

Appendix I

Stormwater Pollution Prevention Plan



PRELIMINARY STORMWATER POLLUTION PREVENTION PLAN

ELMWOOD PRESERVE

**850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK**

*Applicant/Operator/
Owner:* **Ridgewood Elmwood Owner, LLC**
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Prepared by:



JMC Project 17099

Date: 04/02/2020
Last Revised: 08/01/2023

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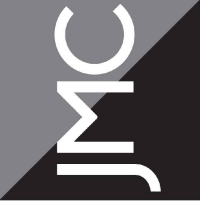
REFERENCED DRAWINGS FOR SWPPP DESIGN AND DETAILS

JMC SITE PLANS

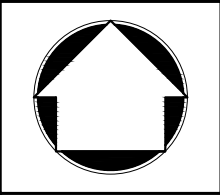
<u>Dwg. No.</u>	<u>Title</u>	<u>Latest Rev. Date</u>
C-000	“Cover Sheet”	8/07/2023
C-010	“Existing Conditions Map – Area 1”	8/07/2023
C-011	“Existing Conditions Map – Area 2”	8/07/2023
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C-013	“Existing Conditions Map – Area 4”	8/07/2023
C-020	“Preliminary Site Demolition Plan – Area 1”	8/07/2023
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C-030	“Existing Steep Slopes Disturbance Map”	8/07/2023
C-040	“Overall Tree Plan”	8/07/2023
C-041	“Tree Removal Plan – Area 1”	8/07/2023
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C-043	“Tree Removal Plan – Area 3”	8/07/2023
C-044	“Tree Removal Plan – Area 4”	8/07/2023
C-100	“Conservation District Subdivision Plan”	8/07/2023
C-101	“Preliminary Layout Plan – Area 1”	8/07/2023
C-102	“Preliminary Layout Plan – Area 2”	8/07/2023
C-103	“Preliminary Layout Plan – Area 3”	8/07/2023
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C-200	“Preliminary Grading Plan – Area 1”	8/07/2023
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C-310	“Water Main Profiles”	8/07/2023
C-320	“Sanitary Sewer Profiles”	8/07/2023
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C-402	“Erosion and Sediment Control Plan (Phase I C)”	8/07/2023
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C-900	“Construction Details”	8/07/2023
C-901	“Construction Details”	8/07/2023
C-902	“Construction Details”	8/07/2023

C-903	“Construction Details”	8/07/2023
C-904	“Construction Details”	8/07/2023
C-905	“Construction Details”	8/07/2023
C-906	“Construction Details”	8/07/2023
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C-908	“Construction Details”	8/07/2023
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L-104	“Typical Unit Landscape Plan”	8/07/2023





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SITE LOCATION MAP

ELMWOOD PRESERVE

850 DOBBS FERRY ROAD TOWN OF GREENBURGH, NEW YORK

JMC PROJECT No: 17099

DATE: 04/02/2020

FIGURE No: SLM-1

SCALE: 1" = 400'

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I. INTRODUCTION

This Stormwater Pollution Prevention Plan has been prepared for the 106.8-acre Elmwood Preserve Site, located in the Town of Greenburgh, Westchester County, New York (hereinafter referred to as the "Site"). The Site is bordered by residences to the north, Game on Golf Center to the south, Rum Brook Park to the east, and residences to the west. The development has been designed in accordance with the following:

- Requirements of the New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-20-001, effective January 29, 2020.
- Chapter 248 "Stormwater Management" of the Town of Greenburgh Code.
- New York State Stormwater Management Design Manual, last revised January 2015.

Ridgewood/Elmwood Owner, LLC (the "Applicant" and Owner) is proposing the redevelopment and repurposing of the 106.8-acre, former Elmwood Country Club, located at 850 Dobbs Ferry Road, Greenburgh, New York. This project has been under SEQRA review by the Town and other reviewing agencies for over five years. The Town Board as part of their SEQRA review determined that the single-family development alternative is the preferred use of the property (not the previously analyzed multi-family or age-restricted developments). Based on the Town Board's direction and input from involved agencies including the Greenburgh Planning Board, Conservation Advisory Council, County Planning Board and the public, the Applicant is now proposing a 113-lot single-family Conservation District Overlay development on the property which is consistent with the underlying residential zoning and does not increase unit density. The single-family development continues to propose the same access connections to NY 100B as previously proposed which is a one full movement access aligned opposite The Chelsea at Greenburgh driveway as well as a right-in/right-out access opposite Westchester View Lane. The two existing driveways adjacent to Worthington Road will be removed as part of the proposed development. There is no residential development or curb cuts proposed on the 14.2 acre parcel east of the Consolidated Edison property. The development continues to propose sidewalks along the property's frontage along NY 100B as well as the installation of a NY 100B eastbound left turn lane at the proposed full movement access. The proposed accesses

and roadways internal to the subject property are proposed to be public roads to be maintained by the Town of Greenburgh once they are constructed.

The applicant is proposing to subdivide the property into 125 total lots, with 113 of these lots to be designated as residential lots with proposed residences and 12 of these lots to be designated as conservation lots.

II. STORMWATER MANAGEMENT PLANNING

In order to be eligible for coverage under the NYSDEC SPDES General Permit No. GP-0-20-001 for Stormwater Discharges from Construction Activities, the Stormwater Pollution Prevention Plan (SWPPP) includes stormwater management practices (SMP's) from the publication "New York State Stormwater Management Design Manual," last revised January 2015.

A Stormwater Pollution Prevention Plan has been prepared for this project because it is a construction activity that involves:

- Soil disturbances of one (1) or more acres of land. A total of approximately 86 acres of land will be disturbed.

The proposed stormwater facilities have been designed such that the quantity and quality of stormwater runoff during and after construction are not adversely altered when compared to pre-development conditions.

A pond/drainage feature and a watercourse comprise the wetlands and watercourses on the site. The pond and watercourse are defined by topography and are depicted on site surveys and plans. The pond is located near the southern boundary of the property to the east of the existing maintenance building and to the north of Dobbs Ferry Road. An overgrown maintenance access driveway is present between the pond and Dobbs Ferry Road. The pond discharges intermittently (when the water level in the pond is high) through 2-18" pipes to a drainage

feature adjacent to Dobbs Ferry Road. The watercourse is located in the north-central portion of the site; it traverses the northern half of the Site in a north-northeasterly direction and then exits the site at the northeastern corner of the Site. Flow in the watercourse is intermittent; the watercourse flows to the Rum Brook, a Class C stream located off site.

The Six Step Process for Stormwater Site Planning and Practice Selection

Stormwater management using green infrastructure is summarized in the six-step process described below. The six-step process was adhered to when developing this SWPPP.

Information is provided in this SWPPP which documents compliance with the required process as follows:

Step 1: Site Planning

Implement planning practices that protect natural resources and utilize the hydrology of the site. Strong consideration must be given to reducing impervious cover to aid in the preservation of natural resources including protecting natural areas, avoiding sensitive areas and minimizing grading and soil disturbance.

Step 2: Determine Water Quality Treatment Volume (WQv)

Determine the required WQv for the site based on the site layout, impervious areas and sub-catchment areas. This initial calculation of WQv will have to be revised after green infrastructure techniques are applied. The following method has been used to calculate the WQv.

- **90% Rule** - According to the New York State Stormwater Design Manual, Section 4.1, the water quality volume is determined from the 90% rule. The method is based on 90% of the average annual stormwater runoff volume which must be provided due to impervious surfaces. The Water Quality Volume (denoted as the WQv) is designed to improve water quality sizing to capture and treat 90% of the average annual

stormwater runoff volume. The WQv is directly related to the amount of impervious cover created at a site. The average rainfall storm depth for 90% of storms in New York State in one year is used to calculate a volume of runoff. The rainfall depth depends on the location of the site within the state. From this depth of rainfall, the required water quality volume is calculated.

The project is a combination of new development and redevelopment and therefore will also comply with the strategies outlined within Chapter 9: Redevelopment Projects of the Design Manual. There are different options to control water quality depending on the redevelopment.

The NYSDEC Redevelopment Standards include specific criteria for the implementation of surface water quality improvements. A combination of standard and non-standard practices is proposed, and all facilities will treat the required water quality volume from the entire contributing area. Therefore, Water Quality Treatment Options II & III will be utilized. According to Option III of the Redevelopment Standards, alternative or non-standard practices such as manufactured treatment devices are acceptable if they treat 75% of the water quality volume from the disturbed areas as well as any additional runoff directed to the practice. According to Option II, standard practices such as infiltration basins can be sized to treat the water quality volume generated from 25% of the existing impervious area plus 100% of the new impervious area. Green practices such as green roofs and porous pavement can be used towards credit in meeting the water quality volume requirements.

Proposed standard SMP's will effectively treat 100% of the 1-year storm for all existing and new impervious areas and the proposed alternative SMP's will also treat 100% of the 1-year storm for all existing impervious areas which is above and beyond the water quality requirements for Redevelopment Projects.

Step 3: Runoff Reduction Volumes (RRv) by Applying Green Infrastructure Techniques and Standard SMP's

RRv is required for this project since it is a combination of both new development and redevelopment.

Green infrastructure techniques or standard SMP's with RRv capacity can potentially reduce the required WQv by incorporating combinations of green infrastructure techniques and standard SMP's within each drainage area on the site.

Green infrastructure techniques are grouped into two categories:

- Practices resulting in a reduction of contributing area such as preservation/restoration of conservation areas, vegetated channels, etc.
- Practices resulting in a reduction of contributing volume such as green roofs, stormwater planters, rain gardens and infiltration basins.

Apply a combination of green infrastructure techniques and standard SMPs with RRv capacity to provide 100% of the WQv calculated in Step 2. If the RRv calculated in this step is greater than or equal to the WQv in Step 2, the RRv requirement has been met and Step 4 can be skipped. If the RRv provided cannot meet or exceed 100% of the WQv, the project must, at a minimum, reduce a percentage of the runoff from impervious areas to be constructed on the site. The percent reduction is based on the Hydrologic Soil Group(s) (HSG) of the site and is defined as Specific Reduction Factor (S).

The following green infrastructure techniques and practices are provided in the Design Manual and are being proposed for this project:

- **Vegetated Swales**
 - The use of sheet flow into vegetated swales will be implemented throughout the site as pretreatment and primary treatment. The water quality and quantity benefits of the vegetated swales was not included in the stormwater calculations, as to maintain an ultra-conservative design.

- **Standard Practices with RRv Capacity**

- **Infiltration Practices** – Several infiltration systems are proposed to treat and retain runoff from the majority of the site.

The Minimum RRv capacity required must be provided by green infrastructure techniques to verify that the RRv requirement has been met. The RRv that is provided by the green infrastructure techniques can then be subtracted from the Total Required WQv that must be provided by the SMP's.

Step 4: Determine the minimum RRv Required

The minimum RRv is calculated like the WQv. However, it is determined using only the new impervious cover and accounts for the hydrologic soil group present. In no case shall the runoff reduction achieved from the newly constructed impervious area be less than the minimum runoff reduction volume (RRv_{min}).

Step 5: Apply Standard Stormwater Management Practices to Address Remaining Water Quality Volume

Apply the standard SMP's to meet additional water quality volume requirements that cannot be addressed by applying the green infrastructure techniques. The standard SMP's with RRv capacity must be implemented to verify that the RRv requirement has been met.

- **Infiltration Practices** – Several infiltration basins are proposed to treat and retain runoff from the site along with a bioretention basin. These practices are located in areas where groundwater and bedrock elevations are acceptable to provide the required 3-foot separation. According to the New York State Stormwater Design Manual, dated January 2015, 100% of the WQv provided by an Infiltration Practice can be applied towards meeting the RRv criteria.

- o **Wet Extended Detention Ponds (P-3)** - ponds that treat the majority of the water quality volume through extended detention. Each Wet Extended Detention Pond has a water quality structure for pretreatment, an aquatic bench, a 12-foot-wide maintenance access path, 4 foot horizontal to 1-foot vertical side slopes for added safety and ease for maintenance and outlet control structures to slowly release the water quantity storms to reduce the peak flow rates and for channel protection. For this project, water quality structures were utilized for treatment instead of forebays.

Step 6: Apply Volume and Peak Rate Control Practices to Meet Water Quantity Requirements

The Channel Protection Volume (CPv), Overbank Flood Control (Qp) and Extreme Flood Control (Qf) must be met for the plan to be completed. This is accomplished by using practices such as infiltration basins, wet detention basins, etc. to meet water quantity requirements. The following standards must be met:

I. Stream Channel Protection (CPv)

Stream Channel Protection Volume Requirements (CPv) are designed to protect stream channels from erosion. In New York State this goal is accomplished by providing 24-hour extended detention of the one-year, 24-hour storm event, remained from runoff reduction. Reduction of runoff for meeting stream channel protection objectives, where site conditions allow, is encouraged and the volume reduction achieved through green infrastructure can be deducted from CPv. Trout waters may be exempted from the 24-hour ED requirement, with only 12 hours of extended detention required to meet this criterion. Detention time may be calculated using either a center of mass method or plug flow calculation method.

- CPv is not required because reduction of the entire CPv volume is achieved on site through green infrastructure and infiltration systems.

2. Overbank Flood (Q_p) which is the 10-year storm.

Overbank control requires storage to attenuate the post development 10-year, 24-hour peak discharge rate (Q_p) to predevelopment rates.

The overbank flood control requirement (Q_p) does not apply in certain conditions, including:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- A downstream analysis reveals that overbank control is not needed.

3. Extreme Storm (Q_f) which is the 100-year storm.

100 Year Control requires storage to attenuate the post development 100-year, 24-hour peak discharge rate (Q_f) to predevelopment rates.

The 100-year storm control requirement can be waived if:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- Development is prohibited within the ultimate 100-year floodplain
- A downstream analysis reveals that 100-year control is not needed.
- If redevelopment results in no increase in impervious area or changes to hydrology that increase the discharge rate from the Site the hundred-year criteria does not apply.

Based on the foregoing, this project is eligible for coverage under NYSDEC SPDES General Permit No. GP-0-20-001.

III. STUDY METHODOLOGY

Runoff rates were calculated based upon the standards set forth by the United States Department of Agriculture Natural Resources Conservation Service Technical Release 55, Urban Hydrology for Small Watersheds (TR-55), dated June 1986. The methodology set forth in TR-55 considers a multitude of characteristics for watershed areas including soil types, soil permeability, vegetative cover, time of concentration, topography, rainfall intensity, ponding areas, etc.

The 1, 2, 10, 25 & 100-year storm recurrence intervals were reviewed in the design of the stormwater management facilities (see Appendices A & B Existing/Proposed Hydrologic Calculations).

Anticipated drainage conditions were analyzed taking into account the rate of runoff which will result from the construction of buildings, parking areas and other impervious surfaces associated with the site development.

Base Data and Design Criteria

For the stormwater management analysis, the following base information and methodology were used:

1. The site drainage patterns, and outfall facilities were reviewed by JMC personnel for the purpose of gathering background data and confirming existing mapping of the watershed areas.
2. An Existing Drainage Area Map was developed from the topographical survey. The drainage area map reflects the existing conditions within and around the project area.

3. A Proposed Drainage Area Map was developed from the proposed grading design superimposed over the topographical survey. The drainage area map reflects the proposed conditions within the project area and the existing conditions to remain in the surrounding area.
4. The United States Department of Agriculture (USDA) Web Soil Survey of the site available on its website at <http://websoilsurvey.nrcd.usda.gov>.
5. Soil Survey of Putnam and Westchester Counties, 1994.
6. The United States Department of Agriculture Natural Resources Conservation Service National Engineering Handbook, Section 4 - Hydrology", dated March 1985.
7. The United States Department of Agriculture Natural Resources Conservation Service Technical Report No. 55, Urban Hydrology for Small Watersheds (TR-55), dated June 1986.
8. United States Department of Commerce Weather Bureau Technical Release No. 40 Rainfall Frequency Atlas of the United States.

The time of concentration was calculated using the methods described in Chapter 3 of TR-55, Second Edition, June 1986. Manning's kinematics wave equation was used to determine the travel time of sheet flow. The 2-year 24-hour precipitation amount of 3.4 inches was used in the equation for all storm events. The travel time for shallow concentrated flow was computed using Figure 3-1 and Table 3-1 of TR-55. Manning's Equation was used to determine the travel time for channel reaches.

9. All hydrologic calculations were performed with the Bentley PondPack software package version 10.02.00.01.

10. All hydraulic calculations will be performed with the Civil 3D Storm and Sanitary Analysis, software package version 2022.
11. The New York State Stormwater Management Design Manual, revised January 2015.
12. New York Standards and Specifications for Erosion and Sediment Control, November 2016.
13. The storm flows for the 1, 2, 10, 25 & 100-year recurrence interval storms were analyzed for the total watershed areas. The Type III distribution design storm for a 24-hour duration was used and the mass rainfall for each design storm was taken from the Extreme Precipitation in New York & New England developed by the Natural Resource Conservation Service (NRCS) and the Northeast Regional Climate Center (NRCC) as follows:

24 Hour Rainfall Amounts

Design Storm Recurrence Interval	Inches of Rainfall
1 Year	2.81
2 Year	3.43
10 Year	5.09
25 Year	6.38
100 Year	8.98

IV. EXISTING CONDITIONS

The approximately 106.8-acre Elmwood Country Club has been closed and the Property dormant since the fall of 2017. Since that time, the Property and buildings have been used by the local police and fire department for training purposes. The interiors of many of the buildings have been significantly damaged by these activities, as well as by neglect and exposure to the elements. Many of the buildings have significant infestations of mold and other decay. The golf course has been untended during the past two years, and vegetation and weeds are growing in many places. Most, if not all, of the sand traps or bunkers have sprouted invasive high weeds.

This project has been under SEQRA review by the Town and other reviewing agencies for over five years. The Town Board as part of their SEQRA review determined that the single-family development alternative is the preferred use of the property (not the previously analyzed multi-family or age-restricted developments). Based on the Town Board's direction and input from involved agencies including the Greenburgh Planning Board, Conservation Advisory Council, County Planning Board and the public, the Applicant is now proposing a 113-lot single-family Conservation District Overlay development on the property which is consistent with the underlying residential zoning and does not increase unit density. The single-family development continues to propose the same access connections to NY 100B as previously proposed which is a one full movement access aligned opposite The Chelsea at Greenburgh driveway as well as a right-in/right-out access opposite Westchester View Lane. The two existing driveways adjacent to Worthington Road will be removed as part of the proposed development. There is no residential development or curb cuts proposed on the 14.2 acre parcel east of the Consolidated Edison property. The development continues to propose sidewalks along the property's frontage along NY 100B as well as the installation of a NY 100B eastbound left turn lane at the proposed full movement access. The proposed accesses and roadways internal to the subject property are proposed to be public roads to be maintained by the Town of Greenburgh once they are constructed.

The Site, as many golf courses are, is made up of rolling topography without tremendous grade change. There is an existing pond and intermittent stream on site, which have been designated as Design Points 2 and 3, respectively. This stream outlets into the Rum Brook. Design Point 4 is a relative low point located in the northwest corner of the Site that is the start of a swale that continues to flow north behind the adjacent residences. An existing drain inlet located in the existing parking lot (in the southwest corner of the Site), has been designated as Design Point 1. Design Point 1A is located along Worthington Road.

The following natural features, conservation areas, resource areas and drainage patterns of the project site have been identified and utilized to develop Drawings DA-1A, DA-1B, DA-1C & DA-1D "Existing Drainage Area Map" which is included in Appendix M:

- Wetlands (jurisdictional, wetland of special concern)
- Waterways (major, perennial, intermittent, springs)
- Buffers (stream, wetland, forest, etc.)
- Forest, vegetative cover
- Critical areas
- Topography (contour lines, existing flow paths, steep slopes, etc.)
- Soil (hydrologic soil groups, highly erodible soils, etc.)
- Bedrock, significant geology features

Based on the USDA web soil survey, the majority of on-site soils are well drained and belong to hydrological soil group B while the remaining soils are also well drained but belong to hydrological soil group C. Portions of the Site that were previously developed and to remain after construction contain areas of urban fill. The soil types, boundaries and drainage areas/designations are depicted on the drainage drawings included in Appendix M.

Five separate Design Points (1, 1A, 2, 3 & 4) were identified for comparing peak rates of runoff under existing and proposed conditions. Similarly, five separate drainage areas were identified in existing conditions based on the existing drainage divides at the site. The numbers included in the name of each drainage area correspond to the Design Point they drain towards.

To more accurately model the Site's drainage patterns and runoff rates and volumes, the existing on-site pond was modeled in the computer program used to generate the stormwater model and design. The environmental consultant on the project, has indicated that a nearby swale may be somewhat supported by groundwater, indicating the pond may also be somewhat supported by groundwater. Because of the unknown variables associated with this pond, that include but are not limited to off-site drainage entering the pond and the fact that the pond may be somewhat supported by groundwater, it is JMC's opinion that an attempt to model this pond and the outlet culvert would be unrealistic. Despite this fact, using best engineering practices, JMC will make assumptions that will be coordinated with the town and reviewing engineer to allow this pond to be modeled in this document. A constant water surface elevation was determined from the surveyed contours and spot grades to be 319.17. These contours were

used to determine the surface area at this elevation and height of the pond above this elevation. If and when the pond overflows, it would first be outletted through the 2-18" pipes located in the southeastern corner of the pond and during the larger storms, when these pipes would not be able to handle the amount of water, water overflows from two separate areas over the banks of the berm separating the pond and the drainage feature adjacent to Dobbs Ferry Road. These were also both modeled.

The following is a description of each of the drainage areas analyzed in the existing conditions analysis:

Existing Drainage Area I (EDA-I) is 5.873 acres in size and is in the southwestern portion of the site along Dobbs Ferry Road and Worthington Road. This area is made up of the existing driveway, a portion of the existing tennis courts, associated parking lot and associated structures that are all proposed to be demolished. It also consists of existing walkways and grass & landscaped areas that are also to be demolished. The auxiliary lot just to the east of the main driveway along with the pool and all pool appurtenances are to be demolished. This drainage area drains south towards an existing inlet, designated as Design Point I, located near the southwestern corner of the Site. Runoff from this area is then conveyed through existing pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of this stream was taken from Westchester County GIS information. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 81 and 5.00 minutes, respectively. Refer to Drawing DA-1C in Appendix M.

Existing Drainage Area IA (EDA-IA) is 2.924 acres in size and is in the southwestern portion of the site along Dobbs Ferry Road and Worthington Road. This area is made up of the remainder of the existing tennis courts and existing walkways and grass & landscaped areas that are also to be demolished. A large portion of this drainage area is to be disturbed during the proposed construction. This drainage area drains west towards a portion of Worthington Road, designated as Design Point IA, located just north of the southwestern corner of the Site. Runoff from this area is conveyed through existing pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of

this stream was taken from Westchester County GIS information. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 76 and 5.00 minutes, respectively. Refer to Drawing DA-1C in Appendix M.

Existing Drainage Area 2 (EDA-2) is 35.989 acres in size and is in the south-central portion of the site along Dobbs Ferry Road. This area is made up of most of the existing golf course and pool area and also contains the majority of the proposed development, including a large portion of the residences and loop road. Runoff from this drainage area drains south overland and through inlets and pipes towards the existing pond, designated as Design Point 2, located in the south-central portion of the Site. It was understood by the environmental engineering consultant that this pond is fed not only from stormwater runoff but may also be fed from groundwater. This pond outlets through 2-18" HDPE pipes, into an existing drainage feature, into a 24" reinforced concrete pipe running under Dobbs Ferry Road, into infrastructure running along Dobbs Ferry Road and eventually outlets to the Sprain Brook/Bronx River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 67 and 24.06 minutes, respectively. Refer to Drawing DA-1A, C & D in Appendix M.

Existing Drainage Area 3 (EDA-3) is 35.523 acres in size and is in the northeastern portion of the site abutting residences to the north along Parkview Road and the East Rum Brook Park to the east. This area is made up of a significant portion of the existing golf course and some wooded areas and contains a large portion of the proposed development, including a large portion of the residences and loop road. It also consists of existing walkways and grass & landscaped areas with an existing stream bisecting this drainage area. This drainage area drains into this stream and the location where this stream exits the Site is designated as Design Point 3. This stream outlets into the Rum Brook. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 66 and 23.52 minutes, respectively. Refer to Drawing DA-1A, B, C & D in Appendix M.

Existing Drainage Area 4 (EDA-4) is 13.757 acres in size and is in the northwestern portion of the site abutting residences along Country Club Drive, Overhill Road, Valleyview Road and Parkview Road. This area is made up of a small portion of the existing golf course and some wooded areas and contains a small amount of the proposed development. This drainage area

drains to a low point located in the northwest corner of the Site designated as Design Point 4. There are four drainage pipes that empty into this low point under current conditions. The pipes consist of 2-4" HDPE pipes, a 6" HDPE pipe and a 10" HDPE pipe. All four of these pipes collect runoff from the Project Site and convey runoff to this low point. When originally constructed, this area may have been designed as an infiltration pond but with the inability to quantify how this was originally designed, or how it has changed since then because of siltation, etc., this area was modeled as simply a detention pond with no infiltration. Designing this pond as a detention pond also allowed for a more conservative design. From this relative low point of the Site, runoff continues to flow to the existing swale in the rear of the neighboring property. This swale empties into a stream that runs under 9A and outlets into the Saw Mill River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 63 and 12.12 minutes, respectively. Refer to Drawing DA-1A in Appendix M.

The peak rates of runoff to the design points of each of the drainage areas for each analyzed storm are shown in the table below:

Table 1
Summary of Peak Rates of Runoff under Existing Conditions
(Cubic Feet per Second)

Storm Recurrence Interval	DP-1	DP-1A	DP-2	DP-3	DP-4
1 year	7.12	2.63	1.40	8.74	2.71
2 year	10.10	3.97	3.25	16.71	6.01
10 year	18.52	7.93	19.97	44.05	18.06
25 year	25.24	11.20	51.48	68.84	29.19
100 year	38.83	17.94	117.62	123.29	54.08

The total runoff volumes to the design point of each of the drainage areas for each analyzed storm are shown in the table on the next page:

Table 2
Summary of Runoff Volumes under Existing Conditions
(Cubic Feet)

Storm Recurrence Interval	DP-1	DP-1A	DP-2	DP-3	DP-4
1 year	24,923	9,437	57,877	58,924	17,787
2 year	35,216	13,953	98,903	98,325	31,247
10 year	65,336	27,698	235,910	230,673	78,195
25 year	90,206	39,377	360,222	351,345	122,113
100 year	142,231	64,288	637,554	621,860	222,373

V. PROPOSED CONDITIONS

Ridgewood/Elmwood Owner, LLC (the "Applicant" and Owner) is proposing the redevelopment and repurposing of the 106.8-acre, former Elmwood Country Club, located at 850 Dobbs Ferry Road, Greenburgh, New York. This project has been under SEQRA review by the Town and other reviewing agencies for over five years. The Town Board as part of their SEQRA review determined that the single-family development alternative is the preferred use of the property (not the previously analyzed multi-family or age-restricted developments). Based on the Town Board's direction and input from involved agencies including the Greenburgh Planning Board, Conservation Advisory Council, County Planning Board and the public, the Applicant is now proposing a 113-lot single-family Conservation District Overlay development on the property which is consistent with the underlying residential zoning and does not increase unit density. The single-family development continues to propose the same access connections to NY 100B as previously proposed which is a one full movement access aligned opposite The Chelsea at Greenburgh driveway as well as a right-in/right-out access opposite Westchester View Lane. The two existing driveways adjacent to Worthington Road will be removed as part of the proposed development. There is no residential development or curb cuts proposed on the 14.2 acre parcel east of the Consolidated Edison property. The development continues to propose sidewalks along the property's frontage along NY 100B as well as the installation of a NY 100B eastbound left turn lane at the proposed full movement access. The proposed accesses and roadways internal to the subject property are proposed to be public roads to be maintained by the Town of Greenburgh once they are constructed.

The applicant is proposing to subdivide the property into 125 total lots, with 113 of these lots to be designated as residential lots with proposed residences and 12 of these lots to be designated as conservation lots.

The proposed drainage improvements include a variety of stormwater practices, such as vegetated swales and stormwater management ponds with pretreatment. After treatment for water quality and peak rate attenuation, stormwater discharges from the proposed basins will utilize low velocity level-spreaders which will drain to the existing stream and pond, will be piped into the existing pond, will be piped into existing stormwater infrastructure, or will be daylighted as runoff was under existing conditions. The vegetated practices and overland discharges provide multiple opportunities for water quality enhancement and infiltration in addition to the proposed stormwater management basins.

Furthermore, in addition to these practices, the overall design of the project is consistent with the latest trends in Low Impact Development. The combination of stormwater ponds, swales, biofilters and level-spreader discharges provide redundant opportunities to enhance water quality and mitigate stormwater runoff rates from the development areas. The vegetated swales and other overland conveyances of stormwater runoff will result in additional infiltration not considered in the SWPPP's hydrologic model, resulting in a conservative analysis.

This section describes the design and analysis of the proposed conditions used to demonstrate that the SWPPP meets the requirements of the General Permit.

The Six Step Process For Stormwater Site Planning and Practice Selection

Step 1: Site Planning

The following practices and site features were incorporated in the site design:

- Preserving hydrology - Maintaining drainage divides

- Floodplain considerations - The site does not lie within the 100-year flood zone according to the National Flood Insurance Program Flood Insurance Rate Map (FIRM).
- Reduction of impervious surfaces such as:
 - i. Reduced roadway, driveway & sidewalks widths
- Waterways (major, perennial, intermittent, springs) – The location, setback, cross section, etc. of the existing waterway has been maintained.
- Forest, vegetative cover – The maximum amount of forest and vegetative cover has been maintained and/or provided to the extent practicable.
- Critical areas have been preserved.
- Topography (contour lines, existing flow paths, steep slopes, etc.) has been maintained or disturbed to the minimum extent practicable.
- Soil (hydrologic soil groups, highly erodible soils, etc.) – Extensive geotechnical investigations have been performed supplemented by soils information gathered from the USDA Web Soil Survey. All stormwater features have been designed taking into account information gathered from the above-mentioned sources.
- Bedrock, significant geology features have been accounted for.
- Wetlands and buffers – Town-regulated 100-foot wetland buffers are present on site surrounding the pond and the watercourse, including two drainage features located outside of the pond. Direct impacts to these areas would occur from the installation and removal of impervious surfaces and associated clearing and grading. A total of approximately 4.96 acres of wetland buffer disturbance is proposed, with a net decrease in impervious surfaces of approximately 0.23 acres. Proposed impacts to the pond buffer total approximately 4.39 acres for removal of cart paths and a portion of the maintenance building and parking lot, and the installation of a sidewalk along Dobbs Ferry Road; in addition, the edge of the relocated site access road on the east side of the pond extends into the wetland buffer. The sidewalk continues adjacent to Dobbs Ferry Road, extending through the pond wetland buffer and continuing to the west entrance driveway. Currently, this area of wetland buffer is developed as a maintained golf course but does not have a sidewalk. Of the total wetland area, approximately 2,034 sq. ft. (0.05 acres) are associated with 2 drainage features. One feature is the area between the pond discharge

pipes and discharge pipes from Dobbs Ferry Road. This approximately 245 sq. ft. area collects outflow from the pond when the water level is high and appears to collect stormwater runoff from the road. This area appears to be isolated between the pond and Dobbs Ferry Road. The addition of the sidewalk along Dobbs Ferry Road will disturb approximately 18 sf of this area. This sidewalk runs on top of this area so a proposed 24" reinforced concrete pipe will now convey runoff under the walkway and connect into a proposed drainage structure that will connect into the existing 24" reinforced concrete pipe running underneath Dobbs Ferry Road. The second feature is an approximately 1,787 sq. ft. swale that collects runoff from Dobbs Ferry Road; the runoff is isolated between the road and the berm surrounding the pond. The addition of the sidewalk along Dobbs Ferry Road will disturb approximately 73 sf of this area. This disturbance will be mitigated per Town of Greenburgh requirements. Proposed impacts to the watercourse buffer total approximately 0.58 acres for removal of cart paths and outbuildings. No impervious surfaces would be added to, and approximately 0.14 acres would be removed from, the watercourse buffer. Impervious surfaces removed within both wetland buffers would total approximately 0.23 acres. Objects to be removed from the buffers include cart paths, outbuildings, and a portion of the maintenance building and parking lot. Therefore, a net reduction of approximately 0.23 acres of impervious surfaces would occur within the pond and watercourse buffers. The functions of the buffers would be improved by the removal of the majority of the impervious surfaces within the buffers, as would the addition of plantings along the pond edge and changes to the landscaping maintenance in this area (see mitigation section for details). The plantings and natural vegetative regrowth would add some wildlife habitat, and removal of the cart path and other impervious surfaces would improve the infiltration of precipitation and stormwater runoff into the ground.

Step 2: Determine Water Quality Treatment Volume (WQv)

Determine the required WQv for the site based on the site layout, impervious areas and sub-catchments. This initial calculation of WQv will have to be revised after green infrastructure techniques are applied. The following method has been used to calculate the WQv.

- **90% Rule** - According to the New York State Stormwater Design Manual, Section 4.1, the water quality volume is determined from the 90% rule. The method is based on 90% of the average annual stormwater runoff volume which must be provided due to impervious surfaces. The Water Quality Volume (denoted as the WQv) is designed to improve water quality sizing to capture and treat 90% of the average annual stormwater runoff volume. The WQv is directly related to the amount of impervious cover created at a site. The average rainfall storm depth for 90% of storms in New York State in one year is used to calculate a volume of runoff. The rainfall depth depends on the location of the site within the state. From this depth of rainfall, the required water quality volume is calculated.

A portion of the project is a redevelopment and therefore will comply with the strategies outlined within Chapter 9: Redevelopment Projects of the Design Manual. There are different options to control water quality depending on the redevelopment.

The NYSDEC Redevelopment Standards include specific criteria for the implementation of surface water quality improvements. A combination of standard and non-standard practices are proposed and all facilities will treat the required water quality volume from the entire contributing area. Therefore, Water Quality Treatment Options II & III will be utilized. According to Option III of the Redevelopment Standards, alternative or non-standard practices such as manufactured treatment devices are acceptable if they treat 75% of the water quality volume from the disturbed areas as well as any additional runoff directed to the practice. According to Option II, standard practices such as subsurface infiltration systems can be sized to treat the water quality volume generated from 25% of the existing impervious area plus 100% of the new impervious area. Green practices such as green roofs and porous pavement can be used towards credit in meeting the water quality volume requirements.

Proposed standard SMP's will effectively treat 100% of the 1-year storm for all existing and new impervious areas and the proposed alternative SMP's will also treat 100% of the 1-year storm for all existing impervious areas which is above and beyond the water quality requirements for

Redevelopment Projects.

Step 3: Runoff Reduction Volumes (RRv) by Applying Green Infrastructure Techniques and Standard SMP's

RRv is required for this project since it is a combination of both new development and redevelopment.

Green infrastructure techniques or standard SMP's with RRv capacity can potentially reduce the required WQv by incorporating combinations of green infrastructure techniques and standard SMP's within each drainage area on the site.

Green infrastructure techniques are grouped into two categories:

- Practices resulting in a reduction of contributing area such as preservation/restoration of conservation areas, vegetated channels, etc.
- Practices resulting in a reduction of contributing volume such as green roofs, stormwater planters, and rain gardens.

Apply a combination of green infrastructure techniques and standard SMPs with RRv capacity to provide 100% of the WQv calculated in Step 2. If the RRv calculated in this step is greater than or equal to the WQv in Step 2, the RRv requirement has been met and Step 4 can be skipped. If the RRv provided cannot meet or exceed 100% of the WQv, the project must, at a minimum, reduce a percentage of the runoff from impervious areas to be constructed on the site. The percent reduction is based on the Hydrologic Soil Group(s) (HSG) of the site and is defined as Specific Reduction Factor (S).

The following green infrastructure techniques and practices are provided in the Design Manual and are being proposed for this project:

- **Vegetated Swales**

- The use of sheet flow into vegetated swales will be implemented throughout the site as pretreatment and primary treatment. The water quality and quantity benefits of the vegetated swales was not included in the stormwater calculations, as to maintain an even more conservative design.

- **Standard Practices with RRv Capacity**

- **Infiltration Practices** – Several infiltration systems are proposed to treat and retain runoff from the majority of the site.

The Minimum RRv capacity required must be provided by green infrastructure techniques to verify that the RRv requirement has been met. The RRv that is provided by the green infrastructure techniques can then be subtracted from the Total Required WQv that must be provided by the SMP's.

Step 4: Determine the minimum RRv Required

RRv_{min} calculations can be found in Appendix 'C'. RRv_{min} was met through the proposed infiltration basins and a bioretention area.

Step 5: Apply Standard Stormwater Management Practices to Address Remaining Water Quality Volume

- **PONDS**

Wet Extended Detention Pond (P-3)

Description

Pond that treats a portion of the water quality volume by detaining storm flows above a permanent pool for a specified minimum detention time.

- **INFILTRATION SYSTEMS**

- Infiltration Basin (I-2)

- Description

- An infiltration practice that stores the water quality volume in a shallow depression, before it is infiltrated it into the ground.

- **OPEN CHANNELS**

- Dry Swale (O-I)

- Description

- An open drainage channel or depression explicitly designed to detain and promote the filtration of stormwater runoff into the soil media.

- Non Standard/Alternative SMP's to Address Remaining Water Quality Volume (for Redevelopment Projects)

- **Hydrodynamic Separator**

Step 6: Apply Volume and Peak Rate Control Practices to Meet Water Quantity Requirements

The Channel Protection Volume (CPv), Overbank Flood Control (Qp) and Extreme Flood Control (Qf) must be met for the plan to be completed. This is accomplished by using practices such as infiltration basins, wet detention basins, etc. to meet water quantity requirements. The following standards must be met:

1. Stream Channel Protection (CPv)

Stream Channel Protection Volume Requirements (CPv) are designed to protect stream channels from erosion. In New York State this goal is accomplished by providing 24-hour extended detention of the one-year, 24-hour storm event, remained from runoff reduction. Reduction of runoff for meeting stream channel protection objectives, where site conditions allow, is encouraged and the volume reduction achieved through green infrastructure can be deducted from CPv. Trout waters may be exempted from the 24-hour ED requirement, with only 12 hours of extended detention required to meet this criterion. Detention time may be calculated using either a center of mass method or plug flow calculation method.

- CPv is not required because reduction of the entire CPv volume is achieved on site through green infrastructure and infiltration systems.

2. Overbank Flood (Qp) which is the 10-year storm.

Overbank control requires storage to attenuate the post development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates.

The overbank flood control requirement (Qp) does not apply in certain conditions, including:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- A downstream analysis reveals that overbank control is not needed.

3. Extreme Storm (Qf) which is the 100-year storm.

100 Year Control requires storage to attenuate the post development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates.

The 100-year storm control requirement can be waived if:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- Development is prohibited within the ultimate 100-year floodplain
- A downstream analysis reveals that 100-year control is not needed.
- If redevelopment results in no increase in impervious area or changes to hydrology that increase the discharge rate from the Site the hundred-year criteria does not apply.

All practices exceed the required elements of SMP criteria as outlined in Chapter 6 of the NYS Stormwater Management Design Manual. A summary of each category is provided below.

1. Feasibility – Stormwater practices are designed based upon unique physical environmental considerations noted in the NYS Stormwater Management Design Manual (NYSSMDM).
2. Conveyance – The design conveys runoff to the designed stormwater practice in a manner that is safe, minimizes erosion and disruption to natural drainage channel and promotes filtering and infiltration.
3. Pretreatment – All stormwater practices provide pretreatment as required in accordance with NYSSMDM design guidelines.

4. Treatment Geometry – The plan provides water quality treatment in accordance with NYSSMDM guidelines.
5. Environmental/Landscaping – Extensive landscaping has been provided for each proposed stormwater practice to enhance pollutant removal and provide aesthetic enhancement to the property.
6. Maintenance – Maintenance for the environmental practices has been provided within the SWPPP Report as required. Maintenance access is provided in the site plans.

Based on the foregoing, this project is eligible for coverage under NYSDEC SPDES General Permit No. GP-0-20-001.

To determine the post-development rates of runoff generated on-site, the following drainage areas were analyzed in the post-development conditions. These areas are graphically depicted on Drawing DA-2A, DA-2B, DA-2C and DA-2D "Proposed Drainage Area Map" located in Appendix "O".

Five separate Design Points (1, 1A, 2, 3 & 4) were identified when comparing peak rates of runoff and runoff volumes under existing and proposed conditions. Similarly, five separate drainage areas were identified under proposed conditions based on the updated drainage divides at the site. Drainage Area 1, which itself drains overland towards existing drainage infrastructure in the intersection of Dobbs Ferry Road and Worthington Road, has two sub drainage areas (1B & 1C). Sub drainage areas 1B & 1C drain overland into proposed inlets and are conveyed towards Infiltration Basin #1 and the existing drainage infrastructure along Dobbs Ferry Road, respectively. Drainage Area 1A, which itself drains overland towards existing drainage infrastructure in Worthington Road, has no sub drainage areas. Drainage Area 2, which itself drains directly into the existing pond, has three sub drainage areas (2A, 2B & 2C). Sub drainage area 2A drains overland into proposed inlets and is conveyed to Wet Extended Detention Basin #2A, while sub drainage area 2B drains to Infiltration Basin #2 through proposed inlets and

pipng. Sub drainage area 2C includes the portion of Dobbs Ferry Road that drains into the proposed drainage structure CI-K-1. Drainage Area 3, which itself drains directly into the existing watercourse, has two sub drainage areas (3A & 3B). Sub drainage area 3A drains overland into proposed inlets and is conveyed to Infiltration Basin #3. Sub drainage area 3B drains overland into proposed inlets and is conveyed to Infiltration Basin #3A. Drainage Area 4 drains to the low point in the northwest corner of the Site but not before being detained and treated in Infiltration Basin #4.

Since the Elmwood Subdivision application was last discussed by the board, we have had extensive discussions with the Town Engineering Department and Public Works on how to comprehensively address the stormwater management across the entire 92-acre parcel including on the 113 individual lots. Based on these discussions, we agree that the best scenario is to keep the roadway rights-of-way as publicly dedicated streets and utilities with the stormwater management basin areas located on the conservation areas as owned and maintained on land by the future Homeowners Association. To achieve these improvements, each of the 113 residential lots will have certain minimum on-site stormwater management facilities maintained by individual homeowners. In accordance with the Town Code, these components will be designed to detain stormwater from the 25-year storm event on each lot before being conveyed to the public right-of-way and ultimately to the HOA maintained infiltration and detention ponds. Each residence's roof would be collected by gutters and conveyed through roof drain leaders and piped via a 6" HDPE pipe to a 46 foot long, flat 24" HDPE pipe located underneath the driveway. A large portion of the driveway surface will also be conveyed to this 24" pipe via a drain inlet. Runoff would be collected in this 24" pipe and be released slowly via a 4" orifice in a Nyloplast (or similar) outlet control structure into drainage structures in the Town's right-of-way or in a stormwater easement depending on the lot configuration. The detention systems on each lot will be designed to provide reasonable access for maintenance purposes and the overall site water quality requirements will continue to be met with the open infiltration basins located throughout the site.

The following is a description of each of the drainage areas analyzed in the proposed conditions analysis:

Proposed Drainage Area I (PDA-I) is 3.555 acres in size and is in the southwestern portion of the site along Dobbs Ferry Road and Worthington Road. The disturbance in this area is limited to grading and a small portion of the main entrance driveway. Besides the driveway and the proposed sidewalk, no new impervious surfaces are being proposed in this area. Runoff from this drainage area is conveyed towards both Dobbs Ferry Road and Worthington Road and eventually towards an existing drain inlet at the intersection, designated as Design Point I, located in the southwestern corner of the Site. Runoff from this area is then conveyed through existing pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of this stream was taken from Westchester County GIS information. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 67 and 6.00 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area IA (PDA-IA) is 2.144 acres in size and is in the southwestern portion of the site along Worthington Road. The disturbance in this area is limited to grading to accommodate three proposed residences. This drainage area drains west towards a low point along Worthington Road, designated as Design Point IA, located in the southwestern corner of the Site. Runoff from this area is then conveyed through pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of this stream was taken from Westchester County GIS information. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 69 and 6.48 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area IB (PDA-IB) is 9.094 acres in size and is in the central western portion of the site. This area is made up of a significant portion of the development that includes many residences and a large portion of the ring road. Runoff from this drainage area is conveyed through proposed inlets and piping into Infiltration Basin #1 where it will be treated, detained, and slowly released into proposed infrastructure and then conveyed towards a proposed Water Quality Structure where the runoff will again be treated and then conveyed to the existing inlet along Worthington Road, designated as Design Point I, located in the southwestern corner of

the Site. Runoff from this area is then conveyed through pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of this stream was taken from Westchester County GIS information. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 77 and 8.76 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area 1A (PDA-1C) is 4.605 acres in size and is in the southwestern portion of the site along Dobbs Ferry Road and Worthington Road. This area is made up of a significant portion of the development that includes many residences and a large portion of the ring road. Runoff is collected in proposed inlets and is conveyed through proposed pipes into Detention Basin #1C where it will first be treated in a proposed Water Quality Structure. Runoff will be controlled and slowly be released by an outlet control structure that connects into an existing drainage structure. Runoff from this area is then conveyed through pipes and into a stream running behind the residences along Worthington Road that eventually outlets into the Saw Mill River. The approximate location of this stream was taken from Westchester County GIS information and is now shown on the plans. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 78 and 6.12 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area 2 (PDA-2) is 7.776 acres in size and is located along the southern property line of the Site, along Dobbs Ferry Road. This drainage area contains the existing pond, and much of this drainage area is to remain undeveloped and contains a small amount of new impervious area. Runoff from this drainage area flows overland into the existing pond, designated as Design Point 2, located in the south-central portion of the Site. It was understood by the environmental engineering consultant that this pond is fed not only from stormwater runoff but may also be fed from groundwater. This pond outlets through 2-18" HDPE pipes, into an existing drainage feature, into a 24" reinforced concrete pipe running under Dobbs Ferry Road, into infrastructure running along Dobbs Ferry Road and eventually outlets to the Sprain Brook/Bronx River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 69 and 7.56 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area 2A (PDA-2A) is 8.948 acres in size and is in the southeastern portion of the Site and contains a significant portion of the proposed development and new impervious area. Runoff from this drainage area will be conveyed through proposed inlets and piping into Wet Extended Detention Basin #2A and the treated runoff is eventually conveyed into the existing pond, designated as Design Point 2, located in the south-central portion of the Site. It was understood by the environmental engineering consultant that this pond is fed not only from stormwater runoff but may also be fed from groundwater. This pond outlets through 2-18" HDPE pipes, into an existing drainage feature, into a 24" reinforced concrete pipe running under Dobbs Ferry Road, into infrastructure running along Dobbs Ferry Road and eventually outlets to the Sprain Brook/Bronx River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 75 and 9.48 minutes, respectively. Refer to Drawing DA-2D in Appendix M.

Proposed Drainage Area 2B (PDA-2B) is 13.169 acres in size and is located in the southcentral portion of the Site. This area contains a significant amount of development and new impervious area. Runoff from this drainage area is conveyed through proposed inlets and piping into proposed Infiltration Basin #2, where it is treated and the treated runoff that was not infiltrated is eventually conveyed into the existing pond, designated as Design Point 2, located in the southern portion of the Site. It was understood by the environmental engineering consultant that this pond is fed not only from stormwater runoff but may also be fed from groundwater. This pond outlets through 2-18" HDPE pipes, into an existing drainage feature, into a 24" reinforced concrete pipe running under Dobbs Ferry Road, into infrastructure running along Dobbs Ferry Road and eventually outlets to the Sprain Brook/Bronx River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 70 and 13.26 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area 2C (PDA-2C) is 0.298 acres in size and includes a portion of Dobbs Ferry Road. Runoff is conveyed to low point in the road adjacent to an existing drainage feature, where a proposed sewer quality structure with an inlet will collect this runoff and convey it into a 24" reinforced concrete pipe running under Dobbs Ferry Road, into infrastructure running along Dobbs Ferry Road and eventually outlets to the Sprain Brook/Bronx River. The Curve Number

(CN) and Time of Concentration (Tc) for this drainage area are 98 and 5.00 minutes, respectively. Refer to Drawing DA-2C in Appendix M.

Proposed Drainage Area 3 (PDA-3) is 6.824 acres in size and is located along the eastern property of the Site. This area is, for the most part, to remain undisturbed except for a small amount of grading. Runoff from this drainage area flows overland into the existing stream and the location where this stream exits the Coned portion of the subject property is designated as Design Point 3. This stream outlets into the Rum Brook. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 64 and 10.08 minutes, respectively. Refer to Drawing DA-2A, B & D in Appendix M.

Proposed Drainage Area 3A (PDA-3A) is 12.928 acres in size and is located in the northeastern portion of the Site. This area contains a significant amount of development and new impervious area. Runoff from this drainage is conveyed through proposed inlets and piping into Infiltration Basin #3. An outlet control structure then slowly outlets runoff that was not infiltrated towards the existing stream and the location where this stream exits the Coned portion of the subject property is designated as Design Point 3. This stream outlets into the Rum Brook. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 69 and 6.60 minutes, respectively. Refer to Drawing DA-2B & D in Appendix M.

Proposed Drainage Area 3B (PDA-3B) is 11.124 acres in size and is located in the northeastern portion of the Site. This area contains a significant amount of development and new impervious area. Runoff from this drainage is conveyed through proposed inlets and piping into Infiltration Basin #3A. An outlet control structure then slowly outlets runoff that was not infiltrated towards the existing stream and the location where this stream exits the Coned portion of the subject property is designated as Design Point 3. This stream outlets into the Rum Brook. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 80 and 10.20 minutes, respectively. Refer to Drawing DA-2A & B in Appendix M.

Proposed Drainage Area 4 (PDA-4) is 13.601 acres in size and is located in the northwestern corner of the Site. This drainage area contains a significant portion of the proposed development

and new impervious area. Runoff from this drainage area flows overland and is collected by proposed inlets and is conveyed into Infiltration Basin #4 where the overflow is conveyed to a low point located in the northwest corner of the Site, designated as Design Point 4. From this relative low point of the Site, runoff continues to flow to the existing swale in the rear of the neighboring property. As previously described in the EDA-4 section, there were four drainage pipes that emptied into this low point. All four pipes will be removed, and the stormwater model and design will include this change and accommodate the change in drainage patterns in this area due to the pipes being removed. When originally constructed, this area may have been designed as an infiltration pond but with the inability to quantify how this was originally designed, or how it has changed since then because of siltation, etc., this area was modeled as simply a detention pond with no infiltration. Designing this pond as a detention pond also allowed for a more conservative design. From this relative low point of the Site, runoff continues to flow to the existing swale in the rear of the neighboring property. This swale empties into a stream that runs under 9A and outlets into the Saw Mill River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 70 and 6.84 minutes, respectively. Refer to Drawing DA-2A in Appendix M.

The peak rates of runoff to the design point of each of the analyzed drainage area for each analyzed storm are shown in the table below:

Table 3
Summary of Proposed Peak Rates of Runoff under Proposed Conditions
(Cubic Feet per Second)

Storm Recurrence Interval	DP-1	DP-1A	DP-2	DP-3	DP-4
1 year	4.05	1.05	0.70	1.43	0.00
2 year	6.91	1.81	1.23	3.97	0.00
10 year	16.94	4.32	8.60	37.02	0.00
25 year	25.17	6.50	21.54	67.56	10.14
100 year	38.81	11.20	53.99	112.63	52.50

The total runoff volumes to the design points of each of the drainage areas for each analyzed storm are shown in the table below:

Table 4
Summary of Runoff Volumes under Proposed Conditions
(Cubic Feet)

Storm Recurrence Interval	DP-1	DP-1A	DP-2	DP-3	DP-4
1 year	22,807	4,441	13,492	8,825	0
2 year	36,200	7,101	43,171	37,745	0
10 year	90,539	15,747	151,940	155,245	0
25 year	140,964	23,448	251,501	262,505	14,792
100 year	253,133	40,429	474,580	502,974	77,597

The reductions in peak rates of runoff from existing to proposed conditions are shown in the table on the next page.

Table 5
Percent Reductions in Peak Rates of Runoff (Existing vs. Proposed Conditions)
(Cubic Feet per Second)

Design Point	Storm Recurrence Frequency (Years)	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)	Percent Reduction (%)
1	1 year	7.12	4.05	43.1
	2 year	10.10	6.91	31.6
	10 year	18.52	16.94	8.5
	25 year	25.24	25.17	0.3
	100 year	38.83	38.81	0.1
1A	1 year	2.63	1.05	60.1
	2 year	3.97	1.81	54.4
	10 year	7.93	4.32	45.5
	25 year	11.20	6.50	42.0
	100 year	17.94	11.20	37.6
2	1 year	1.40	0.70	50.0
	2 year	3.25	1.23	62.2
	10 year	19.97	8.60	56.9
	25 year	51.48	21.54	58.2
	100 year	117.62	53.99	54.1
3	1 year	8.74	1.43	83.6
	2 year	16.71	3.97	76.2
	10 year	44.05	37.02	16.0
	25 year	68.84	67.56	1.9
	100 year	123.29	112.63	8.6

4	1 year	2.71	0.00	100.0
	2 year	6.01	0.00	100.0
	10 year	18.06	0.00	100.0
	25 year	29.19	10.14	65.3
	100 year	54.08	52.50	2.9

The reductions in runoff volumes from existing to proposed conditions are shown on the table on the next page:

Table 6
Percent Reductions in Runoff Volumes (Existing vs. Proposed Conditions)
(Cubic Feet)

Design Point	Storm Recurrence Frequency (Years)	Existing Volume (cf)	Proposed Volume (cf)	Percent Reduction (%)
1	1 year	24,923	22,807	8.5
	2 year	35,216	36,200	+2.7
	10 year	65,336	90,539	+27.8
	25 year	90,206	140,964	+36.0
	100 year	142,231	253,133	+43.8
1A	1 year	9,437	4,441	52.9
	2 year	13,953	7,101	49.1
	10 year	27,698	15,747	43.1
	25 year	39,377	23,448	40.5
	100 year	64,288	40,429	37.1
2	1 year	57,877	13,492	76.7
	2 year	98,903	43,171	56.4
	10 year	235,910	151,940	35.6
	25 year	360,222	251,501	30.2
	100 year	637,554	474,580	25.6
3	1 year	58,924	8,825	85.0
	2 year	98,325	37,745	61.6
	10 year	230,673	155,245	32.7
	25 year	351,345	262,505	25.3
	100 year	621,860	502,974	19.1
4	1 year	17,787	0	100.0
	2 year	31,247	0	100.0
	10 year	78,195	0	100.0
	25 year	122,113	14,792	87.9
	100 year	222,373	77,597	65.1

As demonstrated in Table 5, the proposed stormwater improvements will result in reductions of peak rates of runoff when compared to existing conditions for all storms and design points analyzed. As demonstrated in Table 6, the proposed stormwater improvements will also result in reductions of stormwater runoff volumes at all design points except in a few design storms (2-year, 10-year, 25-year and 100-year storm events) at Design Point 1, when comparing existing and proposed conditions. It should be noted that there are potential flooding concerns on Dobbs Ferry Road adjacent to this existing pond and these reductions in volume and in peak flow will reduce the potential for flooding in this location. A drainage structure will be constructed along Dobbs Ferry Road, adjacent to the existing pond to further reduce the potential for flooding in this area.

As mentioned above, each of the 113 residential lots will have certain minimum on-site stormwater management facilities maintained by individual homeowners. In accordance with the Town Code, these components will be designed to detain stormwater from the 25-year storm event on each lot before being conveyed to the public right-of-way and ultimately to the HOA maintained infiltration and detention ponds. Each residence's roof would be collected by gutters and conveyed through roof drain leaders and piped via a 6" HDPE pipe to a 46 foot long, flat 24" HDPE pipe located underneath the driveway. A large portion of the driveway surface will also be conveyed to this 24" pipe via a drain inlet. Runoff would be collected in this 24" pipe and be released slowly via a 4" orifice in a Nyloplast (or similar) outlet control structure into drainage structures in the Town's right-of-way or in a stormwater easement depending on the lot configuration.

Calculations for the conditions without this 24" detention pipe and with this 24" detention pipe have been included in Appendix A and Appendix B, respectively. The peak flow to the Town's system without the 24" detention pipe is 0.59 cfs and has been reduced to 0.51 cfs with the 24" detention pipe, about at 15% reduction.

VI. SOIL EROSION & SEDIMENT CONTROL

A potential impact of the proposed development on any soils or slopes will be that of erosion and transport of sediment during construction. An Erosion and Sediment Control Management Program will be established for the proposed development, beginning at the start of construction and continuing throughout its course, as outlined in the "New York State Standards and Specifications for Erosion and Sediment Control," November 2016. A continuing maintenance program will be implemented for the control of sediment transport and erosion control after construction and throughout the useful life of the project.

The Operator shall have a qualified professional conduct an assessment of the site prior to the commencement of construction and certify that the appropriate erosion and sediment controls, as shown on the Erosion & Sediment Control Plans, have been adequately installed to ensure overall preparedness of the Site for the commencement of construction. In addition, the Operator shall have a qualified professional conduct one site inspection at least every seven calendar days and at least two site inspections every seven calendar days when greater than five acres of soil is disturbed at any one time. It should be noted that any disturbance at any given time over 5 acres requires a "5-acre waiver" from the Municipal Separate Storm Sewer System (MS4).

Prior to the commencement of construction activity, the owner or operator must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The owner or operator shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the trained contractor. The owner or operator shall ensure that at least one trained contractor is on site on a daily basis when soil disturbance activities are being performed.

The owner or operator shall have each of the contractors and subcontractors identified above sign a copy of the certification statement provided in Appendix F before they commence any construction activity.

Because of the amount of disturbance that is being proposed, JMC has put together a Construction Phasing and Erosion & Sediment Control Narrative that has been included on the Erosion & Sediment Control Plans and has also been included below:

CONSTRUCTION PHASING AND EROSION & SEDIMENT CONTROL NARRATIVE

NOTES

CONSTRUCTION SHALL BE SEQUENCED IN SUCH A MANNER THAT ANY AREA WHICH IS DISTURBED SHALL FIRST BE PROTECTED WITH SEDIMENT EROSION CONTROLS AS INDICATED ON THIS PLAN.

A WAIVER SHALL BE REQUIRED FROM THE TOWN OF GREENBURGH MS4 OFFICER TO DISTURB MORE THAN 5 ACRES AT ONE TIME DURING THE SUBDIVISION CONSTRUCTION.

ALL LOCAL, STATE AND FEDERAL REGULATIONS SHALL BE FOLLOWED BY THE BLASTING CONTRACTOR SHOULD SITE CONDITIONS REQUIRE BLASTING FOR THE PURPOSES OF ROCK REMOVAL.

MASS GRADING, EARTHWORK AND STABILIZATION OPERATIONS SHALL BE UNDERTAKEN ON A CONTINUOUS AND SIMULTANEOUS BASIS ACROSS EACH PHASE TO ACHIEVE THE SUBDIVISION DESIGN. THE AREAS OF EACH PHASE SHALL BE COORDINATED AND UTILIZED FOR DEPOSITING OF CUTS AND FILLS ON AN ONGOING BASIS AS WORK PROGRESSES.

ANY AREA THAT IS IDLE FOR MORE THAN SEVEN CALENDAR DAYS SHALL BE IMMEDIATELY STABILIZED USING TEMPORARY STABILIZATION METHODS WHICH MAY INCLUDE

MULCHING, HYDROSEEDING OR OTHER APPROVED METHODS AND SURROUNDED BY A SEDIMENT CONTROL MEASURE (SILT FENCE, WATTLE, ETC.).

ROADWAYS SHALL BE CLEANED WITH A STREET SWEEPER DAILY AS NEEDED AS CONSTRUCTION TRAFFIC TRAVERSES ASPHALT ROADS FROM PREVIOUS PHASES.

THESE PLANS TAKE THE FOLLOWING DESIGN ASSUMPTIONS INTO CONSIDERATION:

- PER THE GEOTECHNICAL REPORT PREPARED BY CARLIN SIMPSON & ASSOCIATES, IT IS ASSUMED THAT AN AVERAGE OF 2 FEET OF EXISTING FILL EXISTS SPORADICALLY IN POCKETS AROUND THE SITE. IN AREAS OF BUILDING AND ROADWAY CONSTRUCTION WHERE THIS EXISTING FILL IS ENCOUNTERED, IT SHALL BE TESTED, STRIPED & REMOVED AND UTILIZED IN AN AREA OF THE SITE DEEMED SUITABLE.
- PER THE ROUX ASSOCIATES, INC. BROWNFIELD CLEANUP PROGRAM APPLICATION AND SOILS MANAGEMENT PLAN, APPROXIMATELY 50,000 CUBIC YARDS OF IMPACTED SOILS COULD BE PRESENT ON THE SITE. THIS IS SUBJECT TO FURTHER TESTING BY ROUX ASSOCIATES. IF IMPACTED SOILS ARE ENCOUNTERED, THE TOP 2-3 FEET OF MATERIAL WILL BE FURTHER TESTED AND CHARACTERIZED FOR REUSE AND/OR DISPOSAL. WHEN SOILS ARE CHARACTERIZED THEY WILL BE UTILIZED IN A SUITABLE AREA ON-SITE OR IF NECESSARY, REMOVED FROM THE SITE IN ACCORDANCE WITH ALL STATE AND LOCAL REQUIREMENTS. FINE GRADING REQUIREMENTS WILL BE DETERMINED AFTER SAMPLING BY ROUX ASSOCIATES TO DETERMINE HOW MUCH SOIL MAY NEED TO BE EXPORTED. CLEAN FILL (TOPSOIL) MAY BE ADDED TO THE INDIVIDUAL LOTS FOR LANDSCAPING PURPOSES.

PHASE 1A (APPROXIMATELY 30.31 ACRES)

- I. A PRECONSTRUCTION SITE MEETING SHALL OCCUR WITH REPRESENTATIVES OF THE TOWN OF GREENBURGH, A QUALIFIED INSPECTOR IN SEDIMENT AND EROSION CONTROL, THE APPLICANT AND CONTRACTOR.

2. UTILIZE THE EXISTING SITE DRIVEWAY TO INITIALLY STAGE EQUIPMENT, PARKING AND MATERIAL.
3. INSTALL ALL SILT FENCES AND EROSION AND SEDIMENT CONTROL MEASURES IN ACCORDANCE WITH EROSION AND SEDIMENT CONTROL PLANS AND ESTABLISH STABILIZED CONSTRUCTION ENTRANCES AT THE TWO PROPOSED SITE DRIVEWAYS ON DOBBS FERRY ROAD.
4. FOLLOWING THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND FLAGGING OF LIMITS OF DISTURBANCE, THE TOWN REPRESENTATIVE AND QUALIFIED INSPECTOR IN EROSION AND SEDIMENT CONTROL SHALL PROVIDE AN INSPECTION AND WRITTEN APPROVAL OF INSTALLATION OF THE EROSION AND SEDIMENT CONTROL MEASURES FOR THE SITE PRIOR TO ANY CLEARING OR GRADING ACTIVITIES TAKING PLACE.
5. STAKE OUT LIMITS OF DISTURBANCE FOR PHASE 1A AND INFILTRATION BASINS #1 AND #2 AND INSTALL ORANGE CONSTRUCTION FENCE AROUND BASIN LIMITS TO PREVENT CONSTRUCTION VEHICLES FROM DRIVING WITHIN THE BASIN PRIOR TO THE COMMENCEMENT OF EARTHWORK ACTIVITIES.
6. CLEAR THE AREA OF TREES AND VEGETATION TO BE CONSTRUCTED IN PHASE 1A. VEGETATIVE WASTE WILL BE CHIPPED AND SPREAD ON-SITE AS AN INTERIM STABILIZATION METHOD.
7. REMOVE ANY REMAINING ASPHALT PATHS AND OTHER STRUCTURES LOCATED WITHIN PHASE 1A.
8. CONSTRUCT TEMPORARY SEDIMENT TRAP NUMBERS 1A-1 THROUGH 1A-7 AND TEMPORARY SEDIMENT DIKES 1A-8 THROUGH 1A-13. CONSTRUCT TEMPORARY DRAINAGE SWALES TO DIRECT DRAINAGE TO TRAPS AS EARTHWORK PROGRESSES.

9. COMMENCE MASS GRADING EARTHWORK ACTIVITIES. REMOVE AND RELOCATE EARTHWORK WITHIN DISTURBANCE AREA ON PHASE 1A. THE APPROXIMATE 45,344 CUBIC YARDS OF CUT FROM PHASE 1A SHALL BE UTILIZED AS MASS GRADING FOR THE APPROXIMATELY 13,071 CUBIC YARDS OF FILL NEEDED FOR PHASE 1B. THE REMAINING $\pm 32,273$ CUBIC YARDS OF FILL FROM PHASE 1A SHALL BE USED IN PHASE 1D. EARTHWORK ACTIVITIES WILL BE SIMULTANEOUS AND COORDINATED ACROSS EACH PHASE. STABILIZATION OF AREAS IDLE FOR MORE THAN 7 DAYS SHALL BE CONTINUOUS AND ONGOING AS THE AREAS ARE BROUGHT UP TO GRADE.
10. PROCEED WITH ROUGH GRADING OF THE AREAS UNDER ACTIVE CONSTRUCTION, BEGIN ROADWAY CONSTRUCTION OF PATTERSON COURT AND TILLINGHAST TRAIL AND SOUTHERN PORTION OF CURRIE WAY.
11. COMMENCE BUILDING FOUNDATION EXCAVATIONS FOR LOTS 1-10, 58-63, 82-86, AND 97-113. BUILDING FOUNDATIONS SHALL BE EXCAVATED TO A DEPTH OF 9.5' FOR A "WALKOUT" BASEMENT CONDITION AND 6.5' FOR A "WALKUP" BASEMENT CONDITION.
12. INSTALL STORMWATER DRAINAGE SYSTEMS LOCATED WITHIN PHASE 1A ROADWAYS. IMMEDIATELY INSTALL INLET PROTECTION ON ALL DRAIN INLETS AS COMPLETED. ALL INFILTRATION BASIN INLETS SHALL BE PLUGGED DURING CONSTRUCTION AND KEPT OFFLINE UNTIL THE TRIBUTARY AREA IS SUFFICIENTLY STABILIZED.
13. TRENCH AND INSTALL OTHER UTILITIES AS REQUIRED (WATER, SEWER, ELECTRIC, DATA).
14. INSTALL ASPHALT PAVEMENT.

15. SEED, MULCH, AND INSTALL SILT FENCING AROUND ANY TEMPORARY TOPSOIL STOCKPILE LOCATION FOR EROSION CONTROL PURPOSES FOR AREAS LEFT IDLE FOR MORE THAN 7 DAYS.
16. CLEAN SURROUNDING AREA AND STORM DRAINAGE SYSTEMS OF ALL ACCUMULATED SEDIMENT.
17. FINISH GRADING, REDISTRIBUTE TOPSOIL AND ESTABLISH VEGETATION. TEMPORARY SEDIMENT TRAP NUMBERS 1A-1, 1A-4 AND 1A-5 CAN BE TRANSITIONED TO PERMANENT STORMWATER MANAGEMENT BASINS WHEN 80% GERMINATION OF DISTURBED CONTRIBUTING AREAS HAS BEEN CONFIRMED BY TOWN REPRESENTATIVES, THE QUALIFIED INSPECTOR IN EROSION & SEDIMENT CONTROL, APPLICANT AND CONTRACTOR. DEWATERING, DE-MUCKING AND REMOVING ACCUMULATED SEDIMENT AS NECESSARY SHALL OCCUR DURING THE TRANSITION AND FINAL STABILIZATION.

PHASE 1B (APPROXIMATELY 18.13 ACRES)

1. A PRECONSTRUCTION SITE MEETING SHALL OCCUR WITH REPRESENTATIVES OF THE TOWN OF GREENBURGH, A QUALIFIED INSPECTOR IN SEDIMENT AND EROSION CONTROL, THE APPLICANT AND CONTRACTOR.
2. INSTALL ALL SILT FENCES AND EROSION AND SEDIMENT CONTROL MEASURES IN ACCORDANCE WITH EROSION AND SEDIMENT CONTROL PLANS AND ESTABLISH STABILIZED CONSTRUCTION ENTRANCE AT THE SOUTHERN END OF CURRIE WAY.
3. STAKE OUT LIMITS OF DISTURBANCE FOR PHASE 1B AND INFILTRATION BASIN #4 AND INSTALL ORANGE CONSTRUCTION FENCE AROUND BASIN LIMITS PRIOR TO THE COMMENCEMENT OF EARTHWORK. THE EXCESS FILL MATERIAL FROM PHASE 1A WILL BE UTILIZED TO ACHIEVE THE FILLS NEEDED IN PHASE 1B.

4. FOLLOWING THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND FLAGGING OF LIMITS OF DISTURBANCE, THE TOWN REPRESENTATIVE AND QUALIFIED INSPECTOR IN EROSION AND SEDIMENT CONTROL SHALL PROVIDE AN INSPECTION AND WRITTEN APPROVAL OF INSTALLATION OF THE EROSION AND SEDIMENT CONTROL MEASURES FOR THE SITE PRIOR TO ANY CLEARING OR GRADING ACTIVITIES TAKING PLACE.
5. CLEAR THE AREA OF TREES AND VEGETATION TO BE CONSTRUCTED IN PHASE 1B. VEGETATIVE WASTE WILL BE CHIPPED AND SPREAD ON-SITE AS AN INTERIM STABILIZATION METHOD.
6. REMOVE ANY REMAINING ASPHALT PATHS OR OTHER STRUCTURES LOCATED WITHIN PHASE 1B.
7. CONTINUE MASS GRADING ACTIVITIES UTILIZING THE APPROXIMATE 13,071 CUBIC YARDS OF IMPORT FROM PHASE 1A. STABILIZATION OF AREAS IDLE FOR MORE THAN 7 DAYS SHALL BE CONTINUOUS AND ONGOING AS THE AREAS ARE BROUGHT UP TO GRADE.
8. CONSTRUCT TEMPORARY SEDIMENT TRAP NUMBERS 1B-1 THROUGH 1B-6 AND TEMPORARY DIVERSION SWALES TO DIRECT DRAINAGE TO TRAPS AS EARTHWORK PROGRESSES. TEMPORARY SEDIMENT TRAP 1A-1 FROM PHASE 1A SHALL BE MAINTAINED DURING PHASE 1B.
9. PROCEED WITH ROUGH GRADING OF THE AREA UNDER ACTIVE CONSTRUCTION, BEGIN ROADWAY CONSTRUCTION OF TULL COURT AND BLUM STREET AND NORTHERN PORTION OF CURRIE WAY.
10. COMMENCE BUILDING FOUNDATION EXCAVATIONS FOR LOTS 11-28, 44 AND 51-57. BUILDING FOUNDATIONS SHALL BE EXCAVATED TO A DEPTH OF 9.5' FOR A

“WALKOUT” BASEMENT CONDITION AND 6.5' FOR A “WALKUP” BASEMENT CONDITION.

11. INSTALL STORMWATER DRAINAGE SYSTEMS LOCATED WITHIN PHASE 1B ROADWAYS. IMMEDIATELY INSTALL INLET PROTECTION ON ALL DRAIN INLETS AS COMPLETED. ALL INFILTRATION BASIN INLETS SHALL BE PLUGGED DURING CONSTRUCTION AND KEPT OFFLINE UNTIL THE TRIBUTARY AREA IS SUFFICIENTLY STABILIZED.
12. TRENCH AND INSTALL OTHER UTILITIES AS REQUIRED (WATER, SEWER, ELECTRIC, DATA).
13. INSTALL ASPHALT PAVEMENT.
14. SEED, MULCH, AND INSTALL SILT FENCING AROUND ANY TEMPORARY TOPSOIL STOCKPILE LOCATION FOR EROSION CONTROL PURPOSES FOR AREAS LEFT IDLE FOR MORE THAN 7 DAYS.
15. CLEAN SURROUNDING AREA AND STORM DRAINAGE SYSTEMS OF ALL ACCUMULATED SEDIMENT.
16. FINISH GRADING, REDISTRIBUTE TOPSOIL AND ESTABLISH VEGETATION. TEMPORARY SEDIMENT TRAP NUMBER 1C-8 CAN BE TRANSITIONED TO PERMANENT STORMWATER MANAGEMENT BASINS WHEN 80% GERMINATION OF DISTURBED CONTRIBUTING AREAS HAS BEEN CONFIRMED BY TOWN REPRESENTATIVES, THE QUALIFIED INSPECTOR IN EROSION & SEDIMENT CONTROL, APPLICANT AND CONTRACTOR. DEWATERING, DE-MUCKING AND REMOVING ACCUMULATED SEDIMENT AS NECESSARY SHALL OCCUR DURING THE TRANSITION AND FINAL STABILIZATION.
17. COMPLETE PHASE 1B CONSTRUCTION.

PHASE 1C (APPROXIMATELY 25.35 ACRES)

1. A PRECONSTRUCTION SITE MEETING SHALL OCCUR WITH REPRESENTATIVES OF THE TOWN OF GREENBURGH, A QUALIFIED INSPECTOR IN SEDIMENT AND EROSION CONTROL, THE APPLICANT AND CONTRACTOR.
2. INSTALL ALL SILT FENCES AND EROSION AND SEDIMENT CONTROL MEASURES IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLANS AND ESTABLISH STABILIZED CONSTRUCTION ENTRANCES ON CURRIE WAY, BLUM STREET AND DAY DRIVE.
3. STAKEOUT LIMITS OF DISTURBANCE FOR PHASE 1C.
4. FOLLOWING THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND FLAGGING OF LIMITS OF DISTURBANCE, THE TOWN REPRESENTATIVE AND QUALIFIED INSPECTOR IN EROSION AND SEDIMENT CONTROL SHALL PROVIDE AN INSPECTION AND WRITTEN APPROVAL OF INSTALLATION OF THE EROSION AND SEDIMENT CONTROL MEASURES FOR THE SITE PRIOR TO ANY CLEARING OR GRADING ACTIVITIES TAKING PLACE.
5. CLEAR THE AREA OF TREES AND VEGETATION TO BE CONSTRUCTED IN PHASE 1C. VEGETATIVE WASTE WILL BE CHIPPED AND SPREAD ON-SITE AS AN INTERIM STABILIZATION METHOD.
6. REMOVE ANY REMAINING ASPHALT PATHS OR OTHER STRUCTURES LOCATED WITHIN PHASE 1C.
7. APPROXIMATELY 32,797 CUBIC YARDS OF EXCESS EARTHWORK FROM MASS GRADING ACTIVITIES IN PHASE 1C WILL CONTINUOUSLY AND SIMULTANEOUSLY MOVED AS NEEDED FOR USE IN PHASE 1D. STABILIZATION OF AREAS IDLE FOR MORE THAN 7

DAYS SHALL BE CONTINUOUS AND ONGOING AS THE AREAS ARE BROUGHT UP TO GRADE.

8. CONSTRUCT TEMPORARY SEDIMENT TRAP NUMBERS IC-1 THROUGH IC-10 AND TEMPORARY DIVERSION SWALES TO DIRECT DRAINAGE TO TRAPS AS EARTHWORK PROGRESSES. TEMPORARY SEDIMENT TRAP IB-1 FROM PHASE IB SHALL BE MAINTAINED DURING PHASE IB.
9. PROCEED WITH ROUGH GRADING OF THE AREA UNDER ACTIVE CONSTRUCTION, CONTINUE ROADWAY CONSTRUCTION OF DAY DRIVE AND BLUM STREET AND NORTHERN PORTION OF CURRIE WAY. CONSTRUCT RETAINING WALLS ALONG NORTHERN PROPERTY BOUNDARY.
10. CONSTRUCT EMERGENCY ACCESS DRIVEWAY CONNECTION TO VALLEYVIEW ROAD. COMMENCE BUILDING FOUNDATION EXCAVATIONS FOR LOTS 29-43, 45-50 AND 63-82. BUILDING FOUNDATIONS SHALL BE EXCAVATED TO A DEPTH OF 9.5' FOR A "WALKOUT" BASEMENT CONDITION AND 6.5' FOR A "WALKUP" BASEMENT CONDITION.
11. INSTALL STORMWATER DRAINAGE SYSTEMS LOCATED WITHIN PHASE IC ROADWAYS. IMMEDIATELY INSTALL INLET PROTECTION ON ALL DRAIN INLETS AS COMPLETED. ALL INFILTRATION BASIN INLETS SHALL BE PLUGGED DURING CONSTRUCTION AND KEPT OFFLINE UNTIL THE TRIBUTARY AREA IS SUFFICIENTLY STABILIZED.
12. TRENCH AND INSTALL OTHER UTILITIES AS REQUIRED (WATER, SEWER, ELECTRIC, DATA).
13. INSTALL ASPHALT PAVEMENT.

14. SEED, MULCH, AND INSTALL SILT FENCING AROUND ANY TEMPORARY TOPSOIL STOCKPILE LOCATION FOR EROSION CONTROL PURPOSES FOR AREAS LEFT IDLE FOR MORE THAN 7 DAYS.
15. CLEAN SURROUNDING AREA AND STORM DRAINAGE SYSTEMS OF ALL ACCUMULATED SEDIMENT.
16. FINISH GRADING, REDISTRIBUTE TOPSOIL AND ESTABLISH VEGETATION. TEMPORARY SEDIMENT TRAP NUMBER 1C-8 CAN BE TRANSITIONED TO PERMANENT STORMWATER MANAGEMENT BASINS WHEN 80% GERMINATION OF DISTURBED CONTRIBUTING AREAS HAS BEEN CONFIRMED BY TOWN REPRESENTATIVES, THE QUALIFIED INSPECTOR IN EROSION & SEDIMENT CONTROL, APPLICANT AND CONTRACTOR. DEWATERING, DE-MUCKING AND REMOVING ACCUMULATED SEDIMENT AS NECESSARY SHALL OCCUR DURING THE TRANSITION AND FINAL STABILIZATION.
17. COMPLETE PHASE 1C CONSTRUCTION.

PHASE 1D (APPROXIMATELY 11.10 ACRES)

1. A PRECONSTRUCTION SITE MEETING SHALL OCCUR WITH REPRESENTATIVES OF THE TOWN OF GREENBURGH, A QUALIFIED INSPECTOR IN SEDIMENT AND EROSION CONTROL, THE APPLICANT AND CONTRACTOR.
2. INSTALL ALL SILT FENCES AND EROSION AND SEDIMENT CONTROL MEASURES IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLANS AND ESTABLISH STABILIZED CONSTRUCTION ENTRANCES ON THE NORTHERN AND SOUTHERN ENDS OF CURRIE WAY.
3. STAKEOUT LIMITS OF DISTURBANCE FOR PHASE 1D.

4. FOLLOWING THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND FLAGGING OF LIMITS OF DISTURBANCE, THE TOWN REPRESENTATIVE AND QUALIFIED INSPECTOR IN EROSION AND SEDIMENT CONTROL SHALL PROVIDE AN INSPECTION AND WRITTEN APPROVAL OF INSTALLATION OF THE EROSION AND SEDIMENT CONTROL MEASURES FOR THE SITE PRIOR TO ANY CLEARING OR GRADING ACTIVITIES TAKING PLACE.
5. CLEAR THE AREA OF TREES AND VEGETATION TO BE CONSTRUCTED IN PHASE ID. VEGETATIVE WASTE WILL BE CHIPPED AND SPREAD ON-SITE AS AN INTERIM STABILIZATION METHOD.
6. REMOVE ANY REMAINING ASPHALT PATHS OR OTHER STRUCTURES LOCATED WITHIN PHASE ID.
7. APPROXIMATELY 32,797 CUBIC YARDS OF EXCESS EARTHWORK FROM MASS GRADING ACTIVITIES IN PHASE IC WILL BE CONTINUOUSLY MOVED AS NEEDED FOR USE IN PHASE ID TO ACHIEVE THE FILLS REQUIRED. THESE EARTHWORK ACTIVITIES SHALL BE SIMULTANEOUS AND CONTINUOUS DURING EACH PHASE OF WORK TO MAXIMIZE THE EFFICIENCY OF IMPLEMENTING THE SUBDIVISION DESIGN. STABILIZATION OF AREAS IDLE FOR MORE THAN 7 DAYS SHALL BE CONTINUOUS AND ONGOING AS THE AREAS ARE BROUGHT UP TO GRADE.
8. CONSTRUCT TEMPORARY SEDIMENT TRAP NUMBERS ID-1 THROUGH ID-4 AND TEMPORARY DIVERSION SWALES TO DIRECT DRAINAGE TO TRAPS AS EARTHWORK PROGRESSES. SEDIMENT TRAP IC-8 SHALL BE MAINTAINED DURING PHASE ID.
9. PROCEED WITH ROUGH GRADING OF THE AREA UNDER ACTIVE CONSTRUCTION, COMPLETE ROADWAY CONSTRUCTION OF CURRIE WAY.

10. COMMENCE BUILDING FOUNDATION EXCAVATIONS FOR LOTS 86-96. BUILDING FOUNDATIONS SHALL BE EXCAVATED TO A DEPTH OF 9.5' FOR A "WALKOUT" BASEMENT CONDITION AND 6.5' FOR A "WALKUP" BASEMENT CONDITION.
11. INSTALL STORMWATER DRAINAGE SYSTEMS LOCATED WITHIN PHASE 1 D ROADWAYS. IMMEDIATELY INSTALL INLET PROTECTION ON ALL DRAIN INLETS AS COMPLETED. ALL INFILTRATION BASIN INLETS SHALL BE PLUGGED DURING CONSTRUCTION AND KEPT OFFLINE UNTIL THE TRIBUTARY AREA IS SUFFICIENTLY STABILIZED.
12. TRENCH AND INSTALL OTHER UTILITIES AS REQUIRED (WATER, SEWER, ELECTRIC, DATA).
13. INSTALL ASPHALT PAVEMENT.
14. SEED, MULCH, AND INSTALL SILT FENCING AROUND ANY TEMPORARY TOPSOIL STOCKPILE LOCATION FOR EROSION CONTROL PURPOSES FOR AREAS LEFT IDLE FOR MORE THAN 7 DAYS.
15. CLEAN SURROUNDING AREA AND STORM DRAINAGE SYSTEMS OF ALL ACCUMULATED SEDIMENT.
16. FINISH GRADING, REDISTRIBUTE TOPSOIL AND ESTABLISH VEGETATION. TEMPORARY SEDIMENT TRAP NUMBERS 1C-8 AND 1D-3 CAN BE TRANSITIONED TO PERMANENT STORMWATER MANAGEMENT BASINS WHEN 80% GERMINATION OF DISTURBED CONTRIBUTING AREAS HAS BEEN CONFIRMED BY TOWN REPRESENTATIVES, THE QUALIFIED INSPECTOR IN EROSION & SEDIMENT CONTROL, APPLICANT AND CONTRACTOR. DEWATERING, DE-MUCKING AND REMOVING ACCUMULATED SEDIMENT AS NECESSARY SHALL OCCUR DURING THE TRANSITION AND FINAL STABILIZATION.

17. COMPLETE PHASE 1D CONSTRUCTION.

WAIVER OF 5-ACRE DISTURBANCE NOTES:

THE PROVISIONS FOR DISTURBANCE BEYOND THE FIVE-ACRE THRESHOLD ARE OUTLINED IN THE GENERAL PERMIT 0-20-001, PART II, PERMIT COVERAGE – D. GENERAL REQUIREMENTS FOR OWNERS OR OPERATORS WITH PERMIT COVERAGE, 3. AND PART IV. INSPECTION AND MAINTENANCE REQUIREMENTS, C. QUALIFIED INSPECTOR INSPECTION REQUIREMENTS. THE STORMWATER POLLUTION PREVENTION PLAN (SWPPP) INCLUDES THE FOLLOWING GP-0-20-001 REQUIREMENTS:

- §1.0 - THE PROJECT DESCRIPTION INCLUDES THE NEED FOR THE DISTURBANCE OF GREATER THAN 5-ACRES AT ONE TIME.
- §3.0 – EROSION AND SEDIMENT CONTROLS PROVIDES A DESCRIPTION OF THE PHASING PLAN FOR THE PROJECT ALONG WITH THE NEED FOR AT LEAST TWO SITE INSPECTIONS EVERY SEVEN CALENDAR DAYS FOR AS LONG AS GREATER THAN FIVE ACRES OF SOIL REMAIN DISTURBED AND STABILIZATION MEASURES TO BE INITIATED BY THE END OF THE NEXT BUSINESS DAY AND COMPLETED WITHIN SEVEN DAYS FROM THE DATE THE CURRENT SOIL DISTURBANCE ACTIVITY CEASED.
- §3.2 – THE CONSTRUCTION SEQUENCING PROVIDES A TABLE SHOWING THE MAXIMUM DISTURBED AREA OF EACH CONSTRUCTION PHASE WITH THE CORRESPONDING CUT AND FILL VOLUMES REQUIRED.
- §4.0 – INSPECTION AND MAINTENANCE PROVIDES THE NEED FOR AT LEAST TWO SITE INSPECTIONS EVERY SEVEN CALENDAR DAYS FOR AS LONG AS GREATER THAN FIVE ACRES OF SOIL REMAIN DISTURBED.

- DRAWINGS C-400 THROUGH C-403 "EROSION AND SEDIMENT CONTROL PLANS (PHASE 1A -1D) PROVIDES NOTES ASSOCIATED WITH THE DISTURBANCE OF GREATER THAN FIVE ACRES.

Soil Description

As provided by the United States Department of Agriculture, Soil Conservation Service "Web Soil Survey," soil classifications which exist on the subject site are described below.

Soils are placed into four hydrologic groups: A, B, C, and D. In the definitions of the classes, infiltration rate is the rate at which water enters the soil at the surface and is controlled by the surface conditions. Transmission rate is the rate at which water moves in the soil and is controlled by soil properties. Definitions of the classes are as follows:

- A. (Low runoff potential). The soils have a high infiltration rate even when thoroughly wetted. They chiefly consist of deep, well drained to excessively drained sands or gravels. They have a high rate of water transmission.
- B. The soils have a moderate infiltration rate when thoroughly wetted. They chiefly are moderately deep to deep, moderately well drained to well drained soils that have moderately fine to moderately coarse textures. They have a moderate rate of water transmission.
- C. The soils have a slow infiltration rate when thoroughly wetted. They chiefly have a layer that impedes downward movement of water or have moderately fine to fine texture. They have a slow rate of water transmission.
- D. (High runoff potential). The soils have a very slow infiltration rate when thoroughly wetted. They chiefly consist of clay soils that have a high swelling potential, soils that have a permanent high water table, soils that have a claypan or clay layer at or near the surface, and

shallow soils over nearly impervious material. They have a very slow rate of water transmission.

A soil's tendency to erode is also described in the USDA web soil survey. The ratings in this interpretation indicate the hazard of soil loss from unsurfaced areas. The ratings are based on soil erosion factor K, slope, and content of rock fragments. The hazard is described as "slight," "moderate," or "SEVERE." A rating of "slight" indicates that little or no erosion is likely; "moderate" indicates that some erosion is likely, that the temporarily unsurfaced / unstabilized during construction may require occasional maintenance, and that simple erosion-control measures are needed; and "SEVERE" indicates that significant erosion is expected, that the roads or trails require frequent maintenance, and that erosion-control measures are needed. Per the Soil Survey, the following soils listed below are present at the site. Following this list is a detailed description of each soil type found on the property:

SYM. HYDRO. SOIL GROUP		DESCRIPTION
ChC	B	Charlton fine sandy loam, 8-15% slopes
ChB	B	Charlton fine sandy loam, 3-8% slopes
UIC	n/a	Urban land-charlton-chatfield complex, rolling, very Rocky
PnC	C	Paxton fine sandy loam, 8-15% slopes
ChE	B	Charlton loam, 25-35% slopes
PnB	C	Paxton fine sandy loam, 3-8% slopes
WdC	C/D	Woodbridge loam, 8-15% slopes
SuB	B	Sutton Loam, 3-8% slopes
Ub	B	Udorthents, smoothed
LeB	A/D	Leicester loam, 2-8% slopes, very stony
W	n/a	Water
Uc	A/D	Udorthents, web substratum
Uf	n/a	Urban land
CrC	B	Charlton-chatfield complex, 0-15% slopes, very Rocky
CsD	B	Chatfield-charlton complex, 15-35% slopes, very Rocky

ChC, Charlton fine sandy loam, 8-15% slopes

This soil is well drained. The parent material consists of coarse-loamy melt-out till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: B

Erosion Hazard Rating: Moderate

ChB, Charlton fine sandy loam, 3-8% slopes

This soil is well drained. The parent material consists of coarse-loamy melt-out till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: B

Erosion Hazard Rating: Moderate

UIC, Urban land-charlton-chatfield complex, rolling, very rocky

Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: n/a

Erosion Hazard Rating: n/a

PnC, Paxton fine sandy loam, 8-15% slopes

This soil is well drained. The parent material consists of coarse-loamy lodgment till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is 2 feet.

Hydrologic group: C

Erosion Hazard Rating: Moderate

ChE, Charlton loam, 25-35% slopes

This soil is well drained. The parent material consists of acid-loamy till derived mainly from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: B

Erosion Hazard Rating: Moderate

PnB, Paxton fine sandy loam, 3-8% slopes

This soil is well drained. The parent material consists of coarse-loamy lodgment till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is 2 feet.

Hydrologic group: C

Erosion Hazard Rating: Moderate

WdC, Woodbridge loam, 8-15% slopes

This soil is moderately well drained. The parent material consists of coarse-loamy lodgment till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is 1.5 feet.

Hydrologic group: C/D

Erosion Hazard Rating: Moderate

SuB, Sutton Loam, 3-8% slopes

This soil is moderately well drained. The parent material consists of coarse-loamy melt-out till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is 2 feet.

Hydrologic group: B

Erosion Hazard Rating: Moderate

Ub, Udorthents, smoothed

This soil is moderately well drained. Depth to the top of a seasonal high-water table is 2.5 feet.

Hydrologic group: B

Erosion Hazard Rating: Moderate

LeB, Leicester loam, 2-8% slopes, very stony

This soil is somewhat poorly drained. The parent material consists of loamy acid till derived mostly from schist and gneiss. Depth to the top of a seasonal high-water table is 1 foot.

Hydrologic group: A/D

Erosion Hazard Rating: Moderate

W, Water

Uc, Udorthents, web substratum

This soil is somewhat poorly drained. Depth to the top of a seasonal high-water table is 1.25 feet.

Hydrologic group: A/D

Erosion Hazard Rating: Moderate

Uf, Urban land

Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: n/a

Erosion Hazard Rating: n/a

CrC, Charlton-chatfield complex, 0-15% slopes, very rocky

This soil is well drained. The parent material consists of coarse-loamy melt-out till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: B

Erosion Hazard Rating: n/a

CsD, Chatfield-charlton complex, 15-35% slopes, very rocky

This soil is well drained. The parent material consists of coarse-loamy melt-out till derived from granite, gneiss, and/or schist. Depth to the top of a seasonal high-water table is greater than 6.5 feet.

Hydrologic group: B

Erosion Hazard Rating: n/a

On-Site Pollution Prevention

There are temporary pollution prevention measures used to control litter and construction debris on site, such as:

- Temporary Riser and Anti-Vortex Device

- Silt Fence
- Silt Sack
- Stone Check Dam
- Excavated Drop Inlet Protection
- Stone & Block Drop Inlet Protection

There will be inlet protection provided for all storm drains and inlets with the use of curb gutter inlet protection structures and stone & block drop inlet protection, which keep silt, sediment and construction litter and debris out of the on-site stormwater drainage system.

Temporary Control Measures

Temporary control measures and facilities will include silt fences, diversion swales, stabilized construction access, temporary seeding, mulching and sediment traps.

Throughout the construction of the proposed redevelopment, temporary control facilities will be implemented to control on-site erosion and sediment transfer. Diversion swales, if required, will be used to direct stormwater runoff to temporary sediment traps for settlement. The sediment basins and traps will be constructed as part of this project and will serve remove sediment and pollutants from the stormwater runoff produced during construction. Descriptions of the temporary sediment & erosion controls that will be used during the development of the site including silt fence, stabilized construction access, seeding, mulching and inlet protection are as follows:

- I. Silt Fence is constructed using a geotextile fabric. The fence will be either 18 inches or 30 inches high. The height of the fence can be increased in the event of placing these devices on uncompacted fills or extremely loose undisturbed soils. The fences will not be placed in areas which receive concentrated flows such as ditches, swales and channels nor will the filter fabric material be placed across the entrance to pipes, culverts, spillway structures, sediment traps or basins.

2. Stabilized Construction Access consists of AASHTO No. 1 rock. The rock entrance will be a minimum of 50 feet in length by 24 feet in width by 8 inches in depth.
3. Seeding will be used to create a vegetative surface to stabilize disturbed earth until at least 80% of the disturbed area has a perennial vegetative cover. This amount is required to adequately function as a sediment and erosion control facility. Grass lining will also be used to line temporary channels and the surrounding disturbed areas.
4. Mulching is used as an anchor for seeding and disturbed areas to reduce soil loss due to storm events. These areas will be mulched with straw at a rate of 3 tons per acre such that the mulch forms a continuous blanket. Mulch must be placed after seeding or within 48 hours after seeding is completed.
5. Inlet Protection will be provided for all stormwater basins and inlets with the use of curb & gutter inlet protection and stone & block inlet protection structures, which will keep silt, sediment and construction debris out of the storm system. Existing structures within existing paved areas will be protected using "Silt Sacks" inside the structures.
6. Sediments Traps will be used with the permanent SMP's until their contributing areas drainage are stabilized. Once stabilized, the temporary risers will be removed, and final grading/planting of the basins will be completed for permanent use as Stormwater Management basins.

The contractor shall be responsible for maintaining the temporary sediment and erosion control measures throughout construction. This maintenance will include, but not be limited to, the following tasks:

1. For dust control purposes, moisten all exposed graded areas with water at least twice a day in those areas where soil is exposed and cannot be planted with a temporary cover due to construction operations or the season (December through March).

2. Inspection of erosion and sediment control measures shall be performed at the end of each construction day and immediately following each rainfall event. All required repairs shall be immediately executed by the contractor.
3. Sediment deposits shall be removed when they reach approximately $\frac{1}{3}$ the height of the silt fence. All such sediment shall be properly disposed of in fill areas on the site, as directed by the Owner's Field Representative. Fill shall be protected following disposal with mulch, temporary and/or permanent vegetation and be completely circumscribed on the downhill side by silt fence.
4. Rake all exposed areas parallel to the slope during earthwork operations.
5. Following final grading, the disturbed area shall be stabilized with a permanent surface treatment (i.e. turf grass, pavement or sidewalk). During rough grading, areas which are not to be disturbed for fourteen or more days shall be stabilized with the temporary seed mixture, as defined on the plans. Seed all piles of dirt in exposed soil areas that will not receive a permanent surface treatment.

Concrete Material and Equipment Management

Concrete washouts shall be used to contain concrete and liquids when the chutes of concrete mixers and hoppers of concrete pumps are rinsed out after delivery. The washout facilities consolidate solid for easier disposal and prevent runoff of liquids. The wash water is alkaline and contains high levels of chromium, which can leach into the ground and contaminate groundwater. It can also migrate to a storm drain, which can increase the pH of area waters and harm aquatic life. Solids that are improperly disposed of can clog storm drain pipes and cause flooding. Installing concrete washout facilities not only prevents pollution but also is a matter of good housekeeping at your construction site.

Prefabricated concrete washout containers can be delivered to the site to provide maintenance and disposal of materials. Regular pick-ups of solid and liquid waste materials will be necessary. To prevent leaks on the job site, ensure that prefabricated washout containers are watertight. A self installed concrete washout facility can be utilized although they are much less reliable than prefabricated containers and are prone to leaks. There are many design options for the washout, but they are preferably built below-grade to prevent breaches and reduce the likelihood of runoff. Above-grade structures can also be used if they are sized and constructed correctly and are diligently maintained. One of the most common problems with self-installed concrete washout facilities is that they can leak or be breached as a result of constant use, therefore the contractor shall be sure to use quality materials and inspect the facilities daily.

Washouts must be sized to handle solids, wash water, and rainfall to prevent overflow. Concrete Washout Systems, Inc. estimates that 7 gallons of wash water are used to wash one truck chute and 50 gallons are used to wash out the hopper of a concrete pump truck.

For larger sites, a below-grade washout should be at least 10 feet wide and sized to contain all liquid and solid waste expected to be generated in between cleanout periods. A minimum of 12-inches of freeboard must be provided. The pit must be lined with plastic sheeting of at least 10-mil thickness without holes or tears to prevent leaching of liquids into the ground. Concrete wash water should never be placed in a pit that is connected to the storm drain system or that drains to nearby waterways.

An above-grade washout can be constructed at least 10 feet wide by 10 feet long and sized to contain all liquid and solid waste expected to be generated in between cleanout periods. A minimum of 4-inches of freeboard must be provided. The washout structures can be constructed with staked straw bales or sandbags double-or triple lined with plastic sheeting of at least 10-mil thickness without holes or tears.

Concrete washout facilities shall not be located within 50 feet of storm drains, open ditches, or water bodies and should be placed in locations that allow for convenient access for concrete trucks. The contractor shall check all concrete washout facilities daily to determine if they have

been filled to 75 percent capacity, which is when materials need to be removed. Both above-and below-ground self-installed washouts should be inspected daily to ensure that plastic linings are intact, and sidewalls have not been damaged by construction activities. Prefabricated washout containers should be inspected daily as well as to ensure the container is not leaking or nearing 75 percent capacity. Inspectors should also note whether the facilities are being used regularly. Additional signage for washouts may be needed in more convenient locations if concrete truck operators are not utilizing them.

The washout structures must be drained or covered prior to predicted rainstorms to prevent overflows. Hardened solids either whole or broken must be removed and then they may be reused onsite or hauled away for recycling.

Once materials are removed from the concrete washout, a new structure must be built or excavated, or if the previous structure is still intact, inspect it for signs of weakening or damage and make any necessary repairs. Line the structure with new plastic that is free of holes or tears and replace signage if necessary. It is very important that new plastic be used after every cleaning because pumps and concrete removal equipment can damage the existing liner.

Construction Site Chemical Control

The purpose of this management measure is to prevent the generation of nonpoint source pollution from construction sites due to improper handling and usage of nutrients and toxic substances, and to prevent the movement of toxic substances from the construction site.

Many potential pollutants other than sediment are associated with construction activities. These pollutants include pesticides; fertilizers used for vegetative stabilization; petrochemicals; construction chemicals such as concrete products, sealers, and paints; wash water associated with these products; paper; wood; garbage; and sanitary waste.

Disposal of excess pesticides and pesticide-related wastes should conform to registered label directions for the disposal and storage of pesticides and pesticide containers set forth in

applicable Federal, State and local regulations that govern their usage, handling, storage, and disposal.

Pesticides should be disposed of through either a licensed waste management firm or a treatment, storage and disposal (TSD) facility. Containers should be triple rinsed before disposal, and rinse waters should be reused as product. Other practices include setting aside a locked storage area, tightly closing lids, storing in a cool, dry place, checking containers periodically for leaks or deterioration, maintaining a list of products in storage, using plastic sheeting to line the storage areas, and notifying neighboring property owners prior to spraying.

When storing petroleum products, follow these guidelines:

- Create a shelter around the area with cover and wind protection;
- Line the storage area with a double layer of plastic sheeting or similar material;
- Create an impervious berm around the perimeter with a capacity of 110 percent greater than that of the largest container;
- Clearly label all products;
- Keep tanks off the ground; and
- Keep lids securely fastened.

Post spill procedure information and have persons trained in spill handling on site or on call at all times. Materials for cleaning up spills should be kept on site and easily available. Spills should be cleaned up immediately and the contaminated material properly disposed of. Maintain and wash equipment and machinery in confined areas specifically designed to control runoff.

Thinners or solvents should not be discharged into sanitary or storm systems when cleaning machinery. Use alternative methods for cleaning larger equipment parts, such as high-pressure, high-temperature water washes, or steam cleaning. Equipment-washing detergents can be used, and wash water may be discharged into sanitary sewers if solids are removed from the solution first. (This practice should be verified with the local sewer authority.) Small parts can be cleaned with degreasing solvents, which can then be reused or recycled.

Solid Waste Management and Portable Sanitary Management

The purpose of this management measure is to prevent the potential for solid waste such as construction debris, trash, etc. from construction sites due to improper handling and storage. Debris and litter should be removed periodically from the BMP's and surrounding areas to prevent clogging of pipes and structures. All construction material shall be stored in designated staging areas. Roll-off containers shall be placed on site and all empty containers, construction debris and litter shall be placed in the containers.

Portable sanitary units may be utilized on-site or bathrooms will be provided within construction trailers. A sanitation removal company will be hired to pump/remove any sanitary waste. In the event that portable sanitary units are used and then cleaned after being emptied, the rinse water may not be disposed of to the storm drain system. It shall be contained for later disposal if it can't be disposed of on-site. Remove paper and trash before cleaning the portable sanitary units. The portable sanitary units shall be located away from the storm drain system if possible. Provide over head cover for wash areas if possible. Maintain spill response material and equipment on site to eliminate the potential for contaminants and wash water from entering the storm drain system.

Permanent Control Measures and Facilities for Long Term Protection

Towards the completion of construction, permanent sediment and erosion control measures will be developed for long term erosion protection. The following permanent control measures and facilities have been proposed to be implemented for the project:

1. Vegetated Swales will function to provide additional treatment of stormwater runoff by removal of pollutants and will promote a reduction of peak flows and provide runoff infiltration.
2. Infiltration Basins will be used to treat the runoff volume generated from the developed area and provide improvement to water quality control. The proposed basins will provide water

quality for 90% of the average annual stormwater runoff volume. The water quality volume will be retained, and higher storms will be released gradually. Refer to the water quality volume calculations, in Appendix 'C'.

3. Catch Basins will be used to remove some of the coarse sand and grit sediment before entering the drainage system. Each catch basin will be constructed with an 18-inch-deep sump.
4. Wet Extended Detention Ponds (P-3) which are ponds that treat the majority of the water quality volume through extended detention and incorporate a micropool at the outlet of the pond to prevent sediment resuspension. Each Extended Detention Pond has a forebay for pretreatment, an aquatic bench and micropool for treatment, a 12-foot-wide maintenance access path, 4 foot horizontal to 1-foot vertical side slopes for added safety and ease for maintenance and outlet control structures to slowly release the water quantity storms to reduce the peak flow rates and for channel protection.
5. Rip-Rap Energy Dissipators at discharge points from the stormwater drainage system into the stormwater management basins, rip-rap pads consisting of angular rocks will be placed to dissipate velocity and reduce the risk of erosion. The rip-rap pads will be 10 feet wide by 10 feet long.
6. Seeding of at least 80% perennial vegetative cover will be used to produce a permanent uniform erosion resistant surface. The seeded areas will be mulched with straw at a rate of 2 tons per acre such that the mulch forms a continuous blanket.
7. Contech Water Quality Structure will be used to provide pretreatment of the water quality flow rate for separating sediment, debris, floatables, etc. from the runoff prior to discharge to the SMP's. The CDS Water Quality Structure has been designed to treat up to the required water quality volume and appropriately handle all storm frequencies without the resuspension of solids. The system will provide 80% TSS removal rate for particles having a mean particle size of 125 microns for stormwater runoff.

Specifications for Soil Restoration

Prior to the final stabilization of the disturbed areas, soil restoration will be required for all vegetated areas to recover the original properties and porosity of the soil. Soil Restoration Requirements are provided in the table below:

Table 7

Soil Restoration Requirements

Type of Soil Disturbance	Soil Restoration Requirement		Comments/Examples
No soil disturbance	Restoration not permitted		Preservation of Natural Features
Minimal soil disturbance	Restoration not required		Clearing and grubbing
Areas where topsoil is stripped only – no change in grade	HSG A&B	HSG C&D	Protect area from any ongoing construction activities
	apply 6 inches of topsoil	Aerate* and apply 6 inches of topsoil	
Areas of cut or fill	HSG A&B	HSG C&D	Clearing and grubbing
	Aerate and apply 6 inches of topsoil	Apply full Soil Restoration**	
Heavy traffic areas on site (especially) in a zone 5-25 feet around buildings but not within a 5-foot perimeter around foundation walls)	Apply full Soil Restoration (decompaction and compost enhancement)		
Areas where Runoff Reduction and/or Infiltration practices are applied	Restoration not required but may be applied to enhance the reduction specified for appropriate practices.		Keep construction equipment from crossing these areas. To protect newly installed practice from any ongoing construction activities construct a single-phase operation fence area.
Redevelopment projects	Soil Restoration is required on redevelopment projects in areas where existing impervious area will be converted to pervious area.		

* Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

** Per "Deep Ripping and De-compaction, DEC 2008."

During periods of relatively low to moderate subsoil moisture, the disturbed subsoils are returned to rough grade and the following full soil restoration steps applied:

1. Apply 3 inches of compost over subsoil.
2. Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils.
3. Rock-pick until uplifted stone/rock materials of four inches and larger size are cleaned off the site.

Specifications for Final Stabilization of Graded Areas

Final stabilization of graded areas consists of the placement of topsoil and installation of landscaping (unless the area is to be paved, or a building is to be constructed in the location). Topsoil is to be spread as soon as grading operations are completed. Topsoil is to be placed to a minimum depth of six inches on all embankments, planting areas and seeding/sod areas. The subgrade is to be scarified to a depth of two inches to provide a bond of the topsoil with the subsoil. Topsoil is to be raked to an even surface and cleared of all debris, roots, stones and other unsatisfactory material.

Planting operations shall be conducted under favorable weather conditions as follows:

- Permanent Lawns - April 15 (provided soil is frost-free and not excessively moist) to May 15; August 15 to October 15.

- Temporary Lawn Seeding - if outside of the time periods noted above, the areas shall be seeded immediately on completion of topsoil operations with annual ryegrass (Italian rye) at a rate of six pounds per 1,000 square feet. Temporary lawn installation is permitted provided the soil is frost-free and not excessively moist. The permanent lawn is to be installed the next planting season.

On slopes with a grade of 3 horizontal to 1 vertical or greater, and in swales, a geotextile netting or mat shall be installed for stabilization purposes as shown on the Plans. Seeded areas are to be mulched with straw or hay at an application rate of 70-90 pounds per 1,000 s.f. Straw or hay mulch must be spread uniformly and anchored immediately after spreading to prevent wind blowing. Mulches must be inspected periodically and in particular after rainstorms to check for erosion. If erosion is observed, additional mulch must be applied. Netting shall be inspected after rainstorms for dislocation or failure; any damage shall be repaired immediately.

All denuded surfaces which will be exposed for a period of over two months or more shall be temporarily hydroseeded with (a) perennial ryegrass at a rate of 40 lbs per acre (1.0 lb per 1000 square feet); (b) Certified "Aroostook" winter rye (cereal rye) @ 100 lb per acre (2.5 lb/1000 s.f.) to be used in the months of October and November.

Permanent turfgrass cover is to consist of a seed mixture as follows:

(a) Sunny sites

Kentucky Bluegrass	2.0-2.6 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet
Fine Fescue	0.4-0.6 pounds/1000 square feet

(b) Shady sites

Kentucky Bluegrass	0.8-1.0 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet

Fine Fescue

2.6-3.3 pounds/1000 square feet

All plant materials shall comply with the standards of the American Association Of Nurserymen with respect to height and caliper as described in its publication American Standard for Nursery Stock, latest edition.

VII. CONSTRUCTION PHASE AND POST-CONSTRUCTION MAINTENANCE

During the construction phase and following construction of the project, a number of maintenance measures will be taken with respect to the site maintenance. Measures to be taken included the following:

I. During Construction

A comprehensive erosion and sediment control plan will be in place during the construction period. Maintenance measures for sediment and erosion controls will include:

A qualified professional acceptable to the municipality will be hired by the owner or operator to monitor the installation and maintenance of the sediment and erosion control plans. The qualified professional shall report directly to the Engineering Consultant and shall be responsible for ensuring compliance with the design of the sediment and erosion control plans.

The qualified professional so hired will inspect all sediment and erosion control measures at least every seven calendar days. In the event that there has been a variance with the design of the sediment and erosion control measures so that the ability of the measures to adequately perform the intended function is lessened or compromised and/or the facilities are not adequately maintained, the qualified professional shall be required to report such variance to the Engineering Consultant within 48 hours and shall be empowered to order immediate repairs to the sediment and erosion control measures.

The qualified professional will also be responsible for observing the adequacy of the vegetation growth (trees, shrubs, groundcovers and turfgrasses) in newly graded areas and for ordering

additional plantings in the event that the established plant materials do not adequately protect the ground surface from erosion.

2. Following Construction

Site maintenance activities on the property will include:

- Grounds maintenance, including mowing of lawns;
- Planting of trees, shrubs and groundcovers; pruning of trees and shrubs;
- Application of fertilizer and herbicides;
- Maintenance of stormwater management area;

Grounds maintenance on the site will be performed by landscaping contractor.

Fertilizer is typically applied twice in the year - once in the spring and once in the fall. The application of fertilizer is usually necessary to maintain healthy lawn growth due to competition for nutrients with trees and shrubs and since the clippings are often removed. It is not recommended that fertilizer be applied during the summer. It is at this time that lawns are typically dormant.

Fertilizers come in three basic types: (1) Organic; (2) Soluble synthetic and (3) Slow release.

Organic fertilizers are derived from plant or animal waste. Since they are heavier and bulkier than other fertilizers, it is necessary to apply a much greater amount at one time. Soluble synthetic fertilizers are predictable with determining the exact impact on a lawn. However, more applications are necessary since their effect is often short term. Slow release fertilizers have a high percentage of nitrogen so quantities that need be handled at one time are smaller. Slow release fertilizers will be utilized by the project.

A complete fertilizer contains all three of the primary nutrients - nitrogen (N), phosphorus (P) and potassium in the form of potash (K). Typically, a 3-1-2 ratio of nutrients (N-P-K) is used for lawn applications.

Fertilizer shall be applied by the landscape contractor in accordance with the manufacturer's instructions. The application of fertilizer does require some skill on the part of the operator. Should there be a spill of fertilizer, the landscape contractor shall be required to scrape or vacuum it up. The area will then be watered in accordance with the manufacturer's instructions to ensure that the fertilizer becomes soluble and available to plants and does not run off.

The Owner/Operator will be responsible for the long-term operation and maintenance of the permanent stormwater management practices. The permanent stormwater management practices shall be maintained in accordance with the Maintenance Inspection Checklists provided in Appendix F.

VIII. CONCLUSION

This Stormwater Pollution Prevention Plan has been prepared to describe the project's pre and post-development stormwater management improvements and its erosion and sediment control improvements to be utilized during construction. The proposed permanent improvements and the interim improvements to be utilized during construction have been designed in accordance with the requirements of the:

- New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-20-001, effective January 29, 2020.
- Chapter 248 "Stormwater Management" of the Town of Greenburgh Code.
- New York State Stormwater Management Design Manual, last revised January 2015.

The project employs a variety of practices to enhance stormwater quality and reduce peak rates of runoff associated with the proposed improvements. These measures include vegetated swales and stormwater management basins. These improvements will also mitigate runoff volumes from the proposed improvements as runoff volumes will be reduced in all of the analyzed storms.

