.
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.

Definitions of Identification Terms for Granular Soils

Our experience has shown that the following field identification system, which is patterned somewhat after the Burmister System, permits a more detailed breakdown of the components within a soil sample than other identification systems allow. It also compels the supervising technician to examine a sample quite closely in order to accurately describe the components within the sample.

Principal Component (All Capitalized)

•	GRAVEL	More than 50% of the sample by weight is Gravel
•	SAND	More than 50% of the sample by weight is Sand
•		More than 50% of the sample by weight is Silt

Minor Component (Proper Case)

•	Gravel	Less than 50% of the sample by weight is Gravel
•	Sand	Less than 50% of the sample by weight is Sand
•		Less than 50% of the sample by weight is Silt

Proportion Terms

•	and	Component ranges from 35% to 50% of the sample by weight
•	some	Component ranges from 20% to 35% of the sample by weight
•	little	Component ranges from 10% to 20% of the sample by weight
•	trace	Component ranges from 0% to 10% of the sample by weight

Size of Soil Components

- Gravel
 - o Coarse gravel ranges from 3 inches to 1 inch
 - o Medium gravel ranges from 1 inch to 3/8 inch
 - o Fine gravel ranges from 3/8 inch to No. 10 sieve
- Sand
 - o Coarse sand ranges from No. 10 sieve to No. 30 sieve
 - o Medium sand ranges from No. 30 sieve to No. 60 sieve
 - Fine sand ranges from No. 60 sieve to No. 200 sieve
- Silt
 - o Material which passes the No. 200 sieve
- Clay
 - o Material which passes the No. 200 sieve
 - o Exhibits varying degrees of plasticity

Gradation Designations

•	Coarse to fine (c-f)	All fractions greater than 10% of the component
•	Coarse to medium (c-m)	Less than 10% of the component is fine
•	Medium to fine (m-f)	Less than 10% of the component is coarse
•	Coarse (c)	Less than 10% of the component is medium and fine
•	Medium (m)	Less than 10% of the component is coarse and fine
•	Fine (f)	Less than 10% of the component is coarse and medium

APPENDIX

	SOI 90		100K		,	.	CLIE	NT:			auncey W ite Plains		nd	SHEET_1_OF_1 HOLE NO.
	(O	FOI	RD, (CT O	6478		PRO.	JECT N			2-9243-12			HOLE NO.
			03) 2				PRO.	JECT N	IAME				* * * * * * * * * * * * * * * * * * * *	BORING LOCATIONS
_			14) 9	46-4	850					80 a	nd 90 - 96	Westch	ester Ave.	per Plan
FO	REMAN - MD/pe	DRIL	LER				LOCA	NOITA		1000 10				
INS	PECTOR					_	-			whit	e Plains,	New Yor SAMPLER		
							1	TYPI	=		HSA	SS	CORE BAR	OFFSET DATE START 12/1
GR	OUND W	ATEF	OBS	ERV/	AOITA	IS	1	SIZE	_		4 1/4"	1 3/8"	2 1/8"	DATE START 12/1 DATE FINISH 12/1
	<u>17</u> FT				RS			HAM	MER W	Τ.		140#	BIT	SURFACE ELEV.
AT.	_FT_AF	TER					ļ	HAM	MER FA	LL.		30"	dia	GROUND WATER ELEV.
			_	SAM	PLE				<u> </u>					
I	CASING				1				ER 6 IN	CORE	DENSITY	STRATA	FIELD IDI	ENTIFICATION OF SOIL REMA
DEPTH	BLOWS	NO	Туре	PEN	REC			N SAMI		TIME	CONSIST		INCL. C	OLOR, LOSS OF WASH WATE SEAMS IN ROCK, ETC.
۵										OLANO IN ROCK, ETC.				
-	1001	1	SS	24"	8"	@ BOT	4	3	T	(MIN)	MOIST	0'2½"	DIACICTOR	
					Ľ		4	5	1		loose	02/3	BLACKTOP bm FM SAND	.sm C sand,F gravel (poss fill)
		0	-	0.40	400	4102		-]		bm FM SAND	sm C sand, F gravel (poss fill)
5		2	SS	24°	13"	4'6"	5	5	+	-	dry loose	1		- 11 /
		3	SS	24"	14*	7'0"	3	3	 	 	dry	İ	It bm M SAND	sm C sand
		-		041	400	0/04	4	4			loose			ion o sente
		4	SS	24"	16"	9'0"	4	3	+	_	dry			
10								-	 	-	loose	10'0"	bm F SAND (poss fill)
		5	SS	24"	10"	12'0"	5	4			dry	100	It bm M-C SAN	ID
ł		6	SS	24"	13"	14'0"	5	7			compact			
ŀ			33	27	10	140	7	9	 		dry		bm F-C SAND	& F GRAVEL
15											Compact			
}		7_	<u>\$\$</u>	24*	16"	17'0"	5	5			moist/wet		bm F SAND	
ŀ							3	3			loose			
[
20		8	88	2/1	100	22'0"	2 .	-						
ŀ		-	23	24	10	220	2	2			wet loose		bm F SAND,tr	silt,C sand
											10000			
25												24'0"		
7		9	SS	24"	13"	27'0"	10	21			wet		gry F SAND, sm	C sand, lit silt, F gravel
							30	43			v dense	27'0"		
H	· · · ·	4	С	60"	52"	33'0"	D/	QD = 8	20/		ı			red BEDROCK
30				VV	UL.	22 W	77(417 = Q	70	2	- 1	28'0"	Refusal	
										2	1	1	BEDROCK (Gr	nelss / Schist)
-				_						2		1	140	
ŀ		-+			-					2		33'0"		A D 20/01
5												- 1		E,O.B. 33'0"
F		\dashv	\dashv		\neg							1		
-		-	-	-	\dashv		-					Į		
												1		
0														
101	con	aitic	ns a	it sp	ecifi	c locati	ons a	and m	av no	ation t repre	represen esent	t		
RO	CON UND SUR	FACI	ns a	t oth	<u>ier lo</u> F	ocation T. US	s or 1 ED	umes		CASING	S THEN	CA	SING TO	FT. HOLE NO. B
= A	UGER L	IP = 1	JNDIS	STUR	BED I	PISTON		T = TH	INWALL	_ 1	V = VANE T			
/UK S=	= WEIGH	II OF	- KUL AMPI	JS LER	1	WOH≃V H.S.A. =	VEIGH HOUL	I OF H DW ST	AMMER EM ALIC	K&ROD	98			C = COARSE
	2007101	0.110	ED.	TDAC	= ~ n	40% I	ITTI E	= 10 - 1	2004 84	OME = 1	20 - 35% A	ND -25 - 50	NG/ F	# = MEDIUM F = FINE

			EST				CLIENT: Saber Chauncey WP LLC and							SHEET 1 OF 1	
	9	0 DC	ONC	VAN	RD.		Saber White Plains LLC							1101 110	3-2
	02	KFO	RD,	CT 0	6478	3	PRO.	ECT N			2-9243-12				
			03) 2				PRO.	ECT N	AME					BORING LOCATIONS	
_			14) 9	<u> 46-4</u>	850					80 ar	nd 90 - 96	Westch	ester Ave.	per Plan	
FO	REMAN -		LLER				LOCA	TION							
INS	MD/pe SPECTOR						-			Whit	e Plains,				
11140	PECIO	`						TVDE			CASING	SAMPLER	CORE BAR	OFFSET	
GR	OUND W	ATE	2 OBS	SEDV	ATION	JC.	1	TYPE			HSA	SS		DATE START 12/11	1/12
	20_FT					10		SIZE	I.D. MER WI	r	4 1/4"	1 3/8" 140#	DIT	DATE FINISH 12/11	1/12
	FT AI			_					MER FA			30"	BIT	SURFACE ELEV. GROUND WATER ELEV.	
				SAM	PLE					T				GROUND WATER ELEV.	
]	-		T	Т		┪				DENSITY	STRATA	EIEI D IDE	ENTIFICATION OF COURT	
王	CASING						BLO	WS PE	R6IN	CORE	OR	CHANGE	INCL C	ENTIFICATION OF SOIL REMAR OLOR, LOSS OF WASH WATE	₹KS
	BLOWS PER	NO	Тур	e PEN	REC	DEPTH	(FOR	CE ON	TUBE)	PER	CONSIST	DEPTH		SEAMS IN ROCK, ETC.	Γ.,
	FOOT	ı			1	@ BOT	0-6	6 - 12	12- 18	(MIN)	MOIST	ELEV			
		1	SS	24"	18"		7	3			dry	0,3,	BLACKTOP		
					-		5	7			loose	1'0"		ND,sm silt,C sand,lit F gravel	
		2	SS	24"	16"	4'0"	7	9	ļ		dry		II bm M-C SAN	D (poss fill)	
5			\vdash		1	-	 	8	-		compact]		
		3	SS	24"	12"	7'0"	4	5			dry		It bm F-C SAN	O & E CDAVE	
			_				5	5			loose		IN SHITT O CHAN	3 4 1 GIOAVEL	
		4	SS	24"	11*	9'0"	5	7			dry		SAME		
10			_		1	-	6	7			compact				
		5	SS	24"	9"	12'0"	5	5			dry		SAME,sm Fgr	mol	
				-			8	7			compact		Oraniciani i gi	avei	
		6	SS	24"	16"	14'0"	7	6			dry		it bm F SAND,s	m M-C sand,lit F gravel	
15				-	-		6	7			compact			•	
		7	SS	24"	13"	17'0"	3	4			dry	1	It how M.C. CANI	2 MA to any 5 amount	
							6	9			loose		IN DITH WITC SAM	O,lit to sm F gravel	
-				<u> </u>]				
20			-	-											
		8	SS	24"	16"	22'0"	2	4			wet		ho E CAND III I	1. December 199	
							4	3		\neg	loose		bm F SAND,iit i	M-C sand, ir siit	
-				_											
25		\dashv	Н		\vdash										
		9	SS	24°	24*	27'0"	3	4	-		wet		brn VF F SAND	61A _ 196	
							5	5			loose		DITT OF THE SAME	III SIIT	
-															
30				\vdash							. [
		10	SS	24"	12"	32'0"	13	14			wet	31'0"			-
							23	41			dense		DIV FM SAND S	n C sand,F gravel,lit siit	
-												ľ	9-7 1 0 1140 01	in o sailo'i. Aisaasi'iit aiif	
35		-									1				
\sim		11	SS	16"	16"	36'4"	23	21	-			Ĭ,	hm E CAND	015	- [
							50/4"						שוויור אמעט,אווי	C sand, F gravel, lit sitt	
ŀ		-										- 1			
40		12	22	OH	Ou	40'0"	EO/OII				wet				
-			CON	ditio	ne r	AVES LOC	hv#	ie in	octi-	otion -	v dense epresent	40'0"	Auger refusal E	.O.B. 40'0"	
6	con	ditio	ıns a	it sd	eciti	c locati	ions a	nd m:	av nof	inon i	epresent sent				
	con	ditio	ns a	it oth	ıer k	ocation	s or t	imes.		•					
ROI	UND SUR UGER L	IP = 1	F TO	STIID	BED I	T. US		T = THII	ARAZAT I	CASING		CA	SING TO	FT. HOLE NO. B-2	<u>-</u>
	= WEIGH					WOH = V					/ = VANE TE S	:01	^	= COARSE	
											-				
	SPLIT TU				!	H.S.A. =	HOLLO	WSTE	M AUG	ER	0-35% AN		N.	= MEDIUM	