Based on the foregoing, it is our professional opinion that the proposed improvements will provide water quantity and quality enhancements which exceed the above-mentioned requirements and are not anticipated to have any adverse impacts to the site or any surrounding areas.

APPENDIX A

EXISTING HYDROLOGIC CALCULATIONS

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

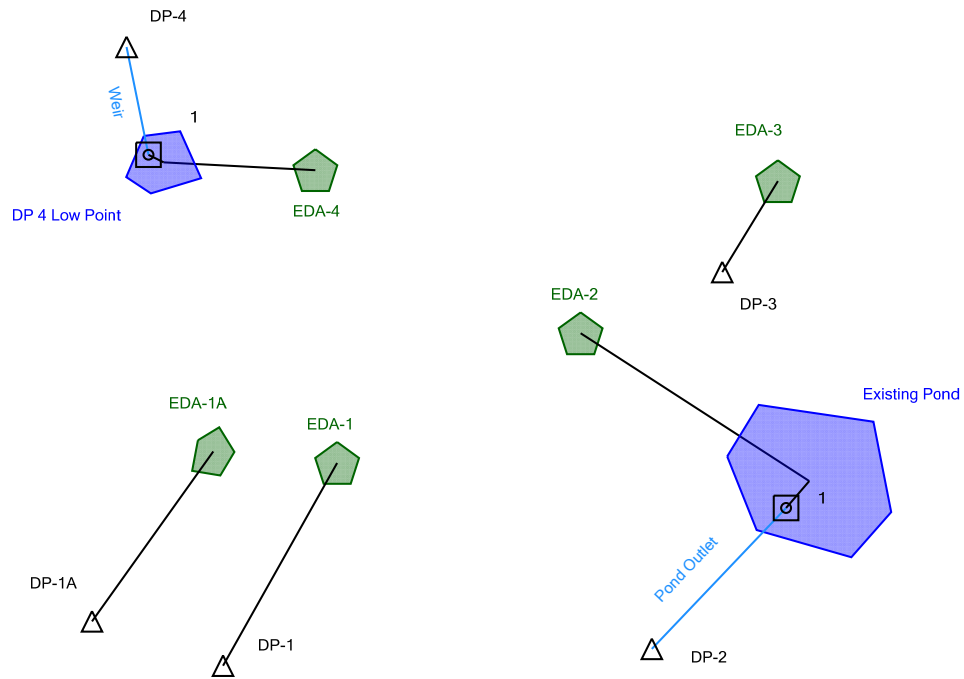


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Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
EDA-4	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	17,787.000	12.250	2.75
EDA-4	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	31,247.000	12.200	6.14
EDA-4	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	78,195.000	12.150	18.08
EDA-4	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	122,113.000	12.150	29.48
EDA-4	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	222,373.000	12.150	55.08
EDA-1	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	24,923.000	12.100	7.12
EDA-1	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	35,216.000	12.100	10.10
EDA-1	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	65,336.000	12.100	18.52
EDA-1	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	90,206.000	12.100	25.24
EDA-1	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	142,231.000	12.100	38.83
EDA-2	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	64,481.000	12.400	9.88
EDA-2	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	105,997.000	12.350	18.30
EDA-2	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	243,856.000	12.300	46.57
EDA-2	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	368,547.000	12.300	72.02
EDA-2	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	646,483.000	12.300	127.61

Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
EDA-3	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	58,924.000	12.400	8.74
EDA-3	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	98,325.000	12.350	16.71
EDA-3	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	230,673.000	12.300	44.05
EDA-3	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	351,345.000	12.300	68.84
EDA-3	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	621,860.000	12.300	123.29
EDA-1A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	9,437.000	12.100	2.63
EDA-1A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	13,953.000	12.100	3.97
EDA-1A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	27,698.000	12.100	7.93
EDA-1A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	39,377.000	12.100	11.20
EDA-1A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	64,288.000	12.100	17.94

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
DP-1	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	24,923.000	12.100	7.12
DP-1	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	35,216.000	12.100	10.10
DP-1	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	65,336.000	12.100	18.52

Watershed

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
DP-1	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	90,206.000	12.100	25.24
DP-1	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	142,231.000	12.100	38.83
DP-4	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	17,787.000	12.250	2.71
DP-4	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	31,247.000	12.250	6.01
DP-4	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	78,195.000	12.200	18.06
DP-4	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	122,113.000	12.200	29.19
DP-4	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	222,373.000	12.200	54.08
DP-2	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	57,877.000	15.600	1.40
DP-2	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	98,903.000	13.950	3.25
DP-2	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	235,910.000	12.750	19.97
DP-2	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	360,222.000	12.550	51.48
DP-2	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	637,554.000	12.400	117.62
DP-3	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	58,924.000	12.400	8.74
DP-3	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	98,325.000	12.350	16.71
DP-3	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	230,673.000	12.300	44.05

Watershed

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
DP-3	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	351,345.000	12.300	68.84
DP-3	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	621,860.000	12.300	123.29
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	9,437.000	12.100	2.63
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	13,953.000	12.100	3.97
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	27,698.000	12.100	7.93
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	39,377.000	12.100	11.20
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	64,288.000	12.100	17.94

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Existing Pond (IN)	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	64,481.000	12.400	9.88	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	57,877.000	15.600	1.40	319.65	29,198.000
Existing Pond (IN)	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	105,997.000	12.350	18.30	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	98,903.000	13.950	3.25	319.87	43,027.000

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	243,856.000	12.300	46.57	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	235,910.000	12.750	19.97	320.48	85,909.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	368,547.000	12.300	72.02	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	360,222.000	12.550	51.48	320.74	107,002.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	646,483.000	12.300	127.61	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	637,554.000	12.400	117.62	320.98	128,190.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	17,787.000	12.250	2.75	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	17,787.000	12.250	2.71	296.45	176.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	31,247.000	12.200	6.14	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	31,247.000	12.250	6.01	296.61	274.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	78,195.000	12.150	18.08	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	78,195.000	12.200	18.06	297.02	641.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	122,113.000	12.150	29.48	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	122,113.000	12.200	29.19	297.20	868.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	222,373.000	12.150	55.08	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	222,373.000	12.200	54.08	297.57	1,453.000

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time-Depth Curve: TypeIII 24hr (2.8 in)

Label	TypeIII 24hr (2.8 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	1 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.0	0.0	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.1	0.1	0.1
4.500	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.2	0.2
6.500	0.2	0.2	0.2	0.2	0.2
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.3
8.000	0.3	0.3	0.3	0.3	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.4	0.4	0.5
9.500	0.5	0.5	0.5	0.5	0.5
10.000	0.5	0.5	0.6	0.6	0.6
10.500	0.6	0.6	0.6	0.7	0.7
11.000	0.7	0.7	0.7	0.8	0.8
11.500	0.8	0.9	1.0	1.0	1.2
12.000	1.4	1.6	1.8	1.9	1.9
12.500	2.0	2.0	2.0	2.1	2.1
13.000	2.1	2.1	2.1	2.2	2.2
13.500	2.2	2.2	2.2	2.3	2.3
14.000	2.3	2.3	2.3	2.3	2.3
14.500	2.3	2.4	2.4	2.4	2.4
15.000	2.4	2.4	2.4	2.4	2.4
15.500	2.4	2.5	2.5	2.5	2.5
16.000	2.5	2.5	2.5	2.5	2.5

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	2.5	2.5	2.5	2.5	2.5
17.000	2.6	2.6	2.6	2.6	2.6
17.500	2.6	2.6	2.6	2.6	2.6
18.000	2.6	2.6	2.6	2.6	2.6
18.500	2.6	2.6	2.6	2.6	2.6
19.000	2.7	2.7	2.7	2.7	2.7
19.500	2.7	2.7	2.7	2.7	2.7
20.000	2.7	2.7	2.7	2.7	2.7
20.500	2.7	2.7	2.7	2.7	2.7
21.000	2.7	2.7	2.7	2.7	2.7
21.500	2.7	2.7	2.7	2.7	2.8
22.000	2.8	2.8	2.8	2.8	2.8
22.500	2.8	2.8	2.8	2.8	2.8
23.000	2.8	2.8	2.8	2.8	2.8
23.500	2.8	2.8	2.8	2.8	2.8
24.000	2.8	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time-Depth Curve: TypeIII 24hr (3.4 in)

Label	TypeIII 24hr (3.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	2 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.2	0.2	0.2	0.2
4.500	0.2	0.2	0.2	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.3	0.3	0.3	0.3
6.500	0.3	0.3	0.3	0.3	0.3
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.4	0.4	0.4	0.4
8.000	0.4	0.4	0.4	0.4	0.4
8.500	0.4	0.5	0.5	0.5	0.5
9.000	0.5	0.5	0.5	0.5	0.6
9.500	0.6	0.6	0.6	0.6	0.6
10.000	0.6	0.7	0.7	0.7	0.7
10.500	0.7	0.8	0.8	0.8	0.8
11.000	0.9	0.9	0.9	0.9	1.0
11.500	1.0	1.1	1.2	1.3	1.4
12.000	1.7	2.0	2.1	2.3	2.4
12.500	2.4	2.4	2.5	2.5	2.5
13.000	2.6	2.6	2.6	2.6	2.7
13.500	2.7	2.7	2.7	2.7	2.8
14.000	2.8	2.8	2.8	2.8	2.8
14.500	2.9	2.9	2.9	2.9	2.9
15.000	2.9	2.9	3.0	3.0	3.0
15.500	3.0	3.0	3.0	3.0	3.0
16.000	3.0	3.0	3.1	3.1	3.1

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	3.1	3.1	3.1	3.1	3.1
17.000	3.1	3.1	3.1	3.1	3.1
17.500	3.2	3.2	3.2	3.2	3.2
18.000	3.2	3.2	3.2	3.2	3.2
18.500	3.2	3.2	3.2	3.2	3.2
19.000	3.2	3.2	3.2	3.3	3.3
19.500	3.3	3.3	3.3	3.3	3.3
20.000	3.3	3.3	3.3	3.3	3.3
20.500	3.3	3.3	3.3	3.3	3.3
21.000	3.3	3.3	3.3	3.3	3.3
21.500	3.3	3.3	3.4	3.4	3.4
22.000	3.4	3.4	3.4	3.4	3.4
22.500	3.4	3.4	3.4	3.4	3.4
23.000	3.4	3.4	3.4	3.4	3.4
23.500	3.4	3.4	3.4	3.4	3.4
24.000	3.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time-Depth Curve: TypeIII 24hr (5.1 in)

Label	TypeIII 24hr (5.1 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	10 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.2	0.2	0.2	0.2	0.2
4.500	0.3	0.3	0.3	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.3
5.500	0.3	0.3	0.3	0.4	0.4
6.000	0.4	0.4	0.4	0.4	0.4
6.500	0.4	0.4	0.4	0.4	0.5
7.000	0.5	0.5	0.5	0.5	0.5
7.500	0.5	0.5	0.5	0.6	0.6
8.000	0.6	0.6	0.6	0.6	0.6
8.500	0.7	0.7	0.7	0.7	0.7
9.000	0.7	0.8	0.8	0.8	0.8
9.500	0.8	0.9	0.9	0.9	0.9
10.000	1.0	1.0	1.0	1.0	1.1
10.500	1.1	1.1	1.2	1.2	1.2
11.000	1.3	1.3	1.4	1.4	1.5
11.500	1.5	1.6	1.7	1.9	2.1
12.000	2.5	3.0	3.2	3.4	3.5
12.500	3.6	3.6	3.7	3.7	3.8
13.000	3.8	3.9	3.9	3.9	4.0
13.500	4.0	4.0	4.0	4.1	4.1
14.000	4.1	4.2	4.2	4.2	4.2
14.500	4.2	4.3	4.3	4.3	4.3
15.000	4.3	4.4	4.4	4.4	4.4
15.500	4.4	4.5	4.5	4.5	4.5
16.000	4.5	4.5	4.5	4.5	4.6

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	4.6	4.6	4.6	4.6	4.6
17.000	4.6	4.6	4.7	4.7	4.7
17.500	4.7	4.7	4.7	4.7	4.7
18.000	4.7	4.7	4.7	4.7	4.8
18.500	4.8	4.8	4.8	4.8	4.8
19.000	4.8	4.8	4.8	4.8	4.8
19.500	4.8	4.8	4.9	4.9	4.9
20.000	4.9	4.9	4.9	4.9	4.9
20.500	4.9	4.9	4.9	4.9	4.9
21.000	4.9	4.9	4.9	5.0	5.0
21.500	5.0	5.0	5.0	5.0	5.0
22.000	5.0	5.0	5.0	5.0	5.0
22.500	5.0	5.0	5.0	5.0	5.0
23.000	5.0	5.0	5.1	5.1	5.1
23.500	5.1	5.1	5.1	5.1	5.1
24.000	5.1	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time-Depth Curve: TypeIII 24hr (6.4 in)

Label	TypeIII 24hr (6.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.2
2.500	0.2	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.3	0.3
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.3	0.3	0.3	0.4
5.000	0.4	0.4	0.4	0.4	0.4
5.500	0.4	0.4	0.4	0.4	0.4
6.000	0.5	0.5	0.5	0.5	0.5
6.500	0.5	0.5	0.5	0.6	0.6
7.000	0.6	0.6	0.6	0.6	0.6
7.500	0.6	0.7	0.7	0.7	0.7
8.000	0.7	0.7	0.8	0.8	0.8
8.500	0.8	0.8	0.9	0.9	0.9
9.000	0.9	1.0	1.0	1.0	1.0
9.500	1.1	1.1	1.1	1.1	1.2
10.000	1.2	1.2	1.3	1.3	1.3
10.500	1.4	1.4	1.5	1.5	1.5
11.000	1.6	1.6	1.7	1.8	1.8
11.500	1.9	2.0	2.2	2.4	2.7
12.000	3.2	3.7	4.0	4.2	4.4
12.500	4.5	4.6	4.6	4.7	4.7
13.000	4.8	4.8	4.9	4.9	5.0
13.500	5.0	5.0	5.1	5.1	5.1
14.000	5.2	5.2	5.2	5.3	5.3
14.500	5.3	5.3	5.4	5.4	5.4
15.000	5.5	5.5	5.5	5.5	5.5
15.500	5.6	5.6	5.6	5.6	5.6
16.000	5.7	5.7	5.7	5.7	5.7

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	5.7	5.7	5.8	5.8	5.8
17.000	5.8	5.8	5.8	5.8	5.9
17.500	5.9	5.9	5.9	5.9	5.9
18.000	5.9	5.9	5.9	6.0	6.0
18.500	6.0	6.0	6.0	6.0	6.0
19.000	6.0	6.0	6.0	6.0	6.1
19.500	6.1	6.1	6.1	6.1	6.1
20.000	6.1	6.1	6.1	6.1	6.1
20.500	6.1	6.2	6.2	6.2	6.2
21.000	6.2	6.2	6.2	6.2	6.2
21.500	6.2	6.2	6.2	6.2	6.3
22.000	6.3	6.3	6.3	6.3	6.3
22.500	6.3	6.3	6.3	6.3	6.3
23.000	6.3	6.3	6.3	6.3	6.3
23.500	6.4	6.4	6.4	6.4	6.4
24.000	6.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time-Depth Curve: TypeIII 24hr (9.0 in)

Label	TypeIII 24hr (9.0 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	100 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.1	0.1	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.2	0.2	0.2
2.000	0.2	0.2	0.2	0.2	0.2
2.500	0.2	0.2	0.2	0.3	0.3
3.000	0.3	0.3	0.3	0.3	0.3
3.500	0.3	0.3	0.4	0.4	0.4
4.000	0.4	0.4	0.4	0.4	0.4
4.500	0.4	0.5	0.5	0.5	0.5
5.000	0.5	0.5	0.5	0.5	0.6
5.500	0.6	0.6	0.6	0.6	0.6
6.000	0.6	0.7	0.7	0.7	0.7
6.500	0.7	0.7	0.8	0.8	0.8
7.000	0.8	0.8	0.9	0.9	0.9
7.500	0.9	0.9	1.0	1.0	1.0
8.000	1.0	1.0	1.1	1.1	1.1
8.500	1.2	1.2	1.2	1.2	1.3
9.000	1.3	1.3	1.4	1.4	1.5
9.500	1.5	1.5	1.6	1.6	1.7
10.000	1.7	1.7	1.8	1.8	1.9
10.500	1.9	2.0	2.1	2.1	2.2
11.000	2.2	2.3	2.4	2.5	2.6
11.500	2.7	2.8	3.0	3.4	3.7
12.000	4.5	5.2	5.6	5.9	6.2
12.500	6.3	6.4	6.5	6.6	6.7
13.000	6.7	6.8	6.9	6.9	7.0
13.500	7.0	7.1	7.1	7.2	7.2
14.000	7.3	7.3	7.4	7.4	7.5
14.500	7.5	7.5	7.6	7.6	7.6
15.000	7.7	7.7	7.7	7.8	7.8
15.500	7.8	7.9	7.9	7.9	7.9
16.000	8.0	8.0	8.0	8.0	8.0

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	8.1	8.1	8.1	8.1	8.1
17.000	8.2	8.2	8.2	8.2	8.2
17.500	8.3	8.3	8.3	8.3	8.3
18.000	8.3	8.3	8.4	8.4	8.4
18.500	8.4	8.4	8.4	8.4	8.5
19.000	8.5	8.5	8.5	8.5	8.5
19.500	8.5	8.5	8.6	8.6	8.6
20.000	8.6	8.6	8.6	8.6	8.6
20.500	8.7	8.7	8.7	8.7	8.7
21.000	8.7	8.7	8.7	8.7	8.7
21.500	8.8	8.8	8.8	8.8	8.8
22.000	8.8	8.8	8.8	8.8	8.8
22.500	8.9	8.9	8.9	8.9	8.9
23.000	8.9	8.9	8.9	8.9	8.9
23.500	8.9	8.9	9.0	9.0	9.0
24.000	9.0	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time of Concentration Calculations

Label: EDA-1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	150.00 ft
Manning's n	0.013
Slope	0.087 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	2.42 ft/s
Segment Time of Concentration	0.017 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	568.00 ft
Is Paved?	True
Slope	0.056 ft/ft
Average Velocity	4.81 ft/s
Segment Time of Concentration	0.033 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.083 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: EDA-1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$(L_f / V) / 3600$$

Where:

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V}$$
$$V = 16.1345 * (S_f^{0.5})$$

$$\text{Paved Surface:}$$
$$V = 20.3282 * (S_f^{0.5})$$

$$(L_f / V) / 3600$$

Where:

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: EDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	26.00 ft
Manning's n	0.240
Slope	0.192 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.23 ft/s
Segment Time of Concentration	0.032 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	337.00 ft
Is Paved?	False
Slope	0.095 ft/ft
Average Velocity	4.97 ft/s
Segment Time of Concentration	0.019 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.083 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: EDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: EDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	150.00 ft
Manning's n	0.240
Slope	0.020 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.13 ft/s
Segment Time of Concentration	0.318 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	1,058.00 ft
Is Paved?	False
Slope	0.048 ft/ft
Average Velocity	3.53 ft/s
Segment Time of Concentration	0.083 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.401 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: EDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$(L_f / V) / 3600$$

Where:

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V}$$
$$V = 16.1345 * (S_f^{0.5})$$

$$\text{Paved Surface:}$$
$$V = 20.3282 * (S_f^{0.5})$$
$$(L_f / V) / 3600$$

Where:

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: EDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	150.00 ft
Manning's n	0.240
Slope	0.080 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.23 ft/s
Segment Time of Concentration	0.182 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	1,054.00 ft
Is Paved?	False
Slope	0.062 ft/ft
Average Velocity	4.02 ft/s
Segment Time of Concentration	0.073 hours

Segment #3: TR-55 Channel Flow

Flow Area	7.0 ft ²
Hydraulic Length	910.00 ft
Manning's n	0.120
Slope	0.033 ft/ft
Wetted Perimeter	9.42 ft
Average Velocity	1.85 ft/s
Segment Time of Concentration	0.137 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.392 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: EDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \text{Unpaved surface:}$$
$$V = 16.1345 * (S_f^{0.5})$$

$$\text{Paved Surface:}$$
$$V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{0.007 * ((n * L_f)^{0.8})}{((P^{0.5}) * (S_f^{0.4}))}$$

Where:

Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: EDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	150.00 ft
Manning's n	0.240
Slope	0.100 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.25 ft/s
Segment Time of Concentration	0.167 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	634.00 ft
Is Paved?	False
Slope	0.095 ft/ft
Average Velocity	4.97 ft/s
Segment Time of Concentration	0.035 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.202 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: EDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$(L_f / V) / 3600$$

Where:

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V}$$
$$V = 16.1345 * (S_f^{0.5})$$

$$\text{Paved Surface:}$$
$$V = 20.3282 * (S_f^{0.5})$$

$$(L_f / V) / 3600$$

Where:

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Runoff CN-Area

Label: EDA-1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	129,980.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil D	80.000	13,009.000	0.0	0.0	80.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	96,495.000	0.0	0.0	61.000
Woods - good - Soil D	77.000	1,940.000	0.0	0.0	77.000
Woods - good - Soil B	55.000	14,395.000	0.0	0.0	55.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	255,819.000	(N/A)	(N/A)	80.549

Watershed

Subsection: Runoff CN-Area

Label: EDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	47,651.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	28,626.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil D	80.000	17,468.000	0.0	0.0	80.000
Woods - good - Soil B	55.000	32,109.000	0.0	0.0	55.000
Woods - good - Soil D	77.000	1,513.000	0.0	0.0	77.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	127,367.000	(N/A)	(N/A)	76.126

Watershed

Subsection: Runoff CN-Area

Label: EDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	202,305.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	163,009.000	0.0	0.0	74.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	1,067,085.000	0.0	0.0	61.000
Woods - good - Soil C	70.000	3,836.000	0.0	0.0	70.000
Woods - good - Soil B	55.000	69,717.000	0.0	0.0	55.000
Woods - grass combination - good - Soil B	58.000	61,711.000	0.0	0.0	58.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	1,567,663.000	(N/A)	(N/A)	66.764

Watershed

Subsection: Runoff CN-Area

Label: EDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	30,695.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	513,602.000	0.0	0.0	74.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	957,560.000	0.0	0.0	61.000
Woods - good - Soil C	70.000	33,413.000	0.0	0.0	70.000
Woods - good - Soil B	55.000	12,107.000	0.0	0.0	55.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	1,547,377.00 0	(N/A)	(N/A)	66.196

Watershed

Subsection: Runoff CN-Area

Label: EDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	22,923.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	54,686.000	0.0	0.0	74.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	458,325.000	0.0	0.0	61.000
Woods - good - Soil C	70.000	11,905.000	0.0	0.0	70.000
Woods - good - Soil B	55.000	51,400.000	0.0	0.0	55.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	599,239.000	(N/A)	(N/A)	63.266

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.02	296.07	296.18	296.30	296.41
12.250	296.45	296.45	296.43	296.41	296.39
12.500	296.35	296.31	296.27	296.23	296.20
12.750	296.19	296.17	296.16	296.16	296.15
13.000	296.14	296.14	296.13	296.13	296.12
13.250	296.12	296.12	296.12	296.12	296.11
13.500	296.11	296.11	296.11	296.11	296.11
13.750	296.11	296.11	296.10	296.10	296.10
14.000	296.10	296.10	296.10	296.09	296.09
14.250	296.09	296.09	296.09	296.09	296.09
14.500	296.09	296.09	296.09	296.09	296.08
14.750	296.08	296.08	296.08	296.08	296.08
15.000	296.08	296.08	296.08	296.08	296.08
15.250	296.08	296.07	296.07	296.07	296.07
15.500	296.07	296.07	296.07	296.07	296.07
15.750	296.07	296.06	296.06	296.06	296.06
16.000	296.06	296.06	296.06	296.06	296.06
16.250	296.06	296.05	296.05	296.05	296.05
16.500	296.05	296.05	296.05	296.05	296.05
16.750	296.05	296.05	296.05	296.05	296.05
17.000	296.05	296.05	296.05	296.05	296.05
17.250	296.05	296.05	296.05	296.04	296.04
17.500	296.04	296.04	296.04	296.04	296.04
17.750	296.04	296.04	296.04	296.04	296.04
18.000	296.04	296.04	296.04	296.04	296.04
18.250	296.04	296.04	296.04	296.04	296.04
18.500	296.04	296.04	296.04	296.04	296.04
18.750	296.04	296.03	296.03	296.03	296.03
19.000	296.03	296.03	296.03	296.03	296.03
19.250	296.03	296.03	296.03	296.03	296.03
19.500	296.03	296.03	296.03	296.03	296.03
19.750	296.03	296.03	296.03	296.03	296.03
20.000	296.03	296.03	296.03	296.03	296.03

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.03	296.03	296.03	296.03	296.03
20.500	296.03	296.03	296.03	296.03	296.03
20.750	296.03	296.03	296.03	296.03	296.03
21.000	296.03	296.03	296.03	296.03	296.03
21.250	296.03	296.03	296.03	296.03	296.03
21.500	296.03	296.03	296.03	296.03	296.03
21.750	296.03	296.03	296.03	296.03	296.03
22.000	296.03	296.03	296.03	296.03	296.03
22.250	296.03	296.03	296.03	296.03	296.03
22.500	296.03	296.03	296.03	296.03	296.03
22.750	296.02	296.02	296.02	296.02	296.02
23.000	296.02	296.02	296.02	296.02	296.02
23.250	296.02	296.02	296.02	296.02	296.02
23.500	296.02	296.02	296.02	296.02	296.02
23.750	296.02	296.02	296.02	296.02	296.02
24.000	296.02	296.02	296.02	296.01	296.01
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.02	296.05	296.10
12.000	296.22	296.40	296.53	296.58	296.60
12.250	296.61	296.59	296.58	296.56	296.55
12.500	296.53	296.51	296.48	296.41	296.35
12.750	296.32	296.30	296.28	296.26	296.25
13.000	296.24	296.23	296.22	296.21	296.21
13.250	296.20	296.20	296.19	296.19	296.19
13.500	296.19	296.18	296.18	296.18	296.18
13.750	296.17	296.17	296.17	296.17	296.16
14.000	296.16	296.16	296.15	296.15	296.15
14.250	296.15	296.15	296.14	296.14	296.14
14.500	296.14	296.14	296.14	296.14	296.13
14.750	296.13	296.13	296.13	296.13	296.13
15.000	296.13	296.12	296.12	296.12	296.12
15.250	296.12	296.12	296.12	296.11	296.11
15.500	296.11	296.11	296.11	296.11	296.10
15.750	296.10	296.10	296.10	296.10	296.10
16.000	296.09	296.09	296.09	296.09	296.09
16.250	296.09	296.09	296.08	296.08	296.08
16.500	296.08	296.08	296.08	296.08	296.08
16.750	296.08	296.08	296.08	296.08	296.08
17.000	296.07	296.07	296.07	296.07	296.07
17.250	296.07	296.07	296.07	296.07	296.07
17.500	296.07	296.07	296.07	296.06	296.06
17.750	296.06	296.06	296.06	296.06	296.06
18.000	296.06	296.06	296.06	296.06	296.06
18.250	296.06	296.06	296.06	296.06	296.05
18.500	296.05	296.05	296.05	296.05	296.05
18.750	296.05	296.05	296.05	296.05	296.05
19.000	296.05	296.05	296.05	296.05	296.05
19.250	296.05	296.05	296.05	296.05	296.05
19.500	296.05	296.05	296.05	296.05	296.05
19.750	296.05	296.05	296.05	296.05	296.05
20.000	296.05	296.05	296.05	296.05	296.05

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.05	296.05	296.05	296.05	296.05
20.500	296.05	296.05	296.05	296.05	296.05
20.750	296.04	296.04	296.04	296.04	296.04
21.000	296.04	296.04	296.04	296.04	296.04
21.250	296.04	296.04	296.04	296.04	296.04
21.500	296.04	296.04	296.04	296.04	296.04
21.750	296.04	296.04	296.04	296.04	296.04
22.000	296.04	296.04	296.04	296.04	296.04
22.250	296.04	296.04	296.04	296.04	296.04
22.500	296.04	296.04	296.04	296.04	296.04
22.750	296.04	296.04	296.04	296.04	296.04
23.000	296.04	296.04	296.04	296.04	296.04
23.250	296.04	296.04	296.04	296.04	296.03
23.500	296.03	296.03	296.03	296.03	296.03
23.750	296.03	296.03	296.03	296.03	296.03
24.000	296.03	296.03	296.03	296.02	296.01
24.250	296.01	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.01
11.000	296.01	296.01	296.02	296.03	296.03
11.250	296.04	296.05	296.06	296.07	296.08
11.500	296.09	296.11	296.13	296.17	296.22
11.750	296.29	296.39	296.50	296.54	296.58
12.000	296.66	296.77	296.90	297.00	297.02
12.250	297.00	296.94	296.88	296.82	296.78
12.500	296.73	296.68	296.64	296.60	296.57
12.750	296.55	296.54	296.53	296.52	296.52
13.000	296.51	296.50	296.50	296.48	296.46
13.250	296.45	296.44	296.43	296.42	296.42
13.500	296.41	296.40	296.40	296.39	296.39
13.750	296.38	296.37	296.37	296.36	296.35
14.000	296.35	296.34	296.33	296.33	296.32
14.250	296.32	296.31	296.31	296.31	296.30
14.500	296.30	296.30	296.29	296.29	296.29
14.750	296.28	296.28	296.28	296.27	296.27
15.000	296.27	296.26	296.26	296.26	296.25
15.250	296.25	296.25	296.24	296.24	296.23
15.500	296.23	296.23	296.22	296.22	296.22
15.750	296.21	296.21	296.21	296.20	296.20
16.000	296.19	296.19	296.19	296.18	296.18
16.250	296.18	296.18	296.18	296.17	296.17
16.500	296.17	296.17	296.17	296.17	296.16
16.750	296.16	296.16	296.16	296.16	296.16
17.000	296.15	296.15	296.15	296.15	296.15
17.250	296.15	296.14	296.14	296.14	296.14
17.500	296.14	296.14	296.13	296.13	296.13
17.750	296.13	296.13	296.13	296.12	296.12
18.000	296.12	296.12	296.12	296.12	296.12
18.250	296.11	296.11	296.11	296.11	296.11
18.500	296.11	296.11	296.11	296.11	296.11
18.750	296.11	296.11	296.11	296.11	296.11
19.000	296.11	296.11	296.11	296.11	296.10
19.250	296.10	296.10	296.10	296.10	296.10
19.500	296.10	296.10	296.10	296.10	296.10
19.750	296.10	296.10	296.10	296.10	296.10
20.000	296.10	296.10	296.10	296.10	296.09

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.09	296.09	296.09	296.09	296.09
20.500	296.09	296.09	296.09	296.09	296.09
20.750	296.09	296.09	296.09	296.09	296.09
21.000	296.09	296.09	296.09	296.09	296.09
21.250	296.09	296.09	296.09	296.09	296.09
21.500	296.08	296.08	296.08	296.08	296.08
21.750	296.08	296.08	296.08	296.08	296.08
22.000	296.08	296.08	296.08	296.08	296.08
22.250	296.08	296.08	296.08	296.08	296.08
22.500	296.08	296.08	296.08	296.08	296.08
22.750	296.08	296.07	296.07	296.07	296.07
23.000	296.07	296.07	296.07	296.07	296.07
23.250	296.07	296.07	296.07	296.07	296.07
23.500	296.07	296.07	296.07	296.07	296.07
23.750	296.07	296.07	296.07	296.07	296.07
24.000	296.06	296.06	296.05	296.04	296.02
24.250	296.01	296.01	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.01	296.01
10.250	296.01	296.02	296.02	296.03	296.03
10.500	296.04	296.04	296.05	296.06	296.06
10.750	296.07	296.08	296.08	296.09	296.10
11.000	296.11	296.12	296.12	296.14	296.15
11.250	296.16	296.18	296.20	296.22	296.24
11.500	296.27	296.29	296.34	296.40	296.50
11.750	296.53	296.57	296.62	296.67	296.75
12.000	296.87	297.02	297.11	297.18	297.20
12.250	297.17	297.12	297.07	297.03	296.98
12.500	296.90	296.83	296.76	296.70	296.66
12.750	296.63	296.61	296.60	296.58	296.57
13.000	296.57	296.56	296.55	296.54	296.54
13.250	296.53	296.53	296.53	296.53	296.52
13.500	296.52	296.52	296.52	296.52	296.51
13.750	296.51	296.51	296.51	296.51	296.50
14.000	296.50	296.49	296.48	296.47	296.47
14.250	296.46	296.45	296.45	296.44	296.44
14.500	296.43	296.43	296.42	296.42	296.41
14.750	296.41	296.40	296.40	296.39	296.39
15.000	296.38	296.38	296.37	296.37	296.36
15.250	296.36	296.35	296.35	296.34	296.34
15.500	296.33	296.33	296.32	296.32	296.31
15.750	296.31	296.30	296.30	296.29	296.28
16.000	296.28	296.27	296.27	296.26	296.26
16.250	296.26	296.25	296.25	296.25	296.25
16.500	296.24	296.24	296.24	296.24	296.23
16.750	296.23	296.23	296.23	296.23	296.22
17.000	296.22	296.22	296.22	296.21	296.21
17.250	296.21	296.21	296.20	296.20	296.20
17.500	296.20	296.19	296.19	296.19	296.19
17.750	296.18	296.18	296.18	296.18	296.17
18.000	296.17	296.17	296.17	296.17	296.16
18.250	296.16	296.16	296.16	296.16	296.16
18.500	296.16	296.16	296.16	296.16	296.16
18.750	296.15	296.15	296.15	296.15	296.15
19.000	296.15	296.15	296.15	296.15	296.15
19.250	296.15	296.15	296.15	296.15	296.14
19.500	296.14	296.14	296.14	296.14	296.14
19.750	296.14	296.14	296.14	296.14	296.14
20.000	296.14	296.14	296.14	296.13	296.13

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.13	296.13	296.13	296.13	296.13
20.500	296.13	296.13	296.13	296.13	296.13
20.750	296.13	296.13	296.13	296.13	296.13
21.000	296.13	296.13	296.12	296.12	296.12
21.250	296.12	296.12	296.12	296.12	296.12
21.500	296.12	296.12	296.12	296.12	296.12
21.750	296.12	296.12	296.12	296.12	296.12
22.000	296.11	296.11	296.11	296.11	296.11
22.250	296.11	296.11	296.11	296.11	296.11
22.500	296.11	296.11	296.11	296.11	296.11
22.750	296.11	296.11	296.10	296.10	296.10
23.000	296.10	296.10	296.10	296.10	296.10
23.250	296.10	296.10	296.10	296.10	296.10
23.500	296.10	296.10	296.10	296.10	296.10
23.750	296.09	296.09	296.09	296.09	296.09
24.000	296.09	296.09	296.08	296.06	296.03
24.250	296.02	296.01	296.01	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.01	296.01	296.01
9.000	296.02	296.02	296.03	296.03	296.04
9.250	296.04	296.05	296.05	296.06	296.06
9.500	296.07	296.07	296.08	296.08	296.09
9.750	296.10	296.10	296.11	296.12	296.12

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.13	296.14	296.15	296.15	296.16
10.250	296.17	296.18	296.19	296.20	296.22
10.500	296.23	296.24	296.25	296.27	296.28
10.750	296.29	296.31	296.32	296.34	296.35
11.000	296.37	296.38	296.40	296.43	296.45
11.250	296.49	296.51	296.51	296.52	296.53
11.500	296.54	296.56	296.58	296.61	296.66
11.750	296.72	296.80	296.90	297.00	297.07
12.000	297.17	297.33	297.49	297.56	297.57
12.250	297.53	297.44	297.35	297.27	297.20
12.500	297.13	297.07	297.01	296.92	296.85
12.750	296.80	296.76	296.74	296.72	296.70
13.000	296.69	296.67	296.66	296.65	296.64
13.250	296.63	296.62	296.62	296.62	296.61
13.500	296.61	296.61	296.60	296.60	296.59
13.750	296.59	296.59	296.58	296.58	296.58
14.000	296.57	296.57	296.56	296.56	296.56
14.250	296.56	296.55	296.55	296.55	296.55
14.500	296.55	296.54	296.54	296.54	296.54
14.750	296.54	296.54	296.53	296.53	296.53
15.000	296.53	296.53	296.52	296.52	296.52
15.250	296.52	296.52	296.52	296.51	296.51
15.500	296.51	296.51	296.51	296.50	296.50
15.750	296.50	296.49	296.48	296.47	296.46
16.000	296.45	296.45	296.44	296.43	296.42
16.250	296.42	296.41	296.41	296.40	296.40
16.500	296.40	296.39	296.39	296.39	296.38
16.750	296.38	296.37	296.37	296.37	296.36
17.000	296.36	296.35	296.35	296.35	296.34
17.250	296.34	296.33	296.33	296.33	296.32
17.500	296.32	296.31	296.31	296.31	296.30
17.750	296.30	296.29	296.29	296.29	296.28
18.000	296.28	296.27	296.27	296.27	296.26
18.250	296.26	296.26	296.26	296.26	296.26
18.500	296.26	296.25	296.25	296.25	296.25
18.750	296.25	296.25	296.25	296.25	296.25
19.000	296.24	296.24	296.24	296.24	296.24
19.250	296.24	296.24	296.24	296.23	296.23
19.500	296.23	296.23	296.23	296.23	296.23
19.750	296.23	296.23	296.22	296.22	296.22
20.000	296.22	296.22	296.22	296.22	296.22

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.22	296.21	296.21	296.21	296.21
20.500	296.21	296.21	296.21	296.21	296.21
20.750	296.21	296.20	296.20	296.20	296.20
21.000	296.20	296.20	296.20	296.20	296.20
21.250	296.20	296.20	296.20	296.19	296.19
21.500	296.19	296.19	296.19	296.19	296.19
21.750	296.19	296.19	296.19	296.19	296.18
22.000	296.18	296.18	296.18	296.18	296.18
22.250	296.18	296.18	296.18	296.18	296.17
22.500	296.17	296.17	296.17	296.17	296.17
22.750	296.17	296.17	296.17	296.17	296.17
23.000	296.17	296.16	296.16	296.16	296.16
23.250	296.16	296.16	296.16	296.16	296.16
23.500	296.16	296.16	296.15	296.15	296.15
23.750	296.15	296.15	296.15	296.15	296.15
24.000	296.15	296.14	296.12	296.09	296.05
24.250	296.03	296.02	296.01	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.17
10.750	319.17	319.17	319.17	319.17	319.17
11.000	319.17	319.17	319.17	319.17	319.17
11.250	319.17	319.17	319.17	319.17	319.17
11.500	319.17	319.17	319.17	319.17	319.17
11.750	319.17	319.17	319.17	319.17	319.17
12.000	319.17	319.18	319.19	319.20	319.22
12.250	319.24	319.27	319.29	319.32	319.35
12.500	319.38	319.40	319.42	319.44	319.46
12.750	319.48	319.49	319.50	319.51	319.52
13.000	319.53	319.54	319.55	319.55	319.56
13.250	319.56	319.57	319.57	319.58	319.58
13.500	319.59	319.59	319.59	319.60	319.60
13.750	319.60	319.61	319.61	319.61	319.61
14.000	319.62	319.62	319.62	319.62	319.63
14.250	319.63	319.63	319.63	319.63	319.63
14.500	319.64	319.64	319.64	319.64	319.64
14.750	319.64	319.64	319.64	319.64	319.65
15.000	319.65	319.65	319.65	319.65	319.65
15.250	319.65	319.65	319.65	319.65	319.65
15.500	319.65	319.65	319.65	319.65	319.65
15.750	319.65	319.65	319.65	319.65	319.65
16.000	319.65	319.65	319.65	319.65	319.65
16.250	319.65	319.65	319.64	319.64	319.64
16.500	319.64	319.64	319.64	319.64	319.64
16.750	319.64	319.64	319.64	319.63	319.63
17.000	319.63	319.63	319.63	319.63	319.63
17.250	319.63	319.63	319.62	319.62	319.62
17.500	319.62	319.62	319.62	319.62	319.62
17.750	319.61	319.61	319.61	319.61	319.61
18.000	319.61	319.61	319.60	319.60	319.60
18.250	319.60	319.60	319.60	319.60	319.59
18.500	319.59	319.59	319.59	319.59	319.59
18.750	319.59	319.58	319.58	319.58	319.58
19.000	319.58	319.58	319.58	319.57	319.57
19.250	319.57	319.57	319.57	319.57	319.57
19.500	319.57	319.56	319.56	319.56	319.56
19.750	319.56	319.56	319.56	319.55	319.55
20.000	319.55	319.55	319.55	319.55	319.55

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.55	319.54	319.54	319.54	319.54
20.500	319.54	319.54	319.54	319.54	319.53
20.750	319.53	319.53	319.53	319.53	319.53
21.000	319.53	319.53	319.53	319.52	319.52
21.250	319.52	319.52	319.52	319.52	319.52
21.500	319.52	319.52	319.51	319.51	319.51
21.750	319.51	319.51	319.51	319.51	319.51
22.000	319.51	319.50	319.50	319.50	319.50
22.250	319.50	319.50	319.50	319.50	319.50
22.500	319.49	319.49	319.49	319.49	319.49
22.750	319.49	319.49	319.49	319.49	319.49
23.000	319.48	319.48	319.48	319.48	319.48
23.250	319.48	319.48	319.48	319.48	319.48
23.500	319.48	319.47	319.47	319.47	319.47
23.750	319.47	319.47	319.47	319.47	319.47
24.000	319.47	319.46	319.46	319.46	319.46
24.250	319.46	319.46	319.46	319.46	319.45
24.500	319.45	319.45	319.45	319.45	319.44
24.750	319.44	319.44	319.44	319.44	319.43
25.000	319.43	319.43	319.43	319.43	319.43
25.250	319.42	319.42	319.42	319.42	319.42
25.500	319.42	319.41	319.41	319.41	319.41
25.750	319.41	319.41	319.40	319.40	319.40
26.000	319.40	319.40	319.40	319.40	319.39
26.250	319.39	319.39	319.39	319.39	319.39
26.500	319.39	319.38	319.38	319.38	319.38
26.750	319.38	319.38	319.38	319.38	319.37
27.000	319.37	319.37	319.37	319.37	319.37
27.250	319.37	319.37	319.37	319.36	319.36
27.500	319.36	319.36	319.36	319.36	319.36
27.750	319.36	319.36	319.35	319.35	319.35
28.000	319.35	319.35	319.35	319.35	319.35
28.250	319.35	319.35	319.35	319.34	319.34
28.500	319.34	319.34	319.34	319.34	319.34
28.750	319.34	319.34	319.34	319.34	319.33
29.000	319.33	319.33	319.33	319.33	319.33
29.250	319.33	319.33	319.33	319.33	319.33
29.500	319.33	319.33	319.33	319.32	319.32
29.750	319.32	319.32	319.32	319.32	319.32
30.000	319.32	319.32	319.32	319.32	319.32
30.250	319.32	319.32	319.32	319.31	319.31

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.31	319.31	319.31	319.31	319.31
30.750	319.31	319.31	319.31	319.31	319.31
31.000	319.31	319.31	319.31	319.31	319.31
31.250	319.31	319.31	319.30	319.30	319.30
31.500	319.30	319.30	319.30	319.30	319.30
31.750	319.30	319.30	319.30	319.30	319.30
32.000	319.30	319.30	319.30	319.30	319.30
32.250	319.30	319.30	319.30	319.30	319.30
32.500	319.30	319.29	319.29	319.29	319.29
32.750	319.29	319.29	319.29	319.29	319.29
33.000	319.29	319.29	319.29	319.29	319.29
33.250	319.29	319.29	319.29	319.29	319.29
33.500	319.29	319.29	319.29	319.29	319.29
33.750	319.29	319.29	319.29	319.29	319.29
34.000	319.29	319.29	319.28	319.28	319.28
34.250	319.28	319.28	319.28	319.28	319.28
34.500	319.28	319.28	319.28	319.28	319.28
34.750	319.28	319.28	319.28	319.28	319.28
35.000	319.28	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.17
10.750	319.17	319.17	319.17	319.17	319.17
11.000	319.17	319.17	319.17	319.17	319.17
11.250	319.17	319.17	319.17	319.17	319.17
11.500	319.17	319.17	319.17	319.17	319.17
11.750	319.17	319.17	319.18	319.18	319.19
12.000	319.20	319.21	319.24	319.27	319.31
12.250	319.35	319.40	319.45	319.50	319.55
12.500	319.60	319.64	319.68	319.71	319.73
12.750	319.75	319.77	319.79	319.80	319.81
13.000	319.82	319.83	319.84	319.84	319.85
13.250	319.85	319.85	319.86	319.86	319.86
13.500	319.86	319.87	319.87	319.87	319.87
13.750	319.87	319.87	319.87	319.87	319.87
14.000	319.87	319.87	319.87	319.87	319.87
14.250	319.87	319.87	319.87	319.87	319.86
14.500	319.86	319.86	319.86	319.86	319.86
14.750	319.86	319.86	319.85	319.85	319.85
15.000	319.85	319.85	319.85	319.84	319.84
15.250	319.84	319.84	319.84	319.83	319.83
15.500	319.83	319.83	319.83	319.82	319.82
15.750	319.82	319.82	319.82	319.81	319.81
16.000	319.81	319.81	319.80	319.80	319.80
16.250	319.80	319.79	319.79	319.79	319.79
16.500	319.78	319.78	319.78	319.78	319.77
16.750	319.77	319.77	319.77	319.76	319.76
17.000	319.76	319.76	319.75	319.75	319.75
17.250	319.75	319.75	319.74	319.74	319.74
17.500	319.74	319.73	319.73	319.73	319.73
17.750	319.73	319.72	319.72	319.72	319.72
18.000	319.72	319.71	319.71	319.71	319.71
18.250	319.71	319.70	319.70	319.70	319.70
18.500	319.70	319.69	319.69	319.69	319.69
18.750	319.69	319.69	319.68	319.68	319.68
19.000	319.68	319.68	319.68	319.67	319.67
19.250	319.67	319.67	319.67	319.67	319.67
19.500	319.66	319.66	319.66	319.66	319.66
19.750	319.66	319.65	319.65	319.65	319.65
20.000	319.65	319.65	319.65	319.64	319.64

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.64	319.64	319.64	319.64	319.64
20.500	319.63	319.63	319.63	319.63	319.63
20.750	319.63	319.63	319.62	319.62	319.62
21.000	319.62	319.62	319.62	319.62	319.61
21.250	319.61	319.61	319.61	319.61	319.61
21.500	319.61	319.61	319.60	319.60	319.60
21.750	319.60	319.60	319.60	319.60	319.59
22.000	319.59	319.59	319.59	319.59	319.59
22.250	319.59	319.59	319.58	319.58	319.58
22.500	319.58	319.58	319.58	319.58	319.58
22.750	319.57	319.57	319.57	319.57	319.57
23.000	319.57	319.57	319.57	319.56	319.56
23.250	319.56	319.56	319.56	319.56	319.56
23.500	319.56	319.55	319.55	319.55	319.55
23.750	319.55	319.55	319.55	319.55	319.54
24.000	319.54	319.54	319.54	319.54	319.54
24.250	319.54	319.53	319.53	319.53	319.53
24.500	319.52	319.52	319.52	319.52	319.51
24.750	319.51	319.51	319.51	319.50	319.50
25.000	319.50	319.50	319.49	319.49	319.49
25.250	319.49	319.48	319.48	319.48	319.48
25.500	319.48	319.47	319.47	319.47	319.47
25.750	319.46	319.46	319.46	319.46	319.46
26.000	319.45	319.45	319.45	319.45	319.45
26.250	319.44	319.44	319.44	319.44	319.44
26.500	319.43	319.43	319.43	319.43	319.43
26.750	319.43	319.42	319.42	319.42	319.42
27.000	319.42	319.41	319.41	319.41	319.41
27.250	319.41	319.41	319.41	319.40	319.40
27.500	319.40	319.40	319.40	319.40	319.39
27.750	319.39	319.39	319.39	319.39	319.39
28.000	319.39	319.39	319.38	319.38	319.38
28.250	319.38	319.38	319.38	319.38	319.38
28.500	319.37	319.37	319.37	319.37	319.37
28.750	319.37	319.37	319.37	319.36	319.36
29.000	319.36	319.36	319.36	319.36	319.36
29.250	319.36	319.36	319.36	319.35	319.35
29.500	319.35	319.35	319.35	319.35	319.35
29.750	319.35	319.35	319.35	319.34	319.34
30.000	319.34	319.34	319.34	319.34	319.34
30.250	319.34	319.34	319.34	319.34	319.34

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.33	319.33	319.33	319.33	319.33
30.750	319.33	319.33	319.33	319.33	319.33
31.000	319.33	319.33	319.33	319.32	319.32
31.250	319.32	319.32	319.32	319.32	319.32
31.500	319.32	319.32	319.32	319.32	319.32
31.750	319.32	319.32	319.32	319.32	319.31
32.000	319.31	319.31	319.31	319.31	319.31
32.250	319.31	319.31	319.31	319.31	319.31
32.500	319.31	319.31	319.31	319.31	319.31
32.750	319.31	319.31	319.31	319.30	319.30
33.000	319.30	319.30	319.30	319.30	319.30
33.250	319.30	319.30	319.30	319.30	319.30
33.500	319.30	319.30	319.30	319.30	319.30
33.750	319.30	319.30	319.30	319.30	319.30
34.000	319.30	319.29	319.29	319.29	319.29
34.250	319.29	319.29	319.29	319.29	319.29
34.500	319.29	319.29	319.29	319.29	319.29
34.750	319.29	319.29	319.29	319.29	319.29
35.000	319.29	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.17
10.750	319.18	319.18	319.18	319.18	319.18
11.000	319.18	319.19	319.19	319.19	319.20
11.250	319.20	319.21	319.21	319.22	319.22
11.500	319.23	319.24	319.24	319.25	319.26
11.750	319.28	319.29	319.31	319.34	319.37
12.000	319.41	319.47	319.54	319.63	319.73
12.250	319.84	319.96	320.08	320.19	320.27
12.500	320.35	320.40	320.44	320.46	320.47
12.750	320.48	320.48	320.47	320.47	320.46
13.000	320.44	320.43	320.42	320.41	320.40
13.250	320.39	320.38	320.37	320.36	320.36
13.500	320.35	320.34	320.34	320.33	320.33
13.750	320.32	320.32	320.31	320.31	320.31
14.000	320.30	320.30	320.30	320.30	320.29
14.250	320.29	320.29	320.28	320.28	320.27
14.500	320.27	320.27	320.26	320.26	320.26
14.750	320.25	320.25	320.24	320.24	320.23
15.000	320.23	320.23	320.22	320.22	320.21
15.250	320.21	320.20	320.20	320.20	320.19
15.500	320.19	320.18	320.18	320.17	320.17
15.750	320.16	320.16	320.15	320.15	320.14
16.000	320.13	320.13	320.12	320.12	320.11
16.250	320.11	320.10	320.10	320.09	320.08
16.500	320.08	320.07	320.07	320.06	320.06
16.750	320.05	320.05	320.04	320.04	320.03
17.000	320.02	320.02	320.01	320.01	320.00
17.250	320.00	319.99	319.99	319.99	319.98
17.500	319.98	319.97	319.97	319.96	319.96
17.750	319.95	319.95	319.94	319.94	319.93
18.000	319.93	319.93	319.92	319.92	319.91
18.250	319.91	319.90	319.90	319.90	319.89
18.500	319.89	319.88	319.88	319.88	319.87
18.750	319.87	319.87	319.86	319.86	319.85
19.000	319.85	319.85	319.84	319.84	319.84
19.250	319.84	319.83	319.83	319.83	319.82
19.500	319.82	319.82	319.82	319.81	319.81
19.750	319.81	319.80	319.80	319.80	319.80
20.000	319.79	319.79	319.79	319.79	319.79

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.78	319.78	319.78	319.78	319.77
20.500	319.77	319.77	319.77	319.77	319.76
20.750	319.76	319.76	319.76	319.76	319.75
21.000	319.75	319.75	319.75	319.75	319.75
21.250	319.74	319.74	319.74	319.74	319.74
21.500	319.74	319.73	319.73	319.73	319.73
21.750	319.73	319.73	319.73	319.72	319.72
22.000	319.72	319.72	319.72	319.72	319.72
22.250	319.71	319.71	319.71	319.71	319.71
22.500	319.71	319.71	319.71	319.70	319.70
22.750	319.70	319.70	319.70	319.70	319.70
23.000	319.70	319.69	319.69	319.69	319.69
23.250	319.69	319.69	319.69	319.69	319.69
23.500	319.68	319.68	319.68	319.68	319.68
23.750	319.68	319.68	319.68	319.68	319.68
24.000	319.67	319.67	319.67	319.67	319.67
24.250	319.67	319.66	319.66	319.66	319.66
24.500	319.65	319.65	319.64	319.64	319.64
24.750	319.63	319.63	319.63	319.62	319.62
25.000	319.61	319.61	319.61	319.60	319.60
25.250	319.60	319.59	319.59	319.59	319.58
25.500	319.58	319.57	319.57	319.57	319.57
25.750	319.56	319.56	319.56	319.55	319.55
26.000	319.55	319.54	319.54	319.54	319.53
26.250	319.53	319.53	319.53	319.52	319.52
26.500	319.52	319.51	319.51	319.51	319.51
26.750	319.50	319.50	319.50	319.50	319.49
27.000	319.49	319.49	319.49	319.48	319.48
27.250	319.48	319.48	319.48	319.47	319.47
27.500	319.47	319.47	319.46	319.46	319.46
27.750	319.46	319.46	319.45	319.45	319.45
28.000	319.45	319.45	319.44	319.44	319.44
28.250	319.44	319.44	319.43	319.43	319.43
28.500	319.43	319.43	319.43	319.42	319.42
28.750	319.42	319.42	319.42	319.42	319.41
29.000	319.41	319.41	319.41	319.41	319.41
29.250	319.40	319.40	319.40	319.40	319.40
29.500	319.40	319.40	319.39	319.39	319.39
29.750	319.39	319.39	319.39	319.39	319.38
30.000	319.38	319.38	319.38	319.38	319.38
30.250	319.38	319.38	319.37	319.37	319.37

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.37	319.37	319.37	319.37	319.37
30.750	319.36	319.36	319.36	319.36	319.36
31.000	319.36	319.36	319.36	319.36	319.36
31.250	319.35	319.35	319.35	319.35	319.35
31.500	319.35	319.35	319.35	319.35	319.35
31.750	319.34	319.34	319.34	319.34	319.34
32.000	319.34	319.34	319.34	319.34	319.34
32.250	319.34	319.34	319.33	319.33	319.33
32.500	319.33	319.33	319.33	319.33	319.33
32.750	319.33	319.33	319.33	319.33	319.33
33.000	319.32	319.32	319.32	319.32	319.32
33.250	319.32	319.32	319.32	319.32	319.32
33.500	319.32	319.32	319.32	319.32	319.32
33.750	319.32	319.31	319.31	319.31	319.31
34.000	319.31	319.31	319.31	319.31	319.31
34.250	319.31	319.31	319.31	319.31	319.31
34.500	319.31	319.31	319.31	319.31	319.31
34.750	319.30	319.30	319.30	319.30	319.30
35.000	319.30	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.18	319.18

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.18	319.18	319.18	319.18	319.19
10.250	319.19	319.19	319.19	319.20	319.20
10.500	319.20	319.21	319.21	319.22	319.22
10.750	319.23	319.23	319.24	319.25	319.25
11.000	319.26	319.27	319.28	319.29	319.29
11.250	319.30	319.31	319.32	319.34	319.35
11.500	319.36	319.38	319.39	319.41	319.43
11.750	319.45	319.48	319.52	319.56	319.62
12.000	319.69	319.77	319.88	320.01	320.17
12.250	320.32	320.45	320.56	320.65	320.71
12.500	320.73	320.74	320.73	320.72	320.70
12.750	320.69	320.67	320.66	320.64	320.61
13.000	320.59	320.57	320.55	320.53	320.51
13.250	320.49	320.48	320.46	320.45	320.43
13.500	320.42	320.41	320.40	320.39	320.39
13.750	320.38	320.37	320.37	320.36	320.36
14.000	320.35	320.35	320.34	320.34	320.34
14.250	320.33	320.33	320.33	320.33	320.32
14.500	320.32	320.32	320.32	320.31	320.31
14.750	320.31	320.31	320.31	320.30	320.30
15.000	320.30	320.30	320.30	320.30	320.29
15.250	320.29	320.29	320.29	320.29	320.28
15.500	320.28	320.28	320.27	320.27	320.27
15.750	320.26	320.26	320.26	320.25	320.25
16.000	320.24	320.24	320.23	320.23	320.23
16.250	320.22	320.22	320.21	320.21	320.20
16.500	320.20	320.19	320.19	320.18	320.18
16.750	320.17	320.17	320.16	320.16	320.15
17.000	320.15	320.14	320.14	320.13	320.13
17.250	320.12	320.12	320.11	320.11	320.10
17.500	320.10	320.09	320.09	320.08	320.08
17.750	320.07	320.07	320.06	320.06	320.05
18.000	320.05	320.04	320.04	320.03	320.03
18.250	320.02	320.02	320.01	320.01	320.00
18.500	320.00	320.00	319.99	319.99	319.98
18.750	319.98	319.97	319.97	319.97	319.96
19.000	319.96	319.95	319.95	319.95	319.94
19.250	319.94	319.94	319.93	319.93	319.92
19.500	319.92	319.92	319.91	319.91	319.91
19.750	319.90	319.90	319.90	319.90	319.89
20.000	319.89	319.89	319.88	319.88	319.88

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.88	319.87	319.87	319.87	319.86
20.500	319.86	319.86	319.86	319.85	319.85
20.750	319.85	319.85	319.85	319.84	319.84
21.000	319.84	319.84	319.83	319.83	319.83
21.250	319.83	319.83	319.82	319.82	319.82
21.500	319.82	319.82	319.81	319.81	319.81
21.750	319.81	319.81	319.80	319.80	319.80
22.000	319.80	319.80	319.79	319.79	319.79
22.250	319.79	319.79	319.79	319.78	319.78
22.500	319.78	319.78	319.78	319.78	319.78
22.750	319.77	319.77	319.77	319.77	319.77
23.000	319.77	319.76	319.76	319.76	319.76
23.250	319.76	319.76	319.76	319.75	319.75
23.500	319.75	319.75	319.75	319.75	319.75
23.750	319.74	319.74	319.74	319.74	319.74
24.000	319.74	319.74	319.73	319.73	319.73
24.250	319.73	319.72	319.72	319.72	319.71
24.500	319.71	319.70	319.70	319.69	319.69
24.750	319.69	319.68	319.68	319.67	319.67
25.000	319.66	319.66	319.66	319.65	319.65
25.250	319.64	319.64	319.64	319.63	319.63
25.500	319.62	319.62	319.62	319.61	319.61
25.750	319.61	319.60	319.60	319.59	319.59
26.000	319.59	319.58	319.58	319.58	319.57
26.250	319.57	319.57	319.56	319.56	319.56
26.500	319.55	319.55	319.55	319.55	319.54
26.750	319.54	319.54	319.53	319.53	319.53
27.000	319.53	319.52	319.52	319.52	319.51
27.250	319.51	319.51	319.51	319.50	319.50
27.500	319.50	319.50	319.49	319.49	319.49
27.750	319.49	319.48	319.48	319.48	319.48
28.000	319.47	319.47	319.47	319.47	319.47
28.250	319.46	319.46	319.46	319.46	319.46
28.500	319.45	319.45	319.45	319.45	319.45
28.750	319.44	319.44	319.44	319.44	319.44
29.000	319.43	319.43	319.43	319.43	319.43
29.250	319.42	319.42	319.42	319.42	319.42
29.500	319.42	319.41	319.41	319.41	319.41
29.750	319.41	319.41	319.41	319.40	319.40
30.000	319.40	319.40	319.40	319.40	319.39
30.250	319.39	319.39	319.39	319.39	319.39

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.39	319.39	319.38	319.38	319.38
30.750	319.38	319.38	319.38	319.38	319.38
31.000	319.37	319.37	319.37	319.37	319.37
31.250	319.37	319.37	319.37	319.36	319.36
31.500	319.36	319.36	319.36	319.36	319.36
31.750	319.36	319.36	319.35	319.35	319.35
32.000	319.35	319.35	319.35	319.35	319.35
32.250	319.35	319.35	319.35	319.34	319.34
32.500	319.34	319.34	319.34	319.34	319.34
32.750	319.34	319.34	319.34	319.34	319.34
33.000	319.33	319.33	319.33	319.33	319.33
33.250	319.33	319.33	319.33	319.33	319.33
33.500	319.33	319.33	319.33	319.32	319.32
33.750	319.32	319.32	319.32	319.32	319.32
34.000	319.32	319.32	319.32	319.32	319.32
34.250	319.32	319.32	319.32	319.32	319.31
34.500	319.31	319.31	319.31	319.31	319.31
34.750	319.31	319.31	319.31	319.31	319.31
35.000	319.31	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.18	319.18	319.18
8.750	319.18	319.18	319.18	319.19	319.19
9.000	319.19	319.20	319.20	319.20	319.21
9.250	319.21	319.21	319.22	319.22	319.23
9.500	319.23	319.24	319.24	319.25	319.26
9.750	319.26	319.27	319.28	319.29	319.29

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.30	319.31	319.32	319.33	319.34
10.250	319.35	319.36	319.37	319.38	319.39
10.500	319.40	319.41	319.43	319.44	319.45
10.750	319.47	319.48	319.50	319.51	319.53
11.000	319.55	319.56	319.58	319.60	319.62
11.250	319.64	319.66	319.69	319.71	319.73
11.500	319.76	319.79	319.82	319.85	319.89
11.750	319.93	319.98	320.05	320.13	320.22
12.000	320.32	320.43	320.56	320.70	320.81
12.250	320.89	320.94	320.97	320.98	320.97
12.500	320.95	320.92	320.88	320.85	320.82
12.750	320.79	320.76	320.74	320.71	320.70
13.000	320.68	320.67	320.66	320.64	320.63
13.250	320.61	320.59	320.58	320.56	320.55
13.500	320.53	320.52	320.51	320.50	320.49
13.750	320.48	320.47	320.47	320.46	320.45
14.000	320.44	320.44	320.43	320.43	320.42
14.250	320.42	320.41	320.41	320.40	320.40
14.500	320.40	320.39	320.39	320.38	320.38
14.750	320.38	320.38	320.37	320.37	320.37
15.000	320.37	320.36	320.36	320.36	320.36
15.250	320.35	320.35	320.35	320.35	320.35
15.500	320.34	320.34	320.34	320.34	320.33
15.750	320.33	320.33	320.33	320.33	320.32
16.000	320.32	320.32	320.32	320.32	320.31
16.250	320.31	320.31	320.31	320.31	320.30
16.500	320.30	320.30	320.30	320.30	320.29
16.750	320.29	320.29	320.29	320.29	320.28
17.000	320.28	320.28	320.28	320.27	320.27
17.250	320.27	320.26	320.26	320.26	320.25
17.500	320.25	320.25	320.24	320.24	320.24
17.750	320.23	320.23	320.23	320.22	320.22
18.000	320.21	320.21	320.21	320.20	320.20
18.250	320.19	320.19	320.19	320.18	320.18
18.500	320.17	320.17	320.16	320.16	320.16
18.750	320.15	320.15	320.14	320.14	320.13
19.000	320.13	320.13	320.12	320.12	320.11
19.250	320.11	320.11	320.10	320.10	320.10
19.500	320.09	320.09	320.09	320.08	320.08
19.750	320.07	320.07	320.07	320.06	320.06
20.000	320.06	320.05	320.05	320.05	320.04

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	320.04	320.04	320.03	320.03	320.03
20.500	320.03	320.02	320.02	320.02	320.01
20.750	320.01	320.01	320.01	320.00	320.00
21.000	320.00	319.99	319.99	319.99	319.99
21.250	319.98	319.98	319.98	319.98	319.97
21.500	319.97	319.97	319.97	319.96	319.96
21.750	319.96	319.96	319.95	319.95	319.95
22.000	319.95	319.94	319.94	319.94	319.94
22.250	319.93	319.93	319.93	319.93	319.93
22.500	319.92	319.92	319.92	319.92	319.91
22.750	319.91	319.91	319.91	319.91	319.90
23.000	319.90	319.90	319.90	319.90	319.89
23.250	319.89	319.89	319.89	319.89	319.88
23.500	319.88	319.88	319.88	319.88	319.87
23.750	319.87	319.87	319.87	319.87	319.86
24.000	319.86	319.86	319.86	319.85	319.85
24.250	319.85	319.84	319.84	319.83	319.82
24.500	319.82	319.81	319.80	319.80	319.79
24.750	319.78	319.78	319.77	319.76	319.76
25.000	319.75	319.75	319.74	319.73	319.73
25.250	319.72	319.72	319.71	319.71	319.70
25.500	319.70	319.69	319.69	319.68	319.68
25.750	319.67	319.67	319.67	319.66	319.66
26.000	319.65	319.65	319.65	319.64	319.64
26.250	319.63	319.63	319.63	319.62	319.62
26.500	319.61	319.61	319.61	319.60	319.60
26.750	319.60	319.59	319.59	319.59	319.58
27.000	319.58	319.58	319.57	319.57	319.57
27.250	319.56	319.56	319.56	319.55	319.55
27.500	319.55	319.54	319.54	319.54	319.53
27.750	319.53	319.53	319.53	319.52	319.52
28.000	319.52	319.52	319.51	319.51	319.51
28.250	319.50	319.50	319.50	319.50	319.49
28.500	319.49	319.49	319.49	319.48	319.48
28.750	319.48	319.48	319.48	319.47	319.47
29.000	319.47	319.47	319.46	319.46	319.46
29.250	319.46	319.46	319.45	319.45	319.45
29.500	319.45	319.45	319.44	319.44	319.44
29.750	319.44	319.44	319.43	319.43	319.43
30.000	319.43	319.43	319.43	319.42	319.42
30.250	319.42	319.42	319.42	319.42	319.41

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.41	319.41	319.41	319.41	319.41
30.750	319.40	319.40	319.40	319.40	319.40
31.000	319.40	319.40	319.39	319.39	319.39
31.250	319.39	319.39	319.39	319.39	319.38
31.500	319.38	319.38	319.38	319.38	319.38
31.750	319.38	319.38	319.37	319.37	319.37
32.000	319.37	319.37	319.37	319.37	319.37
32.250	319.36	319.36	319.36	319.36	319.36
32.500	319.36	319.36	319.36	319.36	319.36
32.750	319.35	319.35	319.35	319.35	319.35
33.000	319.35	319.35	319.35	319.35	319.35
33.250	319.34	319.34	319.34	319.34	319.34
33.500	319.34	319.34	319.34	319.34	319.34
33.750	319.34	319.34	319.33	319.33	319.33
34.000	319.33	319.33	319.33	319.33	319.33
34.250	319.33	319.33	319.33	319.33	319.33
34.500	319.32	319.32	319.32	319.32	319.32
34.750	319.32	319.32	319.32	319.32	319.32
35.000	319.32	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	205.739	597.837	0.00	3.02	5.30
297.00	17.07	622.716	1,094.949	0.00	17.07	23.99
297.50	47.04	1,325.568	1,741.337	0.00	47.04	61.77
298.00	96.57	2,388.932	2,537.000	0.00	96.57	123.11
298.50	180.43	3,771.140	2,998.247	0.00	180.43	222.33
299.00	290.21	5,393.596	3,497.996	0.00	290.21	350.14
299.50	428.77	7,275.552	4,036.247	0.00	428.77	509.61
300.00	598.36	9,436.260	4,613.000	0.00	598.36	703.21

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	205.739	597.837	0.00	3.02	5.30
297.00	17.07	622.716	1,094.949	0.00	17.07	23.99
297.50	47.04	1,325.568	1,741.337	0.00	47.04	61.77
298.00	96.57	2,388.932	2,537.000	0.00	96.57	123.11
298.50	180.43	3,771.140	2,998.247	0.00	180.43	222.33
299.00	290.21	5,393.596	3,497.996	0.00	290.21	350.14
299.50	428.77	7,275.552	4,036.247	0.00	428.77	509.61
300.00	598.36	9,436.260	4,613.000	0.00	598.36	703.21

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	205.739	597.837	0.00	3.02	5.30
297.00	17.07	622.716	1,094.949	0.00	17.07	23.99
297.50	47.04	1,325.568	1,741.337	0.00	47.04	61.77
298.00	96.57	2,388.932	2,537.000	0.00	96.57	123.11
298.50	180.43	3,771.140	2,998.247	0.00	180.43	222.33
299.00	290.21	5,393.596	3,497.996	0.00	290.21	350.14
299.50	428.77	7,275.552	4,036.247	0.00	428.77	509.61
300.00	598.36	9,436.260	4,613.000	0.00	598.36	703.21

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	205.739	597.837	0.00	3.02	5.30
297.00	17.07	622.716	1,094.949	0.00	17.07	23.99
297.50	47.04	1,325.568	1,741.337	0.00	47.04	61.77
298.00	96.57	2,388.932	2,537.000	0.00	96.57	123.11
298.50	180.43	3,771.140	2,998.247	0.00	180.43	222.33
299.00	290.21	5,393.596	3,497.996	0.00	290.21	350.14
299.50	428.77	7,275.552	4,036.247	0.00	428.77	509.61
300.00	598.36	9,436.260	4,613.000	0.00	598.36	703.21

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	205.739	597.837	0.00	3.02	5.30
297.00	17.07	622.716	1,094.949	0.00	17.07	23.99
297.50	47.04	1,325.568	1,741.337	0.00	47.04	61.77
298.00	96.57	2,388.932	2,537.000	0.00	96.57	123.11
298.50	180.43	3,771.140	2,998.247	0.00	180.43	222.33
299.00	290.21	5,393.596	3,497.996	0.00	290.21	350.14
299.50	428.77	7,275.552	4,036.247	0.00	428.77	509.61
300.00	598.36	9,436.260	4,613.000	0.00	598.36	703.21

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

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Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

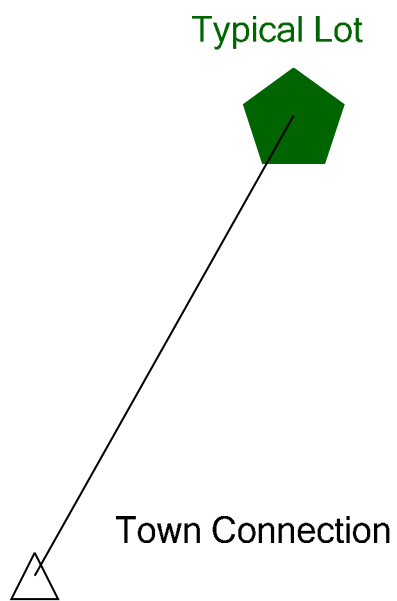


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Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Typical Lot	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.100	0.59

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Town Connection	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.100	0.59

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time-Depth Curve: TypeIII 24hr (6.4 in)

Label	TypeIII 24hr (6.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.2
2.500	0.2	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.3	0.3
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.3	0.3	0.3	0.4
5.000	0.4	0.4	0.4	0.4	0.4
5.500	0.4	0.4	0.4	0.4	0.4
6.000	0.5	0.5	0.5	0.5	0.5
6.500	0.5	0.5	0.5	0.6	0.6
7.000	0.6	0.6	0.6	0.6	0.6
7.500	0.6	0.7	0.7	0.7	0.7
8.000	0.7	0.7	0.8	0.8	0.8
8.500	0.8	0.8	0.9	0.9	0.9
9.000	0.9	1.0	1.0	1.0	1.0
9.500	1.1	1.1	1.1	1.1	1.2
10.000	1.2	1.2	1.3	1.3	1.3
10.500	1.4	1.4	1.5	1.5	1.5
11.000	1.6	1.6	1.7	1.8	1.8
11.500	1.9	2.0	2.2	2.4	2.7
12.000	3.2	3.7	4.0	4.2	4.4
12.500	4.5	4.6	4.6	4.7	4.7
13.000	4.8	4.8	4.9	4.9	5.0
13.500	5.0	5.0	5.1	5.1	5.1
14.000	5.2	5.2	5.2	5.3	5.3
14.500	5.3	5.3	5.4	5.4	5.4
15.000	5.5	5.5	5.5	5.5	5.5
15.500	5.6	5.6	5.6	5.6	5.6
16.000	5.7	5.7	5.7	5.7	5.7

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	5.7	5.7	5.8	5.8	5.8
17.000	5.8	5.8	5.8	5.8	5.9
17.500	5.9	5.9	5.9	5.9	5.9
18.000	5.9	5.9	5.9	6.0	6.0
18.500	6.0	6.0	6.0	6.0	6.0
19.000	6.0	6.0	6.0	6.0	6.1
19.500	6.1	6.1	6.1	6.1	6.1
20.000	6.1	6.1	6.1	6.1	6.1
20.500	6.1	6.2	6.2	6.2	6.2
21.000	6.2	6.2	6.2	6.2	6.2
21.500	6.2	6.2	6.2	6.2	6.3
22.000	6.3	6.3	6.3	6.3	6.3
22.500	6.3	6.3	6.3	6.3	6.3
23.000	6.3	6.3	6.3	6.3	6.3
23.500	6.4	6.4	6.4	6.4	6.4
24.000	6.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Runoff CN-Area

Label: Typical Lot

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	4,800.000	0.0	0.0	98.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	4,800.000	(N/A)	(N/A)	98.000

Watershed

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APPENDIX B

PROPOSED HYDROLOGIC CALCULATIONS

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

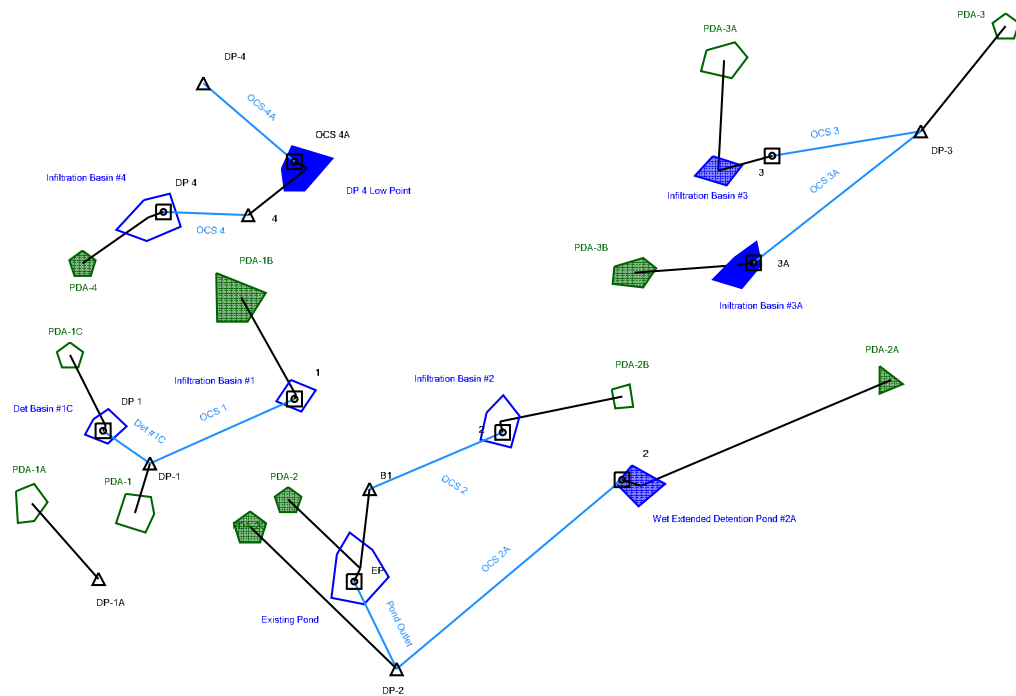


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Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
PDA-4	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	30,181.000	12.150	7.31
PDA-4	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	47,651.000	12.100	12.18
PDA-4	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	103,842.000	12.100	28.35
PDA-4	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	153,529.000	12.100	42.36
PDA-4	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	262,521.000	12.100	72.29
PDA-1C	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	16,641.000	12.100	4.55
PDA-1C	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	24,143.000	12.100	6.72
PDA-1C	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	46,606.000	12.100	13.05
PDA-1C	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	65,466.000	12.100	18.21
PDA-1C	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	105,365.000	12.100	28.77
PDA-2	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	16,103.000	12.150	3.78
PDA-2	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	25,750.000	12.150	6.50
PDA-2	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	57,099.000	12.100	15.15
PDA-2	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	85,024.000	12.100	22.93
PDA-2	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	146,597.000	12.100	39.67

Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
PDA-3	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	8,825.000	12.200	1.43
PDA-3	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	15,504.000	12.150	3.19
PDA-3	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	38,800.000	12.150	9.64
PDA-3	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	60,593.000	12.150	15.55
PDA-3	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	110,342.000	12.150	28.75
PDA-2A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	27,246.000	12.150	6.81
PDA-2A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	40,683.000	12.150	10.49
PDA-2A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	81,934.000	12.150	21.49
PDA-2A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	117,201.000	12.150	30.64
PDA-2A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	192,748.000	12.150	49.62
PDA-2B	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	29,223.000	12.200	6.03
PDA-2B	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	46,139.000	12.200	10.29
PDA-2B	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	100,548.000	12.150	23.76
PDA-2B	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	148,661.000	12.150	35.76
PDA-2B	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	254,197.000	12.150	61.52

Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
PDA-3B	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	44,793.000	12.150	11.44
PDA-3B	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	63,835.000	12.150	16.48
PDA-3B	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	119,978.000	12.150	30.95
PDA-3B	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	166,584.000	12.150	42.58
PDA-3B	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	264,430.000	12.150	66.25
PDA-3A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	27,853.000	12.150	6.61
PDA-3A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	44,540.000	12.100	11.33
PDA-3A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	98,769.000	12.100	27.07
PDA-3A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	147,074.000	12.100	40.79
PDA-3A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	253,585.000	12.100	70.29
PDA-1B	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	31,079.000	12.150	8.02
PDA-1B	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	45,510.000	12.150	11.94
PDA-1B	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	89,072.000	12.150	23.46
PDA-1B	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	125,860.000	12.100	32.91
PDA-1B	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	204,002.000	12.100	52.82

Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
PDA-1A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	4,441.000	12.150	1.05
PDA-1A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	7,101.000	12.100	1.81
PDA-1A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	15,747.000	12.100	4.32
PDA-1A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	23,448.000	12.100	6.50
PDA-1A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	40,429.000	12.100	11.20
PDA-2C	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	2,791.000	12.100	0.69
PDA-2C	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	3,460.000	12.100	0.85
PDA-2C	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	5,253.000	12.100	1.27
PDA-2C	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	6,647.000	12.100	1.60
PDA-2C	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	9,459.000	12.100	2.25
PDA-1	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	6,166.000	12.150	1.37
PDA-1	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	10,200.000	12.100	2.54
PDA-1	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	23,659.000	12.100	6.51
PDA-1	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	35,875.000	12.100	10.04
PDA-1	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	63,168.000	12.100	17.73

Watershed

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
DP-1	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	22,807.000	12.200	4.05
DP-1	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	36,200.000	12.150	6.91
DP-1	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	90,539.000	12.250	16.94
DP-1	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	140,964.000	12.150	25.17
DP-1	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	253,133.000	12.150	38.81
DP-2	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	13,492.000	12.100	0.70
DP-2	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	43,171.000	15.850	1.23
DP-2	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	151,940.000	12.850	8.60
DP-2	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	251,501.000	12.850	21.54
DP-2	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	474,580.000	12.550	53.99
DP-3	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	8,825.000	12.200	1.43
DP-3	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	37,745.000	12.500	3.97
DP-3	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	155,245.000	12.300	37.02
DP-3	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	262,505.000	12.200	67.56
DP-3	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	502,974.000	12.200	112.63

Watershed

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	4,441.000	12.150	1.05
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	7,101.000	12.100	1.81
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	15,747.000	12.100	4.32
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	23,448.000	12.100	6.50
DP-1A	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	40,429.000	12.100	11.20
DP-4	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00
DP-4	WESTCHESTER-NEW - Synthetic Curve, 2 yrs	2	0.000	0.000	0.00
DP-4	WESTCHESTER-NEW - Synthetic Curve, 10 yrs	10	0.000	0.000	0.00
DP-4	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	14,792.000	12.450	10.14
DP-4	WESTCHESTER-NEW - Synthetic Curve, 100 yrs	100	77,597.000	12.200	52.50

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #3 (IN)	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	27,853.000	12.150	6.61	(N/A)	(N/A)
Infiltration Basin #3 (OUT)	WESTCHESTER-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	291.97	14,808.000

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #3 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	44,540.000	12.100	11.33	(N/A)	(N/A)
Infiltration Basin #3 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	10,284.000	13.550	0.97	292.57	20,201.000
Infiltration Basin #3 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	98,769.000	12.100	27.07	(N/A)	(N/A)
Infiltration Basin #3 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	60,994.000	12.350	13.17	293.56	29,916.000
Infiltration Basin #3 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	147,074.000	12.100	40.79	(N/A)	(N/A)
Infiltration Basin #3 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	107,491.000	12.200	27.25	294.26	37,496.000
Infiltration Basin #3 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	253,585.000	12.100	70.29	(N/A)	(N/A)
Infiltration Basin #3 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	211,271.000	12.200	44.84	295.77	55,970.000
Infiltration Basin #1 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	31,079.000	12.150	8.02	(N/A)	(N/A)
Infiltration Basin #1 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	345.73	7,470.000
Infiltration Basin #1 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	45,510.000	12.150	11.94	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #1 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	1,858.000	12.500	0.88	346.19	12,561.000
Infiltration Basin #1 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	89,072.000	12.150	23.46	(N/A)	(N/A)
Infiltration Basin #1 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	20,273.000	12.450	5.66	347.24	25,183.000
Infiltration Basin #1 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	125,860.000	12.100	32.91	(N/A)	(N/A)
Infiltration Basin #1 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	39,623.000	12.450	7.97	348.13	37,022.000
Infiltration Basin #1 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	204,002.000	12.100	52.82	(N/A)	(N/A)
Infiltration Basin #1 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	84,601.000	12.500	11.11	349.99	65,514.000
Wet Extended Detention Pond #2A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	27,246.000	12.150	6.81	(N/A)	(N/A)
Wet Extended Detention Pond #2A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	321.59	91,423.000
Wet Extended Detention Pond #2A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	40,683.000	12.150	10.49	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Wet Extended Detention Pond #2A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	13,279.000	16.200	0.53	321.72	93,819.000
Wet Extended Detention Pond #2A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	81,934.000	12.150	21.49	(N/A)	(N/A)
Wet Extended Detention Pond #2A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	54,530.000	12.650	4.59	322.24	103,516.000
Wet Extended Detention Pond #2A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	117,201.000	12.150	30.64	(N/A)	(N/A)
Wet Extended Detention Pond #2A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	89,797.000	12.550	9.28	322.85	115,480.000
Wet Extended Detention Pond #2A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	192,748.000	12.150	49.62	(N/A)	(N/A)
Wet Extended Detention Pond #2A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	165,343.000	12.550	12.91	324.44	150,026.000
Infiltration Basin #2 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	29,223.000	12.200	6.03	(N/A)	(N/A)
Infiltration Basin #2 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	326.18	7,556.000

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #2 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	46,139.000	12.200	10.29	(N/A)	(N/A)
Infiltration Basin #2 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	6,454.000	12.600	2.19	326.86	12,629.000
Infiltration Basin #2 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	100,548.000	12.150	23.76	(N/A)	(N/A)
Infiltration Basin #2 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	41,436.000	12.400	12.88	328.06	22,766.000
Infiltration Basin #2 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	148,661.000	12.150	35.76	(N/A)	(N/A)
Infiltration Basin #2 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	76,705.000	12.400	19.84	329.00	31,944.000
Infiltration Basin #2 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	254,197.000	12.150	61.52	(N/A)	(N/A)
Infiltration Basin #2 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	160,300.000	12.350	35.10	330.98	55,024.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	16,103.000	12.150	3.78	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	10,701.000	16.500	0.24	319.33	9,407.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	32,204.000	12.150	6.50	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	26,432.000	14.200	0.75	319.47	18,062.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	98,535.000	12.300	20.84	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	92,157.000	13.250	4.54	320.02	52,292.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	161,729.000	12.150	36.53	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	155,057.000	12.900	12.90	320.38	78,045.000
Existing Pond (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	306,897.000	12.150	61.17	(N/A)	(N/A)
Existing Pond (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	299,777.000	12.550	40.68	320.70	103,673.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	296.00	0.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	0.000	0.000	0.00	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	0.000	0.000	0.00	296.00	0.000

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	0.000	0.000	0.00	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	0.000	0.000	0.00	296.00	0.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	14,792.000	12.400	10.17	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	14,792.000	12.450	10.14	296.75	336.000
DP 4 Low Point (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	77,597.000	12.200	53.11	(N/A)	(N/A)
DP 4 Low Point (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	77,597.000	12.200	52.50	297.56	1,134.000
Infiltration Basin #4 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	30,181.000	12.150	7.31	(N/A)	(N/A)
Infiltration Basin #4 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	300.68	4,840.000
Infiltration Basin #4 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	47,651.000	12.100	12.18	(N/A)	(N/A)
Infiltration Basin #4 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	0.000	0.000	0.00	301.48	11,142.000
Infiltration Basin #4 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	103,842.000	12.100	28.35	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #4 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	0.000	0.000	0.00	303.92	34,905.000
Infiltration Basin #4 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	153,529.000	12.100	42.36	(N/A)	(N/A)
Infiltration Basin #4 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	14,792.000	12.400	10.17	305.11	49,224.000
Infiltration Basin #4 (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	262,521.000	12.100	72.29	(N/A)	(N/A)
Infiltration Basin #4 (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	77,597.000	12.200	53.11	305.84	58,971.000
Infiltration Basin #3A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	44,793.000	12.150	11.44	(N/A)	(N/A)
Infiltration Basin #3A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	0.000	0.000	0.00	321.62	21,053.000
Infiltration Basin #3A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	63,835.000	12.150	16.48	(N/A)	(N/A)
Infiltration Basin #3A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	11,957.000	12.650	2.68	322.19	26,318.000
Infiltration Basin #3A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	119,978.000	12.150	30.95	(N/A)	(N/A)
Infiltration Basin #3A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	55,451.000	12.300	17.23	323.20	36,763.000

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Infiltration Basin #3A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	166,584.000	12.150	42.58	(N/A)	(N/A)
Infiltration Basin #3A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	94,422.000	12.300	27.21	323.92	44,895.000
Infiltration Basin #3A (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	264,430.000	12.150	66.25	(N/A)	(N/A)
Infiltration Basin #3A (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	181,362.000	12.250	43.89	325.23	61,676.000
Det Basin #1C (IN)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	16,641.000	12.100	4.55	(N/A)	(N/A)
Det Basin #1C (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 1 yrs	1	16,641.000	12.250	2.96	316.13	2,893.000
Det Basin #1C (IN)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	24,143.000	12.100	6.72	(N/A)	(N/A)
Det Basin #1C (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 2 yrs	2	24,143.000	12.200	4.68	316.49	4,005.000
Det Basin #1C (IN)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	46,606.000	12.100	13.05	(N/A)	(N/A)
Det Basin #1C (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 10 yrs	10	46,606.000	12.250	8.04	317.51	7,885.000
Det Basin #1C (IN)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	65,466.000	12.100	18.21	(N/A)	(N/A)

Watershed

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Det Basin #1C (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 25 yrs	25	65,466.000	12.250	10.26	318.31	11,634.000
Det Basin #1C (IN)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	105,365.000	12.100	28.77	(N/A)	(N/A)
Det Basin #1C (OUT)	WESTCHESTE R-NEW - Synthetic Curve, 100 yrs	100	105,365.000	12.250	15.36	319.77	20,244.000

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time-Depth Curve: TypeIII 24hr (2.8 in)

Label	TypeIII 24hr (2.8 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	1 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.0	0.0	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.1	0.1	0.1
4.500	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.2	0.2
6.500	0.2	0.2	0.2	0.2	0.2
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.3
8.000	0.3	0.3	0.3	0.3	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.4	0.4	0.5
9.500	0.5	0.5	0.5	0.5	0.5
10.000	0.5	0.5	0.6	0.6	0.6
10.500	0.6	0.6	0.6	0.7	0.7
11.000	0.7	0.7	0.7	0.8	0.8
11.500	0.8	0.9	1.0	1.0	1.2
12.000	1.4	1.6	1.8	1.9	1.9
12.500	2.0	2.0	2.0	2.1	2.1
13.000	2.1	2.1	2.1	2.2	2.2
13.500	2.2	2.2	2.2	2.3	2.3
14.000	2.3	2.3	2.3	2.3	2.3
14.500	2.3	2.4	2.4	2.4	2.4
15.000	2.4	2.4	2.4	2.4	2.4
15.500	2.4	2.5	2.5	2.5	2.5
16.000	2.5	2.5	2.5	2.5	2.5

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	2.5	2.5	2.5	2.5	2.5
17.000	2.6	2.6	2.6	2.6	2.6
17.500	2.6	2.6	2.6	2.6	2.6
18.000	2.6	2.6	2.6	2.6	2.6
18.500	2.6	2.6	2.6	2.6	2.6
19.000	2.7	2.7	2.7	2.7	2.7
19.500	2.7	2.7	2.7	2.7	2.7
20.000	2.7	2.7	2.7	2.7	2.7
20.500	2.7	2.7	2.7	2.7	2.7
21.000	2.7	2.7	2.7	2.7	2.7
21.500	2.7	2.7	2.7	2.7	2.8
22.000	2.8	2.8	2.8	2.8	2.8
22.500	2.8	2.8	2.8	2.8	2.8
23.000	2.8	2.8	2.8	2.8	2.8
23.500	2.8	2.8	2.8	2.8	2.8
24.000	2.8	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time-Depth Curve: TypeIII 24hr (3.4 in)

Label	TypeIII 24hr (3.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	2 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.2	0.2	0.2	0.2
4.500	0.2	0.2	0.2	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.3	0.3	0.3	0.3
6.500	0.3	0.3	0.3	0.3	0.3
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.4	0.4	0.4	0.4
8.000	0.4	0.4	0.4	0.4	0.4
8.500	0.4	0.5	0.5	0.5	0.5
9.000	0.5	0.5	0.5	0.5	0.6
9.500	0.6	0.6	0.6	0.6	0.6
10.000	0.6	0.7	0.7	0.7	0.7
10.500	0.7	0.8	0.8	0.8	0.8
11.000	0.9	0.9	0.9	0.9	1.0
11.500	1.0	1.1	1.2	1.3	1.4
12.000	1.7	2.0	2.1	2.3	2.4
12.500	2.4	2.4	2.5	2.5	2.5
13.000	2.6	2.6	2.6	2.6	2.7
13.500	2.7	2.7	2.7	2.7	2.8
14.000	2.8	2.8	2.8	2.8	2.8
14.500	2.9	2.9	2.9	2.9	2.9
15.000	2.9	2.9	3.0	3.0	3.0
15.500	3.0	3.0	3.0	3.0	3.0
16.000	3.0	3.0	3.1	3.1	3.1

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	3.1	3.1	3.1	3.1	3.1
17.000	3.1	3.1	3.1	3.1	3.1
17.500	3.2	3.2	3.2	3.2	3.2
18.000	3.2	3.2	3.2	3.2	3.2
18.500	3.2	3.2	3.2	3.2	3.2
19.000	3.2	3.2	3.2	3.3	3.3
19.500	3.3	3.3	3.3	3.3	3.3
20.000	3.3	3.3	3.3	3.3	3.3
20.500	3.3	3.3	3.3	3.3	3.3
21.000	3.3	3.3	3.3	3.3	3.3
21.500	3.3	3.3	3.4	3.4	3.4
22.000	3.4	3.4	3.4	3.4	3.4
22.500	3.4	3.4	3.4	3.4	3.4
23.000	3.4	3.4	3.4	3.4	3.4
23.500	3.4	3.4	3.4	3.4	3.4
24.000	3.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time-Depth Curve: TypeIII 24hr (5.1 in)

Label	TypeIII 24hr (5.1 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	10 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.2	0.2	0.2	0.2	0.2
4.500	0.3	0.3	0.3	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.3
5.500	0.3	0.3	0.3	0.4	0.4
6.000	0.4	0.4	0.4	0.4	0.4
6.500	0.4	0.4	0.4	0.4	0.5
7.000	0.5	0.5	0.5	0.5	0.5
7.500	0.5	0.5	0.5	0.6	0.6
8.000	0.6	0.6	0.6	0.6	0.6
8.500	0.7	0.7	0.7	0.7	0.7
9.000	0.7	0.8	0.8	0.8	0.8
9.500	0.8	0.9	0.9	0.9	0.9
10.000	1.0	1.0	1.0	1.0	1.1
10.500	1.1	1.1	1.2	1.2	1.2
11.000	1.3	1.3	1.4	1.4	1.5
11.500	1.5	1.6	1.7	1.9	2.1
12.000	2.5	3.0	3.2	3.4	3.5
12.500	3.6	3.6	3.7	3.7	3.8
13.000	3.8	3.9	3.9	3.9	4.0
13.500	4.0	4.0	4.0	4.1	4.1
14.000	4.1	4.2	4.2	4.2	4.2
14.500	4.2	4.3	4.3	4.3	4.3
15.000	4.3	4.4	4.4	4.4	4.4
15.500	4.4	4.5	4.5	4.5	4.5
16.000	4.5	4.5	4.5	4.5	4.6

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	4.6	4.6	4.6	4.6	4.6
17.000	4.6	4.6	4.7	4.7	4.7
17.500	4.7	4.7	4.7	4.7	4.7
18.000	4.7	4.7	4.7	4.7	4.8
18.500	4.8	4.8	4.8	4.8	4.8
19.000	4.8	4.8	4.8	4.8	4.8
19.500	4.8	4.8	4.9	4.9	4.9
20.000	4.9	4.9	4.9	4.9	4.9
20.500	4.9	4.9	4.9	4.9	4.9
21.000	4.9	4.9	4.9	5.0	5.0
21.500	5.0	5.0	5.0	5.0	5.0
22.000	5.0	5.0	5.0	5.0	5.0
22.500	5.0	5.0	5.0	5.0	5.0
23.000	5.0	5.0	5.1	5.1	5.1
23.500	5.1	5.1	5.1	5.1	5.1
24.000	5.1	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time-Depth Curve: TypeIII 24hr (6.4 in)

Label	TypeIII 24hr (6.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.2
2.500	0.2	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.3	0.3
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.3	0.3	0.3	0.4
5.000	0.4	0.4	0.4	0.4	0.4
5.500	0.4	0.4	0.4	0.4	0.4
6.000	0.5	0.5	0.5	0.5	0.5
6.500	0.5	0.5	0.5	0.6	0.6
7.000	0.6	0.6	0.6	0.6	0.6
7.500	0.6	0.7	0.7	0.7	0.7
8.000	0.7	0.7	0.8	0.8	0.8
8.500	0.8	0.8	0.9	0.9	0.9
9.000	0.9	1.0	1.0	1.0	1.0
9.500	1.1	1.1	1.1	1.1	1.2
10.000	1.2	1.2	1.3	1.3	1.3
10.500	1.4	1.4	1.5	1.5	1.5
11.000	1.6	1.6	1.7	1.8	1.8
11.500	1.9	2.0	2.2	2.4	2.7
12.000	3.2	3.7	4.0	4.2	4.4
12.500	4.5	4.6	4.6	4.7	4.7
13.000	4.8	4.8	4.9	4.9	5.0
13.500	5.0	5.0	5.1	5.1	5.1
14.000	5.2	5.2	5.2	5.3	5.3
14.500	5.3	5.3	5.4	5.4	5.4
15.000	5.5	5.5	5.5	5.5	5.5
15.500	5.6	5.6	5.6	5.6	5.6
16.000	5.7	5.7	5.7	5.7	5.7

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	5.7	5.7	5.8	5.8	5.8
17.000	5.8	5.8	5.8	5.8	5.9
17.500	5.9	5.9	5.9	5.9	5.9
18.000	5.9	5.9	5.9	6.0	6.0
18.500	6.0	6.0	6.0	6.0	6.0
19.000	6.0	6.0	6.0	6.0	6.1
19.500	6.1	6.1	6.1	6.1	6.1
20.000	6.1	6.1	6.1	6.1	6.1
20.500	6.1	6.2	6.2	6.2	6.2
21.000	6.2	6.2	6.2	6.2	6.2
21.500	6.2	6.2	6.2	6.2	6.3
22.000	6.3	6.3	6.3	6.3	6.3
22.500	6.3	6.3	6.3	6.3	6.3
23.000	6.3	6.3	6.3	6.3	6.3
23.500	6.4	6.4	6.4	6.4	6.4
24.000	6.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time-Depth Curve: TypeIII 24hr (9.0 in)

Label	TypeIII 24hr (9.0 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	100 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.1	0.1	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.2	0.2	0.2
2.000	0.2	0.2	0.2	0.2	0.2
2.500	0.2	0.2	0.2	0.3	0.3
3.000	0.3	0.3	0.3	0.3	0.3
3.500	0.3	0.3	0.4	0.4	0.4
4.000	0.4	0.4	0.4	0.4	0.4
4.500	0.4	0.5	0.5	0.5	0.5
5.000	0.5	0.5	0.5	0.5	0.6
5.500	0.6	0.6	0.6	0.6	0.6
6.000	0.6	0.7	0.7	0.7	0.7
6.500	0.7	0.7	0.8	0.8	0.8
7.000	0.8	0.8	0.9	0.9	0.9
7.500	0.9	0.9	1.0	1.0	1.0
8.000	1.0	1.0	1.1	1.1	1.1
8.500	1.2	1.2	1.2	1.2	1.3
9.000	1.3	1.3	1.4	1.4	1.5
9.500	1.5	1.5	1.6	1.6	1.7
10.000	1.7	1.7	1.8	1.8	1.9
10.500	1.9	2.0	2.1	2.1	2.2
11.000	2.2	2.3	2.4	2.5	2.6
11.500	2.7	2.8	3.0	3.4	3.7
12.000	4.5	5.2	5.6	5.9	6.2
12.500	6.3	6.4	6.5	6.6	6.7
13.000	6.7	6.8	6.9	6.9	7.0
13.500	7.0	7.1	7.1	7.2	7.2
14.000	7.3	7.3	7.4	7.4	7.5
14.500	7.5	7.5	7.6	7.6	7.6
15.000	7.7	7.7	7.7	7.8	7.8
15.500	7.8	7.9	7.9	7.9	7.9
16.000	8.0	8.0	8.0	8.0	8.0

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	8.1	8.1	8.1	8.1	8.1
17.000	8.2	8.2	8.2	8.2	8.2
17.500	8.3	8.3	8.3	8.3	8.3
18.000	8.3	8.3	8.4	8.4	8.4
18.500	8.4	8.4	8.4	8.4	8.5
19.000	8.5	8.5	8.5	8.5	8.5
19.500	8.5	8.5	8.6	8.6	8.6
20.000	8.6	8.6	8.6	8.6	8.6
20.500	8.7	8.7	8.7	8.7	8.7
21.000	8.7	8.7	8.7	8.7	8.7
21.500	8.8	8.8	8.8	8.8	8.8
22.000	8.8	8.8	8.8	8.8	8.8
22.500	8.9	8.9	8.9	8.9	8.9
23.000	8.9	8.9	8.9	8.9	8.9
23.500	8.9	8.9	9.0	9.0	9.0
24.000	9.0	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.150 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.27 ft/s
Segment Time of Concentration	0.103 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	133.00 ft
Is Paved?	False
Slope	0.188 ft/ft
Average Velocity	7.00 ft/s
Segment Time of Concentration	0.005 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.108 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$(L_f / V) / 3600$$

Where:

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{V}{16.1345 * (S_f^{0.5})}$$

Paved Surface:

$$V = 20.3282 * (S_f^{0.5})$$
$$(L_f / V) / 3600$$

Where:

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.090 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.22 ft/s
Segment Time of Concentration	0.126 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	43.00 ft
Is Paved?	False
Slope	0.209 ft/ft
Average Velocity	7.38 ft/s
Segment Time of Concentration	0.002 hours

Segment #3: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	750.00 ft
Manning's n	0.012
Slope	0.009 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	11.78 ft/s
Segment Time of Concentration	0.018 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.146 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{(0.007 * ((n * L_f)^{0.8}))}{((P^{0.5}) * (S_f^{0.4}))}$$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.230 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.32 ft/s
Segment Time of Concentration	0.086 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	43.00 ft
Is Paved?	False
Slope	0.020 ft/ft
Average Velocity	2.28 ft/s
Segment Time of Concentration	0.005 hours

Segment #3: TR-55 Shallow Concentrated Flow

Hydraulic Length	163.00 ft
Is Paved?	True
Slope	0.060 ft/ft
Average Velocity	4.98 ft/s
Segment Time of Concentration	0.009 hours

Segment #4: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	117.00 ft
Manning's n	0.012
Slope	0.030 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	21.32 ft/s
Segment Time of Concentration	0.002 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.102 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \text{Unpaved surface:}$$
$$V = 16.1345 * (S_f^{0.5})$$

$$\text{Paved Surface:}$$
$$V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{0.007 * ((n * L_f)^{0.8})}{((P^{0.5}) * (S_f^{0.4}))}$$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.110 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of Concentration	0.116 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	118.00 ft
Is Paved?	False
Slope	0.042 ft/ft
Average Velocity	3.31 ft/s
Segment Time of Concentration	0.010 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.126 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.110 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of Concentration	0.117 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	189.00 ft
Is Paved?	False
Slope	0.050 ft/ft
Average Velocity	3.61 ft/s
Segment Time of Concentration	0.015 hours

Segment #3: TR-55 Shallow Concentrated Flow

Hydraulic Length	113.00 ft
Is Paved?	True
Slope	0.010 ft/ft
Average Velocity	2.03 ft/s
Segment Time of Concentration	0.015 hours

Segment #4: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	613.00 ft
Manning's n	0.012
Slope	0.015 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	15.21 ft/s
Segment Time of Concentration	0.011 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.158 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

Where:

$$(L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface:

$$V = 16.1345 * (S_f^{0.5})$$

Paved Surface:

$$V = 20.3282 * (S_f^{0.5})$$

Where:

$$(L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{0.007 * ((n * L_f)^{0.8})}{((P^{0.5}) * (S_f^{0.4}))}$$

Where:

Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.16 ft/s
Segment Time of Concentration	0.175 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	419.00 ft
Is Paved?	False
Slope	0.034 ft/ft
Average Velocity	2.98 ft/s
Segment Time of Concentration	0.039 hours

Segment #3: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	550.00 ft
Manning's n	0.014
Slope	0.040 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	21.29 ft/s
Segment Time of Concentration	0.007 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.221 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-2B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

Where:

$(L_f / V) / 3600$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{(L_f / V) / 3600}{\text{Unpaved surface:}}$$
$$V = 16.1345 * (S_f^{0.5})$$

Paved Surface:

$$V = 20.3282 * (S_f^{0.5})$$

Where:

$(L_f / V) / 3600$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{(0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))}{\text{Where:}}$$

Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.050 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.17 ft/s
Segment Time of Concentration	0.160 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	121.00 ft
Is Paved?	False
Slope	0.070 ft/ft
Average Velocity	4.27 ft/s
Segment Time of Concentration	0.008 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.168 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.190 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.30 ft/s
Segment Time of Concentration	0.094 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	132.00 ft
Is Paved?	False
Slope	0.050 ft/ft
Average Velocity	3.61 ft/s
Segment Time of Concentration	0.010 hours

Segment #3: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	457.00 ft
Manning's n	0.012
Slope	0.031 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	21.86 ft/s
Segment Time of Concentration	0.006 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.110 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R}{Q_a / W_p}$$
$$V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n$$

Where:

$$(L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface:

$$V = 16.1345 * (S_f^{0.5})$$

Paved Surface:

$$V = 20.3282 * (S_f^{0.5})$$

Where:

$$(L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{0.007 * ((n * L_f)^{0.8})}{((P^{0.5}) * (S_f^{0.4}))}$$

Where:

Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.060 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.19 ft/s
Segment Time of Concentration	0.149 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	172.00 ft
Is Paved?	False
Slope	0.100 ft/ft
Average Velocity	5.10 ft/s
Segment Time of Concentration	0.009 hours

Segment #3: TR-55 Shallow Concentrated Flow

Hydraulic Length	83.00 ft
Is Paved?	True
Slope	0.060 ft/ft
Average Velocity	4.98 ft/s
Segment Time of Concentration	0.005 hours

Segment #4: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	593.00 ft
Manning's n	0.012
Slope	0.032 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	22.21 ft/s
Segment Time of Concentration	0.007 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.170 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-3B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{(0.007 * ((n * L_f)^{0.8}))}{((P^{0.5}) * (S_f^{0.4}))}$$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Time of Concentration Calculations

Label: PDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.240
Slope	0.210 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.31 ft/s
Segment Time of Concentration	0.090 hours

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	266.00 ft
Is Paved?	False
Slope	0.105 ft/ft
Average Velocity	5.23 ft/s
Segment Time of Concentration	0.014 hours

Segment #3: TR-55 Channel Flow

Flow Area	3.1 ft ²
Hydraulic Length	619.00 ft
Manning's n	0.012
Slope	0.020 ft/ft
Wetted Perimeter	3.14 ft
Average Velocity	17.56 ft/s
Segment Time of Concentration	0.010 hours

Time of Concentration (Composite)

Time of Concentration (Composite)	0.114 hours
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Watershed

Subsection: Time of Concentration Calculations

Label: PDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

==== SCS Channel Flow

$$T_c = \frac{R = Q_a / W_p}{V = (1.49 * (R^{2/3}) * (S_f^{-0.5})) / n}$$

$$\text{Where: } (L_f / V) / 3600$$

R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

$$T_c = \frac{\text{Unpaved surface:}}{V = 16.1345 * (S_f^{0.5})}$$

$$\text{Paved Surface: } V = 20.3282 * (S_f^{0.5})$$

$$\text{Where: } (L_f / V) / 3600$$

V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

$$T_c = \frac{(0.007 * ((n * L_f)^{0.8}))}{((P^{0.5}) * (S_f^{0.4}))}$$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Watershed

Subsection: Runoff CN-Area

Label: PDA-1A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil D	80.000	38,899.000	0.0	0.0	80.000
Woods - good - Soil B	55.000	14,243.000	0.0	0.0	55.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	38,681.000	0.0	0.0	61.000
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil D	98.000	1,580.000	0.0	0.0	98.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	93,403.000	(N/A)	(N/A)	68.624

Watershed

Subsection: Runoff CN-Area

Label: PDA-1B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	136,909.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	176,113.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	83,112.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	396,134.000	(N/A)	(N/A)	76.515

Watershed

Subsection: Runoff CN-Area

Label: PDA-1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	64,243.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil D	80.000	49,658.000	0.0	0.0	80.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	86,700.000	0.0	0.0	61.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	200,601.000	(N/A)	(N/A)	77.553

Watershed

Subsection: Runoff CN-Area

Label: PDA-2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	75,359.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	257,004.000	0.0	0.0	61.000
Woods - good - Soil B	55.000	6,339.000	0.0	0.0	55.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	338,702.000	(N/A)	(N/A)	69.120

Watershed

Subsection: Runoff CN-Area

Label: PDA-2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	111,751.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	185,958.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	92,086.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	389,795.000	(N/A)	(N/A)	74.679

Watershed

Subsection: Runoff CN-Area

Label: PDA-2B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	144,182.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	424,492.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	4,966.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	573,640.000	(N/A)	(N/A)	70.412

Watershed

Subsection: Runoff CN-Area

Label: PDA-2C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil B	98.000	12,988.000	0.0	0.0	98.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	12,988.000	(N/A)	(N/A)	98.000

Watershed

Subsection: Runoff CN-Area

Label: PDA-3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	213,531.000	0.0	0.0	61.000
Woods - good - Soil B	55.000	25,228.000	0.0	0.0	55.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	58,501.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	297,260.000	(N/A)	(N/A)	63.049

Watershed

Subsection: Runoff CN-Area

Label: PDA-3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	116,436.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	433,074.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	36,373.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	585,883.000	(N/A)	(N/A)	69.160

Watershed

Subsection: Runoff CN-Area

Label: PDA-3B

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	159,909.000	0.0	0.0	98.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	63,510.000	0.0	0.0	61.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	261,142.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	484,561.000	(N/A)	(N/A)	80.216

Watershed

Subsection: Runoff CN-Area

Label: PDA-4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads	98.000	125,583.000	0.0	0.0	98.000
Woods - good - Soil B	55.000	15,280.000	0.0	0.0	55.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil B	61.000	401,797.000	0.0	0.0	61.000
Woods - good - Soil C	70.000	2,842.000	0.0	0.0	70.000
Open space (Lawns,parks etc.) - Good condition; grass cover > 75% - Soil C	74.000	46,958.000	0.0	0.0	74.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	592,460.000	(N/A)	(N/A)	69.762

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	315.00	315.00	315.00	315.00	315.00
0.250	315.00	315.00	315.00	315.00	315.00
0.500	315.00	315.00	315.00	315.00	315.00
0.750	315.00	315.00	315.00	315.00	315.00
1.000	315.00	315.00	315.00	315.00	315.00
1.250	315.00	315.00	315.00	315.00	315.00
1.500	315.00	315.00	315.00	315.00	315.00
1.750	315.00	315.00	315.00	315.00	315.00
2.000	315.00	315.00	315.00	315.00	315.00
2.250	315.00	315.00	315.00	315.00	315.00
2.500	315.00	315.00	315.00	315.00	315.00
2.750	315.00	315.00	315.00	315.00	315.00
3.000	315.00	315.00	315.00	315.00	315.00
3.250	315.00	315.00	315.00	315.00	315.00
3.500	315.00	315.00	315.00	315.00	315.00
3.750	315.00	315.00	315.00	315.00	315.00
4.000	315.00	315.00	315.00	315.00	315.00
4.250	315.00	315.00	315.00	315.00	315.00
4.500	315.00	315.00	315.00	315.00	315.00
4.750	315.00	315.00	315.00	315.00	315.00
5.000	315.00	315.00	315.00	315.00	315.00
5.250	315.00	315.00	315.00	315.00	315.00
5.500	315.00	315.00	315.00	315.00	315.00
5.750	315.00	315.00	315.00	315.00	315.00
6.000	315.00	315.00	315.00	315.00	315.00
6.250	315.00	315.00	315.00	315.00	315.00
6.500	315.00	315.00	315.00	315.00	315.00
6.750	315.00	315.00	315.00	315.00	315.00
7.000	315.00	315.00	315.00	315.00	315.00
7.250	315.00	315.00	315.00	315.00	315.00
7.500	315.00	315.00	315.00	315.00	315.00
7.750	315.00	315.00	315.00	315.00	315.00
8.000	315.00	315.00	315.00	315.00	315.00
8.250	315.00	315.00	315.00	315.00	315.00
8.500	315.00	315.00	315.00	315.00	315.00
8.750	315.00	315.00	315.00	315.00	315.00
9.000	315.00	315.00	315.00	315.00	315.00
9.250	315.00	315.00	315.00	315.00	315.00
9.500	315.00	315.00	315.00	315.00	315.00
9.750	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	315.00	315.00	315.00	315.00	315.00
10.250	315.00	315.00	315.00	315.00	315.00
10.500	315.00	315.00	315.00	315.01	315.01
10.750	315.01	315.01	315.02	315.02	315.02
11.000	315.03	315.03	315.03	315.04	315.04
11.250	315.05	315.05	315.06	315.07	315.08
11.500	315.09	315.10	315.11	315.13	315.16
11.750	315.19	315.24	315.30	315.38	315.48
12.000	315.61	315.77	315.94	316.07	316.13
12.250	316.13	316.12	316.09	316.06	316.02
12.500	315.96	315.91	315.85	315.80	315.75
12.750	315.71	315.68	315.65	315.62	315.60
13.000	315.58	315.56	315.55	315.54	315.53
13.250	315.52	315.51	315.50	315.50	315.49
13.500	315.48	315.47	315.47	315.46	315.45
13.750	315.45	315.44	315.43	315.43	315.42
14.000	315.41	315.41	315.40	315.39	315.39
14.250	315.38	315.37	315.37	315.36	315.36
14.500	315.35	315.35	315.34	315.34	315.33
14.750	315.33	315.33	315.32	315.32	315.31
15.000	315.31	315.30	315.30	315.30	315.29
15.250	315.29	315.28	315.28	315.28	315.27
15.500	315.27	315.26	315.26	315.26	315.25
15.750	315.25	315.25	315.24	315.24	315.23
16.000	315.23	315.23	315.22	315.22	315.22
16.250	315.21	315.21	315.21	315.20	315.20
16.500	315.20	315.20	315.19	315.19	315.19
16.750	315.19	315.18	315.18	315.18	315.18
17.000	315.17	315.17	315.17	315.17	315.17
17.250	315.16	315.16	315.16	315.16	315.16
17.500	315.16	315.15	315.15	315.15	315.15
17.750	315.15	315.14	315.14	315.14	315.14
18.000	315.14	315.14	315.13	315.13	315.13
18.250	315.13	315.13	315.13	315.13	315.12
18.500	315.12	315.12	315.12	315.12	315.12
18.750	315.12	315.12	315.12	315.12	315.12
19.000	315.11	315.11	315.11	315.11	315.11
19.250	315.11	315.11	315.11	315.11	315.11
19.500	315.11	315.11	315.11	315.11	315.11
19.750	315.10	315.10	315.10	315.10	315.10
20.000	315.10	315.10	315.10	315.10	315.10

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	315.10	315.10	315.10	315.10	315.10
20.500	315.10	315.10	315.10	315.10	315.10
20.750	315.09	315.09	315.09	315.09	315.09
21.000	315.09	315.09	315.09	315.09	315.09
21.250	315.09	315.09	315.09	315.09	315.09
21.500	315.09	315.09	315.09	315.09	315.09
21.750	315.09	315.09	315.09	315.09	315.09
22.000	315.08	315.08	315.08	315.08	315.08
22.250	315.08	315.08	315.08	315.08	315.08
22.500	315.08	315.08	315.08	315.08	315.08
22.750	315.08	315.08	315.08	315.08	315.08
23.000	315.08	315.08	315.08	315.08	315.08
23.250	315.07	315.07	315.07	315.07	315.07
23.500	315.07	315.07	315.07	315.07	315.07
23.750	315.07	315.07	315.07	315.07	315.07
24.000	315.07	315.07	315.06	315.06	315.05
24.250	315.05	315.05	315.04	315.04	315.03
24.500	315.03	315.03	315.03	315.02	315.02
24.750	315.02	315.02	315.02	315.01	315.01
25.000	315.01	315.01	315.01	315.01	315.01
25.250	315.01	315.01	315.01	315.01	315.01
25.500	315.00	315.00	315.00	315.00	315.00
25.750	315.00	315.00	315.00	315.00	315.00
26.000	315.00	315.00	315.00	315.00	315.00
26.250	315.00	315.00	315.00	315.00	315.00
26.500	315.00	315.00	315.00	315.00	315.00
26.750	315.00	315.00	315.00	315.00	315.00
27.000	315.00	315.00	315.00	315.00	315.00
27.250	315.00	315.00	315.00	315.00	315.00
27.500	315.00	315.00	315.00	315.00	315.00
27.750	315.00	315.00	315.00	315.00	315.00
28.000	315.00	315.00	315.00	315.00	315.00
28.250	315.00	315.00	315.00	315.00	315.00
28.500	315.00	315.00	315.00	315.00	315.00
28.750	315.00	315.00	315.00	315.00	315.00
29.000	315.00	315.00	315.00	315.00	315.00
29.250	315.00	315.00	315.00	315.00	315.00
29.500	315.00	315.00	315.00	315.00	315.00
29.750	315.00	315.00	315.00	315.00	315.00
30.000	315.00	315.00	315.00	315.00	315.00
30.250	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	315.00	315.00	315.00	315.00	315.00
30.750	315.00	315.00	315.00	315.00	315.00
31.000	315.00	315.00	315.00	315.00	315.00
31.250	315.00	315.00	315.00	315.00	315.00
31.500	315.00	315.00	315.00	315.00	315.00
31.750	315.00	315.00	315.00	315.00	315.00
32.000	315.00	315.00	315.00	315.00	315.00
32.250	315.00	315.00	315.00	315.00	315.00
32.500	315.00	315.00	315.00	315.00	315.00
32.750	315.00	315.00	315.00	315.00	315.00
33.000	315.00	315.00	315.00	315.00	315.00
33.250	315.00	315.00	315.00	315.00	315.00
33.500	315.00	315.00	315.00	315.00	315.00
33.750	315.00	315.00	315.00	315.00	315.00
34.000	315.00	315.00	315.00	315.00	315.00
34.250	315.00	315.00	315.00	315.00	315.00
34.500	315.00	315.00	315.00	315.00	315.00
34.750	315.00	315.00	315.00	315.00	315.00
35.000	315.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	315.00	315.00	315.00	315.00	315.00
0.250	315.00	315.00	315.00	315.00	315.00
0.500	315.00	315.00	315.00	315.00	315.00
0.750	315.00	315.00	315.00	315.00	315.00
1.000	315.00	315.00	315.00	315.00	315.00
1.250	315.00	315.00	315.00	315.00	315.00
1.500	315.00	315.00	315.00	315.00	315.00
1.750	315.00	315.00	315.00	315.00	315.00
2.000	315.00	315.00	315.00	315.00	315.00
2.250	315.00	315.00	315.00	315.00	315.00
2.500	315.00	315.00	315.00	315.00	315.00
2.750	315.00	315.00	315.00	315.00	315.00
3.000	315.00	315.00	315.00	315.00	315.00
3.250	315.00	315.00	315.00	315.00	315.00
3.500	315.00	315.00	315.00	315.00	315.00
3.750	315.00	315.00	315.00	315.00	315.00
4.000	315.00	315.00	315.00	315.00	315.00
4.250	315.00	315.00	315.00	315.00	315.00
4.500	315.00	315.00	315.00	315.00	315.00
4.750	315.00	315.00	315.00	315.00	315.00
5.000	315.00	315.00	315.00	315.00	315.00
5.250	315.00	315.00	315.00	315.00	315.00
5.500	315.00	315.00	315.00	315.00	315.00
5.750	315.00	315.00	315.00	315.00	315.00
6.000	315.00	315.00	315.00	315.00	315.00
6.250	315.00	315.00	315.00	315.00	315.00
6.500	315.00	315.00	315.00	315.00	315.00
6.750	315.00	315.00	315.00	315.00	315.00
7.000	315.00	315.00	315.00	315.00	315.00
7.250	315.00	315.00	315.00	315.00	315.00
7.500	315.00	315.00	315.00	315.00	315.00
7.750	315.00	315.00	315.00	315.00	315.00
8.000	315.00	315.00	315.00	315.00	315.00
8.250	315.00	315.00	315.00	315.00	315.00
8.500	315.00	315.00	315.00	315.00	315.00
8.750	315.00	315.00	315.00	315.00	315.00
9.000	315.00	315.00	315.00	315.00	315.00
9.250	315.00	315.00	315.00	315.00	315.00
9.500	315.00	315.00	315.00	315.00	315.00
9.750	315.00	315.00	315.00	315.01	315.01

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	315.01	315.01	315.01	315.02	315.02
10.250	315.02	315.03	315.03	315.03	315.04
10.500	315.04	315.04	315.05	315.05	315.06
10.750	315.06	315.07	315.07	315.08	315.09
11.000	315.09	315.10	315.11	315.11	315.12
11.250	315.13	315.14	315.15	315.17	315.18
11.500	315.20	315.21	315.24	315.27	315.32
11.750	315.38	315.45	315.54	315.63	315.74
12.000	315.91	316.11	316.29	316.43	316.49
12.250	316.47	316.43	316.37	316.31	316.24
12.500	316.17	316.10	316.03	315.97	315.90
12.750	315.85	315.80	315.77	315.73	315.70
13.000	315.68	315.65	315.63	315.62	315.60
13.250	315.59	315.58	315.57	315.56	315.55
13.500	315.55	315.54	315.54	315.53	315.53
13.750	315.52	315.52	315.52	315.51	315.51
14.000	315.50	315.50	315.50	315.49	315.49
14.250	315.48	315.48	315.47	315.47	315.46
14.500	315.46	315.45	315.45	315.44	315.44
14.750	315.43	315.43	315.43	315.42	315.42
15.000	315.41	315.41	315.40	315.40	315.39
15.250	315.39	315.38	315.38	315.37	315.37
15.500	315.36	315.36	315.35	315.35	315.34
15.750	315.34	315.33	315.33	315.32	315.32
16.000	315.31	315.31	315.30	315.30	315.29
16.250	315.29	315.28	315.28	315.27	315.27
16.500	315.27	315.26	315.26	315.26	315.25
16.750	315.25	315.25	315.24	315.24	315.24
17.000	315.24	315.23	315.23	315.23	315.23
17.250	315.22	315.22	315.22	315.21	315.21
17.500	315.21	315.21	315.20	315.20	315.20
17.750	315.20	315.19	315.19	315.19	315.19
18.000	315.19	315.18	315.18	315.18	315.18
18.250	315.17	315.17	315.17	315.17	315.17
18.500	315.17	315.16	315.16	315.16	315.16
18.750	315.16	315.16	315.16	315.16	315.15
19.000	315.15	315.15	315.15	315.15	315.15
19.250	315.15	315.15	315.15	315.15	315.15
19.500	315.14	315.14	315.14	315.14	315.14
19.750	315.14	315.14	315.14	315.14	315.14
20.000	315.14	315.14	315.14	315.13	315.13

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	315.13	315.13	315.13	315.13	315.13
20.500	315.13	315.13	315.13	315.13	315.13
20.750	315.13	315.13	315.13	315.13	315.12
21.000	315.12	315.12	315.12	315.12	315.12
21.250	315.12	315.12	315.12	315.12	315.12
21.500	315.12	315.12	315.12	315.12	315.12
21.750	315.12	315.12	315.12	315.11	315.11
22.000	315.11	315.11	315.11	315.11	315.11
22.250	315.11	315.11	315.11	315.11	315.11
22.500	315.11	315.11	315.11	315.11	315.11
22.750	315.11	315.10	315.10	315.10	315.10
23.000	315.10	315.10	315.10	315.10	315.10
23.250	315.10	315.10	315.10	315.10	315.10
23.500	315.10	315.10	315.10	315.10	315.10
23.750	315.09	315.09	315.09	315.09	315.09
24.000	315.09	315.09	315.09	315.08	315.07
24.250	315.07	315.06	315.06	315.05	315.05
24.500	315.04	315.04	315.03	315.03	315.03
24.750	315.03	315.02	315.02	315.02	315.02
25.000	315.02	315.01	315.01	315.01	315.01
25.250	315.01	315.01	315.01	315.01	315.01
25.500	315.01	315.01	315.01	315.00	315.00
25.750	315.00	315.00	315.00	315.00	315.00
26.000	315.00	315.00	315.00	315.00	315.00
26.250	315.00	315.00	315.00	315.00	315.00
26.500	315.00	315.00	315.00	315.00	315.00
26.750	315.00	315.00	315.00	315.00	315.00
27.000	315.00	315.00	315.00	315.00	315.00
27.250	315.00	315.00	315.00	315.00	315.00
27.500	315.00	315.00	315.00	315.00	315.00
27.750	315.00	315.00	315.00	315.00	315.00
28.000	315.00	315.00	315.00	315.00	315.00
28.250	315.00	315.00	315.00	315.00	315.00
28.500	315.00	315.00	315.00	315.00	315.00
28.750	315.00	315.00	315.00	315.00	315.00
29.000	315.00	315.00	315.00	315.00	315.00
29.250	315.00	315.00	315.00	315.00	315.00
29.500	315.00	315.00	315.00	315.00	315.00
29.750	315.00	315.00	315.00	315.00	315.00
30.000	315.00	315.00	315.00	315.00	315.00
30.250	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	315.00	315.00	315.00	315.00	315.00
30.750	315.00	315.00	315.00	315.00	315.00
31.000	315.00	315.00	315.00	315.00	315.00
31.250	315.00	315.00	315.00	315.00	315.00
31.500	315.00	315.00	315.00	315.00	315.00
31.750	315.00	315.00	315.00	315.00	315.00
32.000	315.00	315.00	315.00	315.00	315.00
32.250	315.00	315.00	315.00	315.00	315.00
32.500	315.00	315.00	315.00	315.00	315.00
32.750	315.00	315.00	315.00	315.00	315.00
33.000	315.00	315.00	315.00	315.00	315.00
33.250	315.00	315.00	315.00	315.00	315.00
33.500	315.00	315.00	315.00	315.00	315.00
33.750	315.00	315.00	315.00	315.00	315.00
34.000	315.00	315.00	315.00	315.00	315.00
34.250	315.00	315.00	315.00	315.00	315.00
34.500	315.00	315.00	315.00	315.00	315.00
34.750	315.00	315.00	315.00	315.00	315.00
35.000	315.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	315.00	315.00	315.00	315.00	315.00
0.250	315.00	315.00	315.00	315.00	315.00
0.500	315.00	315.00	315.00	315.00	315.00
0.750	315.00	315.00	315.00	315.00	315.00
1.000	315.00	315.00	315.00	315.00	315.00
1.250	315.00	315.00	315.00	315.00	315.00
1.500	315.00	315.00	315.00	315.00	315.00
1.750	315.00	315.00	315.00	315.00	315.00
2.000	315.00	315.00	315.00	315.00	315.00
2.250	315.00	315.00	315.00	315.00	315.00
2.500	315.00	315.00	315.00	315.00	315.00
2.750	315.00	315.00	315.00	315.00	315.00
3.000	315.00	315.00	315.00	315.00	315.00
3.250	315.00	315.00	315.00	315.00	315.00
3.500	315.00	315.00	315.00	315.00	315.00
3.750	315.00	315.00	315.00	315.00	315.00
4.000	315.00	315.00	315.00	315.00	315.00
4.250	315.00	315.00	315.00	315.00	315.00
4.500	315.00	315.00	315.00	315.00	315.00
4.750	315.00	315.00	315.00	315.00	315.00
5.000	315.00	315.00	315.00	315.00	315.00
5.250	315.00	315.00	315.00	315.00	315.00
5.500	315.00	315.00	315.00	315.00	315.00
5.750	315.00	315.00	315.00	315.00	315.00
6.000	315.00	315.00	315.00	315.00	315.00
6.250	315.00	315.00	315.00	315.00	315.00
6.500	315.00	315.00	315.00	315.00	315.00
6.750	315.00	315.00	315.00	315.00	315.00
7.000	315.00	315.00	315.00	315.00	315.00
7.250	315.00	315.00	315.00	315.00	315.00
7.500	315.00	315.00	315.00	315.00	315.00
7.750	315.00	315.00	315.00	315.00	315.00
8.000	315.00	315.00	315.00	315.00	315.00
8.250	315.00	315.00	315.01	315.01	315.01
8.500	315.01	315.01	315.02	315.02	315.02
8.750	315.02	315.03	315.03	315.03	315.04
9.000	315.04	315.04	315.05	315.05	315.05
9.250	315.06	315.06	315.07	315.07	315.08
9.500	315.08	315.09	315.09	315.10	315.10
9.750	315.11	315.12	315.12	315.13	315.13

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	315.14	315.15	315.15	315.16	315.17
10.250	315.18	315.19	315.19	315.20	315.21
10.500	315.22	315.23	315.24	315.25	315.27
10.750	315.28	315.29	315.30	315.32	315.33
11.000	315.34	315.36	315.37	315.39	315.41
11.250	315.43	315.45	315.48	315.51	315.53
11.500	315.55	315.58	315.61	315.66	315.73
11.750	315.81	315.92	316.04	316.16	316.32
12.000	316.56	316.86	317.16	317.39	317.51
12.250	317.51	317.47	317.39	317.29	317.18
12.500	317.05	316.90	316.73	316.58	316.44
12.750	316.31	316.21	316.13	316.07	316.02
13.000	315.97	315.92	315.88	315.85	315.82
13.250	315.80	315.78	315.76	315.74	315.73
13.500	315.72	315.71	315.70	315.69	315.68
13.750	315.67	315.67	315.66	315.65	315.64
14.000	315.64	315.63	315.62	315.62	315.61
14.250	315.61	315.60	315.60	315.60	315.59
14.500	315.59	315.58	315.58	315.58	315.57
14.750	315.57	315.57	315.56	315.56	315.56
15.000	315.56	315.55	315.55	315.55	315.54
15.250	315.54	315.54	315.53	315.53	315.53
15.500	315.52	315.52	315.52	315.51	315.51
15.750	315.51	315.51	315.50	315.50	315.49
16.000	315.49	315.48	315.48	315.47	315.47
16.250	315.46	315.46	315.45	315.45	315.44
16.500	315.44	315.44	315.43	315.43	315.42
16.750	315.42	315.41	315.41	315.40	315.40
17.000	315.40	315.39	315.39	315.38	315.38
17.250	315.38	315.37	315.37	315.36	315.36
17.500	315.35	315.35	315.35	315.34	315.34
17.750	315.33	315.33	315.33	315.32	315.32
18.000	315.31	315.31	315.31	315.30	315.30
18.250	315.30	315.29	315.29	315.29	315.28
18.500	315.28	315.28	315.28	315.27	315.27
18.750	315.27	315.27	315.27	315.26	315.26
19.000	315.26	315.26	315.26	315.26	315.25
19.250	315.25	315.25	315.25	315.25	315.25
19.500	315.25	315.24	315.24	315.24	315.24
19.750	315.24	315.24	315.24	315.23	315.23
20.000	315.23	315.23	315.23	315.23	315.23

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	315.23	315.22	315.22	315.22	315.22
20.500	315.22	315.22	315.22	315.22	315.22
20.750	315.21	315.21	315.21	315.21	315.21
21.000	315.21	315.21	315.21	315.21	315.21
21.250	315.21	315.20	315.20	315.20	315.20
21.500	315.20	315.20	315.20	315.20	315.20
21.750	315.20	315.19	315.19	315.19	315.19
22.000	315.19	315.19	315.19	315.19	315.19
22.250	315.19	315.19	315.18	315.18	315.18
22.500	315.18	315.18	315.18	315.18	315.18
22.750	315.18	315.18	315.18	315.17	315.17
23.000	315.17	315.17	315.17	315.17	315.17
23.250	315.17	315.17	315.17	315.17	315.16
23.500	315.16	315.16	315.16	315.16	315.16
23.750	315.16	315.16	315.16	315.16	315.16
24.000	315.15	315.15	315.14	315.13	315.12
24.250	315.11	315.10	315.09	315.08	315.08
24.500	315.07	315.06	315.06	315.05	315.05
24.750	315.04	315.04	315.04	315.03	315.03
25.000	315.03	315.03	315.02	315.02	315.02
25.250	315.02	315.02	315.01	315.01	315.01
25.500	315.01	315.01	315.01	315.01	315.01
25.750	315.01	315.01	315.01	315.01	315.00
26.000	315.00	315.00	315.00	315.00	315.00
26.250	315.00	315.00	315.00	315.00	315.00
26.500	315.00	315.00	315.00	315.00	315.00
26.750	315.00	315.00	315.00	315.00	315.00
27.000	315.00	315.00	315.00	315.00	315.00
27.250	315.00	315.00	315.00	315.00	315.00
27.500	315.00	315.00	315.00	315.00	315.00
27.750	315.00	315.00	315.00	315.00	315.00
28.000	315.00	315.00	315.00	315.00	315.00
28.250	315.00	315.00	315.00	315.00	315.00
28.500	315.00	315.00	315.00	315.00	315.00
28.750	315.00	315.00	315.00	315.00	315.00
29.000	315.00	315.00	315.00	315.00	315.00
29.250	315.00	315.00	315.00	315.00	315.00
29.500	315.00	315.00	315.00	315.00	315.00
29.750	315.00	315.00	315.00	315.00	315.00
30.000	315.00	315.00	315.00	315.00	315.00
30.250	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	315.00	315.00	315.00	315.00	315.00
30.750	315.00	315.00	315.00	315.00	315.00
31.000	315.00	315.00	315.00	315.00	315.00
31.250	315.00	315.00	315.00	315.00	315.00
31.500	315.00	315.00	315.00	315.00	315.00
31.750	315.00	315.00	315.00	315.00	315.00
32.000	315.00	315.00	315.00	315.00	315.00
32.250	315.00	315.00	315.00	315.00	315.00
32.500	315.00	315.00	315.00	315.00	315.00
32.750	315.00	315.00	315.00	315.00	315.00
33.000	315.00	315.00	315.00	315.00	315.00
33.250	315.00	315.00	315.00	315.00	315.00
33.500	315.00	315.00	315.00	315.00	315.00
33.750	315.00	315.00	315.00	315.00	315.00
34.000	315.00	315.00	315.00	315.00	315.00
34.250	315.00	315.00	315.00	315.00	315.00
34.500	315.00	315.00	315.00	315.00	315.00
34.750	315.00	315.00	315.00	315.00	315.00
35.000	315.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	315.00	315.00	315.00	315.00	315.00
0.250	315.00	315.00	315.00	315.00	315.00
0.500	315.00	315.00	315.00	315.00	315.00
0.750	315.00	315.00	315.00	315.00	315.00
1.000	315.00	315.00	315.00	315.00	315.00
1.250	315.00	315.00	315.00	315.00	315.00
1.500	315.00	315.00	315.00	315.00	315.00
1.750	315.00	315.00	315.00	315.00	315.00
2.000	315.00	315.00	315.00	315.00	315.00
2.250	315.00	315.00	315.00	315.00	315.00
2.500	315.00	315.00	315.00	315.00	315.00
2.750	315.00	315.00	315.00	315.00	315.00
3.000	315.00	315.00	315.00	315.00	315.00
3.250	315.00	315.00	315.00	315.00	315.00
3.500	315.00	315.00	315.00	315.00	315.00
3.750	315.00	315.00	315.00	315.00	315.00
4.000	315.00	315.00	315.00	315.00	315.00
4.250	315.00	315.00	315.00	315.00	315.00
4.500	315.00	315.00	315.00	315.00	315.00
4.750	315.00	315.00	315.00	315.00	315.00
5.000	315.00	315.00	315.00	315.00	315.00
5.250	315.00	315.00	315.00	315.00	315.00
5.500	315.00	315.00	315.00	315.00	315.00
5.750	315.00	315.00	315.00	315.00	315.00
6.000	315.00	315.00	315.00	315.00	315.00
6.250	315.00	315.00	315.00	315.00	315.00
6.500	315.00	315.00	315.00	315.00	315.00
6.750	315.00	315.00	315.00	315.00	315.00
7.000	315.00	315.00	315.00	315.00	315.00
7.250	315.00	315.00	315.01	315.01	315.01
7.500	315.01	315.01	315.01	315.02	315.02
7.750	315.02	315.02	315.02	315.03	315.03
8.000	315.03	315.04	315.04	315.04	315.04
8.250	315.05	315.05	315.06	315.06	315.06
8.500	315.07	315.07	315.08	315.08	315.09
8.750	315.09	315.10	315.10	315.11	315.11
9.000	315.12	315.13	315.13	315.14	315.15
9.250	315.15	315.16	315.17	315.18	315.18
9.500	315.19	315.20	315.21	315.22	315.23
9.750	315.23	315.24	315.25	315.26	315.27

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	315.28	315.29	315.30	315.31	315.32
10.250	315.34	315.35	315.36	315.38	315.39
10.500	315.41	315.42	315.44	315.45	315.47
10.750	315.49	315.50	315.52	315.53	315.54
11.000	315.56	315.57	315.58	315.59	315.61
11.250	315.62	315.64	315.66	315.68	315.70
11.500	315.73	315.76	315.80	315.87	315.96
11.750	316.07	316.20	316.34	316.50	316.70
12.000	317.03	317.43	317.81	318.11	318.27
12.250	318.31	318.29	318.22	318.12	318.00
12.500	317.84	317.66	317.47	317.27	317.08
12.750	316.90	316.74	316.59	316.46	316.34
13.000	316.24	316.16	316.10	316.05	316.01
13.250	315.97	315.94	315.92	315.90	315.88
13.500	315.86	315.84	315.83	315.82	315.80
13.750	315.79	315.78	315.77	315.76	315.75
14.000	315.74	315.73	315.73	315.72	315.71
14.250	315.70	315.70	315.69	315.69	315.68
14.500	315.68	315.67	315.67	315.66	315.66
14.750	315.65	315.65	315.64	315.64	315.64
15.000	315.63	315.63	315.62	315.62	315.61
15.250	315.61	315.61	315.60	315.60	315.59
15.500	315.59	315.59	315.58	315.58	315.57
15.750	315.57	315.56	315.56	315.56	315.55
16.000	315.55	315.54	315.54	315.54	315.53
16.250	315.53	315.53	315.52	315.52	315.52
16.500	315.52	315.51	315.51	315.51	315.51
16.750	315.51	315.50	315.50	315.50	315.50
17.000	315.49	315.49	315.49	315.48	315.48
17.250	315.48	315.47	315.47	315.47	315.46
17.500	315.46	315.45	315.45	315.44	315.44
17.750	315.43	315.43	315.42	315.42	315.42
18.000	315.41	315.41	315.40	315.40	315.39
18.250	315.39	315.38	315.38	315.38	315.37
18.500	315.37	315.37	315.36	315.36	315.36
18.750	315.35	315.35	315.35	315.35	315.35
19.000	315.34	315.34	315.34	315.34	315.33
19.250	315.33	315.33	315.33	315.33	315.32
19.500	315.32	315.32	315.32	315.32	315.32
19.750	315.31	315.31	315.31	315.31	315.31
20.000	315.31	315.30	315.30	315.30	315.30

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	315.30	315.30	315.29	315.29	315.29
20.500	315.29	315.29	315.29	315.29	315.28
20.750	315.28	315.28	315.28	315.28	315.28
21.000	315.28	315.28	315.27	315.27	315.27
21.250	315.27	315.27	315.27	315.27	315.27
21.500	315.26	315.26	315.26	315.26	315.26
21.750	315.26	315.26	315.26	315.25	315.25
22.000	315.25	315.25	315.25	315.25	315.25
22.250	315.25	315.24	315.24	315.24	315.24
22.500	315.24	315.24	315.24	315.24	315.23
22.750	315.23	315.23	315.23	315.23	315.23
23.000	315.23	315.23	315.22	315.22	315.22
23.250	315.22	315.22	315.22	315.22	315.22
23.500	315.22	315.21	315.21	315.21	315.21
23.750	315.21	315.21	315.21	315.21	315.20
24.000	315.20	315.20	315.19	315.18	315.16
24.250	315.15	315.13	315.12	315.11	315.10
24.500	315.09	315.08	315.08	315.07	315.06
24.750	315.06	315.05	315.05	315.04	315.04
25.000	315.04	315.03	315.03	315.03	315.02
25.250	315.02	315.02	315.02	315.02	315.02
25.500	315.01	315.01	315.01	315.01	315.01
25.750	315.01	315.01	315.01	315.01	315.01
26.000	315.01	315.01	315.00	315.00	315.00
26.250	315.00	315.00	315.00	315.00	315.00
26.500	315.00	315.00	315.00	315.00	315.00
26.750	315.00	315.00	315.00	315.00	315.00
27.000	315.00	315.00	315.00	315.00	315.00
27.250	315.00	315.00	315.00	315.00	315.00
27.500	315.00	315.00	315.00	315.00	315.00
27.750	315.00	315.00	315.00	315.00	315.00
28.000	315.00	315.00	315.00	315.00	315.00
28.250	315.00	315.00	315.00	315.00	315.00
28.500	315.00	315.00	315.00	315.00	315.00
28.750	315.00	315.00	315.00	315.00	315.00
29.000	315.00	315.00	315.00	315.00	315.00
29.250	315.00	315.00	315.00	315.00	315.00
29.500	315.00	315.00	315.00	315.00	315.00
29.750	315.00	315.00	315.00	315.00	315.00
30.000	315.00	315.00	315.00	315.00	315.00
30.250	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	315.00	315.00	315.00	315.00	315.00
30.750	315.00	315.00	315.00	315.00	315.00
31.000	315.00	315.00	315.00	315.00	315.00
31.250	315.00	315.00	315.00	315.00	315.00
31.500	315.00	315.00	315.00	315.00	315.00
31.750	315.00	315.00	315.00	315.00	315.00
32.000	315.00	315.00	315.00	315.00	315.00
32.250	315.00	315.00	315.00	315.00	315.00
32.500	315.00	315.00	315.00	315.00	315.00
32.750	315.00	315.00	315.00	315.00	315.00
33.000	315.00	315.00	315.00	315.00	315.00
33.250	315.00	315.00	315.00	315.00	315.00
33.500	315.00	315.00	315.00	315.00	315.00
33.750	315.00	315.00	315.00	315.00	315.00
34.000	315.00	315.00	315.00	315.00	315.00
34.250	315.00	315.00	315.00	315.00	315.00
34.500	315.00	315.00	315.00	315.00	315.00
34.750	315.00	315.00	315.00	315.00	315.00
35.000	315.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	315.00	315.00	315.00	315.00	315.00
0.250	315.00	315.00	315.00	315.00	315.00
0.500	315.00	315.00	315.00	315.00	315.00
0.750	315.00	315.00	315.00	315.00	315.00
1.000	315.00	315.00	315.00	315.00	315.00
1.250	315.00	315.00	315.00	315.00	315.00
1.500	315.00	315.00	315.00	315.00	315.00
1.750	315.00	315.00	315.00	315.00	315.00
2.000	315.00	315.00	315.00	315.00	315.00
2.250	315.00	315.00	315.00	315.00	315.00
2.500	315.00	315.00	315.00	315.00	315.00
2.750	315.00	315.00	315.00	315.00	315.00
3.000	315.00	315.00	315.00	315.00	315.00
3.250	315.00	315.00	315.00	315.00	315.00
3.500	315.00	315.00	315.00	315.00	315.00
3.750	315.00	315.00	315.00	315.00	315.00
4.000	315.00	315.00	315.00	315.00	315.00
4.250	315.00	315.00	315.00	315.00	315.00
4.500	315.00	315.00	315.00	315.00	315.00
4.750	315.00	315.00	315.00	315.00	315.00
5.000	315.00	315.00	315.00	315.00	315.00
5.250	315.00	315.00	315.00	315.00	315.00
5.500	315.00	315.00	315.00	315.00	315.00
5.750	315.00	315.00	315.01	315.01	315.01
6.000	315.01	315.01	315.01	315.01	315.02
6.250	315.02	315.02	315.02	315.03	315.03
6.500	315.03	315.03	315.04	315.04	315.04
6.750	315.05	315.05	315.05	315.06	315.06
7.000	315.06	315.07	315.07	315.08	315.08
7.250	315.08	315.09	315.09	315.10	315.10
7.500	315.11	315.11	315.12	315.12	315.13
7.750	315.13	315.14	315.14	315.15	315.16
8.000	315.16	315.17	315.17	315.18	315.19
8.250	315.19	315.20	315.21	315.22	315.23
8.500	315.23	315.24	315.25	315.26	315.27
8.750	315.28	315.29	315.30	315.31	315.33
9.000	315.34	315.35	315.36	315.37	315.39
9.250	315.40	315.41	315.43	315.44	315.45
9.500	315.47	315.48	315.50	315.51	315.52
9.750	315.53	315.54	315.55	315.56	315.57

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	315.57	315.58	315.59	315.60	315.61
10.250	315.61	315.62	315.63	315.64	315.65
10.500	315.67	315.68	315.69	315.70	315.71
10.750	315.72	315.74	315.75	315.76	315.77
11.000	315.79	315.80	315.82	315.83	315.86
11.250	315.88	315.91	315.94	315.98	316.01
11.500	316.04	316.08	316.14	316.23	316.36
11.750	316.52	316.70	316.93	317.17	317.50
12.000	317.97	318.53	319.06	319.49	319.71
12.250	319.77	319.75	319.69	319.60	319.48
12.500	319.33	319.15	318.94	318.72	318.50
12.750	318.27	318.05	317.83	317.62	317.42
13.000	317.23	317.06	316.89	316.74	316.60
13.250	316.49	316.38	316.30	316.23	316.18
13.500	316.14	316.11	316.08	316.06	316.04
13.750	316.02	316.01	316.00	315.98	315.97
14.000	315.95	315.94	315.93	315.91	315.90
14.250	315.89	315.88	315.87	315.87	315.86
14.500	315.85	315.84	315.84	315.83	315.82
14.750	315.82	315.81	315.80	315.80	315.79
15.000	315.78	315.78	315.77	315.76	315.76
15.250	315.75	315.75	315.74	315.73	315.73
15.500	315.72	315.71	315.71	315.70	315.69
15.750	315.69	315.68	315.68	315.67	315.66
16.000	315.66	315.65	315.64	315.64	315.63
16.250	315.63	315.62	315.62	315.62	315.61
16.500	315.61	315.61	315.60	315.60	315.60
16.750	315.59	315.59	315.59	315.58	315.58
17.000	315.58	315.58	315.57	315.57	315.57
17.250	315.56	315.56	315.56	315.56	315.55
17.500	315.55	315.55	315.55	315.54	315.54
17.750	315.54	315.53	315.53	315.53	315.53
18.000	315.52	315.52	315.52	315.51	315.51
18.250	315.51	315.51	315.51	315.51	315.51
18.500	315.50	315.50	315.50	315.50	315.50
18.750	315.50	315.50	315.50	315.49	315.49
19.000	315.49	315.49	315.49	315.49	315.48
19.250	315.48	315.48	315.48	315.48	315.47
19.500	315.47	315.47	315.47	315.47	315.46
19.750	315.46	315.46	315.46	315.45	315.45
20.000	315.45	315.45	315.45	315.44	315.44

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	315.44	315.44	315.43	315.43	315.43
20.500	315.43	315.43	315.42	315.42	315.42
20.750	315.42	315.42	315.41	315.41	315.41
21.000	315.41	315.41	315.41	315.40	315.40
21.250	315.40	315.40	315.40	315.39	315.39
21.500	315.39	315.39	315.39	315.38	315.38
21.750	315.38	315.38	315.38	315.38	315.37
22.000	315.37	315.37	315.37	315.37	315.36
22.250	315.36	315.36	315.36	315.36	315.36
22.500	315.35	315.35	315.35	315.35	315.35
22.750	315.34	315.34	315.34	315.34	315.34
23.000	315.34	315.33	315.33	315.33	315.33
23.250	315.33	315.32	315.32	315.32	315.32
23.500	315.32	315.32	315.31	315.31	315.31
23.750	315.31	315.31	315.30	315.30	315.30
24.000	315.30	315.29	315.28	315.26	315.24
24.250	315.22	315.20	315.18	315.16	315.15
24.500	315.14	315.12	315.11	315.10	315.09
24.750	315.09	315.08	315.07	315.06	315.06
25.000	315.05	315.05	315.04	315.04	315.04
25.250	315.03	315.03	315.03	315.03	315.02
25.500	315.02	315.02	315.02	315.02	315.01
25.750	315.01	315.01	315.01	315.01	315.01
26.000	315.01	315.01	315.01	315.01	315.01
26.250	315.01	315.00	315.00	315.00	315.00
26.500	315.00	315.00	315.00	315.00	315.00
26.750	315.00	315.00	315.00	315.00	315.00
27.000	315.00	315.00	315.00	315.00	315.00
27.250	315.00	315.00	315.00	315.00	315.00
27.500	315.00	315.00	315.00	315.00	315.00
27.750	315.00	315.00	315.00	315.00	315.00
28.000	315.00	315.00	315.00	315.00	315.00
28.250	315.00	315.00	315.00	315.00	315.00
28.500	315.00	315.00	315.00	315.00	315.00
28.750	315.00	315.00	315.00	315.00	315.00
29.000	315.00	315.00	315.00	315.00	315.00
29.250	315.00	315.00	315.00	315.00	315.00
29.500	315.00	315.00	315.00	315.00	315.00
29.750	315.00	315.00	315.00	315.00	315.00
30.000	315.00	315.00	315.00	315.00	315.00
30.250	315.00	315.00	315.00	315.00	315.00

Watershed

Subsection: Time vs. Elevation

Label: Det Basin #1C (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	315.00	315.00	315.00	315.00	315.00
30.750	315.00	315.00	315.00	315.00	315.00
31.000	315.00	315.00	315.00	315.00	315.00
31.250	315.00	315.00	315.00	315.00	315.00
31.500	315.00	315.00	315.00	315.00	315.00
31.750	315.00	315.00	315.00	315.00	315.00
32.000	315.00	315.00	315.00	315.00	315.00
32.250	315.00	315.00	315.00	315.00	315.00
32.500	315.00	315.00	315.00	315.00	315.00
32.750	315.00	315.00	315.00	315.00	315.00
33.000	315.00	315.00	315.00	315.00	315.00
33.250	315.00	315.00	315.00	315.00	315.00
33.500	315.00	315.00	315.00	315.00	315.00
33.750	315.00	315.00	315.00	315.00	315.00
34.000	315.00	315.00	315.00	315.00	315.00
34.250	315.00	315.00	315.00	315.00	315.00
34.500	315.00	315.00	315.00	315.00	315.00
34.750	315.00	315.00	315.00	315.00	315.00
35.000	315.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.00	296.00	296.00	296.00	296.00
12.250	296.00	296.00	296.00	296.00	296.00
12.500	296.00	296.00	296.00	296.00	296.00
12.750	296.00	296.00	296.00	296.00	296.00
13.000	296.00	296.00	296.00	296.00	296.00
13.250	296.00	296.00	296.00	296.00	296.00
13.500	296.00	296.00	296.00	296.00	296.00
13.750	296.00	296.00	296.00	296.00	296.00
14.000	296.00	296.00	296.00	296.00	296.00
14.250	296.00	296.00	296.00	296.00	296.00
14.500	296.00	296.00	296.00	296.00	296.00
14.750	296.00	296.00	296.00	296.00	296.00
15.000	296.00	296.00	296.00	296.00	296.00
15.250	296.00	296.00	296.00	296.00	296.00
15.500	296.00	296.00	296.00	296.00	296.00
15.750	296.00	296.00	296.00	296.00	296.00
16.000	296.00	296.00	296.00	296.00	296.00
16.250	296.00	296.00	296.00	296.00	296.00
16.500	296.00	296.00	296.00	296.00	296.00
16.750	296.00	296.00	296.00	296.00	296.00
17.000	296.00	296.00	296.00	296.00	296.00
17.250	296.00	296.00	296.00	296.00	296.00
17.500	296.00	296.00	296.00	296.00	296.00
17.750	296.00	296.00	296.00	296.00	296.00
18.000	296.00	296.00	296.00	296.00	296.00
18.250	296.00	296.00	296.00	296.00	296.00
18.500	296.00	296.00	296.00	296.00	296.00
18.750	296.00	296.00	296.00	296.00	296.00
19.000	296.00	296.00	296.00	296.00	296.00
19.250	296.00	296.00	296.00	296.00	296.00
19.500	296.00	296.00	296.00	296.00	296.00
19.750	296.00	296.00	296.00	296.00	296.00
20.000	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.00	296.00	296.00	296.00	296.00
20.500	296.00	296.00	296.00	296.00	296.00
20.750	296.00	296.00	296.00	296.00	296.00
21.000	296.00	296.00	296.00	296.00	296.00
21.250	296.00	296.00	296.00	296.00	296.00
21.500	296.00	296.00	296.00	296.00	296.00
21.750	296.00	296.00	296.00	296.00	296.00
22.000	296.00	296.00	296.00	296.00	296.00
22.250	296.00	296.00	296.00	296.00	296.00
22.500	296.00	296.00	296.00	296.00	296.00
22.750	296.00	296.00	296.00	296.00	296.00
23.000	296.00	296.00	296.00	296.00	296.00
23.250	296.00	296.00	296.00	296.00	296.00
23.500	296.00	296.00	296.00	296.00	296.00
23.750	296.00	296.00	296.00	296.00	296.00
24.000	296.00	296.00	296.00	296.00	296.00
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.00	296.00	296.00	296.00	296.00
12.250	296.00	296.00	296.00	296.00	296.00
12.500	296.00	296.00	296.00	296.00	296.00
12.750	296.00	296.00	296.00	296.00	296.00
13.000	296.00	296.00	296.00	296.00	296.00
13.250	296.00	296.00	296.00	296.00	296.00
13.500	296.00	296.00	296.00	296.00	296.00
13.750	296.00	296.00	296.00	296.00	296.00
14.000	296.00	296.00	296.00	296.00	296.00
14.250	296.00	296.00	296.00	296.00	296.00
14.500	296.00	296.00	296.00	296.00	296.00
14.750	296.00	296.00	296.00	296.00	296.00
15.000	296.00	296.00	296.00	296.00	296.00
15.250	296.00	296.00	296.00	296.00	296.00
15.500	296.00	296.00	296.00	296.00	296.00
15.750	296.00	296.00	296.00	296.00	296.00
16.000	296.00	296.00	296.00	296.00	296.00
16.250	296.00	296.00	296.00	296.00	296.00
16.500	296.00	296.00	296.00	296.00	296.00
16.750	296.00	296.00	296.00	296.00	296.00
17.000	296.00	296.00	296.00	296.00	296.00
17.250	296.00	296.00	296.00	296.00	296.00
17.500	296.00	296.00	296.00	296.00	296.00
17.750	296.00	296.00	296.00	296.00	296.00
18.000	296.00	296.00	296.00	296.00	296.00
18.250	296.00	296.00	296.00	296.00	296.00
18.500	296.00	296.00	296.00	296.00	296.00
18.750	296.00	296.00	296.00	296.00	296.00
19.000	296.00	296.00	296.00	296.00	296.00
19.250	296.00	296.00	296.00	296.00	296.00
19.500	296.00	296.00	296.00	296.00	296.00
19.750	296.00	296.00	296.00	296.00	296.00
20.000	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.00	296.00	296.00	296.00	296.00
20.500	296.00	296.00	296.00	296.00	296.00
20.750	296.00	296.00	296.00	296.00	296.00
21.000	296.00	296.00	296.00	296.00	296.00
21.250	296.00	296.00	296.00	296.00	296.00
21.500	296.00	296.00	296.00	296.00	296.00
21.750	296.00	296.00	296.00	296.00	296.00
22.000	296.00	296.00	296.00	296.00	296.00
22.250	296.00	296.00	296.00	296.00	296.00
22.500	296.00	296.00	296.00	296.00	296.00
22.750	296.00	296.00	296.00	296.00	296.00
23.000	296.00	296.00	296.00	296.00	296.00
23.250	296.00	296.00	296.00	296.00	296.00
23.500	296.00	296.00	296.00	296.00	296.00
23.750	296.00	296.00	296.00	296.00	296.00
24.000	296.00	296.00	296.00	296.00	296.00
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.00	296.00	296.00	296.00	296.00
12.250	296.00	296.00	296.00	296.00	296.00
12.500	296.00	296.00	296.00	296.00	296.00
12.750	296.00	296.00	296.00	296.00	296.00
13.000	296.00	296.00	296.00	296.00	296.00
13.250	296.00	296.00	296.00	296.00	296.00
13.500	296.00	296.00	296.00	296.00	296.00
13.750	296.00	296.00	296.00	296.00	296.00
14.000	296.00	296.00	296.00	296.00	296.00
14.250	296.00	296.00	296.00	296.00	296.00
14.500	296.00	296.00	296.00	296.00	296.00
14.750	296.00	296.00	296.00	296.00	296.00
15.000	296.00	296.00	296.00	296.00	296.00
15.250	296.00	296.00	296.00	296.00	296.00
15.500	296.00	296.00	296.00	296.00	296.00
15.750	296.00	296.00	296.00	296.00	296.00
16.000	296.00	296.00	296.00	296.00	296.00
16.250	296.00	296.00	296.00	296.00	296.00
16.500	296.00	296.00	296.00	296.00	296.00
16.750	296.00	296.00	296.00	296.00	296.00
17.000	296.00	296.00	296.00	296.00	296.00
17.250	296.00	296.00	296.00	296.00	296.00
17.500	296.00	296.00	296.00	296.00	296.00
17.750	296.00	296.00	296.00	296.00	296.00
18.000	296.00	296.00	296.00	296.00	296.00
18.250	296.00	296.00	296.00	296.00	296.00
18.500	296.00	296.00	296.00	296.00	296.00
18.750	296.00	296.00	296.00	296.00	296.00
19.000	296.00	296.00	296.00	296.00	296.00
19.250	296.00	296.00	296.00	296.00	296.00
19.500	296.00	296.00	296.00	296.00	296.00
19.750	296.00	296.00	296.00	296.00	296.00
20.000	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.00	296.00	296.00	296.00	296.00
20.500	296.00	296.00	296.00	296.00	296.00
20.750	296.00	296.00	296.00	296.00	296.00
21.000	296.00	296.00	296.00	296.00	296.00
21.250	296.00	296.00	296.00	296.00	296.00
21.500	296.00	296.00	296.00	296.00	296.00
21.750	296.00	296.00	296.00	296.00	296.00
22.000	296.00	296.00	296.00	296.00	296.00
22.250	296.00	296.00	296.00	296.00	296.00
22.500	296.00	296.00	296.00	296.00	296.00
22.750	296.00	296.00	296.00	296.00	296.00
23.000	296.00	296.00	296.00	296.00	296.00
23.250	296.00	296.00	296.00	296.00	296.00
23.500	296.00	296.00	296.00	296.00	296.00
23.750	296.00	296.00	296.00	296.00	296.00
24.000	296.00	296.00	296.00	296.00	296.00
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.00	296.00	296.00	296.00	296.00
12.250	296.13	296.50	296.62	296.72	296.75
12.500	296.73	296.68	296.62	296.58	296.55
12.750	296.53	296.51	296.47	296.39	296.32
13.000	296.26	296.20	296.14	296.09	296.04
13.250	296.01	296.00	296.00	296.00	296.00
13.500	296.00	296.00	296.00	296.00	296.00
13.750	296.00	296.00	296.00	296.00	296.00
14.000	296.00	296.00	296.00	296.00	296.00
14.250	296.00	296.00	296.00	296.00	296.00
14.500	296.00	296.00	296.00	296.00	296.00
14.750	296.00	296.00	296.00	296.00	296.00
15.000	296.00	296.00	296.00	296.00	296.00
15.250	296.00	296.00	296.00	296.00	296.00
15.500	296.00	296.00	296.00	296.00	296.00
15.750	296.00	296.00	296.00	296.00	296.00
16.000	296.00	296.00	296.00	296.00	296.00
16.250	296.00	296.00	296.00	296.00	296.00
16.500	296.00	296.00	296.00	296.00	296.00
16.750	296.00	296.00	296.00	296.00	296.00
17.000	296.00	296.00	296.00	296.00	296.00
17.250	296.00	296.00	296.00	296.00	296.00
17.500	296.00	296.00	296.00	296.00	296.00
17.750	296.00	296.00	296.00	296.00	296.00
18.000	296.00	296.00	296.00	296.00	296.00
18.250	296.00	296.00	296.00	296.00	296.00
18.500	296.00	296.00	296.00	296.00	296.00
18.750	296.00	296.00	296.00	296.00	296.00
19.000	296.00	296.00	296.00	296.00	296.00
19.250	296.00	296.00	296.00	296.00	296.00
19.500	296.00	296.00	296.00	296.00	296.00
19.750	296.00	296.00	296.00	296.00	296.00
20.000	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.00	296.00	296.00	296.00	296.00
20.500	296.00	296.00	296.00	296.00	296.00
20.750	296.00	296.00	296.00	296.00	296.00
21.000	296.00	296.00	296.00	296.00	296.00
21.250	296.00	296.00	296.00	296.00	296.00
21.500	296.00	296.00	296.00	296.00	296.00
21.750	296.00	296.00	296.00	296.00	296.00
22.000	296.00	296.00	296.00	296.00	296.00
22.250	296.00	296.00	296.00	296.00	296.00
22.500	296.00	296.00	296.00	296.00	296.00
22.750	296.00	296.00	296.00	296.00	296.00
23.000	296.00	296.00	296.00	296.00	296.00
23.250	296.00	296.00	296.00	296.00	296.00
23.500	296.00	296.00	296.00	296.00	296.00
23.750	296.00	296.00	296.00	296.00	296.00
24.000	296.00	296.00	296.00	296.00	296.00
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	296.00	296.00	296.00	296.00	296.00
0.250	296.00	296.00	296.00	296.00	296.00
0.500	296.00	296.00	296.00	296.00	296.00
0.750	296.00	296.00	296.00	296.00	296.00
1.000	296.00	296.00	296.00	296.00	296.00
1.250	296.00	296.00	296.00	296.00	296.00
1.500	296.00	296.00	296.00	296.00	296.00
1.750	296.00	296.00	296.00	296.00	296.00
2.000	296.00	296.00	296.00	296.00	296.00
2.250	296.00	296.00	296.00	296.00	296.00
2.500	296.00	296.00	296.00	296.00	296.00
2.750	296.00	296.00	296.00	296.00	296.00
3.000	296.00	296.00	296.00	296.00	296.00
3.250	296.00	296.00	296.00	296.00	296.00
3.500	296.00	296.00	296.00	296.00	296.00
3.750	296.00	296.00	296.00	296.00	296.00
4.000	296.00	296.00	296.00	296.00	296.00
4.250	296.00	296.00	296.00	296.00	296.00
4.500	296.00	296.00	296.00	296.00	296.00
4.750	296.00	296.00	296.00	296.00	296.00
5.000	296.00	296.00	296.00	296.00	296.00
5.250	296.00	296.00	296.00	296.00	296.00
5.500	296.00	296.00	296.00	296.00	296.00
5.750	296.00	296.00	296.00	296.00	296.00
6.000	296.00	296.00	296.00	296.00	296.00
6.250	296.00	296.00	296.00	296.00	296.00
6.500	296.00	296.00	296.00	296.00	296.00
6.750	296.00	296.00	296.00	296.00	296.00
7.000	296.00	296.00	296.00	296.00	296.00
7.250	296.00	296.00	296.00	296.00	296.00
7.500	296.00	296.00	296.00	296.00	296.00
7.750	296.00	296.00	296.00	296.00	296.00
8.000	296.00	296.00	296.00	296.00	296.00
8.250	296.00	296.00	296.00	296.00	296.00
8.500	296.00	296.00	296.00	296.00	296.00
8.750	296.00	296.00	296.00	296.00	296.00
9.000	296.00	296.00	296.00	296.00	296.00
9.250	296.00	296.00	296.00	296.00	296.00
9.500	296.00	296.00	296.00	296.00	296.00
9.750	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	296.00	296.00	296.00	296.00	296.00
10.250	296.00	296.00	296.00	296.00	296.00
10.500	296.00	296.00	296.00	296.00	296.00
10.750	296.00	296.00	296.00	296.00	296.00
11.000	296.00	296.00	296.00	296.00	296.00
11.250	296.00	296.00	296.00	296.00	296.00
11.500	296.00	296.00	296.00	296.00	296.00
11.750	296.00	296.00	296.00	296.00	296.00
12.000	296.00	296.26	296.93	297.37	297.56
12.250	297.52	297.40	297.29	297.19	297.13
12.500	297.08	297.02	296.91	296.80	296.72
12.750	296.65	296.61	296.59	296.57	296.56
13.000	296.55	296.54	296.52	296.51	296.50
13.250	296.45	296.41	296.37	296.34	296.31
13.500	296.28	296.25	296.23	296.20	296.18
13.750	296.16	296.14	296.12	296.10	296.08
14.000	296.06	296.04	296.02	296.00	296.00
14.250	296.00	296.00	296.00	296.00	296.00
14.500	296.00	296.00	296.00	296.00	296.00
14.750	296.00	296.00	296.00	296.00	296.00
15.000	296.00	296.00	296.00	296.00	296.00
15.250	296.00	296.00	296.00	296.00	296.00
15.500	296.00	296.00	296.00	296.00	296.00
15.750	296.00	296.00	296.00	296.00	296.00
16.000	296.00	296.00	296.00	296.00	296.00
16.250	296.00	296.00	296.00	296.00	296.00
16.500	296.00	296.00	296.00	296.00	296.00
16.750	296.00	296.00	296.00	296.00	296.00
17.000	296.00	296.00	296.00	296.00	296.00
17.250	296.00	296.00	296.00	296.00	296.00
17.500	296.00	296.00	296.00	296.00	296.00
17.750	296.00	296.00	296.00	296.00	296.00
18.000	296.00	296.00	296.00	296.00	296.00
18.250	296.00	296.00	296.00	296.00	296.00
18.500	296.00	296.00	296.00	296.00	296.00
18.750	296.00	296.00	296.00	296.00	296.00
19.000	296.00	296.00	296.00	296.00	296.00
19.250	296.00	296.00	296.00	296.00	296.00
19.500	296.00	296.00	296.00	296.00	296.00
19.750	296.00	296.00	296.00	296.00	296.00
20.000	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	296.00	296.00	296.00	296.00	296.00
20.500	296.00	296.00	296.00	296.00	296.00
20.750	296.00	296.00	296.00	296.00	296.00
21.000	296.00	296.00	296.00	296.00	296.00
21.250	296.00	296.00	296.00	296.00	296.00
21.500	296.00	296.00	296.00	296.00	296.00
21.750	296.00	296.00	296.00	296.00	296.00
22.000	296.00	296.00	296.00	296.00	296.00
22.250	296.00	296.00	296.00	296.00	296.00
22.500	296.00	296.00	296.00	296.00	296.00
22.750	296.00	296.00	296.00	296.00	296.00
23.000	296.00	296.00	296.00	296.00	296.00
23.250	296.00	296.00	296.00	296.00	296.00
23.500	296.00	296.00	296.00	296.00	296.00
23.750	296.00	296.00	296.00	296.00	296.00
24.000	296.00	296.00	296.00	296.00	296.00
24.250	296.00	296.00	296.00	296.00	296.00
24.500	296.00	296.00	296.00	296.00	296.00
24.750	296.00	296.00	296.00	296.00	296.00
25.000	296.00	296.00	296.00	296.00	296.00
25.250	296.00	296.00	296.00	296.00	296.00
25.500	296.00	296.00	296.00	296.00	296.00
25.750	296.00	296.00	296.00	296.00	296.00
26.000	296.00	296.00	296.00	296.00	296.00
26.250	296.00	296.00	296.00	296.00	296.00
26.500	296.00	296.00	296.00	296.00	296.00
26.750	296.00	296.00	296.00	296.00	296.00
27.000	296.00	296.00	296.00	296.00	296.00
27.250	296.00	296.00	296.00	296.00	296.00
27.500	296.00	296.00	296.00	296.00	296.00
27.750	296.00	296.00	296.00	296.00	296.00
28.000	296.00	296.00	296.00	296.00	296.00
28.250	296.00	296.00	296.00	296.00	296.00
28.500	296.00	296.00	296.00	296.00	296.00
28.750	296.00	296.00	296.00	296.00	296.00
29.000	296.00	296.00	296.00	296.00	296.00
29.250	296.00	296.00	296.00	296.00	296.00
29.500	296.00	296.00	296.00	296.00	296.00
29.750	296.00	296.00	296.00	296.00	296.00
30.000	296.00	296.00	296.00	296.00	296.00
30.250	296.00	296.00	296.00	296.00	296.00

Watershed

Subsection: Time vs. Elevation

Label: DP 4 Low Point (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	296.00	296.00	296.00	296.00	296.00
30.750	296.00	296.00	296.00	296.00	296.00
31.000	296.00	296.00	296.00	296.00	296.00
31.250	296.00	296.00	296.00	296.00	296.00
31.500	296.00	296.00	296.00	296.00	296.00
31.750	296.00	296.00	296.00	296.00	296.00
32.000	296.00	296.00	296.00	296.00	296.00
32.250	296.00	296.00	296.00	296.00	296.00
32.500	296.00	296.00	296.00	296.00	296.00
32.750	296.00	296.00	296.00	296.00	296.00
33.000	296.00	296.00	296.00	296.00	296.00
33.250	296.00	296.00	296.00	296.00	296.00
33.500	296.00	296.00	296.00	296.00	296.00
33.750	296.00	296.00	296.00	296.00	296.00
34.000	296.00	296.00	296.00	296.00	296.00
34.250	296.00	296.00	296.00	296.00	296.00
34.500	296.00	296.00	296.00	296.00	296.00
34.750	296.00	296.00	296.00	296.00	296.00
35.000	296.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.17
10.750	319.17	319.17	319.17	319.17	319.17
11.000	319.17	319.17	319.17	319.17	319.17
11.250	319.17	319.17	319.17	319.17	319.17
11.500	319.17	319.17	319.17	319.17	319.17
11.750	319.17	319.17	319.17	319.17	319.18
12.000	319.18	319.19	319.20	319.21	319.22
12.250	319.23	319.23	319.24	319.25	319.25
12.500	319.26	319.26	319.27	319.27	319.27
12.750	319.27	319.28	319.28	319.28	319.28
13.000	319.28	319.29	319.29	319.29	319.29
13.250	319.29	319.29	319.29	319.30	319.30
13.500	319.30	319.30	319.30	319.30	319.30
13.750	319.30	319.31	319.31	319.31	319.31
14.000	319.31	319.31	319.31	319.31	319.31
14.250	319.31	319.31	319.31	319.31	319.32
14.500	319.32	319.32	319.32	319.32	319.32
14.750	319.32	319.32	319.32	319.32	319.32
15.000	319.32	319.32	319.32	319.32	319.32
15.250	319.32	319.32	319.32	319.32	319.32
15.500	319.33	319.33	319.33	319.33	319.33
15.750	319.33	319.33	319.33	319.33	319.33
16.000	319.33	319.33	319.33	319.33	319.33
16.250	319.33	319.33	319.33	319.33	319.33
16.500	319.33	319.33	319.33	319.33	319.33
16.750	319.33	319.33	319.33	319.33	319.33
17.000	319.33	319.33	319.33	319.33	319.33
17.250	319.33	319.33	319.33	319.33	319.33
17.500	319.33	319.33	319.33	319.33	319.33
17.750	319.33	319.33	319.32	319.32	319.32
18.000	319.32	319.32	319.32	319.32	319.32
18.250	319.32	319.32	319.32	319.32	319.32
18.500	319.32	319.32	319.32	319.32	319.32
18.750	319.32	319.32	319.32	319.32	319.32
19.000	319.32	319.32	319.32	319.32	319.32
19.250	319.32	319.32	319.32	319.32	319.32
19.500	319.32	319.32	319.32	319.32	319.32
19.750	319.32	319.32	319.32	319.32	319.32
20.000	319.32	319.32	319.32	319.32	319.32

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.32	319.32	319.32	319.32	319.32
20.500	319.31	319.31	319.31	319.31	319.31
20.750	319.31	319.31	319.31	319.31	319.31
21.000	319.31	319.31	319.31	319.31	319.31
21.250	319.31	319.31	319.31	319.31	319.31
21.500	319.31	319.31	319.31	319.31	319.31
21.750	319.31	319.31	319.31	319.31	319.31
22.000	319.31	319.31	319.31	319.31	319.31
22.250	319.31	319.31	319.31	319.31	319.31
22.500	319.31	319.31	319.31	319.31	319.31
22.750	319.31	319.31	319.31	319.31	319.31
23.000	319.31	319.31	319.31	319.31	319.31
23.250	319.31	319.30	319.30	319.30	319.30
23.500	319.30	319.30	319.30	319.30	319.30
23.750	319.30	319.30	319.30	319.30	319.30
24.000	319.30	319.30	319.30	319.30	319.30
24.250	319.30	319.30	319.30	319.30	319.30
24.500	319.30	319.30	319.30	319.30	319.30
24.750	319.30	319.30	319.30	319.30	319.30
25.000	319.29	319.29	319.29	319.29	319.29
25.250	319.29	319.29	319.29	319.29	319.29
25.500	319.29	319.29	319.29	319.29	319.29
25.750	319.29	319.29	319.29	319.29	319.29
26.000	319.29	319.29	319.29	319.29	319.29
26.250	319.29	319.29	319.29	319.29	319.29
26.500	319.29	319.28	319.28	319.28	319.28
26.750	319.28	319.28	319.28	319.28	319.28
27.000	319.28	319.28	319.28	319.28	319.28
27.250	319.28	319.28	319.28	319.28	319.28
27.500	319.28	319.28	319.28	319.28	319.28
27.750	319.28	319.28	319.28	319.28	319.28
28.000	319.28	319.28	319.28	319.28	319.28
28.250	319.28	319.28	319.28	319.28	319.28
28.500	319.28	319.28	319.28	319.28	319.28
28.750	319.28	319.28	319.28	319.27	319.27
29.000	319.27	319.27	319.27	319.27	319.27
29.250	319.27	319.27	319.27	319.27	319.27
29.500	319.27	319.27	319.27	319.27	319.27
29.750	319.27	319.27	319.27	319.27	319.27
30.000	319.27	319.27	319.27	319.27	319.27
30.250	319.27	319.27	319.27	319.27	319.27

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.27	319.27	319.27	319.27	319.27
30.750	319.27	319.27	319.27	319.27	319.27
31.000	319.27	319.27	319.27	319.27	319.27
31.250	319.27	319.27	319.27	319.27	319.27
31.500	319.27	319.27	319.27	319.27	319.27
31.750	319.27	319.27	319.27	319.27	319.27
32.000	319.27	319.27	319.27	319.27	319.27
32.250	319.27	319.27	319.27	319.27	319.27
32.500	319.27	319.27	319.27	319.27	319.27
32.750	319.27	319.27	319.27	319.26	319.26
33.000	319.26	319.26	319.26	319.26	319.26
33.250	319.26	319.26	319.26	319.26	319.26
33.500	319.26	319.26	319.26	319.26	319.26
33.750	319.26	319.26	319.26	319.26	319.26
34.000	319.26	319.26	319.26	319.26	319.26
34.250	319.26	319.26	319.26	319.26	319.26
34.500	319.26	319.26	319.26	319.26	319.26
34.750	319.26	319.26	319.26	319.26	319.26
35.000	319.26	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Existing Pond (OUT)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.17
10.750	319.17	319.17	319.17	319.17	319.17
11.000	319.17	319.17	319.17	319.17	319.17
11.250	319.17	319.17	319.17	319.17	319.17
11.500	319.17	319.17	319.17	319.17	319.17
11.750	319.18	319.18	319.18	319.19	319.19
12.000	319.20	319.21	319.23	319.25	319.27
12.250	319.28	319.30	319.31	319.32	319.34
12.500	319.35	319.36	319.37	319.38	319.39
12.750	319.40	319.41	319.41	319.42	319.42
13.000	319.43	319.43	319.44	319.44	319.45
13.250	319.45	319.45	319.45	319.46	319.46
13.500	319.46	319.46	319.46	319.46	319.46
13.750	319.47	319.47	319.47	319.47	319.47
14.000	319.47	319.47	319.47	319.47	319.47
14.250	319.47	319.47	319.47	319.47	319.47
14.500	319.47	319.47	319.47	319.47	319.47
14.750	319.47	319.47	319.47	319.46	319.46
15.000	319.46	319.46	319.46	319.46	319.46
15.250	319.46	319.46	319.46	319.46	319.46
15.500	319.46	319.46	319.46	319.46	319.45
15.750	319.45	319.45	319.45	319.45	319.45
16.000	319.45	319.45	319.45	319.45	319.45
16.250	319.45	319.44	319.44	319.44	319.44
16.500	319.44	319.44	319.44	319.44	319.44
16.750	319.44	319.44	319.43	319.43	319.43
17.000	319.43	319.43	319.43	319.43	319.43
17.250	319.43	319.43	319.43	319.43	319.42
17.500	319.42	319.42	319.42	319.42	319.42
17.750	319.42	319.42	319.42	319.42	319.42
18.000	319.41	319.41	319.41	319.41	319.41
18.250	319.41	319.41	319.41	319.41	319.41
18.500	319.41	319.40	319.40	319.40	319.40
18.750	319.40	319.40	319.40	319.40	319.40
19.000	319.40	319.40	319.40	319.39	319.39
19.250	319.39	319.39	319.39	319.39	319.39
19.500	319.39	319.39	319.39	319.39	319.39
19.750	319.39	319.39	319.38	319.38	319.38
20.000	319.38	319.38	319.38	319.38	319.38

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.38	319.38	319.38	319.38	319.38
20.500	319.38	319.37	319.37	319.37	319.37
20.750	319.37	319.37	319.37	319.37	319.37
21.000	319.37	319.37	319.37	319.37	319.37
21.250	319.37	319.37	319.37	319.36	319.36
21.500	319.36	319.36	319.36	319.36	319.36
21.750	319.36	319.36	319.36	319.36	319.36
22.000	319.36	319.36	319.36	319.36	319.36
22.250	319.36	319.35	319.35	319.35	319.35
22.500	319.35	319.35	319.35	319.35	319.35
22.750	319.35	319.35	319.35	319.35	319.35
23.000	319.35	319.35	319.35	319.35	319.35
23.250	319.35	319.34	319.34	319.34	319.34
23.500	319.34	319.34	319.34	319.34	319.34
23.750	319.34	319.34	319.34	319.34	319.34
24.000	319.34	319.34	319.34	319.34	319.34
24.250	319.34	319.33	319.33	319.33	319.33
24.500	319.33	319.33	319.33	319.33	319.33
24.750	319.33	319.33	319.33	319.33	319.32
25.000	319.32	319.32	319.32	319.32	319.32
25.250	319.32	319.32	319.32	319.32	319.32
25.500	319.32	319.32	319.32	319.32	319.32
25.750	319.31	319.31	319.31	319.31	319.31
26.000	319.31	319.31	319.31	319.31	319.31
26.250	319.31	319.31	319.31	319.31	319.31
26.500	319.31	319.31	319.31	319.31	319.30
26.750	319.30	319.30	319.30	319.30	319.30
27.000	319.30	319.30	319.30	319.30	319.30
27.250	319.30	319.30	319.30	319.30	319.30
27.500	319.30	319.30	319.30	319.30	319.30
27.750	319.30	319.30	319.29	319.29	319.29
28.000	319.29	319.29	319.29	319.29	319.29
28.250	319.29	319.29	319.29	319.29	319.29
28.500	319.29	319.29	319.29	319.29	319.29
28.750	319.29	319.29	319.29	319.29	319.29
29.000	319.29	319.29	319.29	319.29	319.29
29.250	319.29	319.29	319.29	319.28	319.28
29.500	319.28	319.28	319.28	319.28	319.28
29.750	319.28	319.28	319.28	319.28	319.28
30.000	319.28	319.28	319.28	319.28	319.28
30.250	319.28	319.28	319.28	319.28	319.28

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.28	319.28	319.28	319.28	319.28
30.750	319.28	319.28	319.28	319.28	319.28
31.000	319.28	319.28	319.28	319.28	319.28
31.250	319.28	319.28	319.28	319.28	319.28
31.500	319.28	319.28	319.28	319.28	319.28
31.750	319.27	319.27	319.27	319.27	319.27
32.000	319.27	319.27	319.27	319.27	319.27
32.250	319.27	319.27	319.27	319.27	319.27
32.500	319.27	319.27	319.27	319.27	319.27
32.750	319.27	319.27	319.27	319.27	319.27
33.000	319.27	319.27	319.27	319.27	319.27
33.250	319.27	319.27	319.27	319.27	319.27
33.500	319.27	319.27	319.27	319.27	319.27
33.750	319.27	319.27	319.27	319.27	319.27
34.000	319.27	319.27	319.27	319.27	319.27
34.250	319.27	319.27	319.27	319.27	319.27
34.500	319.27	319.27	319.27	319.27	319.27
34.750	319.27	319.27	319.27	319.27	319.27
35.000	319.27	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.17	319.17	319.17

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.17	319.17	319.17	319.17	319.17
10.250	319.17	319.17	319.17	319.17	319.17
10.500	319.17	319.17	319.17	319.17	319.18
10.750	319.18	319.18	319.18	319.18	319.18
11.000	319.18	319.18	319.18	319.18	319.19
11.250	319.19	319.19	319.19	319.19	319.20
11.500	319.20	319.20	319.21	319.21	319.22
11.750	319.22	319.23	319.24	319.26	319.28
12.000	319.30	319.34	319.38	319.44	319.49
12.250	319.55	319.61	319.67	319.72	319.77
12.500	319.81	319.85	319.88	319.91	319.93
12.750	319.95	319.96	319.98	319.99	320.00
13.000	320.00	320.01	320.01	320.01	320.02
13.250	320.02	320.02	320.02	320.01	320.01
13.500	320.01	320.01	320.01	320.00	320.00
13.750	320.00	319.99	319.99	319.99	319.98
14.000	319.98	319.97	319.97	319.96	319.96
14.250	319.95	319.95	319.94	319.94	319.93
14.500	319.93	319.92	319.92	319.91	319.91
14.750	319.90	319.90	319.89	319.89	319.88
15.000	319.88	319.87	319.87	319.86	319.86
15.250	319.85	319.85	319.84	319.84	319.83
15.500	319.83	319.82	319.82	319.81	319.81
15.750	319.80	319.80	319.80	319.79	319.79
16.000	319.78	319.78	319.77	319.77	319.77
16.250	319.76	319.76	319.75	319.75	319.74
16.500	319.74	319.74	319.73	319.73	319.73
16.750	319.72	319.72	319.71	319.71	319.71
17.000	319.70	319.70	319.70	319.69	319.69
17.250	319.69	319.69	319.68	319.68	319.68
17.500	319.67	319.67	319.67	319.67	319.66
17.750	319.66	319.66	319.66	319.65	319.65
18.000	319.65	319.64	319.64	319.64	319.64
18.250	319.63	319.63	319.63	319.63	319.62
18.500	319.62	319.62	319.62	319.61	319.61
18.750	319.61	319.61	319.60	319.60	319.60
19.000	319.60	319.59	319.59	319.59	319.59
19.250	319.59	319.58	319.58	319.58	319.58
19.500	319.57	319.57	319.57	319.57	319.57
19.750	319.56	319.56	319.56	319.56	319.56
20.000	319.55	319.55	319.55	319.55	319.55

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.54	319.54	319.54	319.54	319.54
20.500	319.53	319.53	319.53	319.53	319.53
20.750	319.53	319.52	319.52	319.52	319.52
21.000	319.52	319.52	319.51	319.51	319.51
21.250	319.51	319.51	319.51	319.50	319.50
21.500	319.50	319.50	319.50	319.50	319.49
21.750	319.49	319.49	319.49	319.49	319.49
22.000	319.49	319.48	319.48	319.48	319.48
22.250	319.48	319.48	319.48	319.47	319.47
22.500	319.47	319.47	319.47	319.47	319.47
22.750	319.46	319.46	319.46	319.46	319.46
23.000	319.46	319.46	319.46	319.45	319.45
23.250	319.45	319.45	319.45	319.45	319.45
23.500	319.45	319.44	319.44	319.44	319.44
23.750	319.44	319.44	319.44	319.44	319.44
24.000	319.43	319.43	319.43	319.43	319.43
24.250	319.43	319.43	319.42	319.42	319.42
24.500	319.42	319.42	319.42	319.41	319.41
24.750	319.41	319.41	319.41	319.41	319.40
25.000	319.40	319.40	319.40	319.40	319.40
25.250	319.40	319.39	319.39	319.39	319.39
25.500	319.39	319.39	319.39	319.38	319.38
25.750	319.38	319.38	319.38	319.38	319.38
26.000	319.38	319.37	319.37	319.37	319.37
26.250	319.37	319.37	319.37	319.37	319.36
26.500	319.36	319.36	319.36	319.36	319.36
26.750	319.36	319.36	319.36	319.36	319.35
27.000	319.35	319.35	319.35	319.35	319.35
27.250	319.35	319.35	319.35	319.35	319.34
27.500	319.34	319.34	319.34	319.34	319.34
27.750	319.34	319.34	319.34	319.34	319.34
28.000	319.34	319.33	319.33	319.33	319.33
28.250	319.33	319.33	319.33	319.33	319.33
28.500	319.33	319.33	319.33	319.33	319.32
28.750	319.32	319.32	319.32	319.32	319.32
29.000	319.32	319.32	319.32	319.32	319.32
29.250	319.32	319.32	319.32	319.32	319.32
29.500	319.31	319.31	319.31	319.31	319.31
29.750	319.31	319.31	319.31	319.31	319.31
30.000	319.31	319.31	319.31	319.31	319.31
30.250	319.31	319.31	319.31	319.31	319.30

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.30	319.30	319.30	319.30	319.30
30.750	319.30	319.30	319.30	319.30	319.30
31.000	319.30	319.30	319.30	319.30	319.30
31.250	319.30	319.30	319.30	319.30	319.30
31.500	319.30	319.30	319.29	319.29	319.29
31.750	319.29	319.29	319.29	319.29	319.29
32.000	319.29	319.29	319.29	319.29	319.29
32.250	319.29	319.29	319.29	319.29	319.29
32.500	319.29	319.29	319.29	319.29	319.29
32.750	319.29	319.29	319.29	319.29	319.29
33.000	319.29	319.29	319.29	319.28	319.28
33.250	319.28	319.28	319.28	319.28	319.28
33.500	319.28	319.28	319.28	319.28	319.28
33.750	319.28	319.28	319.28	319.28	319.28
34.000	319.28	319.28	319.28	319.28	319.28
34.250	319.28	319.28	319.28	319.28	319.28
34.500	319.28	319.28	319.28	319.28	319.28
34.750	319.28	319.28	319.28	319.28	319.28
35.000	319.28	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.17	319.17	319.17	319.17	319.17
8.750	319.17	319.17	319.17	319.17	319.17
9.000	319.17	319.17	319.17	319.17	319.17
9.250	319.17	319.17	319.17	319.17	319.17
9.500	319.17	319.17	319.17	319.17	319.17
9.750	319.17	319.17	319.18	319.18	319.18

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.18	319.18	319.18	319.18	319.18
10.250	319.18	319.18	319.18	319.19	319.19
10.500	319.19	319.19	319.19	319.19	319.20
10.750	319.20	319.20	319.20	319.20	319.21
11.000	319.21	319.21	319.21	319.22	319.22
11.250	319.22	319.23	319.23	319.23	319.24
11.500	319.24	319.25	319.26	319.26	319.27
11.750	319.29	319.30	319.32	319.34	319.37
12.000	319.42	319.49	319.57	319.67	319.76
12.250	319.85	319.94	320.01	320.08	320.15
12.500	320.20	320.25	320.29	320.32	320.35
12.750	320.36	320.37	320.38	320.38	320.38
13.000	320.37	320.37	320.36	320.35	320.35
13.250	320.34	320.33	320.33	320.32	320.31
13.500	320.31	320.30	320.30	320.29	320.29
13.750	320.28	320.27	320.27	320.26	320.26
14.000	320.25	320.24	320.24	320.23	320.22
14.250	320.21	320.21	320.20	320.19	320.19
14.500	320.18	320.17	320.17	320.16	320.15
14.750	320.14	320.14	320.13	320.12	320.11
15.000	320.10	320.10	320.09	320.08	320.07
15.250	320.07	320.06	320.05	320.04	320.04
15.500	320.03	320.02	320.02	320.01	320.00
15.750	319.99	319.99	319.98	319.97	319.97
16.000	319.96	319.95	319.94	319.94	319.93
16.250	319.92	319.92	319.91	319.90	319.90
16.500	319.89	319.89	319.88	319.87	319.87
16.750	319.86	319.86	319.85	319.84	319.84
17.000	319.83	319.83	319.82	319.82	319.81
17.250	319.81	319.80	319.80	319.79	319.79
17.500	319.78	319.78	319.77	319.77	319.76
17.750	319.76	319.76	319.75	319.75	319.74
18.000	319.74	319.74	319.73	319.73	319.72
18.250	319.72	319.72	319.71	319.71	319.71
18.500	319.70	319.70	319.70	319.69	319.69
18.750	319.69	319.68	319.68	319.68	319.68
19.000	319.67	319.67	319.67	319.67	319.66
19.250	319.66	319.66	319.66	319.65	319.65
19.500	319.65	319.65	319.64	319.64	319.64
19.750	319.64	319.63	319.63	319.63	319.63
20.000	319.62	319.62	319.62	319.62	319.61

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.61	319.61	319.61	319.61	319.60
20.500	319.60	319.60	319.60	319.60	319.59
20.750	319.59	319.59	319.59	319.58	319.58
21.000	319.58	319.58	319.58	319.58	319.57
21.250	319.57	319.57	319.57	319.57	319.56
21.500	319.56	319.56	319.56	319.56	319.55
21.750	319.55	319.55	319.55	319.55	319.55
22.000	319.54	319.54	319.54	319.54	319.54
22.250	319.54	319.53	319.53	319.53	319.53
22.500	319.53	319.53	319.52	319.52	319.52
22.750	319.52	319.52	319.52	319.51	319.51
23.000	319.51	319.51	319.51	319.51	319.51
23.250	319.50	319.50	319.50	319.50	319.50
23.500	319.50	319.50	319.49	319.49	319.49
23.750	319.49	319.49	319.49	319.49	319.48
24.000	319.48	319.48	319.48	319.48	319.48
24.250	319.47	319.47	319.47	319.47	319.46
24.500	319.46	319.46	319.46	319.46	319.45
24.750	319.45	319.45	319.45	319.45	319.44
25.000	319.44	319.44	319.44	319.44	319.43
25.250	319.43	319.43	319.43	319.43	319.43
25.500	319.42	319.42	319.42	319.42	319.42
25.750	319.42	319.41	319.41	319.41	319.41
26.000	319.41	319.41	319.40	319.40	319.40
26.250	319.40	319.40	319.40	319.40	319.39
26.500	319.39	319.39	319.39	319.39	319.39
26.750	319.39	319.38	319.38	319.38	319.38
27.000	319.38	319.38	319.38	319.38	319.37
27.250	319.37	319.37	319.37	319.37	319.37
27.500	319.37	319.37	319.36	319.36	319.36
27.750	319.36	319.36	319.36	319.36	319.36
28.000	319.36	319.36	319.35	319.35	319.35
28.250	319.35	319.35	319.35	319.35	319.35
28.500	319.35	319.35	319.34	319.34	319.34
28.750	319.34	319.34	319.34	319.34	319.34
29.000	319.34	319.34	319.34	319.34	319.33
29.250	319.33	319.33	319.33	319.33	319.33
29.500	319.33	319.33	319.33	319.33	319.33
29.750	319.33	319.33	319.33	319.32	319.32
30.000	319.32	319.32	319.32	319.32	319.32
30.250	319.32	319.32	319.32	319.32	319.32

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.32	319.32	319.32	319.31	319.31
30.750	319.31	319.31	319.31	319.31	319.31
31.000	319.31	319.31	319.31	319.31	319.31
31.250	319.31	319.31	319.31	319.31	319.31
31.500	319.31	319.31	319.30	319.30	319.30
31.750	319.30	319.30	319.30	319.30	319.30
32.000	319.30	319.30	319.30	319.30	319.30
32.250	319.30	319.30	319.30	319.30	319.30
32.500	319.30	319.30	319.30	319.30	319.30
32.750	319.29	319.29	319.29	319.29	319.29
33.000	319.29	319.29	319.29	319.29	319.29
33.250	319.29	319.29	319.29	319.29	319.29
33.500	319.29	319.29	319.29	319.29	319.29
33.750	319.29	319.29	319.29	319.29	319.29
34.000	319.29	319.29	319.29	319.29	319.29
34.250	319.29	319.29	319.28	319.28	319.28
34.500	319.28	319.28	319.28	319.28	319.28
34.750	319.28	319.28	319.28	319.28	319.28
35.000	319.28	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Return Event: 100 years

Label: Existing Pond (OUT)

Storm Event: TypeIII 24hr (9.0 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	319.17	319.17	319.17	319.17	319.17
0.250	319.17	319.17	319.17	319.17	319.17
0.500	319.17	319.17	319.17	319.17	319.17
0.750	319.17	319.17	319.17	319.17	319.17
1.000	319.17	319.17	319.17	319.17	319.17
1.250	319.17	319.17	319.17	319.17	319.17
1.500	319.17	319.17	319.17	319.17	319.17
1.750	319.17	319.17	319.17	319.17	319.17
2.000	319.17	319.17	319.17	319.17	319.17
2.250	319.17	319.17	319.17	319.17	319.17
2.500	319.17	319.17	319.17	319.17	319.17
2.750	319.17	319.17	319.17	319.17	319.17
3.000	319.17	319.17	319.17	319.17	319.17
3.250	319.17	319.17	319.17	319.17	319.17
3.500	319.17	319.17	319.17	319.17	319.17
3.750	319.17	319.17	319.17	319.17	319.17
4.000	319.17	319.17	319.17	319.17	319.17
4.250	319.17	319.17	319.17	319.17	319.17
4.500	319.17	319.17	319.17	319.17	319.17
4.750	319.17	319.17	319.17	319.17	319.17
5.000	319.17	319.17	319.17	319.17	319.17
5.250	319.17	319.17	319.17	319.17	319.17
5.500	319.17	319.17	319.17	319.17	319.17
5.750	319.17	319.17	319.17	319.17	319.17
6.000	319.17	319.17	319.17	319.17	319.17
6.250	319.17	319.17	319.17	319.17	319.17
6.500	319.17	319.17	319.17	319.17	319.17
6.750	319.17	319.17	319.17	319.17	319.17
7.000	319.17	319.17	319.17	319.17	319.17
7.250	319.17	319.17	319.17	319.17	319.17
7.500	319.17	319.17	319.17	319.17	319.17
7.750	319.17	319.17	319.17	319.17	319.17
8.000	319.17	319.17	319.17	319.17	319.17
8.250	319.17	319.17	319.17	319.17	319.17
8.500	319.18	319.18	319.18	319.18	319.18
8.750	319.18	319.18	319.18	319.18	319.18
9.000	319.18	319.18	319.19	319.19	319.19
9.250	319.19	319.19	319.19	319.19	319.20
9.500	319.20	319.20	319.20	319.20	319.21
9.750	319.21	319.21	319.21	319.22	319.22

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.22	319.22	319.23	319.23	319.23
10.250	319.24	319.24	319.24	319.25	319.25
10.500	319.25	319.26	319.26	319.27	319.27
10.750	319.28	319.28	319.28	319.29	319.30
11.000	319.30	319.31	319.31	319.32	319.32
11.250	319.33	319.34	319.35	319.35	319.36
11.500	319.38	319.39	319.40	319.42	319.45
11.750	319.48	319.52	319.57	319.64	319.71
12.000	319.81	319.94	320.09	320.23	320.35
12.250	320.45	320.52	320.59	320.64	320.68
12.500	320.69	320.70	320.69	320.68	320.67
12.750	320.67	320.66	320.65	320.64	320.62
13.000	320.61	320.60	320.58	320.57	320.55
13.250	320.53	320.51	320.49	320.47	320.46
13.500	320.44	320.43	320.41	320.40	320.39
13.750	320.38	320.37	320.36	320.35	320.35
14.000	320.34	320.33	320.33	320.32	320.32
14.250	320.31	320.31	320.30	320.30	320.30
14.500	320.29	320.29	320.29	320.28	320.28
14.750	320.27	320.27	320.26	320.26	320.25
15.000	320.25	320.24	320.24	320.23	320.23
15.250	320.22	320.22	320.21	320.21	320.20
15.500	320.20	320.19	320.19	320.18	320.18
15.750	320.17	320.16	320.16	320.15	320.14
16.000	320.14	320.13	320.12	320.12	320.11
16.250	320.10	320.10	320.09	320.08	320.08
16.500	320.07	320.06	320.05	320.05	320.04
16.750	320.03	320.03	320.02	320.01	320.01
17.000	320.00	319.99	319.99	319.98	319.98
17.250	319.97	319.96	319.96	319.95	319.94
17.500	319.94	319.93	319.93	319.92	319.91
17.750	319.91	319.90	319.90	319.89	319.88
18.000	319.88	319.87	319.87	319.86	319.86
18.250	319.85	319.85	319.84	319.84	319.83
18.500	319.83	319.82	319.82	319.81	319.81
18.750	319.81	319.80	319.80	319.79	319.79
19.000	319.78	319.78	319.78	319.77	319.77
19.250	319.76	319.76	319.76	319.75	319.75
19.500	319.75	319.74	319.74	319.74	319.73
19.750	319.73	319.73	319.72	319.72	319.72
20.000	319.71	319.71	319.71	319.71	319.70

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	319.70	319.70	319.69	319.69	319.69
20.500	319.69	319.69	319.68	319.68	319.68
20.750	319.68	319.67	319.67	319.67	319.67
21.000	319.67	319.66	319.66	319.66	319.66
21.250	319.66	319.65	319.65	319.65	319.65
21.500	319.64	319.64	319.64	319.64	319.64
21.750	319.63	319.63	319.63	319.63	319.63
22.000	319.63	319.62	319.62	319.62	319.62
22.250	319.62	319.61	319.61	319.61	319.61
22.500	319.61	319.60	319.60	319.60	319.60
22.750	319.60	319.60	319.59	319.59	319.59
23.000	319.59	319.59	319.59	319.58	319.58
23.250	319.58	319.58	319.58	319.58	319.57
23.500	319.57	319.57	319.57	319.57	319.57
23.750	319.56	319.56	319.56	319.56	319.56
24.000	319.56	319.55	319.55	319.55	319.55
24.250	319.54	319.54	319.54	319.54	319.53
24.500	319.53	319.53	319.52	319.52	319.52
24.750	319.52	319.51	319.51	319.51	319.51
25.000	319.50	319.50	319.50	319.49	319.49
25.250	319.49	319.49	319.49	319.48	319.48
25.500	319.48	319.48	319.47	319.47	319.47
25.750	319.47	319.46	319.46	319.46	319.46
26.000	319.46	319.45	319.45	319.45	319.45
26.250	319.45	319.44	319.44	319.44	319.44
26.500	319.44	319.43	319.43	319.43	319.43
26.750	319.43	319.43	319.42	319.42	319.42
27.000	319.42	319.42	319.42	319.41	319.41
27.250	319.41	319.41	319.41	319.41	319.40
27.500	319.40	319.40	319.40	319.40	319.40
27.750	319.40	319.39	319.39	319.39	319.39
28.000	319.39	319.39	319.39	319.38	319.38
28.250	319.38	319.38	319.38	319.38	319.38
28.500	319.38	319.37	319.37	319.37	319.37
28.750	319.37	319.37	319.37	319.37	319.37
29.000	319.36	319.36	319.36	319.36	319.36
29.250	319.36	319.36	319.36	319.36	319.35
29.500	319.35	319.35	319.35	319.35	319.35
29.750	319.35	319.35	319.35	319.35	319.34
30.000	319.34	319.34	319.34	319.34	319.34
30.250	319.34	319.34	319.34	319.34	319.34

Watershed

Subsection: Time vs. Elevation

Label: Existing Pond (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.34	319.33	319.33	319.33	319.33
30.750	319.33	319.33	319.33	319.33	319.33
31.000	319.33	319.33	319.33	319.33	319.33
31.250	319.32	319.32	319.32	319.32	319.32
31.500	319.32	319.32	319.32	319.32	319.32
31.750	319.32	319.32	319.32	319.32	319.32
32.000	319.31	319.31	319.31	319.31	319.31
32.250	319.31	319.31	319.31	319.31	319.31
32.500	319.31	319.31	319.31	319.31	319.31
32.750	319.31	319.31	319.31	319.31	319.30
33.000	319.30	319.30	319.30	319.30	319.30
33.250	319.30	319.30	319.30	319.30	319.30
33.500	319.30	319.30	319.30	319.30	319.30
33.750	319.30	319.30	319.30	319.30	319.30
34.000	319.30	319.30	319.29	319.29	319.29
34.250	319.29	319.29	319.29	319.29	319.29
34.500	319.29	319.29	319.29	319.29	319.29
34.750	319.29	319.29	319.29	319.29	319.29
35.000	319.29	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	345.00	345.00	345.00	345.00	345.00
0.250	345.00	345.00	345.00	345.00	345.00
0.500	345.00	345.00	345.00	345.00	345.00
0.750	345.00	345.00	345.00	345.00	345.00
1.000	345.00	345.00	345.00	345.00	345.00
1.250	345.00	345.00	345.00	345.00	345.00
1.500	345.00	345.00	345.00	345.00	345.00
1.750	345.00	345.00	345.00	345.00	345.00
2.000	345.00	345.00	345.00	345.00	345.00
2.250	345.00	345.00	345.00	345.00	345.00
2.500	345.00	345.00	345.00	345.00	345.00
2.750	345.00	345.00	345.00	345.00	345.00
3.000	345.00	345.00	345.00	345.00	345.00
3.250	345.00	345.00	345.00	345.00	345.00
3.500	345.00	345.00	345.00	345.00	345.00
3.750	345.00	345.00	345.00	345.00	345.00
4.000	345.00	345.00	345.00	345.00	345.00
4.250	345.00	345.00	345.00	345.00	345.00
4.500	345.00	345.00	345.00	345.00	345.00
4.750	345.00	345.00	345.00	345.00	345.00
5.000	345.00	345.00	345.00	345.00	345.00
5.250	345.00	345.00	345.00	345.00	345.00
5.500	345.00	345.00	345.00	345.00	345.00
5.750	345.00	345.00	345.00	345.00	345.00
6.000	345.00	345.00	345.00	345.00	345.00
6.250	345.00	345.00	345.00	345.00	345.00
6.500	345.00	345.00	345.00	345.00	345.00
6.750	345.00	345.00	345.00	345.00	345.00
7.000	345.00	345.00	345.00	345.00	345.00
7.250	345.00	345.00	345.00	345.00	345.00
7.500	345.00	345.00	345.00	345.00	345.00
7.750	345.00	345.00	345.00	345.00	345.00
8.000	345.00	345.00	345.00	345.00	345.00
8.250	345.00	345.00	345.00	345.00	345.00
8.500	345.00	345.00	345.00	345.00	345.00
8.750	345.00	345.00	345.00	345.00	345.00
9.000	345.00	345.00	345.00	345.00	345.00
9.250	345.00	345.00	345.00	345.00	345.00
9.500	345.00	345.00	345.00	345.00	345.00
9.750	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	345.00	345.00	345.00	345.00	345.00
10.250	345.00	345.00	345.00	345.00	345.00
10.500	345.00	345.00	345.00	345.00	345.00
10.750	345.00	345.00	345.00	345.00	345.00
11.000	345.01	345.01	345.01	345.01	345.01
11.250	345.01	345.02	345.02	345.02	345.02
11.500	345.03	345.03	345.03	345.04	345.05
11.750	345.06	345.08	345.10	345.12	345.16
12.000	345.21	345.28	345.38	345.47	345.55
12.250	345.61	345.66	345.69	345.71	345.72
12.500	345.73	345.72	345.71	345.69	345.67
12.750	345.65	345.62	345.59	345.57	345.54
13.000	345.51	345.48	345.45	345.43	345.40
13.250	345.38	345.36	345.34	345.32	345.31
13.500	345.29	345.28	345.27	345.26	345.25
13.750	345.24	345.23	345.22	345.21	345.21
14.000	345.20	345.19	345.19	345.18	345.18
14.250	345.17	345.17	345.16	345.16	345.15
14.500	345.15	345.15	345.14	345.14	345.14
14.750	345.14	345.13	345.13	345.13	345.13
15.000	345.13	345.12	345.12	345.12	345.12
15.250	345.12	345.11	345.11	345.11	345.11
15.500	345.11	345.11	345.10	345.10	345.10
15.750	345.10	345.10	345.10	345.09	345.09
16.000	345.09	345.09	345.09	345.09	345.08
16.250	345.08	345.08	345.08	345.08	345.08
16.500	345.08	345.08	345.08	345.07	345.07
16.750	345.07	345.07	345.07	345.07	345.07
17.000	345.07	345.07	345.07	345.07	345.07
17.250	345.06	345.06	345.06	345.06	345.06
17.500	345.06	345.06	345.06	345.06	345.06
17.750	345.06	345.06	345.06	345.06	345.05
18.000	345.05	345.05	345.05	345.05	345.05
18.250	345.05	345.05	345.05	345.05	345.05
18.500	345.05	345.05	345.05	345.05	345.05
18.750	345.05	345.05	345.05	345.05	345.05
19.000	345.04	345.04	345.04	345.04	345.04
19.250	345.04	345.04	345.04	345.04	345.04
19.500	345.04	345.04	345.04	345.04	345.04
19.750	345.04	345.04	345.04	345.04	345.04
20.000	345.04	345.04	345.04	345.04	345.04

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	345.04	345.04	345.04	345.04	345.04
20.500	345.04	345.04	345.04	345.04	345.04
20.750	345.04	345.04	345.04	345.04	345.04
21.000	345.04	345.04	345.04	345.04	345.04
21.250	345.04	345.04	345.04	345.04	345.04
21.500	345.04	345.03	345.03	345.03	345.03
21.750	345.03	345.03	345.03	345.03	345.03
22.000	345.03	345.03	345.03	345.03	345.03
22.250	345.03	345.03	345.03	345.03	345.03
22.500	345.03	345.03	345.03	345.03	345.03
22.750	345.03	345.03	345.03	345.03	345.03
23.000	345.03	345.03	345.03	345.03	345.03
23.250	345.03	345.03	345.03	345.03	345.03
23.500	345.03	345.03	345.03	345.03	345.03
23.750	345.03	345.03	345.03	345.03	345.03
24.000	345.03	345.03	345.03	345.02	345.02
24.250	345.02	345.02	345.02	345.02	345.01
24.500	345.01	345.01	345.01	345.01	345.01
24.750	345.01	345.01	345.01	345.01	345.00
25.000	345.00	345.00	345.00	345.00	345.00
25.250	345.00	345.00	345.00	345.00	345.00
25.500	345.00	345.00	345.00	345.00	345.00
25.750	345.00	345.00	345.00	345.00	345.00
26.000	345.00	345.00	345.00	345.00	345.00
26.250	345.00	345.00	345.00	345.00	345.00
26.500	345.00	345.00	345.00	345.00	345.00
26.750	345.00	345.00	345.00	345.00	345.00
27.000	345.00	345.00	345.00	345.00	345.00
27.250	345.00	345.00	345.00	345.00	345.00
27.500	345.00	345.00	345.00	345.00	345.00
27.750	345.00	345.00	345.00	345.00	345.00
28.000	345.00	345.00	345.00	345.00	345.00
28.250	345.00	345.00	345.00	345.00	345.00
28.500	345.00	345.00	345.00	345.00	345.00
28.750	345.00	345.00	345.00	345.00	345.00
29.000	345.00	345.00	345.00	345.00	345.00
29.250	345.00	345.00	345.00	345.00	345.00
29.500	345.00	345.00	345.00	345.00	345.00
29.750	345.00	345.00	345.00	345.00	345.00
30.000	345.00	345.00	345.00	345.00	345.00
30.250	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	345.00	345.00	345.00	345.00	345.00
30.750	345.00	345.00	345.00	345.00	345.00
31.000	345.00	345.00	345.00	345.00	345.00
31.250	345.00	345.00	345.00	345.00	345.00
31.500	345.00	345.00	345.00	345.00	345.00
31.750	345.00	345.00	345.00	345.00	345.00
32.000	345.00	345.00	345.00	345.00	345.00
32.250	345.00	345.00	345.00	345.00	345.00
32.500	345.00	345.00	345.00	345.00	345.00
32.750	345.00	345.00	345.00	345.00	345.00
33.000	345.00	345.00	345.00	345.00	345.00
33.250	345.00	345.00	345.00	345.00	345.00
33.500	345.00	345.00	345.00	345.00	345.00
33.750	345.00	345.00	345.00	345.00	345.00
34.000	345.00	345.00	345.00	345.00	345.00
34.250	345.00	345.00	345.00	345.00	345.00
34.500	345.00	345.00	345.00	345.00	345.00
34.750	345.00	345.00	345.00	345.00	345.00
35.000	345.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	345.00	345.00	345.00	345.00	345.00
0.250	345.00	345.00	345.00	345.00	345.00
0.500	345.00	345.00	345.00	345.00	345.00
0.750	345.00	345.00	345.00	345.00	345.00
1.000	345.00	345.00	345.00	345.00	345.00
1.250	345.00	345.00	345.00	345.00	345.00
1.500	345.00	345.00	345.00	345.00	345.00
1.750	345.00	345.00	345.00	345.00	345.00
2.000	345.00	345.00	345.00	345.00	345.00
2.250	345.00	345.00	345.00	345.00	345.00
2.500	345.00	345.00	345.00	345.00	345.00
2.750	345.00	345.00	345.00	345.00	345.00
3.000	345.00	345.00	345.00	345.00	345.00
3.250	345.00	345.00	345.00	345.00	345.00
3.500	345.00	345.00	345.00	345.00	345.00
3.750	345.00	345.00	345.00	345.00	345.00
4.000	345.00	345.00	345.00	345.00	345.00
4.250	345.00	345.00	345.00	345.00	345.00
4.500	345.00	345.00	345.00	345.00	345.00
4.750	345.00	345.00	345.00	345.00	345.00
5.000	345.00	345.00	345.00	345.00	345.00
5.250	345.00	345.00	345.00	345.00	345.00
5.500	345.00	345.00	345.00	345.00	345.00
5.750	345.00	345.00	345.00	345.00	345.00
6.000	345.00	345.00	345.00	345.00	345.00
6.250	345.00	345.00	345.00	345.00	345.00
6.500	345.00	345.00	345.00	345.00	345.00
6.750	345.00	345.00	345.00	345.00	345.00
7.000	345.00	345.00	345.00	345.00	345.00
7.250	345.00	345.00	345.00	345.00	345.00
7.500	345.00	345.00	345.00	345.00	345.00
7.750	345.00	345.00	345.00	345.00	345.00
8.000	345.00	345.00	345.00	345.00	345.00
8.250	345.00	345.00	345.00	345.00	345.00
8.500	345.00	345.00	345.00	345.00	345.00
8.750	345.00	345.00	345.00	345.00	345.00
9.000	345.00	345.00	345.00	345.00	345.00
9.250	345.00	345.00	345.00	345.00	345.00
9.500	345.00	345.00	345.00	345.00	345.00
9.750	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	345.00	345.00	345.00	345.00	345.00
10.250	345.00	345.01	345.01	345.01	345.01
10.500	345.01	345.01	345.01	345.02	345.02
10.750	345.02	345.02	345.02	345.02	345.03
11.000	345.03	345.03	345.03	345.04	345.04
11.250	345.04	345.05	345.05	345.06	345.06
11.500	345.07	345.07	345.08	345.09	345.11
11.750	345.13	345.16	345.19	345.23	345.29
12.000	345.37	345.48	345.61	345.76	345.90
12.250	346.00	346.07	346.12	346.16	346.18
12.500	346.19	346.18	346.17	346.15	346.13
12.750	346.10	346.08	346.05	346.03	346.00
13.000	345.98	345.95	345.92	345.89	345.87
13.250	345.84	345.81	345.78	345.76	345.73
13.500	345.70	345.67	345.64	345.61	345.58
13.750	345.56	345.53	345.50	345.47	345.44
14.000	345.42	345.39	345.37	345.35	345.34
14.250	345.32	345.30	345.29	345.28	345.27
14.500	345.26	345.25	345.24	345.23	345.22
14.750	345.22	345.21	345.21	345.20	345.19
15.000	345.19	345.19	345.18	345.18	345.17
15.250	345.17	345.17	345.16	345.16	345.16
15.500	345.15	345.15	345.15	345.14	345.14
15.750	345.14	345.14	345.13	345.13	345.13
16.000	345.13	345.12	345.12	345.12	345.12
16.250	345.12	345.11	345.11	345.11	345.11
16.500	345.11	345.10	345.10	345.10	345.10
16.750	345.10	345.10	345.10	345.10	345.09
17.000	345.09	345.09	345.09	345.09	345.09
17.250	345.09	345.09	345.09	345.08	345.08
17.500	345.08	345.08	345.08	345.08	345.08
17.750	345.08	345.08	345.08	345.08	345.07
18.000	345.07	345.07	345.07	345.07	345.07
18.250	345.07	345.07	345.07	345.07	345.07
18.500	345.07	345.06	345.06	345.06	345.06
18.750	345.06	345.06	345.06	345.06	345.06
19.000	345.06	345.06	345.06	345.06	345.06
19.250	345.06	345.06	345.06	345.06	345.06
19.500	345.06	345.06	345.06	345.06	345.06
19.750	345.06	345.06	345.06	345.05	345.05
20.000	345.05	345.05	345.05	345.05	345.05

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	345.05	345.05	345.05	345.05	345.05
20.500	345.05	345.05	345.05	345.05	345.05
20.750	345.05	345.05	345.05	345.05	345.05
21.000	345.05	345.05	345.05	345.05	345.05
21.250	345.05	345.05	345.05	345.05	345.05
21.500	345.05	345.05	345.05	345.05	345.05
21.750	345.05	345.05	345.05	345.05	345.05
22.000	345.05	345.04	345.04	345.04	345.04
22.250	345.04	345.04	345.04	345.04	345.04
22.500	345.04	345.04	345.04	345.04	345.04
22.750	345.04	345.04	345.04	345.04	345.04
23.000	345.04	345.04	345.04	345.04	345.04
23.250	345.04	345.04	345.04	345.04	345.04
23.500	345.04	345.04	345.04	345.04	345.04
23.750	345.04	345.04	345.04	345.04	345.04
24.000	345.04	345.04	345.03	345.03	345.03
24.250	345.03	345.02	345.02	345.02	345.02
24.500	345.02	345.01	345.01	345.01	345.01
24.750	345.01	345.01	345.01	345.01	345.01
25.000	345.01	345.01	345.00	345.00	345.00
25.250	345.00	345.00	345.00	345.00	345.00
25.500	345.00	345.00	345.00	345.00	345.00
25.750	345.00	345.00	345.00	345.00	345.00
26.000	345.00	345.00	345.00	345.00	345.00
26.250	345.00	345.00	345.00	345.00	345.00
26.500	345.00	345.00	345.00	345.00	345.00
26.750	345.00	345.00	345.00	345.00	345.00
27.000	345.00	345.00	345.00	345.00	345.00
27.250	345.00	345.00	345.00	345.00	345.00
27.500	345.00	345.00	345.00	345.00	345.00
27.750	345.00	345.00	345.00	345.00	345.00
28.000	345.00	345.00	345.00	345.00	345.00
28.250	345.00	345.00	345.00	345.00	345.00
28.500	345.00	345.00	345.00	345.00	345.00
28.750	345.00	345.00	345.00	345.00	345.00
29.000	345.00	345.00	345.00	345.00	345.00
29.250	345.00	345.00	345.00	345.00	345.00
29.500	345.00	345.00	345.00	345.00	345.00
29.750	345.00	345.00	345.00	345.00	345.00
30.000	345.00	345.00	345.00	345.00	345.00
30.250	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	345.00	345.00	345.00	345.00	345.00
30.750	345.00	345.00	345.00	345.00	345.00
31.000	345.00	345.00	345.00	345.00	345.00
31.250	345.00	345.00	345.00	345.00	345.00
31.500	345.00	345.00	345.00	345.00	345.00
31.750	345.00	345.00	345.00	345.00	345.00
32.000	345.00	345.00	345.00	345.00	345.00
32.250	345.00	345.00	345.00	345.00	345.00
32.500	345.00	345.00	345.00	345.00	345.00
32.750	345.00	345.00	345.00	345.00	345.00
33.000	345.00	345.00	345.00	345.00	345.00
33.250	345.00	345.00	345.00	345.00	345.00
33.500	345.00	345.00	345.00	345.00	345.00
33.750	345.00	345.00	345.00	345.00	345.00
34.000	345.00	345.00	345.00	345.00	345.00
34.250	345.00	345.00	345.00	345.00	345.00
34.500	345.00	345.00	345.00	345.00	345.00
34.750	345.00	345.00	345.00	345.00	345.00
35.000	345.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	345.00	345.00	345.00	345.00	345.00
0.250	345.00	345.00	345.00	345.00	345.00
0.500	345.00	345.00	345.00	345.00	345.00
0.750	345.00	345.00	345.00	345.00	345.00
1.000	345.00	345.00	345.00	345.00	345.00
1.250	345.00	345.00	345.00	345.00	345.00
1.500	345.00	345.00	345.00	345.00	345.00
1.750	345.00	345.00	345.00	345.00	345.00
2.000	345.00	345.00	345.00	345.00	345.00
2.250	345.00	345.00	345.00	345.00	345.00
2.500	345.00	345.00	345.00	345.00	345.00
2.750	345.00	345.00	345.00	345.00	345.00
3.000	345.00	345.00	345.00	345.00	345.00
3.250	345.00	345.00	345.00	345.00	345.00
3.500	345.00	345.00	345.00	345.00	345.00
3.750	345.00	345.00	345.00	345.00	345.00
4.000	345.00	345.00	345.00	345.00	345.00
4.250	345.00	345.00	345.00	345.00	345.00
4.500	345.00	345.00	345.00	345.00	345.00
4.750	345.00	345.00	345.00	345.00	345.00
5.000	345.00	345.00	345.00	345.00	345.00
5.250	345.00	345.00	345.00	345.00	345.00
5.500	345.00	345.00	345.00	345.00	345.00
5.750	345.00	345.00	345.00	345.00	345.00
6.000	345.00	345.00	345.00	345.00	345.00
6.250	345.00	345.00	345.00	345.00	345.00
6.500	345.00	345.00	345.00	345.00	345.00
6.750	345.00	345.00	345.00	345.00	345.00
7.000	345.00	345.00	345.00	345.00	345.00
7.250	345.00	345.00	345.00	345.00	345.00
7.500	345.00	345.00	345.00	345.00	345.00
7.750	345.00	345.00	345.00	345.00	345.00
8.000	345.00	345.00	345.00	345.00	345.00
8.250	345.00	345.00	345.00	345.00	345.00
8.500	345.00	345.00	345.00	345.00	345.00
8.750	345.00	345.01	345.01	345.01	345.01
9.000	345.01	345.01	345.01	345.01	345.02
9.250	345.02	345.02	345.02	345.02	345.02
9.500	345.03	345.03	345.03	345.03	345.03
9.750	345.04	345.04	345.04	345.04	345.05

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	345.05	345.05	345.05	345.06	345.06
10.250	345.06	345.07	345.07	345.07	345.08
10.500	345.08	345.08	345.09	345.09	345.10
10.750	345.10	345.11	345.11	345.12	345.12
11.000	345.13	345.13	345.14	345.15	345.15
11.250	345.16	345.17	345.18	345.19	345.20
11.500	345.22	345.23	345.25	345.27	345.31
11.750	345.36	345.42	345.49	345.58	345.70
12.000	345.88	346.12	346.40	346.67	346.89
12.250	347.04	347.14	347.20	347.23	347.24
12.500	347.23	347.20	347.15	347.10	347.04
12.750	346.98	346.92	346.86	346.80	346.75
13.000	346.69	346.64	346.60	346.55	346.51
13.250	346.47	346.43	346.39	346.36	346.32
13.500	346.29	346.25	346.22	346.19	346.16
13.750	346.14	346.11	346.08	346.06	346.04
14.000	346.01	345.99	345.96	345.94	345.92
14.250	345.89	345.87	345.85	345.82	345.80
14.500	345.78	345.76	345.73	345.71	345.69
14.750	345.66	345.64	345.61	345.59	345.57
15.000	345.54	345.52	345.49	345.47	345.44
15.250	345.42	345.40	345.39	345.37	345.36
15.500	345.34	345.33	345.32	345.31	345.30
15.750	345.29	345.28	345.27	345.26	345.25
16.000	345.25	345.24	345.23	345.23	345.22
16.250	345.22	345.21	345.21	345.20	345.20
16.500	345.19	345.19	345.19	345.18	345.18
16.750	345.18	345.17	345.17	345.17	345.17
17.000	345.16	345.16	345.16	345.16	345.16
17.250	345.15	345.15	345.15	345.15	345.15
17.500	345.14	345.14	345.14	345.14	345.14
17.750	345.13	345.13	345.13	345.13	345.13
18.000	345.13	345.12	345.12	345.12	345.12
18.250	345.12	345.12	345.12	345.11	345.11
18.500	345.11	345.11	345.11	345.11	345.11
18.750	345.11	345.11	345.11	345.11	345.10
19.000	345.10	345.10	345.10	345.10	345.10
19.250	345.10	345.10	345.10	345.10	345.10
19.500	345.10	345.10	345.10	345.10	345.10
19.750	345.10	345.09	345.09	345.09	345.09
20.000	345.09	345.09	345.09	345.09	345.09