	SOI					C.	CLIEN	√T:			uncey W	SHEET 1 OF 1		
			NO/								te Plains			HOLE NO. B-3
			73) 2		6478			ECT N		G172	2-9243-12			
			14) 9				PROJ	ECT N	AME	80 aı	nd 90 - 96	Westeh	ester Ave.	BORING LOCATIONS
F	OREMAN -						LOCA	TION			10 00 - 30	Trestelli	Paral WAR	per Plan
	MD/pe						 			Whit	e Plains, I	New Yor	<u>k</u>	
	SPECTOR							TYPE			CASING	SAMPLER	CORE BAR	OFFSET
G	ROUND W	ATER	OBS	FRV	ATION	IS	1	SIZE			FW/MR 4"/31/4"	SS		DATE START 12/12/12
	<u>10</u> FT								ILER WI	r.	4/3%	1 3/8" 140#	BIT	DATE FINISH 12/12/12 SURFACE ELEV12' *
A	FT_AF	TER	но	URS					MER FA			30"	DII	SURFACE ELEV12' * GROUND WATER ELEV.
	1		- (SAM	PLE									
	CARING						BLO	WS PE	RGIN	CORE	DENSITY	STRATA	FIELD IDE	ENTIFICATION OF SOIL REMARKS
	BLOWS NO Type PEN REC ON SAMPLER TIME OR CHANGE INCL. C													OLOR, LOSS OF WASH WATER.
ă			``			DEPTH		CE ON 6 - 12	TUBE)	FT			* 0' = Ton	SEAMS IN ROCK, ETC. of slab (lower level of parking garage)
┝	FOOT		-	-	-	@ BOT	1 1 1			(MIN)	MOIST	ELEV		or slab (lower level of parking garage)
		1	SS	24"	15"	2'6"	2	5		 	dry	0'5"	CONCRETE bm F SAND sr	π M-C sand,lit F gravel
		2	SS	24*	12"	4'6"	15	14			compact			III W O Sand, III I Graver
1	<u>, </u>		55	24	12	40	17	15 13	-		dry compact	5'0"	hm Manual III /	Daniel E
		3	SS	24"	5*	7'0"	4	4			dry	30	bm M sand,lit (bm F SAND	sand,F gravel
		4	SS	24"	5"	9'0"	3	3			loose			
			- 00	2.4		30	3	3			dry loose			
10		_		0.41	4.49	40101								
		5	SS	24"	11"	12'0"	3	6			wet		2115	
		6	SS	24"	16 ⁿ	14'0"	6	5			loose		SAME	:
15							4	7			loose			
19		7	SS	24°	6"	17'0"	58	29			wet	15'0"	hm FN OALD	
							20	24			dense		DITI FIN SAND,	sm C sand, lit silt, F gravel, tr C gravel
	- 				Щ									
20		8	SS	6"	1"	20'6"	85/6"				v dense	20'0"		
											V 40/130		partially decomp	posed BEDROCK
			\dashv								j		•	
											wet			
25		9	SS	0"	0"	25'0"	50/0"	- "			v dense			
				\neg								26'0"	Roller bit refusa	
													•	E.O.B. 26'0"
30		-												
•														i
		\dashv										ļ		
		-	-	\neg	-						ŀ			
35												ĺ		
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40 NO	TE: Sub-	e cil	CON	1141	ne -	woole	d bus at	la I=		-47	represent			
40	cond	litio	ns a	t sp	ecifi	c locat ocation	ions a	nd m	av not	repre	represent esent			
	DUND SUR	FACE	ETO		F	T. US	SED				THEN_	CA	SING TO	FT. HOLE NO. B-3
	AUGER U								NWALL	. 1	/ = VANE TE			
SS :	SPLIT TU	BE S	AMPL	.ER	-	NOH = V H.S.A. =	HOLLO	W STE	M AUG	ER			II.	C = COARSE M = MEDIUM
PRO	PORTION	SUS	ED: T	RAC	E = 0	- 10% L	JTTLE =	= 10 - 2	0% SC	OME = 2	0 - 35% A	ND =35 - 50		= FINE

	SOI	LTE	EST	ING	, IN	C.	CLIE	VT:	Sab	er Cha	uncey W	SHEET_1_OF_1		
			NO								ite Plains			HOLE NO. B-4
					6478	}	7.7	ECT N		G172	2-9243-12			
			03) 2 14) 9				PROJ	ECT N	AME	00	-100 00			BORING LOCATIONS
F	DREMAN -			10-1	000		LOCA	TION		ou ai	10 90 - 96	westche	ster Ave.	per Plan
	MD/pe									Whit	e Plains, l	New Yor	k	
ÎN	SPECTOR										CASING	SAMPLER		OFFSET
L			· -			—		TYPE			FW/MR	SS		DATE START 12/11/12
	ROUND W					IS	1	SIZE	I.D.		4"/3%"	1 3/8"		DATE FINISH 12/12/12
	<u>10</u> FT . FT AF								MER WI			140#	BIT	SURFACE ELEV12' +
Ë					PLE			HAM	MER FA	LL		30"		GROUND WATER ELEV.
			T	J-KIVI	T		1				DENSITY	CTDATA	EVEL D. ID.	
표	CASING							WS PEI I SAMP		CORE	OR	STRATA CHANGE	INCL C	ENTIFICATION OF SOIL REMARKS OLOR, LOSS OF WASH WATER,
DEPTH	BLOWS	NO	Туре	PEN	REC	DEPTH	(FOR	CE ON	TUBE)	PER	CONSIST	DEPTH		SEAMS IN ROCK, ETC.
L	FOOT					@ BOT		6 - 12	12- 18	FT (MIN)	MOIST	ELEV	*0'= Top	of slab (lower level of parking garage)
		4		100	144							0'8"	CONCRETE	
		1	SS	24"	14"	3'0"	5	4			dry		bm FM SAND,	sm to lit C sand (with a 6" layer of) drk brn
		2	SS	24"	13"	5'0"	3	4			loose dry		F sand, silt SAME,no drk b	nam laura
5				2.17			5	5			loose		OUMENIO OW T	an ayer
		3	SS	24 ⁿ	18"	7'0"	7	8			dry		bm M SAND	
		4	SS	24"	16 ⁿ	9'0"	6	7			compact dry		hm M CAND	
Ì.,							5	5			compact		bm M SAND,si	m F gravel
10	1	5	SS	24"	6"	11'0"	6	6			wet			
					\vdash			- 8			compact		SAME	
 15	 	-	<u> </u>	_										
'		6	SS	24"	6"	17'0"	24	14			wet		SAME	
							10	17			compact		OVINE	
													BOULDER 18 -	· 19'6"
20														
		7	SS	24"	16"	22'0"	7	4			wet	1	SAME, lit F sand	ч
							7	8			compact		o major i ocin	
25												25'0"		
		8	SS	24"	13"	27'0°	19 13	16			wet		gry bm FM SAN	ND,sm C sand,F gravel,lit silt
	-	_					13	12			compact]		
30		9	SS	40#	408	31'6"	00	- 24						
		*	55	10	10	310	30 41	24			wet v dense	32'0"	SAME,lit C grav	rel
											1 00/156		partially decome	posed BEDROCK
35		10	SS	9"	9"	25101	64	EQ/08			wet	ľ		
3		10	55	3	9	35'9"	64	50/3"	-		v dense			
												ļ		
ŀ		-	_	_								38'6"	Roller bit refusa	<u> </u>
40		\dashv								-			E	E.O.B. 38'6"
	TE: Sub	soll	con	ditio	ns r	evealed	by th	is inv	estia	ation :	represent			
	cond	IITIO	ns a	t sp	ecifi	c locat	ions a	ınd ma	av not	repre	sent			
3RC	DUND SUR	FACI	ÉTO,		F	T. US	ED_		(CASING	THEN_		SING TO	FT. HOLE NO. B-4
	AUGER U R=WEIGH					PISTON WOH = V	NEIGHJ	T = THI		8 POD	/ = VANE TE	EST		
3S =	SPLIT TU	BE S	AMPL	ER	ŀ	H.S.A. =	HOLLO	W STE	M AUG	ER			N.	C = COARSE M = MEDIUM
PRC	PORTION	SUS	ED: 1	RAC	E = 0	- 10% L	.ITTLE :	= 10 - 2	0% SC	DME = 2	20 - 35% AN	ND =35 - 50	% F	= FINE