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	345.09	345.09	345.09	345.09	345.09
20.500	345.09	345.09	345.09	345.09	345.09
20.750	345.09	345.09	345.09	345.08	345.08
21.000	345.08	345.08	345.08	345.08	345.08
21.250	345.08	345.08	345.08	345.08	345.08
21.500	345.08	345.08	345.08	345.08	345.08
21.750	345.08	345.08	345.08	345.08	345.08
22.000	345.08	345.08	345.08	345.08	345.08
22.250	345.07	345.07	345.07	345.07	345.07
22.500	345.07	345.07	345.07	345.07	345.07
22.750	345.07	345.07	345.07	345.07	345.07
23.000	345.07	345.07	345.07	345.07	345.07
23.250	345.07	345.07	345.07	345.07	345.07
23.500	345.07	345.07	345.06	345.06	345.06
23.750	345.06	345.06	345.06	345.06	345.06
24.000	345.06	345.06	345.06	345.06	345.05
24.250	345.05	345.04	345.04	345.03	345.03
24.500	345.03	345.03	345.02	345.02	345.02
24.750	345.02	345.02	345.01	345.01	345.01
25.000	345.01	345.01	345.01	345.01	345.01
25.250	345.01	345.01	345.00	345.00	345.00
25.500	345.00	345.00	345.00	345.00	345.00
25.750	345.00	345.00	345.00	345.00	345.00
26.000	345.00	345.00	345.00	345.00	345.00
26.250	345.00	345.00	345.00	345.00	345.00
26.500	345.00	345.00	345.00	345.00	345.00
26.750	345.00	345.00	345.00	345.00	345.00
27.000	345.00	345.00	345.00	345.00	345.00
27.250	345.00	345.00	345.00	345.00	345.00
27.500	345.00	345.00	345.00	345.00	345.00
27.750	345.00	345.00	345.00	345.00	345.00
28.000	345.00	345.00	345.00	345.00	345.00
28.250	345.00	345.00	345.00	345.00	345.00
28.500	345.00	345.00	345.00	345.00	345.00
28.750	345.00	345.00	345.00	345.00	345.00
29.000	345.00	345.00	345.00	345.00	345.00
29.250	345.00	345.00	345.00	345.00	345.00
29.500	345.00	345.00	345.00	345.00	345.00
29.750	345.00	345.00	345.00	345.00	345.00
30.000	345.00	345.00	345.00	345.00	345.00
30.250	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	345.00	345.00	345.00	345.00	345.00
30.750	345.00	345.00	345.00	345.00	345.00
31.000	345.00	345.00	345.00	345.00	345.00
31.250	345.00	345.00	345.00	345.00	345.00
31.500	345.00	345.00	345.00	345.00	345.00
31.750	345.00	345.00	345.00	345.00	345.00
32.000	345.00	345.00	345.00	345.00	345.00
32.250	345.00	345.00	345.00	345.00	345.00
32.500	345.00	345.00	345.00	345.00	345.00
32.750	345.00	345.00	345.00	345.00	345.00
33.000	345.00	345.00	345.00	345.00	345.00
33.250	345.00	345.00	345.00	345.00	345.00
33.500	345.00	345.00	345.00	345.00	345.00
33.750	345.00	345.00	345.00	345.00	345.00
34.000	345.00	345.00	345.00	345.00	345.00
34.250	345.00	345.00	345.00	345.00	345.00
34.500	345.00	345.00	345.00	345.00	345.00
34.750	345.00	345.00	345.00	345.00	345.00
35.000	345.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	345.00	345.00	345.00	345.00	345.00
0.250	345.00	345.00	345.00	345.00	345.00
0.500	345.00	345.00	345.00	345.00	345.00
0.750	345.00	345.00	345.00	345.00	345.00
1.000	345.00	345.00	345.00	345.00	345.00
1.250	345.00	345.00	345.00	345.00	345.00
1.500	345.00	345.00	345.00	345.00	345.00
1.750	345.00	345.00	345.00	345.00	345.00
2.000	345.00	345.00	345.00	345.00	345.00
2.250	345.00	345.00	345.00	345.00	345.00
2.500	345.00	345.00	345.00	345.00	345.00
2.750	345.00	345.00	345.00	345.00	345.00
3.000	345.00	345.00	345.00	345.00	345.00
3.250	345.00	345.00	345.00	345.00	345.00
3.500	345.00	345.00	345.00	345.00	345.00
3.750	345.00	345.00	345.00	345.00	345.00
4.000	345.00	345.00	345.00	345.00	345.00
4.250	345.00	345.00	345.00	345.00	345.00
4.500	345.00	345.00	345.00	345.00	345.00
4.750	345.00	345.00	345.00	345.00	345.00
5.000	345.00	345.00	345.00	345.00	345.00
5.250	345.00	345.00	345.00	345.00	345.00
5.500	345.00	345.00	345.00	345.00	345.00
5.750	345.00	345.00	345.00	345.00	345.00
6.000	345.00	345.00	345.00	345.00	345.00
6.250	345.00	345.00	345.00	345.00	345.00
6.500	345.00	345.00	345.00	345.00	345.00
6.750	345.00	345.00	345.00	345.00	345.00
7.000	345.00	345.00	345.00	345.00	345.00
7.250	345.00	345.00	345.00	345.00	345.00
7.500	345.00	345.00	345.00	345.00	345.00
7.750	345.00	345.00	345.01	345.01	345.01
8.000	345.01	345.01	345.01	345.01	345.01
8.250	345.01	345.02	345.02	345.02	345.02
8.500	345.02	345.02	345.02	345.03	345.03
8.750	345.03	345.03	345.03	345.04	345.04
9.000	345.04	345.04	345.05	345.05	345.05
9.250	345.05	345.06	345.06	345.06	345.07
9.500	345.07	345.07	345.08	345.08	345.08
9.750	345.09	345.09	345.09	345.10	345.10

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	345.10	345.11	345.11	345.12	345.12
10.250	345.13	345.13	345.14	345.14	345.15
10.500	345.15	345.16	345.16	345.17	345.18
10.750	345.18	345.19	345.20	345.21	345.21
11.000	345.22	345.23	345.24	345.25	345.26
11.250	345.27	345.28	345.30	345.32	345.33
11.500	345.35	345.37	345.40	345.44	345.49
11.750	345.56	345.66	345.78	345.93	346.11
12.000	346.36	346.68	347.04	347.37	347.65
12.250	347.84	347.97	348.06	348.11	348.13
12.500	348.12	348.09	348.04	347.98	347.90
12.750	347.83	347.75	347.67	347.59	347.50
13.000	347.42	347.34	347.26	347.18	347.11
13.250	347.03	346.96	346.90	346.83	346.78
13.500	346.72	346.67	346.63	346.59	346.55
13.750	346.51	346.47	346.44	346.40	346.37
14.000	346.33	346.30	346.27	346.24	346.21
14.250	346.19	346.16	346.14	346.11	346.09
14.500	346.07	346.05	346.03	346.01	345.99
14.750	345.97	345.95	345.93	345.91	345.89
15.000	345.87	345.85	345.83	345.81	345.79
15.250	345.77	345.75	345.73	345.71	345.69
15.500	345.66	345.64	345.62	345.60	345.57
15.750	345.55	345.52	345.50	345.48	345.45
16.000	345.43	345.41	345.40	345.38	345.36
16.250	345.35	345.34	345.33	345.31	345.30
16.500	345.30	345.29	345.28	345.27	345.26
16.750	345.26	345.25	345.25	345.24	345.24
17.000	345.23	345.23	345.22	345.22	345.22
17.250	345.21	345.21	345.21	345.20	345.20
17.500	345.20	345.19	345.19	345.19	345.18
17.750	345.18	345.18	345.18	345.17	345.17
18.000	345.17	345.17	345.16	345.16	345.16
18.250	345.16	345.16	345.15	345.15	345.15
18.500	345.15	345.15	345.15	345.15	345.14
18.750	345.14	345.14	345.14	345.14	345.14
19.000	345.14	345.14	345.14	345.14	345.13
19.250	345.13	345.13	345.13	345.13	345.13
19.500	345.13	345.13	345.13	345.13	345.13
19.750	345.13	345.13	345.12	345.12	345.12
20.000	345.12	345.12	345.12	345.12	345.12

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	345.12	345.12	345.12	345.12	345.12
20.500	345.12	345.12	345.12	345.11	345.11
20.750	345.11	345.11	345.11	345.11	345.11
21.000	345.11	345.11	345.11	345.11	345.11
21.250	345.11	345.11	345.11	345.11	345.11
21.500	345.11	345.11	345.11	345.10	345.10
21.750	345.10	345.10	345.10	345.10	345.10
22.000	345.10	345.10	345.10	345.10	345.10
22.250	345.10	345.10	345.10	345.10	345.10
22.500	345.10	345.10	345.10	345.09	345.09
22.750	345.09	345.09	345.09	345.09	345.09
23.000	345.09	345.09	345.09	345.09	345.09
23.250	345.09	345.09	345.09	345.09	345.09
23.500	345.09	345.09	345.09	345.09	345.08
23.750	345.08	345.08	345.08	345.08	345.08
24.000	345.08	345.08	345.08	345.07	345.07
24.250	345.06	345.06	345.05	345.05	345.04
24.500	345.04	345.03	345.03	345.03	345.02
24.750	345.02	345.02	345.02	345.02	345.01
25.000	345.01	345.01	345.01	345.01	345.01
25.250	345.01	345.01	345.01	345.01	345.01
25.500	345.00	345.00	345.00	345.00	345.00
25.750	345.00	345.00	345.00	345.00	345.00
26.000	345.00	345.00	345.00	345.00	345.00
26.250	345.00	345.00	345.00	345.00	345.00
26.500	345.00	345.00	345.00	345.00	345.00
26.750	345.00	345.00	345.00	345.00	345.00
27.000	345.00	345.00	345.00	345.00	345.00
27.250	345.00	345.00	345.00	345.00	345.00
27.500	345.00	345.00	345.00	345.00	345.00
27.750	345.00	345.00	345.00	345.00	345.00
28.000	345.00	345.00	345.00	345.00	345.00
28.250	345.00	345.00	345.00	345.00	345.00
28.500	345.00	345.00	345.00	345.00	345.00
28.750	345.00	345.00	345.00	345.00	345.00
29.000	345.00	345.00	345.00	345.00	345.00
29.250	345.00	345.00	345.00	345.00	345.00
29.500	345.00	345.00	345.00	345.00	345.00
29.750	345.00	345.00	345.00	345.00	345.00
30.000	345.00	345.00	345.00	345.00	345.00
30.250	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	345.00	345.00	345.00	345.00	345.00
30.750	345.00	345.00	345.00	345.00	345.00
31.000	345.00	345.00	345.00	345.00	345.00
31.250	345.00	345.00	345.00	345.00	345.00
31.500	345.00	345.00	345.00	345.00	345.00
31.750	345.00	345.00	345.00	345.00	345.00
32.000	345.00	345.00	345.00	345.00	345.00
32.250	345.00	345.00	345.00	345.00	345.00
32.500	345.00	345.00	345.00	345.00	345.00
32.750	345.00	345.00	345.00	345.00	345.00
33.000	345.00	345.00	345.00	345.00	345.00
33.250	345.00	345.00	345.00	345.00	345.00
33.500	345.00	345.00	345.00	345.00	345.00
33.750	345.00	345.00	345.00	345.00	345.00
34.000	345.00	345.00	345.00	345.00	345.00
34.250	345.00	345.00	345.00	345.00	345.00
34.500	345.00	345.00	345.00	345.00	345.00
34.750	345.00	345.00	345.00	345.00	345.00
35.000	345.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	345.00	345.00	345.00	345.00	345.00
0.250	345.00	345.00	345.00	345.00	345.00
0.500	345.00	345.00	345.00	345.00	345.00
0.750	345.00	345.00	345.00	345.00	345.00
1.000	345.00	345.00	345.00	345.00	345.00
1.250	345.00	345.00	345.00	345.00	345.00
1.500	345.00	345.00	345.00	345.00	345.00
1.750	345.00	345.00	345.00	345.00	345.00
2.000	345.00	345.00	345.00	345.00	345.00
2.250	345.00	345.00	345.00	345.00	345.00
2.500	345.00	345.00	345.00	345.00	345.00
2.750	345.00	345.00	345.00	345.00	345.00
3.000	345.00	345.00	345.00	345.00	345.00
3.250	345.00	345.00	345.00	345.00	345.00
3.500	345.00	345.00	345.00	345.00	345.00
3.750	345.00	345.00	345.00	345.00	345.00
4.000	345.00	345.00	345.00	345.00	345.00
4.250	345.00	345.00	345.00	345.00	345.00
4.500	345.00	345.00	345.00	345.00	345.00
4.750	345.00	345.00	345.00	345.00	345.00
5.000	345.00	345.00	345.00	345.00	345.00
5.250	345.00	345.00	345.00	345.00	345.00
5.500	345.00	345.00	345.00	345.00	345.00
5.750	345.00	345.00	345.00	345.00	345.00
6.000	345.00	345.00	345.00	345.00	345.00
6.250	345.00	345.00	345.01	345.01	345.01
6.500	345.01	345.01	345.01	345.01	345.01
6.750	345.01	345.01	345.02	345.02	345.02
7.000	345.02	345.02	345.02	345.02	345.03
7.250	345.03	345.03	345.03	345.03	345.04
7.500	345.04	345.04	345.04	345.04	345.04
7.750	345.05	345.05	345.05	345.05	345.06
8.000	345.06	345.06	345.06	345.07	345.07
8.250	345.07	345.07	345.08	345.08	345.08
8.500	345.09	345.09	345.09	345.10	345.10
8.750	345.10	345.11	345.11	345.12	345.12
9.000	345.13	345.13	345.14	345.14	345.15
9.250	345.15	345.16	345.16	345.17	345.17
9.500	345.18	345.18	345.19	345.20	345.20
9.750	345.21	345.21	345.22	345.23	345.23

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	345.24	345.25	345.25	345.26	345.27
10.250	345.28	345.29	345.29	345.30	345.31
10.500	345.32	345.33	345.34	345.36	345.37
10.750	345.38	345.39	345.40	345.42	345.43
11.000	345.44	345.45	345.47	345.48	345.50
11.250	345.52	345.55	345.58	345.61	345.65
11.500	345.69	345.74	345.81	345.89	346.00
11.750	346.13	346.30	346.51	346.73	346.99
12.000	347.33	347.78	348.29	348.78	349.19
12.250	349.48	349.68	349.82	349.92	349.97
12.500	349.99	349.98	349.94	349.88	349.81
12.750	349.73	349.65	349.56	349.47	349.38
13.000	349.28	349.19	349.09	348.99	348.89
13.250	348.79	348.69	348.59	348.50	348.40
13.500	348.30	348.21	348.11	348.02	347.93
13.750	347.84	347.74	347.66	347.57	347.48
14.000	347.40	347.31	347.23	347.16	347.09
14.250	347.02	346.95	346.89	346.83	346.77
14.500	346.72	346.68	346.64	346.60	346.56
14.750	346.52	346.49	346.46	346.43	346.40
15.000	346.37	346.34	346.31	346.29	346.26
15.250	346.24	346.21	346.19	346.17	346.15
15.500	346.13	346.11	346.09	346.07	346.05
15.750	346.03	346.01	345.99	345.98	345.96
16.000	345.94	345.92	345.90	345.88	345.86
16.250	345.84	345.82	345.80	345.78	345.76
16.500	345.74	345.72	345.70	345.68	345.66
16.750	345.64	345.62	345.60	345.58	345.56
17.000	345.53	345.51	345.49	345.47	345.45
17.250	345.43	345.41	345.40	345.39	345.37
17.500	345.36	345.35	345.34	345.33	345.32
17.750	345.31	345.30	345.30	345.29	345.28
18.000	345.28	345.27	345.27	345.26	345.25
18.250	345.25	345.25	345.24	345.24	345.23
18.500	345.23	345.23	345.23	345.22	345.22
18.750	345.22	345.22	345.21	345.21	345.21
19.000	345.21	345.21	345.21	345.20	345.20
19.250	345.20	345.20	345.20	345.20	345.20
19.500	345.19	345.19	345.19	345.19	345.19
19.750	345.19	345.19	345.19	345.19	345.18
20.000	345.18	345.18	345.18	345.18	345.18

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	345.18	345.18	345.18	345.18	345.17
20.500	345.17	345.17	345.17	345.17	345.17
20.750	345.17	345.17	345.17	345.17	345.17
21.000	345.17	345.16	345.16	345.16	345.16
21.250	345.16	345.16	345.16	345.16	345.16
21.500	345.16	345.16	345.16	345.16	345.15
21.750	345.15	345.15	345.15	345.15	345.15
22.000	345.15	345.15	345.15	345.15	345.15
22.250	345.15	345.15	345.15	345.14	345.14
22.500	345.14	345.14	345.14	345.14	345.14
22.750	345.14	345.14	345.14	345.14	345.14
23.000	345.14	345.14	345.13	345.13	345.13
23.250	345.13	345.13	345.13	345.13	345.13
23.500	345.13	345.13	345.13	345.13	345.13
23.750	345.12	345.12	345.12	345.12	345.12
24.000	345.12	345.12	345.12	345.11	345.10
24.250	345.09	345.08	345.07	345.07	345.06
24.500	345.05	345.05	345.04	345.04	345.04
24.750	345.03	345.03	345.03	345.02	345.02
25.000	345.02	345.02	345.02	345.01	345.01
25.250	345.01	345.01	345.01	345.01	345.01
25.500	345.01	345.01	345.01	345.01	345.00
25.750	345.00	345.00	345.00	345.00	345.00
26.000	345.00	345.00	345.00	345.00	345.00
26.250	345.00	345.00	345.00	345.00	345.00
26.500	345.00	345.00	345.00	345.00	345.00
26.750	345.00	345.00	345.00	345.00	345.00
27.000	345.00	345.00	345.00	345.00	345.00
27.250	345.00	345.00	345.00	345.00	345.00
27.500	345.00	345.00	345.00	345.00	345.00
27.750	345.00	345.00	345.00	345.00	345.00
28.000	345.00	345.00	345.00	345.00	345.00
28.250	345.00	345.00	345.00	345.00	345.00
28.500	345.00	345.00	345.00	345.00	345.00
28.750	345.00	345.00	345.00	345.00	345.00
29.000	345.00	345.00	345.00	345.00	345.00
29.250	345.00	345.00	345.00	345.00	345.00
29.500	345.00	345.00	345.00	345.00	345.00
29.750	345.00	345.00	345.00	345.00	345.00
30.000	345.00	345.00	345.00	345.00	345.00
30.250	345.00	345.00	345.00	345.00	345.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #1 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	345.00	345.00	345.00	345.00	345.00
30.750	345.00	345.00	345.00	345.00	345.00
31.000	345.00	345.00	345.00	345.00	345.00
31.250	345.00	345.00	345.00	345.00	345.00
31.500	345.00	345.00	345.00	345.00	345.00
31.750	345.00	345.00	345.00	345.00	345.00
32.000	345.00	345.00	345.00	345.00	345.00
32.250	345.00	345.00	345.00	345.00	345.00
32.500	345.00	345.00	345.00	345.00	345.00
32.750	345.00	345.00	345.00	345.00	345.00
33.000	345.00	345.00	345.00	345.00	345.00
33.250	345.00	345.00	345.00	345.00	345.00
33.500	345.00	345.00	345.00	345.00	345.00
33.750	345.00	345.00	345.00	345.00	345.00
34.000	345.00	345.00	345.00	345.00	345.00
34.250	345.00	345.00	345.00	345.00	345.00
34.500	345.00	345.00	345.00	345.00	345.00
34.750	345.00	345.00	345.00	345.00	345.00
35.000	345.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	325.00	325.00	325.00	325.00	325.00
0.250	325.00	325.00	325.00	325.00	325.00
0.500	325.00	325.00	325.00	325.00	325.00
0.750	325.00	325.00	325.00	325.00	325.00
1.000	325.00	325.00	325.00	325.00	325.00
1.250	325.00	325.00	325.00	325.00	325.00
1.500	325.00	325.00	325.00	325.00	325.00
1.750	325.00	325.00	325.00	325.00	325.00
2.000	325.00	325.00	325.00	325.00	325.00
2.250	325.00	325.00	325.00	325.00	325.00
2.500	325.00	325.00	325.00	325.00	325.00
2.750	325.00	325.00	325.00	325.00	325.00
3.000	325.00	325.00	325.00	325.00	325.00
3.250	325.00	325.00	325.00	325.00	325.00
3.500	325.00	325.00	325.00	325.00	325.00
3.750	325.00	325.00	325.00	325.00	325.00
4.000	325.00	325.00	325.00	325.00	325.00
4.250	325.00	325.00	325.00	325.00	325.00
4.500	325.00	325.00	325.00	325.00	325.00
4.750	325.00	325.00	325.00	325.00	325.00
5.000	325.00	325.00	325.00	325.00	325.00
5.250	325.00	325.00	325.00	325.00	325.00
5.500	325.00	325.00	325.00	325.00	325.00
5.750	325.00	325.00	325.00	325.00	325.00
6.000	325.00	325.00	325.00	325.00	325.00
6.250	325.00	325.00	325.00	325.00	325.00
6.500	325.00	325.00	325.00	325.00	325.00
6.750	325.00	325.00	325.00	325.00	325.00
7.000	325.00	325.00	325.00	325.00	325.00
7.250	325.00	325.00	325.00	325.00	325.00
7.500	325.00	325.00	325.00	325.00	325.00
7.750	325.00	325.00	325.00	325.00	325.00
8.000	325.00	325.00	325.00	325.00	325.00
8.250	325.00	325.00	325.00	325.00	325.00
8.500	325.00	325.00	325.00	325.00	325.00
8.750	325.00	325.00	325.00	325.00	325.00
9.000	325.00	325.00	325.00	325.00	325.00
9.250	325.00	325.00	325.00	325.00	325.00
9.500	325.00	325.00	325.00	325.00	325.00
9.750	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	325.00	325.00	325.00	325.00	325.00
10.250	325.00	325.00	325.00	325.00	325.00
10.500	325.00	325.00	325.00	325.00	325.00
10.750	325.00	325.00	325.00	325.00	325.00
11.000	325.00	325.00	325.00	325.00	325.00
11.250	325.00	325.00	325.00	325.00	325.00
11.500	325.00	325.00	325.00	325.00	325.00
11.750	325.00	325.01	325.02	325.04	325.06
12.000	325.11	325.17	325.27	325.39	325.53
12.250	325.65	325.76	325.86	325.94	326.01
12.500	326.07	326.11	326.14	326.16	326.17
12.750	326.18	326.18	326.18	326.17	326.17
13.000	326.16	326.15	326.14	326.13	326.12
13.250	326.11	326.09	326.08	326.06	326.05
13.500	326.03	326.02	326.00	325.99	325.97
13.750	325.95	325.93	325.92	325.90	325.88
14.000	325.86	325.84	325.82	325.80	325.78
14.250	325.76	325.74	325.72	325.70	325.68
14.500	325.66	325.64	325.62	325.60	325.58
14.750	325.56	325.54	325.52	325.49	325.47
15.000	325.45	325.43	325.42	325.40	325.39
15.250	325.37	325.36	325.35	325.33	325.32
15.500	325.31	325.30	325.30	325.29	325.28
15.750	325.27	325.26	325.26	325.25	325.24
16.000	325.24	325.23	325.23	325.22	325.22
16.250	325.21	325.21	325.20	325.20	325.19
16.500	325.19	325.19	325.18	325.18	325.18
16.750	325.17	325.17	325.17	325.17	325.16
17.000	325.16	325.16	325.16	325.16	325.15
17.250	325.15	325.15	325.15	325.15	325.14
17.500	325.14	325.14	325.14	325.14	325.14
17.750	325.13	325.13	325.13	325.13	325.13
18.000	325.13	325.12	325.12	325.12	325.12
18.250	325.12	325.12	325.11	325.11	325.11
18.500	325.11	325.11	325.11	325.11	325.11
18.750	325.11	325.11	325.10	325.10	325.10
19.000	325.10	325.10	325.10	325.10	325.10
19.250	325.10	325.10	325.10	325.10	325.10
19.500	325.10	325.10	325.10	325.10	325.09
19.750	325.09	325.09	325.09	325.09	325.09
20.000	325.09	325.09	325.09	325.09	325.09

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	325.09	325.09	325.09	325.09	325.09
20.500	325.09	325.09	325.09	325.09	325.09
20.750	325.09	325.08	325.08	325.08	325.08
21.000	325.08	325.08	325.08	325.08	325.08
21.250	325.08	325.08	325.08	325.08	325.08
21.500	325.08	325.08	325.08	325.08	325.08
21.750	325.08	325.08	325.08	325.08	325.08
22.000	325.08	325.08	325.08	325.08	325.08
22.250	325.07	325.07	325.07	325.07	325.07
22.500	325.07	325.07	325.07	325.07	325.07
22.750	325.07	325.07	325.07	325.07	325.07
23.000	325.07	325.07	325.07	325.07	325.07
23.250	325.07	325.07	325.07	325.07	325.07
23.500	325.07	325.07	325.07	325.07	325.06
23.750	325.06	325.06	325.06	325.06	325.06
24.000	325.06	325.06	325.06	325.06	325.06
24.250	325.05	325.05	325.05	325.04	325.04
24.500	325.04	325.03	325.03	325.03	325.03
24.750	325.02	325.02	325.02	325.02	325.02
25.000	325.01	325.01	325.01	325.01	325.01
25.250	325.01	325.01	325.01	325.01	325.01
25.500	325.01	325.01	325.01	325.00	325.00
25.750	325.00	325.00	325.00	325.00	325.00
26.000	325.00	325.00	325.00	325.00	325.00
26.250	325.00	325.00	325.00	325.00	325.00
26.500	325.00	325.00	325.00	325.00	325.00
26.750	325.00	325.00	325.00	325.00	325.00
27.000	325.00	325.00	325.00	325.00	325.00
27.250	325.00	325.00	325.00	325.00	325.00
27.500	325.00	325.00	325.00	325.00	325.00
27.750	325.00	325.00	325.00	325.00	325.00
28.000	325.00	325.00	325.00	325.00	325.00
28.250	325.00	325.00	325.00	325.00	325.00
28.500	325.00	325.00	325.00	325.00	325.00
28.750	325.00	325.00	325.00	325.00	325.00
29.000	325.00	325.00	325.00	325.00	325.00
29.250	325.00	325.00	325.00	325.00	325.00
29.500	325.00	325.00	325.00	325.00	325.00
29.750	325.00	325.00	325.00	325.00	325.00
30.000	325.00	325.00	325.00	325.00	325.00
30.250	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	325.00	325.00	325.00	325.00	325.00
30.750	325.00	325.00	325.00	325.00	325.00
31.000	325.00	325.00	325.00	325.00	325.00
31.250	325.00	325.00	325.00	325.00	325.00
31.500	325.00	325.00	325.00	325.00	325.00
31.750	325.00	325.00	325.00	325.00	325.00
32.000	325.00	325.00	325.00	325.00	325.00
32.250	325.00	325.00	325.00	325.00	325.00
32.500	325.00	325.00	325.00	325.00	325.00
32.750	325.00	325.00	325.00	325.00	325.00
33.000	325.00	325.00	325.00	325.00	325.00
33.250	325.00	325.00	325.00	325.00	325.00
33.500	325.00	325.00	325.00	325.00	325.00
33.750	325.00	325.00	325.00	325.00	325.00
34.000	325.00	325.00	325.00	325.00	325.00
34.250	325.00	325.00	325.00	325.00	325.00
34.500	325.00	325.00	325.00	325.00	325.00
34.750	325.00	325.00	325.00	325.00	325.00
35.000	325.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	325.00	325.00	325.00	325.00	325.00
0.250	325.00	325.00	325.00	325.00	325.00
0.500	325.00	325.00	325.00	325.00	325.00
0.750	325.00	325.00	325.00	325.00	325.00
1.000	325.00	325.00	325.00	325.00	325.00
1.250	325.00	325.00	325.00	325.00	325.00
1.500	325.00	325.00	325.00	325.00	325.00
1.750	325.00	325.00	325.00	325.00	325.00
2.000	325.00	325.00	325.00	325.00	325.00
2.250	325.00	325.00	325.00	325.00	325.00
2.500	325.00	325.00	325.00	325.00	325.00
2.750	325.00	325.00	325.00	325.00	325.00
3.000	325.00	325.00	325.00	325.00	325.00
3.250	325.00	325.00	325.00	325.00	325.00
3.500	325.00	325.00	325.00	325.00	325.00
3.750	325.00	325.00	325.00	325.00	325.00
4.000	325.00	325.00	325.00	325.00	325.00
4.250	325.00	325.00	325.00	325.00	325.00
4.500	325.00	325.00	325.00	325.00	325.00
4.750	325.00	325.00	325.00	325.00	325.00
5.000	325.00	325.00	325.00	325.00	325.00
5.250	325.00	325.00	325.00	325.00	325.00
5.500	325.00	325.00	325.00	325.00	325.00
5.750	325.00	325.00	325.00	325.00	325.00
6.000	325.00	325.00	325.00	325.00	325.00
6.250	325.00	325.00	325.00	325.00	325.00
6.500	325.00	325.00	325.00	325.00	325.00
6.750	325.00	325.00	325.00	325.00	325.00
7.000	325.00	325.00	325.00	325.00	325.00
7.250	325.00	325.00	325.00	325.00	325.00
7.500	325.00	325.00	325.00	325.00	325.00
7.750	325.00	325.00	325.00	325.00	325.00
8.000	325.00	325.00	325.00	325.00	325.00
8.250	325.00	325.00	325.00	325.00	325.00
8.500	325.00	325.00	325.00	325.00	325.00
8.750	325.00	325.00	325.00	325.00	325.00
9.000	325.00	325.00	325.00	325.00	325.00
9.250	325.00	325.00	325.00	325.00	325.00
9.500	325.00	325.00	325.00	325.00	325.00
9.750	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	325.00	325.00	325.00	325.00	325.00
10.250	325.00	325.00	325.00	325.00	325.00
10.500	325.00	325.00	325.00	325.00	325.00
10.750	325.00	325.00	325.00	325.00	325.00
11.000	325.00	325.00	325.00	325.00	325.00
11.250	325.00	325.00	325.01	325.01	325.01
11.500	325.02	325.02	325.03	325.04	325.05
11.750	325.07	325.09	325.13	325.17	325.23
12.000	325.32	325.45	325.62	325.83	326.06
12.250	326.28	326.46	326.60	326.70	326.78
12.500	326.83	326.86	326.86	326.86	326.84
12.750	326.82	326.79	326.76	326.73	326.71
13.000	326.68	326.66	326.63	326.61	326.59
13.250	326.57	326.55	326.53	326.51	326.50
13.500	326.48	326.47	326.45	326.44	326.42
13.750	326.41	326.39	326.38	326.36	326.35
14.000	326.33	326.32	326.30	326.29	326.27
14.250	326.26	326.24	326.23	326.22	326.20
14.500	326.19	326.18	326.16	326.15	326.13
14.750	326.12	326.10	326.09	326.07	326.06
15.000	326.04	326.02	326.01	325.99	325.97
15.250	325.96	325.94	325.92	325.90	325.88
15.500	325.86	325.84	325.82	325.80	325.78
15.750	325.76	325.74	325.72	325.70	325.68
16.000	325.66	325.64	325.62	325.59	325.57
16.250	325.55	325.53	325.50	325.48	325.46
16.500	325.44	325.42	325.40	325.39	325.37
16.750	325.36	325.35	325.33	325.32	325.31
17.000	325.30	325.29	325.28	325.28	325.27
17.250	325.26	325.26	325.25	325.24	325.24
17.500	325.23	325.23	325.22	325.22	325.21
17.750	325.21	325.21	325.20	325.20	325.19
18.000	325.19	325.19	325.18	325.18	325.18
18.250	325.18	325.17	325.17	325.17	325.17
18.500	325.16	325.16	325.16	325.16	325.16
18.750	325.16	325.15	325.15	325.15	325.15
19.000	325.15	325.15	325.15	325.15	325.14
19.250	325.14	325.14	325.14	325.14	325.14
19.500	325.14	325.14	325.14	325.14	325.14
19.750	325.13	325.13	325.13	325.13	325.13
20.000	325.13	325.13	325.13	325.13	325.13

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	325.13	325.13	325.13	325.13	325.12
20.500	325.12	325.12	325.12	325.12	325.12
20.750	325.12	325.12	325.12	325.12	325.12
21.000	325.12	325.12	325.12	325.12	325.12
21.250	325.12	325.12	325.11	325.11	325.11
21.500	325.11	325.11	325.11	325.11	325.11
21.750	325.11	325.11	325.11	325.11	325.11
22.000	325.11	325.11	325.11	325.11	325.11
22.250	325.11	325.11	325.10	325.10	325.10
22.500	325.10	325.10	325.10	325.10	325.10
22.750	325.10	325.10	325.10	325.10	325.10
23.000	325.10	325.10	325.10	325.10	325.10
23.250	325.10	325.10	325.09	325.09	325.09
23.500	325.09	325.09	325.09	325.09	325.09
23.750	325.09	325.09	325.09	325.09	325.09
24.000	325.09	325.09	325.09	325.08	325.08
24.250	325.07	325.07	325.06	325.06	325.05
24.500	325.05	325.05	325.04	325.04	325.04
24.750	325.03	325.03	325.03	325.03	325.02
25.000	325.02	325.02	325.02	325.02	325.01
25.250	325.01	325.01	325.01	325.01	325.01
25.500	325.01	325.01	325.01	325.01	325.01
25.750	325.01	325.01	325.00	325.00	325.00
26.000	325.00	325.00	325.00	325.00	325.00
26.250	325.00	325.00	325.00	325.00	325.00
26.500	325.00	325.00	325.00	325.00	325.00
26.750	325.00	325.00	325.00	325.00	325.00
27.000	325.00	325.00	325.00	325.00	325.00
27.250	325.00	325.00	325.00	325.00	325.00
27.500	325.00	325.00	325.00	325.00	325.00
27.750	325.00	325.00	325.00	325.00	325.00
28.000	325.00	325.00	325.00	325.00	325.00
28.250	325.00	325.00	325.00	325.00	325.00
28.500	325.00	325.00	325.00	325.00	325.00
28.750	325.00	325.00	325.00	325.00	325.00
29.000	325.00	325.00	325.00	325.00	325.00
29.250	325.00	325.00	325.00	325.00	325.00
29.500	325.00	325.00	325.00	325.00	325.00
29.750	325.00	325.00	325.00	325.00	325.00
30.000	325.00	325.00	325.00	325.00	325.00
30.250	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	325.00	325.00	325.00	325.00	325.00
30.750	325.00	325.00	325.00	325.00	325.00
31.000	325.00	325.00	325.00	325.00	325.00
31.250	325.00	325.00	325.00	325.00	325.00
31.500	325.00	325.00	325.00	325.00	325.00
31.750	325.00	325.00	325.00	325.00	325.00
32.000	325.00	325.00	325.00	325.00	325.00
32.250	325.00	325.00	325.00	325.00	325.00
32.500	325.00	325.00	325.00	325.00	325.00
32.750	325.00	325.00	325.00	325.00	325.00
33.000	325.00	325.00	325.00	325.00	325.00
33.250	325.00	325.00	325.00	325.00	325.00
33.500	325.00	325.00	325.00	325.00	325.00
33.750	325.00	325.00	325.00	325.00	325.00
34.000	325.00	325.00	325.00	325.00	325.00
34.250	325.00	325.00	325.00	325.00	325.00
34.500	325.00	325.00	325.00	325.00	325.00
34.750	325.00	325.00	325.00	325.00	325.00
35.000	325.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	325.00	325.00	325.00	325.00	325.00
0.250	325.00	325.00	325.00	325.00	325.00
0.500	325.00	325.00	325.00	325.00	325.00
0.750	325.00	325.00	325.00	325.00	325.00
1.000	325.00	325.00	325.00	325.00	325.00
1.250	325.00	325.00	325.00	325.00	325.00
1.500	325.00	325.00	325.00	325.00	325.00
1.750	325.00	325.00	325.00	325.00	325.00
2.000	325.00	325.00	325.00	325.00	325.00
2.250	325.00	325.00	325.00	325.00	325.00
2.500	325.00	325.00	325.00	325.00	325.00
2.750	325.00	325.00	325.00	325.00	325.00
3.000	325.00	325.00	325.00	325.00	325.00
3.250	325.00	325.00	325.00	325.00	325.00
3.500	325.00	325.00	325.00	325.00	325.00
3.750	325.00	325.00	325.00	325.00	325.00
4.000	325.00	325.00	325.00	325.00	325.00
4.250	325.00	325.00	325.00	325.00	325.00
4.500	325.00	325.00	325.00	325.00	325.00
4.750	325.00	325.00	325.00	325.00	325.00
5.000	325.00	325.00	325.00	325.00	325.00
5.250	325.00	325.00	325.00	325.00	325.00
5.500	325.00	325.00	325.00	325.00	325.00
5.750	325.00	325.00	325.00	325.00	325.00
6.000	325.00	325.00	325.00	325.00	325.00
6.250	325.00	325.00	325.00	325.00	325.00
6.500	325.00	325.00	325.00	325.00	325.00
6.750	325.00	325.00	325.00	325.00	325.00
7.000	325.00	325.00	325.00	325.00	325.00
7.250	325.00	325.00	325.00	325.00	325.00
7.500	325.00	325.00	325.00	325.00	325.00
7.750	325.00	325.00	325.00	325.00	325.00
8.000	325.00	325.00	325.00	325.00	325.00
8.250	325.00	325.00	325.00	325.00	325.00
8.500	325.00	325.00	325.00	325.00	325.00
8.750	325.00	325.00	325.00	325.00	325.00
9.000	325.00	325.00	325.00	325.00	325.00
9.250	325.00	325.00	325.00	325.00	325.00
9.500	325.00	325.00	325.00	325.00	325.00
9.750	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	325.01	325.01	325.01	325.01	325.02
10.250	325.02	325.03	325.03	325.03	325.04
10.500	325.05	325.05	325.06	325.06	325.07
10.750	325.08	325.09	325.10	325.10	325.11
11.000	325.12	325.13	325.14	325.15	325.17
11.250	325.18	325.19	325.21	325.23	325.25
11.500	325.27	325.29	325.32	325.35	325.40
11.750	325.46	325.54	325.65	325.78	325.97
12.000	326.21	326.52	326.89	327.27	327.59
12.250	327.83	327.97	328.04	328.06	328.04
12.500	327.99	327.92	327.84	327.74	327.65
12.750	327.56	327.47	327.39	327.32	327.26
13.000	327.20	327.15	327.10	327.06	327.02
13.250	326.99	326.96	326.93	326.90	326.87
13.500	326.85	326.82	326.80	326.78	326.77
13.750	326.75	326.73	326.71	326.70	326.69
14.000	326.67	326.66	326.64	326.63	326.62
14.250	326.61	326.60	326.59	326.58	326.57
14.500	326.56	326.55	326.55	326.54	326.53
14.750	326.52	326.52	326.51	326.50	326.50
15.000	326.49	326.48	326.48	326.47	326.46
15.250	326.45	326.45	326.44	326.43	326.42
15.500	326.41	326.40	326.39	326.38	326.37
15.750	326.36	326.35	326.34	326.33	326.31
16.000	326.30	326.29	326.28	326.27	326.26
16.250	326.24	326.23	326.22	326.21	326.20
16.500	326.19	326.17	326.16	326.15	326.14
16.750	326.12	326.11	326.10	326.08	326.07
17.000	326.06	326.04	326.03	326.01	326.00
17.250	325.98	325.97	325.95	325.94	325.92
17.500	325.90	325.88	325.87	325.85	325.83
17.750	325.81	325.80	325.78	325.76	325.74
18.000	325.72	325.70	325.68	325.67	325.65
18.250	325.63	325.61	325.59	325.57	325.55
18.500	325.53	325.51	325.49	325.47	325.45
18.750	325.44	325.42	325.41	325.39	325.38
19.000	325.37	325.36	325.35	325.34	325.33
19.250	325.33	325.32	325.31	325.31	325.30
19.500	325.30	325.29	325.29	325.28	325.28
19.750	325.27	325.27	325.27	325.26	325.26
20.000	325.26	325.26	325.25	325.25	325.25

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	325.25	325.24	325.24	325.24	325.24
20.500	325.24	325.23	325.23	325.23	325.23
20.750	325.23	325.23	325.22	325.22	325.22
21.000	325.22	325.22	325.22	325.22	325.22
21.250	325.21	325.21	325.21	325.21	325.21
21.500	325.21	325.21	325.21	325.21	325.20
21.750	325.20	325.20	325.20	325.20	325.20
22.000	325.20	325.20	325.20	325.20	325.20
22.250	325.19	325.19	325.19	325.19	325.19
22.500	325.19	325.19	325.19	325.19	325.19
22.750	325.18	325.18	325.18	325.18	325.18
23.000	325.18	325.18	325.18	325.18	325.18
23.250	325.18	325.17	325.17	325.17	325.17
23.500	325.17	325.17	325.17	325.17	325.17
23.750	325.17	325.16	325.16	325.16	325.16
24.000	325.16	325.16	325.16	325.15	325.15
24.250	325.14	325.13	325.12	325.11	325.10
24.500	325.09	325.08	325.08	325.07	325.06
24.750	325.06	325.05	325.05	325.05	325.04
25.000	325.04	325.04	325.03	325.03	325.03
25.250	325.02	325.02	325.02	325.02	325.02
25.500	325.02	325.01	325.01	325.01	325.01
25.750	325.01	325.01	325.01	325.01	325.01
26.000	325.01	325.01	325.01	325.01	325.00
26.250	325.00	325.00	325.00	325.00	325.00
26.500	325.00	325.00	325.00	325.00	325.00
26.750	325.00	325.00	325.00	325.00	325.00
27.000	325.00	325.00	325.00	325.00	325.00
27.250	325.00	325.00	325.00	325.00	325.00
27.500	325.00	325.00	325.00	325.00	325.00
27.750	325.00	325.00	325.00	325.00	325.00
28.000	325.00	325.00	325.00	325.00	325.00
28.250	325.00	325.00	325.00	325.00	325.00
28.500	325.00	325.00	325.00	325.00	325.00
28.750	325.00	325.00	325.00	325.00	325.00
29.000	325.00	325.00	325.00	325.00	325.00
29.250	325.00	325.00	325.00	325.00	325.00
29.500	325.00	325.00	325.00	325.00	325.00
29.750	325.00	325.00	325.00	325.00	325.00
30.000	325.00	325.00	325.00	325.00	325.00
30.250	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	325.00	325.00	325.00	325.00	325.00
30.750	325.00	325.00	325.00	325.00	325.00
31.000	325.00	325.00	325.00	325.00	325.00
31.250	325.00	325.00	325.00	325.00	325.00
31.500	325.00	325.00	325.00	325.00	325.00
31.750	325.00	325.00	325.00	325.00	325.00
32.000	325.00	325.00	325.00	325.00	325.00
32.250	325.00	325.00	325.00	325.00	325.00
32.500	325.00	325.00	325.00	325.00	325.00
32.750	325.00	325.00	325.00	325.00	325.00
33.000	325.00	325.00	325.00	325.00	325.00
33.250	325.00	325.00	325.00	325.00	325.00
33.500	325.00	325.00	325.00	325.00	325.00
33.750	325.00	325.00	325.00	325.00	325.00
34.000	325.00	325.00	325.00	325.00	325.00
34.250	325.00	325.00	325.00	325.00	325.00
34.500	325.00	325.00	325.00	325.00	325.00
34.750	325.00	325.00	325.00	325.00	325.00
35.000	325.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	325.00	325.00	325.00	325.00	325.00
0.250	325.00	325.00	325.00	325.00	325.00
0.500	325.00	325.00	325.00	325.00	325.00
0.750	325.00	325.00	325.00	325.00	325.00
1.000	325.00	325.00	325.00	325.00	325.00
1.250	325.00	325.00	325.00	325.00	325.00
1.500	325.00	325.00	325.00	325.00	325.00
1.750	325.00	325.00	325.00	325.00	325.00
2.000	325.00	325.00	325.00	325.00	325.00
2.250	325.00	325.00	325.00	325.00	325.00
2.500	325.00	325.00	325.00	325.00	325.00
2.750	325.00	325.00	325.00	325.00	325.00
3.000	325.00	325.00	325.00	325.00	325.00
3.250	325.00	325.00	325.00	325.00	325.00
3.500	325.00	325.00	325.00	325.00	325.00
3.750	325.00	325.00	325.00	325.00	325.00
4.000	325.00	325.00	325.00	325.00	325.00
4.250	325.00	325.00	325.00	325.00	325.00
4.500	325.00	325.00	325.00	325.00	325.00
4.750	325.00	325.00	325.00	325.00	325.00
5.000	325.00	325.00	325.00	325.00	325.00
5.250	325.00	325.00	325.00	325.00	325.00
5.500	325.00	325.00	325.00	325.00	325.00
5.750	325.00	325.00	325.00	325.00	325.00
6.000	325.00	325.00	325.00	325.00	325.00
6.250	325.00	325.00	325.00	325.00	325.00
6.500	325.00	325.00	325.00	325.00	325.00
6.750	325.00	325.00	325.00	325.00	325.00
7.000	325.00	325.00	325.00	325.00	325.00
7.250	325.00	325.00	325.00	325.00	325.00
7.500	325.00	325.00	325.00	325.00	325.00
7.750	325.00	325.00	325.00	325.00	325.00
8.000	325.00	325.00	325.00	325.00	325.00
8.250	325.00	325.00	325.00	325.00	325.00
8.500	325.00	325.00	325.00	325.00	325.00
8.750	325.00	325.00	325.00	325.00	325.00
9.000	325.00	325.00	325.01	325.01	325.01
9.250	325.01	325.02	325.02	325.02	325.03
9.500	325.03	325.04	325.04	325.05	325.05
9.750	325.06	325.06	325.07	325.08	325.08

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	325.09	325.10	325.10	325.11	325.12
10.250	325.13	325.14	325.15	325.16	325.17
10.500	325.18	325.19	325.20	325.21	325.23
10.750	325.24	325.25	325.27	325.28	325.30
11.000	325.31	325.33	325.35	325.36	325.38
11.250	325.41	325.43	325.46	325.49	325.52
11.500	325.55	325.59	325.64	325.71	325.79
11.750	325.91	326.06	326.25	326.47	326.72
12.000	327.03	327.39	327.78	328.17	328.51
12.250	328.75	328.91	328.98	329.00	328.96
12.500	328.89	328.77	328.63	328.47	328.29
12.750	328.13	327.98	327.84	327.72	327.62
13.000	327.54	327.46	327.39	327.33	327.27
13.250	327.22	327.18	327.14	327.11	327.08
13.500	327.05	327.03	327.01	326.99	326.98
13.750	326.96	326.94	326.92	326.91	326.89
14.000	326.87	326.86	326.84	326.83	326.81
14.250	326.80	326.79	326.77	326.76	326.75
14.500	326.74	326.73	326.72	326.71	326.70
14.750	326.69	326.68	326.67	326.67	326.66
15.000	326.65	326.64	326.63	326.63	326.62
15.250	326.61	326.60	326.60	326.59	326.58
15.500	326.57	326.57	326.56	326.55	326.54
15.750	326.54	326.53	326.52	326.52	326.51
16.000	326.50	326.49	326.49	326.48	326.47
16.250	326.46	326.45	326.44	326.43	326.42
16.500	326.41	326.40	326.39	326.38	326.37
16.750	326.36	326.36	326.35	326.34	326.33
17.000	326.32	326.31	326.30	326.29	326.28
17.250	326.27	326.26	326.25	326.24	326.23
17.500	326.22	326.21	326.20	326.19	326.18
17.750	326.17	326.16	326.15	326.14	326.13
18.000	326.11	326.10	326.09	326.08	326.06
18.250	326.05	326.04	326.02	326.01	325.99
18.500	325.98	325.96	325.95	325.93	325.92
18.750	325.90	325.89	325.88	325.86	325.85
19.000	325.83	325.82	325.80	325.78	325.77
19.250	325.75	325.74	325.72	325.71	325.69
19.500	325.68	325.66	325.65	325.63	325.62
19.750	325.60	325.59	325.57	325.55	325.54
20.000	325.52	325.51	325.49	325.48	325.46

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	325.45	325.44	325.42	325.41	325.40
20.500	325.39	325.39	325.38	325.37	325.36
20.750	325.36	325.35	325.35	325.34	325.34
21.000	325.33	325.33	325.32	325.32	325.32
21.250	325.31	325.31	325.31	325.30	325.30
21.500	325.30	325.30	325.29	325.29	325.29
21.750	325.29	325.28	325.28	325.28	325.28
22.000	325.28	325.27	325.27	325.27	325.27
22.250	325.27	325.27	325.26	325.26	325.26
22.500	325.26	325.26	325.26	325.26	325.25
22.750	325.25	325.25	325.25	325.25	325.25
23.000	325.25	325.24	325.24	325.24	325.24
23.250	325.24	325.24	325.24	325.23	325.23
23.500	325.23	325.23	325.23	325.23	325.23
23.750	325.23	325.22	325.22	325.22	325.22
24.000	325.22	325.22	325.21	325.21	325.20
24.250	325.19	325.17	325.16	325.15	325.13
24.500	325.12	325.11	325.10	325.10	325.09
24.750	325.08	325.07	325.07	325.06	325.06
25.000	325.05	325.05	325.04	325.04	325.04
25.250	325.03	325.03	325.03	325.03	325.02
25.500	325.02	325.02	325.02	325.02	325.02
25.750	325.01	325.01	325.01	325.01	325.01
26.000	325.01	325.01	325.01	325.01	325.01
26.250	325.01	325.01	325.00	325.00	325.00
26.500	325.00	325.00	325.00	325.00	325.00
26.750	325.00	325.00	325.00	325.00	325.00
27.000	325.00	325.00	325.00	325.00	325.00
27.250	325.00	325.00	325.00	325.00	325.00
27.500	325.00	325.00	325.00	325.00	325.00
27.750	325.00	325.00	325.00	325.00	325.00
28.000	325.00	325.00	325.00	325.00	325.00
28.250	325.00	325.00	325.00	325.00	325.00
28.500	325.00	325.00	325.00	325.00	325.00
28.750	325.00	325.00	325.00	325.00	325.00
29.000	325.00	325.00	325.00	325.00	325.00
29.250	325.00	325.00	325.00	325.00	325.00
29.500	325.00	325.00	325.00	325.00	325.00
29.750	325.00	325.00	325.00	325.00	325.00
30.000	325.00	325.00	325.00	325.00	325.00
30.250	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	325.00	325.00	325.00	325.00	325.00
30.750	325.00	325.00	325.00	325.00	325.00
31.000	325.00	325.00	325.00	325.00	325.00
31.250	325.00	325.00	325.00	325.00	325.00
31.500	325.00	325.00	325.00	325.00	325.00
31.750	325.00	325.00	325.00	325.00	325.00
32.000	325.00	325.00	325.00	325.00	325.00
32.250	325.00	325.00	325.00	325.00	325.00
32.500	325.00	325.00	325.00	325.00	325.00
32.750	325.00	325.00	325.00	325.00	325.00
33.000	325.00	325.00	325.00	325.00	325.00
33.250	325.00	325.00	325.00	325.00	325.00
33.500	325.00	325.00	325.00	325.00	325.00
33.750	325.00	325.00	325.00	325.00	325.00
34.000	325.00	325.00	325.00	325.00	325.00
34.250	325.00	325.00	325.00	325.00	325.00
34.500	325.00	325.00	325.00	325.00	325.00
34.750	325.00	325.00	325.00	325.00	325.00
35.000	325.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	325.00	325.00	325.00	325.00	325.00
0.250	325.00	325.00	325.00	325.00	325.00
0.500	325.00	325.00	325.00	325.00	325.00
0.750	325.00	325.00	325.00	325.00	325.00
1.000	325.00	325.00	325.00	325.00	325.00
1.250	325.00	325.00	325.00	325.00	325.00
1.500	325.00	325.00	325.00	325.00	325.00
1.750	325.00	325.00	325.00	325.00	325.00
2.000	325.00	325.00	325.00	325.00	325.00
2.250	325.00	325.00	325.00	325.00	325.00
2.500	325.00	325.00	325.00	325.00	325.00
2.750	325.00	325.00	325.00	325.00	325.00
3.000	325.00	325.00	325.00	325.00	325.00
3.250	325.00	325.00	325.00	325.00	325.00
3.500	325.00	325.00	325.00	325.00	325.00
3.750	325.00	325.00	325.00	325.00	325.00
4.000	325.00	325.00	325.00	325.00	325.00
4.250	325.00	325.00	325.00	325.00	325.00
4.500	325.00	325.00	325.00	325.00	325.00
4.750	325.00	325.00	325.00	325.00	325.00
5.000	325.00	325.00	325.00	325.00	325.00
5.250	325.00	325.00	325.00	325.00	325.00
5.500	325.00	325.00	325.00	325.00	325.00
5.750	325.00	325.00	325.00	325.00	325.00
6.000	325.00	325.00	325.00	325.00	325.00
6.250	325.00	325.00	325.00	325.00	325.00
6.500	325.00	325.00	325.00	325.00	325.00
6.750	325.00	325.00	325.00	325.00	325.00
7.000	325.00	325.00	325.00	325.00	325.00
7.250	325.00	325.00	325.00	325.00	325.00
7.500	325.00	325.00	325.00	325.00	325.01
7.750	325.01	325.01	325.01	325.02	325.02
8.000	325.02	325.02	325.03	325.03	325.04
8.250	325.04	325.04	325.05	325.05	325.06
8.500	325.06	325.07	325.08	325.08	325.09
8.750	325.10	325.10	325.11	325.12	325.13
9.000	325.14	325.14	325.15	325.16	325.17
9.250	325.18	325.19	325.20	325.21	325.23
9.500	325.24	325.25	325.26	325.27	325.29
9.750	325.30	325.31	325.33	325.34	325.35

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	325.37	325.38	325.40	325.41	325.43
10.250	325.45	325.47	325.48	325.50	325.52
10.500	325.54	325.57	325.60	325.62	325.66
10.750	325.69	325.73	325.77	325.81	325.85
11.000	325.90	325.95	326.00	326.06	326.12
11.250	326.18	326.25	326.33	326.40	326.49
11.500	326.57	326.65	326.73	326.83	326.95
11.750	327.10	327.27	327.47	327.69	327.94
12.000	328.25	328.67	329.18	329.75	330.28
12.250	330.67	330.90	330.98	330.98	330.92
12.500	330.81	330.67	330.52	330.34	330.14
12.750	329.93	329.71	329.48	329.25	329.03
13.000	328.80	328.58	328.38	328.20	328.04
13.250	327.90	327.79	327.69	327.61	327.55
13.500	327.49	327.44	327.40	327.36	327.33
13.750	327.30	327.27	327.24	327.22	327.20
14.000	327.18	327.16	327.14	327.12	327.11
14.250	327.09	327.08	327.07	327.06	327.05
14.500	327.04	327.03	327.02	327.01	327.00
14.750	326.99	326.98	326.97	326.97	326.96
15.000	326.95	326.94	326.93	326.92	326.91
15.250	326.90	326.89	326.88	326.86	326.85
15.500	326.84	326.83	326.82	326.81	326.80
15.750	326.79	326.78	326.77	326.76	326.75
16.000	326.73	326.72	326.71	326.70	326.69
16.250	326.68	326.67	326.66	326.65	326.64
16.500	326.64	326.63	326.62	326.61	326.61
16.750	326.60	326.59	326.59	326.58	326.58
17.000	326.57	326.56	326.56	326.55	326.55
17.250	326.54	326.54	326.53	326.53	326.52
17.500	326.52	326.51	326.51	326.50	326.49
17.750	326.49	326.48	326.48	326.47	326.46
18.000	326.45	326.45	326.44	326.43	326.42
18.250	326.41	326.40	326.40	326.39	326.38
18.500	326.37	326.36	326.36	326.35	326.34
18.750	326.33	326.33	326.32	326.31	326.31
19.000	326.30	326.29	326.29	326.28	326.27
19.250	326.27	326.26	326.25	326.25	326.24
19.500	326.24	326.23	326.22	326.22	326.21
19.750	326.21	326.20	326.20	326.19	326.18
20.000	326.18	326.17	326.16	326.16	326.15

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	326.14	326.14	326.13	326.12	326.11
20.500	326.11	326.10	326.09	326.08	326.08
20.750	326.07	326.06	326.05	326.04	326.03
21.000	326.03	326.02	326.01	326.00	325.99
21.250	325.98	325.97	325.96	325.95	325.94
21.500	325.93	325.92	325.91	325.90	325.89
21.750	325.88	325.87	325.86	325.85	325.84
22.000	325.83	325.82	325.81	325.80	325.78
22.250	325.77	325.76	325.75	325.74	325.73
22.500	325.72	325.70	325.69	325.68	325.67
22.750	325.66	325.64	325.63	325.62	325.61
23.000	325.60	325.58	325.57	325.56	325.55
23.250	325.53	325.52	325.51	325.49	325.48
23.500	325.47	325.46	325.45	325.44	325.43
23.750	325.42	325.41	325.40	325.40	325.39
24.000	325.38	325.38	325.37	325.35	325.34
24.250	325.31	325.29	325.27	325.25	325.23
24.500	325.21	325.19	325.18	325.16	325.15
24.750	325.14	325.12	325.11	325.10	325.10
25.000	325.09	325.08	325.07	325.07	325.06
25.250	325.06	325.05	325.05	325.04	325.04
25.500	325.04	325.03	325.03	325.03	325.03
25.750	325.02	325.02	325.02	325.02	325.02
26.000	325.02	325.01	325.01	325.01	325.01
26.250	325.01	325.01	325.01	325.01	325.01
26.500	325.01	325.01	325.01	325.01	325.00
26.750	325.00	325.00	325.00	325.00	325.00
27.000	325.00	325.00	325.00	325.00	325.00
27.250	325.00	325.00	325.00	325.00	325.00
27.500	325.00	325.00	325.00	325.00	325.00
27.750	325.00	325.00	325.00	325.00	325.00
28.000	325.00	325.00	325.00	325.00	325.00
28.250	325.00	325.00	325.00	325.00	325.00
28.500	325.00	325.00	325.00	325.00	325.00
28.750	325.00	325.00	325.00	325.00	325.00
29.000	325.00	325.00	325.00	325.00	325.00
29.250	325.00	325.00	325.00	325.00	325.00
29.500	325.00	325.00	325.00	325.00	325.00
29.750	325.00	325.00	325.00	325.00	325.00
30.000	325.00	325.00	325.00	325.00	325.00
30.250	325.00	325.00	325.00	325.00	325.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #2 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	325.00	325.00	325.00	325.00	325.00
30.750	325.00	325.00	325.00	325.00	325.00
31.000	325.00	325.00	325.00	325.00	325.00
31.250	325.00	325.00	325.00	325.00	325.00
31.500	325.00	325.00	325.00	325.00	325.00
31.750	325.00	325.00	325.00	325.00	325.00
32.000	325.00	325.00	325.00	325.00	325.00
32.250	325.00	325.00	325.00	325.00	325.00
32.500	325.00	325.00	325.00	325.00	325.00
32.750	325.00	325.00	325.00	325.00	325.00
33.000	325.00	325.00	325.00	325.00	325.00
33.250	325.00	325.00	325.00	325.00	325.00
33.500	325.00	325.00	325.00	325.00	325.00
33.750	325.00	325.00	325.00	325.00	325.00
34.000	325.00	325.00	325.00	325.00	325.00
34.250	325.00	325.00	325.00	325.00	325.00
34.500	325.00	325.00	325.00	325.00	325.00
34.750	325.00	325.00	325.00	325.00	325.00
35.000	325.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	290.00	290.00	290.00	290.00	290.00
0.250	290.00	290.00	290.00	290.00	290.00
0.500	290.00	290.00	290.00	290.00	290.00
0.750	290.00	290.00	290.00	290.00	290.00
1.000	290.00	290.00	290.00	290.00	290.00
1.250	290.00	290.00	290.00	290.00	290.00
1.500	290.00	290.00	290.00	290.00	290.00
1.750	290.00	290.00	290.00	290.00	290.00
2.000	290.00	290.00	290.00	290.00	290.00
2.250	290.00	290.00	290.00	290.00	290.00
2.500	290.00	290.00	290.00	290.00	290.00
2.750	290.00	290.00	290.00	290.00	290.00
3.000	290.00	290.00	290.00	290.00	290.00
3.250	290.00	290.00	290.00	290.00	290.00
3.500	290.00	290.00	290.00	290.00	290.00
3.750	290.00	290.00	290.00	290.00	290.00
4.000	290.00	290.00	290.00	290.00	290.00
4.250	290.00	290.00	290.00	290.00	290.00
4.500	290.00	290.00	290.00	290.00	290.00
4.750	290.00	290.00	290.00	290.00	290.00
5.000	290.00	290.00	290.00	290.00	290.00
5.250	290.00	290.00	290.00	290.00	290.00
5.500	290.00	290.00	290.00	290.00	290.00
5.750	290.00	290.00	290.00	290.00	290.00
6.000	290.00	290.00	290.00	290.00	290.00
6.250	290.00	290.00	290.00	290.00	290.00
6.500	290.00	290.00	290.00	290.00	290.00
6.750	290.00	290.00	290.00	290.00	290.00
7.000	290.00	290.00	290.00	290.00	290.00
7.250	290.00	290.00	290.00	290.00	290.00
7.500	290.00	290.00	290.00	290.00	290.00
7.750	290.00	290.00	290.00	290.00	290.00
8.000	290.00	290.00	290.00	290.00	290.00
8.250	290.00	290.00	290.00	290.00	290.00
8.500	290.00	290.00	290.00	290.00	290.00
8.750	290.00	290.00	290.00	290.00	290.00
9.000	290.00	290.00	290.00	290.00	290.00
9.250	290.00	290.00	290.00	290.00	290.00
9.500	290.00	290.00	290.00	290.00	290.00
9.750	290.00	290.00	290.00	290.00	290.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	290.00	290.00	290.00	290.00	290.00
10.250	290.00	290.00	290.00	290.00	290.00
10.500	290.00	290.00	290.00	290.00	290.00
10.750	290.00	290.00	290.00	290.00	290.00
11.000	290.00	290.00	290.00	290.00	290.00
11.250	290.00	290.00	290.00	290.00	290.00
11.500	290.00	290.00	290.00	290.00	290.00
11.750	290.00	290.01	290.02	290.05	290.09
12.000	290.16	290.27	290.42	290.58	290.72
12.250	290.84	290.94	291.03	291.11	291.17
12.500	291.23	291.27	291.31	291.34	291.37
12.750	291.39	291.42	291.44	291.46	291.48
13.000	291.50	291.52	291.53	291.55	291.56
13.250	291.58	291.59	291.60	291.62	291.63
13.500	291.64	291.65	291.66	291.68	291.69
13.750	291.70	291.71	291.72	291.73	291.74
14.000	291.75	291.76	291.77	291.77	291.78
14.250	291.79	291.80	291.80	291.81	291.82
14.500	291.83	291.83	291.84	291.85	291.85
14.750	291.86	291.87	291.87	291.88	291.88
15.000	291.89	291.89	291.90	291.90	291.91
15.250	291.91	291.92	291.92	291.93	291.93
15.500	291.93	291.94	291.94	291.94	291.95
15.750	291.95	291.95	291.95	291.95	291.96
16.000	291.96	291.96	291.96	291.96	291.96
16.250	291.96	291.97	291.97	291.97	291.97
16.500	291.97	291.97	291.97	291.97	291.97
16.750	291.97	291.97	291.97	291.97	291.97
17.000	291.97	291.97	291.97	291.97	291.97
17.250	291.97	291.96	291.96	291.96	291.96
17.500	291.96	291.96	291.96	291.96	291.95
17.750	291.95	291.95	291.95	291.95	291.95
18.000	291.94	291.94	291.94	291.94	291.93
18.250	291.93	291.93	291.93	291.92	291.92
18.500	291.92	291.92	291.91	291.91	291.91
18.750	291.91	291.90	291.90	291.90	291.90
19.000	291.89	291.89	291.89	291.89	291.88
19.250	291.88	291.88	291.87	291.87	291.87
19.500	291.87	291.86	291.86	291.86	291.85
19.750	291.85	291.85	291.85	291.84	291.84
20.000	291.84	291.83	291.83	291.83	291.82

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	291.82	291.82	291.81	291.81	291.81
20.500	291.80	291.80	291.80	291.79	291.79
20.750	291.79	291.78	291.78	291.78	291.77
21.000	291.77	291.77	291.76	291.76	291.76
21.250	291.75	291.75	291.74	291.74	291.74
21.500	291.73	291.73	291.73	291.72	291.72
21.750	291.71	291.71	291.71	291.70	291.70
22.000	291.70	291.69	291.69	291.68	291.68
22.250	291.68	291.67	291.67	291.66	291.66
22.500	291.66	291.65	291.65	291.64	291.64
22.750	291.64	291.63	291.63	291.62	291.62
23.000	291.62	291.61	291.61	291.60	291.60
23.250	291.60	291.59	291.59	291.58	291.58
23.500	291.57	291.57	291.57	291.56	291.56
23.750	291.55	291.55	291.54	291.54	291.53
24.000	291.53	291.52	291.52	291.51	291.50
24.250	291.50	291.49	291.48	291.47	291.46
24.500	291.45	291.44	291.44	291.43	291.42
24.750	291.41	291.40	291.39	291.38	291.38
25.000	291.37	291.36	291.35	291.34	291.33
25.250	291.32	291.32	291.31	291.30	291.29
25.500	291.28	291.27	291.27	291.26	291.25
25.750	291.24	291.23	291.22	291.22	291.21
26.000	291.20	291.19	291.18	291.17	291.17
26.250	291.16	291.15	291.14	291.13	291.13
26.500	291.12	291.11	291.10	291.09	291.08
26.750	291.08	291.07	291.06	291.05	291.04
27.000	291.04	291.03	291.02	291.01	291.00
27.250	291.00	290.99	290.98	290.97	290.96
27.500	290.95	290.94	290.94	290.93	290.92
27.750	290.91	290.90	290.89	290.88	290.88
28.000	290.87	290.86	290.85	290.84	290.83
28.250	290.82	290.82	290.81	290.80	290.79
28.500	290.78	290.77	290.77	290.76	290.75
28.750	290.74	290.73	290.72	290.72	290.71
29.000	290.70	290.69	290.68	290.67	290.67
29.250	290.66	290.65	290.64	290.63	290.63
29.500	290.62	290.61	290.60	290.59	290.58
29.750	290.58	290.57	290.56	290.55	290.54
30.000	290.54	290.53	290.52	290.51	290.50
30.250	290.50	290.49	290.48	290.47	290.46

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	290.45	290.45	290.44	290.43	290.42
30.750	290.42	290.41	290.40	290.40	290.39
31.000	290.38	290.38	290.37	290.36	290.36
31.250	290.35	290.34	290.34	290.33	290.33
31.500	290.32	290.32	290.31	290.31	290.30
31.750	290.30	290.29	290.28	290.28	290.28
32.000	290.27	290.27	290.26	290.26	290.25
32.250	290.25	290.24	290.24	290.24	290.23
32.500	290.23	290.22	290.22	290.22	290.21
32.750	290.21	290.21	290.20	290.20	290.19
33.000	290.19	290.19	290.18	290.18	290.18
33.250	290.18	290.17	290.17	290.17	290.16
33.500	290.16	290.16	290.16	290.15	290.15
33.750	290.15	290.15	290.14	290.14	290.14
34.000	290.14	290.13	290.13	290.13	290.13
34.250	290.12	290.12	290.12	290.12	290.12
34.500	290.11	290.11	290.11	290.11	290.11
34.750	290.10	290.10	290.10	290.10	290.10
35.000	290.10	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	290.00	290.00	290.00	290.00	290.00
0.250	290.00	290.00	290.00	290.00	290.00
0.500	290.00	290.00	290.00	290.00	290.00
0.750	290.00	290.00	290.00	290.00	290.00
1.000	290.00	290.00	290.00	290.00	290.00
1.250	290.00	290.00	290.00	290.00	290.00
1.500	290.00	290.00	290.00	290.00	290.00
1.750	290.00	290.00	290.00	290.00	290.00
2.000	290.00	290.00	290.00	290.00	290.00
2.250	290.00	290.00	290.00	290.00	290.00
2.500	290.00	290.00	290.00	290.00	290.00
2.750	290.00	290.00	290.00	290.00	290.00
3.000	290.00	290.00	290.00	290.00	290.00
3.250	290.00	290.00	290.00	290.00	290.00
3.500	290.00	290.00	290.00	290.00	290.00
3.750	290.00	290.00	290.00	290.00	290.00
4.000	290.00	290.00	290.00	290.00	290.00
4.250	290.00	290.00	290.00	290.00	290.00
4.500	290.00	290.00	290.00	290.00	290.00
4.750	290.00	290.00	290.00	290.00	290.00
5.000	290.00	290.00	290.00	290.00	290.00
5.250	290.00	290.00	290.00	290.00	290.00
5.500	290.00	290.00	290.00	290.00	290.00
5.750	290.00	290.00	290.00	290.00	290.00
6.000	290.00	290.00	290.00	290.00	290.00
6.250	290.00	290.00	290.00	290.00	290.00
6.500	290.00	290.00	290.00	290.00	290.00
6.750	290.00	290.00	290.00	290.00	290.00
7.000	290.00	290.00	290.00	290.00	290.00
7.250	290.00	290.00	290.00	290.00	290.00
7.500	290.00	290.00	290.00	290.00	290.00
7.750	290.00	290.00	290.00	290.00	290.00
8.000	290.00	290.00	290.00	290.00	290.00
8.250	290.00	290.00	290.00	290.00	290.00
8.500	290.00	290.00	290.00	290.00	290.00
8.750	290.00	290.00	290.00	290.00	290.00
9.000	290.00	290.00	290.00	290.00	290.00
9.250	290.00	290.00	290.00	290.00	290.00
9.500	290.00	290.00	290.00	290.00	290.00
9.750	290.00	290.00	290.00	290.00	290.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	290.00	290.00	290.00	290.00	290.00
10.250	290.00	290.00	290.00	290.00	290.00
10.500	290.00	290.00	290.00	290.00	290.00
10.750	290.00	290.00	290.00	290.00	290.00
11.000	290.00	290.00	290.00	290.00	290.00
11.250	290.00	290.00	290.00	290.00	290.01
11.500	290.01	290.02	290.03	290.04	290.06
11.750	290.08	290.12	290.17	290.24	290.33
12.000	290.48	290.69	290.94	291.19	291.42
12.250	291.60	291.75	291.88	291.99	292.09
12.500	292.17	292.23	292.29	292.33	292.37
12.750	292.40	292.43	292.46	292.48	292.50
13.000	292.52	292.53	292.54	292.55	292.56
13.250	292.56	292.57	292.57	292.57	292.57
13.500	292.57	292.57	292.57	292.57	292.57
13.750	292.57	292.57	292.57	292.57	292.56
14.000	292.56	292.56	292.56	292.56	292.55
14.250	292.55	292.55	292.55	292.55	292.54
14.500	292.54	292.54	292.54	292.54	292.54
14.750	292.53	292.53	292.53	292.53	292.53
15.000	292.53	292.53	292.52	292.52	292.52
15.250	292.52	292.52	292.52	292.51	292.51
15.500	292.51	292.51	292.51	292.51	292.51
15.750	292.50	292.50	292.50	292.50	292.50
16.000	292.49	292.49	292.49	292.49	292.48
16.250	292.48	292.48	292.48	292.47	292.47
16.500	292.47	292.47	292.46	292.46	292.46
16.750	292.46	292.45	292.45	292.45	292.44
17.000	292.44	292.44	292.44	292.43	292.43
17.250	292.43	292.42	292.42	292.42	292.42
17.500	292.41	292.41	292.41	292.40	292.40
17.750	292.40	292.40	292.39	292.39	292.39
18.000	292.38	292.38	292.38	292.38	292.37
18.250	292.37	292.37	292.36	292.36	292.36
18.500	292.36	292.35	292.35	292.35	292.35
18.750	292.35	292.34	292.34	292.34	292.34
19.000	292.34	292.33	292.33	292.33	292.33
19.250	292.33	292.32	292.32	292.32	292.32
19.500	292.32	292.32	292.31	292.31	292.31
19.750	292.31	292.31	292.31	292.31	292.30
20.000	292.30	292.30	292.30	292.30	292.30

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	292.30	292.29	292.29	292.29	292.29
20.500	292.29	292.29	292.28	292.28	292.28
20.750	292.28	292.28	292.28	292.27	292.27
21.000	292.27	292.27	292.27	292.27	292.26
21.250	292.26	292.26	292.26	292.26	292.25
21.500	292.25	292.25	292.25	292.24	292.24
21.750	292.24	292.24	292.24	292.23	292.23
22.000	292.23	292.23	292.22	292.22	292.22
22.250	292.22	292.21	292.21	292.21	292.21
22.500	292.20	292.20	292.20	292.20	292.19
22.750	292.19	292.19	292.18	292.18	292.18
23.000	292.18	292.17	292.17	292.17	292.16
23.250	292.16	292.16	292.15	292.15	292.15
23.500	292.14	292.14	292.14	292.14	292.13
23.750	292.13	292.13	292.12	292.12	292.12
24.000	292.11	292.11	292.10	292.09	292.09
24.250	292.08	292.07	292.06	292.05	292.05
24.500	292.04	292.03	292.02	292.01	292.00
24.750	292.00	291.99	291.98	291.97	291.96
25.000	291.95	291.94	291.94	291.93	291.92
25.250	291.91	291.90	291.89	291.89	291.88
25.500	291.87	291.86	291.85	291.84	291.83
25.750	291.83	291.82	291.81	291.80	291.79
26.000	291.78	291.78	291.77	291.76	291.75
26.250	291.74	291.73	291.73	291.72	291.71
26.500	291.70	291.69	291.68	291.68	291.67
26.750	291.66	291.65	291.64	291.63	291.63
27.000	291.62	291.61	291.60	291.59	291.59
27.250	291.58	291.57	291.56	291.55	291.55
27.500	291.54	291.53	291.52	291.51	291.50
27.750	291.50	291.49	291.48	291.47	291.46
28.000	291.45	291.44	291.44	291.43	291.42
28.250	291.41	291.40	291.39	291.39	291.38
28.500	291.37	291.36	291.35	291.34	291.33
28.750	291.33	291.32	291.31	291.30	291.29
29.000	291.28	291.28	291.27	291.26	291.25
29.250	291.24	291.23	291.23	291.22	291.21
29.500	291.20	291.19	291.18	291.18	291.17
29.750	291.16	291.15	291.14	291.13	291.13
30.000	291.12	291.11	291.10	291.09	291.09
30.250	291.08	291.07	291.06	291.05	291.05

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	291.04	291.03	291.02	291.01	291.00
30.750	291.00	290.99	290.98	290.97	290.96
31.000	290.95	290.94	290.94	290.93	290.92
31.250	290.91	290.90	290.89	290.89	290.88
31.500	290.87	290.86	290.85	290.84	290.83
31.750	290.83	290.82	290.81	290.80	290.79
32.000	290.78	290.78	290.77	290.76	290.75
32.250	290.74	290.73	290.73	290.72	290.71
32.500	290.70	290.69	290.68	290.68	290.67
32.750	290.66	290.65	290.64	290.63	290.63
33.000	290.62	290.61	290.60	290.59	290.59
33.250	290.58	290.57	290.56	290.55	290.55
33.500	290.54	290.53	290.52	290.51	290.50
33.750	290.50	290.49	290.48	290.47	290.46
34.000	290.46	290.45	290.44	290.43	290.43
34.250	290.42	290.41	290.40	290.40	290.39
34.500	290.38	290.38	290.37	290.36	290.36
34.750	290.35	290.35	290.34	290.33	290.33
35.000	290.32	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	290.00	290.00	290.00	290.00	290.00
0.250	290.00	290.00	290.00	290.00	290.00
0.500	290.00	290.00	290.00	290.00	290.00
0.750	290.00	290.00	290.00	290.00	290.00
1.000	290.00	290.00	290.00	290.00	290.00
1.250	290.00	290.00	290.00	290.00	290.00
1.500	290.00	290.00	290.00	290.00	290.00
1.750	290.00	290.00	290.00	290.00	290.00
2.000	290.00	290.00	290.00	290.00	290.00
2.250	290.00	290.00	290.00	290.00	290.00
2.500	290.00	290.00	290.00	290.00	290.00
2.750	290.00	290.00	290.00	290.00	290.00
3.000	290.00	290.00	290.00	290.00	290.00
3.250	290.00	290.00	290.00	290.00	290.00
3.500	290.00	290.00	290.00	290.00	290.00
3.750	290.00	290.00	290.00	290.00	290.00
4.000	290.00	290.00	290.00	290.00	290.00
4.250	290.00	290.00	290.00	290.00	290.00
4.500	290.00	290.00	290.00	290.00	290.00
4.750	290.00	290.00	290.00	290.00	290.00
5.000	290.00	290.00	290.00	290.00	290.00
5.250	290.00	290.00	290.00	290.00	290.00
5.500	290.00	290.00	290.00	290.00	290.00
5.750	290.00	290.00	290.00	290.00	290.00
6.000	290.00	290.00	290.00	290.00	290.00
6.250	290.00	290.00	290.00	290.00	290.00
6.500	290.00	290.00	290.00	290.00	290.00
6.750	290.00	290.00	290.00	290.00	290.00
7.000	290.00	290.00	290.00	290.00	290.00
7.250	290.00	290.00	290.00	290.00	290.00
7.500	290.00	290.00	290.00	290.00	290.00
7.750	290.00	290.00	290.00	290.00	290.00
8.000	290.00	290.00	290.00	290.00	290.00
8.250	290.00	290.00	290.00	290.00	290.00
8.500	290.00	290.00	290.00	290.00	290.00
8.750	290.00	290.00	290.00	290.00	290.00
9.000	290.00	290.00	290.00	290.00	290.00
9.250	290.00	290.00	290.00	290.00	290.00
9.500	290.00	290.00	290.00	290.00	290.00
9.750	290.00	290.00	290.00	290.00	290.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	290.00	290.00	290.01	290.01	290.01
10.250	290.02	290.02	290.03	290.03	290.04
10.500	290.05	290.05	290.06	290.07	290.08
10.750	290.09	290.10	290.11	290.13	290.14
11.000	290.16	290.17	290.19	290.21	290.23
11.250	290.25	290.27	290.30	290.33	290.36
11.500	290.40	290.44	290.49	290.55	290.62
11.750	290.73	290.86	291.02	291.21	291.46
12.000	291.81	292.26	292.73	293.14	293.39
12.250	293.51	293.56	293.56	293.54	293.50
12.500	293.45	293.38	293.31	293.24	293.19
12.750	293.14	293.10	293.06	293.03	293.01
13.000	292.99	292.96	292.94	292.92	292.90
13.250	292.89	292.87	292.86	292.84	292.83
13.500	292.82	292.81	292.80	292.79	292.78
13.750	292.77	292.77	292.76	292.75	292.74
14.000	292.74	292.73	292.72	292.72	292.71
14.250	292.70	292.70	292.69	292.69	292.69
14.500	292.68	292.68	292.67	292.67	292.67
14.750	292.66	292.66	292.66	292.65	292.65
15.000	292.65	292.64	292.64	292.64	292.63
15.250	292.63	292.63	292.62	292.62	292.62
15.500	292.61	292.61	292.61	292.60	292.60
15.750	292.60	292.59	292.59	292.59	292.58
16.000	292.58	292.58	292.58	292.57	292.57
16.250	292.57	292.56	292.56	292.56	292.56
16.500	292.56	292.55	292.55	292.55	292.55
16.750	292.55	292.55	292.54	292.54	292.54
17.000	292.54	292.54	292.54	292.53	292.53
17.250	292.53	292.53	292.53	292.53	292.53
17.500	292.52	292.52	292.52	292.52	292.52
17.750	292.52	292.52	292.51	292.51	292.51
18.000	292.51	292.51	292.51	292.51	292.50
18.250	292.50	292.50	292.50	292.50	292.50
18.500	292.50	292.50	292.50	292.50	292.49
18.750	292.49	292.49	292.49	292.49	292.49
19.000	292.49	292.49	292.49	292.48	292.48
19.250	292.48	292.48	292.48	292.48	292.48
19.500	292.48	292.47	292.47	292.47	292.47
19.750	292.47	292.47	292.47	292.46	292.46
20.000	292.46	292.46	292.46	292.46	292.45

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	292.45	292.45	292.45	292.45	292.45
20.500	292.45	292.44	292.44	292.44	292.44
20.750	292.44	292.44	292.44	292.43	292.43
21.000	292.43	292.43	292.43	292.43	292.43
21.250	292.42	292.42	292.42	292.42	292.42
21.500	292.42	292.42	292.41	292.41	292.41
21.750	292.41	292.41	292.41	292.41	292.40
22.000	292.40	292.40	292.40	292.40	292.40
22.250	292.40	292.39	292.39	292.39	292.39
22.500	292.39	292.39	292.39	292.38	292.38
22.750	292.38	292.38	292.38	292.38	292.38
23.000	292.37	292.37	292.37	292.37	292.37
23.250	292.37	292.37	292.37	292.36	292.36
23.500	292.36	292.36	292.36	292.36	292.36
23.750	292.35	292.35	292.35	292.35	292.35
24.000	292.35	292.34	292.34	292.33	292.32
24.250	292.31	292.31	292.30	292.29	292.28
24.500	292.27	292.26	292.25	292.25	292.24
24.750	292.23	292.22	292.21	292.20	292.20
25.000	292.19	292.18	292.17	292.16	292.15
25.250	292.15	292.14	292.13	292.12	292.11
25.500	292.10	292.10	292.09	292.08	292.07
25.750	292.06	292.05	292.05	292.04	292.03
26.000	292.02	292.01	292.01	292.00	291.99
26.250	291.98	291.97	291.96	291.95	291.95
26.500	291.94	291.93	291.92	291.91	291.90
26.750	291.89	291.89	291.88	291.87	291.86
27.000	291.85	291.84	291.84	291.83	291.82
27.250	291.81	291.80	291.79	291.78	291.78
27.500	291.77	291.76	291.75	291.74	291.73
27.750	291.73	291.72	291.71	291.70	291.69
28.000	291.69	291.68	291.67	291.66	291.65
28.250	291.64	291.64	291.63	291.62	291.61
28.500	291.60	291.59	291.59	291.58	291.57
28.750	291.56	291.55	291.55	291.54	291.53
29.000	291.52	291.51	291.51	291.50	291.49
29.250	291.48	291.47	291.46	291.45	291.45
29.500	291.44	291.43	291.42	291.41	291.40
29.750	291.39	291.39	291.38	291.37	291.36
30.000	291.35	291.34	291.34	291.33	291.32
30.250	291.31	291.30	291.29	291.28	291.28

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	291.27	291.26	291.25	291.24	291.23
30.750	291.23	291.22	291.21	291.20	291.19
31.000	291.19	291.18	291.17	291.16	291.15
31.250	291.14	291.14	291.13	291.12	291.11
31.500	291.10	291.09	291.09	291.08	291.07
31.750	291.06	291.05	291.05	291.04	291.03
32.000	291.02	291.01	291.01	291.00	290.99
32.250	290.98	290.97	290.96	290.95	290.95
32.500	290.94	290.93	290.92	290.91	290.90
32.750	290.89	290.89	290.88	290.87	290.86
33.000	290.85	290.84	290.84	290.83	290.82
33.250	290.81	290.80	290.79	290.78	290.78
33.500	290.77	290.76	290.75	290.74	290.73
33.750	290.73	290.72	290.71	290.70	290.69
34.000	290.68	290.68	290.67	290.66	290.65
34.250	290.64	290.64	290.63	290.62	290.61
34.500	290.60	290.59	290.59	290.58	290.57
34.750	290.56	290.55	290.55	290.54	290.53
35.000	290.52	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	290.00	290.00	290.00	290.00	290.00
0.250	290.00	290.00	290.00	290.00	290.00
0.500	290.00	290.00	290.00	290.00	290.00
0.750	290.00	290.00	290.00	290.00	290.00
1.000	290.00	290.00	290.00	290.00	290.00
1.250	290.00	290.00	290.00	290.00	290.00
1.500	290.00	290.00	290.00	290.00	290.00
1.750	290.00	290.00	290.00	290.00	290.00
2.000	290.00	290.00	290.00	290.00	290.00
2.250	290.00	290.00	290.00	290.00	290.00
2.500	290.00	290.00	290.00	290.00	290.00
2.750	290.00	290.00	290.00	290.00	290.00
3.000	290.00	290.00	290.00	290.00	290.00
3.250	290.00	290.00	290.00	290.00	290.00
3.500	290.00	290.00	290.00	290.00	290.00
3.750	290.00	290.00	290.00	290.00	290.00
4.000	290.00	290.00	290.00	290.00	290.00
4.250	290.00	290.00	290.00	290.00	290.00
4.500	290.00	290.00	290.00	290.00	290.00
4.750	290.00	290.00	290.00	290.00	290.00
5.000	290.00	290.00	290.00	290.00	290.00
5.250	290.00	290.00	290.00	290.00	290.00
5.500	290.00	290.00	290.00	290.00	290.00
5.750	290.00	290.00	290.00	290.00	290.00
6.000	290.00	290.00	290.00	290.00	290.00
6.250	290.00	290.00	290.00	290.00	290.00
6.500	290.00	290.00	290.00	290.00	290.00
6.750	290.00	290.00	290.00	290.00	290.00
7.000	290.00	290.00	290.00	290.00	290.00
7.250	290.00	290.00	290.00	290.00	290.00
7.500	290.00	290.00	290.00	290.00	290.00
7.750	290.00	290.00	290.00	290.00	290.00
8.000	290.00	290.00	290.00	290.00	290.00
8.250	290.00	290.00	290.00	290.00	290.00
8.500	290.00	290.00	290.00	290.00	290.00
8.750	290.00	290.00	290.00	290.00	290.00
9.000	290.00	290.00	290.00	290.00	290.01
9.250	290.01	290.01	290.01	290.02	290.02
9.500	290.03	290.03	290.04	290.05	290.05
9.750	290.06	290.07	290.08	290.09	290.10

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	290.11	290.12	290.13	290.14	290.16
10.250	290.17	290.18	290.20	290.22	290.23
10.500	290.25	290.27	290.29	290.32	290.34
10.750	290.36	290.39	290.41	290.44	290.47
11.000	290.50	290.53	290.56	290.60	290.63
11.250	290.67	290.72	290.77	290.82	290.88
11.500	290.95	291.02	291.10	291.20	291.34
11.750	291.51	291.71	291.96	292.24	292.58
12.000	293.01	293.47	293.86	294.14	294.26
12.250	294.25	294.19	294.10	294.01	293.90
12.500	293.79	293.68	293.57	293.48	293.40
12.750	293.33	293.27	293.22	293.18	293.15
13.000	293.12	293.09	293.07	293.04	293.03
13.250	293.01	293.00	292.99	292.98	292.97
13.500	292.96	292.95	292.94	292.93	292.92
13.750	292.91	292.90	292.89	292.88	292.87
14.000	292.86	292.85	292.85	292.84	292.83
14.250	292.82	292.82	292.81	292.80	292.80
14.500	292.79	292.79	292.78	292.78	292.77
14.750	292.77	292.76	292.76	292.75	292.75
15.000	292.74	292.74	292.73	292.73	292.72
15.250	292.72	292.71	292.71	292.71	292.70
15.500	292.70	292.69	292.69	292.68	292.68
15.750	292.67	292.67	292.67	292.66	292.66
16.000	292.65	292.65	292.64	292.64	292.63
16.250	292.63	292.63	292.62	292.62	292.62
16.500	292.62	292.61	292.61	292.61	292.61
16.750	292.60	292.60	292.60	292.60	292.59
17.000	292.59	292.59	292.59	292.59	292.58
17.250	292.58	292.58	292.58	292.58	292.57
17.500	292.57	292.57	292.57	292.57	292.56
17.750	292.56	292.56	292.56	292.55	292.55
18.000	292.55	292.55	292.55	292.54	292.54
18.250	292.54	292.54	292.54	292.54	292.54
18.500	292.54	292.53	292.53	292.53	292.53
18.750	292.53	292.53	292.53	292.53	292.53
19.000	292.53	292.53	292.53	292.53	292.52
19.250	292.52	292.52	292.52	292.52	292.52
19.500	292.52	292.52	292.52	292.52	292.52
19.750	292.52	292.52	292.52	292.52	292.52
20.000	292.52	292.51	292.51	292.51	292.51

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	292.51	292.51	292.51	292.51	292.51
20.500	292.51	292.51	292.51	292.51	292.51
20.750	292.51	292.51	292.51	292.51	292.51
21.000	292.50	292.50	292.50	292.50	292.50
21.250	292.50	292.50	292.50	292.50	292.50
21.500	292.50	292.50	292.50	292.50	292.50
21.750	292.50	292.50	292.50	292.49	292.49
22.000	292.49	292.49	292.49	292.49	292.49
22.250	292.49	292.49	292.49	292.48	292.48
22.500	292.48	292.48	292.48	292.48	292.48
22.750	292.48	292.47	292.47	292.47	292.47
23.000	292.47	292.47	292.47	292.46	292.46
23.250	292.46	292.46	292.46	292.46	292.45
23.500	292.45	292.45	292.45	292.45	292.45
23.750	292.45	292.44	292.44	292.44	292.44
24.000	292.44	292.43	292.43	292.42	292.40
24.250	292.39	292.38	292.37	292.36	292.35
24.500	292.34	292.33	292.32	292.31	292.30
24.750	292.29	292.29	292.28	292.27	292.26
25.000	292.25	292.24	292.24	292.23	292.22
25.250	292.21	292.20	292.19	292.18	292.18
25.500	292.17	292.16	292.15	292.14	292.13
25.750	292.13	292.12	292.11	292.10	292.09
26.000	292.09	292.08	292.07	292.06	292.05
26.250	292.04	292.04	292.03	292.02	292.01
26.500	292.00	291.99	291.99	291.98	291.97
26.750	291.96	291.95	291.94	291.93	291.93
27.000	291.92	291.91	291.90	291.89	291.88
27.250	291.87	291.87	291.86	291.85	291.84
27.500	291.83	291.82	291.82	291.81	291.80
27.750	291.79	291.78	291.77	291.77	291.76
28.000	291.75	291.74	291.73	291.72	291.72
28.250	291.71	291.70	291.69	291.68	291.67
28.500	291.67	291.66	291.65	291.64	291.63
28.750	291.62	291.62	291.61	291.60	291.59
29.000	291.58	291.58	291.57	291.56	291.55
29.250	291.54	291.54	291.53	291.52	291.51
29.500	291.50	291.49	291.49	291.48	291.47
29.750	291.46	291.45	291.44	291.43	291.43
30.000	291.42	291.41	291.40	291.39	291.38
30.250	291.37	291.37	291.36	291.35	291.34

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	291.33	291.32	291.32	291.31	291.30
30.750	291.29	291.28	291.27	291.27	291.26
31.000	291.25	291.24	291.23	291.22	291.22
31.250	291.21	291.20	291.19	291.18	291.17
31.500	291.17	291.16	291.15	291.14	291.13
31.750	291.12	291.12	291.11	291.10	291.09
32.000	291.08	291.08	291.07	291.06	291.05
32.250	291.04	291.04	291.03	291.02	291.01
32.500	291.00	290.99	290.99	290.98	290.97
32.750	290.96	290.95	290.94	290.93	290.93
33.000	290.92	290.91	290.90	290.89	290.88
33.250	290.87	290.87	290.86	290.85	290.84
33.500	290.83	290.82	290.82	290.81	290.80
33.750	290.79	290.78	290.77	290.77	290.76
34.000	290.75	290.74	290.73	290.72	290.72
34.250	290.71	290.70	290.69	290.68	290.67
34.500	290.67	290.66	290.65	290.64	290.63
34.750	290.62	290.62	290.61	290.60	290.59
35.000	290.58	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	290.00	290.00	290.00	290.00	290.00
0.250	290.00	290.00	290.00	290.00	290.00
0.500	290.00	290.00	290.00	290.00	290.00
0.750	290.00	290.00	290.00	290.00	290.00
1.000	290.00	290.00	290.00	290.00	290.00
1.250	290.00	290.00	290.00	290.00	290.00
1.500	290.00	290.00	290.00	290.00	290.00
1.750	290.00	290.00	290.00	290.00	290.00
2.000	290.00	290.00	290.00	290.00	290.00
2.250	290.00	290.00	290.00	290.00	290.00
2.500	290.00	290.00	290.00	290.00	290.00
2.750	290.00	290.00	290.00	290.00	290.00
3.000	290.00	290.00	290.00	290.00	290.00
3.250	290.00	290.00	290.00	290.00	290.00
3.500	290.00	290.00	290.00	290.00	290.00
3.750	290.00	290.00	290.00	290.00	290.00
4.000	290.00	290.00	290.00	290.00	290.00
4.250	290.00	290.00	290.00	290.00	290.00
4.500	290.00	290.00	290.00	290.00	290.00
4.750	290.00	290.00	290.00	290.00	290.00
5.000	290.00	290.00	290.00	290.00	290.00
5.250	290.00	290.00	290.00	290.00	290.00
5.500	290.00	290.00	290.00	290.00	290.00
5.750	290.00	290.00	290.00	290.00	290.00
6.000	290.00	290.00	290.00	290.00	290.00
6.250	290.00	290.00	290.00	290.00	290.00
6.500	290.00	290.00	290.00	290.00	290.00
6.750	290.00	290.00	290.00	290.00	290.00
7.000	290.00	290.00	290.00	290.00	290.00
7.250	290.00	290.00	290.00	290.00	290.00
7.500	290.00	290.00	290.00	290.00	290.00
7.750	290.00	290.01	290.01	290.01	290.01
8.000	290.02	290.02	290.02	290.03	290.03
8.250	290.04	290.04	290.05	290.06	290.07
8.500	290.07	290.08	290.09	290.10	290.11
8.750	290.12	290.13	290.15	290.16	290.17
9.000	290.19	290.20	290.22	290.23	290.25
9.250	290.27	290.29	290.31	290.33	290.35
9.500	290.37	290.39	290.42	290.44	290.47
9.750	290.49	290.52	290.54	290.57	290.60

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	290.63	290.66	290.69	290.73	290.77
10.250	290.80	290.84	290.89	290.93	290.98
10.500	291.03	291.07	291.12	291.18	291.23
10.750	291.29	291.35	291.41	291.47	291.54
11.000	291.60	291.67	291.74	291.81	291.90
11.250	291.98	292.07	292.17	292.28	292.38
11.500	292.49	292.60	292.71	292.84	293.00
11.750	293.17	293.35	293.54	293.73	293.97
12.000	294.31	294.75	295.21	295.59	295.77
12.250	295.77	295.68	295.54	295.37	295.16
12.500	294.92	294.65	294.37	294.13	293.93
12.750	293.78	293.66	293.57	293.50	293.44
13.000	293.39	293.34	293.30	293.27	293.24
13.250	293.22	293.19	293.18	293.16	293.15
13.500	293.14	293.13	293.12	293.11	293.10
13.750	293.09	293.08	293.07	293.07	293.06
14.000	293.05	293.04	293.04	293.03	293.02
14.250	293.02	293.01	293.01	293.00	293.00
14.500	292.99	292.99	292.98	292.98	292.97
14.750	292.96	292.96	292.95	292.95	292.94
15.000	292.93	292.93	292.92	292.91	292.91
15.250	292.90	292.89	292.89	292.88	292.87
15.500	292.86	292.86	292.85	292.84	292.84
15.750	292.83	292.82	292.82	292.81	292.80
16.000	292.79	292.79	292.78	292.77	292.77
16.250	292.76	292.76	292.75	292.75	292.74
16.500	292.74	292.73	292.73	292.72	292.72
16.750	292.72	292.71	292.71	292.71	292.70
17.000	292.70	292.70	292.69	292.69	292.69
17.250	292.68	292.68	292.68	292.67	292.67
17.500	292.67	292.66	292.66	292.66	292.65
17.750	292.65	292.65	292.64	292.64	292.64
18.000	292.63	292.63	292.63	292.63	292.62
18.250	292.62	292.62	292.62	292.61	292.61
18.500	292.61	292.61	292.61	292.61	292.61
18.750	292.60	292.60	292.60	292.60	292.60
19.000	292.60	292.60	292.60	292.59	292.59
19.250	292.59	292.59	292.59	292.59	292.59
19.500	292.59	292.59	292.59	292.58	292.58
19.750	292.58	292.58	292.58	292.58	292.58
20.000	292.58	292.58	292.58	292.58	292.57

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	292.57	292.57	292.57	292.57	292.57
20.500	292.57	292.57	292.57	292.57	292.57
20.750	292.57	292.56	292.56	292.56	292.56
21.000	292.56	292.56	292.56	292.56	292.56
21.250	292.56	292.56	292.56	292.56	292.56
21.500	292.55	292.55	292.55	292.55	292.55
21.750	292.55	292.55	292.55	292.55	292.55
22.000	292.55	292.55	292.55	292.55	292.54
22.250	292.54	292.54	292.54	292.54	292.54
22.500	292.54	292.54	292.54	292.54	292.54
22.750	292.54	292.54	292.54	292.53	292.53
23.000	292.53	292.53	292.53	292.53	292.53
23.250	292.53	292.53	292.53	292.53	292.53
23.500	292.53	292.53	292.52	292.52	292.52
23.750	292.52	292.52	292.52	292.52	292.52
24.000	292.52	292.52	292.51	292.50	292.48
24.250	292.47	292.45	292.44	292.43	292.41
24.500	292.40	292.39	292.38	292.37	292.36
24.750	292.35	292.34	292.33	292.32	292.31
25.000	292.30	292.29	292.28	292.27	292.27
25.250	292.26	292.25	292.24	292.23	292.22
25.500	292.22	292.21	292.20	292.19	292.18
25.750	292.17	292.17	292.16	292.15	292.14
26.000	292.13	292.12	292.12	292.11	292.10
26.250	292.09	292.08	292.07	292.07	292.06
26.500	292.05	292.04	292.03	292.02	292.02
26.750	292.01	292.00	291.99	291.98	291.97
27.000	291.97	291.96	291.95	291.94	291.93
27.250	291.92	291.91	291.91	291.90	291.89
27.500	291.88	291.87	291.86	291.85	291.85
27.750	291.84	291.83	291.82	291.81	291.80
28.000	291.80	291.79	291.78	291.77	291.76
28.250	291.75	291.75	291.74	291.73	291.72
28.500	291.71	291.70	291.70	291.69	291.68
28.750	291.67	291.66	291.65	291.65	291.64
29.000	291.63	291.62	291.61	291.61	291.60
29.250	291.59	291.58	291.57	291.56	291.56
29.500	291.55	291.54	291.53	291.52	291.52
29.750	291.51	291.50	291.49	291.48	291.47
30.000	291.47	291.46	291.45	291.44	291.43
30.250	291.42	291.41	291.41	291.40	291.39

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	291.38	291.37	291.36	291.35	291.35
30.750	291.34	291.33	291.32	291.31	291.30
31.000	291.30	291.29	291.28	291.27	291.26
31.250	291.25	291.25	291.24	291.23	291.22
31.500	291.21	291.20	291.20	291.19	291.18
31.750	291.17	291.16	291.15	291.15	291.14
32.000	291.13	291.12	291.11	291.11	291.10
32.250	291.09	291.08	291.07	291.06	291.06
32.500	291.05	291.04	291.03	291.02	291.02
32.750	291.01	291.00	290.99	290.98	290.97
33.000	290.97	290.96	290.95	290.94	290.93
33.250	290.92	290.91	290.91	290.90	290.89
33.500	290.88	290.87	290.86	290.85	290.85
33.750	290.84	290.83	290.82	290.81	290.80
34.000	290.80	290.79	290.78	290.77	290.76
34.250	290.75	290.75	290.74	290.73	290.72
34.500	290.71	290.70	290.70	290.69	290.68
34.750	290.67	290.66	290.65	290.65	290.64
35.000	290.63	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	300.00	300.00	300.00	300.00	300.00
0.250	300.00	300.00	300.00	300.00	300.00
0.500	300.00	300.00	300.00	300.00	300.00
0.750	300.00	300.00	300.00	300.00	300.00
1.000	300.00	300.00	300.00	300.00	300.00
1.250	300.00	300.00	300.00	300.00	300.00
1.500	300.00	300.00	300.00	300.00	300.00
1.750	300.00	300.00	300.00	300.00	300.00
2.000	300.00	300.00	300.00	300.00	300.00
2.250	300.00	300.00	300.00	300.00	300.00
2.500	300.00	300.00	300.00	300.00	300.00
2.750	300.00	300.00	300.00	300.00	300.00
3.000	300.00	300.00	300.00	300.00	300.00
3.250	300.00	300.00	300.00	300.00	300.00
3.500	300.00	300.00	300.00	300.00	300.00
3.750	300.00	300.00	300.00	300.00	300.00
4.000	300.00	300.00	300.00	300.00	300.00
4.250	300.00	300.00	300.00	300.00	300.00
4.500	300.00	300.00	300.00	300.00	300.00
4.750	300.00	300.00	300.00	300.00	300.00
5.000	300.00	300.00	300.00	300.00	300.00
5.250	300.00	300.00	300.00	300.00	300.00
5.500	300.00	300.00	300.00	300.00	300.00
5.750	300.00	300.00	300.00	300.00	300.00
6.000	300.00	300.00	300.00	300.00	300.00
6.250	300.00	300.00	300.00	300.00	300.00
6.500	300.00	300.00	300.00	300.00	300.00
6.750	300.00	300.00	300.00	300.00	300.00
7.000	300.00	300.00	300.00	300.00	300.00
7.250	300.00	300.00	300.00	300.00	300.00
7.500	300.00	300.00	300.00	300.00	300.00
7.750	300.00	300.00	300.00	300.00	300.00
8.000	300.00	300.00	300.00	300.00	300.00
8.250	300.00	300.00	300.00	300.00	300.00
8.500	300.00	300.00	300.00	300.00	300.00
8.750	300.00	300.00	300.00	300.00	300.00
9.000	300.00	300.00	300.00	300.00	300.00
9.250	300.00	300.00	300.00	300.00	300.00
9.500	300.00	300.00	300.00	300.00	300.00
9.750	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	300.00	300.00	300.00	300.00	300.00
10.250	300.00	300.00	300.00	300.00	300.00
10.500	300.00	300.00	300.00	300.00	300.00
10.750	300.00	300.00	300.00	300.00	300.00
11.000	300.00	300.00	300.00	300.00	300.00
11.250	300.00	300.00	300.00	300.00	300.00
11.500	300.00	300.00	300.00	300.00	300.00
11.750	300.01	300.02	300.03	300.06	300.09
12.000	300.15	300.24	300.36	300.47	300.55
12.250	300.61	300.64	300.67	300.68	300.68
12.500	300.67	300.65	300.62	300.58	300.54
12.750	300.49	300.45	300.41	300.38	300.35
13.000	300.33	300.30	300.28	300.26	300.25
13.250	300.23	300.22	300.21	300.20	300.20
13.500	300.19	300.18	300.18	300.17	300.17
13.750	300.16	300.16	300.16	300.15	300.15
14.000	300.15	300.14	300.14	300.14	300.13
14.250	300.13	300.13	300.13	300.13	300.12
14.500	300.12	300.12	300.12	300.12	300.12
14.750	300.12	300.11	300.11	300.11	300.11
15.000	300.11	300.11	300.11	300.10	300.10
15.250	300.10	300.10	300.10	300.10	300.10
15.500	300.10	300.09	300.09	300.09	300.09
15.750	300.09	300.09	300.09	300.08	300.08
16.000	300.08	300.08	300.08	300.08	300.08
16.250	300.08	300.07	300.07	300.07	300.07
16.500	300.07	300.07	300.07	300.07	300.07
16.750	300.07	300.07	300.07	300.06	300.06
17.000	300.06	300.06	300.06	300.06	300.06
17.250	300.06	300.06	300.06	300.06	300.06
17.500	300.06	300.06	300.06	300.06	300.05
17.750	300.05	300.05	300.05	300.05	300.05
18.000	300.05	300.05	300.05	300.05	300.05
18.250	300.05	300.05	300.05	300.05	300.05
18.500	300.05	300.05	300.05	300.04	300.04
18.750	300.04	300.04	300.04	300.04	300.04
19.000	300.04	300.04	300.04	300.04	300.04
19.250	300.04	300.04	300.04	300.04	300.04
19.500	300.04	300.04	300.04	300.04	300.04
19.750	300.04	300.04	300.04	300.04	300.04
20.000	300.04	300.04	300.04	300.04	300.04

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	300.04	300.04	300.04	300.04	300.04
20.500	300.04	300.04	300.04	300.04	300.04
20.750	300.04	300.04	300.04	300.04	300.04
21.000	300.04	300.04	300.04	300.04	300.04
21.250	300.04	300.04	300.04	300.03	300.03
21.500	300.03	300.03	300.03	300.03	300.03
21.750	300.03	300.03	300.03	300.03	300.03
22.000	300.03	300.03	300.03	300.03	300.03
22.250	300.03	300.03	300.03	300.03	300.03
22.500	300.03	300.03	300.03	300.03	300.03
22.750	300.03	300.03	300.03	300.03	300.03
23.000	300.03	300.03	300.03	300.03	300.03
23.250	300.03	300.03	300.03	300.03	300.03
23.500	300.03	300.03	300.03	300.03	300.03
23.750	300.03	300.03	300.03	300.03	300.03
24.000	300.03	300.03	300.02	300.02	300.02
24.250	300.02	300.01	300.01	300.01	300.01
24.500	300.01	300.01	300.00	300.00	300.00
24.750	300.00	300.00	300.00	300.00	300.00
25.000	300.00	300.00	300.00	300.00	300.00
25.250	300.00	300.00	300.00	300.00	300.00
25.500	300.00	300.00	300.00	300.00	300.00
25.750	300.00	300.00	300.00	300.00	300.00
26.000	300.00	300.00	300.00	300.00	300.00
26.250	300.00	300.00	300.00	300.00	300.00
26.500	300.00	300.00	300.00	300.00	300.00
26.750	300.00	300.00	300.00	300.00	300.00
27.000	300.00	300.00	300.00	300.00	300.00
27.250	300.00	300.00	300.00	300.00	300.00
27.500	300.00	300.00	300.00	300.00	300.00
27.750	300.00	300.00	300.00	300.00	300.00
28.000	300.00	300.00	300.00	300.00	300.00
28.250	300.00	300.00	300.00	300.00	300.00
28.500	300.00	300.00	300.00	300.00	300.00
28.750	300.00	300.00	300.00	300.00	300.00
29.000	300.00	300.00	300.00	300.00	300.00
29.250	300.00	300.00	300.00	300.00	300.00
29.500	300.00	300.00	300.00	300.00	300.00
29.750	300.00	300.00	300.00	300.00	300.00
30.000	300.00	300.00	300.00	300.00	300.00
30.250	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	300.00	300.00	300.00	300.00	300.00
30.750	300.00	300.00	300.00	300.00	300.00
31.000	300.00	300.00	300.00	300.00	300.00
31.250	300.00	300.00	300.00	300.00	300.00
31.500	300.00	300.00	300.00	300.00	300.00
31.750	300.00	300.00	300.00	300.00	300.00
32.000	300.00	300.00	300.00	300.00	300.00
32.250	300.00	300.00	300.00	300.00	300.00
32.500	300.00	300.00	300.00	300.00	300.00
32.750	300.00	300.00	300.00	300.00	300.00
33.000	300.00	300.00	300.00	300.00	300.00
33.250	300.00	300.00	300.00	300.00	300.00
33.500	300.00	300.00	300.00	300.00	300.00
33.750	300.00	300.00	300.00	300.00	300.00
34.000	300.00	300.00	300.00	300.00	300.00
34.250	300.00	300.00	300.00	300.00	300.00
34.500	300.00	300.00	300.00	300.00	300.00
34.750	300.00	300.00	300.00	300.00	300.00
35.000	300.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	300.00	300.00	300.00	300.00	300.00
0.250	300.00	300.00	300.00	300.00	300.00
0.500	300.00	300.00	300.00	300.00	300.00
0.750	300.00	300.00	300.00	300.00	300.00
1.000	300.00	300.00	300.00	300.00	300.00
1.250	300.00	300.00	300.00	300.00	300.00
1.500	300.00	300.00	300.00	300.00	300.00
1.750	300.00	300.00	300.00	300.00	300.00
2.000	300.00	300.00	300.00	300.00	300.00
2.250	300.00	300.00	300.00	300.00	300.00
2.500	300.00	300.00	300.00	300.00	300.00
2.750	300.00	300.00	300.00	300.00	300.00
3.000	300.00	300.00	300.00	300.00	300.00
3.250	300.00	300.00	300.00	300.00	300.00
3.500	300.00	300.00	300.00	300.00	300.00
3.750	300.00	300.00	300.00	300.00	300.00
4.000	300.00	300.00	300.00	300.00	300.00
4.250	300.00	300.00	300.00	300.00	300.00
4.500	300.00	300.00	300.00	300.00	300.00
4.750	300.00	300.00	300.00	300.00	300.00
5.000	300.00	300.00	300.00	300.00	300.00
5.250	300.00	300.00	300.00	300.00	300.00
5.500	300.00	300.00	300.00	300.00	300.00
5.750	300.00	300.00	300.00	300.00	300.00
6.000	300.00	300.00	300.00	300.00	300.00
6.250	300.00	300.00	300.00	300.00	300.00
6.500	300.00	300.00	300.00	300.00	300.00
6.750	300.00	300.00	300.00	300.00	300.00
7.000	300.00	300.00	300.00	300.00	300.00
7.250	300.00	300.00	300.00	300.00	300.00
7.500	300.00	300.00	300.00	300.00	300.00
7.750	300.00	300.00	300.00	300.00	300.00
8.000	300.00	300.00	300.00	300.00	300.00
8.250	300.00	300.00	300.00	300.00	300.00
8.500	300.00	300.00	300.00	300.00	300.00
8.750	300.00	300.00	300.00	300.00	300.00
9.000	300.00	300.00	300.00	300.00	300.00
9.250	300.00	300.00	300.00	300.00	300.00
9.500	300.00	300.00	300.00	300.00	300.00
9.750	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	300.00	300.00	300.00	300.00	300.00
10.250	300.00	300.00	300.00	300.00	300.00
10.500	300.00	300.00	300.00	300.00	300.00
10.750	300.00	300.00	300.00	300.00	300.00
11.000	300.00	300.00	300.00	300.00	300.00
11.250	300.00	300.01	300.01	300.01	300.01
11.500	300.02	300.02	300.03	300.04	300.06
11.750	300.08	300.10	300.14	300.19	300.26
12.000	300.37	300.53	300.71	300.92	301.09
12.250	301.21	301.30	301.37	301.43	301.46
12.500	301.48	301.48	301.46	301.44	301.41
12.750	301.38	301.35	301.31	301.27	301.23
13.000	301.19	301.15	301.11	301.06	301.02
13.250	300.97	300.93	300.88	300.83	300.78
13.500	300.74	300.69	300.64	300.60	300.55
13.750	300.50	300.45	300.41	300.38	300.35
14.000	300.32	300.30	300.28	300.27	300.25
14.250	300.24	300.23	300.22	300.21	300.20
14.500	300.20	300.19	300.19	300.18	300.18
14.750	300.18	300.17	300.17	300.17	300.16
15.000	300.16	300.16	300.16	300.15	300.15
15.250	300.15	300.15	300.15	300.14	300.14
15.500	300.14	300.14	300.13	300.13	300.13
15.750	300.13	300.13	300.12	300.12	300.12
16.000	300.12	300.12	300.11	300.11	300.11
16.250	300.11	300.11	300.11	300.10	300.10
16.500	300.10	300.10	300.10	300.10	300.10
16.750	300.10	300.10	300.09	300.09	300.09
17.000	300.09	300.09	300.09	300.09	300.09
17.250	300.09	300.09	300.08	300.08	300.08
17.500	300.08	300.08	300.08	300.08	300.08
17.750	300.08	300.08	300.08	300.07	300.07
18.000	300.07	300.07	300.07	300.07	300.07
18.250	300.07	300.07	300.07	300.07	300.07
18.500	300.07	300.06	300.06	300.06	300.06
18.750	300.06	300.06	300.06	300.06	300.06
19.000	300.06	300.06	300.06	300.06	300.06
19.250	300.06	300.06	300.06	300.06	300.06
19.500	300.06	300.06	300.06	300.06	300.06
19.750	300.06	300.06	300.06	300.06	300.06
20.000	300.06	300.06	300.06	300.06	300.05

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	300.05	300.05	300.05	300.05	300.05
20.500	300.05	300.05	300.05	300.05	300.05
20.750	300.05	300.05	300.05	300.05	300.05
21.000	300.05	300.05	300.05	300.05	300.05
21.250	300.05	300.05	300.05	300.05	300.05
21.500	300.05	300.05	300.05	300.05	300.05
21.750	300.05	300.05	300.05	300.05	300.05
22.000	300.05	300.05	300.05	300.05	300.05
22.250	300.05	300.05	300.05	300.05	300.04
22.500	300.04	300.04	300.04	300.04	300.04
22.750	300.04	300.04	300.04	300.04	300.04
23.000	300.04	300.04	300.04	300.04	300.04
23.250	300.04	300.04	300.04	300.04	300.04
23.500	300.04	300.04	300.04	300.04	300.04
23.750	300.04	300.04	300.04	300.04	300.04
24.000	300.04	300.04	300.03	300.03	300.03
24.250	300.02	300.02	300.02	300.01	300.01
24.500	300.01	300.01	300.01	300.01	300.00
24.750	300.00	300.00	300.00	300.00	300.00
25.000	300.00	300.00	300.00	300.00	300.00
25.250	300.00	300.00	300.00	300.00	300.00
25.500	300.00	300.00	300.00	300.00	300.00
25.750	300.00	300.00	300.00	300.00	300.00
26.000	300.00	300.00	300.00	300.00	300.00
26.250	300.00	300.00	300.00	300.00	300.00
26.500	300.00	300.00	300.00	300.00	300.00
26.750	300.00	300.00	300.00	300.00	300.00
27.000	300.00	300.00	300.00	300.00	300.00
27.250	300.00	300.00	300.00	300.00	300.00
27.500	300.00	300.00	300.00	300.00	300.00
27.750	300.00	300.00	300.00	300.00	300.00
28.000	300.00	300.00	300.00	300.00	300.00
28.250	300.00	300.00	300.00	300.00	300.00
28.500	300.00	300.00	300.00	300.00	300.00
28.750	300.00	300.00	300.00	300.00	300.00
29.000	300.00	300.00	300.00	300.00	300.00
29.250	300.00	300.00	300.00	300.00	300.00
29.500	300.00	300.00	300.00	300.00	300.00
29.750	300.00	300.00	300.00	300.00	300.00
30.000	300.00	300.00	300.00	300.00	300.00
30.250	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	300.00	300.00	300.00	300.00	300.00
30.750	300.00	300.00	300.00	300.00	300.00
31.000	300.00	300.00	300.00	300.00	300.00
31.250	300.00	300.00	300.00	300.00	300.00
31.500	300.00	300.00	300.00	300.00	300.00
31.750	300.00	300.00	300.00	300.00	300.00
32.000	300.00	300.00	300.00	300.00	300.00
32.250	300.00	300.00	300.00	300.00	300.00
32.500	300.00	300.00	300.00	300.00	300.00
32.750	300.00	300.00	300.00	300.00	300.00
33.000	300.00	300.00	300.00	300.00	300.00
33.250	300.00	300.00	300.00	300.00	300.00
33.500	300.00	300.00	300.00	300.00	300.00
33.750	300.00	300.00	300.00	300.00	300.00
34.000	300.00	300.00	300.00	300.00	300.00
34.250	300.00	300.00	300.00	300.00	300.00
34.500	300.00	300.00	300.00	300.00	300.00
34.750	300.00	300.00	300.00	300.00	300.00
35.000	300.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	300.00	300.00	300.00	300.00	300.00
0.250	300.00	300.00	300.00	300.00	300.00
0.500	300.00	300.00	300.00	300.00	300.00
0.750	300.00	300.00	300.00	300.00	300.00
1.000	300.00	300.00	300.00	300.00	300.00
1.250	300.00	300.00	300.00	300.00	300.00
1.500	300.00	300.00	300.00	300.00	300.00
1.750	300.00	300.00	300.00	300.00	300.00
2.000	300.00	300.00	300.00	300.00	300.00
2.250	300.00	300.00	300.00	300.00	300.00
2.500	300.00	300.00	300.00	300.00	300.00
2.750	300.00	300.00	300.00	300.00	300.00
3.000	300.00	300.00	300.00	300.00	300.00
3.250	300.00	300.00	300.00	300.00	300.00
3.500	300.00	300.00	300.00	300.00	300.00
3.750	300.00	300.00	300.00	300.00	300.00
4.000	300.00	300.00	300.00	300.00	300.00
4.250	300.00	300.00	300.00	300.00	300.00
4.500	300.00	300.00	300.00	300.00	300.00
4.750	300.00	300.00	300.00	300.00	300.00
5.000	300.00	300.00	300.00	300.00	300.00
5.250	300.00	300.00	300.00	300.00	300.00
5.500	300.00	300.00	300.00	300.00	300.00
5.750	300.00	300.00	300.00	300.00	300.00
6.000	300.00	300.00	300.00	300.00	300.00
6.250	300.00	300.00	300.00	300.00	300.00
6.500	300.00	300.00	300.00	300.00	300.00
6.750	300.00	300.00	300.00	300.00	300.00
7.000	300.00	300.00	300.00	300.00	300.00
7.250	300.00	300.00	300.00	300.00	300.00
7.500	300.00	300.00	300.00	300.00	300.00
7.750	300.00	300.00	300.00	300.00	300.00
8.000	300.00	300.00	300.00	300.00	300.00
8.250	300.00	300.00	300.00	300.00	300.00
8.500	300.00	300.00	300.00	300.00	300.00
8.750	300.00	300.00	300.00	300.00	300.00
9.000	300.00	300.00	300.00	300.00	300.00
9.250	300.00	300.00	300.00	300.00	300.00
9.500	300.00	300.00	300.00	300.00	300.00
9.750	300.00	300.00	300.00	300.00	300.01

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	300.01	300.01	300.01	300.01	300.02
10.250	300.02	300.02	300.03	300.03	300.03
10.500	300.04	300.04	300.04	300.05	300.05
10.750	300.06	300.06	300.07	300.07	300.08
11.000	300.08	300.09	300.10	300.10	300.11
11.250	300.12	300.13	300.14	300.15	300.17
11.500	300.18	300.20	300.22	300.26	300.30
11.750	300.37	300.46	300.56	300.69	300.89
12.000	301.20	301.62	302.09	302.54	302.91
12.250	303.17	303.38	303.55	303.68	303.78
12.500	303.85	303.89	303.91	303.92	303.92
12.750	303.91	303.90	303.89	303.87	303.85
13.000	303.83	303.80	303.77	303.75	303.72
13.250	303.69	303.65	303.62	303.59	303.56
13.500	303.53	303.49	303.46	303.42	303.38
13.750	303.34	303.31	303.27	303.23	303.19
14.000	303.15	303.11	303.07	303.03	302.98
14.250	302.94	302.89	302.85	302.81	302.76
14.500	302.72	302.67	302.63	302.59	302.54
14.750	302.50	302.45	302.40	302.36	302.31
15.000	302.26	302.22	302.17	302.12	302.08
15.250	302.03	301.98	301.93	301.88	301.83
15.500	301.78	301.73	301.68	301.63	301.58
15.750	301.53	301.48	301.42	301.37	301.32
16.000	301.26	301.21	301.15	301.10	301.05
16.250	300.99	300.94	300.88	300.83	300.77
16.500	300.72	300.66	300.61	300.56	300.50
16.750	300.45	300.40	300.37	300.33	300.31
17.000	300.28	300.26	300.25	300.23	300.22
17.250	300.21	300.20	300.19	300.18	300.18
17.500	300.17	300.17	300.16	300.16	300.15
17.750	300.15	300.15	300.15	300.14	300.14
18.000	300.14	300.14	300.13	300.13	300.13
18.250	300.13	300.13	300.12	300.12	300.12
18.500	300.12	300.12	300.12	300.12	300.12
18.750	300.12	300.12	300.12	300.12	300.11
19.000	300.11	300.11	300.11	300.11	300.11
19.250	300.11	300.11	300.11	300.11	300.11
19.500	300.11	300.11	300.11	300.11	300.11
19.750	300.11	300.11	300.10	300.10	300.10
20.000	300.10	300.10	300.10	300.10	300.10

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	300.10	300.10	300.10	300.10	300.10
20.500	300.10	300.10	300.10	300.10	300.10
20.750	300.10	300.10	300.10	300.09	300.09
21.000	300.09	300.09	300.09	300.09	300.09
21.250	300.09	300.09	300.09	300.09	300.09
21.500	300.09	300.09	300.09	300.09	300.09
21.750	300.09	300.09	300.09	300.09	300.09
22.000	300.09	300.09	300.08	300.08	300.08
22.250	300.08	300.08	300.08	300.08	300.08
22.500	300.08	300.08	300.08	300.08	300.08
22.750	300.08	300.08	300.08	300.08	300.08
23.000	300.08	300.08	300.08	300.08	300.08
23.250	300.08	300.07	300.07	300.07	300.07
23.500	300.07	300.07	300.07	300.07	300.07
23.750	300.07	300.07	300.07	300.07	300.07
24.000	300.07	300.07	300.06	300.06	300.05
24.250	300.04	300.03	300.03	300.02	300.02
24.500	300.02	300.01	300.01	300.01	300.01
24.750	300.01	300.01	300.01	300.00	300.00
25.000	300.00	300.00	300.00	300.00	300.00
25.250	300.00	300.00	300.00	300.00	300.00
25.500	300.00	300.00	300.00	300.00	300.00
25.750	300.00	300.00	300.00	300.00	300.00
26.000	300.00	300.00	300.00	300.00	300.00
26.250	300.00	300.00	300.00	300.00	300.00
26.500	300.00	300.00	300.00	300.00	300.00
26.750	300.00	300.00	300.00	300.00	300.00
27.000	300.00	300.00	300.00	300.00	300.00
27.250	300.00	300.00	300.00	300.00	300.00
27.500	300.00	300.00	300.00	300.00	300.00
27.750	300.00	300.00	300.00	300.00	300.00
28.000	300.00	300.00	300.00	300.00	300.00
28.250	300.00	300.00	300.00	300.00	300.00
28.500	300.00	300.00	300.00	300.00	300.00
28.750	300.00	300.00	300.00	300.00	300.00
29.000	300.00	300.00	300.00	300.00	300.00
29.250	300.00	300.00	300.00	300.00	300.00
29.500	300.00	300.00	300.00	300.00	300.00
29.750	300.00	300.00	300.00	300.00	300.00
30.000	300.00	300.00	300.00	300.00	300.00
30.250	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	300.00	300.00	300.00	300.00	300.00
30.750	300.00	300.00	300.00	300.00	300.00
31.000	300.00	300.00	300.00	300.00	300.00
31.250	300.00	300.00	300.00	300.00	300.00
31.500	300.00	300.00	300.00	300.00	300.00
31.750	300.00	300.00	300.00	300.00	300.00
32.000	300.00	300.00	300.00	300.00	300.00
32.250	300.00	300.00	300.00	300.00	300.00
32.500	300.00	300.00	300.00	300.00	300.00
32.750	300.00	300.00	300.00	300.00	300.00
33.000	300.00	300.00	300.00	300.00	300.00
33.250	300.00	300.00	300.00	300.00	300.00
33.500	300.00	300.00	300.00	300.00	300.00
33.750	300.00	300.00	300.00	300.00	300.00
34.000	300.00	300.00	300.00	300.00	300.00
34.250	300.00	300.00	300.00	300.00	300.00
34.500	300.00	300.00	300.00	300.00	300.00
34.750	300.00	300.00	300.00	300.00	300.00
35.000	300.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	300.00	300.00	300.00	300.00	300.00
0.250	300.00	300.00	300.00	300.00	300.00
0.500	300.00	300.00	300.00	300.00	300.00
0.750	300.00	300.00	300.00	300.00	300.00
1.000	300.00	300.00	300.00	300.00	300.00
1.250	300.00	300.00	300.00	300.00	300.00
1.500	300.00	300.00	300.00	300.00	300.00
1.750	300.00	300.00	300.00	300.00	300.00
2.000	300.00	300.00	300.00	300.00	300.00
2.250	300.00	300.00	300.00	300.00	300.00
2.500	300.00	300.00	300.00	300.00	300.00
2.750	300.00	300.00	300.00	300.00	300.00
3.000	300.00	300.00	300.00	300.00	300.00
3.250	300.00	300.00	300.00	300.00	300.00
3.500	300.00	300.00	300.00	300.00	300.00
3.750	300.00	300.00	300.00	300.00	300.00
4.000	300.00	300.00	300.00	300.00	300.00
4.250	300.00	300.00	300.00	300.00	300.00
4.500	300.00	300.00	300.00	300.00	300.00
4.750	300.00	300.00	300.00	300.00	300.00
5.000	300.00	300.00	300.00	300.00	300.00
5.250	300.00	300.00	300.00	300.00	300.00
5.500	300.00	300.00	300.00	300.00	300.00
5.750	300.00	300.00	300.00	300.00	300.00
6.000	300.00	300.00	300.00	300.00	300.00
6.250	300.00	300.00	300.00	300.00	300.00
6.500	300.00	300.00	300.00	300.00	300.00
6.750	300.00	300.00	300.00	300.00	300.00
7.000	300.00	300.00	300.00	300.00	300.00
7.250	300.00	300.00	300.00	300.00	300.00
7.500	300.00	300.00	300.00	300.00	300.00
7.750	300.00	300.00	300.00	300.00	300.00
8.000	300.00	300.00	300.00	300.00	300.00
8.250	300.00	300.00	300.00	300.00	300.00
8.500	300.00	300.00	300.00	300.00	300.00
8.750	300.00	300.00	300.00	300.00	300.00
9.000	300.00	300.00	300.01	300.01	300.01
9.250	300.01	300.01	300.02	300.02	300.02
9.500	300.03	300.03	300.03	300.04	300.04
9.750	300.04	300.05	300.05	300.05	300.06

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	300.06	300.06	300.07	300.07	300.08
10.250	300.08	300.09	300.09	300.10	300.11
10.500	300.11	300.12	300.13	300.13	300.14
10.750	300.15	300.15	300.16	300.17	300.18
11.000	300.19	300.20	300.21	300.22	300.23
11.250	300.25	300.26	300.28	300.30	300.32
11.500	300.35	300.37	300.41	300.47	300.55
11.750	300.66	300.82	301.03	301.28	301.60
12.000	302.07	302.66	303.30	303.91	304.38
12.250	304.72	304.94	305.07	305.11	305.11
12.500	305.08	305.04	305.00	304.96	304.91
12.750	304.87	304.83	304.79	304.76	304.73
13.000	304.70	304.67	304.65	304.63	304.61
13.250	304.59	304.57	304.56	304.54	304.52
13.500	304.49	304.47	304.45	304.42	304.40
13.750	304.37	304.34	304.32	304.29	304.26
14.000	304.23	304.19	304.16	304.13	304.10
14.250	304.06	304.03	304.00	303.96	303.92
14.500	303.89	303.85	303.81	303.78	303.74
14.750	303.70	303.66	303.63	303.59	303.55
15.000	303.51	303.47	303.43	303.39	303.35
15.250	303.30	303.26	303.22	303.18	303.13
15.500	303.09	303.05	303.00	302.96	302.91
15.750	302.86	302.81	302.77	302.72	302.67
16.000	302.62	302.58	302.53	302.48	302.43
16.250	302.37	302.32	302.27	302.22	302.17
16.500	302.12	302.07	302.02	301.97	301.92
16.750	301.87	301.81	301.76	301.71	301.66
17.000	301.61	301.56	301.50	301.45	301.40
17.250	301.34	301.29	301.24	301.18	301.13
17.500	301.08	301.03	300.97	300.92	300.86
17.750	300.81	300.75	300.70	300.64	300.59
18.000	300.54	300.48	300.43	300.39	300.35
18.250	300.32	300.30	300.28	300.26	300.24
18.500	300.23	300.22	300.21	300.20	300.19
18.750	300.19	300.18	300.18	300.17	300.17
19.000	300.17	300.16	300.16	300.16	300.16
19.250	300.16	300.15	300.15	300.15	300.15
19.500	300.15	300.15	300.15	300.15	300.15
19.750	300.15	300.14	300.14	300.14	300.14
20.000	300.14	300.14	300.14	300.14	300.14

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	300.14	300.14	300.14	300.14	300.13
20.500	300.13	300.13	300.13	300.13	300.13
20.750	300.13	300.13	300.13	300.13	300.13
21.000	300.13	300.13	300.13	300.13	300.13
21.250	300.13	300.12	300.12	300.12	300.12
21.500	300.12	300.12	300.12	300.12	300.12
21.750	300.12	300.12	300.12	300.12	300.12
22.000	300.12	300.12	300.12	300.12	300.11
22.250	300.11	300.11	300.11	300.11	300.11
22.500	300.11	300.11	300.11	300.11	300.11
22.750	300.11	300.11	300.11	300.11	300.11
23.000	300.11	300.10	300.10	300.10	300.10
23.250	300.10	300.10	300.10	300.10	300.10
23.500	300.10	300.10	300.10	300.10	300.10
23.750	300.10	300.10	300.10	300.09	300.09
24.000	300.09	300.09	300.09	300.08	300.06
24.250	300.05	300.05	300.04	300.03	300.03
24.500	300.02	300.02	300.02	300.01	300.01
24.750	300.01	300.01	300.01	300.01	300.00
25.000	300.00	300.00	300.00	300.00	300.00
25.250	300.00	300.00	300.00	300.00	300.00
25.500	300.00	300.00	300.00	300.00	300.00
25.750	300.00	300.00	300.00	300.00	300.00
26.000	300.00	300.00	300.00	300.00	300.00
26.250	300.00	300.00	300.00	300.00	300.00
26.500	300.00	300.00	300.00	300.00	300.00
26.750	300.00	300.00	300.00	300.00	300.00
27.000	300.00	300.00	300.00	300.00	300.00
27.250	300.00	300.00	300.00	300.00	300.00
27.500	300.00	300.00	300.00	300.00	300.00
27.750	300.00	300.00	300.00	300.00	300.00
28.000	300.00	300.00	300.00	300.00	300.00
28.250	300.00	300.00	300.00	300.00	300.00
28.500	300.00	300.00	300.00	300.00	300.00
28.750	300.00	300.00	300.00	300.00	300.00
29.000	300.00	300.00	300.00	300.00	300.00
29.250	300.00	300.00	300.00	300.00	300.00
29.500	300.00	300.00	300.00	300.00	300.00
29.750	300.00	300.00	300.00	300.00	300.00
30.000	300.00	300.00	300.00	300.00	300.00
30.250	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	300.00	300.00	300.00	300.00	300.00
30.750	300.00	300.00	300.00	300.00	300.00
31.000	300.00	300.00	300.00	300.00	300.00
31.250	300.00	300.00	300.00	300.00	300.00
31.500	300.00	300.00	300.00	300.00	300.00
31.750	300.00	300.00	300.00	300.00	300.00
32.000	300.00	300.00	300.00	300.00	300.00
32.250	300.00	300.00	300.00	300.00	300.00
32.500	300.00	300.00	300.00	300.00	300.00
32.750	300.00	300.00	300.00	300.00	300.00
33.000	300.00	300.00	300.00	300.00	300.00
33.250	300.00	300.00	300.00	300.00	300.00
33.500	300.00	300.00	300.00	300.00	300.00
33.750	300.00	300.00	300.00	300.00	300.00
34.000	300.00	300.00	300.00	300.00	300.00
34.250	300.00	300.00	300.00	300.00	300.00
34.500	300.00	300.00	300.00	300.00	300.00
34.750	300.00	300.00	300.00	300.00	300.00
35.000	300.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	300.00	300.00	300.00	300.00	300.00
0.250	300.00	300.00	300.00	300.00	300.00
0.500	300.00	300.00	300.00	300.00	300.00
0.750	300.00	300.00	300.00	300.00	300.00
1.000	300.00	300.00	300.00	300.00	300.00
1.250	300.00	300.00	300.00	300.00	300.00
1.500	300.00	300.00	300.00	300.00	300.00
1.750	300.00	300.00	300.00	300.00	300.00
2.000	300.00	300.00	300.00	300.00	300.00
2.250	300.00	300.00	300.00	300.00	300.00
2.500	300.00	300.00	300.00	300.00	300.00
2.750	300.00	300.00	300.00	300.00	300.00
3.000	300.00	300.00	300.00	300.00	300.00
3.250	300.00	300.00	300.00	300.00	300.00
3.500	300.00	300.00	300.00	300.00	300.00
3.750	300.00	300.00	300.00	300.00	300.00
4.000	300.00	300.00	300.00	300.00	300.00
4.250	300.00	300.00	300.00	300.00	300.00
4.500	300.00	300.00	300.00	300.00	300.00
4.750	300.00	300.00	300.00	300.00	300.00
5.000	300.00	300.00	300.00	300.00	300.00
5.250	300.00	300.00	300.00	300.00	300.00
5.500	300.00	300.00	300.00	300.00	300.00
5.750	300.00	300.00	300.00	300.00	300.00
6.000	300.00	300.00	300.00	300.00	300.00
6.250	300.00	300.00	300.00	300.00	300.00
6.500	300.00	300.00	300.00	300.00	300.00
6.750	300.00	300.00	300.00	300.00	300.00
7.000	300.00	300.00	300.00	300.00	300.00
7.250	300.00	300.00	300.00	300.00	300.00
7.500	300.00	300.00	300.00	300.01	300.01
7.750	300.01	300.01	300.01	300.01	300.02
8.000	300.02	300.02	300.02	300.02	300.03
8.250	300.03	300.03	300.04	300.04	300.04
8.500	300.05	300.05	300.05	300.06	300.06
8.750	300.06	300.07	300.07	300.08	300.08
9.000	300.09	300.09	300.10	300.10	300.11
9.250	300.11	300.12	300.12	300.13	300.14
9.500	300.14	300.15	300.15	300.16	300.17
9.750	300.17	300.18	300.19	300.20	300.20

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	300.21	300.22	300.23	300.23	300.24
10.250	300.25	300.26	300.27	300.29	300.30
10.500	300.31	300.32	300.33	300.35	300.36
10.750	300.37	300.39	300.40	300.42	300.43
11.000	300.45	300.47	300.48	300.50	300.53
11.250	300.56	300.60	300.65	300.70	300.77
11.500	300.84	300.92	301.04	301.18	301.39
11.750	301.65	301.97	302.34	302.77	303.30
12.000	304.00	304.83	305.51	305.83	305.84
12.250	305.74	305.64	305.56	305.50	305.43
12.500	305.34	305.26	305.18	305.11	305.06
12.750	305.03	305.00	304.98	304.96	304.94
13.000	304.91	304.88	304.86	304.83	304.81
13.250	304.79	304.77	304.76	304.74	304.73
13.500	304.72	304.71	304.70	304.68	304.67
13.750	304.67	304.66	304.65	304.64	304.63
14.000	304.62	304.61	304.60	304.59	304.59
14.250	304.58	304.56	304.55	304.54	304.53
14.500	304.51	304.50	304.48	304.47	304.45
14.750	304.43	304.41	304.40	304.38	304.35
15.000	304.33	304.31	304.29	304.27	304.24
15.250	304.22	304.19	304.17	304.14	304.11
15.500	304.08	304.05	304.03	304.00	303.96
15.750	303.93	303.90	303.86	303.83	303.79
16.000	303.75	303.72	303.68	303.64	303.61
16.250	303.57	303.53	303.49	303.45	303.41
16.500	303.37	303.33	303.29	303.25	303.21
16.750	303.17	303.13	303.09	303.04	303.00
17.000	302.96	302.92	302.87	302.83	302.79
17.250	302.74	302.70	302.65	302.61	302.57
17.500	302.52	302.48	302.43	302.38	302.34
17.750	302.29	302.24	302.20	302.15	302.10
18.000	302.06	302.01	301.96	301.91	301.86
18.250	301.81	301.76	301.71	301.67	301.62
18.500	301.57	301.52	301.48	301.43	301.38
18.750	301.33	301.28	301.23	301.18	301.14
19.000	301.09	301.04	301.00	300.95	300.90
19.250	300.85	300.80	300.76	300.71	300.66
19.500	300.62	300.57	300.52	300.48	300.44
19.750	300.40	300.37	300.35	300.33	300.31
20.000	300.29	300.28	300.27	300.26	300.25

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	300.24	300.24	300.23	300.23	300.22
20.500	300.22	300.22	300.21	300.21	300.21
20.750	300.21	300.21	300.20	300.20	300.20
21.000	300.20	300.20	300.20	300.20	300.19
21.250	300.19	300.19	300.19	300.19	300.19
21.500	300.19	300.19	300.19	300.19	300.18
21.750	300.18	300.18	300.18	300.18	300.18
22.000	300.18	300.18	300.18	300.18	300.18
22.250	300.17	300.17	300.17	300.17	300.17
22.500	300.17	300.17	300.17	300.17	300.17
22.750	300.17	300.17	300.16	300.16	300.16
23.000	300.16	300.16	300.16	300.16	300.16
23.250	300.16	300.16	300.16	300.15	300.15
23.500	300.15	300.15	300.15	300.15	300.15
23.750	300.15	300.15	300.15	300.15	300.14
24.000	300.14	300.14	300.13	300.12	300.10
24.250	300.08	300.07	300.06	300.05	300.04
24.500	300.04	300.03	300.02	300.02	300.02
24.750	300.01	300.01	300.01	300.01	300.01
25.000	300.01	300.01	300.00	300.00	300.00
25.250	300.00	300.00	300.00	300.00	300.00
25.500	300.00	300.00	300.00	300.00	300.00
25.750	300.00	300.00	300.00	300.00	300.00
26.000	300.00	300.00	300.00	300.00	300.00
26.250	300.00	300.00	300.00	300.00	300.00
26.500	300.00	300.00	300.00	300.00	300.00
26.750	300.00	300.00	300.00	300.00	300.00
27.000	300.00	300.00	300.00	300.00	300.00
27.250	300.00	300.00	300.00	300.00	300.00
27.500	300.00	300.00	300.00	300.00	300.00
27.750	300.00	300.00	300.00	300.00	300.00
28.000	300.00	300.00	300.00	300.00	300.00
28.250	300.00	300.00	300.00	300.00	300.00
28.500	300.00	300.00	300.00	300.00	300.00
28.750	300.00	300.00	300.00	300.00	300.00
29.000	300.00	300.00	300.00	300.00	300.00
29.250	300.00	300.00	300.00	300.00	300.00
29.500	300.00	300.00	300.00	300.00	300.00
29.750	300.00	300.00	300.00	300.00	300.00
30.000	300.00	300.00	300.00	300.00	300.00
30.250	300.00	300.00	300.00	300.00	300.00

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #4 (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	300.00	300.00	300.00	300.00	300.00
30.750	300.00	300.00	300.00	300.00	300.00
31.000	300.00	300.00	300.00	300.00	300.00
31.250	300.00	300.00	300.00	300.00	300.00
31.500	300.00	300.00	300.00	300.00	300.00
31.750	300.00	300.00	300.00	300.00	300.00
32.000	300.00	300.00	300.00	300.00	300.00
32.250	300.00	300.00	300.00	300.00	300.00
32.500	300.00	300.00	300.00	300.00	300.00
32.750	300.00	300.00	300.00	300.00	300.00
33.000	300.00	300.00	300.00	300.00	300.00
33.250	300.00	300.00	300.00	300.00	300.00
33.500	300.00	300.00	300.00	300.00	300.00
33.750	300.00	300.00	300.00	300.00	300.00
34.000	300.00	300.00	300.00	300.00	300.00
34.250	300.00	300.00	300.00	300.00	300.00
34.500	300.00	300.00	300.00	300.00	300.00
34.750	300.00	300.00	300.00	300.00	300.00
35.000	300.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	318.75	318.75	318.75	318.75	318.75
0.250	318.75	318.75	318.75	318.75	318.75
0.500	318.75	318.75	318.75	318.75	318.75
0.750	318.75	318.75	318.75	318.75	318.75
1.000	318.75	318.75	318.75	318.75	318.75
1.250	318.75	318.75	318.75	318.75	318.75
1.500	318.75	318.75	318.75	318.75	318.75
1.750	318.75	318.75	318.75	318.75	318.75
2.000	318.75	318.75	318.75	318.75	318.75
2.250	318.75	318.75	318.75	318.75	318.75
2.500	318.75	318.75	318.75	318.75	318.75
2.750	318.75	318.75	318.75	318.75	318.75
3.000	318.75	318.75	318.75	318.75	318.75
3.250	318.75	318.75	318.75	318.75	318.75
3.500	318.75	318.75	318.75	318.75	318.75
3.750	318.75	318.75	318.75	318.75	318.75
4.000	318.75	318.75	318.75	318.75	318.75
4.250	318.75	318.75	318.75	318.75	318.75
4.500	318.75	318.75	318.75	318.75	318.75
4.750	318.75	318.75	318.75	318.75	318.75
5.000	318.75	318.75	318.75	318.75	318.75
5.250	318.75	318.75	318.75	318.75	318.75
5.500	318.75	318.75	318.75	318.75	318.75
5.750	318.75	318.75	318.75	318.75	318.75
6.000	318.75	318.75	318.75	318.75	318.75
6.250	318.75	318.75	318.75	318.75	318.75
6.500	318.75	318.75	318.75	318.75	318.75
6.750	318.75	318.75	318.75	318.75	318.75
7.000	318.75	318.75	318.75	318.75	318.75
7.250	318.75	318.75	318.75	318.75	318.75
7.500	318.75	318.75	318.75	318.75	318.75
7.750	318.75	318.75	318.75	318.75	318.75
8.000	318.75	318.75	318.75	318.75	318.75
8.250	318.75	318.75	318.75	318.75	318.75
8.500	318.75	318.75	318.75	318.75	318.75
8.750	318.75	318.75	318.75	318.75	318.75
9.000	318.75	318.75	318.75	318.75	318.75
9.250	318.75	318.75	318.75	318.75	318.75
9.500	318.75	318.75	318.75	318.75	318.75
9.750	318.75	318.75	318.75	318.75	318.75

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	318.75	318.75	318.75	318.75	318.75
10.250	318.76	318.76	318.76	318.76	318.76
10.500	318.77	318.77	318.77	318.78	318.78
10.750	318.78	318.79	318.79	318.80	318.80
11.000	318.81	318.82	318.82	318.83	318.84
11.250	318.84	318.85	318.86	318.87	318.89
11.500	318.90	318.91	318.93	318.95	318.98
11.750	319.02	319.07	319.13	319.21	319.31
12.000	319.44	319.64	319.87	320.13	320.38
12.250	320.59	320.76	320.90	321.02	321.12
12.500	321.20	321.27	321.32	321.36	321.39
12.750	321.41	321.44	321.46	321.47	321.49
13.000	321.51	321.52	321.53	321.54	321.55
13.250	321.56	321.56	321.57	321.58	321.58
13.500	321.59	321.59	321.60	321.60	321.61
13.750	321.61	321.61	321.62	321.62	321.62
14.000	321.62	321.62	321.62	321.62	321.62
14.250	321.62	321.62	321.62	321.62	321.62
14.500	321.61	321.61	321.61	321.61	321.60
14.750	321.60	321.60	321.59	321.59	321.59
15.000	321.58	321.58	321.57	321.57	321.56
15.250	321.56	321.55	321.55	321.54	321.54
15.500	321.53	321.52	321.52	321.51	321.50
15.750	321.50	321.49	321.48	321.47	321.46
16.000	321.46	321.45	321.44	321.43	321.42
16.250	321.41	321.40	321.39	321.38	321.37
16.500	321.36	321.35	321.34	321.33	321.32
16.750	321.31	321.30	321.29	321.28	321.27
17.000	321.26	321.25	321.24	321.23	321.22
17.250	321.21	321.19	321.18	321.17	321.16
17.500	321.15	321.13	321.12	321.11	321.10
17.750	321.09	321.07	321.06	321.05	321.03
18.000	321.02	321.01	321.00	320.98	320.97
18.250	320.96	320.94	320.93	320.92	320.91
18.500	320.89	320.88	320.87	320.85	320.84
18.750	320.83	320.81	320.80	320.79	320.78
19.000	320.76	320.75	320.74	320.72	320.71
19.250	320.69	320.68	320.67	320.65	320.64
19.500	320.62	320.61	320.60	320.58	320.57
19.750	320.56	320.54	320.53	320.51	320.50
20.000	320.49	320.47	320.46	320.44	320.43

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	320.42	320.40	320.39	320.38	320.36
20.500	320.35	320.34	320.32	320.31	320.29
20.750	320.28	320.27	320.25	320.24	320.22
21.000	320.21	320.20	320.18	320.17	320.15
21.250	320.14	320.12	320.11	320.09	320.08
21.500	320.06	320.05	320.04	320.02	320.01
21.750	319.99	319.98	319.96	319.95	319.94
22.000	319.92	319.91	319.89	319.88	319.86
22.250	319.85	319.84	319.82	319.81	319.79
22.500	319.78	319.76	319.75	319.74	319.72
22.750	319.71	319.69	319.67	319.66	319.64
23.000	319.63	319.61	319.60	319.58	319.57
23.250	319.55	319.54	319.52	319.51	319.49
23.500	319.48	319.46	319.45	319.43	319.42
23.750	319.40	319.39	319.37	319.36	319.34
24.000	319.33	319.31	319.30	319.28	319.26
24.250	319.24	319.22	319.20	319.18	319.16
24.500	319.15	319.13	319.11	319.10	319.08
24.750	319.07	319.06	319.04	319.03	319.02
25.000	319.01	319.00	318.99	318.98	318.97
25.250	318.96	318.95	318.94	318.93	318.92
25.500	318.92	318.91	318.90	318.90	318.89
25.750	318.88	318.88	318.87	318.87	318.86
26.000	318.86	318.85	318.85	318.84	318.84
26.250	318.84	318.83	318.83	318.83	318.82
26.500	318.82	318.82	318.81	318.81	318.81
26.750	318.81	318.80	318.80	318.80	318.80
27.000	318.80	318.79	318.79	318.79	318.79
27.250	318.79	318.78	318.78	318.78	318.78
27.500	318.78	318.78	318.78	318.78	318.77
27.750	318.77	318.77	318.77	318.77	318.77
28.000	318.77	318.77	318.77	318.77	318.77
28.250	318.77	318.76	318.76	318.76	318.76
28.500	318.76	318.76	318.76	318.76	318.76
28.750	318.76	318.76	318.76	318.76	318.76
29.000	318.76	318.76	318.76	318.76	318.76
29.250	318.76	318.76	318.76	318.76	318.76
29.500	318.76	318.75	318.75	318.75	318.75
29.750	318.75	318.75	318.75	318.75	318.75
30.000	318.75	318.75	318.75	318.75	318.75
30.250	318.75	318.75	318.75	318.75	318.75

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	318.75	318.75	318.75	318.75	318.75
30.750	318.75	318.75	318.75	318.75	318.75
31.000	318.75	318.75	318.75	318.75	318.75
31.250	318.75	318.75	318.75	318.75	318.75
31.500	318.75	318.75	318.75	318.75	318.75
31.750	318.75	318.75	318.75	318.75	318.75
32.000	318.75	318.75	318.75	318.75	318.75
32.250	318.75	318.75	318.75	318.75	318.75
32.500	318.75	318.75	318.75	318.75	318.75
32.750	318.75	318.75	318.75	318.75	318.75
33.000	318.75	318.75	318.75	318.75	318.75
33.250	318.75	318.75	318.75	318.75	318.75
33.500	318.75	318.75	318.75	318.75	318.75
33.750	318.75	318.75	318.75	318.75	318.75
34.000	318.75	318.75	318.75	318.75	318.75
34.250	318.75	318.75	318.75	318.75	318.75
34.500	318.75	318.75	318.75	318.75	318.75
34.750	318.75	318.75	318.75	318.75	318.75
35.000	318.75	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	318.75	318.75	318.75	318.75	318.75
0.250	318.75	318.75	318.75	318.75	318.75
0.500	318.75	318.75	318.75	318.75	318.75
0.750	318.75	318.75	318.75	318.75	318.75
1.000	318.75	318.75	318.75	318.75	318.75
1.250	318.75	318.75	318.75	318.75	318.75
1.500	318.75	318.75	318.75	318.75	318.75
1.750	318.75	318.75	318.75	318.75	318.75
2.000	318.75	318.75	318.75	318.75	318.75
2.250	318.75	318.75	318.75	318.75	318.75
2.500	318.75	318.75	318.75	318.75	318.75
2.750	318.75	318.75	318.75	318.75	318.75
3.000	318.75	318.75	318.75	318.75	318.75
3.250	318.75	318.75	318.75	318.75	318.75
3.500	318.75	318.75	318.75	318.75	318.75
3.750	318.75	318.75	318.75	318.75	318.75
4.000	318.75	318.75	318.75	318.75	318.75
4.250	318.75	318.75	318.75	318.75	318.75
4.500	318.75	318.75	318.75	318.75	318.75
4.750	318.75	318.75	318.75	318.75	318.75
5.000	318.75	318.75	318.75	318.75	318.75
5.250	318.75	318.75	318.75	318.75	318.75
5.500	318.75	318.75	318.75	318.75	318.75
5.750	318.75	318.75	318.75	318.75	318.75
6.000	318.75	318.75	318.75	318.75	318.75
6.250	318.75	318.75	318.75	318.75	318.75
6.500	318.75	318.75	318.75	318.75	318.75
6.750	318.75	318.75	318.75	318.75	318.75
7.000	318.75	318.75	318.75	318.75	318.75
7.250	318.75	318.75	318.75	318.75	318.75
7.500	318.75	318.75	318.75	318.75	318.75
7.750	318.75	318.75	318.75	318.75	318.75
8.000	318.75	318.75	318.75	318.75	318.75
8.250	318.75	318.75	318.75	318.75	318.75
8.500	318.75	318.75	318.75	318.75	318.75
8.750	318.75	318.75	318.75	318.75	318.75
9.000	318.75	318.75	318.75	318.75	318.75
9.250	318.75	318.75	318.75	318.75	318.75
9.500	318.76	318.76	318.76	318.76	318.76
9.750	318.77	318.77	318.77	318.78	318.78

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	318.78	318.79	318.79	318.79	318.80
10.250	318.80	318.81	318.81	318.82	318.83
10.500	318.83	318.84	318.85	318.85	318.86
10.750	318.87	318.88	318.89	318.90	318.91
11.000	318.92	318.93	318.94	318.95	318.96
11.250	318.98	318.99	319.01	319.03	319.04
11.500	319.07	319.09	319.12	319.15	319.20
11.750	319.26	319.33	319.43	319.55	319.70
12.000	319.90	320.18	320.50	320.85	321.17
12.250	321.44	321.66	321.84	321.97	322.06
12.500	322.13	322.17	322.18	322.19	322.18
12.750	322.17	322.15	322.14	322.12	322.11
13.000	322.09	322.07	322.05	322.04	322.02
13.250	322.01	321.99	321.98	321.97	321.95
13.500	321.94	321.93	321.92	321.91	321.90
13.750	321.90	321.89	321.88	321.87	321.87
14.000	321.86	321.85	321.85	321.84	321.83
14.250	321.83	321.82	321.82	321.81	321.81
14.500	321.81	321.80	321.80	321.79	321.79
14.750	321.79	321.78	321.78	321.78	321.77
15.000	321.77	321.77	321.76	321.76	321.76
15.250	321.76	321.75	321.75	321.75	321.74
15.500	321.74	321.74	321.73	321.73	321.72
15.750	321.72	321.72	321.71	321.71	321.70
16.000	321.69	321.69	321.68	321.68	321.67
16.250	321.66	321.66	321.65	321.64	321.64
16.500	321.63	321.62	321.62	321.61	321.60
16.750	321.59	321.59	321.58	321.57	321.56
17.000	321.56	321.55	321.54	321.53	321.52
17.250	321.51	321.51	321.50	321.49	321.48
17.500	321.47	321.46	321.45	321.44	321.43
17.750	321.42	321.41	321.40	321.39	321.38
18.000	321.37	321.36	321.35	321.34	321.33
18.250	321.32	321.31	321.29	321.28	321.27
18.500	321.26	321.25	321.24	321.23	321.22
18.750	321.20	321.19	321.18	321.17	321.16
19.000	321.15	321.13	321.12	321.11	321.10
19.250	321.09	321.07	321.06	321.05	321.04
19.500	321.03	321.02	321.00	320.99	320.98
19.750	320.97	320.96	320.94	320.93	320.92
20.000	320.91	320.90	320.88	320.87	320.86

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Infiltration Basin #3A (IN)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	320.85	320.84	320.82	320.81	320.80
20.500	320.79	320.78	320.76	320.75	320.74
20.750	320.73	320.71	320.70	320.69	320.68
21.000	320.66	320.65	320.64	320.62	320.61
21.250	320.60	320.59	320.57	320.56	320.55
21.500	320.53	320.52	320.51	320.50	320.48
21.750	320.47	320.46	320.44	320.43	320.42
22.000	320.41	320.39	320.38	320.37	320.35
22.250	320.34	320.33	320.32	320.30	320.29
22.500	320.28	320.26	320.25	320.24	320.22
22.750	320.21	320.20	320.18	320.17	320.16
23.000	320.14	320.13	320.11	320.10	320.09
23.250	320.07	320.06	320.04	320.03	320.02
23.500	320.00	319.99	319.98	319.96	319.95
23.750	319.93	319.92	319.91	319.89	319.88
24.000	319.87	319.85	319.84	319.82	319.80
24.250	319.78	319.76	319.74	319.72	319.70
24.500	319.68	319.66	319.63	319.61	319.59
24.750	319.57	319.55	319.53	319.51	319.49
25.000	319.47	319.45	319.42	319.40	319.38
25.250	319.36	319.34	319.32	319.30	319.28
25.500	319.26	319.24	319.22	319.20	319.18
25.750	319.16	319.15	319.13	319.11	319.10
26.000	319.08	319.07	319.05	319.04	319.03
26.250	319.02	319.01	319.00	318.98	318.97
26.500	318.97	318.96	318.95	318.94	318.93
26.750	318.92	318.92	318.91	318.90	318.90
27.000	318.89	318.88	318.88	318.87	318.87
27.250	318.86	318.86	318.85	318.85	318.84
27.500	318.84	318.84	318.83	318.83	318.83
27.750	318.82	318.82	318.82	318.81	318.81
28.000	318.81	318.81	318.80	318.80	318.80
28.250	318.80	318.80	318.79	318.79	318.79
28.500	318.79	318.79	318.78	318.78	318.78
28.750	318.78	318.78	318.78	318.78	318.78
29.000	318.77	318.77	318.77	318.77	318.77
29.250	318.77	318.77	318.77	318.77	318.77
29.500	318.77	318.77	318.76	318.76	318.76
29.750	318.76	318.76	318.76	318.76	318.76
30.000	318.76	318.76	318.76	318.76	318.76
30.250	318.76	318.76	318.76	318.76	318.76

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	318.76	318.76	318.76	318.76	318.76
30.750	318.76	318.76	318.75	318.75	318.75
31.000	318.75	318.75	318.75	318.75	318.75
31.250	318.75	318.75	318.75	318.75	318.75
31.500	318.75	318.75	318.75	318.75	318.75
31.750	318.75	318.75	318.75	318.75	318.75
32.000	318.75	318.75	318.75	318.75	318.75
32.250	318.75	318.75	318.75	318.75	318.75
32.500	318.75	318.75	318.75	318.75	318.75
32.750	318.75	318.75	318.75	318.75	318.75
33.000	318.75	318.75	318.75	318.75	318.75
33.250	318.75	318.75	318.75	318.75	318.75
33.500	318.75	318.75	318.75	318.75	318.75
33.750	318.75	318.75	318.75	318.75	318.75
34.000	318.75	318.75	318.75	318.75	318.75
34.250	318.75	318.75	318.75	318.75	318.75
34.500	318.75	318.75	318.75	318.75	318.75
34.750	318.75	318.75	318.75	318.75	318.75
35.000	318.75	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	318.75	318.75	318.75	318.75	318.75
0.250	318.75	318.75	318.75	318.75	318.75
0.500	318.75	318.75	318.75	318.75	318.75
0.750	318.75	318.75	318.75	318.75	318.75
1.000	318.75	318.75	318.75	318.75	318.75
1.250	318.75	318.75	318.75	318.75	318.75
1.500	318.75	318.75	318.75	318.75	318.75
1.750	318.75	318.75	318.75	318.75	318.75
2.000	318.75	318.75	318.75	318.75	318.75
2.250	318.75	318.75	318.75	318.75	318.75
2.500	318.75	318.75	318.75	318.75	318.75
2.750	318.75	318.75	318.75	318.75	318.75
3.000	318.75	318.75	318.75	318.75	318.75
3.250	318.75	318.75	318.75	318.75	318.75
3.500	318.75	318.75	318.75	318.75	318.75
3.750	318.75	318.75	318.75	318.75	318.75
4.000	318.75	318.75	318.75	318.75	318.75
4.250	318.75	318.75	318.75	318.75	318.75
4.500	318.75	318.75	318.75	318.75	318.75
4.750	318.75	318.75	318.75	318.75	318.75
5.000	318.75	318.75	318.75	318.75	318.75
5.250	318.75	318.75	318.75	318.75	318.75
5.500	318.75	318.75	318.75	318.75	318.75
5.750	318.75	318.75	318.75	318.75	318.75
6.000	318.75	318.75	318.75	318.75	318.75
6.250	318.75	318.75	318.75	318.75	318.75
6.500	318.75	318.75	318.75	318.75	318.75
6.750	318.75	318.75	318.75	318.75	318.75
7.000	318.75	318.75	318.75	318.75	318.75
7.250	318.75	318.75	318.75	318.75	318.75
7.500	318.75	318.75	318.75	318.75	318.75
7.750	318.75	318.75	318.75	318.76	318.76
8.000	318.76	318.76	318.76	318.76	318.77
8.250	318.77	318.77	318.78	318.78	318.78
8.500	318.78	318.79	318.79	318.80	318.80
8.750	318.81	318.81	318.82	318.82	318.83
9.000	318.83	318.84	318.84	318.85	318.86
9.250	318.86	318.87	318.88	318.89	318.89
9.500	318.90	318.91	318.92	318.93	318.94
9.750	318.95	318.96	318.97	318.98	318.99

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.00	319.01	319.02	319.03	319.05
10.250	319.06	319.07	319.09	319.10	319.12
10.500	319.13	319.15	319.17	319.18	319.20
10.750	319.22	319.24	319.26	319.28	319.30
11.000	319.32	319.35	319.37	319.40	319.43
11.250	319.47	319.51	319.55	319.60	319.65
11.500	319.70	319.76	319.83	319.91	320.01
11.750	320.15	320.31	320.51	320.74	321.01
12.000	321.36	321.80	322.28	322.69	322.98
12.250	323.14	323.20	323.20	323.16	323.10
12.500	323.03	322.95	322.86	322.78	322.70
12.750	322.63	322.56	322.51	322.46	322.42
13.000	322.38	322.35	322.32	322.30	322.27
13.250	322.26	322.24	322.22	322.21	322.19
13.500	322.18	322.16	322.15	322.14	322.13
13.750	322.12	322.10	322.09	322.08	322.07
14.000	322.06	322.05	322.04	322.03	322.02
14.250	322.01	322.00	322.00	321.99	321.98
14.500	321.97	321.97	321.96	321.95	321.95
14.750	321.94	321.94	321.93	321.93	321.92
15.000	321.92	321.91	321.91	321.90	321.90
15.250	321.89	321.89	321.88	321.88	321.87
15.500	321.87	321.86	321.86	321.85	321.85
15.750	321.84	321.84	321.84	321.83	321.83
16.000	321.82	321.82	321.81	321.81	321.80
16.250	321.80	321.80	321.79	321.79	321.79
16.500	321.78	321.78	321.78	321.77	321.77
16.750	321.77	321.77	321.76	321.76	321.76
17.000	321.76	321.75	321.75	321.75	321.75
17.250	321.74	321.74	321.74	321.73	321.73
17.500	321.73	321.72	321.72	321.72	321.71
17.750	321.71	321.70	321.70	321.69	321.69
18.000	321.68	321.68	321.67	321.67	321.66
18.250	321.66	321.65	321.65	321.64	321.64
18.500	321.63	321.62	321.62	321.61	321.61
18.750	321.60	321.59	321.59	321.58	321.58
19.000	321.57	321.56	321.56	321.55	321.54
19.250	321.54	321.53	321.52	321.52	321.51
19.500	321.50	321.50	321.49	321.48	321.48
19.750	321.47	321.46	321.46	321.45	321.44
20.000	321.43	321.43	321.42	321.41	321.40

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.40	321.39	321.38	321.37	321.37
20.500	321.36	321.35	321.34	321.33	321.33
20.750	321.32	321.31	321.30	321.29	321.29
21.000	321.28	321.27	321.26	321.25	321.25
21.250	321.24	321.23	321.22	321.21	321.20
21.500	321.19	321.18	321.17	321.17	321.16
21.750	321.15	321.14	321.13	321.12	321.11
22.000	321.10	321.09	321.08	321.07	321.06
22.250	321.05	321.04	321.04	321.03	321.02
22.500	321.01	321.00	320.99	320.98	320.97
22.750	320.96	320.95	320.94	320.93	320.92
23.000	320.91	320.90	320.89	320.88	320.87
23.250	320.86	320.85	320.84	320.83	320.82
23.500	320.81	320.80	320.79	320.78	320.77
23.750	320.76	320.75	320.74	320.72	320.71
24.000	320.70	320.69	320.68	320.66	320.64
24.250	320.62	320.60	320.58	320.56	320.54
24.500	320.52	320.50	320.47	320.45	320.43
24.750	320.41	320.39	320.37	320.35	320.33
25.000	320.31	320.29	320.27	320.25	320.23
25.250	320.21	320.19	320.16	320.14	320.12
25.500	320.10	320.08	320.06	320.04	320.02
25.750	320.00	319.97	319.95	319.93	319.91
26.000	319.89	319.87	319.85	319.83	319.81
26.250	319.79	319.77	319.75	319.73	319.71
26.500	319.69	319.66	319.64	319.62	319.60
26.750	319.58	319.56	319.54	319.52	319.50
27.000	319.47	319.45	319.43	319.41	319.39
27.250	319.37	319.35	319.33	319.31	319.29
27.500	319.27	319.25	319.23	319.21	319.19
27.750	319.17	319.15	319.14	319.12	319.10
28.000	319.09	319.07	319.06	319.05	319.03
28.250	319.02	319.01	319.00	318.99	318.98
28.500	318.97	318.96	318.95	318.94	318.93
28.750	318.93	318.92	318.91	318.90	318.90
29.000	318.89	318.89	318.88	318.87	318.87
29.250	318.86	318.86	318.85	318.85	318.85
29.500	318.84	318.84	318.83	318.83	318.83
29.750	318.82	318.82	318.82	318.81	318.81
30.000	318.81	318.81	318.80	318.80	318.80
30.250	318.80	318.80	318.79	318.79	318.79

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	318.79	318.79	318.79	318.78	318.78
30.750	318.78	318.78	318.78	318.78	318.78
31.000	318.77	318.77	318.77	318.77	318.77
31.250	318.77	318.77	318.77	318.77	318.77
31.500	318.77	318.77	318.76	318.76	318.76
31.750	318.76	318.76	318.76	318.76	318.76
32.000	318.76	318.76	318.76	318.76	318.76
32.250	318.76	318.76	318.76	318.76	318.76
32.500	318.76	318.76	318.76	318.76	318.76
32.750	318.76	318.76	318.76	318.75	318.75
33.000	318.75	318.75	318.75	318.75	318.75
33.250	318.75	318.75	318.75	318.75	318.75
33.500	318.75	318.75	318.75	318.75	318.75
33.750	318.75	318.75	318.75	318.75	318.75
34.000	318.75	318.75	318.75	318.75	318.75
34.250	318.75	318.75	318.75	318.75	318.75
34.500	318.75	318.75	318.75	318.75	318.75
34.750	318.75	318.75	318.75	318.75	318.75
35.000	318.75	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	318.75	318.75	318.75	318.75	318.75
0.250	318.75	318.75	318.75	318.75	318.75
0.500	318.75	318.75	318.75	318.75	318.75
0.750	318.75	318.75	318.75	318.75	318.75
1.000	318.75	318.75	318.75	318.75	318.75
1.250	318.75	318.75	318.75	318.75	318.75
1.500	318.75	318.75	318.75	318.75	318.75
1.750	318.75	318.75	318.75	318.75	318.75
2.000	318.75	318.75	318.75	318.75	318.75
2.250	318.75	318.75	318.75	318.75	318.75
2.500	318.75	318.75	318.75	318.75	318.75
2.750	318.75	318.75	318.75	318.75	318.75
3.000	318.75	318.75	318.75	318.75	318.75
3.250	318.75	318.75	318.75	318.75	318.75
3.500	318.75	318.75	318.75	318.75	318.75
3.750	318.75	318.75	318.75	318.75	318.75
4.000	318.75	318.75	318.75	318.75	318.75
4.250	318.75	318.75	318.75	318.75	318.75
4.500	318.75	318.75	318.75	318.75	318.75
4.750	318.75	318.75	318.75	318.75	318.75
5.000	318.75	318.75	318.75	318.75	318.75
5.250	318.75	318.75	318.75	318.75	318.75
5.500	318.75	318.75	318.75	318.75	318.75
5.750	318.75	318.75	318.75	318.75	318.75
6.000	318.75	318.75	318.75	318.75	318.75
6.250	318.75	318.75	318.75	318.75	318.75
6.500	318.75	318.75	318.75	318.75	318.75
6.750	318.75	318.75	318.75	318.76	318.76
7.000	318.76	318.76	318.76	318.76	318.77
7.250	318.77	318.77	318.77	318.78	318.78
7.500	318.78	318.79	318.79	318.79	318.80
7.750	318.80	318.80	318.81	318.81	318.82
8.000	318.82	318.83	318.83	318.84	318.84
8.250	318.85	318.85	318.86	318.87	318.87
8.500	318.88	318.89	318.90	318.90	318.91
8.750	318.92	318.93	318.94	318.95	318.96
9.000	318.97	318.98	318.99	319.00	319.01
9.250	319.02	319.03	319.05	319.06	319.07
9.500	319.09	319.10	319.11	319.13	319.14
9.750	319.16	319.17	319.19	319.20	319.22

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.24	319.25	319.27	319.29	319.31
10.250	319.33	319.35	319.37	319.40	319.42
10.500	319.45	319.48	319.51	319.55	319.58
10.750	319.62	319.66	319.70	319.74	319.78
11.000	319.82	319.87	319.91	319.96	320.02
11.250	320.07	320.14	320.20	320.27	320.35
11.500	320.43	320.51	320.61	320.73	320.87
11.750	321.05	321.27	321.52	321.81	322.12
12.000	322.48	322.87	323.26	323.58	323.80
12.250	323.91	323.92	323.86	323.77	323.65
12.500	323.51	323.37	323.23	323.08	322.96
12.750	322.86	322.77	322.70	322.63	322.58
13.000	322.53	322.49	322.45	322.42	322.39
13.250	322.36	322.34	322.33	322.31	322.30
13.500	322.28	322.27	322.26	322.26	322.25
13.750	322.24	322.23	322.22	322.21	322.20
14.000	322.19	322.18	322.17	322.16	322.15
14.250	322.14	322.13	322.12	322.11	322.10
14.500	322.10	322.09	322.08	322.07	322.07
14.750	322.06	322.05	322.05	322.04	322.03
15.000	322.03	322.02	322.01	322.01	322.00
15.250	321.99	321.99	321.98	321.98	321.97
15.500	321.96	321.96	321.95	321.95	321.94
15.750	321.93	321.93	321.92	321.92	321.91
16.000	321.90	321.90	321.89	321.89	321.88
16.250	321.87	321.87	321.86	321.86	321.86
16.500	321.85	321.85	321.84	321.84	321.84
16.750	321.83	321.83	321.83	321.82	321.82
17.000	321.82	321.81	321.81	321.81	321.81
17.250	321.80	321.80	321.80	321.79	321.79
17.500	321.79	321.79	321.78	321.78	321.78
17.750	321.77	321.77	321.77	321.77	321.76
18.000	321.76	321.76	321.76	321.75	321.75
18.250	321.75	321.75	321.74	321.74	321.74
18.500	321.74	321.73	321.73	321.73	321.72
18.750	321.72	321.72	321.72	321.71	321.71
19.000	321.71	321.70	321.70	321.70	321.69
19.250	321.69	321.69	321.69	321.68	321.68
19.500	321.68	321.67	321.67	321.67	321.66
19.750	321.66	321.66	321.65	321.65	321.64
20.000	321.64	321.64	321.63	321.63	321.62

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.62	321.62	321.61	321.61	321.60
20.500	321.60	321.59	321.59	321.59	321.58
20.750	321.58	321.57	321.57	321.56	321.56
21.000	321.55	321.55	321.54	321.54	321.53
21.250	321.52	321.52	321.51	321.51	321.50
21.500	321.50	321.49	321.49	321.48	321.47
21.750	321.47	321.46	321.46	321.45	321.44
22.000	321.44	321.43	321.42	321.42	321.41
22.250	321.41	321.40	321.39	321.39	321.38
22.500	321.37	321.37	321.36	321.35	321.34
22.750	321.34	321.33	321.32	321.32	321.31
23.000	321.30	321.29	321.29	321.28	321.27
23.250	321.26	321.26	321.25	321.24	321.23
23.500	321.22	321.21	321.21	321.20	321.19
23.750	321.18	321.17	321.16	321.15	321.15
24.000	321.14	321.13	321.12	321.10	321.08
24.250	321.06	321.04	321.02	321.00	320.98
24.500	320.96	320.94	320.92	320.90	320.88
24.750	320.86	320.84	320.82	320.80	320.78
25.000	320.76	320.73	320.71	320.69	320.67
25.250	320.65	320.63	320.61	320.59	320.56
25.500	320.54	320.52	320.50	320.48	320.46
25.750	320.44	320.42	320.40	320.38	320.36
26.000	320.34	320.32	320.30	320.28	320.26
26.250	320.23	320.21	320.19	320.17	320.15
26.500	320.13	320.11	320.09	320.06	320.04
26.750	320.02	320.00	319.98	319.96	319.94
27.000	319.92	319.90	319.88	319.86	319.84
27.250	319.82	319.80	319.78	319.76	319.73
27.500	319.71	319.69	319.67	319.65	319.63
27.750	319.61	319.59	319.56	319.54	319.52
28.000	319.50	319.48	319.46	319.44	319.42
28.250	319.40	319.38	319.36	319.34	319.32
28.500	319.30	319.28	319.26	319.24	319.21
28.750	319.19	319.18	319.16	319.14	319.12
29.000	319.11	319.09	319.08	319.06	319.05
29.250	319.04	319.03	319.01	319.00	318.99
29.500	318.98	318.97	318.96	318.95	318.94
29.750	318.94	318.93	318.92	318.91	318.91
30.000	318.90	318.89	318.89	318.88	318.88
30.250	318.87	318.87	318.86	318.86	318.85

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	318.85	318.84	318.84	318.84	318.83
30.750	318.83	318.82	318.82	318.82	318.82
31.000	318.81	318.81	318.81	318.81	318.80
31.250	318.80	318.80	318.80	318.79	318.79
31.500	318.79	318.79	318.79	318.79	318.78
31.750	318.78	318.78	318.78	318.78	318.78
32.000	318.78	318.78	318.77	318.77	318.77
32.250	318.77	318.77	318.77	318.77	318.77
32.500	318.77	318.77	318.77	318.76	318.76
32.750	318.76	318.76	318.76	318.76	318.76
33.000	318.76	318.76	318.76	318.76	318.76
33.250	318.76	318.76	318.76	318.76	318.76
33.500	318.76	318.76	318.76	318.76	318.76
33.750	318.76	318.76	318.76	318.76	318.75
34.000	318.75	318.75	318.75	318.75	318.75
34.250	318.75	318.75	318.75	318.75	318.75
34.500	318.75	318.75	318.75	318.75	318.75
34.750	318.75	318.75	318.75	318.75	318.75
35.000	318.75	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	318.75	318.75	318.75	318.75	318.75
0.250	318.75	318.75	318.75	318.75	318.75
0.500	318.75	318.75	318.75	318.75	318.75
0.750	318.75	318.75	318.75	318.75	318.75
1.000	318.75	318.75	318.75	318.75	318.75
1.250	318.75	318.75	318.75	318.75	318.75
1.500	318.75	318.75	318.75	318.75	318.75
1.750	318.75	318.75	318.75	318.75	318.75
2.000	318.75	318.75	318.75	318.75	318.75
2.250	318.75	318.75	318.75	318.75	318.75
2.500	318.75	318.75	318.75	318.75	318.75
2.750	318.75	318.75	318.75	318.75	318.75
3.000	318.75	318.75	318.75	318.75	318.75
3.250	318.75	318.75	318.75	318.75	318.75
3.500	318.75	318.75	318.75	318.75	318.75
3.750	318.75	318.75	318.75	318.75	318.75
4.000	318.75	318.75	318.75	318.75	318.75
4.250	318.75	318.75	318.75	318.75	318.75
4.500	318.75	318.75	318.75	318.75	318.75
4.750	318.75	318.75	318.75	318.75	318.75
5.000	318.75	318.75	318.75	318.75	318.75
5.250	318.75	318.75	318.75	318.75	318.76
5.500	318.76	318.76	318.76	318.76	318.77
5.750	318.77	318.77	318.77	318.78	318.78
6.000	318.78	318.79	318.79	318.79	318.80
6.250	318.80	318.80	318.81	318.81	318.82
6.500	318.82	318.83	318.83	318.84	318.84
6.750	318.85	318.85	318.86	318.86	318.87
7.000	318.88	318.88	318.89	318.90	318.90
7.250	318.91	318.92	318.93	318.93	318.94
7.500	318.95	318.96	318.97	318.98	318.99
7.750	318.99	319.00	319.01	319.02	319.03
8.000	319.04	319.05	319.06	319.08	319.09
8.250	319.10	319.11	319.12	319.14	319.15
8.500	319.16	319.18	319.19	319.21	319.23
8.750	319.24	319.26	319.28	319.29	319.31
9.000	319.33	319.35	319.38	319.40	319.43
9.250	319.45	319.48	319.51	319.54	319.57
9.500	319.60	319.64	319.67	319.71	319.75
9.750	319.78	319.82	319.86	319.90	319.94

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	319.98	320.03	320.07	320.12	320.17
10.250	320.22	320.27	320.32	320.37	320.43
10.500	320.49	320.55	320.61	320.67	320.74
10.750	320.81	320.87	320.94	321.01	321.09
11.000	321.16	321.24	321.32	321.40	321.48
11.250	321.57	321.67	321.77	321.86	321.96
11.500	322.04	322.13	322.23	322.33	322.45
11.750	322.58	322.74	322.91	323.09	323.30
12.000	323.58	323.96	324.41	324.82	325.11
12.250	325.23	325.22	325.13	325.00	324.83
12.500	324.64	324.43	324.20	323.96	323.72
12.750	323.50	323.31	323.16	323.03	322.93
13.000	322.84	322.78	322.72	322.67	322.62
13.250	322.59	322.55	322.53	322.50	322.48
13.500	322.46	322.45	322.43	322.42	322.41
13.750	322.40	322.38	322.37	322.36	322.35
14.000	322.35	322.34	322.33	322.32	322.31
14.250	322.30	322.30	322.29	322.28	322.28
14.500	322.27	322.27	322.26	322.26	322.25
14.750	322.25	322.25	322.24	322.23	322.23
15.000	322.22	322.22	322.21	322.20	322.19
15.250	322.19	322.18	322.17	322.16	322.16
15.500	322.15	322.14	322.13	322.12	322.12
15.750	322.11	322.10	322.09	322.08	322.07
16.000	322.06	322.06	322.05	322.04	322.03
16.250	322.02	322.02	322.01	322.00	322.00
16.500	321.99	321.98	321.98	321.97	321.97
16.750	321.96	321.96	321.95	321.95	321.94
17.000	321.94	321.93	321.93	321.93	321.92
17.250	321.92	321.91	321.91	321.90	321.90
17.500	321.90	321.89	321.89	321.88	321.88
17.750	321.88	321.87	321.87	321.86	321.86
18.000	321.86	321.85	321.85	321.84	321.84
18.250	321.84	321.83	321.83	321.83	321.83
18.500	321.82	321.82	321.82	321.82	321.81
18.750	321.81	321.81	321.81	321.81	321.81
19.000	321.80	321.80	321.80	321.80	321.80
19.250	321.80	321.80	321.79	321.79	321.79
19.500	321.79	321.79	321.79	321.79	321.79
19.750	321.78	321.78	321.78	321.78	321.78
20.000	321.78	321.78	321.78	321.77	321.77

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.77	321.77	321.77	321.77	321.77
20.500	321.77	321.77	321.77	321.76	321.76
20.750	321.76	321.76	321.76	321.76	321.76
21.000	321.76	321.76	321.76	321.76	321.75
21.250	321.75	321.75	321.75	321.75	321.75
21.500	321.75	321.75	321.75	321.75	321.74
21.750	321.74	321.74	321.74	321.74	321.74
22.000	321.73	321.73	321.73	321.73	321.73
22.250	321.72	321.72	321.72	321.72	321.72
22.500	321.71	321.71	321.71	321.71	321.70
22.750	321.70	321.70	321.69	321.69	321.69
23.000	321.69	321.68	321.68	321.68	321.67
23.250	321.67	321.67	321.66	321.66	321.66
23.500	321.66	321.65	321.65	321.64	321.64
23.750	321.64	321.63	321.63	321.63	321.62
24.000	321.62	321.61	321.60	321.59	321.57
24.250	321.55	321.53	321.51	321.49	321.47
24.500	321.45	321.43	321.41	321.39	321.37
24.750	321.35	321.33	321.31	321.29	321.27
25.000	321.25	321.22	321.20	321.18	321.16
25.250	321.14	321.12	321.10	321.07	321.05
25.500	321.03	321.01	320.99	320.97	320.95
25.750	320.93	320.91	320.89	320.87	320.85
26.000	320.83	320.81	320.79	320.77	320.75
26.250	320.72	320.70	320.68	320.66	320.64
26.500	320.62	320.60	320.57	320.55	320.53
26.750	320.51	320.49	320.47	320.45	320.43
27.000	320.41	320.39	320.37	320.35	320.33
27.250	320.31	320.29	320.27	320.25	320.22
27.500	320.20	320.18	320.16	320.14	320.12
27.750	320.10	320.07	320.05	320.03	320.01
28.000	319.99	319.97	319.95	319.93	319.91
28.250	319.89	319.87	319.85	319.83	319.81
28.500	319.79	319.77	319.75	319.72	319.70
28.750	319.68	319.66	319.64	319.62	319.60
29.000	319.57	319.55	319.53	319.51	319.49
29.250	319.47	319.45	319.43	319.41	319.39
29.500	319.37	319.35	319.33	319.31	319.29
29.750	319.27	319.25	319.23	319.20	319.19
30.000	319.17	319.15	319.13	319.12	319.10
30.250	319.09	319.07	319.06	319.04	319.03

Watershed

Subsection: Time vs. Elevation

Label: Infiltration Basin #3A (IN)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	319.02	319.01	319.00	318.99	318.98
30.750	318.97	318.96	318.95	318.94	318.93
31.000	318.92	318.92	318.91	318.90	318.90
31.250	318.89	318.88	318.88	318.87	318.87
31.500	318.86	318.86	318.85	318.85	318.85
31.750	318.84	318.84	318.83	318.83	318.83
32.000	318.82	318.82	318.82	318.81	318.81
32.250	318.81	318.81	318.80	318.80	318.80
32.500	318.80	318.80	318.79	318.79	318.79
32.750	318.79	318.79	318.79	318.78	318.78
33.000	318.78	318.78	318.78	318.78	318.78
33.250	318.77	318.77	318.77	318.77	318.77
33.500	318.77	318.77	318.77	318.77	318.77
33.750	318.77	318.77	318.76	318.76	318.76
34.000	318.76	318.76	318.76	318.76	318.76
34.250	318.76	318.76	318.76	318.76	318.76
34.500	318.76	318.76	318.76	318.76	318.76
34.750	318.76	318.76	318.76	318.76	318.76
35.000	318.76	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	320.00	320.00	320.00	320.00	320.00
0.250	320.00	320.00	320.00	320.00	320.00
0.500	320.00	320.00	320.00	320.00	320.00
0.750	320.00	320.00	320.00	320.00	320.00
1.000	320.00	320.00	320.00	320.00	320.00
1.250	320.00	320.00	320.00	320.00	320.00
1.500	320.00	320.00	320.00	320.00	320.00
1.750	320.00	320.00	320.00	320.00	320.00
2.000	320.00	320.00	320.00	320.00	320.00
2.250	320.00	320.00	320.00	320.00	320.00
2.500	320.00	320.00	320.00	320.00	320.00
2.750	320.00	320.00	320.00	320.00	320.00
3.000	320.00	320.00	320.00	320.00	320.00
3.250	320.00	320.00	320.00	320.00	320.00
3.500	320.00	320.00	320.00	320.00	320.00
3.750	320.00	320.00	320.00	320.00	320.00
4.000	320.00	320.00	320.00	320.00	320.00
4.250	320.00	320.00	320.00	320.00	320.00
4.500	320.00	320.00	320.00	320.00	320.00
4.750	320.00	320.00	320.00	320.00	320.00
5.000	320.00	320.00	320.00	320.00	320.00
5.250	320.00	320.00	320.00	320.00	320.00
5.500	320.00	320.00	320.00	320.00	320.00
5.750	320.00	320.00	320.00	320.00	320.00
6.000	320.00	320.00	320.00	320.00	320.00
6.250	320.00	320.00	320.00	320.00	320.00
6.500	320.00	320.00	320.00	320.00	320.00
6.750	320.00	320.00	320.00	320.00	320.00
7.000	320.00	320.00	320.00	320.00	320.00
7.250	320.00	320.00	320.00	320.00	320.00
7.500	320.00	320.00	320.00	320.00	320.00
7.750	320.00	320.00	320.00	320.00	320.00
8.000	320.00	320.00	320.00	320.00	320.00
8.250	320.00	320.00	320.00	320.00	320.00
8.500	320.00	320.00	320.00	320.00	320.00
8.750	320.00	320.00	320.00	320.00	320.00
9.000	320.00	320.00	320.00	320.00	320.00
9.250	320.00	320.00	320.00	320.00	320.00
9.500	320.00	320.00	320.00	320.00	320.00
9.750	320.00	320.00	320.00	320.00	320.00

Watershed

Subsection: Time vs. Elevation

Return Event: 1 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (2.8 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	320.00	320.00	320.00	320.00	320.00
10.250	320.00	320.00	320.00	320.00	320.00
10.500	320.00	320.00	320.00	320.00	320.00
10.750	320.00	320.00	320.00	320.00	320.00
11.000	320.00	320.00	320.00	320.00	320.00
11.250	320.00	320.00	320.00	320.01	320.01
11.500	320.01	320.01	320.02	320.02	320.03
11.750	320.03	320.04	320.06	320.07	320.10
12.000	320.13	320.18	320.24	320.31	320.39
12.250	320.45	320.50	320.55	320.59	320.63
12.500	320.66	320.68	320.71	320.72	320.74
12.750	320.76	320.77	320.78	320.80	320.81
13.000	320.82	320.83	320.84	320.85	320.86
13.250	320.87	320.88	320.89	320.90	320.91
13.500	320.92	320.93	320.94	320.95	320.95
13.750	320.96	320.97	320.98	320.99	321.00
14.000	321.00	321.01	321.02	321.02	321.03
14.250	321.04	321.04	321.05	321.06	321.06
14.500	321.07	321.08	321.08	321.09	321.10
14.750	321.10	321.11	321.11	321.12	321.12
15.000	321.13	321.14	321.14	321.15	321.15
15.250	321.16	321.16	321.17	321.17	321.18
15.500	321.18	321.19	321.19	321.20	321.20
15.750	321.21	321.21	321.22	321.22	321.22
16.000	321.23	321.23	321.24	321.24	321.24
16.250	321.25	321.25	321.26	321.26	321.26
16.500	321.27	321.27	321.27	321.28	321.28
16.750	321.28	321.29	321.29	321.29	321.30
17.000	321.30	321.30	321.31	321.31	321.31
17.250	321.32	321.32	321.32	321.33	321.33
17.500	321.33	321.33	321.34	321.34	321.34
17.750	321.35	321.35	321.35	321.35	321.36
18.000	321.36	321.36	321.36	321.37	321.37
18.250	321.37	321.37	321.38	321.38	321.38
18.500	321.38	321.39	321.39	321.39	321.39
18.750	321.40	321.40	321.40	321.40	321.41
19.000	321.41	321.41	321.41	321.41	321.42
19.250	321.42	321.42	321.42	321.43	321.43
19.500	321.43	321.43	321.43	321.44	321.44
19.750	321.44	321.44	321.44	321.45	321.45
20.000	321.45	321.45	321.46	321.46	321.46

Watershed

Subsection: Time vs. Elevation

Return Event: 1 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (2.8 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.46	321.46	321.47	321.47	321.47
20.500	321.47	321.47	321.48	321.48	321.48
20.750	321.48	321.48	321.49	321.49	321.49
21.000	321.49	321.49	321.49	321.50	321.50
21.250	321.50	321.50	321.50	321.51	321.51
21.500	321.51	321.51	321.51	321.51	321.52
21.750	321.52	321.52	321.52	321.52	321.52
22.000	321.53	321.53	321.53	321.53	321.53
22.250	321.54	321.54	321.54	321.54	321.54
22.500	321.54	321.54	321.55	321.55	321.55
22.750	321.55	321.55	321.55	321.56	321.56
23.000	321.56	321.56	321.56	321.56	321.57
23.250	321.57	321.57	321.57	321.57	321.57
23.500	321.57	321.58	321.58	321.58	321.58
23.750	321.58	321.58	321.58	321.59	321.59
24.000	321.59	321.59	321.59	321.59	321.59
24.250	321.59	321.59	321.59	321.59	321.59
24.500	321.59	321.59	321.59	321.59	321.59
24.750	321.59	321.59	321.59	321.59	321.59
25.000	321.59	321.59	321.59	321.59	321.59
25.250	321.59	321.59	321.59	321.59	321.59
25.500	321.59	321.59	321.59	321.59	321.59
25.750	321.59	321.59	321.59	321.59	321.59
26.000	321.59	321.59	321.59	321.59	321.59
26.250	321.59	321.59	321.59	321.59	321.59
26.500	321.59	321.59	321.59	321.59	321.59
26.750	321.59	321.59	321.59	321.59	321.59
27.000	321.59	321.59	321.59	321.59	321.59
27.250	321.59	321.59	321.59	321.59	321.59
27.500	321.59	321.59	321.59	321.59	321.59
27.750	321.59	321.59	321.59	321.59	321.59
28.000	321.59	321.59	321.59	321.59	321.59
28.250	321.59	321.59	321.59	321.59	321.59
28.500	321.59	321.59	321.59	321.59	321.59
28.750	321.59	321.59	321.59	321.59	321.59
29.000	321.59	321.59	321.59	321.59	321.59
29.250	321.59	321.59	321.59	321.59	321.59
29.500	321.59	321.59	321.59	321.59	321.59
29.750	321.59	321.59	321.59	321.59	321.59
30.000	321.59	321.59	321.59	321.59	321.59
30.250	321.59	321.59	321.59	321.59	321.59

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	321.59	321.59	321.59	321.59	321.59
30.750	321.59	321.59	321.59	321.59	321.59
31.000	321.59	321.59	321.59	321.59	321.59
31.250	321.59	321.59	321.59	321.59	321.59
31.500	321.59	321.59	321.59	321.59	321.59
31.750	321.59	321.59	321.59	321.59	321.59
32.000	321.59	321.59	321.59	321.59	321.59
32.250	321.59	321.59	321.59	321.59	321.59
32.500	321.59	321.59	321.59	321.59	321.59
32.750	321.59	321.59	321.59	321.59	321.59
33.000	321.59	321.59	321.59	321.59	321.59
33.250	321.59	321.59	321.59	321.59	321.59
33.500	321.59	321.59	321.59	321.59	321.59
33.750	321.59	321.59	321.59	321.59	321.59
34.000	321.59	321.59	321.59	321.59	321.59
34.250	321.59	321.59	321.59	321.59	321.59
34.500	321.59	321.59	321.59	321.59	321.59
34.750	321.59	321.59	321.59	321.59	321.59
35.000	321.59	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	320.00	320.00	320.00	320.00	320.00
0.250	320.00	320.00	320.00	320.00	320.00
0.500	320.00	320.00	320.00	320.00	320.00
0.750	320.00	320.00	320.00	320.00	320.00
1.000	320.00	320.00	320.00	320.00	320.00
1.250	320.00	320.00	320.00	320.00	320.00
1.500	320.00	320.00	320.00	320.00	320.00
1.750	320.00	320.00	320.00	320.00	320.00
2.000	320.00	320.00	320.00	320.00	320.00
2.250	320.00	320.00	320.00	320.00	320.00
2.500	320.00	320.00	320.00	320.00	320.00
2.750	320.00	320.00	320.00	320.00	320.00
3.000	320.00	320.00	320.00	320.00	320.00
3.250	320.00	320.00	320.00	320.00	320.00
3.500	320.00	320.00	320.00	320.00	320.00
3.750	320.00	320.00	320.00	320.00	320.00
4.000	320.00	320.00	320.00	320.00	320.00
4.250	320.00	320.00	320.00	320.00	320.00
4.500	320.00	320.00	320.00	320.00	320.00
4.750	320.00	320.00	320.00	320.00	320.00
5.000	320.00	320.00	320.00	320.00	320.00
5.250	320.00	320.00	320.00	320.00	320.00
5.500	320.00	320.00	320.00	320.00	320.00
5.750	320.00	320.00	320.00	320.00	320.00
6.000	320.00	320.00	320.00	320.00	320.00
6.250	320.00	320.00	320.00	320.00	320.00
6.500	320.00	320.00	320.00	320.00	320.00
6.750	320.00	320.00	320.00	320.00	320.00
7.000	320.00	320.00	320.00	320.00	320.00
7.250	320.00	320.00	320.00	320.00	320.00
7.500	320.00	320.00	320.00	320.00	320.00
7.750	320.00	320.00	320.00	320.00	320.00
8.000	320.00	320.00	320.00	320.00	320.00
8.250	320.00	320.00	320.00	320.00	320.00
8.500	320.00	320.00	320.00	320.00	320.00
8.750	320.00	320.00	320.00	320.00	320.00
9.000	320.00	320.00	320.00	320.00	320.00
9.250	320.00	320.00	320.00	320.00	320.00
9.500	320.00	320.00	320.00	320.00	320.00
9.750	320.00	320.00	320.00	320.00	320.00

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	320.00	320.00	320.00	320.00	320.00
10.250	320.00	320.00	320.00	320.00	320.00
10.500	320.00	320.00	320.00	320.00	320.01
10.750	320.01	320.01	320.01	320.01	320.01
11.000	320.01	320.02	320.02	320.02	320.03
11.250	320.03	320.03	320.04	320.04	320.05
11.500	320.05	320.06	320.07	320.08	320.09
11.750	320.10	320.12	320.15	320.18	320.22
12.000	320.28	320.36	320.46	320.57	320.67
12.250	320.76	320.84	320.91	320.97	321.02
12.500	321.06	321.10	321.13	321.16	321.18
12.750	321.20	321.22	321.24	321.26	321.27
13.000	321.29	321.30	321.32	321.33	321.35
13.250	321.36	321.37	321.39	321.40	321.41
13.500	321.42	321.44	321.45	321.46	321.47
13.750	321.48	321.49	321.50	321.51	321.53
14.000	321.54	321.54	321.55	321.56	321.57
14.250	321.58	321.59	321.60	321.61	321.62
14.500	321.62	321.63	321.64	321.65	321.65
14.750	321.66	321.66	321.67	321.67	321.68
15.000	321.68	321.69	321.69	321.69	321.70
15.250	321.70	321.70	321.70	321.71	321.71
15.500	321.71	321.71	321.71	321.72	321.72
15.750	321.72	321.72	321.72	321.72	321.72
16.000	321.72	321.72	321.72	321.72	321.72
16.250	321.72	321.72	321.72	321.72	321.72
16.500	321.72	321.72	321.72	321.72	321.72
16.750	321.72	321.72	321.72	321.72	321.72
17.000	321.72	321.72	321.72	321.72	321.71
17.250	321.71	321.71	321.71	321.71	321.71
17.500	321.71	321.71	321.71	321.71	321.71
17.750	321.71	321.71	321.70	321.70	321.70
18.000	321.70	321.70	321.70	321.70	321.70
18.250	321.70	321.70	321.70	321.70	321.69
18.500	321.69	321.69	321.69	321.69	321.69
18.750	321.69	321.69	321.69	321.69	321.69
19.000	321.69	321.69	321.69	321.68	321.68
19.250	321.68	321.68	321.68	321.68	321.68
19.500	321.68	321.68	321.68	321.68	321.68
19.750	321.68	321.68	321.68	321.68	321.68
20.000	321.68	321.68	321.68	321.67	321.67

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.67	321.67	321.67	321.67	321.67
20.500	321.67	321.67	321.67	321.67	321.67
20.750	321.67	321.67	321.67	321.67	321.67
21.000	321.67	321.67	321.67	321.67	321.67
21.250	321.67	321.67	321.67	321.67	321.66
21.500	321.66	321.66	321.66	321.66	321.66
21.750	321.66	321.66	321.66	321.66	321.66
22.000	321.66	321.66	321.66	321.66	321.66
22.250	321.66	321.66	321.66	321.66	321.66
22.500	321.66	321.66	321.66	321.66	321.66
22.750	321.66	321.66	321.66	321.66	321.66
23.000	321.66	321.66	321.66	321.65	321.65
23.250	321.65	321.65	321.65	321.65	321.65
23.500	321.65	321.65	321.65	321.65	321.65
23.750	321.65	321.65	321.65	321.65	321.65
24.000	321.65	321.65	321.65	321.65	321.65
24.250	321.64	321.64	321.64	321.64	321.64
24.500	321.64	321.63	321.63	321.63	321.63
24.750	321.63	321.63	321.63	321.63	321.62
25.000	321.62	321.62	321.62	321.62	321.62
25.250	321.62	321.62	321.62	321.62	321.62
25.500	321.62	321.62	321.61	321.61	321.61
25.750	321.61	321.61	321.61	321.61	321.61
26.000	321.61	321.61	321.61	321.61	321.61
26.250	321.61	321.61	321.61	321.61	321.61
26.500	321.61	321.61	321.61	321.61	321.61
26.750	321.61	321.61	321.61	321.60	321.60
27.000	321.60	321.60	321.60	321.60	321.60
27.250	321.60	321.60	321.60	321.60	321.60
27.500	321.60	321.60	321.60	321.60	321.60
27.750	321.60	321.60	321.60	321.60	321.60
28.000	321.60	321.60	321.60	321.60	321.60
28.250	321.60	321.60	321.60	321.60	321.60
28.500	321.60	321.60	321.60	321.60	321.60
28.750	321.60	321.60	321.60	321.60	321.60
29.000	321.60	321.60	321.60	321.60	321.60
29.250	321.60	321.60	321.60	321.60	321.60
29.500	321.60	321.60	321.60	321.60	321.60
29.750	321.60	321.60	321.60	321.60	321.60
30.000	321.60	321.60	321.60	321.60	321.60
30.250	321.60	321.60	321.60	321.60	321.60

Watershed

Subsection: Time vs. Elevation

Return Event: 2 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (3.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	321.60	321.60	321.60	321.60	321.60
30.750	321.60	321.60	321.60	321.60	321.60
31.000	321.60	321.60	321.60	321.60	321.60
31.250	321.60	321.60	321.60	321.60	321.60
31.500	321.60	321.60	321.60	321.60	321.60
31.750	321.60	321.60	321.60	321.60	321.60
32.000	321.60	321.60	321.60	321.60	321.60
32.250	321.60	321.60	321.60	321.60	321.60
32.500	321.60	321.60	321.60	321.60	321.60
32.750	321.60	321.60	321.60	321.60	321.60
33.000	321.60	321.60	321.60	321.60	321.60
33.250	321.60	321.60	321.60	321.60	321.60
33.500	321.60	321.60	321.60	321.60	321.60
33.750	321.60	321.60	321.60	321.60	321.60
34.000	321.60	321.60	321.60	321.60	321.60
34.250	321.60	321.60	321.60	321.60	321.60
34.500	321.60	321.60	321.60	321.60	321.60
34.750	321.60	321.60	321.60	321.60	321.60
35.000	321.60	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	320.00	320.00	320.00	320.00	320.00
0.250	320.00	320.00	320.00	320.00	320.00
0.500	320.00	320.00	320.00	320.00	320.00
0.750	320.00	320.00	320.00	320.00	320.00
1.000	320.00	320.00	320.00	320.00	320.00
1.250	320.00	320.00	320.00	320.00	320.00
1.500	320.00	320.00	320.00	320.00	320.00
1.750	320.00	320.00	320.00	320.00	320.00
2.000	320.00	320.00	320.00	320.00	320.00
2.250	320.00	320.00	320.00	320.00	320.00
2.500	320.00	320.00	320.00	320.00	320.00
2.750	320.00	320.00	320.00	320.00	320.00
3.000	320.00	320.00	320.00	320.00	320.00
3.250	320.00	320.00	320.00	320.00	320.00
3.500	320.00	320.00	320.00	320.00	320.00
3.750	320.00	320.00	320.00	320.00	320.00
4.000	320.00	320.00	320.00	320.00	320.00
4.250	320.00	320.00	320.00	320.00	320.00
4.500	320.00	320.00	320.00	320.00	320.00
4.750	320.00	320.00	320.00	320.00	320.00
5.000	320.00	320.00	320.00	320.00	320.00
5.250	320.00	320.00	320.00	320.00	320.00
5.500	320.00	320.00	320.00	320.00	320.00
5.750	320.00	320.00	320.00	320.00	320.00
6.000	320.00	320.00	320.00	320.00	320.00
6.250	320.00	320.00	320.00	320.00	320.00
6.500	320.00	320.00	320.00	320.00	320.00
6.750	320.00	320.00	320.00	320.00	320.00
7.000	320.00	320.00	320.00	320.00	320.00
7.250	320.00	320.00	320.00	320.00	320.00
7.500	320.00	320.00	320.00	320.00	320.00
7.750	320.00	320.00	320.00	320.00	320.00
8.000	320.00	320.00	320.00	320.00	320.00
8.250	320.00	320.00	320.00	320.00	320.00
8.500	320.00	320.00	320.00	320.00	320.00
8.750	320.00	320.00	320.00	320.00	320.00
9.000	320.00	320.00	320.00	320.00	320.00
9.250	320.01	320.01	320.01	320.01	320.01
9.500	320.01	320.01	320.02	320.02	320.02
9.750	320.02	320.03	320.03	320.03	320.04

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	320.04	320.04	320.05	320.05	320.05
10.250	320.06	320.06	320.07	320.07	320.08
10.500	320.08	320.09	320.10	320.10	320.11
10.750	320.12	320.13	320.13	320.14	320.15
11.000	320.16	320.17	320.18	320.19	320.20
11.250	320.22	320.23	320.24	320.26	320.28
11.500	320.29	320.31	320.34	320.36	320.40
11.750	320.44	320.49	320.56	320.63	320.72
12.000	320.85	321.02	321.21	321.43	321.63
12.250	321.79	321.92	322.03	322.10	322.16
12.500	322.20	322.22	322.24	322.24	322.23
12.750	322.23	322.22	322.21	322.20	322.19
13.000	322.18	322.17	322.16	322.15	322.14
13.250	322.13	322.12	322.12	322.11	322.10
13.500	322.09	322.09	322.08	322.08	322.07
13.750	322.07	322.06	322.06	322.05	322.05
14.000	322.04	322.04	322.04	322.03	322.03
14.250	322.03	322.02	322.02	322.02	322.01
14.500	322.01	322.01	322.01	322.00	322.00
14.750	322.00	322.00	321.99	321.99	321.99
15.000	321.99	321.98	321.98	321.98	321.97
15.250	321.97	321.97	321.97	321.96	321.96
15.500	321.96	321.95	321.95	321.95	321.94
15.750	321.94	321.93	321.93	321.93	321.92
16.000	321.92	321.92	321.91	321.91	321.90
16.250	321.90	321.90	321.89	321.89	321.89
16.500	321.88	321.88	321.88	321.87	321.87
16.750	321.87	321.86	321.86	321.86	321.85
17.000	321.85	321.85	321.85	321.84	321.84
17.250	321.84	321.84	321.83	321.83	321.83
17.500	321.82	321.82	321.82	321.82	321.81
17.750	321.81	321.81	321.81	321.80	321.80
18.000	321.80	321.80	321.79	321.79	321.79
18.250	321.79	321.79	321.78	321.78	321.78
18.500	321.78	321.78	321.77	321.77	321.77
18.750	321.77	321.77	321.77	321.76	321.76
19.000	321.76	321.76	321.76	321.76	321.75
19.250	321.75	321.75	321.75	321.75	321.75
19.500	321.75	321.75	321.75	321.74	321.74
19.750	321.74	321.74	321.74	321.74	321.74
20.000	321.74	321.74	321.73	321.73	321.73

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.73	321.73	321.73	321.73	321.73
20.500	321.73	321.73	321.73	321.73	321.72
20.750	321.72	321.72	321.72	321.72	321.72
21.000	321.72	321.72	321.72	321.72	321.72
21.250	321.72	321.72	321.72	321.71	321.71
21.500	321.71	321.71	321.71	321.71	321.71
21.750	321.71	321.71	321.71	321.71	321.71
22.000	321.71	321.71	321.71	321.71	321.71
22.250	321.70	321.70	321.70	321.70	321.70
22.500	321.70	321.70	321.70	321.70	321.70
22.750	321.70	321.70	321.70	321.70	321.70
23.000	321.70	321.70	321.70	321.70	321.69
23.250	321.69	321.69	321.69	321.69	321.69
23.500	321.69	321.69	321.69	321.69	321.69
23.750	321.69	321.69	321.69	321.69	321.69
24.000	321.69	321.69	321.68	321.68	321.68
24.250	321.68	321.67	321.67	321.67	321.67
24.500	321.66	321.66	321.66	321.66	321.65
24.750	321.65	321.65	321.65	321.64	321.64
25.000	321.64	321.64	321.64	321.64	321.63
25.250	321.63	321.63	321.63	321.63	321.63
25.500	321.63	321.63	321.63	321.62	321.62
25.750	321.62	321.62	321.62	321.62	321.62
26.000	321.62	321.62	321.62	321.62	321.62
26.250	321.61	321.61	321.61	321.61	321.61
26.500	321.61	321.61	321.61	321.61	321.61
26.750	321.61	321.61	321.61	321.61	321.61
27.000	321.61	321.61	321.61	321.61	321.61
27.250	321.61	321.61	321.61	321.61	321.61
27.500	321.61	321.60	321.60	321.60	321.60
27.750	321.60	321.60	321.60	321.60	321.60
28.000	321.60	321.60	321.60	321.60	321.60
28.250	321.60	321.60	321.60	321.60	321.60
28.500	321.60	321.60	321.60	321.60	321.60
28.750	321.60	321.60	321.60	321.60	321.60
29.000	321.60	321.60	321.60	321.60	321.60
29.250	321.60	321.60	321.60	321.60	321.60
29.500	321.60	321.60	321.60	321.60	321.60
29.750	321.60	321.60	321.60	321.60	321.60
30.000	321.60	321.60	321.60	321.60	321.60
30.250	321.60	321.60	321.60	321.60	321.60

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	321.60	321.60	321.60	321.60	321.60
30.750	321.60	321.60	321.60	321.60	321.60
31.000	321.60	321.60	321.60	321.60	321.60
31.250	321.60	321.60	321.60	321.60	321.60
31.500	321.60	321.60	321.60	321.60	321.60
31.750	321.60	321.60	321.60	321.60	321.60
32.000	321.60	321.60	321.60	321.60	321.60
32.250	321.60	321.60	321.60	321.60	321.60
32.500	321.60	321.60	321.60	321.60	321.60
32.750	321.60	321.60	321.60	321.60	321.60
33.000	321.60	321.60	321.60	321.60	321.60
33.250	321.60	321.60	321.60	321.60	321.60
33.500	321.60	321.60	321.60	321.60	321.60
33.750	321.60	321.60	321.60	321.60	321.60
34.000	321.60	321.60	321.60	321.60	321.60
34.250	321.60	321.60	321.60	321.60	321.60
34.500	321.60	321.60	321.60	321.60	321.60
34.750	321.60	321.60	321.60	321.60	321.60
35.000	321.60	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	320.00	320.00	320.00	320.00	320.00
0.250	320.00	320.00	320.00	320.00	320.00
0.500	320.00	320.00	320.00	320.00	320.00
0.750	320.00	320.00	320.00	320.00	320.00
1.000	320.00	320.00	320.00	320.00	320.00
1.250	320.00	320.00	320.00	320.00	320.00
1.500	320.00	320.00	320.00	320.00	320.00
1.750	320.00	320.00	320.00	320.00	320.00
2.000	320.00	320.00	320.00	320.00	320.00
2.250	320.00	320.00	320.00	320.00	320.00
2.500	320.00	320.00	320.00	320.00	320.00
2.750	320.00	320.00	320.00	320.00	320.00
3.000	320.00	320.00	320.00	320.00	320.00
3.250	320.00	320.00	320.00	320.00	320.00
3.500	320.00	320.00	320.00	320.00	320.00
3.750	320.00	320.00	320.00	320.00	320.00
4.000	320.00	320.00	320.00	320.00	320.00
4.250	320.00	320.00	320.00	320.00	320.00
4.500	320.00	320.00	320.00	320.00	320.00
4.750	320.00	320.00	320.00	320.00	320.00
5.000	320.00	320.00	320.00	320.00	320.00
5.250	320.00	320.00	320.00	320.00	320.00
5.500	320.00	320.00	320.00	320.00	320.00
5.750	320.00	320.00	320.00	320.00	320.00
6.000	320.00	320.00	320.00	320.00	320.00
6.250	320.00	320.00	320.00	320.00	320.00
6.500	320.00	320.00	320.00	320.00	320.00
6.750	320.00	320.00	320.00	320.00	320.00
7.000	320.00	320.00	320.00	320.00	320.00
7.250	320.00	320.00	320.00	320.00	320.00
7.500	320.00	320.00	320.00	320.00	320.00
7.750	320.00	320.00	320.00	320.00	320.00
8.000	320.00	320.00	320.00	320.00	320.00
8.250	320.00	320.00	320.01	320.01	320.01
8.500	320.01	320.01	320.01	320.01	320.02
8.750	320.02	320.02	320.02	320.03	320.03
9.000	320.03	320.03	320.04	320.04	320.04
9.250	320.05	320.05	320.06	320.06	320.06
9.500	320.07	320.07	320.08	320.08	320.09
9.750	320.10	320.10	320.11	320.12	320.12

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	320.13	320.14	320.15	320.15	320.16
10.250	320.17	320.18	320.19	320.20	320.21
10.500	320.22	320.23	320.24	320.26	320.27
10.750	320.28	320.30	320.31	320.33	320.34
11.000	320.36	320.38	320.39	320.41	320.43
11.250	320.45	320.48	320.50	320.52	320.55
11.500	320.58	320.61	320.65	320.69	320.74
11.750	320.80	320.88	320.98	321.09	321.22
12.000	321.40	321.63	321.90	322.16	322.38
12.250	322.54	322.65	322.73	322.79	322.83
12.500	322.85	322.85	322.83	322.81	322.78
12.750	322.75	322.71	322.67	322.64	322.60
13.000	322.56	322.53	322.49	322.45	322.42
13.250	322.39	322.36	322.33	322.31	322.29
13.500	322.27	322.25	322.23	322.22	322.20
13.750	322.19	322.18	322.17	322.16	322.15
14.000	322.14	322.13	322.12	322.11	322.11
14.250	322.10	322.09	322.09	322.08	322.08
14.500	322.07	322.07	322.06	322.06	322.06
14.750	322.05	322.05	322.05	322.04	322.04
15.000	322.04	322.03	322.03	322.03	322.03
15.250	322.02	322.02	322.02	322.02	322.01
15.500	322.01	322.01	322.01	322.00	322.00
15.750	322.00	322.00	321.99	321.99	321.99
16.000	321.98	321.98	321.98	321.97	321.97
16.250	321.97	321.96	321.96	321.96	321.95
16.500	321.95	321.95	321.94	321.94	321.94
16.750	321.93	321.93	321.93	321.92	321.92
17.000	321.92	321.92	321.91	321.91	321.91
17.250	321.90	321.90	321.90	321.89	321.89
17.500	321.89	321.89	321.88	321.88	321.88
17.750	321.87	321.87	321.87	321.86	321.86
18.000	321.86	321.86	321.85	321.85	321.85
18.250	321.84	321.84	321.84	321.84	321.83
18.500	321.83	321.83	321.83	321.82	321.82
18.750	321.82	321.82	321.82	321.81	321.81
19.000	321.81	321.81	321.81	321.81	321.80
19.250	321.80	321.80	321.80	321.80	321.80
19.500	321.79	321.79	321.79	321.79	321.79
19.750	321.79	321.79	321.78	321.78	321.78
20.000	321.78	321.78	321.78	321.78	321.78

Watershed

Subsection: Time vs. Elevation

Return Event: 25 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (6.4 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.77	321.77	321.77	321.77	321.77
20.500	321.77	321.77	321.77	321.77	321.77
20.750	321.76	321.76	321.76	321.76	321.76
21.000	321.76	321.76	321.76	321.76	321.76
21.250	321.76	321.75	321.75	321.75	321.75
21.500	321.75	321.75	321.75	321.75	321.75
21.750	321.75	321.75	321.75	321.74	321.74
22.000	321.74	321.74	321.74	321.74	321.74
22.250	321.74	321.74	321.74	321.74	321.74
22.500	321.74	321.74	321.73	321.73	321.73
22.750	321.73	321.73	321.73	321.73	321.73
23.000	321.73	321.73	321.73	321.73	321.73
23.250	321.73	321.72	321.72	321.72	321.72
23.500	321.72	321.72	321.72	321.72	321.72
23.750	321.72	321.72	321.72	321.72	321.72
24.000	321.72	321.71	321.71	321.71	321.71
24.250	321.70	321.70	321.69	321.69	321.69
24.500	321.68	321.68	321.68	321.67	321.67
24.750	321.67	321.66	321.66	321.66	321.66
25.000	321.65	321.65	321.65	321.65	321.65
25.250	321.64	321.64	321.64	321.64	321.64
25.500	321.64	321.63	321.63	321.63	321.63
25.750	321.63	321.63	321.63	321.63	321.62
26.000	321.62	321.62	321.62	321.62	321.62
26.250	321.62	321.62	321.62	321.62	321.62
26.500	321.62	321.62	321.61	321.61	321.61
26.750	321.61	321.61	321.61	321.61	321.61
27.000	321.61	321.61	321.61	321.61	321.61
27.250	321.61	321.61	321.61	321.61	321.61
27.500	321.61	321.61	321.61	321.61	321.61
27.750	321.61	321.61	321.61	321.60	321.60
28.000	321.60	321.60	321.60	321.60	321.60
28.250	321.60	321.60	321.60	321.60	321.60
28.500	321.60	321.60	321.60	321.60	321.60
28.750	321.60	321.60	321.60	321.60	321.60
29.000	321.60	321.60	321.60	321.60	321.60
29.250	321.60	321.60	321.60	321.60	321.60
29.500	321.60	321.60	321.60	321.60	321.60
29.750	321.60	321.60	321.60	321.60	321.60
30.000	321.60	321.60	321.60	321.60	321.60
30.250	321.60	321.60	321.60	321.60	321.60

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	321.60	321.60	321.60	321.60	321.60
30.750	321.60	321.60	321.60	321.60	321.60
31.000	321.60	321.60	321.60	321.60	321.60
31.250	321.60	321.60	321.60	321.60	321.60
31.500	321.60	321.60	321.60	321.60	321.60
31.750	321.60	321.60	321.60	321.60	321.60
32.000	321.60	321.60	321.60	321.60	321.60
32.250	321.60	321.60	321.60	321.60	321.60
32.500	321.60	321.60	321.60	321.60	321.60
32.750	321.60	321.60	321.60	321.60	321.60
33.000	321.60	321.60	321.60	321.60	321.60
33.250	321.60	321.60	321.60	321.60	321.60
33.500	321.60	321.60	321.60	321.60	321.60
33.750	321.60	321.60	321.60	321.60	321.60
34.000	321.60	321.60	321.60	321.60	321.60
34.250	321.60	321.60	321.60	321.60	321.60
34.500	321.60	321.60	321.60	321.60	321.60
34.750	321.60	321.60	321.60	321.60	321.60
35.000	321.60	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Elevation

Return Event: 100 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (9.0 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	320.00	320.00	320.00	320.00	320.00
0.250	320.00	320.00	320.00	320.00	320.00
0.500	320.00	320.00	320.00	320.00	320.00
0.750	320.00	320.00	320.00	320.00	320.00
1.000	320.00	320.00	320.00	320.00	320.00
1.250	320.00	320.00	320.00	320.00	320.00
1.500	320.00	320.00	320.00	320.00	320.00
1.750	320.00	320.00	320.00	320.00	320.00
2.000	320.00	320.00	320.00	320.00	320.00
2.250	320.00	320.00	320.00	320.00	320.00
2.500	320.00	320.00	320.00	320.00	320.00
2.750	320.00	320.00	320.00	320.00	320.00
3.000	320.00	320.00	320.00	320.00	320.00
3.250	320.00	320.00	320.00	320.00	320.00
3.500	320.00	320.00	320.00	320.00	320.00
3.750	320.00	320.00	320.00	320.00	320.00
4.000	320.00	320.00	320.00	320.00	320.00
4.250	320.00	320.00	320.00	320.00	320.00
4.500	320.00	320.00	320.00	320.00	320.00
4.750	320.00	320.00	320.00	320.00	320.00
5.000	320.00	320.00	320.00	320.00	320.00
5.250	320.00	320.00	320.00	320.00	320.00
5.500	320.00	320.00	320.00	320.00	320.00
5.750	320.00	320.00	320.00	320.00	320.00
6.000	320.00	320.00	320.00	320.00	320.00
6.250	320.00	320.00	320.00	320.00	320.00
6.500	320.00	320.00	320.00	320.00	320.00
6.750	320.00	320.00	320.01	320.01	320.01
7.000	320.01	320.01	320.01	320.01	320.01
7.250	320.02	320.02	320.02	320.02	320.02
7.500	320.03	320.03	320.03	320.04	320.04
7.750	320.04	320.04	320.05	320.05	320.05
8.000	320.06	320.06	320.07	320.07	320.08
8.250	320.08	320.09	320.09	320.10	320.10
8.500	320.11	320.11	320.12	320.13	320.13
8.750	320.14	320.15	320.16	320.17	320.17
9.000	320.18	320.19	320.20	320.21	320.22
9.250	320.23	320.24	320.25	320.27	320.28
9.500	320.29	320.30	320.32	320.33	320.35
9.750	320.36	320.38	320.39	320.41	320.42

Watershed

Subsection: Time vs. Elevation

Return Event: 100 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (9.0 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	320.44	320.46	320.47	320.49	320.51
10.250	320.53	320.55	320.57	320.59	320.61
10.500	320.64	320.66	320.68	320.71	320.73
10.750	320.76	320.79	320.82	320.85	320.88
11.000	320.91	320.94	320.97	321.01	321.04
11.250	321.08	321.12	321.16	321.21	321.25
11.500	321.31	321.36	321.42	321.50	321.58
11.750	321.69	321.81	321.95	322.11	322.29
12.000	322.50	322.78	323.11	323.45	323.74
12.250	323.97	324.13	324.25	324.33	324.39
12.500	324.43	324.44	324.43	324.41	324.38
12.750	324.35	324.31	324.27	324.22	324.18
13.000	324.13	324.08	324.03	323.98	323.92
13.250	323.86	323.81	323.75	323.70	323.64
13.500	323.59	323.53	323.48	323.42	323.37
13.750	323.31	323.26	323.20	323.15	323.10
14.000	323.04	322.99	322.93	322.88	322.82
14.250	322.77	322.72	322.68	322.63	322.59
14.500	322.54	322.50	322.46	322.42	322.39
14.750	322.36	322.33	322.31	322.29	322.27
15.000	322.25	322.23	322.21	322.20	322.19
15.250	322.18	322.16	322.15	322.14	322.14
15.500	322.13	322.12	322.11	322.11	322.10
15.750	322.09	322.09	322.08	322.08	322.07
16.000	322.07	322.06	322.06	322.05	322.05
16.250	322.04	322.04	322.04	322.03	322.03
16.500	322.03	322.03	322.02	322.02	322.02
16.750	322.02	322.01	322.01	322.01	322.01
17.000	322.01	322.00	322.00	322.00	322.00
17.250	322.00	321.99	321.99	321.99	321.99
17.500	321.98	321.98	321.98	321.98	321.97
17.750	321.97	321.97	321.97	321.96	321.96
18.000	321.96	321.95	321.95	321.95	321.94
18.250	321.94	321.94	321.94	321.93	321.93
18.500	321.93	321.92	321.92	321.92	321.92
18.750	321.92	321.91	321.91	321.91	321.91
19.000	321.90	321.90	321.90	321.90	321.90
19.250	321.89	321.89	321.89	321.89	321.89
19.500	321.88	321.88	321.88	321.88	321.88
19.750	321.87	321.87	321.87	321.87	321.87
20.000	321.87	321.86	321.86	321.86	321.86

Watershed

Subsection: Time vs. Elevation

Return Event: 100 years

Label: Wet Extended Detention Pond #2A (OUT)

Storm Event: TypeIII 24hr (9.0 in)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	321.86	321.86	321.85	321.85	321.85
20.500	321.85	321.85	321.85	321.85	321.84
20.750	321.84	321.84	321.84	321.84	321.84
21.000	321.84	321.84	321.83	321.83	321.83
21.250	321.83	321.83	321.83	321.83	321.83
21.500	321.82	321.82	321.82	321.82	321.82
21.750	321.82	321.82	321.82	321.82	321.81
22.000	321.81	321.81	321.81	321.81	321.81
22.250	321.81	321.81	321.81	321.81	321.80
22.500	321.80	321.80	321.80	321.80	321.80
22.750	321.80	321.80	321.80	321.79	321.79
23.000	321.79	321.79	321.79	321.79	321.79
23.250	321.79	321.79	321.79	321.78	321.78
23.500	321.78	321.78	321.78	321.78	321.78
23.750	321.78	321.78	321.78	321.77	321.77
24.000	321.77	321.77	321.77	321.76	321.76
24.250	321.75	321.75	321.74	321.74	321.73
24.500	321.72	321.72	321.71	321.71	321.71
24.750	321.70	321.70	321.69	321.69	321.69
25.000	321.68	321.68	321.68	321.67	321.67
25.250	321.67	321.66	321.66	321.66	321.66
25.500	321.65	321.65	321.65	321.65	321.65
25.750	321.64	321.64	321.64	321.64	321.64
26.000	321.64	321.63	321.63	321.63	321.63
26.250	321.63	321.63	321.63	321.63	321.62
26.500	321.62	321.62	321.62	321.62	321.62
26.750	321.62	321.62	321.62	321.62	321.62
27.000	321.62	321.61	321.61	321.61	321.61
27.250	321.61	321.61	321.61	321.61	321.61
27.500	321.61	321.61	321.61	321.61	321.61
27.750	321.61	321.61	321.61	321.61	321.61
28.000	321.61	321.61	321.61	321.61	321.61
28.250	321.61	321.61	321.61	321.60	321.60
28.500	321.60	321.60	321.60	321.60	321.60
28.750	321.60	321.60	321.60	321.60	321.60
29.000	321.60	321.60	321.60	321.60	321.60
29.250	321.60	321.60	321.60	321.60	321.60
29.500	321.60	321.60	321.60	321.60	321.60
29.750	321.60	321.60	321.60	321.60	321.60
30.000	321.60	321.60	321.60	321.60	321.60
30.250	321.60	321.60	321.60	321.60	321.60

Watershed

Subsection: Time vs. Elevation

Label: Wet Extended Detention Pond #2A (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	321.60	321.60	321.60	321.60	321.60
30.750	321.60	321.60	321.60	321.60	321.60
31.000	321.60	321.60	321.60	321.60	321.60
31.250	321.60	321.60	321.60	321.60	321.60
31.500	321.60	321.60	321.60	321.60	321.60
31.750	321.60	321.60	321.60	321.60	321.60
32.000	321.60	321.60	321.60	321.60	321.60
32.250	321.60	321.60	321.60	321.60	321.60
32.500	321.60	321.60	321.60	321.60	321.60
32.750	321.60	321.60	321.60	321.60	321.60
33.000	321.60	321.60	321.60	321.60	321.60
33.250	321.60	321.60	321.60	321.60	321.60
33.500	321.60	321.60	321.60	321.60	321.60
33.750	321.60	321.60	321.60	321.60	321.60
34.000	321.60	321.60	321.60	321.60	321.60
34.250	321.60	321.60	321.60	321.60	321.60
34.500	321.60	321.60	321.60	321.60	321.60
34.750	321.60	321.60	321.60	321.60	321.60
35.000	321.60	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Composite Rating Curve

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
300.00	0.00	(N/A)	0.00
300.50	0.00	(N/A)	0.00
301.00	0.00	(N/A)	0.00
301.50	0.00	(N/A)	0.00
302.00	0.00	(N/A)	0.00
302.50	0.00	(N/A)	0.00
303.00	0.00	(N/A)	0.00
303.50	0.00	(N/A)	0.00
304.00	0.00	(N/A)	0.00
304.50	0.00	(N/A)	0.00
304.60	0.00	(N/A)	0.00
305.00	5.45	(N/A)	0.00
305.50	26.33	(N/A)	0.00
306.00	65.17	(N/A)	0.00

Contributing Structures

None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
300.00	0.00	(N/A)	0.00
300.50	0.00	(N/A)	0.00
301.00	0.00	(N/A)	0.00
301.50	0.00	(N/A)	0.00
302.00	0.00	(N/A)	0.00
302.50	0.00	(N/A)	0.00
303.00	0.00	(N/A)	0.00
303.50	0.00	(N/A)	0.00
304.00	0.00	(N/A)	0.00
304.50	0.00	(N/A)	0.00
304.60	0.00	(N/A)	0.00
305.00	5.45	(N/A)	0.00
305.50	26.33	(N/A)	0.00
306.00	65.17	(N/A)	0.00

Contributing Structures

None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
300.00	0.00	(N/A)	0.00
300.50	0.00	(N/A)	0.00
301.00	0.00	(N/A)	0.00
301.50	0.00	(N/A)	0.00
302.00	0.00	(N/A)	0.00
302.50	0.00	(N/A)	0.00
303.00	0.00	(N/A)	0.00
303.50	0.00	(N/A)	0.00
304.00	0.00	(N/A)	0.00
304.50	0.00	(N/A)	0.00
304.60	0.00	(N/A)	0.00
305.00	5.45	(N/A)	0.00
305.50	26.33	(N/A)	0.00
306.00	65.17	(N/A)	0.00

Contributing Structures

None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
300.00	0.00	(N/A)	0.00
300.50	0.00	(N/A)	0.00
301.00	0.00	(N/A)	0.00
301.50	0.00	(N/A)	0.00
302.00	0.00	(N/A)	0.00
302.50	0.00	(N/A)	0.00
303.00	0.00	(N/A)	0.00
303.50	0.00	(N/A)	0.00
304.00	0.00	(N/A)	0.00
304.50	0.00	(N/A)	0.00
304.60	0.00	(N/A)	0.00
305.00	5.45	(N/A)	0.00
305.50	26.33	(N/A)	0.00
306.00	65.17	(N/A)	0.00

Contributing Structures

None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
300.00	0.00	(N/A)	0.00
300.50	0.00	(N/A)	0.00
301.00	0.00	(N/A)	0.00
301.50	0.00	(N/A)	0.00
302.00	0.00	(N/A)	0.00
302.50	0.00	(N/A)	0.00
303.00	0.00	(N/A)	0.00
303.50	0.00	(N/A)	0.00
304.00	0.00	(N/A)	0.00
304.50	0.00	(N/A)	0.00
304.60	0.00	(N/A)	0.00
305.00	5.45	(N/A)	0.00
305.50	26.33	(N/A)	0.00
306.00	65.17	(N/A)	0.00