SUBJOURNAL NO. OXFORD, CT 06478 CT (203) 262-9328 PY (914) 946-4860 PROJECT No. 6712-9434-72 PRO					ING		C.	CLIE	VT:	Sab	er Cha	илсеу W	SHEET 1 OF	1				
OXF-DVID, LI 10 4678										Sab				. — –				
NY (914) 946-4850							}	PROJ	ECT N	0.	G172	2-9243-12			Ĺ			
COATION White Plains, New York								PROJ	ECT N	AME					BORING LOCATIONS			
MNI/pe	F					1000		LOCA	TION		80 ar	nd 90 - 96	Westch	ester Ave.	per Plan			
SPECTOR	1		<i>D</i> 1 (1)					LUCA	IION		White	e Plaine	New Yor	l.				
TYPE HSA SS	IN							†			441116				OFFOFT			
SAMPLE S	L								TYPE					OUNE BAR		14/40/40		
AT_ET AFTER_HOURS							IS		SIZE	l.D.								
SAMPLE S													140#	BIT				
CASING No Type Pen REC Per Pen REC Per Pen P		FI AF	TER						HAMN	AER FA	<u>LL</u>		30"					
E CASING BLOWS No Type FEN REC DEPTH DEPTH FORC OF OF SAMFLER TWEE CONSIST NO DEPTH FOR OF OF OF OF OF OF O			_	_	SAN	IPLE												
FOOT	=	CASING			1									FIELD IDE	NTIFICATION OF SOIL RE	EMARKS		
FOOT	15	E BLOWS NO Type PEN REC ON SAMPLER CONSIST DEPTH (FORCE ON TUBE) PER CONSIST DEPTH													OLOR, LOSS OF WASH W	ATER,		
1	10				1			اء ۸			FT	14010=		* 0' = Top	OF SIZE (lower level of parting)	marana)		
1 SS 24 14 2 26 10 6 15	-		-	1	+	-	(By DO I			_	(MIN)	MOIST			tioner tores of parking	Ansaha)		
13 10	ı		1	SS	24"	14"	2'6"	10	6			dry			SAND om C annul lit C annul			
10	1		_	-	0.41	100	4400					compact		bm FM SAND,	sm silt, lit C sand, F gravel			
3 88 24 10 70 8 7	5		2	SS	24"	18"	4'6"							bm FM SAND,	tr silt			
10			3	SS	24"	10"	7'0"							bro EM CAND				
10								5						OILL LINE SWIND				
10		<u> </u>	4	88	24"	12"	9'0"							bm F-C SAND,	lit F gravel			
15	10				-	\vdash		4	4			10058		Salar grave				
S S S S S S S S S S			5	SS	24*	5"	12'0"					wet		SAME				
15			6		245	120	4.4108											
15				55	24	13	140"											
20	15											10056		igry FM SAND,II	it slit,C sand,F gravel			
20 8 ss 24" 15" 270" 8 9 wet compact 25 9 ss 24" 16" 270" 11 13 wet bm VF SAND,it sit bm F-C SAND, sm F gravel 30 10 ss 24" 15" 320" 11 11 wet compact 31 12 12 compact 325 9 ss 24" 15" 320" 11 11 wet ompact 326" bm F-C SAND, sm F gravel 326" bm F-C SAND, sm F gravel, it sit 326" bm F-C SAND, sm F gravel, it sit 326" bm F-C SAND, sm F gravel, it sit 326" bm F-C SAND, sm F gravel, it sit 326" bm F-C SAND, sm F gravel, it sit 45 11 ss 24" 7" 370" 53 38 wet v dense wet vet services wet vet vet services wet vet vet services wet vet vet vet vet vet vet vet vet vet v			7	SS	24"	18"	17'0"					wet		It bm F SAND,t	relit			
8 SS 24" 15" 22"0" 8 9 wet compact 25 9 SS 24" 16" 27"0" 11 13 wet compact 30 10 SS 24" 15" 32"0" 11 11 wet compact 31 10 SS 24" 15" 32"0" 11 11 wet compact 32"6" 35 11 SS 24" 7" 37"0" 53 38 wet	l			-	-	-		12	11			compact						
8 SS 24" 15" 22"0" 8 9 wet compact 25 9 SS 24" 16" 27"0" 11 13 wet compact 30 10 SS 24" 15" 32"0" 11 11 wet compact 31 10 SS 24" 15" 32"0" 11 11 wet compact 32"6" 35 11 SS 24" 7" 37"0" 53 38 wet														}				
25	20		0		0.47	4.50												
25 9 SS 24" 16" 2770" 11 13 wet compact bm VF SAND, lit slit bm F-C SAND, sm F gravel 30 10 SS 24" 15" 32'0" 11 11 compact compact bm F-C SAND, sm F gravel 31 11 SS 24" 7" 37'0" 53 38 wet wet v dense wet yet v dense wet conditions at specific locations and may not represent conditions at other locations and may not represent conditions at other locations or times. 32'6" bm F-C SAND, sm F gravel, lit slit bm F-C SAND, sm F gravel, lit slit lit lit lit lit lit lit lit lit lit			8	SS	24"	15"	22'0"							SAME				
9 SS 24" 16" 27"0" 11 13			\neg						-12			compact						
9 SS 24" 16" 27"0" 11 13												1						
14 10 10 10 10 10 10 10	25		0	60	244	461	07/00	44	40			ľ						
30 10 ss 24" 15" 32'0" 11 11 35			*	22	24	10	2/0											
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35	30		10 (SS	24°	15° (32'0"	11	11					- Falle				
32'6" 32'6" bm F-C SAND,sm F gravel,lit silt							02.0							DM F SAND,(FC bra VE-E SAND	sand			
11 ss 24" 7" 37'0" 53 38 wet v dense wet 39'6" 12 ss 0" 0" 40'0" 50/0" v dense wet 39'6" NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TO FT. USED CASING THEN CASING TO FT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM													32'6"	PILLAL OWNER	in sign c sand			
11 ss 24" 7" 37'0" 53 38 wet v dense wet 39'6" 12 ss 0" 0" 40'0" 50/0" v dense wet 39'6" NOTE: Subsoil conditions revealed by this Investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TO FT. USED CASING THEN CASING TO FT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM	35	-	-	\dashv	-	-				$-\downarrow$				bm F-C SAND,s	m F gravel,lit silt			
29 21 v dense wet 39'6" 12 ss 0" 0" 40'0" 50/0" v dense V dense 12 ss 0" 0" 40'0" 50/0" v dense NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TO FT. USED CASING THEN CASING TO FT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON	-		11	SS	24°	7"	37'0"	53	38	_		wat						
NOTE: Subsoil conditions revealed by this Investigation represent conditions at specific locations and may not represent conditions at other locations or times. SROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS																-		
NOTE: Subsoil conditions revealed by this Investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS	ŀ		12	00	OI I	OF	40108	COIOR	-	\Box								
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS	45		-	\$8	-	0	400	50/0"	-	-		v dense	44101	partially decomp				
conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TO FT. USED CASING THEN CASING TO FT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = METILIAN		TE: Sub	soil	COR	ditio	ns re	veale	by th	is Inv	estica	ation r	epresent	410 1	voller bit refusal	E.O.B. 41'0"			
GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. [HOLE NO. B-5] A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = METULIAN		cond	OIJIE	ns a	it sp	eciti	c locat	ions a	nd ma	ву по	repre	sent	•					
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM	GRO	CONC DUND SUR	FAC	ns a	II OT	ier k	cation T. US		mes.		ASING	THEN		SING TO				
SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = METILIA	A = /	AUGER U	P = 1	JNDI	STUR	BED F	PISTON	-		NWALL	. \	= VANE TE		UITG 10	HOLE NO.	R-2		
PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND =35 - 50% F = FINE						1	484 = V	VEIGHT	OF HA	MMER	& ROD	S						
	PRO	PORTION	SUS	ED:	TRAC	E=0	- 10% L	.ITTLE :	= 10 - 2	700 0% S0	:\ DME = 2	0-35% At	ND =35 - 50	M % F				

i GA

		ILTI				C.	CLIENT: Saber Chauncey WP LLC and							SHEET 1 OF 1		
1		0 DC									ite Plains			HOLE NO. B-6		
		XFO						ECT N		G172	2-9243-12					
-		T (2 Y (9					PROJ	ECT N	AME	80 aı	ad 90 96	Westch	ester Ave.	BORING LOCATIONS		
F	OREMAN	- DRIL					LOCA	TION		00 41	10 00 - 50	WOSLCIN	BOLET AVE.	per Plan		
-	MD/pe						<u> </u>			Whit	e Plains,					
- "	NSPECTO	₹						TYPE	,		CASING	SAMPLER	CORE BAR	OFFSET		
ē	ROUND V	ATE	R OBS	ERV	TION	S	1	SIZE			HSA 4 1/4"	SS 1 3/8"		DATE START 11/19/12		
	T_FT A								IER W	г.	7 /4	140#	BIT	DATE FINISH 12/13/12 SURFACE ELEV12' *		
Δ	T_FT A	FTER	HC	URS				HAM	IER FA	<u>LL</u>		30"		GROUND WATER ELEV.		
r				SAM	PLE											
- Contract	CASING BLOWS PER FOOT		Тур	PEN	REC	DEPTH @ BOT	ON (FOR	WS PE I SAMP CE ON 6 - 12	LER TUBE)	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANGE DEPTH	INCL. C	ENTIFICATION OF SOIL REMARKS OLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC. of siab (lower level of parking garage)		
Γ		1		24"	400	DIDI										
l	<u> </u>	-	SS	24	18"	3'0"	9	5 14			dry/moist compact		bm M SAND,li	t C sand,F gravel,tr C gravel		
	_	2	SS	24"	13"	5'0"	12	11			dry/moist					
ł	5	3	SS	24"	9"	7'0"	9	9			compact wet		j			
							5	4			loose		bm VF SAND,	sm slit		
		4	SS	24"	18"	9'0"	4	4			wet					
1	o			-	-		3	4		<u> </u>	loose		bm FM SAND,	lit to sm C sand		
		5	SS	24"	22"	12'0"	3	4			wet		SAME,lit C san	nd,F grave)		
		6	SS	24"	20°	14'0"	<u>6</u>	3			eacol					
١.						170	4	6			wet loose					
1	5	7	SS	24"	40%	17'0"	0	2								
		-	35	24	19	170	5	3 6			wet loose		bm F SAND,tr	F gravel		
											10036					
20	J			-												
_		8	SS	24"	8"	22'0"	2	3			wet		SAME			
							2	4			loose		O7 1171L			
2	5											25'0"				
		9	SS	24"	18"	27'0"	17 36	18 29			wet		bm F-M SAND,	sm C sand, lit F gravel, silt, tr C gravel		
							30	_ 20			v dense	27'6"				
30	<u> </u>										1		partially decomp	posed BEDROCK		
JL	 	1	C	60°	58"	35'0"	RC	D = 52	%	1.5	}	30'0"	Roller blt refusa	1		
									70	2						
					\dashv					2		j	BEDROCK (Gr	neiss / Schist)		
35										2		35'0"				
														E.O.B, 35'0"		
		-														
40 N/				4541 -	75	- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	l beer 42	ala t								
	con	ditio	ons a ons a	ıt sp ıt oti	ecifi ier l	c locati ocation	ions a s or t	ınd m	av no	ation t repre	represent esent	i e				
GR	OUND SU	RFAC	E TO		F	T. US	SED_			CASING			SING TO	FT. HOLE NO. B-6		
W	R = WEIG	HT OI	F ROI	os	1	WOH = V	VEIGHT	OF H	NWALL AMMER	& ROD	V≃VANETI S	EST	,	= COARSE		
55	= SPLIT II	JBE 8	AMP	_EK		H.S.A. =	HOLLO	DW STE	EM AUG	ER				M = MEDIUM		
LK.	OFURIO	40 US	icu:	IRAU	c = 0	- 10% L	11 1 LE	= 10 - 2	:0% S	JME = 2	20 - 35% AI	ND =35 - 50)% F	= FINE		

				AITE VOOI				CLI	ENT:	S	aber R	eal Estate	Adviso	rs	SHEET_1_OF_1
), C				DD.	O IEO	NO		W 444 4			HOLE NO. B-
) 26:					OJECT	NO.		7-9648-14			
L				94				FR	DJECI	NAME		ystone So	uuara Dh	iaea II	BORING LOCATIONS
F	OREMAN		ILLE	R				LO	CATIO	N	(70	- 96 Wes	tcheste	Avenue)	per Plan
Ŀ	TP/ch	-									Wh	ite Plains	, New Y	ork	
lir.	ISPECTO	K										CASING	SAMPL	ER CORE BAR	OFFSET
	ROUND V	/ATE	D O	DOE	3/ /Λ·	TION	10	=	TY	_		HSA	SS		DATE START 3/25/1
	r_ <u>16_</u> fT						10			'E I.D. MMER '		4 1/4"	1 3/8		DATE FINISH 3/25/1
	FT Α		_							MMER I			140 1 30"		SURFACE ELEV.
F	T .	T		S/	MF	PLE							T 00	ula	GROUND WATER ELEV.
				T				7	OWO I	PER 6 II	CORE	DENSIT	STRAT	A FIELD IDE	ENTIFICATION OF SOIL REMARK
DEPTH	CASING		 Tu	/pe/P	EN	DEC				/PLER	TIME	OR	CHANG	>⊏ I INCL. C	OLOR, LOSS OF WASH WATER
			''	peli		NEC	DEPTH	(FO	RCE C	N TUB	PER	CONSIST	DEPTI	4	SEAMS IN ROCK, ETC.
	FOOT	_	_	\perp	4		@ BO	T 0-0	5 6 - 1	2 12-1	8 (MIN)	MOIST	ELEV	-	1.00
	<u> </u>	-	+-	-	-			-						2" ASPHALT	
		\vdash	+	+	\dashv			+-	+-	+-	-	-		bm F-M SAND,	lit F gravel, tr silt
			\perp	\bot								-	j	1	
5		4	-	2 2	4"	420	7101	144					1		
	<u> </u>		53	<u>s 2</u>	+	13"	7'0"	11 2	8 2	+-	+	dry		bm F-M SAND,	sm concrete, F gravel, lit brick (fill)
										_	-	compact	1	ľ	
10			├-	-	-								ł		
U		2	SS	24	(#) ·	12"	12'0"	15	16	+-	+				
								39	19	_	+	moist v dense		DM FM SAND, S	im brick, F gravel, lit slit, concrete (fill)
				+	\perp	\neg								1	
5			-	+	+	\dashv			-	╂	-				
		3	SS	24	" 1	4"	17'0°	4	4	+-	+	moist-wet	16'0"	hm FM SAND a	no olić lik II ovoroval ko koli to drum
				\bot		\Box		44	17			v dense	100	gry FMC SAND.	m silt, lit F gravel, tr brick (fill) sm silt, lit F gravel, tr C gravel
ł				┼	+-	-			 	-	-	1 1		,	
οĮ					土					1					
-		4	SS	20	1	2"	21'8"	25	39			moist-wet	21'0"		
ŀ		\dashv		\vdash	╀	+		54	60/2*	-		v dense		wh orange highly	weathered BEDROCK
Į						+				 				partially decompo	and REDDOOM
5													25'0"	partially decompo Auger refusal	sed BEDROCK set 3" casing at 25'
ŀ		11	CF	60"	52	2"	30'0"	RO	QD = 4	0%	1,5				Set & Briefly Ot 20
H		+		\vdash	+-	- -					1.5 1.75				
		\Box									1.75				
-	-	2	~	COT	1 60	NR 4	2EIOI	DC	D - 0	107	1.75	1			
H		4	CT	60"	l or	+	35'0"	- KU	D = 6	1%	1.75 1.75	ľ		BEDROCK (Schie	st)
L											2.0			!	
L			_			T					2.0				ē1
H	-	+	-		_	+					2.0		35'0"		
L	_	\top	1		\vdash	+	-							E.0	.В. 35'0"
匚		\bot												Set well at 24'	
-		+	\dashv		_	+		-	-						
T	E: Suhe	ء ازو	on ₄	ditio	ne	rev	paled	by th	ie in	(Option	etion -	epresent			
v 11 .	cond	tior	is a	it sp	eci	fic	locations	ons a	nd m	av noi	repre	epresent sent			
	ND SURF	ACE	TO.			FT.	USI	D_			CASING			SING TO	FT. HOLE NO. B-7
	IGER UP = WEIGHT									NWALL	& RODS	' = VANE TE			
: S	PLIT TUB	E SA	MPL	.ER		H.8	S.A. = 1	IOLLO	W STE	M AUG	ER			M ~	COARSE MEDIUM
P	ORTIONS	USE	D: T	RAC	E =	0 - 1	0% LI	TTLE =	10-2	0% SC	ME = 20	0-35% AN	D =35 - 50		FINE

Phone (203) 262-9328 Telefax (203) 264-3414



SOILTESTING, INC.