Contributing Structures

None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
None Contributing
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.60	(N/A)	0.00
316.00	2.31	(N/A)	0.00
316.50	4.75	(N/A)	0.00
317.00	6.34	(N/A)	0.00
317.50	8.01	(N/A)	0.00
318.00	9.45	(N/A)	0.00
318.50	10.74	(N/A)	0.00
319.00	11.94	(N/A)	0.00
319.25	12.51	(N/A)	0.00
319.50	13.57	(N/A)	0.00
320.00	16.88	(N/A)	0.00
320.50	20.97	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Orifice - 2,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Orifice - 2,Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Orifice - 2,Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Orifice - 2,Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Orifice - 2,Weir - 1)
 Orifice - 1,Orifice - 2,Culvert - 1 (no Q: Weir - 1)
 Orifice - 1,Orifice - 2,Culvert - 1 (no Q: Weir - 1)
 Orifice - 1,Orifice - 2,Culvert - 1 (no Q: Weir - 1)
 Orifice - 1,Orifice - 2,Culvert - 1 (no Q: Weir - 1)
 Orifice - 1,Orifice - 2,Culvert - 1 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.60	(N/A)	0.00
316.00	2.31	(N/A)	0.00
316.50	4.75	(N/A)	0.00
317.00	6.34	(N/A)	0.00
317.50	8.01	(N/A)	0.00
318.00	9.45	(N/A)	0.00
318.50	10.74	(N/A)	0.00
319.00	11.94	(N/A)	0.00
319.25	12.51	(N/A)	0.00
319.50	13.57	(N/A)	0.00
320.00	16.88	(N/A)	0.00
320.50	20.97	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Orifice
- 2,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.60	(N/A)	0.00
316.00	2.31	(N/A)	0.00
316.50	4.75	(N/A)	0.00
317.00	6.34	(N/A)	0.00
317.50	8.01	(N/A)	0.00
318.00	9.45	(N/A)	0.00
318.50	10.74	(N/A)	0.00
319.00	11.94	(N/A)	0.00
319.25	12.51	(N/A)	0.00
319.50	13.57	(N/A)	0.00
320.00	16.88	(N/A)	0.00
320.50	20.97	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Orifice
- 2,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.60	(N/A)	0.00
316.00	2.31	(N/A)	0.00
316.50	4.75	(N/A)	0.00
317.00	6.34	(N/A)	0.00
317.50	8.01	(N/A)	0.00
318.00	9.45	(N/A)	0.00
318.50	10.74	(N/A)	0.00
319.00	11.94	(N/A)	0.00
319.25	12.51	(N/A)	0.00
319.50	13.57	(N/A)	0.00
320.00	16.88	(N/A)	0.00
320.50	20.97	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Orifice
- 2,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.60	(N/A)	0.00
316.00	2.31	(N/A)	0.00
316.50	4.75	(N/A)	0.00
317.00	6.34	(N/A)	0.00
317.50	8.01	(N/A)	0.00
318.00	9.45	(N/A)	0.00
318.50	10.74	(N/A)	0.00
319.00	11.94	(N/A)	0.00
319.25	12.51	(N/A)	0.00
319.50	13.57	(N/A)	0.00
320.00	16.88	(N/A)	0.00
320.50	20.97	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Orifice
- 2,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Culvert - 1
(no Q: Orifice - 2,Weir -
1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)
Orifice - 1,Orifice -
2,Culvert - 1 (no Q: Weir
- 1)

Watershed

Subsection: Composite Rating Curve

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Orifice - 1, Orifice - 2, Weir
- 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
296.00	0.00	(N/A)	0.00
296.50	3.02	(N/A)	0.00
297.00	17.07	(N/A)	0.00
297.50	47.04	(N/A)	0.00
298.00	96.57	(N/A)	0.00
298.50	180.43	(N/A)	0.00
299.00	290.21	(N/A)	0.00
299.50	428.77	(N/A)	0.00
300.00	598.36	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
345.00	0.00	(N/A)	0.00
345.50	0.00	(N/A)	0.00
345.75	0.00	(N/A)	0.00
346.00	0.24	(N/A)	0.00
346.50	1.95	(N/A)	0.00
347.00	4.77	(N/A)	0.00
347.50	6.63	(N/A)	0.00
348.00	7.72	(N/A)	0.00
348.50	8.71	(N/A)	0.00
349.00	9.62	(N/A)	0.00
349.50	10.41	(N/A)	0.00
350.00	11.12	(N/A)	0.00
350.50	11.79	(N/A)	0.00
351.00	15.83	(N/A)	0.00

Contributing Structures

[illegible]

Watershed

Subsection: Composite Rating Curve

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
345.00	0.00	(N/A)	0.00
345.50	0.00	(N/A)	0.00
345.75	0.00	(N/A)	0.00
346.00	0.24	(N/A)	0.00
346.50	1.95	(N/A)	0.00
347.00	4.77	(N/A)	0.00
347.50	6.63	(N/A)	0.00
348.00	7.72	(N/A)	0.00
348.50	8.71	(N/A)	0.00
349.00	9.62	(N/A)	0.00
349.50	10.41	(N/A)	0.00
350.00	11.12	(N/A)	0.00
350.50	11.79	(N/A)	0.00
351.00	15.83	(N/A)	0.00

Contributing Structures

[illegible]

Watershed

Subsection: Composite Rating Curve

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
345.00	0.00	(N/A)	0.00
345.50	0.00	(N/A)	0.00
345.75	0.00	(N/A)	0.00
346.00	0.24	(N/A)	0.00
346.50	1.95	(N/A)	0.00
347.00	4.77	(N/A)	0.00
347.50	6.63	(N/A)	0.00
348.00	7.72	(N/A)	0.00
348.50	8.71	(N/A)	0.00
349.00	9.62	(N/A)	0.00
349.50	10.41	(N/A)	0.00
350.00	11.12	(N/A)	0.00
350.50	11.79	(N/A)	0.00
351.00	15.83	(N/A)	0.00

Contributing Structures

[illegible]

Watershed

Subsection: Composite Rating Curve

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
345.00	0.00	(N/A)	0.00
345.50	0.00	(N/A)	0.00
345.75	0.00	(N/A)	0.00
346.00	0.24	(N/A)	0.00
346.50	1.95	(N/A)	0.00
347.00	4.77	(N/A)	0.00
347.50	6.63	(N/A)	0.00
348.00	7.72	(N/A)	0.00
348.50	8.71	(N/A)	0.00
349.00	9.62	(N/A)	0.00
349.50	10.41	(N/A)	0.00
350.00	11.12	(N/A)	0.00
350.50	11.79	(N/A)	0.00
351.00	15.83	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
345.00	0.00	(N/A)	0.00
345.50	0.00	(N/A)	0.00
345.75	0.00	(N/A)	0.00
346.00	0.24	(N/A)	0.00
346.50	1.95	(N/A)	0.00
347.00	4.77	(N/A)	0.00
347.50	6.63	(N/A)	0.00
348.00	7.72	(N/A)	0.00
348.50	8.71	(N/A)	0.00
349.00	9.62	(N/A)	0.00
349.50	10.41	(N/A)	0.00
350.00	11.12	(N/A)	0.00
350.50	11.79	(N/A)	0.00
351.00	15.83	(N/A)	0.00

Contributing Structures

[illegible]

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
325.00	0.00	(N/A)	0.00
325.50	0.00	(N/A)	0.00
326.00	0.00	(N/A)	0.00
326.20	0.00	(N/A)	0.00
326.50	0.43	(N/A)	0.00
327.00	2.85	(N/A)	0.00
327.50	6.99	(N/A)	0.00
328.00	12.30	(N/A)	0.00
328.50	17.24	(N/A)	0.00
329.00	19.85	(N/A)	0.00
329.50	21.86	(N/A)	0.00
330.00	23.71	(N/A)	0.00
330.25	24.61	(N/A)	0.00
330.50	27.07	(N/A)	0.00
331.00	35.35	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
325.00	0.00	(N/A)	0.00
325.50	0.00	(N/A)	0.00
326.00	0.00	(N/A)	0.00
326.20	0.00	(N/A)	0.00
326.50	0.43	(N/A)	0.00
327.00	2.85	(N/A)	0.00
327.50	6.99	(N/A)	0.00
328.00	12.30	(N/A)	0.00
328.50	17.24	(N/A)	0.00
329.00	19.85	(N/A)	0.00
329.50	21.86	(N/A)	0.00
330.00	23.71	(N/A)	0.00
330.25	24.61	(N/A)	0.00
330.50	27.07	(N/A)	0.00
331.00	35.35	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
325.00	0.00	(N/A)	0.00
325.50	0.00	(N/A)	0.00
326.00	0.00	(N/A)	0.00
326.20	0.00	(N/A)	0.00
326.50	0.43	(N/A)	0.00
327.00	2.85	(N/A)	0.00
327.50	6.99	(N/A)	0.00
328.00	12.30	(N/A)	0.00
328.50	17.24	(N/A)	0.00
329.00	19.85	(N/A)	0.00
329.50	21.86	(N/A)	0.00
330.00	23.71	(N/A)	0.00
330.25	24.61	(N/A)	0.00
330.50	27.07	(N/A)	0.00
331.00	35.35	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
325.00	0.00	(N/A)	0.00
325.50	0.00	(N/A)	0.00
326.00	0.00	(N/A)	0.00
326.20	0.00	(N/A)	0.00
326.50	0.43	(N/A)	0.00
327.00	2.85	(N/A)	0.00
327.50	6.99	(N/A)	0.00
328.00	12.30	(N/A)	0.00
328.50	17.24	(N/A)	0.00
329.00	19.85	(N/A)	0.00
329.50	21.86	(N/A)	0.00
330.00	23.71	(N/A)	0.00
330.25	24.61	(N/A)	0.00
330.50	27.07	(N/A)	0.00
331.00	35.35	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
325.00	0.00	(N/A)	0.00
325.50	0.00	(N/A)	0.00
326.00	0.00	(N/A)	0.00
326.20	0.00	(N/A)	0.00
326.50	0.43	(N/A)	0.00
327.00	2.85	(N/A)	0.00
327.50	6.99	(N/A)	0.00
328.00	12.30	(N/A)	0.00
328.50	17.24	(N/A)	0.00
329.00	19.85	(N/A)	0.00
329.50	21.86	(N/A)	0.00
330.00	23.71	(N/A)	0.00
330.25	24.61	(N/A)	0.00
330.50	27.07	(N/A)	0.00
331.00	35.35	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir - 1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.00	(N/A)	0.00
316.00	0.00	(N/A)	0.00
316.50	0.00	(N/A)	0.00
317.00	0.00	(N/A)	0.00
317.50	0.00	(N/A)	0.00
318.00	0.00	(N/A)	0.00
318.50	0.00	(N/A)	0.00
319.00	0.00	(N/A)	0.00
319.50	0.00	(N/A)	0.00
320.00	0.00	(N/A)	0.00
320.50	0.00	(N/A)	0.00
321.00	0.00	(N/A)	0.00
321.50	0.00	(N/A)	0.00
321.60	0.00	(N/A)	0.00
322.00	1.72	(N/A)	0.00
322.50	7.77	(N/A)	0.00
323.00	9.95	(N/A)	0.00
323.50	10.80	(N/A)	0.00
324.00	12.06	(N/A)	0.00
324.50	13.02	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Contributing Structures
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.00	(N/A)	0.00
316.00	0.00	(N/A)	0.00
316.50	0.00	(N/A)	0.00
317.00	0.00	(N/A)	0.00
317.50	0.00	(N/A)	0.00
318.00	0.00	(N/A)	0.00
318.50	0.00	(N/A)	0.00
319.00	0.00	(N/A)	0.00
319.50	0.00	(N/A)	0.00
320.00	0.00	(N/A)	0.00
320.50	0.00	(N/A)	0.00
321.00	0.00	(N/A)	0.00
321.50	0.00	(N/A)	0.00
321.60	0.00	(N/A)	0.00
322.00	1.72	(N/A)	0.00
322.50	7.77	(N/A)	0.00
323.00	9.95	(N/A)	0.00
323.50	10.80	(N/A)	0.00
324.00	12.06	(N/A)	0.00
324.50	13.02	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Contributing Structures
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.00	(N/A)	0.00
316.00	0.00	(N/A)	0.00
316.50	0.00	(N/A)	0.00
317.00	0.00	(N/A)	0.00
317.50	0.00	(N/A)	0.00
318.00	0.00	(N/A)	0.00
318.50	0.00	(N/A)	0.00
319.00	0.00	(N/A)	0.00
319.50	0.00	(N/A)	0.00
320.00	0.00	(N/A)	0.00
320.50	0.00	(N/A)	0.00
321.00	0.00	(N/A)	0.00
321.50	0.00	(N/A)	0.00
321.60	0.00	(N/A)	0.00
322.00	1.72	(N/A)	0.00
322.50	7.77	(N/A)	0.00
323.00	9.95	(N/A)	0.00
323.50	10.80	(N/A)	0.00
324.00	12.06	(N/A)	0.00
324.50	13.02	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Contributing Structures
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.00	(N/A)	0.00
316.00	0.00	(N/A)	0.00
316.50	0.00	(N/A)	0.00
317.00	0.00	(N/A)	0.00
317.50	0.00	(N/A)	0.00
318.00	0.00	(N/A)	0.00
318.50	0.00	(N/A)	0.00
319.00	0.00	(N/A)	0.00
319.50	0.00	(N/A)	0.00
320.00	0.00	(N/A)	0.00
320.50	0.00	(N/A)	0.00
321.00	0.00	(N/A)	0.00
321.50	0.00	(N/A)	0.00
321.60	0.00	(N/A)	0.00
322.00	1.72	(N/A)	0.00
322.50	7.77	(N/A)	0.00
323.00	9.95	(N/A)	0.00
323.50	10.80	(N/A)	0.00
324.00	12.06	(N/A)	0.00
324.50	13.02	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Contributing Structures
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
315.00	0.00	(N/A)	0.00
315.50	0.00	(N/A)	0.00
316.00	0.00	(N/A)	0.00
316.50	0.00	(N/A)	0.00
317.00	0.00	(N/A)	0.00
317.50	0.00	(N/A)	0.00
318.00	0.00	(N/A)	0.00
318.50	0.00	(N/A)	0.00
319.00	0.00	(N/A)	0.00
319.50	0.00	(N/A)	0.00
320.00	0.00	(N/A)	0.00
320.50	0.00	(N/A)	0.00
321.00	0.00	(N/A)	0.00
321.50	0.00	(N/A)	0.00
321.60	0.00	(N/A)	0.00
322.00	1.72	(N/A)	0.00
322.50	7.77	(N/A)	0.00
323.00	9.95	(N/A)	0.00
323.50	10.80	(N/A)	0.00
324.00	12.06	(N/A)	0.00
324.50	13.02	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
(no Q: Orifice - 1,Weir - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
290.00	0.00	(N/A)	0.00
290.50	0.00	(N/A)	0.00
291.00	0.00	(N/A)	0.00
291.50	0.00	(N/A)	0.00
292.00	0.00	(N/A)	0.00
292.30	0.00	(N/A)	0.00
292.50	0.38	(N/A)	0.00
293.00	4.37	(N/A)	0.00
293.50	11.92	(N/A)	0.00
294.00	21.97	(N/A)	0.00
294.50	32.05	(N/A)	0.00
295.00	36.19	(N/A)	0.00
295.50	41.29	(N/A)	0.00
296.00	47.76	(N/A)	0.00
296.50	53.82	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
290.00	0.00	(N/A)	0.00
290.50	0.00	(N/A)	0.00
291.00	0.00	(N/A)	0.00
291.50	0.00	(N/A)	0.00
292.00	0.00	(N/A)	0.00
292.30	0.00	(N/A)	0.00
292.50	0.38	(N/A)	0.00
293.00	4.37	(N/A)	0.00
293.50	11.92	(N/A)	0.00
294.00	21.97	(N/A)	0.00
294.50	32.05	(N/A)	0.00
295.00	36.19	(N/A)	0.00
295.50	41.29	(N/A)	0.00
296.00	47.76	(N/A)	0.00
296.50	53.82	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
290.00	0.00	(N/A)	0.00
290.50	0.00	(N/A)	0.00
291.00	0.00	(N/A)	0.00
291.50	0.00	(N/A)	0.00
292.00	0.00	(N/A)	0.00
292.30	0.00	(N/A)	0.00
292.50	0.38	(N/A)	0.00
293.00	4.37	(N/A)	0.00
293.50	11.92	(N/A)	0.00
294.00	21.97	(N/A)	0.00
294.50	32.05	(N/A)	0.00
295.00	36.19	(N/A)	0.00
295.50	41.29	(N/A)	0.00
296.00	47.76	(N/A)	0.00
296.50	53.82	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
290.00	0.00	(N/A)	0.00
290.50	0.00	(N/A)	0.00
291.00	0.00	(N/A)	0.00
291.50	0.00	(N/A)	0.00
292.00	0.00	(N/A)	0.00
292.30	0.00	(N/A)	0.00
292.50	0.38	(N/A)	0.00
293.00	4.37	(N/A)	0.00
293.50	11.92	(N/A)	0.00
294.00	21.97	(N/A)	0.00
294.50	32.05	(N/A)	0.00
295.00	36.19	(N/A)	0.00
295.50	41.29	(N/A)	0.00
296.00	47.76	(N/A)	0.00
296.50	53.82	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
290.00	0.00	(N/A)	0.00
290.50	0.00	(N/A)	0.00
291.00	0.00	(N/A)	0.00
291.50	0.00	(N/A)	0.00
292.00	0.00	(N/A)	0.00
292.30	0.00	(N/A)	0.00
292.50	0.38	(N/A)	0.00
293.00	4.37	(N/A)	0.00
293.50	11.92	(N/A)	0.00
294.00	21.97	(N/A)	0.00
294.50	32.05	(N/A)	0.00
295.00	36.19	(N/A)	0.00
295.50	41.29	(N/A)	0.00
296.00	47.76	(N/A)	0.00
296.50	53.82	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Weir - 1,Culvert - 1
 Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1, Weir -
1, Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
318.75	0.00	(N/A)	0.00
319.25	0.00	(N/A)	0.00
319.75	0.00	(N/A)	0.00
320.25	0.00	(N/A)	0.00
320.75	0.00	(N/A)	0.00
321.25	0.00	(N/A)	0.00
321.65	0.00	(N/A)	0.00
321.75	0.09	(N/A)	0.00
322.25	3.05	(N/A)	0.00
322.75	9.41	(N/A)	0.00
323.25	18.05	(N/A)	0.00
323.75	25.61	(N/A)	0.00
324.25	30.41	(N/A)	0.00
324.50	32.53	(N/A)	0.00
324.75	36.41	(N/A)	0.00
325.25	44.20	(N/A)	0.00
325.50	47.73	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1,Culvert - 1
(no Q: Weir - 1)

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
318.75	0.00	(N/A)	0.00
319.25	0.00	(N/A)	0.00
319.75	0.00	(N/A)	0.00
320.25	0.00	(N/A)	0.00
320.75	0.00	(N/A)	0.00
321.25	0.00	(N/A)	0.00
321.65	0.00	(N/A)	0.00
321.75	0.09	(N/A)	0.00
322.25	3.05	(N/A)	0.00
322.75	9.41	(N/A)	0.00
323.25	18.05	(N/A)	0.00
323.75	25.61	(N/A)	0.00
324.25	30.41	(N/A)	0.00
324.50	32.53	(N/A)	0.00
324.75	36.41	(N/A)	0.00
325.25	44.20	(N/A)	0.00
325.50	47.73	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1,Culvert - 1
(no Q: Weir - 1)

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
318.75	0.00	(N/A)	0.00
319.25	0.00	(N/A)	0.00
319.75	0.00	(N/A)	0.00
320.25	0.00	(N/A)	0.00
320.75	0.00	(N/A)	0.00
321.25	0.00	(N/A)	0.00
321.65	0.00	(N/A)	0.00
321.75	0.09	(N/A)	0.00
322.25	3.05	(N/A)	0.00
322.75	9.41	(N/A)	0.00
323.25	18.05	(N/A)	0.00
323.75	25.61	(N/A)	0.00
324.25	30.41	(N/A)	0.00
324.50	32.53	(N/A)	0.00
324.75	36.41	(N/A)	0.00
325.25	44.20	(N/A)	0.00
325.50	47.73	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Contributing Structures
Orifice - 1,Culvert - 1 (no Q: Weir - 1)
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1
Orifice - 1,Weir - 1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
318.75	0.00	(N/A)	0.00
319.25	0.00	(N/A)	0.00
319.75	0.00	(N/A)	0.00
320.25	0.00	(N/A)	0.00
320.75	0.00	(N/A)	0.00
321.25	0.00	(N/A)	0.00
321.65	0.00	(N/A)	0.00
321.75	0.09	(N/A)	0.00
322.25	3.05	(N/A)	0.00
322.75	9.41	(N/A)	0.00
323.25	18.05	(N/A)	0.00
323.75	25.61	(N/A)	0.00
324.25	30.41	(N/A)	0.00
324.50	32.53	(N/A)	0.00
324.75	36.41	(N/A)	0.00
325.25	44.20	(N/A)	0.00
325.50	47.73	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1,Culvert - 1
(no Q: Weir - 1)

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
318.75	0.00	(N/A)	0.00
319.25	0.00	(N/A)	0.00
319.75	0.00	(N/A)	0.00
320.25	0.00	(N/A)	0.00
320.75	0.00	(N/A)	0.00
321.25	0.00	(N/A)	0.00
321.65	0.00	(N/A)	0.00
321.75	0.09	(N/A)	0.00
322.25	3.05	(N/A)	0.00
322.75	9.41	(N/A)	0.00
323.25	18.05	(N/A)	0.00
323.75	25.61	(N/A)	0.00
324.25	30.41	(N/A)	0.00
324.50	32.53	(N/A)	0.00
324.75	36.41	(N/A)	0.00
325.25	44.20	(N/A)	0.00
325.50	47.73	(N/A)	0.00

Contributing Structures

(no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 (no Q: Orifice - 1,Weir - 1,Culvert - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)
 Orifice - 1,Culvert - 1
 (no Q: Weir - 1)

Watershed

Subsection: Composite Rating Curve

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Contributing Structures

Orifice - 1,Culvert - 1
(no Q: Weir - 1)

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Orifice - 1,Weir -
1,Culvert - 1

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Composite Rating Curve

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
319.17	0.00	(N/A)	0.00
319.27	0.04	(N/A)	0.00
319.67	1.47	(N/A)	0.00
320.17	5.90	(N/A)	0.00
320.30	7.41	(N/A)	0.00
320.67	33.37	(N/A)	0.00
321.00	123.80	(N/A)	0.00
321.17	204.57	(N/A)	0.00
321.67	580.91	(N/A)	0.00
322.00	959.09	(N/A)	0.00

Contributing Structures

None Contributing
 North Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2
 North Pipe + South Pipe
 + Weir - 1 + Weir - 2

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Det Basin #1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	315.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	2,128.000	0.00	0.00	0.00
315.50	0.60	1,154.401	2,494.452	0.00	0.60	13.43
316.00	2.31	2,499.301	2,890.000	0.00	2.31	30.08
316.50	4.75	4,050.261	3,318.780	0.00	4.75	49.75
317.00	6.34	5,823.022	3,777.206	0.00	6.34	71.05
317.50	8.01	7,832.408	4,265.280	0.00	8.01	95.03
318.00	9.45	10,093.242	4,783.000	0.00	9.45	121.60
318.50	10.74	12,613.092	5,300.835	0.00	10.74	150.89
319.00	11.94	15,398.513	5,845.280	0.00	11.94	183.04
319.25	12.51	16,894.969	6,127.481	0.00	12.51	200.23
319.50	13.57	18,462.808	6,416.335	0.00	13.57	218.71
320.00	16.88	21,819.283	7,014.000	0.00	16.88	259.31
320.50	20.97	25,613.073	8,176.000	0.00	20.97	305.56

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Det Basin #1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	315.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	2,128.000	0.00	0.00	0.00
315.50	0.60	1,154.401	2,494.452	0.00	0.60	13.43
316.00	2.31	2,499.301	2,890.000	0.00	2.31	30.08
316.50	4.75	4,050.261	3,318.780	0.00	4.75	49.75
317.00	6.34	5,823.022	3,777.206	0.00	6.34	71.05
317.50	8.01	7,832.408	4,265.280	0.00	8.01	95.03
318.00	9.45	10,093.242	4,783.000	0.00	9.45	121.60
318.50	10.74	12,613.092	5,300.835	0.00	10.74	150.89
319.00	11.94	15,398.513	5,845.280	0.00	11.94	183.04
319.25	12.51	16,894.969	6,127.481	0.00	12.51	200.23
319.50	13.57	18,462.808	6,416.335	0.00	13.57	218.71
320.00	16.88	21,819.283	7,014.000	0.00	16.88	259.31
320.50	20.97	25,613.073	8,176.000	0.00	20.97	305.56

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Det Basin #1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	315.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	2,128.000	0.00	0.00	0.00
315.50	0.60	1,154.401	2,494.452	0.00	0.60	13.43
316.00	2.31	2,499.301	2,890.000	0.00	2.31	30.08
316.50	4.75	4,050.261	3,318.780	0.00	4.75	49.75
317.00	6.34	5,823.022	3,777.206	0.00	6.34	71.05
317.50	8.01	7,832.408	4,265.280	0.00	8.01	95.03
318.00	9.45	10,093.242	4,783.000	0.00	9.45	121.60
318.50	10.74	12,613.092	5,300.835	0.00	10.74	150.89
319.00	11.94	15,398.513	5,845.280	0.00	11.94	183.04
319.25	12.51	16,894.969	6,127.481	0.00	12.51	200.23
319.50	13.57	18,462.808	6,416.335	0.00	13.57	218.71
320.00	16.88	21,819.283	7,014.000	0.00	16.88	259.31
320.50	20.97	25,613.073	8,176.000	0.00	20.97	305.56

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Det Basin #1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	315.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	2,128.000	0.00	0.00	0.00
315.50	0.60	1,154.401	2,494.452	0.00	0.60	13.43
316.00	2.31	2,499.301	2,890.000	0.00	2.31	30.08
316.50	4.75	4,050.261	3,318.780	0.00	4.75	49.75
317.00	6.34	5,823.022	3,777.206	0.00	6.34	71.05
317.50	8.01	7,832.408	4,265.280	0.00	8.01	95.03
318.00	9.45	10,093.242	4,783.000	0.00	9.45	121.60
318.50	10.74	12,613.092	5,300.835	0.00	10.74	150.89
319.00	11.94	15,398.513	5,845.280	0.00	11.94	183.04
319.25	12.51	16,894.969	6,127.481	0.00	12.51	200.23
319.50	13.57	18,462.808	6,416.335	0.00	13.57	218.71
320.00	16.88	21,819.283	7,014.000	0.00	16.88	259.31
320.50	20.97	25,613.073	8,176.000	0.00	20.97	305.56

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Det Basin #1C

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	315.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	2,128.000	0.00	0.00	0.00
315.50	0.60	1,154.401	2,494.452	0.00	0.60	13.43
316.00	2.31	2,499.301	2,890.000	0.00	2.31	30.08
316.50	4.75	4,050.261	3,318.780	0.00	4.75	49.75
317.00	6.34	5,823.022	3,777.206	0.00	6.34	71.05
317.50	8.01	7,832.408	4,265.280	0.00	8.01	95.03
318.00	9.45	10,093.242	4,783.000	0.00	9.45	121.60
318.50	10.74	12,613.092	5,300.835	0.00	10.74	150.89
319.00	11.94	15,398.513	5,845.280	0.00	11.94	183.04
319.25	12.51	16,894.969	6,127.481	0.00	12.51	200.23
319.50	13.57	18,462.808	6,416.335	0.00	13.57	218.71
320.00	16.88	21,819.283	7,014.000	0.00	16.88	259.31
320.50	20.97	25,613.073	8,176.000	0.00	20.97	305.56

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	185.992	509.172	0.00	3.02	5.08
297.00	17.07	524.375	859.563	0.00	17.07	22.90
297.50	47.04	1,060.759	1,301.172	0.00	47.04	58.83
298.00	96.57	1,840.751	1,834.000	0.00	96.57	117.02
298.50	180.43	2,856.742	2,236.619	0.00	180.43	212.17
299.00	290.21	4,084.024	2,679.159	0.00	290.21	335.59
299.50	428.77	5,542.555	3,161.619	0.00	428.77	490.35
300.00	598.36	7,252.296	3,684.000	0.00	598.36	678.94

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	185.992	509.172	0.00	3.02	5.08
297.00	17.07	524.375	859.563	0.00	17.07	22.90
297.50	47.04	1,060.759	1,301.172	0.00	47.04	58.83
298.00	96.57	1,840.751	1,834.000	0.00	96.57	117.02
298.50	180.43	2,856.742	2,236.619	0.00	180.43	212.17
299.00	290.21	4,084.024	2,679.159	0.00	290.21	335.59
299.50	428.77	5,542.555	3,161.619	0.00	428.77	490.35
300.00	598.36	7,252.296	3,684.000	0.00	598.36	678.94

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	185.992	509.172	0.00	3.02	5.08
297.00	17.07	524.375	859.563	0.00	17.07	22.90
297.50	47.04	1,060.759	1,301.172	0.00	47.04	58.83
298.00	96.57	1,840.751	1,834.000	0.00	96.57	117.02
298.50	180.43	2,856.742	2,236.619	0.00	180.43	212.17
299.00	290.21	4,084.024	2,679.159	0.00	290.21	335.59
299.50	428.77	5,542.555	3,161.619	0.00	428.77	490.35
300.00	598.36	7,252.296	3,684.000	0.00	598.36	678.94

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	185.992	509.172	0.00	3.02	5.08
297.00	17.07	524.375	859.563	0.00	17.07	22.90
297.50	47.04	1,060.759	1,301.172	0.00	47.04	58.83
298.00	96.57	1,840.751	1,834.000	0.00	96.57	117.02
298.50	180.43	2,856.742	2,236.619	0.00	180.43	212.17
299.00	290.21	4,084.024	2,679.159	0.00	290.21	335.59
299.50	428.77	5,542.555	3,161.619	0.00	428.77	490.35
300.00	598.36	7,252.296	3,684.000	0.00	598.36	678.94

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: DP 4 Low Point

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	296.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
296.00	0.00	0.000	250.000	0.00	0.00	0.00
296.50	3.02	185.992	509.172	0.00	3.02	5.08
297.00	17.07	524.375	859.563	0.00	17.07	22.90
297.50	47.04	1,060.759	1,301.172	0.00	47.04	58.83
298.00	96.57	1,840.751	1,834.000	0.00	96.57	117.02
298.50	180.43	2,856.742	2,236.619	0.00	180.43	212.17
299.00	290.21	4,084.024	2,679.159	0.00	290.21	335.59
299.50	428.77	5,542.555	3,161.619	0.00	428.77	490.35
300.00	598.36	7,252.296	3,684.000	0.00	598.36	678.94

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Existing Pond

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	319.17 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
319.17	0.00	0.000	59,446.000	0.00	0.00	0.00
319.27	0.04	5,971.562	59,985.654	0.00	0.04	66.39
319.67	1.47	30,401.123	62,168.654	0.00	1.47	339.26
320.17	5.90	62,626.669	70,268.795	0.00	5.90	701.75
320.30	7.41	72,084.250	75,261.000	0.00	7.41	808.35
320.67	33.37	101,436.313	83,469.601	0.00	33.37	1,160.44
321.00	123.80	130,239.118	91,149.169	0.00	123.80	1,570.90
321.17	204.57	146,080.688	95,237.182	0.00	204.57	1,827.69
321.67	580.91	196,802.782	107,780.482	0.00	580.91	2,767.61
322.00	959.09	233,797.129	116,484.000	0.00	959.09	3,556.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	12.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	345.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
345.00	0.00	0.000	9,840.000	0.00	0.00	0.00
345.50	0.00	5,072.244	10,452.052	2.90	2.90	59.26
345.75	0.00	7,724.280	10,765.003	2.99	2.99	88.82
346.00	0.24	10,455.130	11,082.570	3.08	3.32	119.49
346.50	1.95	16,157.891	11,731.552	3.26	5.21	184.74
347.00	4.77	22,189.760	12,399.000	3.44	8.21	254.76
347.50	6.63	28,556.038	13,069.052	3.63	10.26	327.55
348.00	7.72	35,261.751	13,756.736	3.82	11.54	403.34
348.50	8.71	42,315.713	14,462.052	4.02	12.72	482.90
349.00	9.62	49,726.742	15,185.000	4.22	13.83	566.35
349.50	10.41	57,500.400	15,912.470	4.42	14.83	653.72
350.00	11.12	65,642.048	16,656.960	4.63	15.75	745.11
350.50	11.79	74,160.196	17,418.470	4.84	16.63	840.64
351.00	15.83	83,063.355	18,197.000	5.05	20.88	943.81

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	12.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	345.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
345.00	0.00	0.000	9,840.000	0.00	0.00	0.00
345.50	0.00	5,072.244	10,452.052	2.90	2.90	59.26
345.75	0.00	7,724.280	10,765.003	2.99	2.99	88.82
346.00	0.24	10,455.130	11,082.570	3.08	3.32	119.49
346.50	1.95	16,157.891	11,731.552	3.26	5.21	184.74
347.00	4.77	22,189.760	12,399.000	3.44	8.21	254.76
347.50	6.63	28,556.038	13,069.052	3.63	10.26	327.55
348.00	7.72	35,261.751	13,756.736	3.82	11.54	403.34
348.50	8.71	42,315.713	14,462.052	4.02	12.72	482.90
349.00	9.62	49,726.742	15,185.000	4.22	13.83	566.35
349.50	10.41	57,500.400	15,912.470	4.42	14.83	653.72
350.00	11.12	65,642.048	16,656.960	4.63	15.75	745.11
350.50	11.79	74,160.196	17,418.470	4.84	16.63	840.64
351.00	15.83	83,063.355	18,197.000	5.05	20.88	943.81

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	12.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	345.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
345.00	0.00	0.000	9,840.000	0.00	0.00	0.00
345.50	0.00	5,072.244	10,452.052	2.90	2.90	59.26
345.75	0.00	7,724.280	10,765.003	2.99	2.99	88.82
346.00	0.24	10,455.130	11,082.570	3.08	3.32	119.49
346.50	1.95	16,157.891	11,731.552	3.26	5.21	184.74
347.00	4.77	22,189.760	12,399.000	3.44	8.21	254.76
347.50	6.63	28,556.038	13,069.052	3.63	10.26	327.55
348.00	7.72	35,261.751	13,756.736	3.82	11.54	403.34
348.50	8.71	42,315.713	14,462.052	4.02	12.72	482.90
349.00	9.62	49,726.742	15,185.000	4.22	13.83	566.35
349.50	10.41	57,500.400	15,912.470	4.42	14.83	653.72
350.00	11.12	65,642.048	16,656.960	4.63	15.75	745.11
350.50	11.79	74,160.196	17,418.470	4.84	16.63	840.64
351.00	15.83	83,063.355	18,197.000	5.05	20.88	943.81

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	12.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	345.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
345.00	0.00	0.000	9,840.000	0.00	0.00	0.00
345.50	0.00	5,072.244	10,452.052	2.90	2.90	59.26
345.75	0.00	7,724.280	10,765.003	2.99	2.99	88.82
346.00	0.24	10,455.130	11,082.570	3.08	3.32	119.49
346.50	1.95	16,157.891	11,731.552	3.26	5.21	184.74
347.00	4.77	22,189.760	12,399.000	3.44	8.21	254.76
347.50	6.63	28,556.038	13,069.052	3.63	10.26	327.55
348.00	7.72	35,261.751	13,756.736	3.82	11.54	403.34
348.50	8.71	42,315.713	14,462.052	4.02	12.72	482.90
349.00	9.62	49,726.742	15,185.000	4.22	13.83	566.35
349.50	10.41	57,500.400	15,912.470	4.42	14.83	653.72
350.00	11.12	65,642.048	16,656.960	4.63	15.75	745.11
350.50	11.79	74,160.196	17,418.470	4.84	16.63	840.64
351.00	15.83	83,063.355	18,197.000	5.05	20.88	943.81

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	12.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	345.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
345.00	0.00	0.000	9,840.000	0.00	0.00	0.00
345.50	0.00	5,072.244	10,452.052	2.90	2.90	59.26
345.75	0.00	7,724.280	10,765.003	2.99	2.99	88.82
346.00	0.24	10,455.130	11,082.570	3.08	3.32	119.49
346.50	1.95	16,157.891	11,731.552	3.26	5.21	184.74
347.00	4.77	22,189.760	12,399.000	3.44	8.21	254.76
347.50	6.63	28,556.038	13,069.052	3.63	10.26	327.55
348.00	7.72	35,261.751	13,756.736	3.82	11.54	403.34
348.50	8.71	42,315.713	14,462.052	4.02	12.72	482.90
349.00	9.62	49,726.742	15,185.000	4.22	13.83	566.35
349.50	10.41	57,500.400	15,912.470	4.42	14.83	653.72
350.00	11.12	65,642.048	16,656.960	4.63	15.75	745.11
350.50	11.79	74,160.196	17,418.470	4.84	16.63	840.64
351.00	15.83	83,063.355	18,197.000	5.05	20.88	943.81

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	10.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	325.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
325.00	0.00	0.000	5,796.000	0.00	0.00	0.00
325.50	0.00	3,024.274	6,304.663	1.46	1.46	35.06
326.00	0.00	6,308.228	6,834.717	1.58	1.58	71.67
326.20	0.00	7,696.915	7,052.728	1.63	1.63	87.15
326.50	0.43	9,862.556	7,386.163	1.71	2.14	111.72
327.00	2.85	13,697.956	7,959.000	1.84	4.69	156.89
327.50	6.99	17,818.671	8,527.124	1.97	8.96	206.95
328.00	12.30	22,228.344	9,114.832	2.11	14.41	261.39
328.50	17.24	26,936.767	9,722.124	2.25	19.49	318.79
329.00	19.85	31,953.732	10,349.000	2.40	22.24	377.29
329.50	21.86	37,284.064	10,975.394	2.54	24.40	438.67
330.00	23.71	42,932.194	11,620.192	2.69	26.40	503.43
330.25	24.61	45,878.308	11,949.493	2.77	27.38	537.14
330.50	27.07	48,907.323	12,283.394	2.84	29.92	573.33
331.00	35.35	55,218.655	12,965.000	3.00	38.35	651.89

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	10.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	325.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
325.00	0.00	0.000	5,796.000	0.00	0.00	0.00
325.50	0.00	3,024.274	6,304.663	1.46	1.46	35.06
326.00	0.00	6,308.228	6,834.717	1.58	1.58	71.67
326.20	0.00	7,696.915	7,052.728	1.63	1.63	87.15
326.50	0.43	9,862.556	7,386.163	1.71	2.14	111.72
327.00	2.85	13,697.956	7,959.000	1.84	4.69	156.89
327.50	6.99	17,818.671	8,527.124	1.97	8.96	206.95
328.00	12.30	22,228.344	9,114.832	2.11	14.41	261.39
328.50	17.24	26,936.767	9,722.124	2.25	19.49	318.79
329.00	19.85	31,953.732	10,349.000	2.40	22.24	377.29
329.50	21.86	37,284.064	10,975.394	2.54	24.40	438.67
330.00	23.71	42,932.194	11,620.192	2.69	26.40	503.43
330.25	24.61	45,878.308	11,949.493	2.77	27.38	537.14
330.50	27.07	48,907.323	12,283.394	2.84	29.92	573.33
331.00	35.35	55,218.655	12,965.000	3.00	38.35	651.89

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	10.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	325.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
325.00	0.00	0.000	5,796.000	0.00	0.00	0.00
325.50	0.00	3,024.274	6,304.663	1.46	1.46	35.06
326.00	0.00	6,308.228	6,834.717	1.58	1.58	71.67
326.20	0.00	7,696.915	7,052.728	1.63	1.63	87.15
326.50	0.43	9,862.556	7,386.163	1.71	2.14	111.72
327.00	2.85	13,697.956	7,959.000	1.84	4.69	156.89
327.50	6.99	17,818.671	8,527.124	1.97	8.96	206.95
328.00	12.30	22,228.344	9,114.832	2.11	14.41	261.39
328.50	17.24	26,936.767	9,722.124	2.25	19.49	318.79
329.00	19.85	31,953.732	10,349.000	2.40	22.24	377.29
329.50	21.86	37,284.064	10,975.394	2.54	24.40	438.67
330.00	23.71	42,932.194	11,620.192	2.69	26.40	503.43
330.25	24.61	45,878.308	11,949.493	2.77	27.38	537.14
330.50	27.07	48,907.323	12,283.394	2.84	29.92	573.33
331.00	35.35	55,218.655	12,965.000	3.00	38.35	651.89

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	10.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	325.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
325.00	0.00	0.000	5,796.000	0.00	0.00	0.00
325.50	0.00	3,024.274	6,304.663	1.46	1.46	35.06
326.00	0.00	6,308.228	6,834.717	1.58	1.58	71.67
326.20	0.00	7,696.915	7,052.728	1.63	1.63	87.15
326.50	0.43	9,862.556	7,386.163	1.71	2.14	111.72
327.00	2.85	13,697.956	7,959.000	1.84	4.69	156.89
327.50	6.99	17,818.671	8,527.124	1.97	8.96	206.95
328.00	12.30	22,228.344	9,114.832	2.11	14.41	261.39
328.50	17.24	26,936.767	9,722.124	2.25	19.49	318.79
329.00	19.85	31,953.732	10,349.000	2.40	22.24	377.29
329.50	21.86	37,284.064	10,975.394	2.54	24.40	438.67
330.00	23.71	42,932.194	11,620.192	2.69	26.40	503.43
330.25	24.61	45,878.308	11,949.493	2.77	27.38	537.14
330.50	27.07	48,907.323	12,283.394	2.84	29.92	573.33
331.00	35.35	55,218.655	12,965.000	3.00	38.35	651.89

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	10.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	325.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
325.00	0.00	0.000	5,796.000	0.00	0.00	0.00
325.50	0.00	3,024.274	6,304.663	1.46	1.46	35.06
326.00	0.00	6,308.228	6,834.717	1.58	1.58	71.67
326.20	0.00	7,696.915	7,052.728	1.63	1.63	87.15
326.50	0.43	9,862.556	7,386.163	1.71	2.14	111.72
327.00	2.85	13,697.956	7,959.000	1.84	4.69	156.89
327.50	6.99	17,818.671	8,527.124	1.97	8.96	206.95
328.00	12.30	22,228.344	9,114.832	2.11	14.41	261.39
328.50	17.24	26,936.767	9,722.124	2.25	19.49	318.79
329.00	19.85	31,953.732	10,349.000	2.40	22.24	377.29
329.50	21.86	37,284.064	10,975.394	2.54	24.40	438.67
330.00	23.71	42,932.194	11,620.192	2.69	26.40	503.43
330.25	24.61	45,878.308	11,949.493	2.77	27.38	537.14
330.50	27.07	48,907.323	12,283.394	2.84	29.92	573.33
331.00	35.35	55,218.655	12,965.000	3.00	38.35	651.89

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	2.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	290.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
290.00	0.00	0.000	6,510.000	0.00	0.00	0.00
290.50	0.00	3,378.840	7,008.425	0.32	0.32	37.87
291.00	0.00	7,011.489	7,525.234	0.35	0.35	78.25
291.50	0.00	10,907.138	8,060.425	0.37	0.37	121.56
292.00	0.00	15,074.978	8,614.000	0.40	0.40	167.90
292.30	0.00	17,708.803	8,945.877	0.41	0.41	197.18
292.50	0.38	19,520.405	9,170.613	0.42	0.80	217.70
293.00	4.37	24,248.495	9,744.650	0.45	4.82	274.25
293.50	11.92	29,267.960	10,336.113	0.48	12.40	337.60
294.00	21.97	34,587.512	10,945.000	0.51	22.48	406.78
294.50	32.05	40,212.783	11,558.877	0.54	32.58	479.39
295.00	36.19	46,149.180	12,189.502	0.56	36.75	549.52
295.50	41.29	52,405.077	12,836.877	0.59	41.89	624.17
296.00	47.76	58,988.848	13,501.000	0.63	48.39	703.82
296.50	53.82	66,424.240	16,284.000	0.75	54.58	792.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	2.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	290.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
290.00	0.00	0.000	6,510.000	0.00	0.00	0.00
290.50	0.00	3,378.840	7,008.425	0.32	0.32	37.87
291.00	0.00	7,011.489	7,525.234	0.35	0.35	78.25
291.50	0.00	10,907.138	8,060.425	0.37	0.37	121.56
292.00	0.00	15,074.978	8,614.000	0.40	0.40	167.90
292.30	0.00	17,708.803	8,945.877	0.41	0.41	197.18
292.50	0.38	19,520.405	9,170.613	0.42	0.80	217.70
293.00	4.37	24,248.495	9,744.650	0.45	4.82	274.25
293.50	11.92	29,267.960	10,336.113	0.48	12.40	337.60
294.00	21.97	34,587.512	10,945.000	0.51	22.48	406.78
294.50	32.05	40,212.783	11,558.877	0.54	32.58	479.39
295.00	36.19	46,149.180	12,189.502	0.56	36.75	549.52
295.50	41.29	52,405.077	12,836.877	0.59	41.89	624.17
296.00	47.76	58,988.848	13,501.000	0.63	48.39	703.82
296.50	53.82	66,424.240	16,284.000	0.75	54.58	792.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	2.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	290.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
290.00	0.00	0.000	6,510.000	0.00	0.00	0.00
290.50	0.00	3,378.840	7,008.425	0.32	0.32	37.87
291.00	0.00	7,011.489	7,525.234	0.35	0.35	78.25
291.50	0.00	10,907.138	8,060.425	0.37	0.37	121.56
292.00	0.00	15,074.978	8,614.000	0.40	0.40	167.90
292.30	0.00	17,708.803	8,945.877	0.41	0.41	197.18
292.50	0.38	19,520.405	9,170.613	0.42	0.80	217.70
293.00	4.37	24,248.495	9,744.650	0.45	4.82	274.25
293.50	11.92	29,267.960	10,336.113	0.48	12.40	337.60
294.00	21.97	34,587.512	10,945.000	0.51	22.48	406.78
294.50	32.05	40,212.783	11,558.877	0.54	32.58	479.39
295.00	36.19	46,149.180	12,189.502	0.56	36.75	549.52
295.50	41.29	52,405.077	12,836.877	0.59	41.89	624.17
296.00	47.76	58,988.848	13,501.000	0.63	48.39	703.82
296.50	53.82	66,424.240	16,284.000	0.75	54.58	792.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	2.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	290.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
290.00	0.00	0.000	6,510.000	0.00	0.00	0.00
290.50	0.00	3,378.840	7,008.425	0.32	0.32	37.87
291.00	0.00	7,011.489	7,525.234	0.35	0.35	78.25
291.50	0.00	10,907.138	8,060.425	0.37	0.37	121.56
292.00	0.00	15,074.978	8,614.000	0.40	0.40	167.90
292.30	0.00	17,708.803	8,945.877	0.41	0.41	197.18
292.50	0.38	19,520.405	9,170.613	0.42	0.80	217.70
293.00	4.37	24,248.495	9,744.650	0.45	4.82	274.25
293.50	11.92	29,267.960	10,336.113	0.48	12.40	337.60
294.00	21.97	34,587.512	10,945.000	0.51	22.48	406.78
294.50	32.05	40,212.783	11,558.877	0.54	32.58	479.39
295.00	36.19	46,149.180	12,189.502	0.56	36.75	549.52
295.50	41.29	52,405.077	12,836.877	0.59	41.89	624.17
296.00	47.76	58,988.848	13,501.000	0.63	48.39	703.82
296.50	53.82	66,424.240	16,284.000	0.75	54.58	792.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	2.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	290.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
290.00	0.00	0.000	6,510.000	0.00	0.00	0.00
290.50	0.00	3,378.840	7,008.425	0.32	0.32	37.87
291.00	0.00	7,011.489	7,525.234	0.35	0.35	78.25
291.50	0.00	10,907.138	8,060.425	0.37	0.37	121.56
292.00	0.00	15,074.978	8,614.000	0.40	0.40	167.90
292.30	0.00	17,708.803	8,945.877	0.41	0.41	197.18
292.50	0.38	19,520.405	9,170.613	0.42	0.80	217.70
293.00	4.37	24,248.495	9,744.650	0.45	4.82	274.25
293.50	11.92	29,267.960	10,336.113	0.48	12.40	337.60
294.00	21.97	34,587.512	10,945.000	0.51	22.48	406.78
294.50	32.05	40,212.783	11,558.877	0.54	32.58	479.39
295.00	36.19	46,149.180	12,189.502	0.56	36.75	549.52
295.50	41.29	52,405.077	12,836.877	0.59	41.89	624.17
296.00	47.76	58,988.848	13,501.000	0.63	48.39	703.82
296.50	53.82	66,424.240	16,284.000	0.75	54.58	792.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	20.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	300.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
300.00	0.00	0.000	6,751.000	0.00	0.00	0.00
300.50	0.00	3,504.154	7,268.804	3.37	3.37	42.30
301.00	0.00	7,271.992	7,805.738	3.61	3.61	84.41
301.50	0.00	11,313.080	8,361.804	3.87	3.87	129.57
302.00	0.00	15,636.984	8,937.000	4.14	4.14	177.88
302.50	0.00	20,245.564	9,500.189	4.40	4.40	229.35
303.00	0.00	25,140.041	10,080.585	4.67	4.67	284.00
303.50	0.00	30,329.018	10,678.189	4.94	4.94	341.93
304.00	0.00	35,821.098	11,293.000	5.23	5.23	403.24
304.50	0.00	41,633.561	11,960.042	5.54	5.54	468.13
304.60	0.00	42,836.344	12,095.747	5.60	5.60	481.56
305.00	5.45	47,784.329	12,646.222	5.85	11.30	542.24
305.50	26.33	54,282.973	13,351.542	6.18	32.51	635.66
306.00	65.17	61,139.061	14,076.000	6.52	71.69	751.01

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	20.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	300.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
300.00	0.00	0.000	6,751.000	0.00	0.00	0.00
300.50	0.00	3,504.154	7,268.804	3.37	3.37	42.30
301.00	0.00	7,271.992	7,805.738	3.61	3.61	84.41
301.50	0.00	11,313.080	8,361.804	3.87	3.87	129.57
302.00	0.00	15,636.984	8,937.000	4.14	4.14	177.88
302.50	0.00	20,245.564	9,500.189	4.40	4.40	229.35
303.00	0.00	25,140.041	10,080.585	4.67	4.67	284.00
303.50	0.00	30,329.018	10,678.189	4.94	4.94	341.93
304.00	0.00	35,821.098	11,293.000	5.23	5.23	403.24
304.50	0.00	41,633.561	11,960.042	5.54	5.54	468.13
304.60	0.00	42,836.344	12,095.747	5.60	5.60	481.56
305.00	5.45	47,784.329	12,646.222	5.85	11.30	542.24
305.50	26.33	54,282.973	13,351.542	6.18	32.51	635.66
306.00	65.17	61,139.061	14,076.000	6.52	71.69	751.01

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	20.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	300.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
300.00	0.00	0.000	6,751.000	0.00	0.00	0.00
300.50	0.00	3,504.154	7,268.804	3.37	3.37	42.30
301.00	0.00	7,271.992	7,805.738	3.61	3.61	84.41
301.50	0.00	11,313.080	8,361.804	3.87	3.87	129.57
302.00	0.00	15,636.984	8,937.000	4.14	4.14	177.88
302.50	0.00	20,245.564	9,500.189	4.40	4.40	229.35
303.00	0.00	25,140.041	10,080.585	4.67	4.67	284.00
303.50	0.00	30,329.018	10,678.189	4.94	4.94	341.93
304.00	0.00	35,821.098	11,293.000	5.23	5.23	403.24
304.50	0.00	41,633.561	11,960.042	5.54	5.54	468.13
304.60	0.00	42,836.344	12,095.747	5.60	5.60	481.56
305.00	5.45	47,784.329	12,646.222	5.85	11.30	542.24
305.50	26.33	54,282.973	13,351.542	6.18	32.51	635.66
306.00	65.17	61,139.061	14,076.000	6.52	71.69	751.01

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	20.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	300.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
300.00	0.00	0.000	6,751.000	0.00	0.00	0.00
300.50	0.00	3,504.154	7,268.804	3.37	3.37	42.30
301.00	0.00	7,271.992	7,805.738	3.61	3.61	84.41
301.50	0.00	11,313.080	8,361.804	3.87	3.87	129.57
302.00	0.00	15,636.984	8,937.000	4.14	4.14	177.88
302.50	0.00	20,245.564	9,500.189	4.40	4.40	229.35
303.00	0.00	25,140.041	10,080.585	4.67	4.67	284.00
303.50	0.00	30,329.018	10,678.189	4.94	4.94	341.93
304.00	0.00	35,821.098	11,293.000	5.23	5.23	403.24
304.50	0.00	41,633.561	11,960.042	5.54	5.54	468.13
304.60	0.00	42,836.344	12,095.747	5.60	5.60	481.56
305.00	5.45	47,784.329	12,646.222	5.85	11.30	542.24
305.50	26.33	54,282.973	13,351.542	6.18	32.51	635.66
306.00	65.17	61,139.061	14,076.000	6.52	71.69	751.01

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #4

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	20.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	300.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
300.00	0.00	0.000	6,751.000	0.00	0.00	0.00
300.50	0.00	3,504.154	7,268.804	3.37	3.37	42.30
301.00	0.00	7,271.992	7,805.738	3.61	3.61	84.41
301.50	0.00	11,313.080	8,361.804	3.87	3.87	129.57
302.00	0.00	15,636.984	8,937.000	4.14	4.14	177.88
302.50	0.00	20,245.564	9,500.189	4.40	4.40	229.35
303.00	0.00	25,140.041	10,080.585	4.67	4.67	284.00
303.50	0.00	30,329.018	10,678.189	4.94	4.94	341.93
304.00	0.00	35,821.098	11,293.000	5.23	5.23	403.24
304.50	0.00	41,633.561	11,960.042	5.54	5.54	468.13
304.60	0.00	42,836.344	12,095.747	5.60	5.60	481.56
305.00	5.45	47,784.329	12,646.222	5.85	11.30	542.24
305.50	26.33	54,282.973	13,351.542	6.18	32.51	635.66
306.00	65.17	61,139.061	14,076.000	6.52	71.69	751.01

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	5.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	318.75 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
318.75	0.00	0.000	5,775.000	0.00	0.00	0.00
319.25	0.00	3,016.988	6,292.760	0.73	0.73	34.25
319.75	0.00	6,295.392	6,824.450	0.79	0.79	70.74
320.25	0.00	9,845.031	7,377.700	0.85	0.85	110.24
320.75	0.00	13,676.685	7,952.510	0.92	0.92	152.88
321.25	0.00	17,799.334	8,534.393	0.99	0.99	198.76
321.65	0.00	21,306.353	9,002.788	1.04	1.04	237.78
321.75	0.09	22,212.578	9,121.842	1.06	1.15	247.95
322.25	3.05	26,924.435	9,728.842	1.13	4.17	303.33
322.75	9.41	31,944.679	10,355.393	1.20	10.61	365.55
323.25	18.05	37,281.961	10,992.449	1.27	19.32	433.57
323.75	25.61	42,939.106	11,639.212	1.35	26.96	504.06
324.25	30.41	48,924.255	12,304.462	1.42	31.84	575.44
324.50	32.53	52,042.719	12,644.020	1.46	33.99	612.24
324.75	36.41	55,246.650	12,988.199	1.50	37.91	651.76
325.25	44.20	61,958.344	14,035.096	1.62	45.83	734.25
325.50	47.73	65,556.235	14,751.000	1.71	49.43	777.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	5.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	318.75 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
318.75	0.00	0.000	5,775.000	0.00	0.00	0.00
319.25	0.00	3,016.988	6,292.760	0.73	0.73	34.25
319.75	0.00	6,295.392	6,824.450	0.79	0.79	70.74
320.25	0.00	9,845.031	7,377.700	0.85	0.85	110.24
320.75	0.00	13,676.685	7,952.510	0.92	0.92	152.88
321.25	0.00	17,799.334	8,534.393	0.99	0.99	198.76
321.65	0.00	21,306.353	9,002.788	1.04	1.04	237.78
321.75	0.09	22,212.578	9,121.842	1.06	1.15	247.95
322.25	3.05	26,924.435	9,728.842	1.13	4.17	303.33
322.75	9.41	31,944.679	10,355.393	1.20	10.61	365.55
323.25	18.05	37,281.961	10,992.449	1.27	19.32	433.57
323.75	25.61	42,939.106	11,639.212	1.35	26.96	504.06
324.25	30.41	48,924.255	12,304.462	1.42	31.84	575.44
324.50	32.53	52,042.719	12,644.020	1.46	33.99	612.24
324.75	36.41	55,246.650	12,988.199	1.50	37.91	651.76
325.25	44.20	61,958.344	14,035.096	1.62	45.83	734.25
325.50	47.73	65,556.235	14,751.000	1.71	49.43	777.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	5.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	318.75 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
318.75	0.00	0.000	5,775.000	0.00	0.00	0.00
319.25	0.00	3,016.988	6,292.760	0.73	0.73	34.25
319.75	0.00	6,295.392	6,824.450	0.79	0.79	70.74
320.25	0.00	9,845.031	7,377.700	0.85	0.85	110.24
320.75	0.00	13,676.685	7,952.510	0.92	0.92	152.88
321.25	0.00	17,799.334	8,534.393	0.99	0.99	198.76
321.65	0.00	21,306.353	9,002.788	1.04	1.04	237.78
321.75	0.09	22,212.578	9,121.842	1.06	1.15	247.95
322.25	3.05	26,924.435	9,728.842	1.13	4.17	303.33
322.75	9.41	31,944.679	10,355.393	1.20	10.61	365.55
323.25	18.05	37,281.961	10,992.449	1.27	19.32	433.57
323.75	25.61	42,939.106	11,639.212	1.35	26.96	504.06
324.25	30.41	48,924.255	12,304.462	1.42	31.84	575.44
324.50	32.53	52,042.719	12,644.020	1.46	33.99	612.24
324.75	36.41	55,246.650	12,988.199	1.50	37.91	651.76
325.25	44.20	61,958.344	14,035.096	1.62	45.83	734.25
325.50	47.73	65,556.235	14,751.000	1.71	49.43	777.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	5.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	318.75 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
318.75	0.00	0.000	5,775.000	0.00	0.00	0.00
319.25	0.00	3,016.988	6,292.760	0.73	0.73	34.25
319.75	0.00	6,295.392	6,824.450	0.79	0.79	70.74
320.25	0.00	9,845.031	7,377.700	0.85	0.85	110.24
320.75	0.00	13,676.685	7,952.510	0.92	0.92	152.88
321.25	0.00	17,799.334	8,534.393	0.99	0.99	198.76
321.65	0.00	21,306.353	9,002.788	1.04	1.04	237.78
321.75	0.09	22,212.578	9,121.842	1.06	1.15	247.95
322.25	3.05	26,924.435	9,728.842	1.13	4.17	303.33
322.75	9.41	31,944.679	10,355.393	1.20	10.61	365.55
323.25	18.05	37,281.961	10,992.449	1.27	19.32	433.57
323.75	25.61	42,939.106	11,639.212	1.35	26.96	504.06
324.25	30.41	48,924.255	12,304.462	1.42	31.84	575.44
324.50	32.53	52,042.719	12,644.020	1.46	33.99	612.24
324.75	36.41	55,246.650	12,988.199	1.50	37.91	651.76
325.25	44.20	61,958.344	14,035.096	1.62	45.83	734.25
325.50	47.73	65,556.235	14,751.000	1.71	49.43	777.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Infiltration Basin #3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	5.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	318.75 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
318.75	0.00	0.000	5,775.000	0.00	0.00	0.00
319.25	0.00	3,016.988	6,292.760	0.73	0.73	34.25
319.75	0.00	6,295.392	6,824.450	0.79	0.79	70.74
320.25	0.00	9,845.031	7,377.700	0.85	0.85	110.24
320.75	0.00	13,676.685	7,952.510	0.92	0.92	152.88
321.25	0.00	17,799.334	8,534.393	0.99	0.99	198.76
321.65	0.00	21,306.353	9,002.788	1.04	1.04	237.78
321.75	0.09	22,212.578	9,121.842	1.06	1.15	247.95
322.25	3.05	26,924.435	9,728.842	1.13	4.17	303.33
322.75	9.41	31,944.679	10,355.393	1.20	10.61	365.55
323.25	18.05	37,281.961	10,992.449	1.27	19.32	433.57
323.75	25.61	42,939.106	11,639.212	1.35	26.96	504.06
324.25	30.41	48,924.255	12,304.462	1.42	31.84	575.44
324.50	32.53	52,042.719	12,644.020	1.46	33.99	612.24
324.75	36.41	55,246.650	12,988.199	1.50	37.91	651.76
325.25	44.20	61,958.344	14,035.096	1.62	45.83	734.25
325.50	47.73	65,556.235	14,751.000	1.71	49.43	777.83

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Wet Extended Detention Pond #2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	320.00 ft
Volume (Initial)	64,178.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	9,800.000	0.00	0.00	0.00
315.50	0.00	5,049.879	10,402.513	0.00	0.00	56.11
316.00	0.00	10,405.509	11,023.000	0.00	0.00	115.62
316.50	0.00	16,059.878	11,596.905	0.00	0.00	178.44
317.00	0.00	22,004.841	12,185.373	0.00	0.00	244.50
317.50	0.00	28,247.679	12,788.405	0.00	0.00	313.86
318.00	0.00	34,795.674	13,406.000	0.00	0.00	386.62
318.50	0.00	41,655.699	14,036.517	0.00	0.00	462.84
319.00	0.00	48,834.605	14,681.523	0.00	0.00	542.61
319.50	0.00	56,339.637	15,341.017	0.00	0.00	626.00
320.00	0.00	64,178.037	16,015.000	0.00	0.00	713.09
320.50	0.00	72,356.712	16,702.103	0.00	0.00	803.96
321.00	0.00	80,882.545	17,403.637	0.00	0.00	898.69
321.50	0.00	89,762.753	18,119.603	0.00	0.00	997.36
321.60	0.00	91,581.955	18,264.527	0.00	0.00	1,017.58
322.00	1.72	99,004.553	18,850.000	0.00	1.72	1,101.77
322.50	7.77	108,614.929	19,593.906	0.00	7.77	1,214.60
323.00	9.95	118,600.858	20,352.208	0.00	9.95	1,327.74
323.50	10.80	128,969.536	21,124.906	0.00	10.80	1,443.79
324.00	12.06	139,728.163	21,912.000	0.00	12.06	1,564.59
324.50	13.02	151,496.345	25,199.000	0.00	13.02	1,696.31

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Wet Extended Detention Pond #2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	320.00 ft
Volume (Initial)	64,178.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	9,800.000	0.00	0.00	0.00
315.50	0.00	5,049.879	10,402.513	0.00	0.00	56.11
316.00	0.00	10,405.509	11,023.000	0.00	0.00	115.62
316.50	0.00	16,059.878	11,596.905	0.00	0.00	178.44
317.00	0.00	22,004.841	12,185.373	0.00	0.00	244.50
317.50	0.00	28,247.679	12,788.405	0.00	0.00	313.86
318.00	0.00	34,795.674	13,406.000	0.00	0.00	386.62
318.50	0.00	41,655.699	14,036.517	0.00	0.00	462.84
319.00	0.00	48,834.605	14,681.523	0.00	0.00	542.61
319.50	0.00	56,339.637	15,341.017	0.00	0.00	626.00
320.00	0.00	64,178.037	16,015.000	0.00	0.00	713.09
320.50	0.00	72,356.712	16,702.103	0.00	0.00	803.96
321.00	0.00	80,882.545	17,403.637	0.00	0.00	898.69
321.50	0.00	89,762.753	18,119.603	0.00	0.00	997.36
321.60	0.00	91,581.955	18,264.527	0.00	0.00	1,017.58
322.00	1.72	99,004.553	18,850.000	0.00	1.72	1,101.77
322.50	7.77	108,614.929	19,593.906	0.00	7.77	1,214.60
323.00	9.95	118,600.858	20,352.208	0.00	9.95	1,327.74
323.50	10.80	128,969.536	21,124.906	0.00	10.80	1,443.79
324.00	12.06	139,728.163	21,912.000	0.00	12.06	1,564.59
324.50	13.02	151,496.345	25,199.000	0.00	13.02	1,696.31

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Wet Extended Detention Pond #2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	320.00 ft
Volume (Initial)	64,178.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	9,800.000	0.00	0.00	0.00
315.50	0.00	5,049.879	10,402.513	0.00	0.00	56.11
316.00	0.00	10,405.509	11,023.000	0.00	0.00	115.62
316.50	0.00	16,059.878	11,596.905	0.00	0.00	178.44
317.00	0.00	22,004.841	12,185.373	0.00	0.00	244.50
317.50	0.00	28,247.679	12,788.405	0.00	0.00	313.86
318.00	0.00	34,795.674	13,406.000	0.00	0.00	386.62
318.50	0.00	41,655.699	14,036.517	0.00	0.00	462.84
319.00	0.00	48,834.605	14,681.523	0.00	0.00	542.61
319.50	0.00	56,339.637	15,341.017	0.00	0.00	626.00
320.00	0.00	64,178.037	16,015.000	0.00	0.00	713.09
320.50	0.00	72,356.712	16,702.103	0.00	0.00	803.96
321.00	0.00	80,882.545	17,403.637	0.00	0.00	898.69
321.50	0.00	89,762.753	18,119.603	0.00	0.00	997.36
321.60	0.00	91,581.955	18,264.527	0.00	0.00	1,017.58
322.00	1.72	99,004.553	18,850.000	0.00	1.72	1,101.77
322.50	7.77	108,614.929	19,593.906	0.00	7.77	1,214.60
323.00	9.95	118,600.858	20,352.208	0.00	9.95	1,327.74
323.50	10.80	128,969.536	21,124.906	0.00	10.80	1,443.79
324.00	12.06	139,728.163	21,912.000	0.00	12.06	1,564.59
324.50	13.02	151,496.345	25,199.000	0.00	13.02	1,696.31

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Wet Extended Detention Pond #2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	320.00 ft
Volume (Initial)	64,178.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	9,800.000	0.00	0.00	0.00
315.50	0.00	5,049.879	10,402.513	0.00	0.00	56.11
316.00	0.00	10,405.509	11,023.000	0.00	0.00	115.62
316.50	0.00	16,059.878	11,596.905	0.00	0.00	178.44
317.00	0.00	22,004.841	12,185.373	0.00	0.00	244.50
317.50	0.00	28,247.679	12,788.405	0.00	0.00	313.86
318.00	0.00	34,795.674	13,406.000	0.00	0.00	386.62
318.50	0.00	41,655.699	14,036.517	0.00	0.00	462.84
319.00	0.00	48,834.605	14,681.523	0.00	0.00	542.61
319.50	0.00	56,339.637	15,341.017	0.00	0.00	626.00
320.00	0.00	64,178.037	16,015.000	0.00	0.00	713.09
320.50	0.00	72,356.712	16,702.103	0.00	0.00	803.96
321.00	0.00	80,882.545	17,403.637	0.00	0.00	898.69
321.50	0.00	89,762.753	18,119.603	0.00	0.00	997.36
321.60	0.00	91,581.955	18,264.527	0.00	0.00	1,017.58
322.00	1.72	99,004.553	18,850.000	0.00	1.72	1,101.77
322.50	7.77	108,614.929	19,593.906	0.00	7.77	1,214.60
323.00	9.95	118,600.858	20,352.208	0.00	9.95	1,327.74
323.50	10.80	128,969.536	21,124.906	0.00	10.80	1,443.79
324.00	12.06	139,728.163	21,912.000	0.00	12.06	1,564.59
324.50	13.02	151,496.345	25,199.000	0.00	13.02	1,696.31

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Wet Extended Detention Pond #2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	320.00 ft
Volume (Initial)	64,178.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
315.00	0.00	0.000	9,800.000	0.00	0.00	0.00
315.50	0.00	5,049.879	10,402.513	0.00	0.00	56.11
316.00	0.00	10,405.509	11,023.000	0.00	0.00	115.62
316.50	0.00	16,059.878	11,596.905	0.00	0.00	178.44
317.00	0.00	22,004.841	12,185.373	0.00	0.00	244.50
317.50	0.00	28,247.679	12,788.405	0.00	0.00	313.86
318.00	0.00	34,795.674	13,406.000	0.00	0.00	386.62
318.50	0.00	41,655.699	14,036.517	0.00	0.00	462.84
319.00	0.00	48,834.605	14,681.523	0.00	0.00	542.61
319.50	0.00	56,339.637	15,341.017	0.00	0.00	626.00
320.00	0.00	64,178.037	16,015.000	0.00	0.00	713.09
320.50	0.00	72,356.712	16,702.103	0.00	0.00	803.96
321.00	0.00	80,882.545	17,403.637	0.00	0.00	898.69
321.50	0.00	89,762.753	18,119.603	0.00	0.00	997.36
321.60	0.00	91,581.955	18,264.527	0.00	0.00	1,017.58
322.00	1.72	99,004.553	18,850.000	0.00	1.72	1,101.77
322.50	7.77	108,614.929	19,593.906	0.00	7.77	1,214.60
323.00	9.95	118,600.858	20,352.208	0.00	9.95	1,327.74
323.50	10.80	128,969.536	21,124.906	0.00	10.80	1,443.79
324.00	12.06	139,728.163	21,912.000	0.00	12.06	1,564.59
324.50	13.02	151,496.345	25,199.000	0.00	13.02	1,696.31

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Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	300.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	306.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	304.60 (N/A)	306.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	306.00
5.00	305.25
10.00	304.60
15.00	304.60
20.00	305.25
25.00	306.00

Lowest Elevation 304.60 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
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Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	300.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	306.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	304.60 (N/A)	306.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	306.00
5.00	305.25
10.00	304.60
15.00	304.60
20.00	305.25
25.00	306.00

Lowest Elevation 304.60 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type Free Outfall

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	300.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	306.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	304.60 (N/A)	306.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	306.00
5.00	305.25
10.00	304.60
15.00	304.60
20.00	305.25
25.00	306.00

Lowest Elevation 304.60 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type Free Outfall

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	300.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	306.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	304.60 (N/A)	306.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	306.00
5.00	305.25
10.00	304.60
15.00	304.60
20.00	305.25
25.00	306.00

Lowest Elevation 304.60 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type Free Outfall

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	300.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	306.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	304.60 (N/A)	306.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: 4A Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	306.00
5.00	305.25
10.00	304.60
15.00	304.60
20.00	305.25
25.00	306.00

Lowest Elevation 304.60 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
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Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	320.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	315.00	320.50
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	317.00	320.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	319.25	320.50
Culvert-Circular	Culvert - 1	Forward	TW	315.00	320.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	319.25 ft
Weir Length	1.75 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	315.00 ft
Orifice Diameter	17.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	30.00 ft
Length (Computed Barrel)	30.02 ft
Slope (Computed)	0.033 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.079
T2 ratio (HW/D)	1.181
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	317.16 ft	T1 Flow	15.55 ft ³ /s
T2 Elevation	317.36 ft	T2 Flow	17.77 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Orifice - 2	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	317.00 ft
Orifice Diameter	5.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	320.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	315.00	320.50
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	317.00	320.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	319.25	320.50
Culvert-Circular	Culvert - 1	Forward	TW	315.00	320.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	319.25 ft
Weir Length	1.75 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	315.00 ft
Orifice Diameter	17.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	30.00 ft
Length (Computed Barrel)	30.02 ft
Slope (Computed)	0.033 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.079
T2 ratio (HW/D)	1.181
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	317.16 ft	T1 Flow	15.55 ft ³ /s
T2 Elevation	317.36 ft	T2 Flow	17.77 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Orifice - 2	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	317.00 ft
Orifice Diameter	5.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	320.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	315.00	320.50
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	317.00	320.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	319.25	320.50
Culvert-Circular	Culvert - 1	Forward	TW	315.00	320.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	319.25 ft
Weir Length	1.75 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	315.00 ft
Orifice Diameter	17.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	30.00 ft
Length (Computed Barrel)	30.02 ft
Slope (Computed)	0.033 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.079
T2 ratio (HW/D)	1.181
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	317.16 ft	T1 Flow	15.55 ft ³ /s
T2 Elevation	317.36 ft	T2 Flow	17.77 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Orifice - 2	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	317.00 ft
Orifice Diameter	5.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	320.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	315.00	320.50
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	317.00	320.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	319.25	320.50
Culvert-Circular	Culvert - 1	Forward	TW	315.00	320.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	319.25 ft
Weir Length	1.75 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	315.00 ft
Orifice Diameter	17.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	30.00 ft
Length (Computed Barrel)	30.02 ft
Slope (Computed)	0.033 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.079
T2 ratio (HW/D)	1.181
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	317.16 ft	T1 Flow	15.55 ft ³ /s
T2 Elevation	317.36 ft	T2 Flow	17.77 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Orifice - 2	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	317.00 ft
Orifice Diameter	5.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	320.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	315.00	320.50
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	317.00	320.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	319.25	320.50
Culvert-Circular	Culvert - 1	Forward	TW	315.00	320.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	319.25 ft
Weir Length	1.75 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	315.00 ft
Orifice Diameter	17.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	30.00 ft
Length (Computed Barrel)	30.02 ft
Slope (Computed)	0.033 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.079
T2 ratio (HW/D)	1.181
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	317.16 ft	T1 Flow	15.55 ft ³ /s
T2 Elevation	317.36 ft	T2 Flow	17.77 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Det 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Orifice - 2	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	317.00 ft
Orifice Diameter	5.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	296.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	300.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	296.00 (N/A)	300.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	300.00
21.00	298.00
37.00	296.00
50.00	298.00
58.00	300.00

Lowest Elevation 296.00 ft

Weir Coefficient 3.33 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	296.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	300.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	296.00 (N/A)	300.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	300.00
21.00	298.00
37.00	296.00
50.00	298.00
58.00	300.00

Lowest Elevation 296.00 ft

Weir Coefficient 3.33 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	296.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	300.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	296.00 (N/A)	300.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	300.00
21.00	298.00
37.00	296.00
50.00	298.00
58.00	300.00

Lowest Elevation 296.00 ft

Weir Coefficient 3.33 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	296.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	300.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	296.00 (N/A)	300.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	300.00
21.00	298.00
37.00	296.00
50.00	298.00
58.00	300.00

Lowest Elevation 296.00 ft

Weir Coefficient 3.33 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	296.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	300.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Irregular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	296.00 (N/A)	300.00 (N/A)

Watershed

Subsection: Outlet Input Data

Label: DP 4 Weir

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	300.00
21.00	298.00
37.00	296.00
50.00	298.00
58.00	300.00

Lowest Elevation 296.00 ft

Weir Coefficient 3.33 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	345.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	351.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	345.75	351.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	350.50	351.00
Culvert-Circular	Culvert - 1	Forward	TW	345.00	351.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	350.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.050 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.070
T2 ratio (HW/D)	1.172
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	346.61 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	346.76 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	345.75 ft
Orifice Diameter	16.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	345.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	351.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	345.75	351.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	350.50	351.00
Culvert-Circular	Culvert - 1	Forward	TW	345.00	351.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	350.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.050 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.070
T2 ratio (HW/D)	1.172
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	346.61 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	346.76 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	345.75 ft
Orifice Diameter	16.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	345.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	351.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	345.75	351.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	350.50	351.00
Culvert-Circular	Culvert - 1	Forward	TW	345.00	351.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	350.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.050 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.070
T2 ratio (HW/D)	1.172
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	346.61 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	346.76 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	345.75 ft
Orifice Diameter	16.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	345.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	351.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	345.75	351.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	350.50	351.00
Culvert-Circular	Culvert - 1	Forward	TW	345.00	351.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1

Structure Type: Rectangular Weir

Number of Openings	1
Elevation	350.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.050 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.070
T2 ratio (HW/D)	1.172
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	346.61 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	346.76 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	345.75 ft
Orifice Diameter	16.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	345.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	351.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	345.75	351.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	350.50	351.00
Culvert-Circular	Culvert - 1	Forward	TW	345.00	351.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	350.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.050 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.070
T2 ratio (HW/D)	1.172
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	346.61 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	346.76 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 1

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	345.75 ft
Orifice Diameter	16.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	325.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	331.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	326.20	331.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	330.25	331.00
Culvert-Circular	Culvert - 1	Forward	TW	325.00	331.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	1
Elevation	326.20 ft
Orifice Diameter	24.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.02 ft
Slope (Computed)	0.020 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.085
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	327.71 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	327.97 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	330.25 ft
Weir Length	5.50 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	325.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	331.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	326.20	331.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	330.25	331.00
Culvert-Circular	Culvert - 1	Forward	TW	325.00	331.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Orifice - 1

Structure Type: Orifice-Circular

Number of Openings	1
Elevation	326.20 ft
Orifice Diameter	24.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.02 ft
Slope (Computed)	0.020 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.085
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	327.71 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	327.97 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	330.25 ft
Weir Length	5.50 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	325.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	331.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	326.20	331.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	330.25	331.00
Culvert-Circular	Culvert - 1	Forward	TW	325.00	331.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Orifice - 1

Structure Type: Orifice-Circular

Number of Openings	1
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Elevation	326.20 ft
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Orifice Diameter	24.0 in
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Orifice Coefficient	0.600
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Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.02 ft
Slope (Computed)	0.020 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.085
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	327.71 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	327.97 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	330.25 ft
Weir Length	5.50 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	325.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	331.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	326.20	331.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	330.25	331.00
Culvert-Circular	Culvert - 1	Forward	TW	325.00	331.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Orifice - 1

Structure Type: Orifice-Circular

Number of Openings	1
Elevation	326.20 ft
Orifice Diameter	24.0 in
Orifice Coefficient	0.600

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.02 ft
Slope (Computed)	0.020 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.085
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	327.71 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	327.97 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	330.25 ft
Weir Length	5.50 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	325.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	331.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	326.20	331.00
Rectangular Weir	Weir - 1	Forward	Culvert - 1	330.25	331.00
Culvert-Circular	Culvert - 1	Forward	TW	325.00	331.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Orifice - 1

Structure Type: Orifice-Circular

Number of Openings	1
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Elevation	326.20 ft
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Orifice Diameter	24.0 in
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Orifice Coefficient	0.600
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Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.02 ft
Slope (Computed)	0.020 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.085
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	327.71 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	327.97 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	330.25 ft
Weir Length	5.50 ft
Weir Coefficient	3.00 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	324.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.60	324.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	323.50	324.50
Culvert-Circular	Culvert - 1	Forward	TW	319.50	324.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	323.50 ft
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	193.00 ft
Length (Computed Barrel)	193.00 ft
Slope (Computed)	0.003 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.14 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.29 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	3
Elevation	321.60 ft
Orifice Diameter	15.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	324.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.60	324.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	323.50	324.50
Culvert-Circular	Culvert - 1	Forward	TW	319.50	324.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	323.50 ft
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	193.00 ft
Length (Computed Barrel)	193.00 ft
Slope (Computed)	0.003 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.14 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.29 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	3
Elevation	321.60 ft
Orifice Diameter	15.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	324.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.60	324.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	323.50	324.50
Culvert-Circular	Culvert - 1	Forward	TW	319.50	324.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1

Structure Type: Rectangular Weir

Number of Openings	1
Elevation	323.50 ft
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	193.00 ft
Length (Computed Barrel)	193.00 ft
Slope (Computed)	0.003 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.14 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.29 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	3
Elevation	321.60 ft
Orifice Diameter	15.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	324.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.60	324.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	323.50	324.50
Culvert-Circular	Culvert - 1	Forward	TW	319.50	324.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1

Structure Type: Rectangular Weir

Number of Openings	1
Elevation	323.50 ft
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	193.00 ft
Length (Computed Barrel)	193.00 ft
Slope (Computed)	0.003 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.14 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.29 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	3
Elevation	321.60 ft
Orifice Diameter	15.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	315.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	324.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.60	324.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	323.50	324.50
Culvert-Circular	Culvert - 1	Forward	TW	319.50	324.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1

Structure Type: Rectangular Weir

Number of Openings	1
Elevation	323.50 ft
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	193.00 ft
Length (Computed Barrel)	193.00 ft
Slope (Computed)	0.003 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.018
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.14 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.29 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 2A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	3
Elevation	321.60 ft
Orifice Diameter	15.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	290.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	296.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	292.30	296.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	295.00	296.50
Culvert-Circular	Culvert - 1	Forward	TW	290.00	296.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.010 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.090
T2 ratio (HW/D)	1.192
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	292.73 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	292.98 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	295.00 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	292.30 ft
Orifice Diameter	23.5 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	290.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	296.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	292.30	296.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	295.00	296.50
Culvert-Circular	Culvert - 1	Forward	TW	290.00	296.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.010 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.090
T2 ratio (HW/D)	1.192
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	292.73 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	292.98 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	295.00 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	292.30 ft
Orifice Diameter	23.5 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	290.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	296.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	292.30	296.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	295.00	296.50
Culvert-Circular	Culvert - 1	Forward	TW	290.00	296.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.010 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.090
T2 ratio (HW/D)	1.192
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	292.73 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	292.98 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	295.00 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	292.30 ft
Orifice Diameter	23.5 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	290.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	296.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	292.30	296.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	295.00	296.50
Culvert-Circular	Culvert - 1	Forward	TW	290.00	296.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.010 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.090
T2 ratio (HW/D)	1.192
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	292.73 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	292.98 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	295.00 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	292.30 ft
Orifice Diameter	23.5 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	290.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	296.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	292.30	296.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	295.00	296.50
Culvert-Circular	Culvert - 1	Forward	TW	290.00	296.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.010 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.090
T2 ratio (HW/D)	1.192
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	292.73 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	292.98 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	295.00 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s
Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	292.30 ft
Orifice Diameter	23.5 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	318.75 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	325.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.65	325.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	324.50	325.50
Culvert-Circular	Culvert - 1	Forward	TW	318.75	325.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.008 ft/ft

Outlet Control Data

Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.092
T2 ratio (HW/D)	1.193
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.48 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	321.73 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	324.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	321.65 ft
Orifice Diameter	21.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	318.75 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	325.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.65	325.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	324.50	325.50
Culvert-Circular	Culvert - 1	Forward	TW	318.75	325.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.008 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.092
T2 ratio (HW/D)	1.193
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.48 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	321.73 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	324.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	321.65 ft
Orifice Diameter	21.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	318.75 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	325.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.65	325.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	324.50	325.50
Culvert-Circular	Culvert - 1	Forward	TW	318.75	325.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.008 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.092
T2 ratio (HW/D)	1.193
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.48 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	321.73 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	324.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	321.65 ft
Orifice Diameter	21.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	318.75 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	325.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.65	325.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	324.50	325.50
Culvert-Circular	Culvert - 1	Forward	TW	318.75	325.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.008 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.092
T2 ratio (HW/D)	1.193
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.48 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	321.73 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	324.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s

Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	321.65 ft
Orifice Diameter	21.0 in
Orifice Coefficient	0.600

Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall

Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	318.75 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	325.50 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	321.65	325.50
Rectangular Weir	Weir - 1	Forward	Culvert - 1	324.50	325.50
Culvert-Circular	Culvert - 1	Forward	TW	318.75	325.50
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	30.0 in
Length	100.00 ft
Length (Computed Barrel)	100.00 ft
Slope (Computed)	0.008 ft/ft

Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.009
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.092
T2 ratio (HW/D)	1.193
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	321.48 ft	T1 Flow	27.16 ft ³ /s
T2 Elevation	321.73 ft	T2 Flow	31.05 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: OCS 3A

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	324.50 ft
Weir Length	5.00 ft
Weir Coefficient	3.00 (ft ^{0.5})/s
Structure ID: Orifice - 1	
Structure Type: Orifice-Circular	
Number of Openings	2
Elevation	321.65 ft
Orifice Diameter	21.0 in
Orifice Coefficient	0.600
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	319.17 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	322.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	North Pipe	Forward	TW	319.17	322.00
Culvert-Circular	South Pipe	Forward	TW	319.27	322.00
Irregular Weir	Weir - 1	Forward	TW	320.30	322.00
Irregular Weir	Weir - 2	Forward	TW	321.00	322.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: North Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.046 ft/ft

Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.073
T2 ratio (HW/D)	1.174
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.78 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	320.93 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: South Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.03 ft
Slope (Computed)	0.054 ft/ft

Outlet Control Data

Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.068
T2 ratio (HW/D)	1.170
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.87 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.02 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

Return Event: 1 years

Storm Event: TypeIII 24hr (2.8 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
68.00	320.90
148.00	320.30
328.00	322.00

Lowest Elevation 320.30 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: Weir - 2

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
13.00	321.00
27.00	322.00

Lowest Elevation 321.00 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	319.17 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	322.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	North Pipe	Forward	TW	319.17	322.00
Culvert-Circular	South Pipe	Forward	TW	319.27	322.00
Irregular Weir	Weir - 1	Forward	TW	320.30	322.00
Irregular Weir	Weir - 2	Forward	TW	321.00	322.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: North Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.046 ft/ft

Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.073
T2 ratio (HW/D)	1.174
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.78 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	320.93 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: South Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.03 ft
Slope (Computed)	0.054 ft/ft

Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.068
T2 ratio (HW/D)	1.170
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.87 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.02 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 2 yrs

Return Event: 2 years

Storm Event: TypeIII 24hr (3.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
68.00	320.90
148.00	320.30
328.00	322.00

Lowest Elevation 320.30 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: Weir - 2

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
13.00	321.00
27.00	322.00

Lowest Elevation 321.00 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	319.17 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	322.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	North Pipe	Forward	TW	319.17	322.00
Culvert-Circular	South Pipe	Forward	TW	319.27	322.00
Irregular Weir	Weir - 1	Forward	TW	320.30	322.00
Irregular Weir	Weir - 2	Forward	TW	321.00	322.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: North Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.046 ft/ft

Outlet Control Data

Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.073
T2 ratio (HW/D)	1.174
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.78 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	320.93 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: South Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.03 ft
Slope (Computed)	0.054 ft/ft

Outlet Control Data

Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.068
T2 ratio (HW/D)	1.170
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.87 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.02 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 10 yrs

Return Event: 10 years

Storm Event: TypeIII 24hr (5.1 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
68.00	320.90
148.00	320.30
328.00	322.00

Lowest Elevation 320.30 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: Weir - 2

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
13.00	321.00
27.00	322.00

Lowest Elevation 321.00 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	319.17 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	322.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	North Pipe	Forward	TW	319.17	322.00
Culvert-Circular	South Pipe	Forward	TW	319.27	322.00
Irregular Weir	Weir - 1	Forward	TW	320.30	322.00
Irregular Weir	Weir - 2	Forward	TW	321.00	322.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: North Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.046 ft/ft
Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.073
T2 ratio (HW/D)	1.174
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.78 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	320.93 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: South Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.03 ft
Slope (Computed)	0.054 ft/ft

Outlet Control Data

Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft

Inlet Control Data

Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.068
T2 ratio (HW/D)	1.170
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.87 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.02 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
68.00	320.90
148.00	320.30
328.00	322.00

Lowest Elevation 320.30 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: Weir - 2

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
13.00	321.00
27.00	322.00

Lowest Elevation 321.00 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Requested Pond Water Surface Elevations

Minimum (Headwater)	319.17 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	322.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	North Pipe	Forward	TW	319.17	322.00
Culvert-Circular	South Pipe	Forward	TW	319.27	322.00
Irregular Weir	Weir - 1	Forward	TW	320.30	322.00
Irregular Weir	Weir - 2	Forward	TW	321.00	322.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: North Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.02 ft
Slope (Computed)	0.046 ft/ft
Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.073
T2 ratio (HW/D)	1.174
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.78 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	320.93 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: South Pipe	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	18.0 in
Length	20.00 ft
Length (Computed Barrel)	20.03 ft
Slope (Computed)	0.054 ft/ft
Outlet Control Data	
Manning's n	0.012
Ke	0.200
Kb	0.016
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
K	0.0045
M	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	1.068
T2 ratio (HW/D)	1.170
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	320.87 ft	T1 Flow	7.58 ft ³ /s
T2 Elevation	321.02 ft	T2 Flow	8.66 ft ³ /s

Watershed

Subsection: Outlet Input Data

Label: Pond Outlet

Scenario: WESTCHESTER-NEW - Synthetic Curve, 100 yrs

Return Event: 100 years

Storm Event: TypeIII 24hr (9.0 in)

Structure ID: Weir - 1

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
68.00	320.90
148.00	320.30
328.00	322.00

Lowest Elevation 320.30 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: Weir - 2

Structure Type: Irregular Weir

Station (ft)	Elevation (ft)
0.00	322.00
13.00	321.00
27.00	322.00

Lowest Elevation 321.00 ft

Weir Coefficient 3.00 (ft^{0.5})/s

Structure ID: TW

Structure Type: TW Setup, DS Channel

Tailwater Type	Free Outfall
----------------	--------------

Convergence Tolerances

Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

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Scenario: WESTCHESTER-NEW - Synthetic Curve, 1 yrs

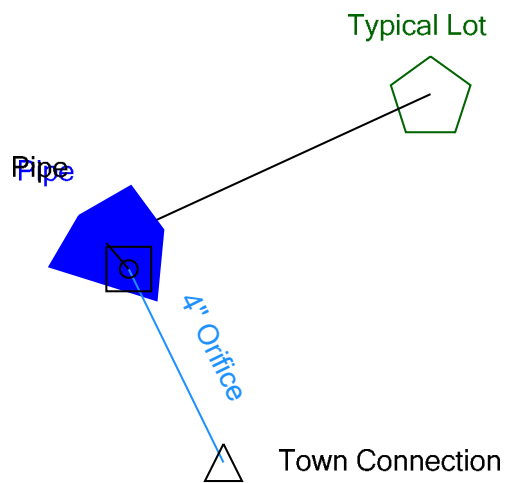


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	Diverted Hydrograph, 25 years (WESTCHESTER-NEW - Synthetic Curve, 25 yrs)	16
Pipe		
	Elevation-Volume-Flow Table (Pond), 25 years (WESTCHESTER-NEW - Synthetic Curve, 25 yrs)	19

Watershed

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Typical Lot	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.100	0.59

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Town Connection	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.150	0.51

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
Pipe (IN)	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.100	0.59	(N/A)	(N/A)
Pipe (OUT)	WESTCHESTER-NEW - Synthetic Curve, 25 yrs	25	2,457.000	12.150	0.51	101.89	142.000

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time-Depth Curve: TypeIII 24hr (6.4 in)

Label	TypeIII 24hr (6.4 in)
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.2
2.500	0.2	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.3	0.3
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.3	0.3	0.3	0.4
5.000	0.4	0.4	0.4	0.4	0.4
5.500	0.4	0.4	0.4	0.4	0.4
6.000	0.5	0.5	0.5	0.5	0.5
6.500	0.5	0.5	0.5	0.6	0.6
7.000	0.6	0.6	0.6	0.6	0.6
7.500	0.6	0.7	0.7	0.7	0.7
8.000	0.7	0.7	0.8	0.8	0.8
8.500	0.8	0.8	0.9	0.9	0.9
9.000	0.9	1.0	1.0	1.0	1.0
9.500	1.1	1.1	1.1	1.1	1.2
10.000	1.2	1.2	1.3	1.3	1.3
10.500	1.4	1.4	1.5	1.5	1.5
11.000	1.6	1.6	1.7	1.8	1.8
11.500	1.9	2.0	2.2	2.4	2.7
12.000	3.2	3.7	4.0	4.2	4.4
12.500	4.5	4.6	4.6	4.7	4.7
13.000	4.8	4.8	4.9	4.9	5.0
13.500	5.0	5.0	5.1	5.1	5.1
14.000	5.2	5.2	5.2	5.3	5.3
14.500	5.3	5.3	5.4	5.4	5.4
15.000	5.5	5.5	5.5	5.5	5.5
15.500	5.6	5.6	5.6	5.6	5.6
16.000	5.7	5.7	5.7	5.7	5.7

Watershed

Subsection: Time-Depth Curve

Label: WESTCHESTER-NEW

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

CUMULATIVE RAINFALL (in)

Output Time Increment = 0.100 hours

Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	5.7	5.7	5.8	5.8	5.8
17.000	5.8	5.8	5.8	5.8	5.9
17.500	5.9	5.9	5.9	5.9	5.9
18.000	5.9	5.9	5.9	6.0	6.0
18.500	6.0	6.0	6.0	6.0	6.0
19.000	6.0	6.0	6.0	6.0	6.1
19.500	6.1	6.1	6.1	6.1	6.1
20.000	6.1	6.1	6.1	6.1	6.1
20.500	6.1	6.2	6.2	6.2	6.2
21.000	6.2	6.2	6.2	6.2	6.2
21.500	6.2	6.2	6.2	6.2	6.3
22.000	6.3	6.3	6.3	6.3	6.3
22.500	6.3	6.3	6.3	6.3	6.3
23.000	6.3	6.3	6.3	6.3	6.3
23.500	6.4	6.4	6.4	6.4	6.4
24.000	6.4	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Runoff CN-Area

Label: Typical Lot

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Runoff Curve Number Data

Soil/Surface Description	CN	Area (ft ²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil C	98.000	4,800.000	0.0	0.0	98.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	4,800.000	(N/A)	(N/A)	98.000

Watershed

Subsection: Unit Hydrograph Summary

Label: Typical Lot

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Storm Event	TypeIII 24hr (6.4 in)
Return Event	25 years
Duration	35.000 hours
Depth	6.4 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	4,800.000 ft ²
Computational Time Increment	0.011 hours
Time to Peak (Computed)	12.095 hours
Flow (Peak, Computed)	0.59 ft ³ /s
Output Increment	0.050 hours
Time to Flow (Peak Interpolated Output)	12.100 hours
Flow (Peak Interpolated Output)	0.59 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	4,800.000 ft ²
Maximum Retention (Pervious)	0.2 in
Maximum Retention (Pervious, 20 percent)	0.0 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.1 in
Runoff Volume (Pervious)	2,456.587 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,457.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.083 hours
Computational Time Increment	0.011 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749

Watershed

Subsection: Unit Hydrograph Summary

Label: Typical Lot

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

SCS Unit Hydrograph Parameters

Receding/Rising, Tr/Tp	1.670
Unit peak, qp	1.50 ft ³ /s
Unit peak time, Tp	0.056 hours
Unit receding limb, Tr	0.222 hours
Total unit time, Tb	0.278 hours

Watershed

Subsection: Time vs. Elevation

Label: Pipe (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	100.00	100.00	100.00	100.00	100.00
0.250	100.00	100.00	100.00	100.00	100.00
0.500	100.00	100.00	100.00	100.00	100.00
0.750	100.00	100.00	100.00	100.01	100.01
1.000	100.01	100.01	100.01	100.01	100.02
1.250	100.02	100.02	100.02	100.02	100.02
1.500	100.03	100.03	100.03	100.03	100.03
1.750	100.03	100.03	100.03	100.03	100.04
2.000	100.04	100.04	100.04	100.04	100.04
2.250	100.04	100.04	100.04	100.04	100.05
2.500	100.05	100.05	100.05	100.05	100.05
2.750	100.05	100.05	100.05	100.05	100.05
3.000	100.05	100.05	100.05	100.05	100.05
3.250	100.05	100.06	100.06	100.06	100.06
3.500	100.06	100.06	100.06	100.06	100.06
3.750	100.06	100.06	100.06	100.06	100.06
4.000	100.06	100.06	100.06	100.06	100.06
4.250	100.06	100.06	100.06	100.06	100.06
4.500	100.06	100.06	100.06	100.06	100.06
4.750	100.06	100.06	100.06	100.07	100.07
5.000	100.07	100.07	100.07	100.07	100.07
5.250	100.07	100.07	100.07	100.07	100.07
5.500	100.07	100.07	100.07	100.07	100.07
5.750	100.07	100.07	100.07	100.07	100.07
6.000	100.07	100.07	100.07	100.07	100.07
6.250	100.07	100.07	100.08	100.08	100.08
6.500	100.08	100.08	100.08	100.08	100.08
6.750	100.08	100.08	100.08	100.08	100.08
7.000	100.09	100.09	100.09	100.09	100.09
7.250	100.09	100.09	100.09	100.09	100.09
7.500	100.09	100.09	100.09	100.09	100.10
7.750	100.10	100.10	100.10	100.10	100.10
8.000	100.10	100.10	100.10	100.10	100.10
8.250	100.10	100.11	100.11	100.11	100.11
8.500	100.11	100.11	100.11	100.11	100.12
8.750	100.12	100.12	100.12	100.12	100.12
9.000	100.12	100.12	100.12	100.13	100.13
9.250	100.13	100.13	100.13	100.13	100.13
9.500	100.13	100.14	100.14	100.14	100.14
9.750	100.14	100.14	100.14	100.14	100.15

Watershed

Subsection: Time vs. Elevation

Label: Pipe (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
10.000	100.15	100.15	100.15	100.15	100.15
10.250	100.16	100.16	100.16	100.16	100.16
10.500	100.17	100.17	100.17	100.17	100.17
10.750	100.18	100.18	100.18	100.18	100.18
11.000	100.18	100.19	100.19	100.20	100.20
11.250	100.21	100.21	100.22	100.23	100.23
11.500	100.24	100.25	100.27	100.31	100.35
11.750	100.41	100.48	100.55	100.64	100.80
12.000	101.10	101.44	101.76	101.89	101.66
12.250	101.37	101.12	100.91	100.73	100.57
12.500	100.43	100.32	100.27	100.25	100.24
12.750	100.23	100.23	100.22	100.21	100.21
13.000	100.20	100.20	100.19	100.19	100.19
13.250	100.18	100.18	100.18	100.18	100.18
13.500	100.17	100.17	100.17	100.17	100.17
13.750	100.16	100.16	100.16	100.16	100.16
14.000	100.15	100.15	100.15	100.15	100.15
14.250	100.15	100.15	100.15	100.14	100.14
14.500	100.14	100.14	100.14	100.14	100.14
14.750	100.14	100.13	100.13	100.13	100.13
15.000	100.13	100.13	100.13	100.13	100.13
15.250	100.12	100.12	100.12	100.12	100.12
15.500	100.12	100.12	100.12	100.11	100.11
15.750	100.11	100.11	100.11	100.11	100.11
16.000	100.11	100.11	100.10	100.10	100.10
16.250	100.10	100.10	100.10	100.10	100.10
16.500	100.10	100.10	100.10	100.10	100.10
16.750	100.10	100.10	100.09	100.09	100.09
17.000	100.09	100.09	100.09	100.09	100.09
17.250	100.09	100.09	100.09	100.09	100.09
17.500	100.09	100.08	100.08	100.08	100.08
17.750	100.08	100.08	100.08	100.08	100.08
18.000	100.08	100.08	100.08	100.08	100.08
18.250	100.08	100.08	100.08	100.08	100.08
18.500	100.08	100.07	100.07	100.07	100.07
18.750	100.07	100.07	100.07	100.07	100.07
19.000	100.07	100.07	100.07	100.07	100.07
19.250	100.07	100.07	100.07	100.07	100.07
19.500	100.07	100.07	100.07	100.07	100.07
19.750	100.07	100.07	100.07	100.07	100.07
20.000	100.07	100.07	100.07	100.07	100.07

Watershed

Subsection: Time vs. Elevation

Label: Pipe (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
20.250	100.07	100.07	100.07	100.07	100.07
20.500	100.07	100.07	100.07	100.07	100.07
20.750	100.07	100.07	100.07	100.07	100.07
21.000	100.07	100.07	100.07	100.07	100.06
21.250	100.06	100.06	100.06	100.06	100.06
21.500	100.06	100.06	100.06	100.06	100.06
21.750	100.06	100.06	100.06	100.06	100.06
22.000	100.06	100.06	100.06	100.06	100.06
22.250	100.06	100.06	100.06	100.06	100.06
22.500	100.06	100.06	100.06	100.06	100.06
22.750	100.06	100.06	100.06	100.06	100.06
23.000	100.06	100.06	100.06	100.06	100.06
23.250	100.06	100.06	100.06	100.06	100.06
23.500	100.06	100.06	100.06	100.06	100.06
23.750	100.06	100.06	100.06	100.06	100.06
24.000	100.06	100.05	100.04	100.02	100.01
24.250	100.01	100.00	100.00	100.00	100.00
24.500	100.00	100.00	100.00	100.00	100.00
24.750	100.00	100.00	100.00	100.00	100.00
25.000	100.00	100.00	100.00	100.00	100.00
25.250	100.00	100.00	100.00	100.00	100.00
25.500	100.00	100.00	100.00	100.00	100.00
25.750	100.00	100.00	100.00	100.00	100.00
26.000	100.00	100.00	100.00	100.00	100.00
26.250	100.00	100.00	100.00	100.00	100.00
26.500	100.00	100.00	100.00	100.00	100.00
26.750	100.00	100.00	100.00	100.00	100.00
27.000	100.00	100.00	100.00	100.00	100.00
27.250	100.00	100.00	100.00	100.00	100.00
27.500	100.00	100.00	100.00	100.00	100.00
27.750	100.00	100.00	100.00	100.00	100.00
28.000	100.00	100.00	100.00	100.00	100.00
28.250	100.00	100.00	100.00	100.00	100.00
28.500	100.00	100.00	100.00	100.00	100.00
28.750	100.00	100.00	100.00	100.00	100.00
29.000	100.00	100.00	100.00	100.00	100.00
29.250	100.00	100.00	100.00	100.00	100.00
29.500	100.00	100.00	100.00	100.00	100.00
29.750	100.00	100.00	100.00	100.00	100.00
30.000	100.00	100.00	100.00	100.00	100.00
30.250	100.00	100.00	100.00	100.00	100.00

Watershed

Subsection: Time vs. Elevation

Label: Pipe (OUT)

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Elevation (ft)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
30.500	100.00	100.00	100.00	100.00	100.00
30.750	100.00	100.00	100.00	100.00	100.00
31.000	100.00	100.00	100.00	100.00	100.00
31.250	100.00	100.00	100.00	100.00	100.00
31.500	100.00	100.00	100.00	100.00	100.00
31.750	100.00	100.00	100.00	100.00	100.00
32.000	100.00	100.00	100.00	100.00	100.00
32.250	100.00	100.00	100.00	100.00	100.00
32.500	100.00	100.00	100.00	100.00	100.00
32.750	100.00	100.00	100.00	100.00	100.00
33.000	100.00	100.00	100.00	100.00	100.00
33.250	100.00	100.00	100.00	100.00	100.00
33.500	100.00	100.00	100.00	100.00	100.00
33.750	100.00	100.00	100.00	100.00	100.00
34.000	100.00	100.00	100.00	100.00	100.00
34.250	100.00	100.00	100.00	100.00	100.00
34.500	100.00	100.00	100.00	100.00	100.00
34.750	100.00	100.00	100.00	100.00	100.00
35.000	100.00	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Time vs. Volume

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Volume (ft³)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
0.250	0.000	0.000	0.000	0.000	0.000
0.500	0.000	0.000	0.000	0.000	0.000
0.750	0.000	0.000	0.000	0.000	0.000
1.000	0.000	0.000	0.000	0.000	0.000
1.250	0.000	1.000	1.000	1.000	1.000
1.500	1.000	1.000	1.000	1.000	1.000
1.750	1.000	1.000	1.000	1.000	1.000
2.000	1.000	1.000	1.000	1.000	1.000
2.250	1.000	1.000	1.000	1.000	1.000
2.500	1.000	1.000	1.000	1.000	1.000
2.750	1.000	1.000	1.000	1.000	1.000
3.000	1.000	1.000	1.000	1.000	1.000
3.250	1.000	1.000	1.000	2.000	2.000
3.500	2.000	2.000	2.000	2.000	2.000
3.750	2.000	2.000	2.000	2.000	2.000
4.000	2.000	2.000	2.000	2.000	2.000
4.250	2.000	2.000	2.000	2.000	2.000
4.500	2.000	2.000	2.000	2.000	2.000
4.750	2.000	2.000	2.000	2.000	2.000
5.000	2.000	2.000	2.000	2.000	2.000
5.250	2.000	2.000	2.000	2.000	2.000
5.500	2.000	2.000	2.000	2.000	2.000
5.750	2.000	2.000	2.000	2.000	2.000
6.000	2.000	2.000	2.000	2.000	2.000
6.250	2.000	2.000	2.000	2.000	2.000
6.500	2.000	2.000	2.000	2.000	2.000
6.750	2.000	2.000	2.000	2.000	2.000
7.000	2.000	2.000	2.000	2.000	2.000
7.250	2.000	2.000	2.000	2.000	2.000
7.500	3.000	3.000	3.000	3.000	3.000
7.750	3.000	3.000	3.000	3.000	3.000
8.000	3.000	3.000	3.000	3.000	3.000
8.250	3.000	3.000	3.000	3.000	3.000
8.500	3.000	3.000	3.000	3.000	3.000
8.750	3.000	4.000	4.000	4.000	4.000
9.000	4.000	4.000	4.000	4.000	4.000
9.250	4.000	4.000	4.000	4.000	4.000
9.500	4.000	4.000	4.000	5.000	5.000
9.750	5.000	5.000	5.000	5.000	5.000

Watershed

Subsection: Time vs. Volume

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Volume (ft³)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
10.000	5.000	5.000	5.000	5.000	5.000
10.250	5.000	5.000	6.000	6.000	6.000
10.500	6.000	6.000	6.000	6.000	6.000
10.750	6.000	6.000	7.000	7.000	7.000
11.000	7.000	7.000	7.000	7.000	8.000
11.250	8.000	8.000	9.000	9.000	9.000
11.500	10.000	11.000	12.000	14.000	17.000
11.750	21.000	26.000	33.000	40.000	54.000
12.000	81.000	112.000	135.000	142.000	128.000
12.250	106.000	83.000	64.000	47.000	34.000
12.500	23.000	15.000	12.000	10.000	10.000
12.750	9.000	9.000	9.000	8.000	8.000
13.000	8.000	7.000	7.000	7.000	7.000
13.250	7.000	7.000	7.000	6.000	6.000
13.500	6.000	6.000	6.000	6.000	6.000
13.750	6.000	6.000	6.000	6.000	5.000
14.000	5.000	5.000	5.000	5.000	5.000
14.250	5.000	5.000	5.000	5.000	5.000
14.500	5.000	5.000	5.000	5.000	4.000
14.750	4.000	4.000	4.000	4.000	4.000
15.000	4.000	4.000	4.000	4.000	4.000
15.250	4.000	4.000	4.000	4.000	4.000
15.500	4.000	4.000	3.000	3.000	3.000
15.750	3.000	3.000	3.000	3.000	3.000
16.000	3.000	3.000	3.000	3.000	3.000
16.250	3.000	3.000	3.000	3.000	3.000
16.500	3.000	3.000	3.000	3.000	3.000
16.750	3.000	3.000	3.000	3.000	3.000
17.000	2.000	2.000	2.000	2.000	2.000
17.250	2.000	2.000	2.000	2.000	2.000
17.500	2.000	2.000	2.000	2.000	2.000
17.750	2.000	2.000	2.000	2.000	2.000
18.000	2.000	2.000	2.000	2.000	2.000
18.250	2.000	2.000	2.000	2.000	2.000
18.500	2.000	2.000	2.000	2.000	2.000
18.750	2.000	2.000	2.000	2.000	2.000
19.000	2.000	2.000	2.000	2.000	2.000
19.250	2.000	2.000	2.000	2.000	2.000
19.500	2.000	2.000	2.000	2.000	2.000
19.750	2.000	2.000	2.000	2.000	2.000
20.000	2.000	2.000	2.000	2.000	2.000

Watershed

Subsection: Time vs. Volume

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Volume (ft³)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
20.250	2.000	2.000	2.000	2.000	2.000
20.500	2.000	2.000	2.000	2.000	2.000
20.750	2.000	2.000	2.000	2.000	2.000
21.000	2.000	2.000	2.000	2.000	2.000
21.250	2.000	2.000	2.000	2.000	2.000
21.500	2.000	2.000	2.000	2.000	2.000
21.750	2.000	2.000	2.000	2.000	2.000
22.000	2.000	2.000	2.000	2.000	2.000
22.250	2.000	2.000	2.000	2.000	2.000
22.500	2.000	2.000	2.000	2.000	2.000
22.750	2.000	2.000	2.000	2.000	2.000
23.000	2.000	2.000	2.000	2.000	2.000
23.250	2.000	2.000	2.000	2.000	2.000
23.500	2.000	2.000	2.000	2.000	2.000
23.750	2.000	2.000	2.000	2.000	2.000
24.000	2.000	1.000	1.000	1.000	0.000
24.250	0.000	0.000	0.000	0.000	0.000
24.500	0.000	0.000	0.000	0.000	0.000
24.750	0.000	0.000	0.000	0.000	0.000
25.000	0.000	0.000	0.000	0.000	0.000
25.250	0.000	0.000	0.000	0.000	0.000
25.500	0.000	0.000	0.000	0.000	0.000
25.750	0.000	0.000	0.000	0.000	0.000
26.000	0.000	0.000	0.000	0.000	0.000
26.250	0.000	0.000	0.000	0.000	0.000
26.500	0.000	0.000	0.000	0.000	0.000
26.750	0.000	0.000	0.000	0.000	0.000
27.000	0.000	0.000	0.000	0.000	0.000
27.250	0.000	0.000	0.000	0.000	0.000
27.500	0.000	0.000	0.000	0.000	0.000
27.750	0.000	0.000	0.000	0.000	0.000
28.000	0.000	0.000	0.000	0.000	0.000
28.250	0.000	0.000	0.000	0.000	0.000
28.500	0.000	0.000	0.000	0.000	0.000
28.750	0.000	0.000	0.000	0.000	0.000
29.000	0.000	0.000	0.000	0.000	0.000
29.250	0.000	0.000	0.000	0.000	0.000
29.500	0.000	0.000	0.000	0.000	0.000
29.750	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
30.250	0.000	0.000	0.000	0.000	0.000

Watershed

Subsection: Time vs. Volume

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Time vs. Volume (ft³)

Output Time increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
30.500	0.000	0.000	0.000	0.000	0.000
30.750	0.000	0.000	0.000	0.000	0.000
31.000	0.000	0.000	0.000	0.000	0.000
31.250	0.000	0.000	0.000	0.000	0.000
31.500	0.000	0.000	0.000	0.000	0.000
31.750	0.000	0.000	0.000	0.000	0.000
32.000	0.000	0.000	0.000	0.000	0.000
32.250	0.000	0.000	0.000	0.000	0.000
32.500	0.000	0.000	0.000	0.000	0.000
32.750	0.000	0.000	0.000	0.000	0.000
33.000	0.000	0.000	0.000	0.000	0.000
33.250	0.000	0.000	0.000	0.000	0.000
33.500	0.000	0.000	0.000	0.000	0.000
33.750	0.000	0.000	0.000	0.000	0.000
34.000	0.000	0.000	0.000	0.000	0.000
34.250	0.000	0.000	0.000	0.000	0.000
34.500	0.000	0.000	0.000	0.000	0.000
34.750	0.000	0.000	0.000	0.000	0.000
35.000	0.000	(N/A)	(N/A)	(N/A)	(N/A)

Watershed

Subsection: Pipe Volume

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Volume Results (Pipe)

Pipe Storage Upstream Invert	100.00 ft
Pipe Storage Downstream Invert	100.00 ft
Pipe Storage Length	46.00 ft
Pipe Storage Diameter	24.0 in
Pipe Storage Number of Barrels	1
Pipe Storage Slice Width	1.00 ft
Pipe Storage Vertical Increment	0.10 ft

Elevation (ft)	Perpendicular Downstream Depth (ft)	Perpendicular Downstream Area (ft ²)	Wetted Length (ft)	Filled Length (ft)	Perpendicular Upstream Depth (ft)	Perpendicular Upstream Area (ft ²)	Total Volume (ft ³)
100.00	0.00	0.0	46.00	0.00	0.00	0.0	0.000
100.10	0.10	0.1	46.00	0.00	0.10	0.1	3.000
100.20	0.20	0.2	46.00	0.00	0.20	0.2	8.000
100.30	0.30	0.3	46.00	0.00	0.30	0.3	14.000
100.40	0.40	0.4	46.00	0.00	0.40	0.4	21.000
100.50	0.50	0.6	46.00	0.00	0.50	0.6	28.000
100.60	0.60	0.8	46.00	0.00	0.60	0.8	36.000
100.70	0.70	1.0	46.00	0.00	0.70	1.0	45.000
100.80	0.80	1.2	46.00	0.00	0.80	1.2	54.000
100.90	0.90	1.4	46.00	0.00	0.90	1.4	63.000
101.00	1.00	1.6	46.00	0.00	1.00	1.6	72.000
101.10	1.10	1.8	46.00	0.00	1.10	1.8	81.000
101.20	1.20	2.0	46.00	0.00	1.20	2.0	91.000
101.30	1.30	2.2	46.00	0.00	1.30	2.2	99.000
101.40	1.40	2.3	46.00	0.00	1.40	2.3	108.000
101.50	1.50	2.5	46.00	0.00	1.50	2.5	116.000
101.60	1.60	2.7	46.00	0.00	1.60	2.7	124.000
101.70	1.70	2.8	46.00	0.00	1.70	2.8	131.000
101.80	1.80	3.0	46.00	0.00	1.80	3.0	137.000
101.90	1.90	3.1	46.00	0.00	1.90	3.1	142.000
102.00	2.00	3.1	46.00	46.00	2.00	3.1	145.000

Watershed

Subsection: Diverted Hydrograph

Label: 4" Orifice

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Peak Discharge	0.51 ft ³ /s
Time to Peak	12.150 hours
Hydrograph Volume	2,455.833 ft ³

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1.050	0.00	0.00	0.00	0.00	0.00
1.300	0.00	0.00	0.00	0.00	0.00
1.550	0.00	0.00	0.00	0.00	0.00
1.800	0.00	0.00	0.00	0.00	0.00
2.050	0.00	0.00	0.00	0.00	0.00
2.300	0.00	0.00	0.00	0.00	0.00
2.550	0.00	0.00	0.00	0.00	0.00
2.800	0.00	0.01	0.01	0.01	0.01
3.050	0.01	0.01	0.01	0.01	0.01
3.300	0.01	0.01	0.01	0.01	0.01
3.550	0.01	0.01	0.01	0.01	0.01
3.800	0.01	0.01	0.01	0.01	0.01
4.050	0.01	0.01	0.01	0.01	0.01
4.300	0.01	0.01	0.01	0.01	0.01
4.550	0.01	0.01	0.01	0.01	0.01
4.800	0.01	0.01	0.01	0.01	0.01
5.050	0.01	0.01	0.01	0.01	0.01
5.300	0.01	0.01	0.01	0.01	0.01
5.550	0.01	0.01	0.01	0.01	0.01
5.800	0.01	0.01	0.01	0.01	0.01
6.050	0.01	0.01	0.01	0.01	0.01
6.300	0.01	0.01	0.01	0.01	0.01
6.550	0.01	0.01	0.01	0.01	0.01
6.800	0.01	0.01	0.01	0.01	0.01
7.050	0.01	0.01	0.01	0.01	0.01
7.300	0.01	0.01	0.01	0.02	0.02
7.550	0.02	0.02	0.02	0.02	0.02
7.800	0.02	0.02	0.02	0.02	0.02
8.050	0.02	0.02	0.02	0.02	0.02
8.300	0.02	0.02	0.02	0.02	0.02
8.550	0.02	0.02	0.02	0.02	0.02
8.800	0.02	0.02	0.02	0.02	0.02
9.050	0.03	0.03	0.03	0.03	0.03
9.300	0.03	0.03	0.03	0.03	0.03
9.550	0.03	0.03	0.03	0.03	0.03
9.800	0.03	0.03	0.03	0.03	0.03
10.050	0.03	0.03	0.03	0.04	0.04

Watershed

Subsection: Diverted Hydrograph

Label: 4" Orifice

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
10.300	0.04	0.04	0.04	0.04	0.04
10.550	0.04	0.04	0.04	0.04	0.05
10.800	0.05	0.05	0.05	0.05	0.05
11.050	0.05	0.05	0.05	0.06	0.06
11.300	0.06	0.07	0.07	0.07	0.08
11.550	0.09	0.10	0.12	0.15	0.17
11.800	0.20	0.22	0.25	0.29	0.36
12.050	0.43	0.49	0.51	0.47	0.42
12.300	0.37	0.32	0.27	0.23	0.18
12.550	0.13	0.10	0.08	0.08	0.07
12.800	0.07	0.07	0.06	0.06	0.06
13.050	0.06	0.05	0.05	0.05	0.05
13.300	0.05	0.05	0.05	0.05	0.04
13.550	0.04	0.04	0.04	0.04	0.04
13.800	0.04	0.04	0.04	0.04	0.04
14.050	0.04	0.03	0.03	0.03	0.03
14.300	0.03	0.03	0.03	0.03	0.03
14.550	0.03	0.03	0.03	0.03	0.03
14.800	0.03	0.03	0.03	0.03	0.03
15.050	0.03	0.03	0.03	0.03	0.03
15.300	0.02	0.02	0.02	0.02	0.02
15.550	0.02	0.02	0.02	0.02	0.02
15.800	0.02	0.02	0.02	0.02	0.02
16.050	0.02	0.02	0.02	0.02	0.02
16.300	0.02	0.02	0.02	0.02	0.02
16.550	0.02	0.02	0.02	0.02	0.02
16.800	0.02	0.02	0.02	0.02	0.02
17.050	0.02	0.01	0.01	0.01	0.01
17.300	0.01	0.01	0.01	0.01	0.01
17.550	0.01	0.01	0.01	0.01	0.01
17.800	0.01	0.01	0.01	0.01	0.01
18.050	0.01	0.01	0.01	0.01	0.01
18.300	0.01	0.01	0.01	0.01	0.01
18.550	0.01	0.01	0.01	0.01	0.01
18.800	0.01	0.01	0.01	0.01	0.01
19.050	0.01	0.01	0.01	0.01	0.01
19.300	0.01	0.01	0.01	0.01	0.01
19.550	0.01	0.01	0.01	0.01	0.01
19.800	0.01	0.01	0.01	0.01	0.01
20.050	0.01	0.01	0.01	0.01	0.01
20.300	0.01	0.01	0.01	0.01	0.01
20.550	0.01	0.01	0.01	0.01	0.01

Watershed

Subsection: Diverted Hydrograph

Label: 4" Orifice

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 0.050 hours

Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
20.800	0.01	0.01	0.01	0.01	0.01
21.050	0.01	0.01	0.01	0.01	0.01
21.300	0.01	0.01	0.01	0.01	0.01
21.550	0.01	0.01	0.01	0.01	0.01
21.800	0.01	0.01	0.01	0.01	0.01
22.050	0.01	0.01	0.01	0.01	0.01
22.300	0.01	0.01	0.01	0.01	0.01
22.550	0.01	0.01	0.01	0.01	0.01
22.800	0.01	0.01	0.01	0.01	0.01
23.050	0.01	0.01	0.01	0.01	0.01
23.300	0.01	0.01	0.01	0.01	0.01
23.550	0.01	0.01	0.01	0.01	0.01
23.800	0.01	0.01	0.01	0.01	0.01
24.050	0.01	0.00	0.00	0.00	0.00

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	100.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
100.00	0.00	0.000	0.000	0.00	0.00	0.00
100.05	0.00	1.351	0.000	0.00	0.00	0.02
100.10	0.02	2.701	0.000	0.00	0.02	0.05
100.15	0.03	5.111	0.000	0.00	0.03	0.09
100.20	0.06	7.521	0.000	0.00	0.06	0.14
100.25	0.09	10.557	0.000	0.00	0.09	0.20
100.30	0.12	13.593	0.000	0.00	0.12	0.27
100.35	0.15	17.084	0.000	0.00	0.15	0.34
100.40	0.17	20.576	0.000	0.00	0.17	0.40
100.45	0.19	24.414	0.000	0.00	0.19	0.46
100.50	0.20	28.253	0.000	0.00	0.20	0.52
100.55	0.22	32.358	0.000	0.00	0.22	0.58
100.60	0.24	36.463	0.000	0.00	0.24	0.64
100.65	0.25	40.770	0.000	0.00	0.25	0.70
100.70	0.26	45.076	0.000	0.00	0.26	0.77
100.75	0.28	49.528	0.000	0.00	0.28	0.83
100.80	0.29	53.980	0.000	0.00	0.29	0.89
100.85	0.30	58.526	0.000	0.00	0.30	0.95
100.90	0.32	63.072	0.000	0.00	0.32	1.02
100.95	0.33	67.664	0.000	0.00	0.33	1.08
101.00	0.34	72.257	0.000	0.00	0.34	1.14
101.05	0.35	76.849	0.000	0.00	0.35	1.21
101.10	0.36	81.441	0.000	0.00	0.36	1.27
101.15	0.37	85.987	0.000	0.00	0.37	1.33
101.20	0.38	90.533	0.000	0.00	0.38	1.39
101.25	0.39	94.985	0.000	0.00	0.39	1.45
101.30	0.40	99.437	0.000	0.00	0.40	1.51
101.35	0.41	103.744	0.000	0.00	0.41	1.57
101.40	0.42	108.050	0.000	0.00	0.42	1.62

Watershed

Subsection: Elevation-Volume-Flow Table (Pond)

Label: Pipe

Scenario: WESTCHESTER-NEW - Synthetic Curve, 25 yrs

Return Event: 25 years

Storm Event: TypeIII 24hr (6.4 in)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (ft ²)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
101.45	0.43	112.156	0.000	0.00	0.43	1.68
101.50	0.44	116.261	0.000	0.00	0.44	1.73
101.55	0.45	120.099	0.000	0.00	0.45	1.79
101.60	0.46	123.938	0.000	0.00	0.46	1.84
101.65	0.47	127.429	0.000	0.00	0.47	1.88
101.70	0.48	130.920	0.000	0.00	0.48	1.93
101.75	0.49	133.956	0.000	0.00	0.49	1.97
101.80	0.49	136.992	0.000	0.00	0.49	2.02
101.85	0.50	139.402	0.000	0.00	0.50	2.05
101.90	0.51	141.812	0.000	0.00	0.51	2.09
101.95	0.52	143.163	0.000	0.00	0.52	2.11
102.00	0.53	144.513	0.000	0.00	0.53	2.13

Watershed

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APPENDIX C

NYSDEC STORMWATER SIZING CALCULATIONS

**WATER QUALITY AND RUNOFF REDUCTION VOLUME SIZING
CALCULATIONS**

Elmwood Preserve
850 Dobbs Ferry Road
Town of Greenburgh, New York

JMC Project: **17099**

Drawing Reference: **DA-1A, 1B, 1C, 1D & DA-2A, 2B, 2C, 2D**

Computed by: **RB**

Checked by: **RB**

Date Printed: 8/4/2023

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **1**

Elmwood Preserve Drainage Area: **PDA 1B**

Initial Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	9.09	3.14	34.56	0.361051243	17,878
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)

DESCRIPTION	Total Area	Impervious Area
SYMBOL	A	I
Conservation of Natural Areas		
Sheetflow to Riparian Buffers or Filter Strips		
Vegetated Swale		
Tree Planting / Tree Pit		
Disconnection of Rooftop Runoff		
Stream Daylighting		
TOTAL		
UNITS	Ac	Ac

Adjusted Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	9.09	3.14	34.56	0.361051243	17,878
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

**WATER QUALITY VOLUME WORKSHEET
FOR REDEVELOPMENT PROJECTS**

JMC Project: **17099**
Design Point: **1**

Elmwood Preserve	Drainage Area:	PDA 1C
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Initial Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Existing Impervious Area	New Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I _E	I _N	%I	R _V	WQ _V
VALUE	1.5	4.61	0.49	.987	32.03	0.338273616	8,482
UNITS	In	Ac	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)						

Runoff Reduction Techniques (Area)

DESCRIPTION	Total Area	Impervious Area
SYMBOL	A	I
Conservation of Natural Areas		
Sheetflow to Riparian Buffers or Filter Strips		
Vegetated Swale		
Tree Planting / Tree Pit		
Disconnection of Rooftop Runoff		
Stream Daylighting		
TOTAL		
UNITS	Ac	Ac

Adjusted Water Quality Treatment Volume from Runoff Reduction Techniques

DESCRIPTION	Design Storm	Area	Adjusted Existing Impervious Area	New Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I _{EA}	I _N	%I	R _V	WQ _V
VALUE	1.5	4.61	0.49	0.99	32.03	0.338273616	8,482
UNITS	In	Ac	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)						

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **2**

Elmwood Preserve	Drainage Area:	PDA 2A
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Initial Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	8.95	2.57	28.67	0.307990612	15,006
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)			
DESCRIPTION	Total Area	Impervious Area	
SYMBOL	A	I	
Conservation of Natural Areas			
Sheetflow to Riparian Buffers or Filter Strips			
Vegetated Swale			
Tree Planting / Tree Pit			
Disconnection of Rooftop Runoff			
Stream Daylighting			
TOTAL			
UNITS	Ac	Ac	

Adjusted Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	8.95	2.57	28.67	0.307990612	15,006
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **2**

Elmwood Preserve	Drainage Area:	PDA 2B
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Initial Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.17	3.31	25.13	0.276213076	19,806
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)			
DESCRIPTION	Total Area	Impervious Area	
SYMBOL	A	I	
Conservation of Natural Areas			
Sheetflow to Riparian Buffers or Filter Strips			
Vegetated Swale			
Tree Planting / Tree Pit			
Disconnection of Rooftop Runoff			
Stream Daylighting			
TOTAL			
UNITS	Ac	Ac	

Adjusted Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.17	3.31	25.13	0.276213076	19,806
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **3**

Elmwood Preserve	Drainage Area:	PDA 3A
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Initial Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.45	2.67	19.87	0.228862454	16,761
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)

DESCRIPTION	Total Area	Impervious Area
SYMBOL	A	I
Conservation of Natural Areas		
Sheetflow to Riparian Buffers or Filter Strips		
Vegetated Swale		
Tree Planting / Tree Pit		
Disconnection of Rooftop Runoff		
Stream Daylighting		
TOTAL		
UNITS	Ac	Ac

Adjusted Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.45	2.67	19.87	0.228862454	16,761
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **3**

Elmwood Preserve	Drainage Area:	PDA 3B
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Initial Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	11.12	3.67	33.00	0.347006472	21,018
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)

DESCRIPTION	Total Area	Impervious Area
SYMBOL	A	I
Conservation of Natural Areas		
Sheetflow to Riparian Buffers or Filter Strips		
Vegetated Swale		
Tree Planting / Tree Pit		
Disconnection of Rooftop Runoff		
Stream Daylighting		
TOTAL		
UNITS	Ac	Ac

Adjusted Water Quality Treatment Volume

DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	11.12	3.67	33.00	0.347006472	21,018
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

WATER QUALITY VOLUME WORKSHEET

JMC Project: **17099**
Design Point: **4**

Elmwood Preserve	Drainage Area:	PDA 4
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Initial Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.60	2.88	21.20	0.240772737	17,831
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

Runoff Reduction Techniques (Area)			
DESCRIPTION	Total Area	Impervious Area	
SYMBOL	A	I	
Conservation of Natural Areas			
Sheetflow to Riparian Buffers or Filter Strips			
Vegetated Swale			
Tree Planting / Tree Pit			
Disconnection of Rooftop Runoff			
Stream Daylighting			
TOTAL			
UNITS	Ac	Ac	

Adjusted Water Quality Treatment Volume						
DESCRIPTION	Design Storm	Area	Impervious Area	Percent Impervious	Runoff Coefficient	Total Required WQ Volume
SYMBOL	P	A	I	%I	R _V	WQ _V
VALUE	1.5	13.60	2.88	21.20	0.240772737	17,831
UNITS	In	Ac	Ac	%	CF	CF
VALUE	Enhanced Phosphorus Removal (WQ _V = 1-yr Storm Runoff)					

RUNOFF REDUCTION VOLUME WORKSHEET

JMC Project: **17099**

Design Point: **Site**

Elmwood Preserve	Drainage Area:	PDA 1B,2B,3A,3B,4
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Total Water Quality Treatment Volume			
DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	108,300	CF
Adjusted Water Quality Volume	WQ _V	108,300	CF

Minimum Runoff Reduction Volume			
DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	18.25	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	7.30	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	37,751	CF

Runoff Reduction Techniques (Volume)			
GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #1	RR _V	17,878	CF
Infiltration Basin #2	RR _V	19,806	CF
Infiltration Basin #3	RR _V	16,761	CF
Infiltration Basin #3A	RR _V	21,018	CF
Infiltration Basin #4	RR _V	17,831	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	93,294	CF

Runoff Reduction	
Is Total RR _V > Adjusted WQ _V ?	NO
Is Total RR _V > Minimum RR _V ?	YES

Note: Drainage Areas 1,1A,2,2A,2C,3 were excluded from the RR_V calculations because the areas were considered a redevelopment with a reduction in impervious area from existing to proposed conditions or no new impervious surfaces are being proposed in each area or existing soil and groundwater conditions made infiltration not feasible so other areas were targeted to meet RR_V requirements.

RUNOFF REDUCTION VOLUME WORKSHEET

JMC Project: **17099**

Design Point: **1**

Elmwood Preserve

Drainage Area: **PDA 1B**

Total Water Quality Treatment Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	17,878	CF
Adjusted Water Quality Volume	WQ _V	17,878	CF

Minimum Runoff Reduction Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	3.14	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	1.26	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	6,503	CF

Runoff Reduction Techniques (Volume)

GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #1	RR _V	17,878	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	17,878	CF

Runoff Reduction

Is Total RR _V > Adjusted WQ _V ?	NO
Is Total RR _V > Minimum RR _V ?	YES

RUNOFF REDUCTION VOLUME WORKSHEET

JMC Project: **17099**

Design Point: **2**

Elmwood Preserve

Drainage Area: **PDA 2B**

Total Water Quality Treatment Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	19,806	CF
Adjusted Water Quality Volume	WQ _V	19,806	CF

Minimum Runoff Reduction Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	3.31	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	1.32	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	6,849	CF

Runoff Reduction Techniques (Volume)

GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #2	RR _V	19,806	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	19,806	CF

Runoff Reduction

Is Total RR _V > Adjusted WQ _V ?	YES
Is Total RR _V > Minimum RR _V ?	YES

RUNOFF REDUCTION VOLUME WORKSHEET

JMC Project: **17099**

Design Point: **3**

Elmwood Preserve

Drainage Area: **PDA 3A**

Total Water Quality Treatment Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	16,761	CF
Adjusted Water Quality Volume	WQ _V	16,761	CF

Minimum Runoff Reduction Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	2.67	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	1.07	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	5,531	CF

Runoff Reduction Techniques (Volume)

GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #3	RR _V	16,761	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	16,761	CF

Runoff Reduction

Is Total RR _V > Adjusted WQ _V ?	YES
Is Total RR _V > Minimum RR _V ?	YES

RUNOFF REDUCTION VOLUME WORKSHEETJMC Project: **17099**Design Point: **3****Elmwood Preserve**Drainage Area: **PDA 3B****Total Water Quality Treatment Volume**

DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	21,018	CF
Adjusted Water Quality Volume	WQ _V	21,018	CF

Minimum Runoff Reduction Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	3.67	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	1.47	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	7,596	CF

Runoff Reduction Techniques (Volume)

GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #3A	RR _V	21,018	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	21,018	CF

Runoff Reduction

Is Total RR _V > Adjusted WQ _V ?	NO
Is Total RR _V > Minimum RR _V ?	YES

RUNOFF REDUCTION VOLUME WORKSHEET

JMC Project: **17099**

Design Point: **4**

Elmwood Preserve

Drainage Area: **PDA 4**

Total Water Quality Treatment Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Initial Water Quality Volume	WQ _V	17,831	CF
Adjusted Water Quality Volume	WQ _V	17,831	CF

Minimum Runoff Reduction Volume

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Total Area of <i>new</i> Impervious Cover	A _{ic}	2.88	Ac
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40	
Runoff Coefficient [0.05 + 0.009 x %I]	R _V	0.95	CF
Impervious Cover targeted for Runoff Reduction [S x A _{ic}]	A _i	1.15	Ac
TOTAL VOLUME Required [RR_V = (P x R_V x A_i) / 12]	RR _V	5,965	CF

Runoff Reduction Techniques (Volume)

GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS
Infiltration Basin #4	RR _V	17,831	CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
	RR _V		CF
TOTAL	RR _V	17,831	CF

Runoff Reduction

Is Total RR _V > Adjusted WQ _V ?	NO
Is Total RR _V > Minimum RR _V ?	YES

INFILTRATION WORKSHEET

JMC Project: **17099**Design Point: **1**PDA: **1B**

Infiltration Basin #1

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	3.14	Ac
Area	A	9.09	Ac
Percent Impervious	%I	34.56	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R_V	0.36	CF
TOTAL VOLUME Required $[WQ_V = (P \times R_V \times A) / 12]$	WQ_V	17,878	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_V = 1\text{-yr Storm Runoff}]$	WQ_V	17,878	CF

Minimum Infiltration Basin Area

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_V	17,878	CF
Depth of the Basin	d_b	6.00	Ft
Required Bottom Area of Infiltration Basin $[A_p = WQ_V / d_b]$	A_p	2,979.69	SF

Proposed Infiltration Trench

DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		9,840.00	SF
Total Area of Infiltration Basin Provided	A_p	9,840.00	SF

Runoff Reduction

DESCRIPTION	SYMBOL	VALUE	UNITS
100% Runoff Reduction capacity	RR_V	17,878	CF

POND WORKSHEET

JMC Project:	17099
Design Point:	2
PDA:	2A

Wet Extended Detention Pond #2A

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.57	Ac
Area	A	8.95	Ac
Percent Impervious	%I	28.67	%
Runoff Coefficient [$0.05 + 0.009 \times \%I$]	R_v	0.31	CF
TOTAL VOLUME Required [$WQ_v = (P \times R_v \times A) / 12$]	WQ_v	15,006	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) [$WQ_v = 1\text{-yr Storm Runoff}$]	WQ_v	15,006	CF

*See "PRETREATMENT FOR DET. BASIN #2A" sheet for further detail

INFILTRATION WORKSHEET

JMC Project: **17099**Design Point: **2**PDA: **2B**

Infiltration Basin #2

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	3.31	Ac
Area	A	13.17	Ac
Percent Impervious	%I	25.13	%
Runoff Coefficient [$0.05 + 0.009 \times \%I$]	R_V	0.28	CF
TOTAL VOLUME Required [$WQ_V = (P \times R_V \times A) / 12$]	WQ_V	19,806	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) [$WQ_V = 1\text{-yr Storm Runoff}$]	WQ_V	19,806	CF

Minimum Infiltration Basin Area

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_V	19,806	CF
Depth of the Basin	d_b	6.00	Ft
Required Bottom Area of Infiltration Basin [$A_p = WQ_V / d_b$]	A_p	3,300.99	SF

Proposed Infiltration Trench

DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		5,796.00	SF
Total Area of Infiltration Basin Provided	A_p	5,796.00	SF

Runoff Reduction

DESCRIPTION	SYMBOL	VALUE	UNITS
100% Runoff Reduction capacity	RR_V	19,806	CF

INFILTRATION WORKSHEET

JMC Project: **17099**Design Point: **3**PDA: **3A**

Infiltration Basin #3

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.67	Ac
Area	A	13.45	Ac
Percent Impervious	%I	19.87	%
Runoff Coefficient [$0.05 + 0.009 \times \%I$]	R_V	0.23	CF
TOTAL VOLUME Required [$WQ_V = (P \times R_V \times A) / 12$]	WQ_V	16,761	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) [$WQ_V = 1\text{-yr Storm Runoff}$]	WQ_V	16,761	CF

Minimum Infiltration Basin Area

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_V	16,761	CF
Depth of the Basin	d_b	5.50	Ft
Required Bottom Area of Infiltration Basin [$A_p = WQ_V / d_b$]	A_p	3,047.42	SF

Proposed Infiltration Trench

DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		6,510.00	SF
Total Area of Infiltration Basin Provided	A_p	6,510.00	SF

Runoff Reduction

DESCRIPTION	SYMBOL	VALUE	UNITS
100% Runoff Reduction capacity	RR_V	16,761	CF

INFILTRATION WORKSHEET

JMC Project: **17099**Design Point: **3**PDA: **3B**

Infiltration Basin #3A

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	3.67	Ac
Area	A	11.12	Ac
Percent Impervious	%I	33.00	%
Runoff Coefficient [$0.05 + 0.009 \times \%I$]	R_V	0.35	CF
TOTAL VOLUME Required [$WQ_V = (P \times R_V \times A) / 12$]	WQ_V	21,018	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) [$WQ_V = 1\text{-yr Storm Runoff}$]	WQ_V	21,018	CF

Minimum Infiltration Basin Area

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_V	21,018	CF
Depth of the Basin	d_b	6.25	Ft
Required Bottom Area of Infiltration Basin [$A_p = WQ_V / d_b$]	A_p	3,362.92	SF

Proposed Infiltration Trench

DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		5,775.00	SF
Total Area of Infiltration Basin Provided	A_p	5,775.00	SF

Runoff Reduction

DESCRIPTION	SYMBOL	VALUE	UNITS
100% Runoff Reduction capacity	RR_V	21,018	CF

INFILTRATION WORKSHEET

JMC Project: **17099**Design Point: **4**PDA: **4**

Infiltration Basin #4

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.88	Ac
Area	A	13.60	Ac
Percent Impervious	%I	21.20	%
Runoff Coefficient [$0.05 + 0.009 \times \%I$]	R_V	0.24	CF
TOTAL VOLUME Required [$WQ_V = (P \times R_V \times A) / 12$]	WQ_V	17,831	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) [$WQ_V = 1\text{-yr Storm Runoff}$]	WQ_V	17,831	CF

Minimum Infiltration Basin Area

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_V	17,831	CF
Depth of the Basin	d_b	3.75	Ft
Required Bottom Area of Infiltration Basin [$A_p = WQ_V / d_b$]	A_p	4,754.94	SF

Proposed Infiltration Trench

DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		6,751.00	SF
Total Area of Infiltration Basin Provided	A_p	6,751.00	SF

Runoff Reduction

DESCRIPTION	SYMBOL	VALUE	UNITS
100% Runoff Reduction capacity	RR_V	17,831	CF

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **1**Drainage Area: **1B****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #1 - WQS-D-2**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀**A****B****C** $[R = I_a / P]$ **C₁** $[C_i = A \times R^2 + B \times R + C]$ **C₂**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	3.14	Ac
Area	A	9.09	Ac
Percent Impervious	%I	34.56	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _V	0.36	CF
TOTAL VOLUME Required $[WQ_V = (P \times R_V \times A) / 12]$	WQ _V	17,878	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_V = 1\text{-yr Storm Runoff}]$	WQ _V	17,878	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _V	17,878	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.1460	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$	Q	0.54	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	87.14	
Curve Number	CN	87	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.30	In
Ratio $[R = I_a / P]$	R	0.20	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.45	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.54	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.15	
Unit Peak Discharge	q _u	626.05	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	4.82	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	7.20	cfs
Water Quality Volume Provided $[WQ_V = 640 \times 3600 \times Q_p / q_u]$	WQ _V	26,497	CF
Model Designation		CS-8	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEET

JMC Project: **17099**

Design Point: **1**

Drainage Area: **1C**

Continuous Deflective Separation Unit

WQS-A-1

Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

$$[R = I_a / P]$$

$$[C_i = A \times R^2 + B \times R + C]$$

	A	B	C
C_0	-1.774	0.3301	2.4577
C_1	1.8622	-0.7397	-0.4627
C_2	-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	1.48	Ac
Area	A	4.61	Ac
Percent Impervious	%I	32.03	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R_v	0.34	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ_v	8,482	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ_v	8,482	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_v	8,482	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t_c	0.1020	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.51	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	86.36	
Curve Number	CN	86	
Initial Abstraction $[I_a = 200 / CN - 2]$	I_a	0.32	In
Ratio $[R = I_a / P]$	R	0.21	
$C_0 = A \times R^2 + B \times R + C$	C_0	2.45	
$C_1 = A \times R^2 + B \times R + C$	C_1	-0.54	
$C_2 = A \times R^2 + B \times R + C$	C_2	-0.15	
Unit Peak Discharge	q_u	682.68	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q_p	2.49	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q_p	2.81	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ_v	9,483	CF
Model Designation		CS-5	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **2**Drainage Area: **2B****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #2 - WQS-B-2**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀**A****B****C** $[R = I_a / P]$ **C₁** $[C_i = A \times R^2 + B \times R + C]$ **C₂**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	0.43	Ac
Area	A	1.03	Ac
Percent Impervious	%I	41.46	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _V	0.42	CF
TOTAL VOLUME Required $[WQ_V = (P \times R_V \times A) / 12]$	WQ _V	2,362	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_V = 1\text{-yr Storm Runoff}]$	WQ _V	2,362	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _V	2,362	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.0830	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$	Q	0.63	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	89.08	
Curve Number	CN	89	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.25	In
Ratio $[R = I_a / P]$	R	0.16	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.46	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.53	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.16	
Unit Peak Discharge	q _u	719.49	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	0.73	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	1.02	cfs
Water Quality Volume Provided $[WQ_V = 640 \times 3600 \times Q_p / q_u]$	WQ _V	3,266	CF
Model Designation		CS-3	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **2**Drainage Area: **2B****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #2 - WQS-C-2**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀

$[R = I_a / P]$

C₁

$[C_i = A \times R^2 + B \times R + C]$

C₂**A****B****C**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.88	Ac
Area	A	11.29	Ac
Percent Impervious	%I	25.51	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _V	0.28	CF
TOTAL VOLUME Required $[WQ_V = (P \times R_V \times A) / 12]$	WQ _V	17,187	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_V = 1\text{-yr Storm Runoff}]$	WQ _V	17,187	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _V	17,187	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.2210	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$	Q	0.42	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	84.14	
Curve Number	CN	84	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.38	In
Ratio $[R = I_a / P]$	R	0.25	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.43	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.53	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.14	
Unit Peak Discharge	q _u	520.67	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	3.85	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	4.05	cfs
Water Quality Volume Provided $[WQ_V = 640 \times 3600 \times Q_p / q_u]$	WQ _V	17,922	CF
Model Designation		CS-6	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **2**Drainage Area: **2A****Continuous Deflective Separation Unit****PRETREATMENT FOR DET. BASIN #2A - WQS-E-2**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀ $[R = I_a / P]$ **C₁** $[C_i = A \times R^2 + B \times R + C]$ **C₂****A****B****C**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.50	In
Impervious Area	I	2.57	Ac
Area	A	8.95	Ac
Percent Impervious	%I	28.67	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _v	0.31	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ _v	15,006	CF
Design Storm [1-yr Storm Depth]	P	2.81	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ _v	15,006	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _v	15,006	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.1580	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.46	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	85.26	
Curve Number	CN	85	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.35	In
Ratio $[R = I_a / P]$	R	0.23	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.44	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.53	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.14	
Unit Peak Discharge	q _u	595.73	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	3.85	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	4.05	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ _v	15,664	CF
Model Designation		CS-6	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **2**Drainage Area: **2 & 2C****Continuous Deflective Separation Unit****WQS-K-2**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

$[R = I_a / P]$

$[C_i = A \times R^2 + B \times R + C]$

C₀**C₁****C₂****A****B****C**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.03	Ac
Area	A	8.07	Ac
Percent Impervious	%I	25.12	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R_v	0.28	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ_v	6,661	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ_v	6,661	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_v	6,661	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t_c	0.0833	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.23	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	77.76	
Curve Number	CN	78	
Initial Abstraction $[I_a = 200 / CN - 2]$	I_a	0.57	In
Ratio $[R = I_a / P]$	R	0.38	
$C_0 = A \times R^2 + B \times R + C$	C_0	2.33	
$C_1 = A \times R^2 + B \times R + C$	C_1	-0.47	
$C_2 = A \times R^2 + B \times R + C$	C_2	-0.12	
Unit Peak Discharge	q_u	503.68	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q_p	1.44	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q_p	1.80	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ_v	8,234	CF
Model Designation		CS-4	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEET

JMC Project: **17099**

Design Point: **3**

Drainage Area: **3A**

Continuous Deflective Separation Unit

PRETREATMENT FOR INFIL. BASIN #3 - WQS-F-1-1

Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

$$[R = I_a / P]$$

$$[C_i = A \times R^2 + B \times R + C]$$

C₀

C₁

C₂

A

B

C

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	2.67	Ac
Area	A	13.45	Ac
Percent Impervious	%I	19.87	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _v	0.23	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ _v	16,761	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ _v	16,761	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _v	16,761	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.1100	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.34	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	81.92	
Curve Number	CN	82	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.44	In
Ratio $[R = I_a / P]$	R	0.29	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.40	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.52	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.13	
Unit Peak Discharge	q _u	599.48	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	4.32	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	7.20	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ _v	27,672	CF
Model Designation		CS-8	
Quantity		1	

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PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **3**Drainage Area: **3B****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #3A - WQS-G-3**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀

$[R = I_a / P]$

C₁

$[C_i = A \times R^2 + B \times R + C]$

C₂**A****B****C**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	3.67	Ac
Area	A	11.12	Ac
Percent Impervious	%I	33.00	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R _V	0.35	CF
TOTAL VOLUME Required $[WQ_V = (P \times R_V \times A) / 12]$	WQ _V	21,018	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_V = 1\text{-yr Storm Runoff}]$	WQ _V	21,018	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ _V	21,018	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t _c	0.1700	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$	Q	0.52	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	86.66	
Curve Number	CN	87	
Initial Abstraction $[I_a = 200 / CN - 2]$	I _a	0.31	In
Ratio $[R = I_a / P]$	R	0.21	
$C_0 = A \times R^2 + B \times R + C$	C ₀	2.45	
$C_1 = A \times R^2 + B \times R + C$	C ₁	-0.54	
$C_2 = A \times R^2 + B \times R + C$	C ₂	-0.15	
Unit Peak Discharge	q _u	595.55	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q _p	5.39	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q _p	7.20	cfs
Water Quality Volume Provided $[WQ_V = 640 \times 3600 \times Q_p / q_u]$	WQ _V	27,855	CF
Model Designation		CS-8	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **4**Drainage Area: **4****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #4 - WQS-J-1**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀ $[R = I_a / P]$ **C₁** $[C_i = A \times R^2 + B \times R + C]$ **C₂****A****B****C**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	1.70	Ac
Area	A	6.74	Ac
Percent Impervious	%I	25.25	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R_v	0.28	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ_v	10,176	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ_v	10,176	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_v	10,176	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t_c	0.1140	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.42	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	84.05	
Curve Number	CN	84	
Initial Abstraction $[I_a = 200 / CN - 2]$	I_a	0.38	In
Ratio $[R = I_a / P]$	R	0.25	
$C_0 = A \times R^2 + B \times R + C$	C_0	2.43	
$C_1 = A \times R^2 + B \times R + C$	C_1	-0.53	
$C_2 = A \times R^2 + B \times R + C$	C_2	-0.14	
Unit Peak Discharge	q_u	636.46	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q_p	2.79	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q_p	2.81	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ_v	10,172	CF
Model Designation		CS-5	
Quantity		1	

PROPRIETARY PRACTICE WORKSHEETJMC Project: **17099**Design Point: **4**Drainage Area: **4****Continuous Deflective Separation Unit****PRETREATMENT FOR INFIL. BASIN #4 - WQS-H-2-1**Rainfall Distribution Type: **III**

Coefficients for the equation unit peak

C₀**A****B****C** $[R = I_a / P]$ **C₁** $[C_i = A \times R^2 + B \times R + C]$ **C₂**

-1.774	0.3301	2.4577
1.8622	-0.7397	-0.4627
-0.0648	0.2276	-0.1932

Site Data for Drainage Area to be Treated by Practice

DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	P	1.5	In
Impervious Area	I	1.18	Ac
Area	A	4.01	Ac
Percent Impervious	%I	29.47	%
Runoff Coefficient $[0.05 + 0.009 \times \%I]$	R_v	0.32	CF
TOTAL VOLUME Required $[WQ_v = (P \times R_v \times A) / 12]$	WQ_v	6,878	CF
Design Storm [1-yr Storm Depth]	P	1.5	In
TOTAL VOLUME Required (TMDL) $[WQ_v = 1\text{-yr Storm Runoff}]$	WQ_v	6,878	CF

Water Quality Peak Flow Calculation

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ_v	6,878	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	P	1.5	In
Time of Concentration	t_c	0.0830	Hr
Runoff Volume $[Q = WQ_v / (A \times 3630)]$	Q	0.47	In
Curve Number $[CN = 1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{1/2})]$	CN	85.53	
Curve Number	CN	86	
Initial Abstraction $[I_a = 200 / CN - 2]$	I_a	0.34	In
Ratio $[R = I_a / P]$	R	0.23	
$C_0 = A \times R^2 + B \times R + C$	C_0	2.44	
$C_1 = A \times R^2 + B \times R + C$	C_1	-0.53	
$C_2 = A \times R^2 + B \times R + C$	C_2	-0.15	
Unit Peak Discharge	q_u	708.55	cfs/mi ² /in
Peak Discharge $[Q_p = q_u \times A \times Q / 640]$	Q_p	2.10	cfs

Proposed Device

DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q_p	2.81	cfs
Water Quality Volume Provided $[WQ_v = 640 \times 3600 \times Q_p / q_u]$	WQ_v	9,137	CF
Model Designation		CS-5	
Quantity		1	

Rick Bohlander, PE

From: Michael Thompson, PE
Sent: Thursday, July 13, 2023 3:51 PM
To: Rick Bohlander, PE
Subject: FW: [EXTERNAL] FW: Peak Flow Rate, Cascade Units

See below regarding pre treatment rates

Sincerely,

MICHAEL THOMPSON, PE
Design Manager

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From: Kate Glode <Kate.Glode@ContechES.com>
Sent: Monday, August 15, 2022 12:34 PM
To: Michael Thompson, PE <mthompson@jmcpllc.com>
Subject: Re: [EXTERNAL] FW: Peak Flow Rate, Cascade Units

Michael,

Sorry about not getting back to you yet - the pretreatment flow rates are basically the flow rates that the unit can handle before bypass. With a net annual pollutant removal calculation (knowing the drainage area, percent impervious and location of project), we can actually show a greater than 80% TSS removal on an annual basis.

Please let me know if you have any other questions - I'll be free to discuss tomorrow on a quick call around 11, if that works?

Thanks,
Kate Glode

Sent from my iPhone

On Aug 15, 2022, at 11:24 AM, Michael Thompson, PE <mthompson@jmcpllc.com> wrote:

Kate,

Just following up on this. How are the pretreatment rates determined on the Cascade units? Is it a lower removal efficiency or higher particle size then what the NJDEP/NYCDEC requires for a full treatment system? Thanks.

Sincerely,

MICHAEL THOMPSON, PE
Senior Designer III

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From: Michael Thompson, PE
Sent: Thursday, August 11, 2022 10:33 AM
To: Kate Glode <Kate.Glode@ContechES.com>
Subject: RE: [EXTERNAL] FW: Peak Flow Rate, Cascade Units

How are the pretreatment rates determined? Is it a lower removal efficiency or higher particle size then what the NJDEP/NYCDEC requires for a full treatment system?

Sincerely,

MICHAEL THOMPSON, PE
Senior Designer III

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From: Kate Glode <Kate.Glode@ContechES.com>
Sent: Thursday, August 11, 2022 10:22 AM
To: Michael Thompson, PE <mthompson@jmcpllc.com>
Subject: RE: [EXTERNAL] FW: Peak Flow Rate, Cascade Units

Michael,

This email should work!

The peak flow rate really is determined by the rim to invert of the structure, so the deeper the system, the higher the flow we can bypass.

Based on the rough rim to inverts, we have the chart below as a start off guide. If you are looking for a specific bypass capacity, I would just need to know the WQ flow you're trying to achieve and the peak flow you're looking to pass – I can then provide you with the minimum rim to invert that you would need to meet it!

	Typical Dia.	Max Pipe Size	Approved Flows (cfs)		Suggested Rim to Invert Elevation*	Bypass Capacity
			Pretreatment	NYS DEC/ NYS DOT		
CS-3	3	15"	1.02	1.02	4'	15-20cfs
CS-4	4	24"	2.13	1.80	4'	15-20cfs
CS-5	5	30"	3.50	2.81	4'	15-20cfs
CS-6	6	42"	7.25	4.05	5'	15-20cfs
CS-8	8	54"	15.00	7.20	5'	15-20cfs
CS-10	10	54"	27.00	11.30	6'	15-20cfs

	Typical Dia.	Max Pipe Size	Approved Flows (cfs)		Suggested Rim to Invert Elevation*	Bypass Capacity
			Pretreatment	NYS DEC/ NYS DOT		

CDS 1515-3	3	15"	0.52	0.52	4'	15-20cfs
CDS 2015-4	4	24"	0.93	0.93	4'	15-20cfs
CDS 2015-5	5	30"	0.93		4'	15-20cfs
CDS 2520-5	5	30"	1.5	1.5	4'	15-20cfs
CDS 2025-5	5	30"	1.6		4'	15-20cfs
CDS 3020-6	6	36"	2.0		5'	15-20cfs
CDS 3025-6	6	36"	2.4		5'	15-20cfs
CDS 3030-6	6	36"	3.0	2.1	5'	15-20cfs
CDS 3035-6	6	36"	3.8		5'	15-20cfs
CDS 3535-7	7	36"	4.2	2.85	5'	15-20cfs
CDS 4030-8	8	48"	4.5		5'	15-20cfs
CDS 4040-8	8	48"	6.0	3.7	5'	15-20cfs
CDS 4045-8	8	48"	7.5		5'	15-20cfs
CDS 5653-10	10	48"	14.0	5.8	6'	15-20cfs

Thanks,

Kate Glode, EIT

NY Stormwater Consultant

Contech Engineered Solutions LLC

Albany, NY 12077

Mobile: 518-410-1287

Kate.Glode@conteches.com

www.ContechES.com

From: Michael Thompson, PE <mthompson@jmcpllc.com>

Sent: Thursday, August 11, 2022 10:12 AM

To: Kate Glode <Kate.Glode@ContechES.com>

Subject: [EXTERNAL] FW: Peak Flow Rate, Cascade Units

CAUTION: This email originated from outside of the organization. Exercise caution when opening attachments or clicking links, especially from *UNKNOWN* senders.

Kate,

Trying the other email address I have for you in the case the previous one no longer works. Please see below. Thanks.

Sincerely,

MICHAEL THOMPSON, PE

Senior Designer III

JMC

SITE DEVELOPMENT CONSULTANTS

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From: Michael Thompson, PE

Sent: Friday, April 29, 2022 2:59 PM

To: Glode, Kate <KGlode@conteches.com>

Cc: Ryan S Thomas <Ryan.Thomas@ContechES.com>; Nicholas Busque <Nicholas.Busque@ContechES.com>

Subject: Peak Flow Rate, Cascade Units

Kate/Ryan,

Would you be able to tell me the allowable peak flow rate for the various size Cascade units? Thanks.

If you have questions or require additional information please contact our office at (914) 273-5225.

Sincerely,

MICHAEL THOMPSON, PE
Senior Designer III

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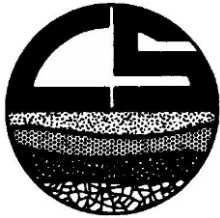
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APPENDIX D

SOIL TESTING DATA



CARLIN • SIMPSON & ASSOCIATES, LLC

Consulting Geotechnical and Environmental Engineers

61 Main Street, Sayreville, New Jersey 08872
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Michal Wroblewski, P.E.
Kurt W. Anke
Eric J. Shaw

29 August 2018
Revised 28 March 2023

Ridgewood Real Estate Partners
25A Hanover Rd., Suite 310
Florham Park, NJ 07932

Attn: Mr. Brett Owings, PE

Re: Report on Subsurface Soil and Groundwater Investigation
Proposed Residential Development – Stormwater Update
Elmwood Country Club Re-Development
Greenburgh, NY (CSA Job #18-40)

Dear Mr. Owings:

In accordance with our proposal dated 20 February 2018, supplemental proposal dated 3 November 2022 and your subsequent authorization, we have completed a Subsurface Soil and Groundwater Investigation for the referenced site. The purpose of this study was to determine the nature and engineering properties of the subsurface soil, the groundwater conditions for the new construction, and to preliminary determine the site soils permeability.

We understand that the planned construction will consist of new single family home residential development. Site development will also include new roadways, utilities, and a new stormwater management system. This report is for the stormwater management portion of this project. To guide us in our study, you have provided us with plans that indicate the location of the proposed stormwater management areas.

Our scope of work for this project included the following:

1. Reviewed the proposed layout, the existing site conditions, the expected soil conditions, and planned this study.
2. Retained General Borings Inc. to advance eight (8) test borings at selected locations at the subject site.
3. Retained Kings Capital Group to perform seventeen (17) test pits at selected locations on the subject site.

4. Retained Lawrence Construction to excavate ten (10) test pits at selected locations at the subject site.
5. Retained Environmental Technical Drilling to advance twelve (12) soil borings at selected locations on the subject site.
6. Retained Traficante Construction to excavate eight (8) test pits at selected locations at the subject site.
7. Performed three (3) percolation tests and five (5) borehole permeability tests to preliminarily determine the site soils permeability.
8. Performed laboratory soil identification tests on selected representative soil samples.
9. Analyzed the field and laboratory test data and prepared this report containing the results of this study.

1.0 SITE DESCRIPTION

The site is located at the Elmwood Country Club on Dobbs Ferry Road in Greenburgh, New York. The site is currently an existing golf course with landscape and water features. Site grades vary from elevations +380.0 to +280.0.

2.0 PROPOSED CONSTRUCTION

We understand that the planned construction will consist of new single family home residential development. The planned single-family homes will all have basements. The basement finished floor elevations range from +367.0 to +291.0. Based on the planned finished floor elevations we anticipate cuts ranging up to 18-feet and fills ranging up to 15-feet will be required to achieve the planned elevations.

The site development will also include new roadways, soil/rock slopes, new utilities, and a new stormwater management system. The stormwater management system will consist of seven (7) stormwater basins located throughout the project site.

The following evaluation is based on information that has been provided to our office as of the date of this report. Once the planned construction has been further developed, a copy of the plans should be forwarded to our office at that time we can review them along with the recommendations in this report. At that time, any changes or additional recommendations can be provided, if required.

3.0 SUBSURFACE CONDITIONS

To determine the subsurface soil and groundwater conditions at the site, eight (8) test borings were advanced by General Borings Inc. in April 2018 and twelve (12) soil borings were advanced by Environmental Technical Drilling in December 2022. The borings were performed using hollow stem augers and split spoon sampling. In July 2018 Kings Capital Group performed seventeen (17) test pits, in May 2021 Lawrence Construction excavated ten (10) test pits, and in February 2023 Traficante Construction performed eight (8) test pits throughout the site. The borings and test pits were performed under the full-time inspection of Carlin-Simpson & Associates. Detailed boring and test pit logs have been prepared and are included in this report. Our field engineer visually identified all of the soil samples obtained during the boring operations and selected samples were tested in our laboratory.

3.1 Soils

The soil descriptions shown on the boring and test pit logs are based on the Burmister Classification System. In this system, the soil is divided into three components: Sand (S), Silt (\$) and Gravel (G). The major component is indicated in all capital letters, the lesser in lower case letters. The following modifiers indicate the quantity of each lesser component:

<u>Modifier</u>	<u>Quantity</u>
trace (t)	0 -10%
little (l)	10% - 20%
some (s)	20% - 35%
and (a)	35% - 50%

When the site soils are plastic, the following indicators are used:

<u>Plasticity</u>	<u>Plasticity Index</u>	<u>Indicator</u>
None	0 - 1	SILT
Slight	1 - 5	Clayey SILT
Low	5 - 10	SILT & CLAY
Medium	10 - 20	CLAY & SILT
High	20 - 40	Silty CLAY
Very High	40+	CLAY

The subsurface soil conditions encountered in the soil borings and test pits can be summarized as follows:

<u>Stratum 1</u>	At the surface throughout the site is topsoil that ranges from
Asphalt	approximately 2 to 18 inches in thickness.

Stratum 2
Existing Fill

Beneath the topsoil in portions of the site is existing fill that generally consists of loose to medium dense brown coarse to fine SAND, little (to and) Silt, trace (to little) coarse to fine Gravel. Debris including glass, wood and organics was encountered in some of the borings and test pits. This stratum was encountered to depths ranging from 0'10" to 12'0" below the existing ground surface at the boring locations.

Stratum 3
Silty Gravelly
Sand or Sandy
Gravelly Silt

Underlying the existing fill and surface layers is brown coarse to fine SAND, little (to and) Silt, little (to some) coarse to fine Gravel or soft to stiff brown SILT and, coarse to fine Sand, little (to some) coarse to fine Gravel with cobbles and boulders. This stratum extends to depths ranging from 1'9" to 14'0" below the existing ground surface. 24 test pits and borings were terminated in this stratum at depths ranging from 6'0" to 14'0" below the existing ground surface.

Stratum 4
Completely
Weathered
Bedrock

Below the Silty Gravelly Sand or Sandy Gravelly Silt in 20 of the test pits and soil borings is highly to completely weathered Gneiss or Schist. This layer is soil like in state, however, there could be denser pockets that cannot be conventionally excavated. The completely weathered rock was encountered beginning at depths ranging from 0'6" to 14'0" below the existing ground surface.

Stratum 5
Bedrock

Auger or bucket refusal on probable Gneiss or Schist bedrock was encountered 34 of the test pits and borings locations at depths ranging from 1'3" to 16'0" below the existing ground surface.

3.2 Bedrock

Based on our experience and the boring observations, the in-situ bedrock at the site will range from completely weathered rock in a soil-like state to a more intact bedrock. The rock generally transitions into harder bedrock with increasing depth. The completely weathered rock was encountered at depths ranging from 0'6" to 14'0" below the ground surface (elevations +372.0 to +288.0). Auger refusal on harder bedrock was encountered at depths ranging from 1'3" to 16'0" below the existing ground surface (elevation +269.8 to +284.0).

Penetration into the bedrock and completely weathered rock with excavation equipment will depend on the degree of weathering and fracturing in the rock. The upper few feet of the "weathered rock" may be "rippable" by using large construction equipment, but we anticipate that the "rippability" of the bedrock and completely weathered rock will be variable and limited. It should not be assumed that the completely weathered rock (very dense material in a soil-like state) can be excavated with conventional equipment. Harder rock may be encountered, and the use of hydraulic hammers and/or localized rock blasting may be required to excavate the harder, intact bedrock.

3.3 Groundwater

During our subsurface investigations, groundwater was encountered in 20 of the soil borings and test pits at depths ranging from 3'9" to 13'0" below the existing ground surface. These depths correlate to groundwater levels ranging from approximately elevation +342.0 to +288.0. In addition, evidence of seasonal high groundwater (i.e., mottling) was observed in 9 locations at depths ranging from 1'9" to 6'6" (elevations +344.0 to +304.5) below the existing ground surface.

Groundwater on the subject site will be controlled by the topography and the underlying bedrock surface. During construction, we expect that perched or trapped water may be encountered within the silty site soils, existing fill, and/or along the soil/rock interface, especially during wet periods. Proper groundwater control measures will be required in the event that water is encountered in the site excavations (i.e., sumps and pumps). Variations in the location of the long-term water table may occur as a result of changes in precipitation, evaporation, surface water runoff, and other factors not immediately apparent at the time of this exploration.

4.0 STORMWATER MANAGEMENT SYSTEM EVALUATION

We understand that the proposed stormwater management system will consists of seven (7) basins: Basin #1, Basin #2, Basin #2A, Basin #3, Basin #3A, Basin #4, and Basin #4A. Each basin is further discussed below.

Basin #1

Basin #1 is located in the central western portion of the site. The existing grades throughout the proposed basin generally slope from southeast to northwest and range from approximate elevation +356.0 to +330.0. The proposed bottom of the basin is +342.0. Borings B-1-1, B-1-2, B-1-3, and B-1-4 were performed for the proposed basin. A summary of the boring observations can be found in Table 1 below.

Table 1 – Basin #1 Summary of Boring Observations

Boring No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
B-1-1	+347.0	3'6" (+343.5)	5'0" (+342.0)	CWR @ 6'0" (+341.0) AR @ 9'0" (+338.0)
B-1-2	+347.0	2'0" (+345.0)	6'0" (+341.0)	AR @ 9'6" (+337.5)
B-1-3	+346.5	11'6" (+335.0)	8'0" (+335.0)	CWR @ 11'6" (+335.0) AR @ 14'0" (+332.0)
B-1-4	+347.0	3'0" (+344.0)	SHGW @ 3'0" (+344.0) 8'0" (+339.0)	CWR @ 10'6" (+336.5) AR @ 11'11" (+335.1)

CWR – Completely Weathered Rock

AR– Auger Refusal on Probable Rock

SHGW – Evidence of Seasonal High Groundwater

Groundwater was encountered in each of the borings at depths ranging from 5'0" to 8'0" (elevations +342.0 to +335.0) below the existing ground surface. In addition, evidence of seasonal high groundwater table was observed in boring B-1-4 at a depth of 3'0" (elevation +344.0) below the existing ground surface. Due to the high groundwater table and seasonal high groundwater table, borehole infiltration tests were not performed at these locations.

Basin #2

Basin #2 is located in the southwest corner of the site. Existing site grades in the proposed basin area are generally flat and range in elevation from +330.0 to +325.0. The proposed bottom of basin elevation is +322.5. Test pits TP-18 and TP-19 and borings B-2-1 and B-2-2 were performed for the proposed basin. A summary of the test pit and borings observations can be found in Table 2 below.

Table 2 – Basin #2 Summary of Boring and Test Pit Observations

Boring/ Test Pit No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
TP-18	+325.5	2'0" (+323.5)	SHGW @ 5'0" (+320.5) 5'3" (+320.25)	NE to 8'6"
TP-19	+326.0	5'0" (+321.0)	SHGW @ 5'0" (+321.0) 6'6" (+320.5)	NE to 10'0"
B-2-1	+327.0	4'0" (+323.0)	6'0" (+321.0)	CWR @ 13'0" (+314.0) AR @ 13'6" (+313.5)
B-2-2	+327.0	5'0" (+322.0)	5'0" (+322.0)	NE to 14'0"

NE – Not Encountered

CWR – Completely Weathered Rock

AR – Auger Refusal on Probable Rock

SHGW – Evidence of Seasonal High Groundwater

Groundwater was encountered in each of the borings and test pits at depths ranging from 5'0" to 6'6" (elevations +322.0 to +320.25) below the existing ground surface. In addition, the evidence of seasonal high groundwater table was observed in test pits TP-18 and TP-19 at a depth of 5'0" (elevations +320.5 and +321.0, respectively) below the existing ground surface. Due to the high groundwater table and seasonal high groundwater table, borehole infiltration tests were not performed at these locations.

Basin #2A

Basin #2A is located in the southeast corner of the site. Existing site grades in the proposed basin area are generally flat and range in elevation from +322.0 to +320.0. The proposed bottom of basin elevation is +316.0. Test pits TP-2-3, TP-2-4, and TP-2-5 were performed for the proposed basin. A summary of the test pit observations can be found in Table 3 below.

Table 3 – Basin #2A Summary of Test Pit Observations

Test Pit No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
TP-2-3	+320.5	1'3" (+319.25)	SHGW @ 1'9" (+318.75) 3'9" (+316.25)	NE to 4'6"
TP-2-4	+321.0	1'9" (+319.25)	SHGW @ 1'9" (+319.25) 3'9" (+317.25)	NE to 6'0"
TP-2-5	+321.0	1'0" (+320.0)	SHGW @ 2'0" (+319.0) 4'6" (+316.5)	NE to 6'6"

NE – Not Encountered

SHGW – Evidence Of Seasonal High Groundwater

Groundwater was encountered in each of the test pits at depths ranging from 3'9" to 4'6" (elevations +317.25 to +316.25) below the existing ground surface. In addition, evidence of seasonal high groundwater was observed in each of the test pits at depths ranging from 1'9" to 2'0" (elevations +319.25 to +318.75) below the existing ground surface. Due to the high groundwater table and seasonal high groundwater table, infiltration tests were not performed at these locations.

Basin #3

Basin #3 is located in the northeast corner of the site. Existing site grades in the proposed basin area are generally flat and range in elevation from +296.0 to +290.0. The proposed bottom of basin elevation is +290.0. Test pits TP-3-4, TP-3-5, and TP-3-6 were performed for the proposed basin. A summary of the test pit observations can be found in Table 4 below.

Table 4 – Basin #3 Summary of Test Pit Observations

Test Pit No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
TP-3-4	+293.0	1'6" (+291.5)	NE to 10'0"	NE to 10'0"
TP-3-5	+293.0	0'10" (+292.2)	NE to 9'3"	NE to 9'3"
TP-3-6	+290.0	2'0" (+288.0)	NE to 9'0"	NE to 9'0"

NE – Not Encountered

Groundwater was not encountered in any of the test pits. We anticipate that groundwater is deeper than approximate elevation +281.0. During this investigation, three (3) infiltration tests were performed in the proposed Basin #3. The infiltration tests were performed in accordance with NYSDEC procedures at depths specified by JMC PLLC. The results of the field infiltration rate are summarized in Table 5 below. In addition, the vertical permeability rate (K_m) was calculated using the equation below.

$$K_m = 1.142R_t \times \frac{\left[\ln \left(\frac{h_1}{h_2} \right) \right]}{(t_2 - t_1)}$$

Table 5 – Summary of Infiltration Results

Test Pit No.	Existing Ground Surface Elevation	Test Depth Below Existing Ground Surface (Elevation)	Field Infiltration Rate (in/hr)	Mean Permeability Coefficient, K_m (in/hr)
TP-3-4	+293.0	5'0" (+288.0)	3.0	0.20
TP-3-5	+293.0	5'0" (+288.0)	2.75	0.15
TP-3-6	+290.0	2'0" (+288.0)	5.0	0.35

Basin #3A

Basin #3A is located in the northeast corner of the site, just to the west of Basin #3. Existing site grades in the proposed basin area generally slope from west to east and range in elevation from +328.0 to +306.0. The proposed bottom of basin elevation is +318.0. Borings B-3-1, B-3-2, and B-3-3 and test pit TP-22 were performed for the proposed basin. A summary of the test pit and boring observations can be found in Table 6 below.

Table 6 – Basin #3A Summary of Test Pit and Boring Observations

Boring/ Test Pit No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
B-3-1	+324.0	NE	NE to 9'9"	CWR @ 9'6" (+314.5) AR @ 9'6" (+314.25)
B-3-2	+326.0	NE	NE to 14'0"	NE to 14'0"
B-3-3	+325.0	1'6" (+323.5)	NE to 12'0"	NE to 12'0"
TP-22	+321.5	1'9" (+319.75)	NE to 7'6"	CWR @ 5'9" (+315.75) BR @ 7'6" (+314.0)

NE – Not Encountered

CWR – Completely Weathered Rock

AR/BR – Auger or Bucket Refusal on Probable Rock

Groundwater was not encountered in any of the test pits or borings. We anticipate that groundwater is deeper than approximate elevation +312.0. During this investigation, three (3) borehole infiltration tests were performed in the proposed Basin #3A. The borehole infiltration tests were performed in accordance with NYSDEC procedures at depths specified by JMC PLLC. The results of the field infiltration rate are summarized in Table 7 below. In addition, the vertical permeability rate (K_m) was calculated using the equation below.

$$K_m = 1.142R_t \times \frac{\left[\ln \left(\frac{h_1}{h_2} \right) \right]}{(t_2 - t_1)}$$

Table 7 – Summary of Borehole Infiltration Results

Boring No.	Existing Ground Surface Elevation	Test Depth Below Existing Ground Surface (Elevation)	Field Infiltration Rate (in/hr)	Mean Permeability Coefficient, K_m (in/hr)
B-3-1	+324.0	6'0" (+318.0)	Greater than 20.0	3.35
B-3-2	+326.0	8'0" (+318.0)	6.0	0.37
B-3-3	+325.0	7'0" (+318.0)	5.0	0.31

Basin #4

Basin #4 is located in the northwest corner of the site. Existing site grades in the proposed basin area generally slope from east to west and range in elevation from +330.0 to +312.0. The proposed bottom of basin elevation is +328.0. Test pits TP-4-1, TP-4-2, TP-25 and TP-26 were performed for the proposed basin. A summary of the test pit observations can be found in Table 8 below.

Table 8 – Basin #4 Summary of Test Pit Observations

Test Pit No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
TP-4-1	+330.0	NE	NE to 1'9"	CWR @ 1'0" (+329.0) BR @ 1'9" (+328.25)
TP-4-2	+332.0	9'9" (+332.25)	NE to 9'9"	BR @ 9'9" (+332.25)
TP-25	+331.5	9'0" (+322.5)	NE to 12'0"	NE to 12'0"
TP-26	+334.0	6'9" (+327.25)	NE to 11'3"	NE to 11'3"

NE – Not Encountered

CWR – Completely Weathered Rock

BR – Bucket Refusal on Probable Rock

Groundwater was not encountered in any of the test pits. We anticipate that groundwater is deeper than approximate elevation +319.5. Bedrock was encountered in test pit TP-4-1 at a depth of 1'9" (elevation +328.25) below the existing ground surface. In addition, existing fill was encountered in the remaining test pits to depths ranging from 6'9" to 9'0" (approximate elevations +327.25 to +322.25) below the existing ground surface. Existing fill is not a suitable infiltration material because it is not a homogeneous material, and the permeability rate can vary significantly throughout this layer. Due to the high bedrock and deep existing fill infiltration tests were not performed in this area.

Basin #4A

Basin #4A is located in the northwest corner of the site, just to the west of Basin #4. Existing site grades in the proposed basin area generally slope from east to west and range in elevation from +312.0 to +303.0. The proposed bottom of basin elevation is +300.0. Borings B-

4-3, B-4-4, and B-4-5 were performed for the proposed basin. A summary of the boring observations can be found in Table 9 below.

Table 9 – Basin #4A Summary of Boring Observations

Boring No.	Approximate Ground Surface Elevation	Depth to Bottom of Existing Fill (Elevation)	Depth to Groundwater (Elevation)	Depth to Probable Bedrock (Elevation)
B-4-3	+304.0	7'0" (+297.0)	NE to 14'0"	CWR @ 13'0" (+291.0) AR @ 14'0" (+290.0)
B-4-4	+303.5	12'0" (+291.5)	NE to 12'0"	NE to 12'0"
B-4-5	+304.0	6'0" (+298.0)	NE to 14'0"	CWR @ 10'6" (+293.5) AR @ 14'0" (+290.0)

NE – Not Encountered

CWR – Completely Weathered Rock

AR – Auger Refusal on Probable Rock

Groundwater was not encountered in any of the borings. We anticipate that groundwater is deeper than approximate elevation +290.0. During this investigation, two (2) borehole infiltration tests were performed in the proposed Basin #4A. The borehole infiltration tests were performed in accordance with NYSDEC procedures at depths specified by JMC PLLC. The results of the field infiltration rate are summarized in Table 10 below. In addition, the vertical permeability rate (K_m) was calculated using the equation below.

$$K_m = 1.142R_t \times \frac{\left[\ln \left(\frac{h_1}{h_2} \right) \right]}{(t_2 - t_1)}$$

Table 10 – Summary of Borehole Infiltration Results

Boring No.	Existing Ground Surface Elevation	Test Depth Below Existing Ground Surface (Elevation)	Field Infiltration Rate (in/hr)	Mean Permeability Coefficient, K_m (in/hr)
B-4-3	+304.0	6'0" (+298.0)	Greater than 20.0	3.80
B-4-5	+304.0	7'0" (+297.0)	Greater than 20.0	3.35

4.1 Berm Construction

We anticipate that each of the stormwater basins will be designed with soil berms around the stormwater management basins. The new soil required to construct the berms fill must have a permeability rate low enough to prevent water seepage through the berms when the basins fill with water. The required soil permeability rate is dependent upon several variables such as the basin and berm geometry, the anticipated maximum water level, and time of detention. Further evaluation of the stormwater basin is required by the design team. Once the stormwater basin configuration is further developed additional recommendations can be provided upon request.

Based on the proposed grading plan, the soil slopes around the berms will be 3.0 horizontal to 1.0 vertical (3.0H:1V) or flatter. Based on the boring and test pit observations, slopes will consist of soil. Cuts and fills are expected to construct these slopes.

For slopes constructed in soil, we recommend a slope angle of 3.0H:1V or flatter. Steeper soil slopes will be prone to instability (i.e. sloughing or sliding) if not designed and constructed properly and if surface water and groundwater seepage are not properly controlled. Design of the soil slopes at the site is beyond the scope of this investigation.

All new slope construction must be overseen by a qualified geotechnical engineer (Carlin-Simpson & Associates), to ensure that they are properly constructed, surface water and groundwater infiltration is directed away from the top of slope, groundwater seepage exiting the slope is properly mitigated, and appropriate vegetation is established on the completed slope.

In order to prepare the new slope area for construction, the existing surface materials (i.e., topsoil and surface vegetation) shall be completely removed from the planned slopes areas receiving new fill. The removal of the topsoil and vegetation shall extend at least 5 feet beyond the construction limits of the new slope, including in front of the toe of the slope and within the areas to be filled. New soil slope embankments shall be constructed from the bottom up. End dumping from the top of the slope shall not be permitted. Each fill layer must be benched into the existing embankment for slope stability.

After the topsoil, surface vegetation, and unsuitable materials have been removed and prior to the placement of new fill, the exposed subgrade shall be proofrolled by several overlapping passes of a large vibratory drum roller (i.e. Dynapac CA 250 or equivalent). Where the existing subgrade is on a slope, the area shall be leveled to permit proper compaction of the subgrade prior to filling. The proofrolling is required to densify the underlying soils. If any soft or otherwise unsuitable soils are noted, the unsuitable material shall be removed and replaced with new compacted fill. Carlin-Simpson & Associates shall be responsible for determining what material, if any, is to be removed and will direct the contractor during this operation.

The subgrade receiving new fill must be firm and non-yielding prior to the placement of the next fill layer. Fill placed on existing slopes must be keyed or benched into the existing slope for slope stability. For existing slopes 4H:1V or flatter, the existing ground surface should be deeply scarified. For existing slopes steeper than 4H:1V, the ground surface should be benched. Benches into the existing slope should be a minimum width of 8 feet (typical small dozer blade width). The depth of the benches will be dictated by the existing embankment conditions.

The new fill shall be placed in layers up to 12 inches in loose thickness and each layer shall be compacted to at least 95% of its Maximum Modified Dry Density (ASTM D1557). Fill layers shall be compacted, tested, and approved before placing subsequent layers.

4.2 Suitability of the In-Situ Soils for Use as Compacted Fill

The suitability of each soil stratum for use as compacted fill is discussed below.

Stratum 1 Topsoil

Topsoil is not suitable for use as compacted fill. During construction, it shall be stripped from the construction areas. The topsoil may be reused in non-structural, non-sloped landscape areas or hauled offsite.

Stratum 2 Existing Fill

The existing fill consists of brown coarse to fine SAND, little (to and) Silt, trace (to little) coarse to fine Gravel.

Debris including glass, wood and organics was encountered in some of the borings and test pits. Debris must be removed prior to reuse as compacted fill. A screener may be required to remove the debris. In some cases, a screener may not be able to remove all debris and the material will then not be able to be reused as structural fill for the sloped areas.

Many cobbles and boulders were also encountered in portions of the existing fill. See bedrock section below for suitability of reuse.

This material may be suitable for use as compacted fill in sloped areas of the project, provided that it remains relatively dry for optimum compaction and all debris has been removed prior to use.

Stratum 3 Silty Gravelly Sand or Sandy Gravelly Silt

The Silty Sand with Gravel generally consists of brown coarse to fine SAND, little (to and) Silt, little (to some) coarse to fine Gravel or brown SILT and, coarse to fine Sand, little (to some) coarse to fine Gravel with cobbles and boulders. This material has can have a very high silt and clay content and will be very moisture sensitive. The high clay and silt content material will be difficult to reuse as compacted fill. However, as long as it remains relatively dry for optimum compaction prior to its use, it can be used as compacted fill.

Stratum 4/5 Bedrock

Excavated rock or completely weathered rock may be used as fill material provided that the material is well graded and has been approved prior to use by Carlin-Simpson & Associates.

All rock fill must be well blended with smaller rock fragments and/or soil. In general site fills, the maximum particle size shall be six (6) inches in diameter. Most of the excavated rock, cobbles, and boulders will be too large for use as compacted fill in structural areas. The excavated rock must therefore be processed through a crusher to provide suitable fill material. Rock fill should not be used where it will interfere with the installation of foundations, floor slabs or utilities. Also, it shall not be used as backfill directly against concrete walls or utilities.

The boring observations indicate that the on-site soils contain a varying percentage of silt (13.3% to greater than 50%). The moderate to high silt content soils, particularly some of the existing fill, will be moisture sensitive. If the soil becomes too wet, it will be difficult to achieve adequate compaction. In addition, the site soils that extend below the groundwater table are completely saturated and therefore, unsuitable for reuse.

Proper moisture conditioning of the soil will be required. New compacted fill should be within 2% (+/-) of its optimum moisture content at the time of placement. In the event that the on-site material is too wet at the time of placement and cannot be adequately compacted, the soil should be aerated and allowed to dry or the material removed and a drier cleaner fill material used. In the event that the on-site material is too dry at the time of placement and cannot be adequately compacted, water may be needed to increase the soil moisture content for proper compaction.

The in-situ soils which exist throughout the site may become soft and weave if exposed to excessive moisture and construction traffic. The instability will occur quickly when exposed to these elements and it will be difficult to stabilize the subgrade. We recommend that adequate site drainage be implemented early in the construction schedule and if the subgrade becomes wet, the contractor should limit construction activity until the soil has dried.

The minimum compaction requirements for the various areas of the site are summarized in Table 11 below.

Table 11 - Minimum Compaction Requirements

Area	Maximum Modified Dry Density (ASTM D-1557)
Soil Slopes	95%
Landscape Areas – non-sloped	90%

5.0 GENERAL

The findings, conclusions and recommendations presented in this report represent our professional opinions concerning subsurface conditions at the site. The opinions presented are relative to the dates of our site work and should not be relied on to represent conditions at later dates or at locations not explored. The opinions included herein are based on information provided to us, the data obtained at specific locations during the study and our past experience. If additional information becomes available that might impact our geotechnical opinions, it will be necessary for Carlin-Simpson & Associates to review the information, reassess the potential concerns, and re-evaluate our conclusions and recommendations.

Regardless of the thoroughness of a geotechnical exploration, there is the possibility that conditions between borings and test pits will differ from those encountered at specific boring or test pit locations, that conditions are not as anticipated by the designers and/or the contractors, or that either natural events or the construction process have altered the subsurface conditions. These variations are an inherent risk associated with subsurface conditions in this region and the

approximate methods used to obtain the data. These variations may not be apparent until construction.

The professional opinions presented in this geotechnical report are not final. Field observations and foundation installation monitoring by the geotechnical engineer, as well as soil density testing and other quality assurance functions associated with site earthwork and foundation construction, are an extension of this report. Therefore, Carlin-Simpson & Associates should be retained by the Owner to observe all earthwork and foundation construction, to document that the conditions anticipated in this study actually exist, and to finalize or amend our conclusions and recommendations. Carlin-Simpson & Associates is not responsible or liable for the conclusions and recommendations presented in this report if Carlin-Simpson & Associates does not perform the observation and testing services.

Therefore, in order to preserve continuity in this project, the Owner must retain the services of Carlin-Simpson & Associates to provide full time geotechnical related monitoring and testing during construction. At a minimum, this shall include the observation and testing of the following: 1) the removal of existing fill and unsuitable soil, where required; 2) the proofrolling of the subgrade soil prior to the placement of new compacted fill; 3) the placement and compaction of controlled fill; and 4) the construction of soil slopes.

This report has been prepared in accordance with generally accepted geotechnical engineering practice. No other warranty is expressed or implied. The evaluations and recommendations presented in this report are based on the available project information, as well as on the results of the exploration. Carlin-Simpson & Associates should be given the opportunity to review the final drawings and site plans for this project to determine if changes to the recommendations outlined in this report are needed. Should the nature of the project change, these recommendations should be re-evaluated.

This report is provided for the exclusive use of Ridgewood Real Estate Partners and the project specific design team and may not be used or relied upon in connection with other projects or by other third parties. Carlin-Simpson & Associates disclaims liability for any such third party use or reliance without express written permission. Use of this report or the findings, conclusions or recommendations by others will be at the sole risk of the user. Carlin-Simpson & Associates is not responsible or liable for the interpretation by others of the data in this report, nor their conclusions, recommendations or opinions.

If the conditions encountered during construction vary significantly from those stated in this report, this office should be notified immediately so that additional recommendations can be made.

Thank you for allowing us to assist you with this project. Should you have any questions or comments, please contact this office.

Very truly yours,

CARLIN-SIMPSON & ASSOCIATES, LLC



CATHERINE K. ANDERSEN, P.E.
Project Engineer



ROBERT B. SIMPSON, P.E.
Principal



CARLIN-SIMPSON & ASSOCIATES, LLC

Consulting Geotechnical & Environmental Engineers

Proposed Residential Development
Elmwood Country Club
Greenburgh, NY
18-40

13 February 2023

TP-2-3 (Elev. +320.5)

0'0"-0'6"	Dark brown topsoil	
0'6"-1'3"	FILL (Brown, dark gray coarse to fine SAND, some Silt, little (-) medium to fine Gravel)	loose, moist
1'3"-1'9"	Dark gray Clayey SILT little (+), coarse to fine Sand, trace (-) fine Gravel	medium stiff, moist
1'9"-3'9"	Mottled light gray, orange brown, red brown coarse to fine Sand, and (+) Silt, trace (+) fine Gravel	medium dense, moist to wet
3'9"-4'6"	Dark gray coarse to fine SAND, little Silt, little (+) medium to fine Gravel	medium dense, moist to wet
4'6"	Brown coarse to fine SAND, trace Silt, trace (+) fine Gravel	medium dense, wet
Groundwater encountered @ 3'9"		
Seasonal high groundwater encountered @ 1'9"		

13 February 2023

TP-2-4 (Elev. +321.0)

0'0"-0'6"	Dark brown topsoil	
0'6"-1'9"	FILL (Brown coarse to fine SAND, some Silt, little medium to fine Gravel)	medium dense, moist
1'9"-3'6"	Slightly mottled light gray, orange brown coarse to fine SAND, and (+) Silt, trace (+) fine Gravel	medium dense, moist
3'6"-5'0"	Gray coarse to fine SAND, little (+) Silt, little (+) medium to fine Gravel	medium dense, moist to wet
5'0"-6'0"	Brown coarse to fine SAND, trace (+) Silt, trace fine Gravel	medium dense, wet
	Groundwater encountered @ 3'9"	
	Seasonal high groundwater encountered @ 1'9"	

TP-2-5 (Elev. +321.0)

0'0"-0'6"	Dark brown topsoil	
0'6"-1'0"	FILL (Brown coarse to fine SAND, some (+) Silt, trace coarse to fine Gravel)	loose, moist
1'0"-2'0"	Dark gray Clayey SILT little (+), coarse to fine Sand, trace (-) to fine Gravel	medium stiff, moist
2'0"-3'6"	Mottled light gray, orange brown, red brown coarse to fine Sand, and (+) Silt, trace medium to fine Gravel	medium dense, moist to wet
3'6"-5'6"	Gray coarse to fine SAND, little (+) Silt, little (+) medium to fine Gravel	medium dense, moist to wet
5'6"-6'6"	Brown coarse to fine SAND, trace (+) Silt, trace fine Gravel	medium dense, wet
	Groundwater encountered @ 4'6"	
	Seasonal high groundwater encountered @ 2'0"	

13 February 2023

TP-3-4 (Elev. +293.0)

0'0"-0'5"	Black topsoil	
0'5"-1'6"	FILL (Brown coarse to fine SAND, some (-) Silt, little (+) medium to fine Gravel)	loose, moist
1'6"-3'9"	Brown coarse to fine SAND, little (+) Silt, little (+) coarse to fine Gravel, with occasional cobbles	medium dense, moist
3'9"-5'0"	Light gray, brown coarse to fine SAND, and (-) Silt	medium dense, moist
5'0"-8'0"	Gray coarse to fine SAND, little (+) Silt, little (+) coarse to fine Gravel, with occasional cobbles, with seams of coarse to fine SAND, and (-) Silt	dense, moist
8'0"-10'0"	Brown coarse to fine SAND, trace (+) Silt, trace (+) medium to fine Gravel	medium dense, moist
No groundwater encountered		

TP-3-5 (Elev. +293.0)

0'0"-0'2"	Black topsoil	
0'2"-0'10"	FILL (Light gray coarse to fine SAND, trace Silt)	loose, moist
0'10"-5'0"	Brown coarse to fine SAND, some (-) Silt, little coarse to fine Gravel, with occasional cobbles	medium dense, moist
5'0"-6'6"	Gray, brown coarse to fine SAND, little Silt, little (+) coarse to fine Gravel	dense, moist
6'6"-9'3"	Brown coarse to fine SAND, trace Silt, trace (+) medium to fine Gravel	medium dense, moist
No groundwater encountered		

13 February 2023

TP-3-6 (Elev. +290.0)

0'0"-0'4"	Black topsoil	
0'4"-2'0"	FILL (Dark brown coarse to fine SAND, little (+) Silt, little medium to fine Gravel)	loose, moist
2'0"-4'6"	Brown coarse to fine Sand, and (+) Silt, little (-) coarse to fine Gravel, with occasional cobbles and boulders	medium dense, moist
4'6"-9'0"	Gray, brown coarse to fine SAND, little (+) Silt, little (+) coarse to fine Gravel, with occasional cobbles	medium dense, moist
No groundwater encountered		

TP-4-1 (Elev. +330.0)

0'0"-0'3"	Black topsoil	
0'3"-1'0"	Brown coarse to fine SAND, little Silt, little (+) coarse to fine Gravel	loose, moist
1'0"-1'9"	Gray Gneiss bedrock, very blocky	rippable
1'9"	Gray Gneiss bedrock	unrippable
No groundwater encountered		

TP-4-2 (Elev. +332.0)

0'0"-0'2"	Black topsoil	
0'2"-9'9"	FILL (Brown coarse to fine SAND, little (+) Silt, little coarse to fine Gravel, with occasional cobbles and boulders, roots, many boulders @ 5'6")	loose, moist
Bucket refusal on boulders @ 9'9"		

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-1-1		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +347.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 28/Dec/22	
28/Dec/22		845	5'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 28/Dec/22	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-0	12		FILL (Br, gr cf G s, cf S, l (-) \$)					Rec = 7" moist	
			4								
2			2		<u>FILL (Brown, gray coarse to fine Sand, some Silt, little coarse to fine Gravel)</u>						
			3								
3		S-1	3		FILL (Br cf S, s \$, l (+) cf G)					Rec = 19" moist	
			8		3'6"						
4			7		Br cf S, l (-) \$, l (+) cf G						
			10		<u>Brown coarse to fine SAND, little (-) Silt, little (+) coarse to fine Gravel</u>						
5		S-2	9		6'0"					Rec = 9" moist	
			22								
6			40								
7		S-3	50/5"		Gneiss, Completely Weathered, Br, gr cf S, l (-) \$, a cf G					Rec = 22" wet	
					<u>Gneiss, Completely Weathered</u>						
8											
9		S-4	17		9'0"					Rec = 9" moist-wet Auger refusal @ 9'0"	
			50/4"								
					<u>End of Boring @ 9'0"</u>						
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-1-2	
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1	
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40	
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +347.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 28/Dec/22
28/Dec/22		1130	6'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 28/Dec/22
					WGHT		140#			DRILLER: M Kane
					FALL		30"			INSPECTOR: JP
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS
1		S-1	4		<u>Topsoil</u>					0'5"
			5		<u>FILL (Brown coarse to fine SAND, some (-) Silt, little (+) coarse to fine Gravel)</u>					Rec = 14" moist
2			8							
			10							2'0"
			8							
3		S-2	7		Gr cf S, l \$, s cf G					Rec = 13" moist
			16							
4			9							
					<u>Gray coarse to fine SAND, little Silt, some coarse to fine Gravel</u>					Auger refusal 4'6" moved 5' south
5			12							
6		S-3	8		same, br l (+) \$, s (-) cf G					Rec = 18" moist-wet
			7							
7			8							
			9							7'6"
8		S-4	10		Br, gr cf S, l \$, s cf G					Rec = 14" wet
			10							
9			16		<u>Brown, gray coarse to fine SAND, little Silt, some coarse to fine Gravel</u>					
10		S-5	50/5"		same					9'11"
					<u>End of Boring @ 9'11"</u>					Rec = 4" wet
11										Auger refusal @ 9'6"
12										
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-1-3		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +346.5		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 28/Dec/22	
28/Dec/22		940	9'6"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 28/Dec/22	
12/28/2022		1000	8'0"	OPEN	WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	21		FILL (Br cf S, l (-) \$, s cf G)					Rec = 10" moist	
			11								
			7								
2		S-2	5		FILL (same, l (+) \$, w/brick)					Rec = 6" moist	
			4								
			5								
3		S-3	3		FILL (same) <u>FILL (Brown coarse to fine SAND, little (-) Silt, some coarse to fine Gravel)</u>					Rec = 6" moist	
			6								
			12								
4		S-4	6		FILL (same)					Rec = 9" moist-wet	
			4								
			2								
5		S-5	3		CWR, Br, gr, or cf S, l \$, a cf G					Rec = 10" wet	
			3								
			4								
6		S-6	7		<u>Gneiss, Completely Weathered</u>					Rec = 15" wet	
			5								
			10								
7			15		<u>End of Boring @ 14'0"</u>					14'0"	
			11								
			21								
8					<u>End of Boring @ 14'0"</u>					14'0"	
9					<u>End of Boring @ 14'0"</u>					14'0"	
10					<u>End of Boring @ 14'0"</u>					14'0"	
11					<u>End of Boring @ 14'0"</u>					14'0"	
12					<u>End of Boring @ 14'0"</u>					14'0"	
13					<u>End of Boring @ 14'0"</u>					14'0"	
14					<u>End of Boring @ 14'0"</u>					14'0"	
15					<u>End of Boring @ 14'0"</u>					14'0"	
16					<u>End of Boring @ 14'0"</u>					14'0"	
17					<u>End of Boring @ 14'0"</u>					14'0"	
18					<u>End of Boring @ 14'0"</u>					14'0"	
19					<u>End of Boring @ 14'0"</u>					14'0"	
20					<u>End of Boring @ 14'0"</u>					14'0"	
21					<u>End of Boring @ 14'0"</u>					14'0"	
22					<u>End of Boring @ 14'0"</u>					14'0"	

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-1-4		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +347.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 28/Dec/22	
28/Dec/22		1030	8'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 28/Dec/22	
					WGHT		140#			DRILLER: Mike K.	
					FALL		30"			INSPECTOR: J.P.	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
			2		<u>Topsoil</u>					0'5"	
1		S-1	4		FILL (Br cf S, s \$, l (+) cf G)					Rec = 9" moist	
			4								
2			8		<u>FILL (Brown coarse to fine SAND, some Silt, little (+) coarse to fine Gravel)</u>					Boulder	
										3'0"	
3											
			3		<u>Mottled light gray, orange, brown SILT some, coarse to fine Sand, little (-) coarse to fine Gravel</u>					Rec = 16" moist	
4		S-2	4							4'6"	
			5								
5			12		Br cf S, l (-) \$, a cf G						
			12								
6		S-3	15		same					Rec = 3" moist	
			13								
7			15		<u>Brown coarse to fine SAND, little (-) Silt, and coarse to fine Gravel</u>						
			19								
8		S-4	44		same, s cf G					Rec = 14" wet	
			17								
9			15								
10											
			10							10'6"	
11		S-5	11							Rec = 24" wet	
			15		<u>Gneiss, Completely Weathered</u>						
12			50/5"							11'11"	
					<u>End of Boring @ 11'11"</u>						
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-2-1	
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1	
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40	
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +327.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE	TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 29/Dec/22	
29/Dec/22	830	6'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 29/Dec/22	
				WGHT		140#			DRILLER: M Kane	
				FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION				REMARKS	
1		S-1	2		FILL (Br cf S, s (-) \$, l cf G, w/glass) <u>FILL (Brown coarse to fine SAND, some (-) Silt, little coarse to fine Gravel, with glass)</u>				Rec = 15" moist	
	3									
2			2							
			3							
3		S-2	4		FILL (same)				Rec = 8" moist	
	6									
4			4							
			5		4'0"					
5										
			6							
6		S-3	4		Br cf S, s (-) \$, l (-) cf G				Rec = 8" wet at tip	
	5									
7			6							
			4							
8		S-4	5	same	<u>Brown coarse to fine SAND, some (-) Silt, little (-) coarse to fine Gravel</u>				Rec = 18" wet	
	6									
9			6							
10										
			5							
11		S-5	2		same, t (+) \$				Rec = 24" wet	
	2									
12			5							
			7							
13		S-6	8		same, a cf G				Rec = 6" wet	
	50/6"									
14										
					<u>Gneiss, Completely Weathered</u>					
					<u>End of Boring @ 13'6"</u>					
15										
16										
17										
18										
19										
20										
21										
22										

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-2-2	
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1	
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40	
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +327.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE	TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 29/Dec/22	
2/29/22	800	5'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 29/Dec/22	
				WGHT		140#			DRILLER: M Kane	
				FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION				REMARKS	
1		S-1	10		FILL (Dk br cf S, s \$, l (+) cf G, w/debris)				Rec = 11" moist	
			3							
2			3							
			8		FILL (No Rec)				Rec = 0 Boulder	
3		S-2	15							
			50/3"							
4					<u>FILL (Dark brown coarse to fine SAND, some Silt, little (+) coarse to fine Gravel, with debris)</u>				5'0"	
5										
6		S-3	1		Mottled bk, gr, or, br cf S, s (+) \$, l cf G, w/t orgs				Rec = 15" wet	
			3							
7			3							
			5		same, br, gr, or				Rec = 16" wet	
8		S-4	7							
			8							
9			6		<u>Black, gray, orange, brown coarse to fine SAND, some (+) Silt, little coarse to fine Gravel</u>				12'0"	
			6							
10										
			3		same, gr, bk, br a Cy \$				Rec = 7" wet	
11		S-5	3							
			5							
12			3		Br cf S, t \$, t (+) cf G				Rec = 6" wet	
13		S-6	1							
			1		<u>Brown coarse to fine SAND, trace Silt, trace (+) coarse to fine Gravel</u>				14'0"	
14			1							
15					<u>End of Boring @ 14'0"</u>					
16										
17										
18										
19										
20										
21										
22										

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-3-1		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +324.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 30/Dec/22	
30/Dec/22		1200	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 30/Dec/22	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	1		<u>Topsoil</u>					0'6"	
			2		Br cf S, s (+) \$, l cf G					Rec = 14" moist	
2			2		<u>Brown coarse to fine SAND, some (+) Silt, little coarse to fine Gravel</u>						
			3								
			4							2'6"	
3		S-2	8		Br, gr cf S, l (-) \$, l (+) cf G					Rec = 12" moist	
			18								
4			20								
5					<u>Brown, gray coarse to fine SAND, little (-) Silt, little (+) coarse to fine Gravel</u>					Rec = 16" moist	
			14								
6		S-3	12		same, t (+) \$						
			12							Rec = 10" moist	
7			14								
			12								
8		S-4	15		same, l (-) \$					Rec = 10" moist	
			10								
9			8								
										9'6"	
10		S-5	50/3"		<u>Gneiss, Completely Weathered</u>					9'9"	
					<u>End of Boring @ 9'9"</u>					Auger refusal @ 9'6"	
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-3-2		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +326.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Jan/23	
3/Jan/23		1015	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Jan/23	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	S y m	IDENTIFICATION					REMARKS	
			1		<u>Topsoil</u>					0'4"	
1		S-1	2		Dk br cf S, s (-) \$, l cf G					Rec = 9"	
			4							moist	
2			5								
			9								
3		S-2	22		same, l \$, s cf G					Rec = 3"	
			50/1"							moist	
4										spoon walking	
										3x auger refusal	
5										moved 10' west	
			20								
6		S-3	17		same, br, gr					Rec = 5"	
			27		<u>Dark brown coarse to fine SAND,</u>					moist	
7			29		<u>little Silt, little coarse to fine Gravel</u>						
			22								
8		S-4	21		same					Rec = 18"	
			21							moist	
9			23								
10											
			11								
11		S-5	13		same					Rec = 23"	
			17							moist	
12			23								
			20								
13		S-6	21		same					Rec = 13"	
			23							moist	
14			21							14'0"	
					<u>End of Boring @ 14'0"</u>						
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-3-3		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +325.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 30/Dec/22	
30/Dec/22		1200	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 30/Dec/22	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	1		Topsoil					0'1"	(Sand trap) Rec = 13" moist
			2		FILL (Brown coarse to fine SAND, trace Silt, some (+) coarse to fine Gravel)					1'6"	
2			2		Br cf S, s (+) \$, l (-) cf G						
			3								
3		S-2	3		same						Rec = 11" moist
			2								
4			4								
			5								
5											
6		S-3	6		same, l (+) \$, s (-) cf G						Rec = 5" moist
			15								
7			23		Brown coarse to fine SAND, little (+) Silt, little (-) coarse to fine Gravel						
			37								
8		S-4	22		same, br, wt, or						Rec = 20" moist
			12								
9			23								
			28								
10											
11		S-5	19		same						Rec = 10" moist
			19								
12			18								
			14								
13					End of Boring @ 12'0"					12'0"	
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-4-3	
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1	
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40	
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +304.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 29/Dec/22
29/Dec/22		1300	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 29/Dec/22
					WGHT		140#			DRILLER: M Kane
					FALL		30"			INSPECTOR: JP
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS
1		S-1	3		Topsoil 0'2"					Rec = 11" moist
			3		FILL (Br, gr cf S, s (-) \$, l cf G)					
2			4		FILL (same, t orgs) <u>FILL (Brown, gray coarse to fine SAND, some (-) Silt, little coarse to fine Gravel with wood, roots, organics)</u>					Rec = 19" moist
			7							
3	S-2	6								
		5								
4		7								
5					FILL (same, dk gr, br w/wood, roots, orgs) 7'0"					Boulder
		3								
6	S-3	3								
		3								
7		3								
8	S-4	4		Br cf S, t \$, l cf G <u>Brown coarse to fine SAND, little (-) Silt, little coarse to fine Gravel</u>					Rec = 6" moist	
		5								
9		4								
		8								
10										same, l (-) \$, s cf G
		7								
11	S-5	8								
		8								
12		8								
13	S-6	7		same 13'0"					Rec = 6" moist	
		9								
14		10								
		11								
15										<u>Gneiss, Completely Weathered</u> 14'0" <u>End of Boring @ 14'0"</u>
16										
17										
18										
19										
20										
21										
22										

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-4-4		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +303.5		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 29/Dec/22	
29/Dec/22		1200	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 29/Dec/22	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	S y m	IDENTIFICATION					REMARKS	
			6		<u>Topsoil</u>					0'2"	
1		S-1	3		FILL (Br, gr cf S, l (-) \$, t (+) cf G)					Rec = 10" moist	
			6								
2			7								
			7							Rec = 4" wet	
3		S-2	11		FILL (same, t \$)						
			4								
4			7		<u>FILL (Brown, gray coarse to fine SAND, little (-) Silt, little (+) coarse to fine Gravel with trace organics)</u>						
5											
			5							Rec = 12" moist	
6		S-3	4		FILL (Gr cf S, l (+) \$, l cf G, w/t orgs)						
			3								
7			5							Rec = 12" moist Boulder	
			5								
8		S-4	5		FILL (same, br)						
			8								
9			8								
										Rec = 10" moist	
10											
			4								
11		S-5	2		FILL (same, bk, br, gr w/org)						
			2								
12			3							12'0"	
					<u>End of Boring @ 12'0"</u>						
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN-SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-4-5		
Project: Proposed Residential Development, Elmwood Country Club, Greenburgh NY									SHEET NO.: 1 of 1		
Client: Ridgewood Elmwood Owner, LLC									JOB NUMBER: 18-40		
Drilling Contractor: Environmental Technical Drilling									ELEVATION: +304.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 29/Dec/22	
29/Dec/22		1345	NWE	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 29/Dec/22	
					WGHT		140#			DRILLER: M Kane	
					FALL		30"			INSPECTOR: JP	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	2		Topsoil					0'2"	
			4		FILL (Dk br cf S, s (-) \$, l cf G)					Rec = 19" moist	
2			4								
			4								
3		S-2	4								
			5		FILL (same)					Rec = 4" moist	
4			3		FILL (Dark brown coarse to fine SAND, some (-) Silt, little coarse to fine Gravel)						
			4								
5											
6		S-3	4								
			3		FILL (same)					Rec = 8" moist	
7			3		Br cf S, l \$, l (+) cf G						
			6								
8		S-4	3								
			4		same, s \$, l (-) cf G					Rec = 12" moist cobble	
9			4								
			12		Brown coarse to fine SAND, little (+) Silt, little (+) coarse to fine Gravel						
10											
11		S-5	2							10'6"	
			15		Br, wt, gld cf S, l \$, s cf G					Rec = 9" moist	
12			10								
			10		Gneiss, Completely Weathered						
13		S-6	12		Brown, white, gold coarse to fine SAND, little Silt, some coarse to fine Gravel,						
			18		same					Rec = 21" moist	
14			19								
			15							14'0"	
15					End of Boring @ 14'0"						
16											
17											
18											
19											
20											
21											
22											

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Consulting Engineers Geotechnical & Environmental

Proposed Residential Development
Elmwood Country Club Re-Development
Greenburgh, NY
18-40

6 May 2021

TEST PIT LOGS

TP-18 (Elev. +325.5)

0'0" - 0'2"	Topsoil	
0'2" - 2'0"	FILL (Dark gray, brown coarse to fine SAND, little (+) Silt, little (+) medium to fine Gravel, with a lot of debris, asphalt, wood)	loose, moist
2'0" - 5'0"	Brown coarse to fine SAND, little Silt, some (-) coarse to fine Gravel with occasional cobbles	medium dense, moist
5'0" - 8'6"	Brown coarse to fine SAND, trace (-) Silt, little medium to fine Gravel, many boulders 6'6" to 8'6"	medium dense, wet
	Groundwater encountered @ 5'3"	
	Seasonal High Groundwater @ 5'0"	

TP-19 (Elev. +326.0)

0'0"-0'8"	Topsoil	
0'8"-2'3"	FILL (Dark gray, gray coarse to fine SAND, some (-) Silt, little medium to fine Gravel, with a lot of mixed topsoil)	loose, moist
2'3"-5'0"	FILL (Brown, gray coarse to fine SAND, little (+) Silt, little medium to fine Gravel)	loose, moist
5'0"-7'6"	Mottled gray, red brown, brown coarse to fine SAND, and (+) Silt, little (-) fine Gravel	medium dense, wet
7'6"-10'0"	Gray coarse to fine SAND, trace Silt	medium dense, wet
	Groundwater encountered @ 6'6"	
	Seasonal High Groundwater @ 5'0"	

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Elmwood Country Club Re-Development
Greenburgh, NY
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6 May 2021

TP-20 (Elev. +349.0)

0'0"-1'0"	Topsoil	
1'0"-1'9"	Brown SILT some (-), coarse to fine Sand, trace (-) fine Gravel	soft, moist
1'9"	Gray Schist bedrock	unrippable
	Refusal on Schist bedrock @ 1'9"	
	No groundwater encountered	

TP-20A (Elev. +350.0)

0'0"-1'0"	Topsoil	
1'0"-2'0"	Brown SILT some, coarse to fine Sand, trace (-) fine Gravel	soft, moist
2'0"	Gray Schist bedrock	unrippable
	Refusal on Schist bedrock @ 2'0"	
	No groundwater encountered	

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Greenburgh, NY
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TP-21 (Elev. +343.5)

0'0"-0'6"	Topsoil	
0'6"-4'6"	FILL (Dark brown coarse to fine SAND, some (-) Silt, trace fine Gravel, with occasional brick)	loose, moist
4'6"-7'0"	Mottled light gray, red brown, brown coarse to fine SAND, and (-) Silt, trace (-) coarse to fine Gravel, with boulders	medium dense, wet
7'0"-8'3"	Brown coarse to fine SAND, trace Silt, some (+) coarse to fine Gravel, with cobbles	medium dense, wet
	Groundwater encountered @ 6'3", rapid inflow Seasonal High Groundwater @ 4'6"	

TP-22 (Elev. +321.5)

0'0"-0'5"	Topsoil	
0'5"-1'9"	FILL (Gray, brown coarse to fine SAND, little (+) Silt, little medium to fine Gravel	loose, moist
1'9"-3'9"	Brown SILT and, coarse to fine Sand, trace (+) fine Gravel	medium stiff, moist
3'9"-5'9"	Gray, brown coarse to fine SAND, trace (+) Silt, little (+) coarse to fine Gravel	medium dense, moist
5'9"-7'6"	Gray highly weathered Schist	rippable, moist
7'6"	Gray Schist bedrock	unrippable
	Refusal on bedrock @ 7'6" No groundwater encountered	

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Greenburgh, NY
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TP-23 (Elev. +311.0)

0'0"-0'8"	Topsoil	
0'8"-3'9"	Gray, brown SILT little (+), coarse to fine Sand, little (-) coarse to fine Gravel, with cobbles	medium stiff, moist
3'9"-6'9"	Gray, brown, orange brown coarse to fine SAND, little (+) Silt, some (-) medium to fine Gravel	medium dense, moist
6'9"-8'0"	Gray highly weathered Schist	rippable, moist to wet
8'0"	Gray Schist Bedrock	unrippable
	Groundwater encountered @ 7'6", slow inflow Seasonal High Groundwater @ 6'6" Refusal on Schist bedrock @ 8'0"	

TP-24 (Elev. +325.0)

0'0"-0'8"	Topsoil	
0'8"-7'0"	Brown coarse to fine SAND, trace Silt, little (+) coarse to fine Gravel	medium dense, moist to wet
7'0"-8'6"	Gray coarse to fine SAND, and Silt, little (+) medium to fine Gravel	medium dense, wet
	Groundwater encountered @ 4'0" Seasonal High Groundwater @ 3'6"	

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Elmwood Country Club Re-Development
Greenburgh, NY
18-40

6 May 2021

TP-25 (Elev. +331.5)

0'0"-0'3"	Topsoil	
0'3"-1'3"	FILL (Light gray coarse to fine SAND, trace (-) Silt)	loose, moist
1'3"-1'5"	FILL (Gray clean 1/4" Gravel)	loose, moist
1'5"-3'3"	FILL (Dark gray, black coarse to fine SAND, some Silt, some coarse to fine Gravel)	dense, moist
3'3"-9'0"	FILL (Brown coarse to fine SAND, little (+) Silt, little (+) coarse to fine Gravel, with many boulders 6'0" to 9'0")	medium dense, moist
9'0"-9'9"	Brown SILT some (-), coarse to fine Sand	medium stiff, moist
9'9"-12'0"	Brown coarse to fine SAND, little Silt, little (+) coarse to fine Gravel	dense, moist
	No groundwater encountered	

TP-26 (Elev. +334.0)

0'0"-0'4"	Topsoil	
0'4"-6'9"	FILL (Dark brown coarse to fine SAND, little (+) Silt, little (-) medium to fine Gravel)	loose, moist
6'9"-8'6"	Gray, brown coarse to fine SAND, and (+) Silt, trace (-) fine Gravel	medium dense, moist
8'6"-11'3"	Brown, gray coarse to fine SAND, little Silt, little (+) coarse to fine Gravel many boulders @ 9'	medium dense, moist
	No groundwater encountered	

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Residential Development
Elmwood Country Club
Greenburgh, NY
18-40

30 July 2018

TEST PIT LOGS

TP-1 (Elevation +356.0)

0'0" - 0'8" Brown topsoil

0'8" - 7'0" Brown coarse to fine SAND, some (-) Silt, medium dense, moist
some coarse to fine Gravel, with cobbles,
boulders

7'0" - Probable bedrock

No Water Encountered

TP-2 (Elevation +324.0)

0'0" - 0'6" Brown topsoil

0'6" - 3'8" Brown SILT and, coarse to fine Sand loose, moist
little to fine Gravel

3'8" - 11'0" Brown coarse to fine SAND, little Silt, loose, moist
little (+) to fine Gravel

No Water Encountered

TP-3 (Elevation +338.0)

0'0" - 0'8" Brown topsoil

0'8" - 10'0" Brown coarse to fine SILT and (+), coarse to medium dense, moist
fine Sand, little coarse to fine Gravel, with
cobbles and boulders

No Water Encountered

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TP-4 (Elevation +323.0)

0'0" – 0'7"	Brown topsoil	
0'7" – 4'5"	Light brown coarse to fine SAND, some (+) Silt, little coarse fine Gravel	medium dense, moist
4'5" – 7'6"	Brown coarse to fine SAND, some (+) Silt, and coarse to fine Gravel, with rock fragments	medium dense, moist
7'6" - 10'0"	Brown coarse to fine SAND, little (-) Silt, little (-) coarse to fine Gravel	medium dense, moist
	No Water Encountered	

TP-5 (Elevation +295.0)

0'0" – 5'6"	Brown coarse to fine SAND, some (+) Silt, Some coarse to fine Gravel, with cobbles	medium dense, moist
5'6" – 7'0"	Brown coarse to fine SAND, little (-) Silt, some coarse to fine Gravel	medium dense, moist
7'0"	Refusal, probable bedrock	
	No Groundwater Encountered	

TP-6 (Elevation +368.0)

0'0" – 0'10"	Brown topsoil	
0'10" – 5'0"	Brown coarse to fine SAND, some (+) Silt, little to fine Gravel	medium dense, moist
5'0" – 6'9"	Brown coarse to fine SAND, little Silt, some coarse to fine Gravel	medium dense, moist
6'9"	Refusal, probable bedrock	
	No Water Encountered	

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TP- 7 **(Elevation +371.0)**

0'0" – 0'7" Brown topsoil

0'7" – 6'9" Brown coarse to fine SILT and (+), coarse to
fine Sand, little coarse to fine Gravel, with
cobbles and boulders medium dense, moist

6'9" Refusal, probable bedrock

TP-8 **(Elevation +370.0)**

0'0"- 0'6" Brown topsoil

0'6" – 1'3" Schist, weathered fractured

1'3" Refusal, bedrock

TP-9 **(Elevation +360.0)**

0'0" – 0'5" Brown topsoil

0'5" – 2'8" Brown coarse to fine SAND, and Silt,
some coarse to fine Gravel, with fractured
bedrock medium dense, moist

2'8" Refusal, bedrock

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TP-10 (Elevation +329.0)

0'0" – 2'0"	FILL (Brown coarse to fine SAND, some Silt, some (-) coarse to fine Gravel, with cobbles)	loose, moist
2'0" – 2'6"	Dark brown topsoil	loose
2'6" – 3'2"	Brown coarse to fine SAND, trace (+) Silt, little coarse to fine Gravel	medium dense, moist
3'2" – 5'8"	Brown coarse to fine SAND, little (+) Silt, some coarse to fine Gravel, with cobbles	medium dense, moist
5'8"	Refusal, bedrock	
	No Water Encountered	

TP-11 (Elevation +329.0)

0'0" – 0'8"	Topsoil	
0'8" – 4'0"	Brown coarse to fine SAND, some (-) Silt, some (+) coarse to fine Gravel, with cobbles	medium dense, moist
4'0" – 10'0"	Brown coarse to fine SAND, little Silt, some (+) coarse to fine Gravel, with cobbles	medium dense, moist
	No Water Encountered	

TP-12 (Elevation +327.0)

0'0" – 0'5"	Brown topsoil	
0'5" – 5'0"	Brown coarse to fine SILT and (+), coarse to fine Sand, little coarse to fine Gravel, with cobbles and boulders	medium dense, moist
5'0"	Refusal, probable bedrock	

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TP-13 (Elevation +312.0)

0'0" – 0'8"	Brown topsoil	
0'8" – 4'5"	Brown coarse to fine SAND, some (+) Silt, some (-) coarse to fine Gravel, with cobbles and boulders	medium dense, moist
4'5" – 6'0"	Brown coarse to fine SAND, little Silt, some coarse to fine Gravel, with many cobbles and boulders	medium dense, moist
6'0"	Refusal on boulders	

TP-14 (Elevation +346.0)

0'0" – 0'7"	Brown topsoil	
0'7" – 5'6"	Brown coarse to fine SILT and (+), coarse to fine Sand, little coarse to fine Gravel, with cobbles and boulders	medium dense, moist
5'6"	Refusal, bedrock	
	No Water Encountered	

TP-15 (Elevation +336.0)

0'0" – 0'10"	Brown topsoil	
0'10" – 5'5"	Orange brown coarse to fine SAND, and Silt, little coarse to fine Gravel, with occasional cobbles and boulders	medium dense, moist
5'5" – 6'6"	Gray brown coarse to fine SAND, little Silt, with Schist completely weathered	
6'6"	Refusal, bedrock	

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TP-16 (Elevation +330.0)

0'0" – 0'8"	Topsoil	
0'8" – 4'4"	Brown coarse to fine SILT and (+), coarse to fine Sand, little coarse to fine Gravel, with cobbles and boulders	medium dense, moist
4'4" – 8'6"	Brown coarse to fine SAND, little (+) Silt, some (+) coarse to fine Gravel, with cobbles and boulders	medium dense, moist
	No Water Encountered	

TP-17 (Elevation +343.0)

0'0" – 2'0"	FILL (Brown topsoil with boulder)	loose, moist
2'0" – 4'1"	Light brown coarse to fine SAND, and Silt, some coarse to fine Gravel, with cobbles and boulders	medium dense, moist
4'1" – 7'0"	Brown coarse to fine SAND, some (+) Silt, little coarse to fine Gravel	medium dense, moist
7'0"	Refusal @ boulder	
	Groundwater @ 7'0" slow inflow	

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-1	
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1	
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40	
Drilling Contractor: General Borings Inc.									ELEVATION: +342.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Apr/18
4/3/2018		10:00	13'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Apr/18
					WGHT		140#			DRILLER: J. Wyant
					FALL		30"			INSPECTOR: CKS
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS
1		S-1	4 50/1"		<u>Topsoil</u>					0'7" Rec = 3" moist
2										
3										
4										
5		S-2	16 25		Br cf S, s \$, s cf G					Rec = 18" moist
6			18 23							
7		S-3	31 50/3"		same, w/weathered rock					Rec = 10" moist
8										
9					<u>Brown coarse to fine SAND,</u> <u>some Silt, some coarse to fine</u> <u>Gravel, with cobbles, boulders,</u>					many cobbles/boulders
10										
11		S-4	21 31		same, l (+) \$					Rec = 16" moist
12			29 32							
13										13'0"
14					<u>Gneiss, Completely Weathered</u>					
15										15'0" spoon refusal @ 15'0"
16			50/0"		<u>End of Boring @ 15'0"</u>					
17										
18										
19										
20										
21										
22										

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-2		
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.									ELEVATION: +325.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Apr/18	
3/Apr/18		1045	4'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Apr/18	
					WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: CKS	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	3 3		<u>Topsoil</u>					1'0"	Rec = 14" moist
2			3 3		<u>FILL (Brown coarse to fine SAND, little (+) Silt, little coarse to fine Gravel)</u>					2'0"	
3		S-2	5 7		Br \$ a, cf S, l cf G						Rec = 10" moist
4			9 10								
5					<u>Brown SILT and, coarse to fine Sand, little coarse to fine Gravel</u>						Rec = 0" wet
6		S-3	7 7	same							
7			7								
8		S-4	5 6		same						Rec = 1" wet
9			16 22								
10											
11		S-5	50/5"								
12					Gr cf S, s (-) \$, l cf G					11'0"	Rec = 4" wet Auger refusal @ 11'0"
13					<u>End of Boring @ 11'0"</u>						
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-3		
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.									ELEVATION: +335.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Apr/18	
3/Apr/18		1145	8'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Apr/18	
					WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: CKS	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	2		<u>Topsoil</u>					Rec = 10" moist Rec = 14" moist	
			5		1'0"						
2			3		<u>FILL (Brown coarse to fine SAND, little (+) Silt, trace medium to fine Gravel)</u>						2'0"
3		S-2	7		Br cf S, s (-) \$, l (+) cf G					Rec = 17" moist	
			18								
			13								
4		S-3	22		<u>Brown coarse to fine SAND, some (-) Silt, little (+) coarse to fine Gravel</u>						Rec = 15" moist to wet
5											
6		S-4	11		same, s (+) \$					Dense drilling 12'-15'	
			15								
			12								
7		S-5	9		Br cf S, l (-) \$, a cf G						Rec = 4" wet Auger refusal 16'0" Probable bedrock
			6								
8			5								
9		S-6	5		<u>Brown coarse to fine SAND, little (-) Silt, and coarse to fine Gravel</u>					16'0"	
10											
11		S-5	8		same						10'6"
			16								
12			17								
13		S-6	16		<u>Brown coarse to fine SAND, and Silt, little (+) coarse to fine Gravel</u>					14'0"	
14											
15		S-6			<u>Gneiss, Completely Weathered</u>						16'0"
16			50/5"								
17		S-6			same, w/weathered rock					16'0"	
18											
19											
20		S-6			<u>End of Boring @ 16'0"</u>						16'0"
21											
22											

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-4					
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1					
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40					
Drilling Contractor: General Borings Inc.									ELEVATION: +298.0					
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo					
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Apr/18				
3/Apr/18		1300	10'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Apr/18				
					WGHT		140#			DRILLER: J. Wyant				
					FALL		30"			INSPECTOR: CKS				
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS				
1		S-1	3		<u>Topsoil</u>					Rec = 10" moist				
			5		1'0"									
2		S-2	9		<u>FILL (Brown coarse to fine SAND, some (-) Silt, little (+) coarse to fine Gravel)</u>					Rec = 8" moist Auger refusal @ 3'6" moved hole 5' east continued hole				
			10		2'0"									
3		S-2	14		Br cf S, l (+) \$, a cf G, w/weathered rock					Rec = 8" moist Auger refusal @ 3'6" moved hole 5' east continued hole				
			40											
4		50/5"												
5														
6														
6		S-3	30		same					Rec = 14" moist				
			49											
7		48												
		43												
8														
9		S-4			<u>Brown coarse to fine SAND, little (+) Silt, and coarse to fine Gravel, with weathered rock</u>					many cobbles and boulders				
10														
11			50/2"		same	<u>Gneiss, Completely Weathered</u>						Rec = 2" wet		
12														
13														
14														
15														
16					14'0"					Auger refusal @ 14'0"				
17					<u>End of Boring @ 14'0"</u>									
18														
19														
20														
21														
22														