Powdered Bentonite

			90 DONOVAN	ROAD - OXFORD, C	ONN. 06478	-1028
GEOTECHNICAL / ENVI						
,	/ "			Vells - Direct Push/ LICAL PILES - SOI	-	ling
, ,	`	7411111		: Saber Real Esta		18
Monitor Well #	B-7			: G37-9648-14	ile Advisors	
				p Vented Locking Steel Cap		/
Top of Casing Elevation			J duck o	b series rocking atest can	Yes	No No
		<u> </u>	Drive/o	ver w/Bolting Cover	X Yes	No
	_	1	Mound	ed Backfill	Yes _X	(No
Ground Surface Elevation		<u> </u>		PH DECOSITIE	168	
	-		28.88			
	_	12"	Concr	ete Collar	X Yes	No
	12'0"		11 1	il Material		
	120		11 1	ormation f Casing Screen		
				SCH 40 PVC		
Borehole Diameter	8"		- 1	2.067" o.p. 2.375	5"	
			Joint T	ype thd'd F.J.		
	1'0"			neable Backfill		
				ntonite Chips		
	T			Material Silica Sand	·	
	2'0" 1	-				
	1			Packing Silica Sand		
Well Point Elevation	<u>25'0"</u> 10'0"		Filter Fa		Yes X	No
_			If yes, T	уре		
	35'0" 10'0"		Screen	Slot Size .010		
Bottom of Boring Elevation	35'0" 10'0"		Backfill	 		
			,	WILLOW WILL		
			Refusal		Yes	No
Screen <u>10'</u> Riser 15'		Bentonite Pellets		Locking Exp. Plug		-
Plug 1		Bentonite Chips Concrete Mix	1/2 bag 1 bag	Lock D/O		-
Slip Cap		Portland Cement		SIU		•
Silica Sand 450	<u>#</u>					•

Γ					3, IN I RD.		CLIE	NT:	Sal	er Re	al Estate	Advisor	S	SHEET_1_	OF_1
					0647		DDC	JECT N	10	C27	-9648-14			HOLE NO.	B-8
			-		9328			JECT N		637	-9048-14				
					4850		I NO	JEQ I I	47-41VILL	Kev	stone Sq	uare Pha	اا مع	BORING LOCATIONS	
FC	REMAN						LOC	ATION		(70	- 96 West	chester	Avenue)	per Plan Interior loca	tion
	TP/cM										te Piains,			mitorior loca	idon
IN:	SPECTOR	3									CASING			OFFSET	
								TYPE			HSA / FW		NWD\$	DATE START	3/26/14
	ROUND W					NS .	1	SIZE	I.D.		2½"/3"	1 3/8'	2 1/8"	DATE FINISH	3/27/14
	_7_FT /						ı		MER W			140#		SURFACE ELEV.	
AI.	_FT A	TEN	H					HAM	MER FA	ALL		30"	dia	GROUND WATER ELEV.	
		<u> </u>	, 	SAN	/IPLE										
DEPTH	CASING BLOWS PER		Тур	e PE	NREC	DEPTH	(FOR	N SAME	ER 6 IN PLER I TUBE) 12- 18	JFT .	CONSIST	CHANGI DEPTH	E INCL. C	ENTIFICATION OF SOIL OLOR, LOSS OF WASH SEAMS IN ROCK, ETC	WATER,
	FOOT	⊢	+-	\vdash	+	@ ВОТ	-	1	7	(MIN)	MOIST	ELEV	1		
			1	+-	+	 	-	-	+-		4		Basement F	loor - 0'	
											1	ł			
_			-	-	-	ļ]	ļ			
5		4	SS	24"	14"	7'0"	5	11							
		1	33	1 47	1-1-	70	7	6			moist-wet		brn F-M SAND,	tr silt	
											Compact		1		
]		1		
10		2	- 00	24"	13"	12'0"							1		
ł		<u></u>	SS	24	1 13	120	<u>5</u> 11	7			wet compact		brn F-M SAND,	lit C sand	
İ	·										Compact				
5		3	.00	24"	18º	471011	7	- 0							
H		3	55	24	10	17'0"	7	<u>9</u> 5			compact		bm F-M SAND		
											Compact				
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ᅊ		4		24"	14"	22'0"	-	_							
H		-	35	24	177	-220	5 11	9 10			wet		gry brn VFF-MS	AND, lit C sand, silt	
								-10			compact				
_ _					-1						1				
5		5		21"	248	26'9"		44							
\vdash		4	SS			209	9 18	11 60/3"	-		wet	27'0" 27'9"	gry brn VF F SAN	D, tr highly weathered BEDR	OCK
		1	cr	60"	50"	32'0"		$D = 35^{\circ}$	%	2.5	compact	218	Meartiered BEDK	OCK (set casing at 27')	
		\Box	\neg							2.5					
╙		-		-	-			<u> </u>		3.0	ĺ		BEDROCK (Sch	ist)	
\vdash	-	\dashv	\dashv	\dashv	\dashv		-		-	2,5 2.75	1	20101			
H		\dashv	_					-		2.75		32'0"	E /	O.B. 32'0"	
		1										ŀ	۵,(J.D, 32 U	
į		4	_	_	-										
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H		十	\dashv		\dashv	-		-	-			j			
T	cond	itiot	าร at	spe	cific	vealed locations	ons ar	าd ma	estiga y not	tion re repres	epresent sent				
	ND SURF	ACE	TO_		FT	. USE	D			ASING		CA	SING TO	_FT. HOLE NO.	B-8
AL	IGER UF) = U	NDIS	TURE	ED P	STON	T	= THIN	WALL	V	= VANE TE	ST			D-0
	= WEIGHT PLIT TUB					/OH = W! .S.A. = H					3			COARSE	
					= 0 -	10% LIT	TLE =	10 - 20	% SON	.r. ИF = 20	- 35% ANI	D =35 509	M=	: MEDIUM FINE	

Γ					G, IN		CLIE	NT:	Sab	er Re	al Estate	Advisors	i	SHEET 1 OF 1
					1 KD 0647		PRO	JECT N	10	G27	-9648-14			HOLE NO. B-9
			-		9328			JECT N		<u> </u>	-9040-14			
		•			4850		1	OLO: I	ALMAIL.	Kev:	stone Squ	iare Pha	se ii	BORING LOCATIONS
F	OREMAN -						LOC	ATION			96 West			per Plan
	TP/cM										te Plains,			
In	ISPECTOR	3									CASING	SAMPLE	CORE BAR	OFFSET
-		-					4	TYP			HSA	SS	NWD4	DATE START 3/25/14
1.	ROUND W					NS	ľ	SIZE		5	4 1/4"	1 3/8"	2 1/8"	DATE FINISH 3/25/14
	Γ <u>15</u> FT Γ <u></u> FT AI								MER W			140#	BIT	SURFACE ELEV.
Ê	IFI^I	- 1125						HAM	MER FA	ILL .		30"	dia	GROUND WATER ELEV.
		\vdash		SAN	NPLE	-	-							
DEPTH	CASING BLOWS PER FOOT		Тур	e PE	N RE	DEPTH @ BOT	(FOR	N SAMP	ER 6 IN PLER I TUBE) 12- 18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANGE DEPTH ELEV		ENTIFICATION OF SOIL REMARKS OLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
								I					3" ASPHALT /	6º CONCRETE
		_	\bot	-	-									, lit F gravel, cobbles, tr silt
ı			+-	+-	+-	-	-		-					
1 8				+-	+-	 	_	 	 		1			
		1	85	24'	18"	7'0"	5	5			dry		bm F-M SAND	, lit C sand, lit F gravel
			-		—	-	5	5			compact			, o out as int . Bitten
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10				+	\vdash			-						
		2	SS	24"	18"	12'0"	5	4			dry		lit bm tan VF F s	SAND
			_	_			5	7			loose			
			-	-	-									
15			_	 	1	 		-	\vdash	-				
		3	SS	24"	18"	17'0"	2	4			moist-wet		brn VF F SAND	& SLT
							5	6	\Box		loose			VD, lit F gravel, lit silt
		1	cr	60"	60"	23'0"	RO	D = 10	10%	1.5	-	18'0"		
20			UI.	- 00	00	200	110	<u> </u>	0 /0	1.25				
										1.25			BEDROCK (Sc	hist)
				L	_					1.25		- 1	•	,
ł		\dashv						-		1.25		23'0"		
25						-		-		-	- 1	1	E	.O.B. 23'0"
												1	Set well at 18	'' 8' seam
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_	cond	itio	ns a	t spe	cific	locati	ons ai	nd ma	estiga y not	tion re	present sent			
BO!	COND JND SURF	Itio	ns a	t oth	er io	cation:	s or ti	mes.		ACIAIO	TUEN		INO TO	
	UGER UF							= THIN	C/	ASING V	THEN = VANE TES		ING TO	FT. HOLE NO. B-9
OR	= WEIGHT	T OF	ROD	S	V	VOH = W	EIGHT	OF HA	MMER 8	RODS			C=	COARSE
	SPLIT TUE										- 35% ANI)-0E E00	, M=	= MEDIUM

Telefax (203) 264-3414



SOILTESTING, INC.

90 DONOVAN ROAD - OXFORD, CONN. 06478-1028

GEOTECHNICAL / ENVIRONMENTAL S	SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling UNDERPINNING - HELICAL PILES - SOIL NAILS
, , ,	CLIENT: Saber Real Estate Advisors
Monitor Well # B-9	
monnter treat is B 0	JOB #: G37-9648-14
Top of Casing Elevation 0'	Stick Up Vented Locking Steel Cap Yes _X No
Top of storing Electronia	Drive/over w/Bolting Cover X Yes No
Ground Surface Elevation 0'	0' Mounded Backfill Yes X No
	W 1993 1993
_	12" Concrete Collar X Yes No
1 7'0"	Backfill Material
-70	Formation
	Type of Casing Screen 2" SCH 40 PVC
	I.D. 2.067" O.D. 2.375"
Borehole Diameter 8"	Joint Type thd'd F.J.
1	
	Impermeable Backfill Bentonite Chips
T	Backfill Material
2'0"_	#1 Silica Sand
- * -	Screen Packing
1910" 910"	#1 Silica Sand
Well Point Elevation 18'0" 8'0"	Filter Fabric Yes X No
	If yes, Type
Bottom of Boring Elevation 23'0" 5'0"	Screen Slot Size .010
	Backfill Material #1 Sand
	Refusal Yes No
2	
Screen 10'	Bentonite Pellets Locking Exp. Plug 1
Riser 10'	Bentonite Chips 1/2 bag Lock
Plug 1	Concrete Mix 1 bag D/O 1
Slip Cap	Portland Cement S/U
Silica Sand 300# Powdered Bentonite	
· OTTUETOU DELITORING	

	S					3, IN		CL	IENT	:	Sal	ber Re	eal Estate	Adviso	rs	SHEET_1 ()E 2
						RD	-	_								HOLE NO.	B-10
				-)647 9328		_	OJEC			G37	7-9648-14				
			-			4850		I''	OJEC	i N	AME	Key	stone Squ	iara Dh	II	BORING LOCATIONS	
F	OREMA	N - C						LO	CATIO	ON		(70	- 96 West	chester	Avenue)	per Plan Interior local	Alam .
	MD/							4				Whi	ite Plains,	New Yo	ork	miterior local	uon
ll.	NSPECT	OR							_				CASING	SAMPLE	R CORE BAR	OFFSET	
	ROUND	1Λ/Δ	ED	OB6	EDV	ATIO	ME	=		YPE			HSA	SS	NWD4	DATE START	3/19/14
	T <u>9</u> FT						NO			IZE I	I.D. IER W	ir.	4 1/4"	1 3/8 140#		DATE FINISH	3/19/14
A.	TFT	AFT	ER_	_HC	URS						IER F			30"	BIT	SURFACE ELEV. GROUND WATER ELEV.	
\vdash	1	T		- (SAN	IPLE		\top								CITOCIAD WATER ELEV.	
DEPTH	CASIN BLOW PER FOOT	/S N	0	Туре	PEN	REC	DEPT @ BO	H (FC	ON SA	MPI ON	R 6 IN LER TUBE) 12- 18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANG DEPTH	E INCL. C	ENTIFICATION OF SOIL OLOR, LOSS OF WASH SEAMS IN ROCK, ETC	WATER.
	-		-			├		+-							Garage floor	to basement = 11'	
			\exists			-		+	+-	\dashv			-		1	••	
_		Ţ	\Box		0.45												
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			土					12	+-'	' 		-	compact			_	
		+	+	-				1	1	\exists			1				
10		1 2	1	SS	24"	15"	11'0"	7	+,	3			wet		b- FM CAND	***	- 1
								7		3			compact		DM FW SAND, S	im silt from 10' - 10'6"	
	<u>-</u>	+	+	\dashv	\dashv			+	+	4			'				1
			士					+-	+-	+	\dashv					7.7	1
15		3		SS	24°	14"	16'0"	3	1				wet		brn F-C SAND, s	m F gravel	
ł		+	+	+	\dashv	-		2	3	-			v loose			grand,	- 1
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Ť		1 4	†	3	10	10	200"	8	5	+	-		wet compact		brn M-C SAND, II	t F sand, F gravel	
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5		5	S	s 1	8"	18"	25'6"	7	13	+	-	-	wet		SAME		-
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E			33	1	+	-	000	13 19	16	+-	+	-	wet dense	ļt.	om FM SAND, sm	C sand, F gravel	
Г	-		,			\top							dense				1
-		8	SS	119	10	17 9	39'1"	50/1"		┼			wet	- 1			
			00	Ľ		,	1 61	30/1		╫	-	-	v dense	C	obbles, C gravel 3	8 - 40 ¹	
TC	E: Sub	soil	ÇO	ndit	ion	rev	ealed	by th	is in	ves	tigat	ion re	present				
	con	aitic	ns	at s	pec		locati	ons a	nd n	nav	not r	epres	ent				
	ND SUF	FAC	ETC)		_FT.	US	ED				ASING	THEN_	CAS	ING TO	FT. HOLE NO.	D 40
	IGER (WEIGH						STON SH = W		T=TH	IINV	/ALL	V:	= VANE TEST	Γ			B-10
= S	PLIT TU	BE S	AME	² LER	t	H.8	S.A. = 1	HOLLO	W ST	EM /	ALIGE	R			8.4 1	COARSE MEDIUM	
OP	DRTION	s us	ED:	TRA	CE :	0 - 1	0% LI	TTLE =	10 - :	20%	SOM	<u>1E</u> = 20	-35% AND	=35 - 50%	6 F≃F		

Γ					G, II		CLI	ENT:	Sa	ber Re	al Estate	Advisor	rs	SHEET_2_OF_2
					0647	_	PP	DJECT	NO	Car	-9648-14			HOLE NO. B-10
				-	-9328	-		DJECT		G3/	-9648-14			
1			-		4850			JUE()	IANAIAIE	Kev	stone Sq	uara Dh	ann II	BORING LOCATIONS
F	OREMAN	- DRI	LLEF	₹			LOC	ATION		(70	- 96 West	chester	Avenue)	per Plan
L	MD/bo			_						Whi	te Plains,	New Yo	rk	Interior location
IN	ISPECTO	R									CASING			OFFSET
L								TYP	E		HSA	SS	NWD4	DATE START 3/19/14
	ROUND W					NS	7	SIZE	E I.D.		4 1/4"	1 3/8"		DATE FINISH 3/19/14
	9 FT /							HAN	MER W	π.		140#		SURFACE ELEV.
AI	FT_A	FIER	<u></u> H		_			HAN	MER F	ALL		30"	dia	GROUND WATER ELEV.
				SAI	MPLE					T		T	T	
DEPTH	CASING BLOWS PER FOOT		Тур	e PE	N RE	DEPTH @ BOT	(FOI	N SAM RCE ON	ER 6 IN PLER N TUBE) 12- 18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANGE DEPTH ELEV		ENTIFICATION OF SOIL REMARKS OLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
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45		9	SS	18'	18"	45'6"	10	6	-					
				10	1	1 430	9	1 0	+		wet compact		bm F SAND, lit	M-C SAND, silt, F gravel, tr C gravel
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0		יונ	SS	9"	9"	49'9"	51	50/3"			v dense		gry brn FM SAN	ID, lit C sand, F gravel, silt
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Ĭ		1	Cr	60°	10"	60'0"	R	QD = 0	%	0.5	- 1	55'0"	Refusal	
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												- 1	E.	O.B. 60'0"
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UN	COND IN	ions	e at	othe	er loc	ations USE	or tir	nes.						*
\U(GER UP	= UN	DIST	URBI	ED PIS	STON	T	= THIN	WALL	SING V≃	THEN	CASī	NG TO	FT. HOLE NO. B-10
? =	WEIGHT	OF R	ODS	;	W	OH = WE	EIGHT (OF HAN	MER &	RODS	17 u ur 1EQ		C=	COARSE
of PO	PLIT TUBE	ISED	NPLE.	K ACE	H.ն • ∩ =	S.A. = H	OLLOV	V STEN	AUGE	₹	35% AND			MEDIUM
	- TONO	لاتاب	117	7UE	- 0 - 1	U70 LII	UFF =	ıu - 209	% SOM	E = 20 -	35% AND	=35 - 50%	F - F	FINE

F

(203) 264-3414



SOILTESTING, INC.

	90 DONOVAN ROAD - OAFORD, COMN. 06478-1028
GEOTECHNICAL / ENVIRONMENTAL S	SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling
	Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling
	UNDERPINNING - HELICAL PILES - SOIL NAILS
	CLIENT: Saber Real Estate Advisors
Monitor Well # B-10	JOB #: G37-9648-14
Top of Casing Elevation 0'	Stick Up Vented Locking Steel Cap Yes X No
Top or Casing Elevation	Drive/over w/Bolting Cover X Yes No.
140	Drive/over w/Bolting Cover X Yes No
Ownerd Confess Florefier	Mounded Backfill Yes _X No
Ground Surface Elevation	
	12" Concrete Collar X Yes No
-	Backfill Material
<u> 26'0"</u>	Formation
-	Type of Casing Screen
	2" SCH 40 PVC
Borehole Diameter 8"	I.D. 2.067" O.D. 2.375"
Borelloie Diameter	Joint Type thd'd F.J.
! 2'0"	Impermeable Backfill
	Bentonite Chips
	Backfill Material
I Olor	#1 Silica Sand
	-
	Screen Packing
Well Point Elevation 40'0" 10'0"	#1 Silica Sand
Well Point Elevation 40'0" 10'0"	Filter Fabric Yes X No
	If yes, Type
Bottom of Boring Elevation 60'0" 20'0"	Screen Slot Size .010
Bottom of Boring Elevation	Backfill Material #1 Sand
	Packill material #1 28110
	Refusal X Yes No
Screen 10'	Dankarth Datt.
Screen 10'	Bentonite Pellets Locking Exp. Plug 1
Plug 1	Bentonite Chips 1/2 bag Lock Concrete Mix 1 bag D/O 1
Slip Cap	Portland Cement 5/U
Silica Sand 300#	
Powdered Bentonite	

	\$					INC		CL	IENT:	Sa	ber Re	eal Estate	e Advis	ors		SHEET_1 O	F 2
				NOV. D, C												HOLE NO.	B-11
				3) 26					OJEC	T NO. T NAME	G37	7-9648-14	4				
				i) 94				l''	OJEC	INAME	Key	stone So	nuara D	lhar	no !!	BORING LOCATIONS	
Ī	OREM	AN - D						LO	CATIO	N	(70	- 96 Wes	tchest	er A	Venue)	per Plan	
		<u>cMc</u>						┸			Whi	ite Plains	, New	Yor	k		
	NSPECT	OR										CASING		LER	CORE BAR	OFFSET	
	ROUNI	1Α/Δ1	ED C	NDOE	DVVV.	CIONO		4		PE		HSA / F		_	NWD4	DATE START	3/19/14
	T <u>10</u> F									ZE I.D.	-	41/4"/3			2 1/8"	DATE FINISH	3/19/14
	I_FT					•				MMER V			140		BIT	SURFACE ELEV.	
F		T		S	AMF	TE			- 11/	nanan-17	<u> </u>		30		dia	GROUND WATER ELEV.	
			T	Ĭ				1_			1	DENSIT	YSTRA	т.	EIELD IDE	A series de la massa de la mas	
	CASII		_ _					BL	OWS IAR MC	PER 6 IN MPLER	CORE	OR	CHAN		INCL C	ENTIFICATION OF SOIL I OLOR, LOSS OF WASH	REMARKS
T C C C C C C C C C C C C C C C C C C C	PER	ימן פע	יון ט	ype F	EN		EPTH	(FO	RCE (ON TUBE	PER	CONSIS	T DEP	TH		SEAMS IN ROCK, ETC.	WATER,
L	F001	·					BOT	0-	6 6 - 1	2 12- 18	(MIN)	MOIST	ELE	$\overline{\mathbf{v}}$			
		_	\perp		\dashv	\Box							+==		2" ASPHALT /	3" lit cohbles	
	-	+	╬	+	\dashv			-	-]	1		brn FM SAND,	sm F gravei, lit silt	
		\top	+	十	+	-			+-		-	-	1				
!												1	5'0"				
		11	- 8	s 2	4"	7"	7'0"	2	3			I moist			brn SILT, sm FN	A sand, tr F gravel, tr asphalt (f	RIIV.
	-		+	+	+	-		2	3			stiff				and a second a probately (1	""/
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		12	S	S 24	4" 1	8" 1	2'0"	<u>7</u>	10			wet		Ŀ	orn VF F SAND,	lit to tr silt	
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		-	<u> </u>		\perp									ol	bm VF SAND,	tr silt	
25		-	-	⊬	+-	+-	-					ľ]
		5	SS	24"	19	27	"0"	4	7	+ +		wet			- CHO OLUB		1
								14	11			compact		PI	I PMC SAND, 8	m F gravel, lit slit	
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30 T				-	+-	\vdash	-	· -		┝╌┼							1
		6	SS	24"	13'	32	0"	28	18		_	wet		hm	EC SAND em	F-C gravel, lit silt	
-					_			21	35			dense	31'6"		11 -0 OM(4D) 8tt	r-c graver, lit slit	
-				_		┼	_							brn	wh weathered	BEDROCK	
35		7	ŞŞ	2"	2"	35'	2" 6	0/2"		-		wet of the wet of the		SAI	ur		
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10		1	cr	60°	42 ⁿ	43'6	3"	RQ	0 = 67	%	2.0	-			er refusal DROCK (Schis	,	
TOP	E: Sub	soil	con	ditio	ns	evea	iled b	v th	le inv	enitae	ion ro	present	<u>(i)</u>	المالية ا	TOOK (OUIS	0	
	COU	aitio	ns a	it SD	ecit	IC 101	catio: ions	ns ai	nd m	av not i	repres	ent					
ROL	ND SUF	RFACE	: TO .			- T.	USE			C/	ASING	THEN	C	ASIN ¹	G TO	ET ISOUE NO	
. = Al	IGER (WEIGH	JP = L	INDIS	STUR	BED	PISTO	NC	T	= THI	NWALL	V=	VANE TE	ST	-OIIV		FT. HOLE NO.	B-11
S = S	PLIT TU	BE S/	AMPL	.ER		H.S.A	. = HC	OLLO	N STE	MMER &	D					COARSE	
ROP	ORTION	S USE	D: 1	RAC	E = 0	- 10%	6 LIT	TLE =	10 - 2	0% SON	/E = 20 ·	-35% ANI	D =35 - 56	0%	M≃l F≈F	MEDIUM	

Γ				rinc VAN	•		CLIE	ENT:	Sak	er Re	al Estate	Advisors		SHEET_2	OF_2
	-			CT ('	PRC	JECT	NO	G27	-9648-14			HOLE NO.	B-11
				262-				JECT		637	-3040-14			DODDING LOCATION	
		-		946-					47 497122	Kev	stone Squ	uare Pha	se II	BORING LOCATIONS per Plan	
F	OREMAN	- DRI	LLER				LOC	ATION	-	(70 -	96 West	chester A	venue)	per Flan	
	TP/cM										te Plains,				
IN	SPECTO	₹									CASING	SAMPLER		OFFSET	
								TYP	E		HSA / FW	SS	NWD4	DATE START	3/19/14
GF	ROUND W	ATE	R OB	SERV	ATIO	NS		SIZE	I.D.		41/4" / 3"	1 3/8"	2 1/8"	DATE FINISH	3/19/14
	_10_FT		-					HAN	MER W	Т.		140#	BIT	SURFACE ELEV.	
AI	FT_A	FIER	H	_			-	HAM	MER FA	ALL		30"	dia	GROUND WATER ELEV	<i>'</i>
				SAM	1PLE		1			Т					
_	CASING		1				BLC)WS PI	ER 6 IN	CORE	DENSITY		FIELD IDE	ENTIFICATION OF SO	IL REMARK
DEPTH	BLOWS		TVD	e PEN	IREC		0	N SAM	PLER	TIME	OR	CHANGE DEPTH	INCL. C	OLOR, LOSS OF WAS	H WATER,
씸	PER		1.36	7 -		DEPTH	1 0 0	RCE ON	N TUBE)	PER FT	CONGIST	DEPTH		SEAMS IN ROCK, ET	C.
_	FOOT		_	₩	_	@ BOT	- 0-0	0-12	12- 18	(MiN)	MOIST	ELEV			
	\vdash	-	-	-	+-	 	┼	+-	-	2.0					
	<u> </u>	-	-	+	+-	 	+	-	-	2.0	-				
				1	1	 	+-		+	2.75	1				
45		2	CT	60"	30"	48'6"	F	RQD =	0%	2.0	1				
										1.75			BEDROCK (S	Schist)	
			 	┼—	-	-	├	-		2.5				•	
			 		├		-	-	-	2.5 3.0					
50		3	СГ	60"	50"	53'6"	R	$\overline{QD} = 5$	0%	2.0					
										2.0					
			_							2.0					
ŀ			-		-		_	-		2.0	[E010#			
55	- 1		<u> </u>	H						2.0		53'6"		O D FOIGH	
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													9)		
T	E: Subs	oil c	ond	ition	s re	vealed	by th	is inv	estiga	tion re	present	· · · · · · · · · · · · · · · · · · ·			
	cond	itior	ıs at	spe	cific	locatio	ons a	nd ma	ay not	repres	sent				
ÖL	IND SURF	ACE	TO.	om	er 10 FT	cations	s or ti	mes.		ASING	THEN	240	ING TO		1
AL	JGER UF	2 = U	NDIS.	TURB	ED P	ISTON	Т		WALL	V	= VANE TE			_FT. HOLE NO.	B-11
	= WEIGHT	-				/OH = W								COARSE	
	SPLIT TUB					.S.A. = 1					- 35% ANI		M=	MEDIUM	

F

1	SOI	LTE	ST	INC	3, II	VC.	CLI	ENT:	S	aber F	Real Estat	e Advisor	s	SHEET 1 OF 1
		(FOF					PRO	OJEC.	TNO	G	37-9648-1	4		HOLE NO. B-1
		T (20						_	NAME		77-9040-1	4		
<u></u>	N	Y (91	4) 9				1	3320	. 147 001		ystone S	nuere Dhe	ano II	BORING LOCATIONS
FOREW			LER				LOC	OITA	N	(70	- 96 Wes	tchester	Avenue)	5' Offset
\vdash	P/cMc	_								WI	nite Plains	, New Yo	rk	
INSPEC	CTOR										CASIN			OFFSET
222111								TY	PE		HSA	SS	NWD4	DATE START 3/21/14
GROUN								SIZ	ZE I.D.		4 1/4"	1 3/8'	2 1/8"	DATE FINISH 3/21/14
AT <u>non</u> ATFT						5			MMER			140#	BIT	SURFACE ELEV.
	7 "	1 1-1						HA	MMER	FALL		30"	dia	GROUND WATER ELEV.
	ŀ	_		SAN	PLE	-	4					T		
CAS BLO PER FOO	ows I	NO	Туре	PEN	RE	DEPTI	(FOF	N SAI	PER 6 I MPLER ON TUB 2 12-1	E) PER	OR	CHANGE T DEPTH	INCL. C	ENTIFICATION OF SOIL REMARK OLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
100	-	-		\vdash	-	@ BO	'	1		(MIN)	MOIST	ELEV		
							1	+-			-		3" ASPHALT /	1" PROCESS GRAVEL
<u> </u>	\perp	_									-		INT SILI & FM	SAND, lit F gravel
5	-	-	_		ļ		-		4_]		1	
—		1	SS	24"	164	7'0"	3	+-	-	+-			1	
					10	1 7 0	5	6	+	+	I moist loose	5'6"	drk brn SILT, sr	m F-M sand, tr F gravel, tr roots
		\Box						Ť	1	+	loose		III DM VF FM SA	AND, lit silt, lit F gravel, tr roots, C sand
<u>,</u>	_	4									j	i		
0	\dashv	2	ss	24"	151	4010#	40						1	
	+	4	55	4	10	12'0"	13	16	+	+	dry	1	brn VF FM SAN	D, sm F gravel, lit silt, tr weathered bedrool
							-	10	+	-	compact		ĺ	
			I	\Box						_	1			
5	-	_			OII	4====					1			
	+	3 8	S	14"	0"	17'0"	<u>13</u> 9	11			dry compact	17'0"	No recovery	
-	+	+	+	+	\dashv								poss weathered I	BEDROCK or BOULDER
											1 1	19'6"	Auger refusal	
-	- 1	_ _c	1 6	0"	12"	24'6"	RC)D = 1	1%	3.0		700	reger relusar	
<u> </u>	+-	+	+		-					4.0				
 		+	+	+	\dashv				-	3.0		Į.	BEDROCK	
							-			4.0		24'6"		
		\Box		I						7.0		24 0	E	O.B. 24'6"
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CÓI	nalu	ons	ai s	pec	ITIC	locatic	ns an	id ma	estiga av not	ation r	epresent			
COI	naiti	ons ons	at s at o	pec the:	me Lloc	locations	ns an	id ma	ay not	repre	sent			
COI COI UND SU	nditio JRFAC	ons ons ETC	at s	pec the	ric Loc FT.	iocations ations	ns an or tin	id ma nes.	ay not	CASING	THEN_	CASI	NG TO	FT. HOLE NO. B-12
COI UND SU UGER = WEIG	ndition JRFAC UP =	ons ons ETC UNDI	at s at o	pec thei	rioc FT. DPIS	iocations ations	ons an	nes. THII	ay not	CASING V	THEN_	CASI		FT. HOLE NO. B-12

						, IN		Ci	JEN	T:	Sal	ber Re	eal Estat	e A	dvisor	3	SHEET_1 O	F 1
						1 KD. 1647:							·				HOLE NO.	B-13
				•		9328	_			CT		G37	7-9648-1	4				
L						4850		IPR	(OJE	CT1	NAME	Key	stone S	alla	ro Dha	II	BORING LOCATIONS	
F	OREMAN		RILLI	ER				LO	CAT	ION		(70	- 96 Wes	stch	ester /	Avenue)	per Plan	
L	TP/cl											Whi	ite Plains	s, N	ew Yo	rk		
"	arec i C	ĸ									_		CASIN		SAMPLE	CORE BAR	OFFSET	
Gi	ROUND	VATI	RC	DBSF	RV	ATIO	VIS.	=		TYP!	E I.D.		HSA		SS	NWD4	DATE START	3/24/14
	<u>16</u> FT						10				MER W	т	4 1/4"		1 3/8"		DATE FINISH	3/24/14
Αī	FT_A	FTE	R	HOL	JRS						MER F				30"	BIT	SURFACE ELEV. GROUND WATER ELEV.	
Г				S	AM	PLE							1				OROGNO WATER ELEV.	
DEPTH	CASING BLOWS PER FOOT) T	уре	PEN	REC	DEPT @ BC	TH (FC	ON S	AME AO E	ER 6 IN PLER I TUBE) 12- 18	CORE TIME PER FT (MIN)	DENSIT OR CONSIS	T I	STRATA CHANGE DEPTH ELEV	FIELD IDE INCL. C	ENTIFICATION OF SOIL F OLOR, LOSS OF WASH I SEAMS IN ROCK, ETC.	REMARKS WATER,
	-	┝	+	\dashv		-	-		Ţ					1		brn FM SAND,	sm silt, lit F-C gravel (fill)	
			1	士				+-	\dashv			-	-				S was (my	
5		-	+	+									1					
		1	+ 5	s :	24"	18"	7'0"	2	+	1		1	l moist					
			I					4		8			loose			DM F SAND & S	SILT, lit M sand, tr F grave! (pos	ss fill)
		┝	+	+	\dashv			-	+				1					
0			上						+				1					
ŀ		2	S	s 2	4 ⁿ	17"	12'0"		T	11			dry			brn F-M SAND,	tr C sand, tr F gravel	
ŀ			+	+				6	+	7			compact			•	a de la composición dela composición de la composición de la composición de la composición de la composición de la composición de la composición de la composición dela composición de la composición de la composición de la composición de la composición de la composición de la composición de la composición de la composición de la composición de la compos	300
_[T														
5		3	85	1 2	A¥	12"	17'0"								ł			
			88	1	+	14	170	24		0			moist-wet dense]	gry bm F-M SAN	D, lit silt, F gravel	
F				I	\Box	\Box							deliae		- 1			
忄			\vdash	+	+	-		-	╀		-+							
		4	SS	24	10	14*	22'0"	9		5			i moist					
F		-		+	+			7	1	1			compact					
H		\dashv		+	+	+		-	╀	\dashv			d	_	0101			
		5	SS	9'		8"	25'9"	62	60	/3"			dry v dense		3'6"	oss weathered R	EDROCK or BOULDERS	
┝		\dashv		-	+	+				\dashv	\neg				ľ	and modelinited D	LDIVOCK BY BOOLDERS	
		\dashv		-	十	+			-	+								
L		\Box			I								dry				th.	
H	_	6	ss cr	0"	1		30'0" 33'6"	60/0"		= 0%	- 1		v dense	30	"0"			
			VI.	-1-	ľ			10	QD.	70		-			pa	artially decompos	ed BEDROCK	
_		4			I	\perp												
	-	2	cr	60"	5	4 3	38'6"	PC	<u>רונ</u>	<u> </u> 47%	-		-	33	16"			1
			Ü,	-			,,,,,		<u> </u>	47 /		-						
		4													BE	DROCK		
_		+	\dashv		┢	+				+		_		001				- 1
						土				士					6"	EO	B. 38'6"	
TE	: Subs	oil (on	diti	ons	rev	ealed	by th	is i	ηve	stigat	ion re	present			E.U.	uu v	
	cona	itioi	1 s a	it si	Jec	ITIC I	locati	ons a s or t	nd	ma	/ not r	epres	ent					
	ND SURF	ACE	TO			_FT.	US	ED			CA	SING	THEN_		CASI	NG TO	FT. HOLE NO.	B-13
	GER UF WEIGHT							TEIGHT	() =]	HIN/	NALL IMER &	V:	VANE TE					2-13
= SI	PLIT TUB	E SA	MPL	.ER		H.S	S.A. = 1	HOLLO	WS	TEM	AUGE	2					COARSE MEDIUM	.
PC	RTIONS	USE	D: 7	RAC	CE =	0 - 1	0% LI	TTLE =	10	209	6 SOM	E = 20	-35% AN	D =3	5 - 50%	F=F		

	\$				G, IN		СП	ENT:	Sal	ber Re	SHEET_1 OF 1							
- 1					N RD 0647	-								HOLE NO.	B-14			
1				-	-9328			DJECT		G37	7-9648-14							
1					-485 0		PRO	DJECT	NAME	Kov	BORING LOCATIONS							
F	OREMA	- '					LOC	ATION	1	(70	per Pla	n						
L	TP/c							_		Whi								
ļ"	VSPECT	OR									OFFSET							
							TVDE HOA OO MANE							DATE START	3/20/14			
	ROUND T_none								E I.D.		4 1/4"	1 3/8"	" 2 1/8" DATE FINISH 3					
	TFT)			MER W			140#	BIT	SURFACE ELEV.				
F	1	1		_	MPLE		+=	MAIN	MER F	ALL.		30"	dia	GROUND WATER ELE	·V			
ı		\vdash		JAI	VIPLE	.	\dashv				DENOM							
12	CASIN		1	- 1		1			ER 6 IN	CORE	DENSITY	STRATA	1	ENTIFICATION OF SO	OIL REMARKS			
DEPTH	BLOW	SINC	Ty	pe PE	NREC		/EOF	N SAM RCE OI			CONSIST		11100,0	OLOR, LOSS OF WA SEAMS IN ROCK, E	SH WATER,			
٦	PER DEPTH 6 BOT					יוי	(FORCE ON TUBE) 0-6 6-12 12-18			MOIOT		-1	OLAMO NA ROCK, ETC.					
Г			_		_	18 201	1	T		(MIN)	MOIST	ELEV	21 ACDUALT					
													S ASPRALI /	6" PROCESS GRAVEL				
			+	+-	+						1							
5	;	+	+	+	+-	 	-	+		-	-							
		1	SS	24"	22"	7'0"	5	7	1		dry	5'6"						
		+					11	17			v stiff	30	It bm SILT, sm), sm silt, lit F gravel (fill) F SAND, tr F gravel, roots				
								-	-					Trylland Course, ii tograver, foots				
10		+	+	+-				_	 			40101	1	•				
		2	SS	24"	17"	12'0"	12	12			1 moist	10'0"	It bm FMC SAN	D 19 E				
i		↓	-				10	5			compact		gry brn FMC SA	D, iit F gravei ND, lit F gravei, siit				
		-	 	-										and the Braton, one				
15		3	SS	3"	2°	15'3"	60/3"		 		l moist v dense	13'6"						
		1	cr	60 ⁿ	57*	20'6"		QD = 5;	2%	3.0	A detise	15'6"	poss partially de Auger refusal	composed BEDROCK from	n 13'6"			
ŀ		-								4.0	ı		Tiogor Toldoza					
ŀ		-								3.0 4.0	- 1							
0										4.0	ĺ		BEDROCK (Sch	nist)				
-												20'6"						
ŀ		$\vdash\vdash$			\rightarrow								E.	O.B. 20'6"				
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T	E: Sub	soil d	ond	itlon	s rev	ealed	by thi	s inv	estigat	ion re	present							
	cond	IITIOI	is at	spe	CITIC	locations	ns ar	ıd ma	y not i	repres	ent			•				
οŪ	ND SUR	FACE	TO_		FT.	USE	D		C/	SING	THEN	CAS	NG TO					
AU	IGER U	P = U	NDIS1	TURB	ED PIS	STON	T	= THIN	WALL	V=	VANE TES	UAS: T		FT. HOLE NO.	B-14			
	= WEIGH PLIT TU				W(DH = WE S.A. = H	:IGHT (DF HAI	MMER &	RODS				COARSE				
					= 0 - 1	0% LIT	TLE =	10 - 20	% SON	IE = 20	- 35% AND	=35 - 50%		MEDIUM FINE				
													L=1	11.4[- 1			

	SOILTESTING, INC. 90 DONOVAN RD.							CLI	ENT:	Sa	ber Re	SHEET 1 OF 1					
			ORD,				ł	DD	JECT	NO	027	0640 44			HOLE NO.	3-15	
			203)				,			NO.	G3/	-9648-14					
			14)					FRO	MEGI	INMINE	Kev	stone Sq	uare Ph	ase II	BORING LOCATIONS		
F	DREMAN		LLEF	₹				LOC	ATION	ı	(70	per Plan					
L	TP/cM										Whi	te Plains	New Y	ork			
IIN	SPECTO	₹													OFFSET		
CF	ROUND W	/ATC	D OD	CED	VAT	ON	0	╡	TYF	_		HSA	SS		DATE START 3/21	/14	
	12 FT						5			E I.D. VIMER V		4 1/4"	1 3/8		DATE FINISH 3/21	/14	
	FT A							1		MMER F			140 1 30"		SURFACE ELEV.		
	T	T		SA	MPI	ΙĒ					1	T	T 00	ula	GROUND WATER ELEV.		
	CASING BLOWS PER FOOT	SING OWS NO Type PEN REC.					(FOI	N SAM	PER 6 IN IPLER N TUBE 2 12-18	TIME PER	DENSITY OR CONSIST	CHANG	BE INCL. C	ENTIFICATION OF SOIL REMAI OLOR, LOSS OF WASH WATE SEAMS IN ROCK, ETC.	RKS R,		
											1	10,0,0,1	LLEV		6" PROCESS GRAVEL		
		-	┼-	+-	+			-				1	1	drk brn gry F-M	I SAND, sm silt, lit F gravel, tr roots (fill	n	
		\vdash	╫	+	+	\dashv	_	-	+		-	-			, , , , , , , , , , , , , , , , , , , ,	,	
5												1					
		1	SS	24	1 2	1"	7'0"	7	4			moist		drk gry F-M SA	ND, sm silt, tr F gravel, roots		
			-	+-	+	\dashv			9	+	-	compact	5'6"				
												1		gry bm SILT, lit	to sm VF sand		
10	2 2 24 45 400										10'0"	bm SILT & VFF	sand				
ŀ		2 ss 24" 15" 12'0"		6 5	6	+		moist-wet compact	1	bm FM SAND	***						
Ī											_	Compact		brn FM SAND, t	T SIII		
5																	
ᆘ		3	SS	s 24" 20" 17'0"			3	4	-			•	gry VF SAND, sm slif, tr F gravel				
t			- 00			1	170	5	5			wet compact		gry VF SAND, s	m slit, tr F gravel		
						4						33.11,					
ŀ				 	╄	+											
		4	\$S	24"	20	•	22'0"	1	2			wet		brn FM SAND	8		
					\Box	Ţ		5	2			loose			sand, lit F gravei, tr silt		
F		-			╀	+						ļ			and a second a second		
-	-	_			\vdash	+	\rightarrow					1					
		5	SS	24"	18	11	27'0"	17	15			wet		bm FMC SAND	sm F gravel, tr sitt		
L			_		<u> </u>	+		15	51			dense			an i graval, ti siit		
┝		\dashv			-	+						-	28'0"				
L		1	cr	60°	58'	1	34'0"	RC	D = 2°	%	1.5		29'0"	partially weathere Auger refusal	d BEDROCK		
L		\dashv	_		_	L					2.0						
H		+	\dashv			╀					2.0			BEDROCK			
卜	_	+				۲					2.0		34'0"			Í	
		\Box											010	E.C	D.B. 34'0"	\dashv	
H		+	+	-		╀		\rightarrow				- 1					
H		十	+			+	-	-			-			Set well at 20'	•		
		工															
				1167		L										- 1	
T	cond	itioi	is at	t sp	ecif	ic i	ealed locations	ons a	nd m	ay not	tion repres	epresent sent					
	ND SURF	ACE	TO_			FT.	USE	D			CASING	THEN	C/	SING TO	_FT. HOLE NO. B-15	_	
	JGER UP = WEIGHT									NWALL	V DODG		\dashv				
= S	PLIT TUB	E SA	MPL	ER		H.8	S.A. = H	HOLLOW STEM AUGER							COARSE		
					E = () - 1	0% LIT	ITLE =	E = 10 - 20% SOME = 20 - 35% AND = 35 - 50% F = FINE								

Telefax (203) 264-3414

Powdered Bentonite



SOILTESTING, INC.

8

	50 DONOVAN ROAD - OXFORD, CONN. 06478-102
GEOTECHNICAL / ENVIRONMENTAL S	SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling
/- \	UNDERPINNING - HELICAL PILES - SOIL NAILS
Monitor Well # B-15	CLIENT: Saber Real Estate Advisors JOB #: G37-9648-14
Top of Casing Elevation0'	Stick Up Vented Locking Steel Cap YesX_ No
Ground Surface Elevation 0'	Drive/over w/Bolting Cover X Yes No Mounded Backfill Yes X No
	12" Concrete Collar X Yes No Backfill Material Formation
Borehole Diameter 8"	Type of Casing Screen 2" SCH 40 PVC I.D. 2.067" o.p. 2.375" Joint Type thd'd F.J. Impermeable Backfill Bentonite Chips Backfill Material
Well Point Elevation 20'0" 10'0"	#1 Silica Sand Screen Packing #1 Silica Sand Filter Fabric Yes X No If yes, Type
Bottom of Boring Elevation 33'6" 13'6"	Screen Slot Size .010 Backfill Material #1 Sand
	Refusal X Yes No
Screen 10' Riser 10' Plug 1 Slip Cap Silica Sand 700#	Bentonite Pellets Bentonite Chips 1/2 bag Lock Concrete Mix 1 bag D/O 1 Portland Cement S/U

	S	OIL.					C.	CL	IENT:	Sa	ber R	SHEET 1 OF 1						
		90 (OXF			AN R		,					<u> </u>			HOLE NO.	B-16		
		CT (-			•		OJECT		G3	7-9648-14						
		NY						PR	OJECI	NAME	Ker	/stone Sq	ware Di		BORING LOCATIONS			
F	OREMAN	V - DF						LO	CATION		(70	per Pian						
L	TP/c					_					Wh							
IIN	SPECTO)R										OFFSET						
GE	ROUND	WATI	=D A	DOE	D\/AT	ION	10	=	TYF			HSA	SS		DATE START	3/24/14		
	18_FT						15			E I.D. MMER W	~	4 1/4"	1 3/8		DATE FINISH	3/25/14		
	_FT /									MER F			140; 30"		SURFACE ELEV.			
	T	T		S/	MP	LE							30	dia	GROUND WATER ELEV.			
			T	T	T	-		٦,	OWO D	FD 6 141	CORE	DENSITY	STRAT	A FIELD IDE	ATICIOATION OF COM			
DEPTH	CASING		1		EN R	יבי	1	C	OVVS P	ER 6 IN	TIME	OR	CHANG	1	ENTIFICATION OF SOIL REMAR! OLOR, LOSS OF WASH WATER SEAMS IN ROCK, ETC.			
	PER		ין''	pell			DEPT	_ (FO	RCE O	N TUBE	PER	CONSIST	DEPTI	1				
	FOOT	_	\perp	\bot	\bot		@ ВОТ	10-6	6 - 12	12- 18	FT (MIN)	MOIST	ELEV	1				
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ı		<u> </u>	1.50	' 	' 	+	70	16	18	+	 	dry dense	1	bm FM SAND, a	sm F gravel, lit brick (fill)			
												delise		Cobbies, boulde	#8 / - 1U'			
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ľ		2	SS	24	" 18	5"	12'0"	15				dry		h 5140 0 41				
						\bot		23				dense		DIT GRY FMC SAI	ND, sm brick, concrete, lit F g	jravel (fili)		
}		-		+	+	+								1				
5		_		+	+	+			-	-			4 5101	1				
Г	3 88		24	20	n	17'0"	16	9			dry	15'0"	brn FM SAND, lit	E ground to all				
ŀ				-	-	Ţ		10	10 13 compact						r gravei, tr slit			
H				+-	+	+								ļ				
		4	88	10"	8"	2	20'10"	11	60/4"			wet v dense		brn F-M SAND, iii	t all the E amount			
H				_		Ŧ	-						21'0"	DITT -W SAME, III	rait, ir ir gravei			
H				-	┼	+						ſ	22'6"	partially decompos	sed BEDROCK			
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20	RTIONS	USE	D: Ti	-11 RACE	1 0 = E	- 10	.a. = 11)% LIT	OLLUV TLE ≃	v 51EN 10 - 20	7 AUGEI % SOM	≺ IE = 20.	- 35% AND	=35 500	M=16 6 F=F	MEDIUM			
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\lceil	SOILTESTING, INC. 90 DONOVAN RD.								IENT:	Sa	ber Re	SHEET_1_C	DF_1					
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			203	•			,	-	OJECT	NO.	G3/	7-9648-14						
			914)					PR	OJECI	NAME	Key	stone Sq	waro Dh	one II	BORING LOCATIONS			
F	OREMAN	- DF	RILLE	R				LO	CATIO	V	(70	- 96 Wes	tchester	' Avenue)	per Plan			
L	TP/cl	_									Whi							
IN	SPECTO	R										OFFSET						
L								4	TY			DATE START	3/20/14					
	ROUND \ [_13_FT						IS			E I.D.		4 1/4"	1 3/8		DATE FINISH	3/20/14		
	FT <i>A</i>					10				MMER V MMER F			140		SURFACE ELEV.			
		7			_	PLE		+	7//	AUNIEK L	ALL		30"	dia	GROUND WATER ELEV.			
			\top	- S/-	VIVII	LE	T -	4				DENINE						
DEPTH	CASING BLOWS PER FOOT	LOWS NO Type PEN REC ER DEPTH		(FO	N SAM	PER 6 IN MPLER IN TUBE 2 12- 18	PER FT	CONSIST	CHANG DEPTH	INCL. C	ENTIFICATION OF SOIL OLOR, LOSS OF WASH SEAMS IN ROCK, ETC	WATER.						
_	1.001	+	+	+	\dashv	-	@ BC	'			(MIN)	MOIST	ELEV		M PROPERTY.			
				+									bm FM SAND,	6" PROCESS GRAEL sm F gravel, lit cobbles, tr siit				
5			土		士			+	+-	+		1	1					
		1	SS	3 2	4"	15"	7'0"	3	7			l moist		bm FM SAND,	lit silt, lit F gravel			
		⊬	+	╫	\dashv			18	14		-	compact	6'6"	gry bm FMC SAND, sm F gravel, lit slit (fill)				
			+	+	+			-	+-	-		1		brn FM SAND,	lit F gravel, lit silt, tr C sand			
10								 	1									
F		2	SS	2	4"	18"	12'0"	7	9			l moist		bm F-M SAND,	lit C sand, tr F gravel			
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5Ľ					土								25'0"					
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		-	\neg		+	+						- 1		lit cobbles at 34'				
		7	SS	24"	20	0" :	37'0"	3	7			wet	36'0"	gry bm FMC SAN	n			
				100				26	16			dense		gry FMC SAND, s		_		
\vdash		8	SS	1"	0	"	40'1"	60/1"				v dense		No recovery				
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R:	= WEIGH	T OF	ROD	os		W	OH = W	/EIGH1	OFH	AMMER	& RODS	- vane ie	રું!	C =	COARSE			
= S	PLIT TUI	BE S	AMPL	ER	`-	H.:	S.A. = 1	HOLLO	W STI	EM AUG	FR			4.4	: MEDIUM			
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F	DREMAN	- DR	LLEF	₹			LOC	ATION		(70	- 96 West	chester	Avenue)	per Plan			
L	TP/cN	-								Whi	te Plains,	New Yo	rk				
IN	SPECTO	₹						CASING SAMPLER CO						OFFSET			
_							4	TYPI	E		HSA	SS		DATE START	3/24/14		
	ROUND W					NS	Ĭ	SIZE			4 1/4"	1 3/8'		DATE FINISH	3/24/14		
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		1 1 2	<u>`</u> ''					HAM	MER FA	<u>\LL</u>		30"		GROUND WATER ELEV.			
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DEPTH	CASING BLOWS PER	OWS NO Type PEN REC			(FOF	OWS PE N SAMF RCE ON	PLER I TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	CHANG	El INCL. C	ENTIFICATION OF SOIL OLOR, LOSS OF WASH SEAMS IN ROCK, ETC.	WATER.				
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					11	12			dry	10'6"	bm F SAND						
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l													bm F-M SAND,	iit F gravel, siit			
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	= WEIGHT PLIT TUB				V	/OH = W .S.A. =	EIGHT	OF HAN	MMER 8	RODS	COARSE						
OPO	ORTIONS	USE	D: T	RACE	-0=	10% LI	TTLE =	10 - 20'	" YUGE	r: /F = 2∩	- 35% AND)-25 500	M=	MEDIUM			

	SOILTESTING, INC. 90 DONOVAN RD.							CLIE	NT:	Sab	er Re	SHEET 1 0	F_1			
			RD,					220	· ·	10	007	0040 44			HOLE NO.	PW-1
			(03)						JECT N		637	-9648-14				
		•	14)					PRO	JECT N	IAME	Kove	BORING LOCATIONS				
F	DREMAN							LOCA	ATION		(70 ·	stone Squ 96 Westo	heeter A	Re II	per Plan	
	TP/cN										Whit	te Plains,	(
ĪN	SPECTO	₹							-			OFFSET				
L									TYP	Ξ		HSA	SS		DATE START	3/28/14
	ROUND W					ONS		1	SIZE	I.D.		9 1/4"	1 3/8"		DATE FINISH	3/28/14
	<u>13</u> FT									MER W			140#	BIT	SURFACE ELEV.	
A	FT_A	H						HAM	MER FA	TT.		30"		GROUND WATER ELEV.	_	
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DEPTH	CASING BLOWS PER		Тур	e PE	N RE	C DEF	РΤН	(FOR	I SAME	TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDE INCL. C	ENTIFICATION OF SOIL DLOR, LOSS OF WASH SEAMS IN ROCK, ETC.	WATER.
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υĽ							I						lr	stalled 6" PVC		1
OT	cond	litio	ns at	t spe	∍cifi	c loc	atic	by thi	nd ma	estiga vy not	tion repres	epresent sent				
રા૦	JND SURI	ACE	TO		F	T.	USE		1145.	C	ASING	THEN	CASI	NG TO	_FT. HOLE NO.	PW-1
= Al	AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST															
	≈ WEIGH SPLIT TUE									MMER (M AUGE		5			COARSE	. [
												- 35% ANI	7-25 FDW	[V] =	MEDIUM	[

Phone (203) 262-9328

Telefax (203) 264-3414

Powdered Bentonite



SOILTESTING, INC. 90 DONOVAN ROAD - OXFORD, CONN. 06478-1028

GEOTECHNICAL / ENV	RONMENTAL S	UBSURFACE I	nvesti	GATIONS - Te	st Borings - (Core D	rillin	ıg	
	\ . W	Ionitoring Well	ls - Reco	very Wells - Di	rect Push/Pro	be Sar	nplin	Lg.	
× /-	1			- HELICAL PI			-	130	
, ,	•								
8.0 10 18.7 H 30	DIM 4			LIENT: Saber		Adviso	rs		
Monitor Well #	PW-1		,	JOB #: G37-9	648-14				
				Stick Up Vented Los	sking Stool Can	V	Y	Ma	
Top of Casing Elevation	0'		/	y odok op vented Lot	mig steel Cap	tes	_	NO	
		1		Drive/over w/Bolting	Cover	Yes		NI-	
				Divisional windowing	7	Y Tes		MO	
	01	J 11		Mounded Backfill		Yes	Y	AI.	
Ground Surface Elevation		_0'		, mounded backing	-	165		NO	
		1801915	14 B		_				
	Ī				_				
		12"		Concrete Collar	_X	Yes		No	
	1 -		-	Backfill Material					
	_10'0"	-1 111		Formation					
		<u>+</u>		Type of Casing Scre	en s				
		1 1		6" SCH 40 P					
				0.5	O.D. 6.75"				
Borehole Diameter	14"		4		hd'd F.J.				
	1			Joint Type	and d 1 .o.				
7/2	1'0"		<i>///</i>	Impermeable Backfi					
		- 6		Bentonite Ch	ips				
			<i>[22]</i>					_	
	1			Backfill Material					
	2'0"			#1 Silica San	<u>a</u>				
				Screen Packing					
	1			#1 Silica San	ď			_	
Well Point Elevation	38'0" 25'0"			Filter Fabric		Yes	X		
	7	-		If yes, Type		162		10	
_	†			" Joos type					
	38'0" 0'			Screen Slot Size	.010				
Bottom of Boring Elevation	300	- [_					
	A		- X-	Backfill Material					
				Refusal	Y	V			
				1/6/03dl		Yes _	N	0	
Screen 25'		Bankanie in it i		_					
101		Bentonite Pellets	4	Lock	ing Exp. Plug $\underline{1}$				
		Bentonite Chips	1 bag						
Plug 1		Concrete Mix	1 bag D/O 1						
Slip Cap	0.11	Portland Cement	½ bag		S/U				
Silica Sand <u>1</u> 80	U#								