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-5	
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1	
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40	
Drilling Contractor: General Borings Inc.									ELEVATION: +340.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE	TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 3/Apr/18	
3/Apr/18	1515	5'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 3/Apr/18	
				WGHT		140#			DRILLER: J. Wyant	
				FALL		30"			INSPECTOR: CKS	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION				REMARKS	
1		S-1	2		<u>Topsoil</u>				Rec = 4" moist	
			5							
			5						1'6"	
2		S-2	2		<u>FILL (Brown coarse to fine SAND, some (-) Silt, little (+) coarse to fine Gravel)</u>				2'6"	
			6		Br \$ a, cf S, l cf G				Rec = 16" moist	
3			8							
			16							
4			16						many cobbles and boulders	
5										
		S-3	34		<u>Brown SILT and, coarse to fine Sand, little coarse to fine Gravel</u>				Rec = 10" wet	
6			28							
			50/3"							
7										
8										
9									9'0"	
10		S-4			Completely weathered rock				Dense drilling 9'-10'	
			50/1"							
11										
12										
13					<u>Gneiss, Completely weathered</u>				Rippable?	
14										
15										
		S-5	50/1"	same	15'0"				Rec = 0" Spoon refusal @ 15'1"	
16										
17										
18										
19										
20										
21										
22										

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-6		
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.									ELEVATION: +360.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18	
No Water Encountered					DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18	
					WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: MW	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	2		<u>Topsoil</u>					Rec = 18" moist	
			1		1'0"						
2			4		FILL (Br cf S, s (+) \$, t (+) f G)					Rec = 16" moist	
			5		<u>FILL (Brown coarse to fine SAND, some (+) Silt, trace (+) fine Gravel)</u>						
3		S-2	8		2'6"					Rec = 16" moist	
			21		Gr cf S, l (+) \$, l mf G						
4			32								
			29								
5										Rec = 16" moist Dense drilling 7' to 10'	
6		S-3	18								
			23	same, gr	<u>Gray coarse to fine SAND, little (+) Silt, little medium to fine Gravel</u>						
7			24								
8		S-4	23							Rec = 12" moist	
			27	same, gr, lt br l (+) mf G							
9			29							Rec = 1" moist Dense drilling 12' to 14'	
			28								
10					10'0"						
		S-5	50/2"		same, gr w/weathered rock						
11										Auger refusal @ 14'6"	
12											
13					<u>Gneiss, Completely Weathered</u>						
14											
15					14'6"					<u>End of Boring @ 14'6"</u>	
16											
17											
18											
19											
20											
21											
22											

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ					TEST BORING LOG					BORING NUMBER B-7		
Project: Proposed Development, Elmwood Country Club, White Plains NY										SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners										JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.										ELEVATION: +374.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo			
DATE	TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18			
No Water Encountered					DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18		
				WGHT		140#			DRILLER: J. Wyant			
				FALL		30"			INSPECTOR: MW			
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	S y m	IDENTIFICATION				REMARKS			
1		S-1	3		<u>Topsoil</u> 0'6"				Rec = 6" moist bouncy spoon			
2			41		Gr cf S, l (-) \$, l mf G, completely weathered rock frag <u>Gray coarse to fine SAND, little (-) Silt, little medium to fine Gravel</u> 2'0"							
3			50/3"									
4		S-2	50/4"		Completely weathered rock				Rec = 2" moist Dense drilling from 2' to 5'			
5												
6		S-3	50/3"		same <u>Completely Weathered Rock</u>				Rec = 2" moist Dense drilling from 5' to 10'			
7												
8									Rippable?			
9												
10												
11		S-4	50/2"		same 10'2"				Rec = 1" moist			
12					<u>End of Boring @ 10'2"</u>							
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												

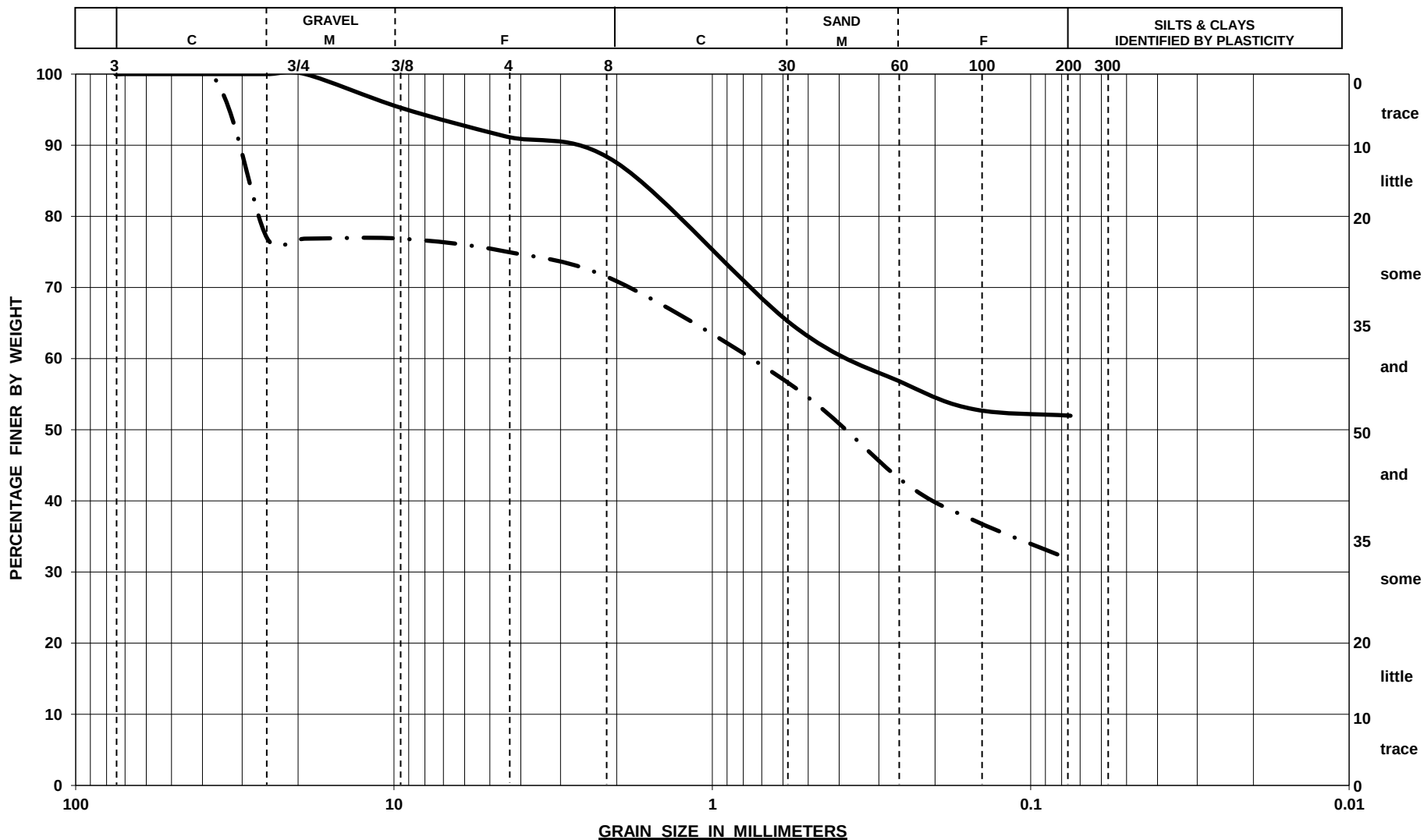
CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ					TEST BORING LOG					BORING NUMBER B-8	
Project: Proposed Development, Elmwood Country Club, White Plains NY										SHEET NO.: 1 of 1	
Client: Ridgewood Real Estate Partners										JOB NUMBER: 18-40	
Drilling Contractor: General Borings Inc.										ELEVATION: +375.0	
GROUNDWATER						CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18	
No Water Encountered					DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18	
					WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: MW	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	2		<u>Topsoil</u>					Rec = 16" moist	
			8								
2			9		<u>FILL (Brown coarse to fine SAND, some Silt, little coarse to fine Gravel)</u>					2'0"	
			6								
3		S-2	8		<u>Brown coarse to fine SAND, little (-) Silt, little medium to fine Gravel</u>					3'0"	
			13								
4			50/5"		Gr completely weathered rock					Rec = 10" moist	
					<u>Completely Weathered Rock</u>					Dense drilling 3' to 5'	
5											
6		S-3	50/2"		same					5'2"	
					<u>End of Boring @ 5'2"</u>					Rec = 1" moist	
7										Auger refusal @ 5'2"	
8										Moved hole 4' South	
9										Dense drilling 3' to 4'6"	
10										Auger refusal @ 5'2"	
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-9	
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1	
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40	
Drilling Contractor: General Borings Inc.									ELEVATION: +341.0	
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo	
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18
No Water Encountered					DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18
					WGHT		140#			DRILLER: J. Wyant
					FALL		30"			INSPECTOR: MW
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS
1		S-1	2		<u>Topsoil</u>					Rec = 12" moist
			2		1'0" FILL (Dk br cf S, s \$, l (-) mf G)					
2			2		FILL (same, a (-) \$) <u>FILL (Dark brown coarse to fine SAND, some Silt, little (-) medium to fine Gravel)</u>					Rec = 12" moist
			2							
3		S-2	1		FILL (same, a (-) \$) <u>FILL (Dark brown coarse to fine SAND, some Silt, little (-) medium to fine Gravel)</u>					Rec = 12" moist
			2							
4			1							
5										
6		S-3	4		FILL (same) 6'0"					Rec = 16" moist
			9		<u>Gray brown coarse to fine SAND, and Silt, trace (-) fine Gravel</u> 7'0"					
7			9							
8		S-4	50		Gr completely weathered rock					Rec = 12" moist
			40/3"							
9					<u>Completely weathered rock</u>					
10										
11		S-5	50/3"		same 11'0"					Rec = 1" moist
12					<u>End of Boring @ 11'0"</u>					Auger Refusal @ 11'0" Probable bedrock
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										

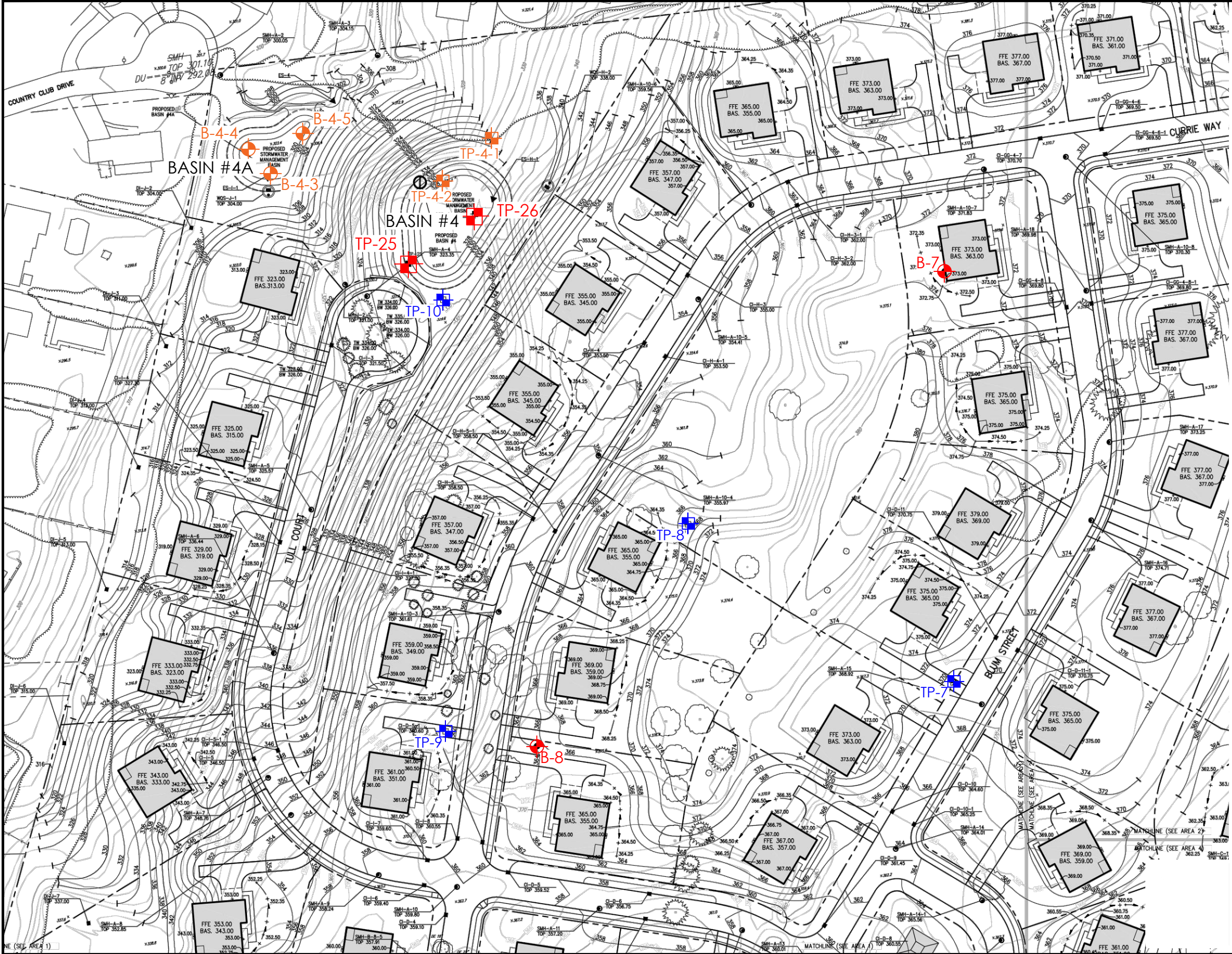
CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-10		
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.									ELEVATION: +339.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18	
4/Apr/18		1400	10'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18	
4/Apr/18		1430	9'0"	HSA	WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: MW	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	2		<u>Topsoil</u>					0'8"	Rec = 12" moist
			4								
2			4		<u>FILL (Brown coarse to fine SAND, some Silt, little medium to fine Gravel)</u>						
		6									
3		S-2	8		Br, gr cf S, s (-) \$, l (-) mf G					Rec = 20" moist	
			14								
			20								
4			23								
5											
6		S-3	16		<u>Brown, gray coarse to fine SAND, some (-) Silt, little (-) medium to fine Gravel, with weathered rock</u>					Rec = 12" moist	
			20	same							
			50/3"								
7											
8											
9		S-4	5							Rec = 16" wet	
			10	same							
			12								
10			13								
11		S-5	13		10'6"					Rec = 10" wet	
			32	same, l \$, w/weathered rock frag							
			50/3"								
12					<u>Gneiss, Completely Weathered</u>						
13											
		S-6								13'1"	Rec = 0 wet
			50/1"	same							
14					<u>End of Boring @ 13'1"</u>						
15											
16											
17											
18											
19											
20											
21											
22											

CARLIN - SIMPSON & ASSOCIATES Sayreville, NJ				TEST BORING LOG					BORING NUMBER B-11		
Project: Proposed Development, Elmwood Country Club, White Plains NY									SHEET NO.: 1 of 1		
Client: Ridgewood Real Estate Partners									JOB NUMBER: 18-40		
Drilling Contractor: General Borings Inc.									ELEVATION: +331.0		
GROUNDWATER					CASING	SAMPLE	CORE	TUBE	DATUM: Topo		
DATE		TIME	DEPTH	CASING	TYPE	HSA	SS			START DATE: 4/Apr/18	
4/Apr/18		1515	6'0"	HSA	DIA.	3 1/4"	1 3/8"			FINISH DATE: 4/Apr/18	
4/Apr/18		1535	6'0"	HSA	WGHT		140#			DRILLER: J. Wyant	
					FALL		30"			INSPECTOR: MW	
Depth (ft.)	Casing Blows per Foot	Sample Number	Blows on Sample Spoon per 6"	Sym	IDENTIFICATION					REMARKS	
1		S-1	3		<u>Topsoil</u>					Rec = 10" moist	
			2		0'8"						
2			3		FILL (Br cf S, s (+) \$, l (-) mf G)						
			9		<u>FILL (Brown coarse to fine SAND, some (+) Silt, little (-) medium to fine Gravel)</u>					Rec = 10" moist	
3		S-2	8		FILL (same)						
			11		3'0"						
4			9		Br cf S, s (+) \$, l mf G					Boulder @ 5' to 6'	
			13		<u>Brown coarse to fine SAND, some (+) Silt, little medium to fine Gravel</u>						
5					5'0"						
			20							Rec = 8" wet	
6		S-3	11		Br, gr cf S, l \$, s (-) mf G						
			6								
7			5							Rec = 12" wet	
			15								
8		S-4	23		same, l (+) mf G						
			22		<u>Brown, gray coarse to fine SAND, little Silt, some (-) medium to fine Gravel</u>					Rec = 14" wet	
9			31								
10											
			22							Rec = 14" wet	
11		S-5	42		same, l mf G						
			30								
12			50/2"		11'8"						
13					<u>End of Boring @ 11'8"</u>						
14											
15											
16											
17											
18											
19											
20											
21											
22											

SIEVE ANALYSIS



SYMBOL	TEST PIT	SAMPLE	DEPTH	DESCRIPTION	NAT MC
—	TP-7	S-1	0'7" - 6'9"	Brown coarse to fine SILT and (+), coarse to fine Sand, little coarse to fine Gravel, with cobbles and boulders	7.3%
— .	TP-13	S-1	0'8" - 4'5"	Brown coarse to fine Sand, and (-) Silt, some coarse to fine Gravel	8.6%



GENERAL NOTES:

1. GENERAL LAYOUT WAS OBTAINED FROM A DRAWING THAT WAS PREPARED BY JMC, PLLC, ENTITLED "PRELIMINARY GRADING PLAN AREA 1", DATED 11.11.2022.
2. BORING AND TEST PIT LOCATIONS WERE LAID OUT IN THE FIELD BY CARLIN-SIMPSON & ASSOCIATES (CSA).
3. THE BORINGS WERE PERFORMED BY GENERAL BORINGS INC. IN APRIL 2018 AND BORINGS WERE PERFORMED BY ENVIRONMENTAL TECHNICAL DRILLING IN DECEMBER 2022. THE TEST PITS WERE PERFORMED BY KINGS CAPITAL GROUP IN JULY 2018, TEST PITS WERE COMPLETED BY LAWRENCE CONSTRUCTION IN MAY 2021 AND TEST PITS WERE PERFORMED BY TRAFICANTE CONSTRUCTION IN FEBRUARY 2023. ALL UNDER THE FULL TIME INSPECTION OF CSA.
4. LOCATIONS ARE APPROXIMATE.

LEGEND:

- BORING LOCATION PERFORMED IN 2018
- TEST PIT LOCATION PERFORMED IN 2018
- TEST PIT LOCATION PERFORMED IN 2021
- BORING LOCATION PERFORMED IN 2022
- TEST PIT LOCATION PERFORMED IN 2023



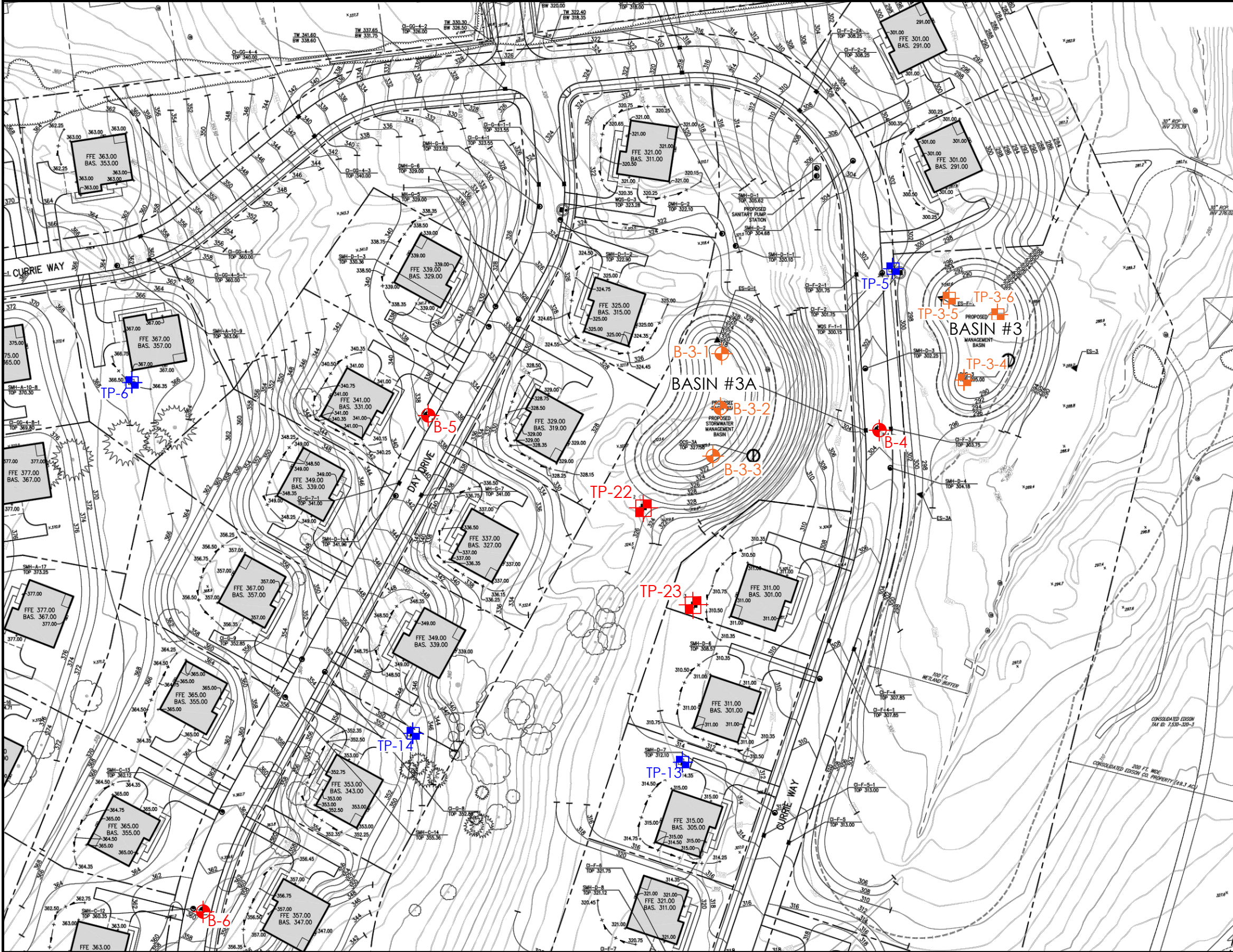
ROBERT B. SIMPSON, P.E.
PROFESSIONAL ENGINEER

BORING & TEST PIT LOCATION PLAN

PROPOSED RESIDENTIAL DEVELOPMENT
ELMWOOD COUNTRY CLUB
850 DOBBS FERRY ROAD
WHITE PLAINS, NEW YORK

DRAWN	CKS	SCALE	1" = 100'	CARLIN-SIMPSON AND ASSOCIATES, LLC 61 Main Street Sayreville, NJ 08872 Consulting Geotechnical and Environmental Engineers
CHECKED	RBS	DATE	03.24.2023	
PROJECT NO.	18-40	DWG NO.	FIG -1	
APPROVED				



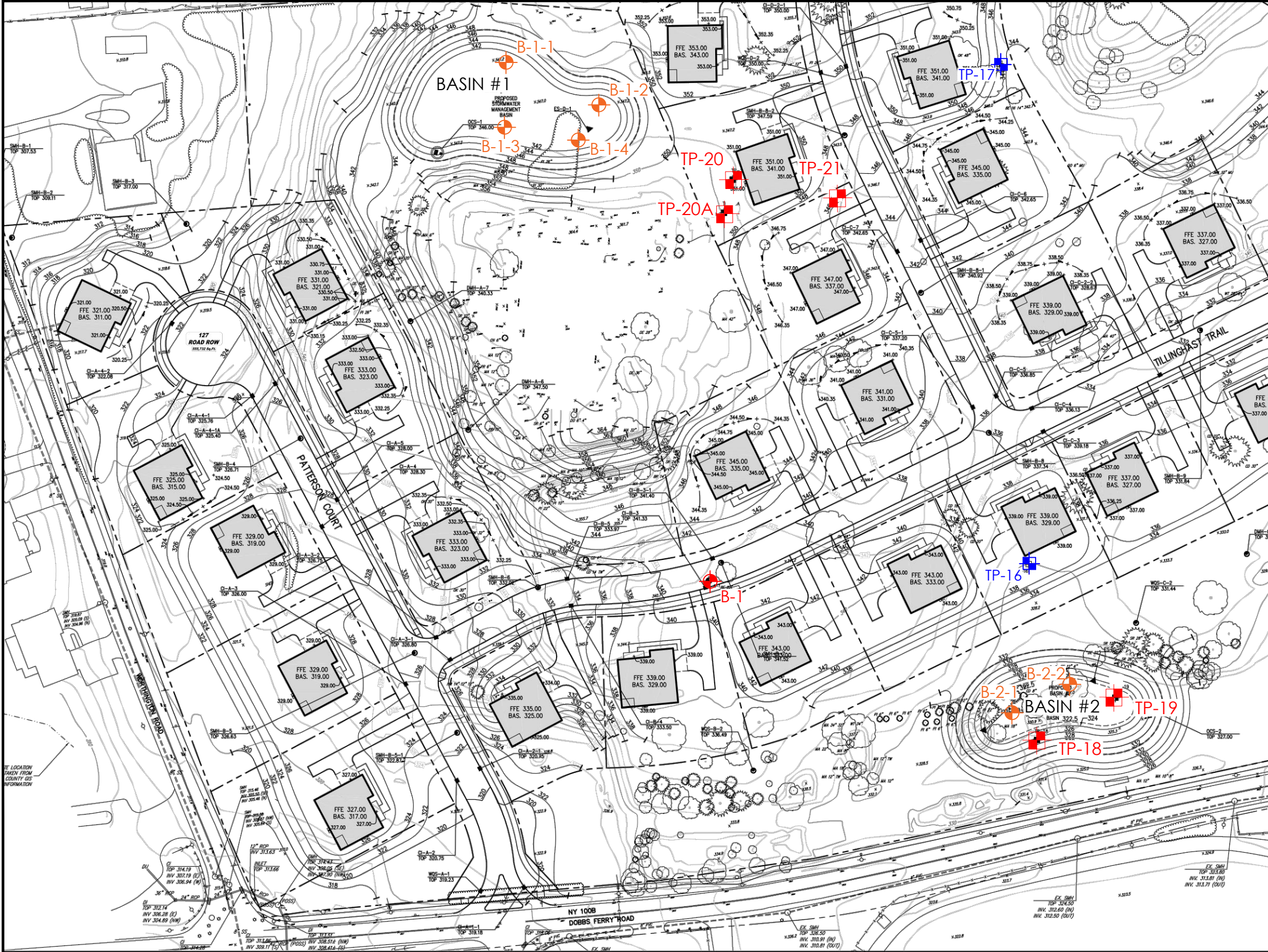


- GENERAL NOTES:
1. GENERAL LAYOUT WAS OBTAINED FROM A DRAWING THAT WAS PREPARED BY JMC, PLLC, ENTITLED "PRELIMINARY GRADING PLAN AREA 2", DATED 11.11.2022.
 2. BORING AND TEST PIT LOCATIONS WERE LAID OUT IN THE FIELD BY CARLIN-SIMPSON & ASSOCIATES (CSA).
 3. THE BORINGS WERE PERFORMED BY GENERAL BORINGS INC. IN APRIL 2018 AND BORINGS WERE PERFORMED BY ENVIRONMENTAL TECHNICAL DRILLING IN DECEMBER 2022. THE TEST PITS WERE PERFORMED BY KINGS CAPITAL GROUP IN JULY 2018, TEST PITS WERE COMPLETED BY LAWRENCE CONSTRUCTION IN MAY 2021 AND TEST PITS WERE PERFORMED BY TRAFICANTE CONSTRUCTION IN FEBRUARY 2023. ALL UNDER THE FULL TIME INSPECTION OF CSA.
 4. LOCATIONS ARE APPROXIMATE.

- LEGEND:
- BORING LOCATION PERFORMED IN 2018
 - TEST PIT LOCATION PERFORMED IN 2018
 - TEST PIT LOCATION PERFORMED IN 2021
 - BORING LOCATION PERFORMED IN 2022
 - TEST PIT LOCATION PERFORMED IN 2023



ROBERT B. SIMPSON, P.E. PROFESSIONAL ENGINEER		
BORING & TEST PIT LOCATION PLAN		
PROPOSED RESIDENTIAL DEVELOPMENT ELMWOOD COUNTRY CLUB 850 DOBBS FERRY ROAD WHITE PLAINS, NEW YORK		
DRAWN CKS	SCALE 1" = 100'	CARLIN-SIMPSON AND ASSOCIATES, LLC 61 Main Street Sayreville, NJ 08872 Consulting Geotechnical and Environmental Engineers 
CHECKED RBS	DATE 03.24.2023	
PROJECT NO. 18-40	DWG NO. FIG - 2	
APPROVED		



GENERAL NOTES:

1. GENERAL LAYOUT WAS OBTAINED FROM A DRAWING THAT WAS PREPARED BY JMC, PLLC, ENTITLED "PRELIMINARY GRADING PLAN AREA 3", DATED 11.11.2022.
2. BORING AND TEST PIT LOCATIONS WERE LAID OUT IN THE FIELD BY CARLIN-SIMPSON & ASSOCIATES (CSA).
3. THE BORINGS WERE PERFORMED BY GENERAL BORINGS INC. IN APRIL 2018 AND BORINGS WERE PERFORMED BY ENVIRONMENTAL TECHNICAL DRILLING IN DECEMBER 2022. THE TEST PITS WERE PERFORMED BY KINGS CAPITAL GROUP IN JULY 2018, TEST PITS WERE COMPLETED BY LAWRENCE CONSTRUCTION IN MAY 2021 AND TEST PITS WERE PERFORMED BY TRAFICANTE CONSTRUCTION IN FEBRUARY 2023. ALL UNDER THE FULL TIME INSPECTION OF CSA.
4. LOCATIONS ARE APPROXIMATE.

LEGEND:

- BORING LOCATION PERFORMED IN 2018
- TEST PIT LOCATION PERFORMED IN 2018
- TEST PIT LOCATION PERFORMED IN 2021
- BORING LOCATION PERFORMED IN 2022
- TEST PIT LOCATION PERFORMED IN 2023



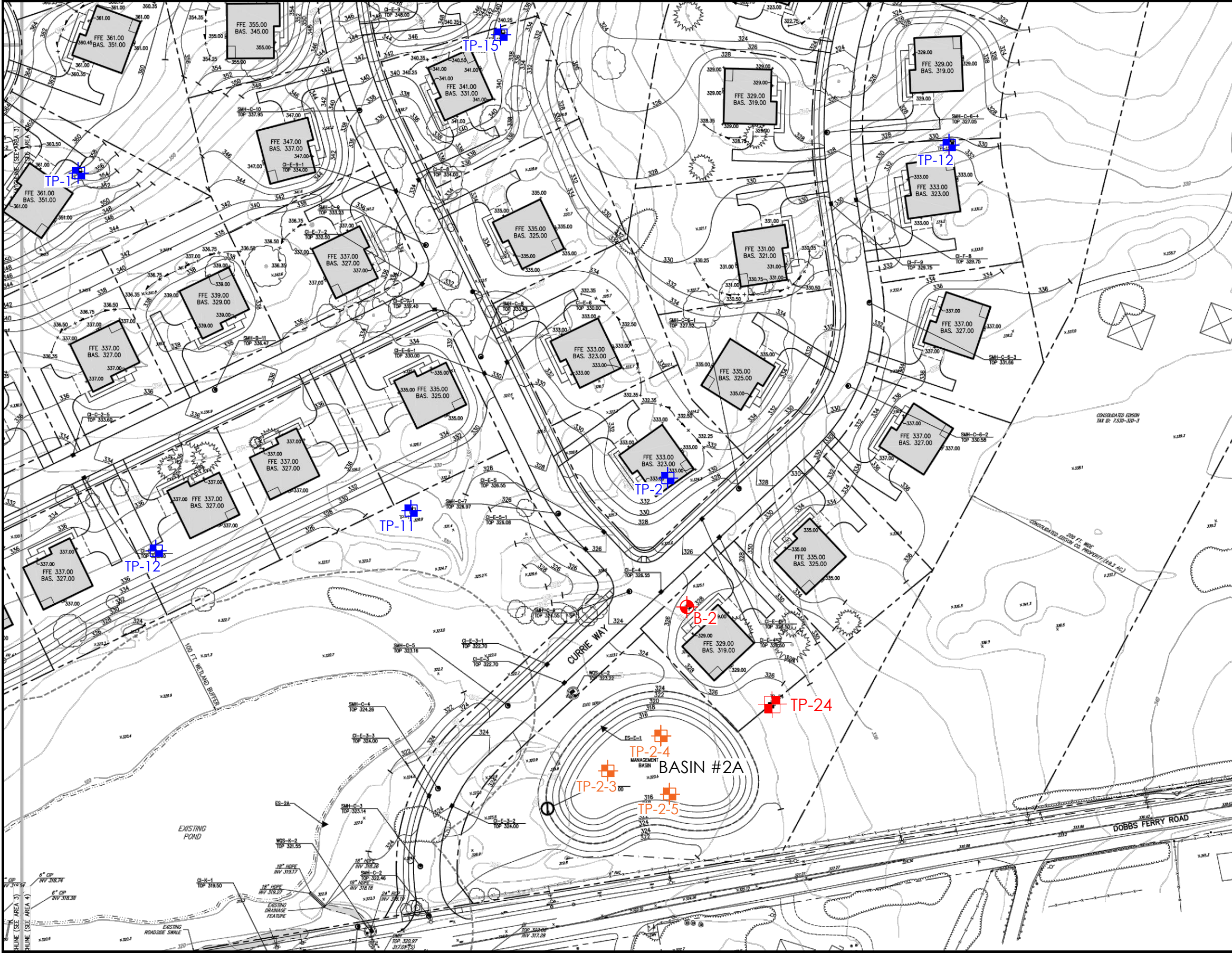
ROBERT B. SIMPSON, P.E.
PROFESSIONAL ENGINEER

BORING & TEST PIT LOCATION PLAN

PROPOSED RESIDENTIAL DEVELOPMENT
ELMWOOD COUNTRY CLUB
850 DOBBS FERRY ROAD
WHITE PLAINS, NEW YORK

DRAWN	CKS	SCALE	1" = 100'	CARLIN-SIMPSON AND ASSOCIATES, LLC 61 Main Street Sayreville, NJ 08872 Consulting Geotechnical and Environmental Engineers
CHECKED	RBS	DATE	03.24.2023	
PROJECT NO.	18-40	DWG NO.	FIG - 3	
APPROVED				





- GENERAL NOTES:
1. GENERAL LAYOUT WAS OBTAINED FROM A DRAWING THAT WAS PREPARED BY JMC, PLLC, ENTITLED "PRELIMINARY GRADING PLAN AREA 4", DATED 11.11.2022.
 2. BORING AND TEST PIT LOCATIONS WERE LAID OUT IN THE FIELD BY CARLIN-SIMPSON & ASSOCIATES (CSA).
 3. THE BORINGS WERE PERFORMED BY GENERAL BORINGS INC. IN APRIL 2018 AND BORINGS WERE PERFORMED BY ENVIRONMENTAL TECHNICAL DRILLING IN DECEMBER 2022. THE TEST PITS WERE PERFORMED BY KINGS CAPITAL GROUP IN JULY 2018, TEST PITS WERE COMPLETED BY LAWRENCE CONSTRUCTION IN MAY 2021 AND TEST PITS WERE PERFORMED BY TRAFICANTE CONSTRUCTION IN FEBRUARY 2023. ALL UNDER THE FULL TIME INSPECTION OF CSA.
 4. LOCATIONS ARE APPROXIMATE.

- LEGEND:
- BORING LOCATION PERFORMED IN 2018
 - TEST PIT LOCATION PERFORMED IN 2018
 - TEST PIT LOCATION PERFORMED IN 2021
 - BORING LOCATION PERFORMED IN 2022
 - TEST PIT LOCATION PERFORMED IN 2023



ROBERT B. SIMPSON, P.E. PROFESSIONAL ENGINEER		
BORING & TEST PIT LOCATION PLAN		
PROPOSED RESIDENTIAL DEVELOPMENT ELMWOOD COUNTRY CLUB 850 DOBBS FERRY ROAD WHITE PLAINS, NEW YORK		
DRAWN CKS	SCALE 1" = 100'	CARLIN-SIMPSON AND ASSOCIATES, LLC 61 Main Street Sayreville, NJ 08872 Consulting Geotechnical and Environmental Engineers
CHECKED RBS	DATE 03.24.2023	
PROJECT NO. 18-40	DWG NO. FIG - 4	
APPROVED		



DESIGN DATA SHEET - STORMWATER INFILTRATION SYSTEM

JOB NO.17099

OwnerRidgewood Elmwood Owner, LLCAddress25A Hanover Road, Suite 310

Located at (Street)850 Dobbs Ferry RoadSec. 7530 Block 320 Lot 1
(Indicate nearest cross st.)

MunicipalityGreenburghWatershedPocantico and Saw Mill River Basin

SOIL INFILTRATION TEST DATA

Presoak Date:4/25/2023

Run Date:4/25/2023

Hole #	CLOCK TIME				INFILTRATION			
Hole Number	Run No.	Start	Stop	Elapse Time Min.	Depth From Grd	To surface water	Water Level Drop In Inches	Soil Rate In/Hr Drop
1 (Closest to Pond)	1	10:00 AM	10:59 AM	59	3 feet	1 feet	24	24.00
	2	11:00 AM	11:56 AM	56	3 feet	1 feet	24	25.00
	3	12:09 PM	1:02 PM	53	3 feet	1 feet	24	27.00
	4	1:05 PM	2:05 PM	60	3 feet	1 feet	14	14.00
2 (Middle)	1	10:08 AM	10:40 AM	32	3 feet	1 feet	24	45.00
	2	11:02 AM	11:54 AM	52	3 feet	1 feet	24	27.00
	3	12:07 PM	1:07 PM	60	3 feet	1 feet	24	24.00
	4	1:10 PM	2:10 PM	60	3 feet	1 feet	20	20.00
3	1	10:06 AM	10:58 AM	52	3 feet	1 feet	24	27.00
	2	11:04 AM	12:04 PM	60	3 feet	1 feet	19	19.00
	3	12:05 PM	1:05 PM	60	3 feet	1 feet	11	11.00
	4	1:08 PM	2:08 PM	60	3 feet	1 feet	11	11.00

Notes:Perc test done by:EL

1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each infiltration test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS THAN ONE INCH.

WAS GROUNDWATER ENCOUNTERED? No _____
INDICATE LEVEL AT WHICH GROUND WATER IS ENCOUNTERED N/A _____
INDICATE LEVEL AT WHICH WATER RISES AFTER BEING ENCOUNTERED N/A _____
DEEP TESTS MADE BY N/A _____ DATE OF DEEP TESTS N/A _____

DESIGN
Soil Rate Used: _____ Min/1" Drop: _____

Name _____ Signature _____
Address JMC, PLLC SEAL
 120 Bedford Road
 Armonk, NY 10504

P:\2017\17099\ARCHIVE\DRAINAGE\2023-04-25 PERC TESTS - EL\PERC TEST STORM - EL.xlsx



20230425_085839



20230425_085843



20230425_085850



20230425_085853



20230425_085856



20230425_085859



20230425_085903



20230425_085906



20230425_085908



20230425_085919



20230425_085950



20230425_085957



20230425_085958



20230425_090013



20230425_090014



20230425_141258



20230425_141312



20230425_141313



20230425_141314



20230425_141315



20230425_141317



20230425_090115



20230425_090117



20230425_090118



20230425_090119



20230425_090125



20230425_090126



20230425_141210



20230425_141212



20230425_141213



20230425_141218



20230425_141221

DESIGN DATA SHEET - STORMWATER INFILTRATION SYSTEM

JOB NO. 17099

Owner

Ridgewood Elmwood Owner, LLC

Address

25A Hanover Road, Suite 310

Located at (Street)

850 Dobbs Ferry Road

Sec. 7530 Block 320 Lot 1

(Indicate nearest cross st.)

Municipality

Greenburgh

Watershed

Pocantico and Saw Mill River Basin

SOIL INFILTRATION TEST DATA

Presoak Date:

4/25/2023

Run Date:

4/25/2023

Hole #	CLOCK TIME				INFILTRATION			
Hole Number	Run No.	Start	Stop	Elapse Time Min.	Depth From Grd	To surface water	Water Level Drop In Inches	Soil Rate In/Hr Drop
4	1	11:27 AM	11:57 AM	30	4 feet	2 feet	12	24.00
	2	12:05 PM	12:50 PM	45	4 feet	2 feet	14	18.00
	3	12:52 PM	1:22 PM	30	4 feet	2 feet	9	18.00
5	1	12:01 PM	12:03 PM	2	4 feet	2 feet	24	720.00
	2	12:03 PM	12:05 PM	2	4 feet	2 feet	24	720.00
6	1	12:56 PM	1:26 PM	30	4 feet	2 feet	8	16.00
	2	1:27 PM	1:57 PM	30	4 feet	2 feet	9	18.00
	3	1:57 PM	2:27 PM	30	4 feet	2 feet	9	18.00

Notes:

Perc test done by: RB

1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each infiltration test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS THAN ONE INCH.

WAS GROUNDWATER ENCOUNTERED? No _____
INDICATE LEVEL AT WHICH GROUND WATER IS ENCOUNTERED N/A _____
INDICATE LEVEL AT WHICH WATER RISES AFTER BEING ENCOUNTERED N/A _____
DEEP TESTS MADE BY N/A _____ DATE OF DEEP TESTS N/A _____

DESIGN
Soil Rate Used: _____ Min/1" Drop: _____

Name _____ Signature _____

Address JMC, PLLC SEAL
 120 Bedford Road
 Armonk, NY 10504

P:\2017\17099\ARCHIVE\DRAINAGE\2023-04-25 PERC TESTS - EL\PERC TEST STORM - RB.xlsx



20230425_084408



20230425_112335



20230425_112337



20230425_112338



20230425_112340



20230425_112343



20230425_112344



20230425_112349



20230425_084435



20230425_121008



20230425_121011



20230425_121015



20230425_121026



20230425_121406



20230425_121008 (1)



20230425_084331



20230425_084337



20230425_084347



20230425_125304



20230425_125305



20230425_125307



20230425_125312



20230425_125315



20230425_125322



20230425_125327

JOB NO. 17099

Municipality Greenburgh **Watershed** Pocantico and Saw Mill River Basin

Presoak Date: 5/23/2023 Run Date: 5/23/2023

Notes: Perc test done by: FG

- 1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each infiltration test hole. All data to be submitted for review.
- 2) Depth measurements to be made from top of hole. DO NOT REPORT

INCREMENTS OF LESS THAN ONE INCH.

WAS GROUNDWATER ENCOUNTERED? No
 INDICATE LEVEL AT WHICH GROUND WATER IS ENCOUNTERED N/A
 INDICATE LEVEL AT WHICH WATER RISES AFTER BEING ENCOUNTERED N/A
 DEEP TESTS MADE BY N/A DATE OF DEEP TESTS N/A

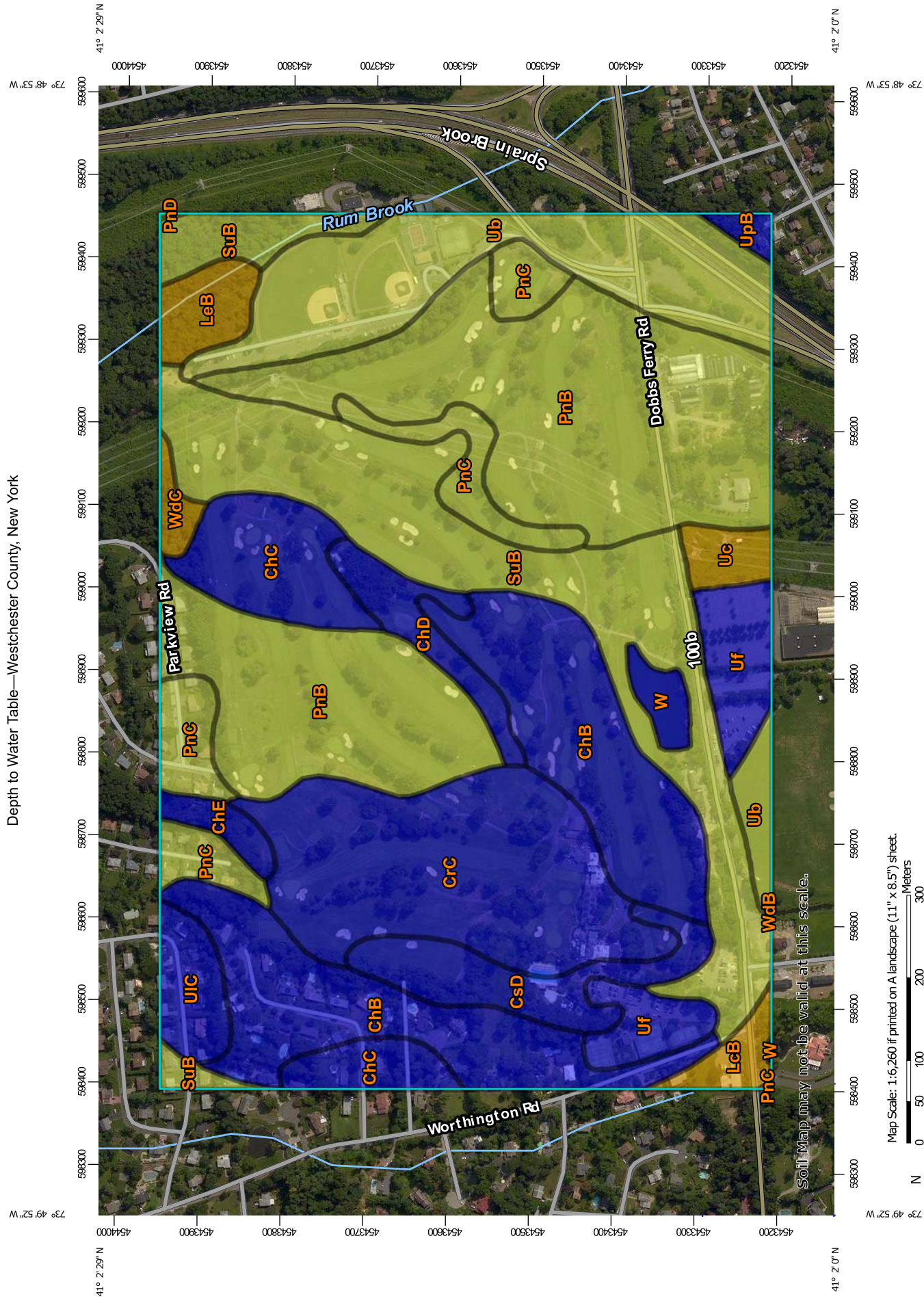
DESIGN

Soil Rate Used: _____ Min/1" Drop:

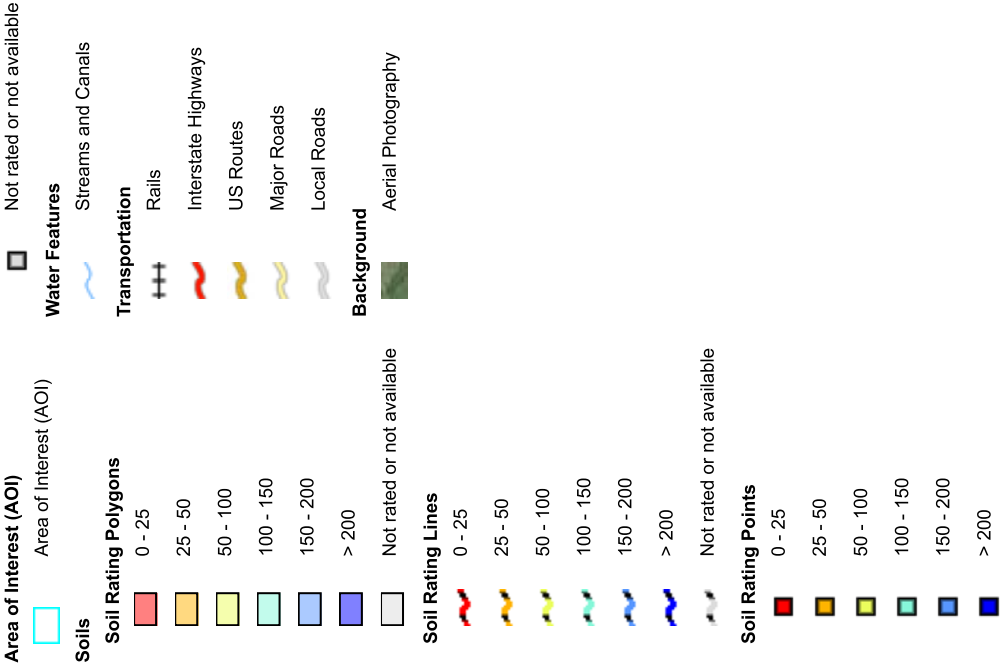
Name _____ Signature _____

Address	JMC, PLLC	SEAL
	120 Bedford Road	
	Armonk, NY 10504	

P:\2017\17099\ARCHIVE\DRAINAGE\2023-04-25 PERC TESTS - EL\PERC TEST STORM -FG.xlsx



MAP LEGEND



MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 13, Oct 8, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 21, 2014—Aug 27, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Depth to Water Table

Map unit symbol	Map unit name	Rating (centimeters)	Acres in AOI	Percent of AOI
ChB	Charlton fine sandy loam, 3 to 8 percent slopes	>200	27.3	14.0%
ChC	Charlton fine sandy loam, 8 to 15 percent slopes	>200	9.2	4.7%
ChD	Charlton fine sandy loam, 15 to 25 percent slopes	>200	3.7	1.9%
ChE	Charlton loam, 25 to 35 percent slopes	>200	1.5	0.8%
CrC	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	>200	21.8	11.2%
CsD	Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky	>200	6.4	3.3%
LcB	Leicester loam, 3 to 8 percent slopes, stony	30	1.8	0.9%
LeB	Leicester loam, 2 to 8 percent slopes, very stony	30	2.8	1.5%
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	61	46.1	23.7%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	61	9.3	4.8%
PnD	Paxton fine sandy loam, 15 to 25 percent slopes	61	0.0	0.0%
SuB	Sutton loam, 3 to 8 percent slopes	61	30.7	15.8%
Ub	Udorthents, smoothed	84	19.0	9.8%
Uc	Udorthents, wet substratum	38	1.8	0.9%
Uf	Urban land	>200	6.7	3.5%
UIC	Urban land-Charlton-Chatfield complex, rolling, very rocky	>200	3.2	1.6%
UpB	Urban land-Paxton complex, 3 to 8 percent slopes	>200	0.7	0.4%
W	Water	>200	1.4	0.7%

Map unit symbol	Map unit name	Rating (centimeters)	Acres in AOI	Percent of AOI
WdB	Woodbridge loam, 3 to 8 percent slopes	46	0.0	0.0%
WdC	Woodbridge loam, 8 to 15 percent slopes	46	1.1	0.6%
Totals for Area of Interest			194.5	100.0%

Description

"Water table" refers to a saturated zone in the soil. It occurs during specified months. Estimates of the upper limit are based mainly on observations of the water table at selected sites and on evidence of a saturated zone, namely grayish colors (redoximorphic features) in the soil. A saturated zone that lasts for less than a month is not considered a water table.

This attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

Rating Options

Units of Measure: centimeters

Aggregation Method: Dominant Component

Component Percent Cutoff: None Specified

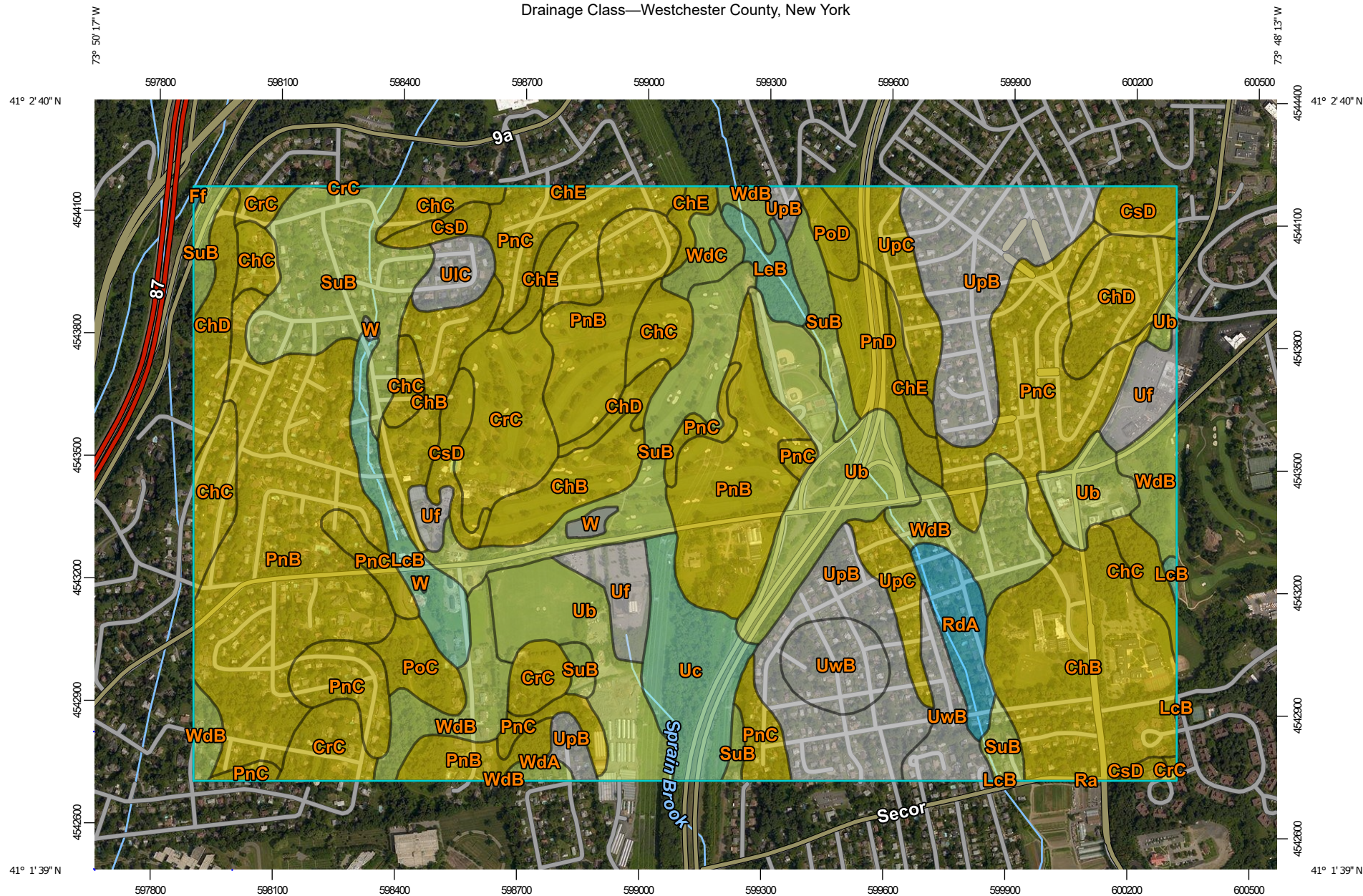
Tie-break Rule: Lower

Interpret Nulls as Zero: No

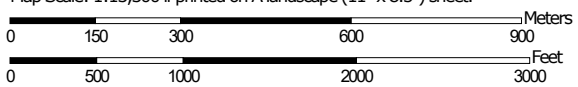
Beginning Month: January

Ending Month: December

Drainage Class—Westchester County, New York



Map Scale: 1:13,300 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey

7/23/2020
Page 1 of 4

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

-  Excessively drained
-  Somewhat excessively drained
-  Well drained
-  Moderately well drained
-  Somewhat poorly drained
-  Poorly drained
-  Very poorly drained
-  Subaqueous
-  Not rated or not available

Soil Rating Lines

-  Excessively drained
-  Somewhat excessively drained
-  Well drained
-  Moderately well drained
-  Somewhat poorly drained
-  Poorly drained
-  Very poorly drained
-  Subaqueous
-  Not rated or not available

Soil Rating Points

-  Excessively drained
-  Somewhat excessively drained
-  Well drained
-  Moderately well drained
-  Somewhat poorly drained
-  Poorly drained
-  Very poorly drained
-  Subaqueous
-  Not rated or not available

Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 16, Jun 11, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 21, 2014—Aug 27, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Drainage Class

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
ChB	Charlton fine sandy loam, 3 to 8 percent slopes	Well drained	82.3	9.4%
ChC	Charlton fine sandy loam, 8 to 15 percent slopes	Well drained	32.6	3.7%
ChD	Charlton fine sandy loam, 15 to 25 percent slopes	Well drained	30.0	3.4%
ChE	Charlton loam, 25 to 35 percent slopes	Well drained	10.0	1.1%
CrC	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	Well drained	39.2	4.5%
CsD	Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky	Well drained	17.1	2.0%
Ff	Fluvaquents-Udifulvents complex, frequently flooded	Poorly drained	0.3	0.0%
LcB	Leicester loam, 3 to 8 percent slopes, stony	Somewhat poorly drained	14.0	1.6%
LeB	Leicester loam, 2 to 8 percent slopes, very stony	Somewhat poorly drained	6.4	0.7%
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	Well drained	143.6	16.4%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	Well drained	88.3	10.1%
PnD	Paxton fine sandy loam, 15 to 25 percent slopes	Well drained	13.7	1.6%
PoC	Paxton fine sandy loam, 8 to 15 percent slopes, very stony	Well drained	7.8	0.9%
PoD	Paxton fine sandy loam, 15 to 25 percent slopes, very stony	Well drained	4.8	0.6%
Ra	Raynham silt loam	Poorly drained	0.0	0.0%
RdA	Ridgebury complex, 0 to 3 percent slopes	Poorly drained	11.5	1.3%
SuB	Sutton loam, 3 to 8 percent slopes	Moderately well drained	81.7	9.4%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
Ub	Udorthents, smoothed	Moderately well drained	68.2	7.8%
Uc	Udorthents, wet substratum	Somewhat poorly drained	24.1	2.8%
Uf	Urban land		19.1	2.2%
UIC	Urban land-Charlton-Chatfield complex, rolling, very rocky		6.7	0.8%
UpB	Urban land-Paxton complex, 3 to 8 percent slopes		73.5	8.4%
UpC	Urban land-Paxton complex, 8 to 15 percent slopes	Well drained	21.7	2.5%
UwB	Urban land-Woodbridge complex, 3 to 8 percent slopes		22.8	2.6%
W	Water		2.0	0.2%
WdA	Woodbridge loam, 0 to 3 percent slopes	Moderately well drained	1.9	0.2%
WdB	Woodbridge loam, 3 to 8 percent slopes	Moderately well drained	44.2	5.1%
WdC	Woodbridge loam, 8 to 15 percent slopes	Moderately well drained	5.5	0.6%
Totals for Area of Interest			872.8	100.0%

Description

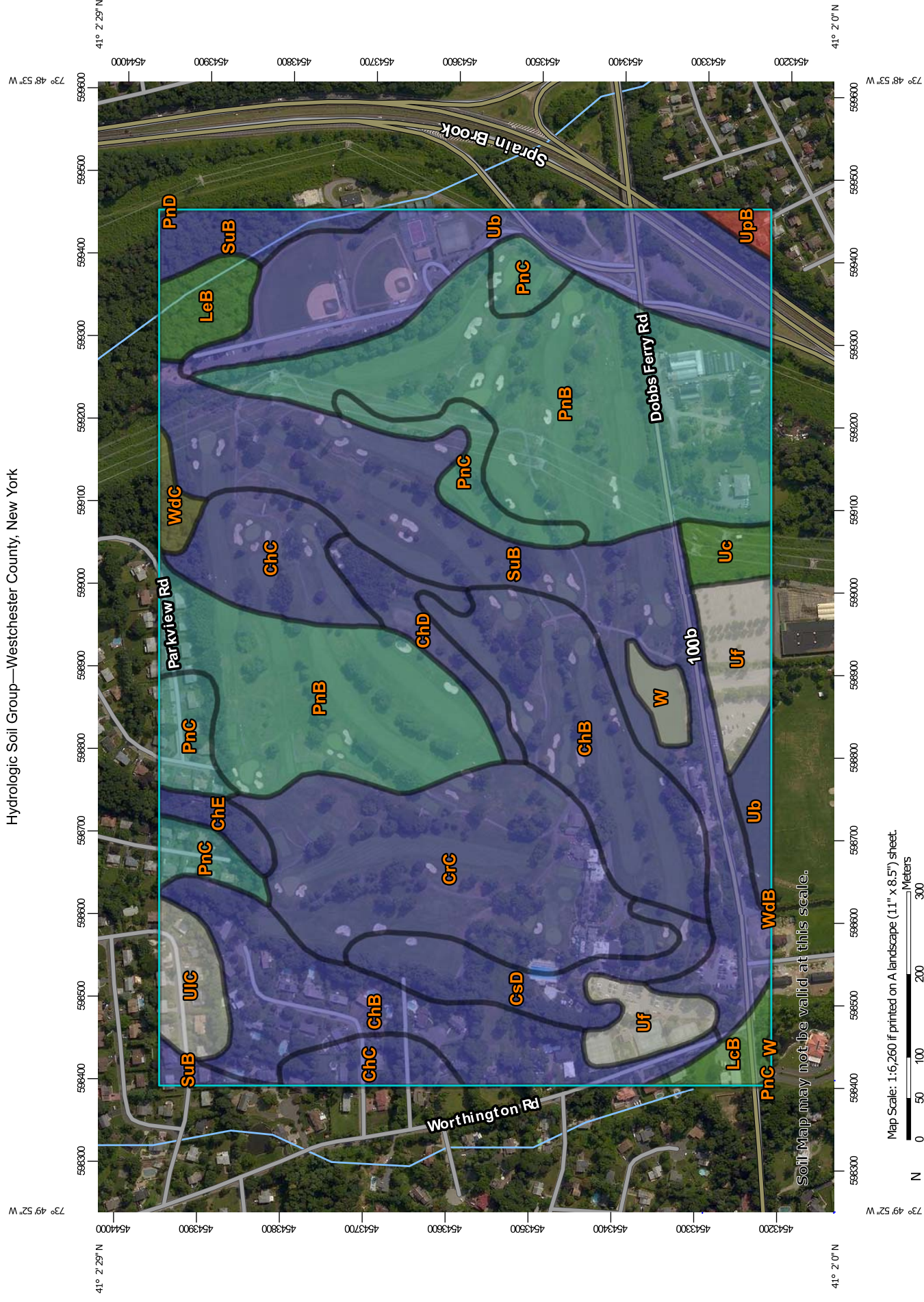
"Drainage class (natural)" refers to the frequency and duration of wet periods under conditions similar to those under which the soil formed. Alterations of the water regime by human activities, either through drainage or irrigation, are not a consideration unless they have significantly changed the morphology of the soil. Seven classes of natural soil drainage are recognized-excessively drained, somewhat excessively drained, well drained, moderately well drained, somewhat poorly drained, poorly drained, and very poorly drained. These classes are defined in the "Soil Survey Manual."

Rating Options

Aggregation Method: Dominant Condition

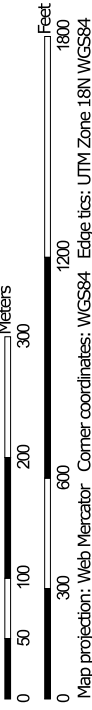
Component Percent Cutoff: None Specified

Tie-break Rule: Higher



Map Scale: 1:6,260 if printed on A landscape (11" x 8.5") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

	A
	A/D
	B
	B/D
	C
	C/D
	D
	Not rated or not available

Soil Rating Lines

	A
	A/D
	B
	B/D
	C
	C/D
	D
	Not rated or not available

Soil Rating Points

	A
	A/D
	B
	B/D

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

C

 C

C/D

 C/D

D

 D

Not rated or not available

 Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 13, Oct 8, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 21, 2014—Aug 27, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
ChB	Charlton fine sandy loam, 3 to 8 percent slopes	B	27.3	14.0%
ChC	Charlton fine sandy loam, 8 to 15 percent slopes	B	9.2	4.7%
ChD	Charlton fine sandy loam, 15 to 25 percent slopes	B	3.7	1.9%
ChE	Charlton loam, 25 to 35 percent slopes	B	1.5	0.8%
CrC	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	B	21.8	11.2%
CsD	Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky	B	6.4	3.3%
LcB	Leicester loam, 3 to 8 percent slopes, stony	A/D	1.8	0.9%
LeB	Leicester loam, 2 to 8 percent slopes, very stony	A/D	2.8	1.5%
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	C	46.1	23.7%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	C	9.3	4.8%
PnD	Paxton fine sandy loam, 15 to 25 percent slopes	C	0.0	0.0%
SuB	Sutton loam, 3 to 8 percent slopes	B	30.7	15.8%
Ub	Udorthents, smoothed	B	19.0	9.8%
Uc	Udorthents, wet substratum	A/D	1.8	0.9%
Uf	Urban land		6.7	3.5%
UIC	Urban land-Charlton-Chatfield complex, rolling, very rocky		3.2	1.6%
UpB	Urban land-Paxton complex, 3 to 8 percent slopes	D	0.7	0.4%
W	Water		1.4	0.7%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
WdB	Woodbridge loam, 3 to 8 percent slopes	C/D	0.0	0.0%
WdC	Woodbridge loam, 8 to 15 percent slopes	C/D	1.1	0.6%
Totals for Area of Interest			194.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

APPENDIX E

OMP-1 “OPERATION AND MAINTENANCE PLAN”

Temporary Erosion and Sediment Control Inspection and Maintenance Checklist

Erosion and Sediment Control Measure	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Stabilized Construction Entrance	Daily	• Periodic top dressing with additional aggregate as required • Clean sediment in public right-of-ways immediately
Silt Fence	Weekly + After Each Rain	• Remove & redistribute sediment when bulges develop in the silt fence.
Inlet Protection	Weekly + After Each Rain	• Remove sediment as necessary and replace filter fabric, crushed stone etc. • Any broken and damaged components should be replaced. • Check all materials for proper anchorage and secure as necessary.
Concrete Washout	Daily	• Damaged or leaking facilities shall be deactivated and repaired or replaced immediately.
	After Each Rain	• Pump excess rainwater that has accumulated over hardened concrete to a stabilized area.
		• Remove accumulated hardened material when 75% of the storage capacity of the structure is filled. Replace plastic liner with each cleaning of the washout facility.

Temporary Erosion and Sediment Control Inspection and Maintenance Checklist
(Cont'd)

Erosion and Sediment Control Measure	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Temporary Swale	Weekly + After Each Rain	• Immediately remove any sediment and reestablish vegetative cover as required. • Inspect entire swale and surrounding area for rilling and repair as required with the placement of additional topsoil, then seeding and mulching, as outlined in Figure No. 3.14 of the NYSDEC New York State Standards and Specifications for Erosion and Sediment Control.
Temporary Sediment Trap	Weekly + After Each Rain	• Remove and redistribute sediment when it reaches an elevation indicated on the construction documents. • Check for rilling within and around the sediment basin and repair as required. • Remove all sediment and debris from the outlet control structure as maybe required.

Permanent Stormwater Management Practice Inspection and Maintenance Checklist

Stormwater Management Practice	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Vegetated Swale	Monthly	• Check that contributing area is clean of debris. • Confirm vegetation is adequately maintained (mowing, fertilizer, etc.) • Check for rilling/erosion and repair as needed. • Confirm dewatering occurs between storms.
Vegetated Swale	Annually + After Major Storms	• Clean sediment and re-vegetate as necessary. • Check condition of outlet and repair as necessary
Rip-Rap Apron/Energy Dissipator and Check Dams	Annually + After Major Storms	• Check for evidence of flows going around the structure. • Check for evidence at downstream toe and repair as needed. • Clean sediment and install additional aggregate as necessary.
Stormwater Management Basin	Monthly	• Check Permanent Pool for undesirable vegetative growth and floatings or floatable debris. Remove as needed. • Check Forebays for sediment and cleanout when it depth <50% design depth. • Check Dry Pond areas for adequate vegetation, undesirable vegetative growth, low flow channels are clear of obstructions, standing water or wet spots and sediment and/or trash accumulation. Repair/remove as necessary.

Permanent Stormwater Management Practice Inspection and Maintenance Checklist (Cont'd)

Stormwater Management Practice	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Stormwater Management Basin	Annually + After Major Storms	• Check adequacy of vegetation and ground cover; for evidence of embankment erosion, animal burrows, unauthorized plantings and cracking, bulging or sliding of dam, clear/properly functioning drains, seeps/leaks on downstream face, failure of slope protection or riprap. Repair/remove as necessary. • Confirm emergency spillway is clear of obstructions and debris. • Confirm all inlets and outlet structures/pipes are operating properly.
Drain Inlets	Monthly	• Check for blockage and/or erosion at top of each inlet. Repair/remove as necessary. • Check for sediment and debris collected within sumps and clean out as necessary.
Subsurface Stormwater Management Detention Facility	Annually + After Major Storms	• Check level of sediment and debris accumulated within the system. • Check structural integrity of the system pipes, structures, etc. for cracking, bulging or deterioration. Repair/remove as necessary. • Confirm all inlets and outlet structures/pipes are operating properly.

Stormwater Management Practice	Inspection/Maintenance Intervals	• Inspection/Maintenance Requirements
Hydrodynamic Water Quality Structure	(See Maintenance Guidelines in Appendix E)	• Open access cover for visual inspection and measure the distance from the standing water surface to the sediment pile with a measuring stick or tape. If less than 4 feet, insert hose from vacuum truck into the sump and screen through both access covers to clean out the standing water, layer of oil, sediment, trash, etc. • The screen must be powerwashed to ensure it is free of trash and debris.

The owner/operator responsible for inspection and maintenance as outlined above:

Contact Company: Ridgewood Elmwood Owner, LLC
 Contact Name: Mr. Jonathan Grebow
 Street Address: 25A Hanover Road, Suite 310
 City, State Zip: Florham Park, NJ 07932
 Phone:
 Fax:
 Email:

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Cascade Separator[®] Inspection and Maintenance Guide



Maintenance

The Cascade Separator® system should be inspected at regular intervals and maintained when necessary to ensure optimum performance. The rate at which the system collects sediment and debris will depend upon on-site activities and site pollutant characteristics. For example, unstable soils or heavy winter sanding will cause the sediment storage sump to fill more quickly but regular sweeping of paved surfaces will slow accumulation.

Inspection

Inspection is the key to effective maintenance and is easily performed. Pollutant transport and deposition may vary from year to year and regular inspections will help ensure that the system is cleaned out at the appropriate time. At a minimum, inspections should be performed twice per year (i.e. spring and fall). However, more frequent inspections may be necessary in climates where winter sanding operations may lead to rapid accumulations, or in equipment wash-down areas. Installations should also be inspected more frequently where excessive amounts of trash are expected.

A visual inspection should ascertain that the system components are in working order and that there are no blockages or obstructions in the inlet chamber, flumes or outlet channel. The inspection should also quantify the accumulation of hydrocarbons, trash and sediment in the system. Measuring pollutant accumulation can be done with a calibrated dipstick, tape measure or other measuring instrument. If absorbent material is used for enhanced removal of hydrocarbons, the level of discoloration of the sorbent material should also be identified during inspection. It is useful and often required as part of an operating permit to keep a record of each inspection. A simple form for doing so is provided in this Inspection and Maintenance Guide.

Access to the Cascade Separator unit is typically achieved through one manhole access cover. The opening allows for inspection and cleanout of the center chamber (cylinder) and sediment storage sump, as well as inspection of the inlet chamber and slanted skirt. For large units, multiple manhole covers allow access to the chambers and sump.

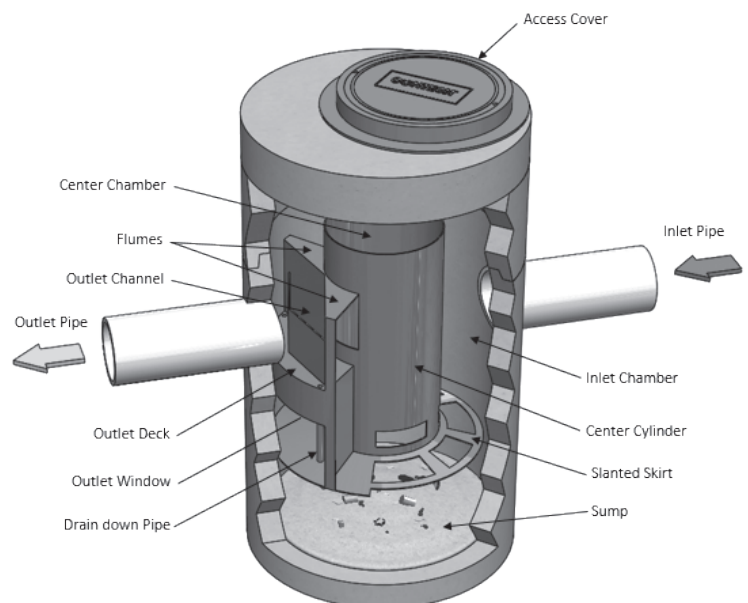
The Cascade Separator system should be cleaned before the level of sediment in the sump reaches the maximum sediment depth and/or when an appreciable level of hydrocarbons and trash has accumulated. If sorbent material is used, it must be replaced when significant discoloration has occurred. Performance may be impacted when maximum sediment storage capacity is exceeded. Contech recommends maintaining the system when sediment level reaches 50% of maximum storage volume. The level of sediment is easily determined by measuring the distance from the system outlet invert (standing water level) to the top of the sediment pile. To avoid underestimating the level of sediment in the chamber, the measuring device must be lowered to the top of the sediment pile carefully. Finer, silty particles at the top of the pile typically offer less resistance to the end of the rod than larger particles toward the bottom of the pile. Once this measurement is recorded, it should be compared to the chart in this document to determine if the height of the sediment pile off the bottom of the sump floor exceeds 50% of the maximum sediment storage.

Cleaning

Cleaning of a Cascade Separator system should be done during dry weather conditions when no flow is entering the system. The use of a vacuum truck is generally the most effective and convenient method of removing pollutants from the system. Simply remove the manhole cover and insert the vacuum tube down through the center chamber and into the sump. The system should be completely drained down and the sump fully evacuated of sediment. The areas outside the center chamber and the slanted skirt should also be washed off if pollutant build-up exists in these areas.

In installations where the risk of petroleum spills is small, liquid contaminants may not accumulate as quickly as sediment. However, the system should be cleaned out immediately in the event of an oil or gasoline spill. Motor oil and other hydrocarbons that accumulate on a more routine basis should be removed when an appreciable layer has been captured. To remove these pollutants, it may be preferable to use absorbent pads since they are usually less expensive to dispose than the oil/water emulsion that may be created by vacuuming the oily layer. Trash and debris can be netted out to separate it from the other pollutants. Then the system should be power washed to ensure it is free of trash and debris.

Manhole covers should be securely seated following cleaning activities to prevent leakage of runoff into the system from above and to ensure proper safety precautions. Confined space entry procedures need to be followed if physical access is required. Disposal of all material removed from the Cascade Separator system must be done in accordance with local regulations. In many locations, disposal of evacuated sediments may be handled in the same manner as disposal of sediments removed from catch basins or deep sump manholes. Check your local regulations for specific requirements on disposal. If any components are damaged, replacement parts can be ordered from the manufacturer.



Cascade Separator® Maintenance Indicators and Sediment Storage Capacities

Model Number	Diameter		Distance from Water Surface to Top of Sediment Pile		Sediment Storage Capacity	
	ft	m	ft	m	y ³	m ³
CS-3	3	0.9	1.5	0.5	0.4	0.3
CS-4	4	1.2	1.5	0.5	0.7	0.5
CS-5	5	1.3	1.5	0.5	1.1	0.8
CS-6	6	1.8	1.5	0.5	1.6	1.2
CS-8	8	2.4	1.5	0.5	2.8	2.1
CS-10	10	3.0	1.5	0.5	4.4	3.3
CS-12	12	3.6	1.5	0.5	6.3	4.8

Note: The information in the chart is for standard units. Units may have been designed with non-standard sediment storage depth.



A Cascade Separator unit can be easily cleaned in less than 30 minutes.



A vacuum truck excavates pollutants from the systems.

Cascade Separator® Inspection & Maintenance Log

[illegible]

1. The depth to sediment is determined by taking a measurement from the manhole outlet invert (standing water level) to the top of the sediment pile. Once this measurement is recorded, it should be compared to the chart in the maintenance guide to determine if the height of the sediment pile off the bottom of the sump floor exceeds 50% of the maximum sediment storage. Note: to avoid underestimating the volume of sediment in the chamber, the measuring device must be carefully lowered to the top of the sediment pile.
2. For optimum performance, the system should be cleaned out when the floating hydrocarbon layer accumulates to an appreciable thickness. In the event of an oil spill, the system should be cleaned immediately.

SUPPORT

- Drawings and specifications are available at www.ContechES.com.
- Site-specific design support is available from our engineers.

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800.925.5240

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Maintenance Inspection Checklists

Conveyance System Maintenance and Management Inspection Checklist (Separate Form For Each Pipe Run)

Project: _____

Location: _____

Site Status: _____

Date: _____ Time: _____

Inspector Signature: _____

Inspector Name(printed): _____

Pipe Number: _____

Inspection/Maintenance Items	Satisfactory (S) or Unsatisfactory (U)	Comments/Corrective Action
1. Inspection (Quarter-annually, After Major Storms)		
1. Accumulated sediment exceeds 10% of the diameter of the pipe.		
2. Vegetation that reduces free movement of water through pipes		
3. Pipe Damage: Any dent that decreases flow area by more than 10% or puncture that impacts performance		
4. Trash accumulated to reduce free movement of water through pipes.		

(Provide sketch to show location of Unsatisfactory Items)

Maintenance Inspection Checklists

Rip-Rap Apron Checklist (Separate Form For Each Apron)

Project: _____

Location: _____

Site Status: _____

Date: _____ Time: _____

Inspector Signature: _____

Inspector Name (printed): _____

Pipe Number: _____

Inspection/Maintenance Items	Satisfactory (S) or Unsatisfactory (U)	Comments/Corrective Action
Rip-Rap Apron/Energy Dissipator and Check Dams		
1. Annually + After Major Storms		
• Check for evidence of flows going around the structure.		
• Check for evidence at downstream toe and repair as needed.		
• Clean sediment and install additional aggregate as necessary.		

(Provide sketch to show location of Unsatisfactory Items)

Whereas, the Municipality of _____ ("Municipality") and the _____ ("facility owner") want to enter into an agreement to provide for the long term maintenance and continuation of stormwater control measures approved by the Municipality for the below named project, and

Whereas, the Municipality and the facility owner desire that the stormwater control measures be built in accordance with the approved project plans and thereafter be maintained, cleaned, repaired, replaced and continued in perpetuity in order to ensure optimum performance of the components. Therefore, the Municipality and the facility owner agree as follows:

1. This agreement binds the Municipality and the facility owner, its successors and assigns, to the maintenance provisions depicted in the approved project plans which are attached as Schedule A of this agreement.
2. The facility owner shall maintain, clean, repair, replace and continue the stormwater control measures as necessary to ensure optimum performance of the measures to design specifications. The stormwater control measures shall include, but shall not be limited to, the following: drainage ditches, swales, infiltrators, drop inlets, pipes, culverts, soil absorption devices and detention ponds.
3. The facility owner shall be responsible for all expenses related to the maintenance of the stormwater control measures and shall establish a means for the collection and distribution of expenses among parties for any commonly owned facilities.
4. The facility owner shall provide for the periodic inspection of the stormwater control measures, not less than once in every five year period, to determine the condition and integrity of the measures. Such inspection shall be performed by a Professional Engineer licensed by the State of New York. The inspecting engineer shall prepare and submit to the Municipality within 30 days of the inspection, a written report of the findings including recommendations for those actions necessary for the continuation of the stormwater control measures.
5. The facility owner shall not authorize, undertake or permit alteration, abandonment, modification or discontinuation of the stormwater control measures except in accordance with written approval of the Municipality.

6. The facility owner shall undertake necessary repairs and replacement of the stormwater control measures at the direction of the Municipality or in accordance with the recommendations of the inspecting engineer.

7. The facility owner shall provide to the Municipality within 30 days of the date of this agreement, a security for the maintenance and continuation of the stormwater control measures in the form of (a Bond, letter of credit or escrow account).

8. This agreement shall be recorded in the Office of the County Clerk, County of _____ together with the deed for the common property and shall be included in the offering plan and/or prospectus approved pursuant to _____.

9. If ever the Municipality determines that the facility owner has failed to construct or maintain the stormwater control measures in accordance with the project plan or has failed to undertake corrective action specified by the Municipality or by the inspecting engineer, the Municipality is authorized to undertake such steps as reasonably necessary for the preservation, continuation or maintenance of the stormwater control measures and to affix the expenses thereof as a lien against the property.

10. This agreement is effective _____

Stormwater Pond/Wetland Operation, Maintenance and Management Inspection Checklist

Project _____
 Location: _____
 Site Status: _____

 Date: _____
 Time: _____

 Inspector: _____

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
1. Embankment and emergency spillway (Annual, After Major Storms)		
1. Vegetation and ground cover adequate		
2. Embankment erosion		
3. Animal burrows		
4. Unauthorized planting		
5. Cracking, bulging, or sliding of dam		
a. Upstream face		
b. Downstream face		
c. At or beyond toe		
downstream		
upstream		
d. Emergency spillway		
6. Pond, toe & chimney drains clear and functioning		
7. Seeps/leaks on downstream face		
8. Slope protection or riprap failure		
9. Vertical/horizontal alignment of top of dam "As-Built"		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
10. Emergency spillway clear of obstructions and debris		
11. Other (specify)		
2. Riser and principal spillway (Annual)		
Type: Reinforced concrete _____ Corrugated pipe _____ Masonry _____		
1. Low flow orifice obstructed		
2. Low flow trash rack. a. Debris removal necessary		
b. Corrosion control		
3. Weir trash rack maintenance a. Debris removal necessary		
b. corrosion control		
4. Excessive sediment accumulation insider riser		
5. Concrete/masonry condition riser and barrels a. cracks or displacement		
b. Minor spalling (<1")		
c. Major spalling (rebars exposed)		
d. Joint failures		
e. Water tightness		
6. Metal pipe condition		
7. Control valve a. Operational/exercised		
b. Chained and locked		
8. Pond drain valve a. Operational/exercised		
b. Chained and locked		
9. Outfall channels functioning		
10. Other (specify)		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
3. Permanent Pool (Wet Ponds) (monthly)		
1. Undesirable vegetative growth		
2. Floating or floatable debris removal required		
3. Visible pollution		
4. Shoreline problem		
5. Other (specify)		
4. Sediment Forebays		
1. Sedimentation noted		
2. Sediment cleanout when depth < 50% design depth		
5. Dry Pond Areas		
1. Vegetation adequate		
2. Undesirable vegetative growth		
3. Undesirable woody vegetation		
4. Low flow channels clear of obstructions		
5. Standing water or wet spots		
6. Sediment and / or trash accumulation		
7. Other (specify)		
6. Condition of Outfalls (Annual , After Major Storms)		
1. Riprap failures		
2. Slope erosion		
3. Storm drain pipes		
4. Endwalls / Headwalls		
5. Other (specify)		
7. Other (Monthly)		
1. Encroachment on pond, wetland or easement area		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
2. Complaints from residents		
3. Aesthetics		
a. Grass growing required		
b. Graffiti removal needed		
c. Other (specify)		
4. Conditions of maintenance access routes.		
5. Signs of hydrocarbon build-up		
6. Any public hazards (specify)		
8. Wetland Vegetation (Annual)		
1. Vegetation healthy and growing Wetland maintaining 50% surface area coverage of wetland plants after the second growing season. (If unsatisfactory, reinforcement plantings needed)		
2. Dominant wetland plants: Survival of desired wetland plant species Distribution according to landscaping plan?		
3. Evidence of invasive species		
4. Maintenance of adequate water depths for desired wetland plant species		
5. Harvesting of emergent plantings needed		
6. Have sediment accumulations reduced pool volume significantly or are plants "choked" with sediment		
7. Eutrophication level of the wetland.		
8. Other (specify)		

Comments:

Actions to be Taken:

Maintenance Inspection Checklists

Manhole / Catch Basin System Maintenance and Management Inspection Checklist (Separate Form For Each Drainage Manhole & Catch Basin)

Project: _____

Location: _____

Site Status: _____

Date: _____ Time: _____

Inspector Signature: _____

Inspector Name (printed): _____

Structure Number: _____

Inspection/Maintenance Items	Satisfactory (S) or Unsatisfactory (U)	Comments/Corrective Action
1. Inspection (Quarter-annually, After Major Storms)		
1. Accumulated debris or sediment depth exceeds sump or impedes flow from inlet or outlet pipes		
2. Inlet or Outlet Pipe Damaged		
3. Contaminants & Pollutants visible		
4. Cover I Grate functioning properly		
5. Structure: No cracks larger than ½"		
6. Ladder		
7. Mosquito Breeding Habitat		
2. Sediment		
1. Depth of sediment (in inches)*		
2. Depth of oil {in inches)**		
3. Sediment and oil have been removed		

*If measured depth of sediment is greater than 12 inches, the structure shall be cleaned immediately

**Any presence of oil shall be removed immediately

(Provide sketch to show location of Unsatisfactory Items)

APPENDIX F

CONTRACTOR CERTIFICATION



Site Planning
Civil Engineering
Landscape Architecture
Land Surveying
Transportation Engineering

Environmental Studies
Entitlements
Construction Services
3D Visualization
Laser Scanning

JMC Project 17099
Elmwood Preserve
850 Dobbs Ferry Road
Town of Greenburgh, NY

CONTRACTOR'S CERTIFICATION

"I hereby certify that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the qualified inspector during a site inspection. I also understand that the owner or operator must comply with the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware there are significant penalties for submitting false information that I do not believe to be true, including the possibility of time and imprisonment for knowing violations."

Company Name: _____

Address: _____

Telephone Number: _____

Name and Title: _____

Signature: _____ Date: _____

Permit Identification No.: _____

Name and Title of Trained Contractor: _____

Elements of the SWPPP Contractor is responsible for: _____

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APPENDIX G

NEW YORK STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL

STANDARD AND SPECIFICATIONS FOR CONCRETE TRUCK WASHOUT



Definition & Scope

A temporary excavated or above ground lined constructed pit where concrete truck mixers and equipment can be washed after their loads have been discharged, to prevent highly alkaline runoff from entering storm drainage systems or leaching into soil.

Conditions Where Practice Applies

Washout facilities shall be provided for every project where concrete will be poured or otherwise formed on the site. This facility will receive highly alkaline wash water from the cleaning of chutes, mixers, hoppers, vibrators, placing equipment, trowels, and screeds. Under no circumstances will wash water from these operations be allowed to infiltrate into the soil or enter surface waters.

Design Criteria

Capacity: The washout facility should be sized to contain solids, wash water, and rainfall and sized to allow for the evaporation of the wash water and rainfall. Wash water shall be estimated at 7 gallons per chute and 50 gallons per hopper of the concrete pump truck and/or discharging drum. The minimum size shall be 8 feet by 8 feet at the bottom and 2 feet deep. If excavated, the side slopes shall be 2 horizontal to 1 vertical.

Location: Locate the facility a minimum of 100 feet from drainage swales, storm drain inlets, wetlands, streams and other surface waters. Prevent surface water from entering the structure except for the access road. Provide appropriate access with a gravel access road sloped down to the structure. Signs shall be placed to direct drivers to the facility after their load is discharged.

Liner: All washout facilities will be lined to prevent

leaching of liquids into the ground. The liner shall be plastic sheeting with a minimum thickness of 10 mils with no holes or tears, and anchored beyond the top of the pit with an earthen berm, sand bags, stone, or other structural appurtenance except at the access point.

If pre-fabricated washouts are used they must ensure the capture and containment of the concrete wash and be sized based on the expected frequency of concrete pours. They shall be sited as noted in the location criteria.

Maintenance

- All concrete washout facilities shall be inspected daily. Damaged or leaking facilities shall be deactivated and repaired or replaced immediately. Excess rainwater that has accumulated over hardened concrete should be pumped to a stabilized area, such as a grass filter strip.
- Accumulated hardened material shall be removed when 75% of the storage capacity of the structure is filled. Any excess wash water shall be pumped into a containment vessel and properly disposed of off site.
- Dispose of the hardened material off-site in a construction/demolition landfill. On-site disposal may be allowed if this has been approved and accepted as part of the projects SWPPP. In that case, the material should be recycled as specified, or buried and covered with a minimum of 2 feet of clean compacted earthfill that is permanently stabilized to prevent erosion.
- The plastic liner shall be replaced with each cleaning of the washout facility.
- Inspect the project site frequently to ensure that no concrete discharges are taking place in non-designated areas.

STANDARD AND SPECIFICATIONS FOR DUST CONTROL



dust control (see Section 3).

Mulch (including gravel mulch) – Mulch offers a fast effective means of controlling dust. This can also include rolled erosion control blankets.

Spray adhesives – These are products generally composed of polymers in a liquid or solid form that are mixed with water to form an emulsion that is sprayed on the soil surface with typical hydroseeding equipment. The mixing ratios and application rates will be in accordance with the manufacturer's recommendations for the specific soils on the site. In no case should the application of these adhesives be made on wet soils or if there is a probability of precipitation within 48 hours of its proposed use. Material Safety Data Sheets will be provided to all applicators and others working with the material.

Definition & Scope

The control of dust resulting from land-disturbing activities, to prevent surface and air movement of dust from disturbed soil surfaces that may cause off-site damage, health hazards, and traffic safety problems.

Conditions Where Practice Applies

On construction roads, access points, and other disturbed areas subject to surface dust movement and dust blowing where off-site damage may occur if dust is not controlled.

Design Criteria

Construction operations should be scheduled to minimize the amount of area disturbed at one time. Buffer areas of vegetation should be left where practical. Temporary or permanent stabilization measures shall be installed. No specific design criteria is given; see construction specifications below for common methods of dust control.

Water quality must be considered when materials are selected for dust control. Where there is a potential for the material to wash off to a stream, ingredient information must be provided to the NYSDEC.

No polymer application shall take place without written approval from the NYSDEC.

Construction Specifications

A. Non-driving Areas – These areas use products and materials applied or placed on soil surfaces to prevent airborne migration of soil particles.

Vegetative Cover – For disturbed areas not subject to traffic, vegetation provides the most practical method of

B. Driving Areas – These areas utilize water, polymer emulsions, and barriers to prevent dust movement from the traffic surface into the air.

Sprinkling – The site may be sprayed with water until the surface is wet. This is especially effective on haul roads and access route to provide short term limited dust control.

Polymer Additives – These polymers are mixed with water and applied to the driving surface by a water truck with a gravity feed drip bar, spray bar or automated distributor truck. The mixing ratios and application rates will be in accordance with the manufacturer's recommendations. Incorporation of the emulsion into the soil will be done to the appropriate depth based on expected traffic. Compaction after incorporation will be by vibratory roller to a minimum of 95%. The prepared surface shall be moist and no application of the polymer will be made if there is a probability of precipitation within 48 hours of its proposed use. Material Safety Data Sheets will be provided to all applicators working with the material.

Barriers – Woven geo-textiles can be placed on the driving surface to effectively reduce dust throw and particle migration on haul roads. Stone can also be used for construction roads for effective dust control.

Windbreak – A silt fence or similar barrier can control air currents at intervals equal to ten times the barrier height. Preserve existing wind barrier vegetation as much as practical.

Maintenance

Maintain dust control measures through dry weather periods until all disturbed areas are stabilized.

STANDARD AND SPECIFICATIONS FOR PROTECTING VEGETATION DURING CONSTRUCTION



Definition & Scope

The protection of trees, shrubs, ground cover and other vegetation from damage by construction equipment. In order to preserve existing vegetation determined to be important for soil erosion control, water quality protection, shade, screening, buffers, wildlife habitat, wetland protection, and other values.

Conditions Where Practices Applies

On planned construction sites where valued vegetation exists and needs to be preserved.

Design Criteria

1. Planning Considerations

A. Inventory:

1) Property boundaries, topography, vegetation and soils information should be gathered. Identify potentially high erosion areas, areas with tree windthrow potential, etc. A vegetative cover type map should be made on a copy of a topographic map which shows other natural and manmade features. Vegetation that is desirable to preserve because of its value for screening, shade, critical erosion control, endangered species, aesthetics, etc., should be identified and marked on the map.

2) Based upon this data, general statements should be prepared about the present condition, potential problem areas, and unique features of the property.

B. Planning:

1) After engineering plans (plot maps) are prepared, another field review should take place and

recommendations made for the vegetation to be saved. Minor adjustments in location of roads, dwellings, and utilities may be needed. Construction on steep slopes, erodible soils, wetlands, and streams should be avoided. Clearing limits should be delineated (See "Determine Limits of Clearing and Grading" on page 2.2).

2) Areas to be seeded and planted should be identified. Remaining vegetation should blend with their surroundings and/or provide special function such as a filter strip, buffer zone, or screen.

3) Trees and shrubs of special seasonal interest, such as flowering dogwood, red maple, striped maple, serviceberry, or shadbush, and valuable potential shade trees should be identified and marked for special protective treatment as appropriate.

4) Trees to be cut should be marked on the plans. If timber can be removed for salable products, a forester should be consulted for marketing advice.

5) Trees that may become a hazard to people, personal property, or utilities should be removed. These include trees that are weak-wooded, disease-prone, subject to windthrow, or those that have severely damaged root systems.

6) The vigor of remaining trees may be improved by a selective thinning. A forester should be consulted for implementing this practice.

2. Measures to Protect Vegetation

A. Limit soil placement over existing tree and shrub roots to a maximum of 3 inches. Soils with loamy texture and good structure should be used.

B. Use retaining walls and terraces to protect roots of trees and shrubs when grades are lowered. Lowered grades should start no closer than the dripline of the tree. For narrow-canopied trees and shrubs, the stem diameter in inches is converted to feet and doubled, such that a 10 inch tree should be protected to 20 feet.

C. Trenching across tree root systems should be the same minimum distance from the trunk, as in "B". Tunnels under root systems for underground utilities should start 18 inches or deeper below the normal ground surface. Tree roots which must be severed should be cut clean. Backfill material that will be in contact with the roots should be topsoil or a prepared planting soil mixture.

D. Construct sturdy fences, or barriers, of wood, steel, or other protective material around valuable

vegetation for protection from construction equipment. Place barriers far enough away from trees, but not less than the specifications in "B", so that tall equipment such as backhoes and dump trucks do not contact tree branches.

E. Construction limits should be identified and clearly marked to exclude equipment.

F. Avoid spills of oil/gas and other contaminants.

G. Obstructive and broken branches should be pruned properly. The branch collar on all branches whether living or dead should not be damaged. The 3 or 4 cut method should be used on all branches larger than two inches at the cut. First cut about one-third the way through the underside of the limb (about 6-12 inches from the tree trunk). Then (approximately an inch further out) make a second cut through the limb from the upper side. When the branch is removed, there is no splintering of the main tree trunk. Remove the stub. If the branch is larger than 5-6 inches in diameter, use the four cut system. Cuts 1 and 2 remain the same and cut 3 should be from the underside of the limb, on the outside of the branch collar. Cut 4 should be from the top and in alignment with the 3rd cut. Cut 3 should be 1/4 to 1/3 the way through the limb. This will prevent the bark from peeling down the trunk. Do not paint the cut surface.

H. Penalties for damage to valuable trees, shrubs, and herbaceous plants should be clearly spelled out in the contract.

PROTECTING TREES IN HEAVY USE AREAS

The compaction of soil over the roots of trees and shrubs by the trampling of recreationists, vehicular traffic, etc., reduces oxygen, water, and nutrient uptake by feeder roots. This weakens and may eventually kill the plants. Table 2.6 rates the "Susceptibility of Tree Species to Compaction."

Where heavy compaction is anticipated, apply and maintain a 3 to 4 inch layer of undecayed wood chips or 2 inches of No. 2 washed, crushed gravel. In addition, use of a wooden or plastic mat may be used to lessen compaction, if applicable.

Table 2.6
Susceptibility of Tree Species to Compaction¹

Resistant:

Box elder.....	<i>Acer negundo</i>	Willows.....	<i>Salix spp.</i>
Green ash.....	<i>Fraxinus pennsylvanica</i>	Honey locust.....	<i>Gleditsia triacanthos</i>
Red elm.....	<i>Ulmus rubra</i>	Eastern cottonwood.....	<i>Populus deltoides</i>
Hawthornes.....	<i>Crataegus spp.</i>	Swamp white oak.....	<i>Quercus bicolor</i>
Bur oak.....	<i>Quercus macrocarpa</i>	Hophornbeam.....	<i>Ostrya virginiana</i>
Northern white cedar....	<i>Thuja occidentalis</i>		

Intermediate:

Red maple.....	<i>Acer rubrum</i>	Sweetgum.....	<i>Liquidambar styraciflua</i>
Silver maple.....	<i>Acer saccharinum</i>	Norway maple.....	<i>Acer platanoides</i>
Hackberry.....	<i>Celtis occidentalis</i>	Shagbark hickory.....	<i>Carya ovata</i>
Black gum.....	<i>Nyssa sylvatica</i>	London plane.....	<i>Platanus x hybrida</i>
Red oak.....	<i>Quercus rubra</i>	Pin oak.....	<i>Quercus palustris</i>
Basswood.....	<i>Tilia americana</i>		

Susceptible:

Sugar maple.....	<i>Acer saccharum</i>	Austrian Pine.....	<i>Pinus nigra</i>
White pine.....	<i>Pinus strobus</i>	White ash.....	<i>Fraxinus americana</i>
Blue spruce.....	<i>Picea pungens</i>	Paper birch.....	<i>Betula papyrifera</i>
White oak.....	<i>Quercus alba</i>	Mountain ash.....	<i>Sorbus aucuparia</i>
Red pine.....	<i>Pinus resinosa</i>	Japanese maple.....	<i>Acer palmatum</i>

¹ If a tree species does not appear on the list, insufficient information is available to rate it for this purpose.

STANDARD AND SPECIFICATIONS FOR SITE POLLUTION PREVENTION



Definition & Scope

A collection of management practices intended to control non-sediment pollutants associated with construction activities to prevent the generation of pollutants due to improper handling, storage, and spills and prevent the movement of toxic substances from the site into surface waters.

Conditions Where Practice Applies

On all construction sites where the earth disturbance exceeds 5,000 square feet, and involves the use of fertilizers, pesticides, petroleum based chemicals, fuels and lubricants, as well as sealers, paints, cleared woody vegetation, garbage, and sanitary wastes.

Design Criteria

The variety of pollutants on a particular site and the severity of their impacts depend on factors such as the nature of the construction activity, the physical characteristics of the construction site, and the proximity of water bodies and conveyances to the pollutant source.

1. All state and federal regulations shall be followed for the storage, handling, application, usage, and disposal of pesticides, fertilizers, and petroleum products.
2. Vehicle and construction equipment staging and maintenance areas will be located away from all drainage ways with their parking areas graded so the runoff from these areas is collected, contained and treated prior to discharge from the site.
3. Provide sanitary facilities for on-site personnel.
4. Store, cover, and isolate construction materials including topsoil, and chemicals, to prevent runoff of



pollutants and contamination of groundwater and surface waters.

5. Develop and implement a spill prevention and control plan. The plan should include NYSDEC's spill reporting and initial notification requirements.
6. Provide adequate disposal for solid waste including woody debris, stumps, and other construction waste and include these methods and directions in the construction details on the site construction drawings. Fill, woody debris, stumps and construction waste shall not be placed in regulated wetlands, streams or other surface waters.
7. Distribute or post informational material regarding proper handling, spill response, spill kit location, and emergency actions to be taken, to all construction personnel.
8. Refueling equipment shall be located at least 100 feet from all wetlands, streams and other surface waters.



STANDARD AND SPECIFICATIONS FOR STABILIZED CONSTRUCTION ACCESS



Definition & Scope

A stabilized pad of aggregate underlain with geotextile located at any point where traffic will be entering or leaving a construction site to or from a public right-of-way, street, alley, sidewalk, or parking area. The purpose of stabilized construction access is to reduce or eliminate the tracking of sediment onto public rights-of-way or streets.

Conditions Where Practice Applies

A stabilized construction access shall be used at all points of construction ingress and egress.

Design Criteria

See Figure 2.1 on page 2.31 for details.

Aggregate Size: Use a matrix of 1-4 inch stone, or reclaimed or recycled concrete equivalent.

Thickness: Not less than six (6) inches.

Width: 12-foot minimum but not less than the full width of points where ingress or egress occurs. 24-foot minimum if there is only one access to the site.

Length: As required, but not less than 50 feet (except on a single residence lot where a 30 foot minimum would apply).

Geotextile: To be placed over the entire area to be covered with aggregate. Filter cloth will not be required on a single-family residence lot. Piping of surface water under entrance shall be provided as required. If piping is impossible, a mountable berm with 5:1 slopes will be permitted.

Criteria for Geotextile: The geotextile shall be woven or nonwoven fabric consisting only of continuous chain polymeric filaments or yarns of polyester. The fabric shall be

inert to commonly encountered chemicals, hydro-carbons, mildew, rot resistant, and conform to the fabric properties as shown:

Fabric Properties ³	Light Duty ¹ Roads Grade Sub-grade	Heavy Duty ² Haul Roads Rough Graded	Test Method
Grab Tensile Strength (lbs)	200	220	ASTM D1682
Elongation at Failure (%)	50	60	ASTM D1682
Mullen Burst Strength (lbs)	190	430	ASTM D3786
Puncture Strength (lbs)	40	125	ASTM D751 Modified
Equivalent	40-80	40-80	US Std Sieve
Opening Size			CW-02215
Aggregate Depth	6	10	-

¹Light Duty Road: Area sites that have been graded to subgrade and where most travel would be single axle vehicles and an occasional multi-axle truck. Acceptable materials are Trevira Spunbond 1115, Mirafi 100X, Typar 3401, or equivalent.

²Heavy Duty Road: Area sites with only rough grading, and where most travel would be multi-axle vehicles. Acceptable materials are Trevira Spunbond 1135, Mirafi 600X, or equivalent.

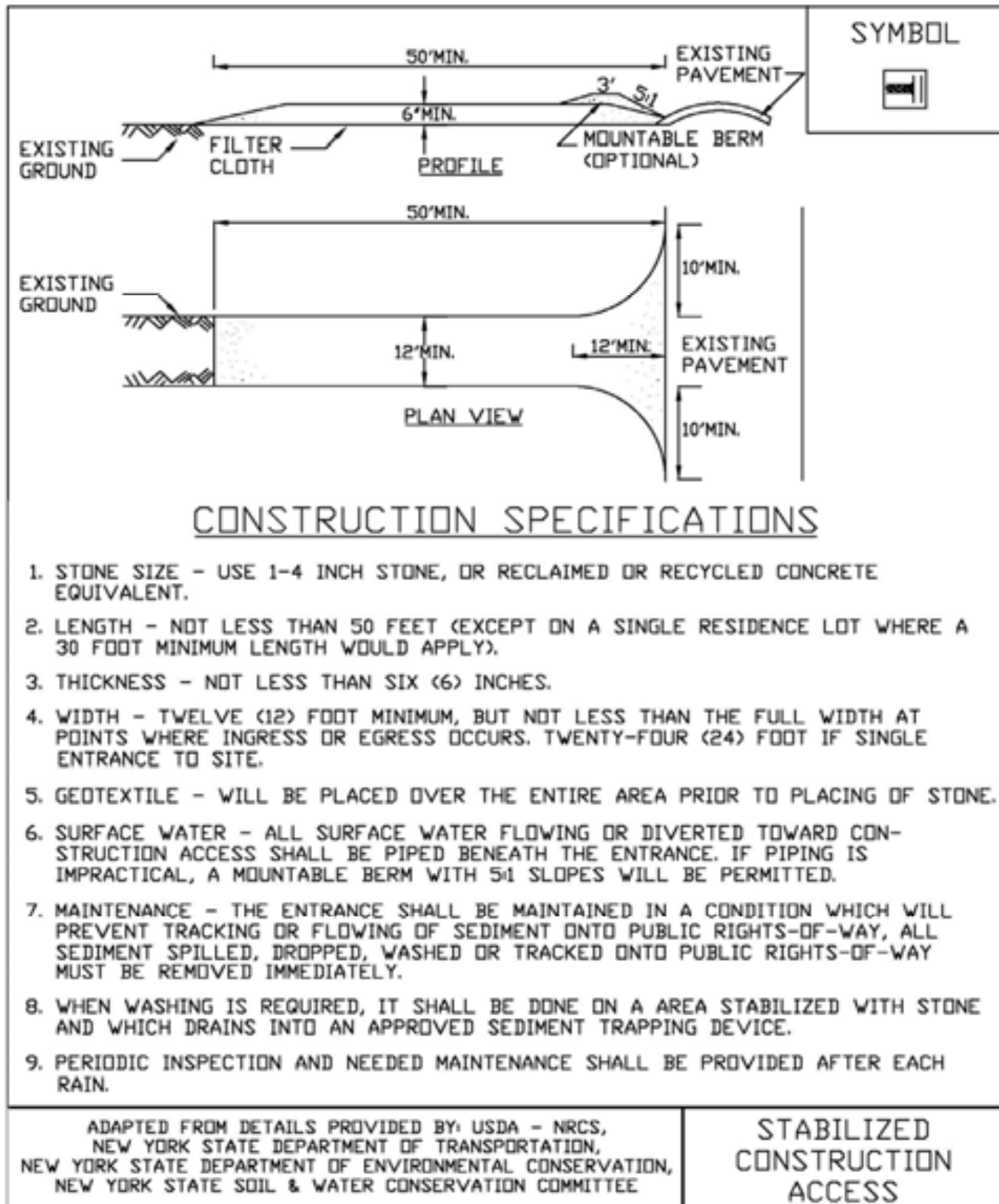
³Fabrics not meeting these specifications may be used only when design procedure and supporting documentation are supplied to determine aggregate depth and fabric strength.

Maintenance

The access shall be maintained in a condition which will prevent tracking of sediment onto public rights-of-way or streets. This may require periodic top dressing with additional aggregate. All sediment spilled, dropped, or washed onto public rights-of-way must be removed immediately.

When necessary, wheels must be cleaned to remove sediment prior to entrance onto public rights-of-way. When washing is required, it shall be done on an area stabilized with aggregate, which drains into an approved sediment-trapping device. All sediment shall be prevented from entering storm drains, ditches, or watercourses.

Figure 2.1
Stabilized Construction Access



STANDARD AND SPECIFICATIONS FOR WINTER STABILIZATION



Definition & Scope

A temporary site specific, enhanced erosion and sediment control plan to manage runoff and sediment at the site during construction activities in the winter months to protect off-site water resources.

Conditions Where Practice Applies

This standard applies to all construction activities involved with ongoing land disturbance and exposure between November 15th to the following April 1st.

Design Criteria

1. Prepare a snow management plan with adequate storage for snow and control of melt water, requiring cleared snow to be stored in a manner not affecting ongoing construction activities.
2. Enlarge and stabilize access points to provide for snow management and stockpiling. Snow management activities must not destroy or degrade installed erosion and sediment control practices.
3. A minimum 25 foot buffer shall be maintained from all perimeter controls such as silt fence. Mark silt fence with tall stakes that are visible above the snow pack.
4. Edges of disturbed areas that drain to a waterbody within 100 feet will have 2 rows of silt fence, 5 feet apart, installed on the contour.
5. Drainage structures must be kept open and free of snow and ice dams. All debris, ice dams, or debris from plowing operations, that restrict the flow of runoff and meltwater, shall be removed.
6. Sediment barriers must be installed at all appropriate

perimeter and sensitive locations. Silt fence and other practices requiring earth disturbance must be installed before the ground freezes.

7. Soil stockpiles must be protected by the use of established vegetation, anchored straw mulch, rolled stabilization matting, or other durable covering. A barrier must be installed at least 15 feet from the toe of the stockpile to prevent soil migration and to capture loose soil.
8. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures should be initiated by the end of the next business day and completed within three (3) days. Rolled erosion control blankets must be used on all slopes 3 horizontal to 1 vertical or steeper.
9. If straw mulch alone is used for temporary stabilization, it shall be applied at double the standard rate of 2 tons per acre, making the application rate 4 tons per acre. Other manufactured mulches should be applied at double the manufacturer's recommended rate.
10. To ensure adequate stabilization of disturbed soil in advance of a melt event, areas of disturbed soil should be stabilized at the end of each work day unless:
 - a. work will resume within 24 hours in the same area and no precipitation is forecast or;
 - b. the work is in disturbed areas that collect and retain runoff, such as open utility trenches, foundation excavations, or water management areas.
11. Use stone paths to stabilize access perimeters of buildings under construction and areas where construction vehicle traffic is anticipated. Stone paths should be a minimum 10 feet in width but wider as necessary to accommodate equipment.

Maintenance

The site shall be inspected frequently to ensure that the erosion and sediment control plan is performing its winter stabilization function. If the site will not have earth disturbing activities ongoing during the "winter season", **all** bare exposed soil must be stabilized by established vegetation, straw or other acceptable mulch, matting, rock, or other approved material such as rolled erosion control products. Seeding of areas with mulch cover is preferred but seeding alone is not acceptable for proper stabilization.

Compliance inspections must be performed and reports filed properly in accordance with the SWPPP for all sites under a winter shutdown.

References

1. Northeastern Illinois Soil and Sedimentation Control Steering Committee. October 1981. Procedures and Standards for Urban Soil Erosion and Sediment Control in Illinois.
2. J.F. Rushing, V.M. Moore, J.S. Tingle, Q. Mason, and T. McCaffery, 2005. Dust Abatement Methods for Lines of Communication and Base Camps in Temperate Climates. ERDC/GSL TR-05-23, October 2005.

STANDARD AND SPECIFICATIONS FOR CHECK DAM



Definition & Scope

Small barriers or dams constructed of stone, bagged sand or gravel, or other durable materials across a drainageway to reduce erosion in a drainage channel by reducing the velocity of flow in the channel.

Conditions Where Practice Applies

This practice is used as a **temporary** and, in some cases, a **permanent** measure to limit erosion by reducing velocities in open channels that are degrading or subject to erosion or where permanent stabilization is impractical due to short period of usefulness and time constraints of construction.

Design Criteria

Drainage Area: Maximum drainage area above the check dam shall not exceed two (2) acres.

Height: Not greater than 2 feet. Center shall be maintained 9 inches lower than abutments at natural ground elevation.

Side Slopes: Shall be 2:1 or flatter.

Spacing: The check dams shall be spaced as necessary in the channel so that the crest of the downstream dam is at the elevation of the toe of the upstream dam. This spacing is equal to the height of the check dam divided by the channel slope.

Therefore:

$$S = \frac{h}{s}$$

Where:

S = spacing interval (ft.)
h = height of check dam (ft.)
s = channel slope (ft./ft.)

Example:

For a channel with
and 2 ft. high stone
they are spaced as

$$S = \frac{2 \text{ ft}}{0.04 \frac{\text{ft}}{\text{ft}}} = 50 \text{ ft}$$

a 4% slope
check dams,
follows:

For stone check dams: Use a well graded stone matrix 2 to 9 inches in size (NYS – DOT Light Stone Fill meets these requirements).

The overflow of the check dams will be stabilized to resist erosion that might be caused by the check dam. See Figure 3.1 on page 3.3 for details.

Check dams should be anchored in the channel by a cutoff trench 1.5 ft. wide and 0.5 ft. deep and lined with filter fabric to prevent soil migration.

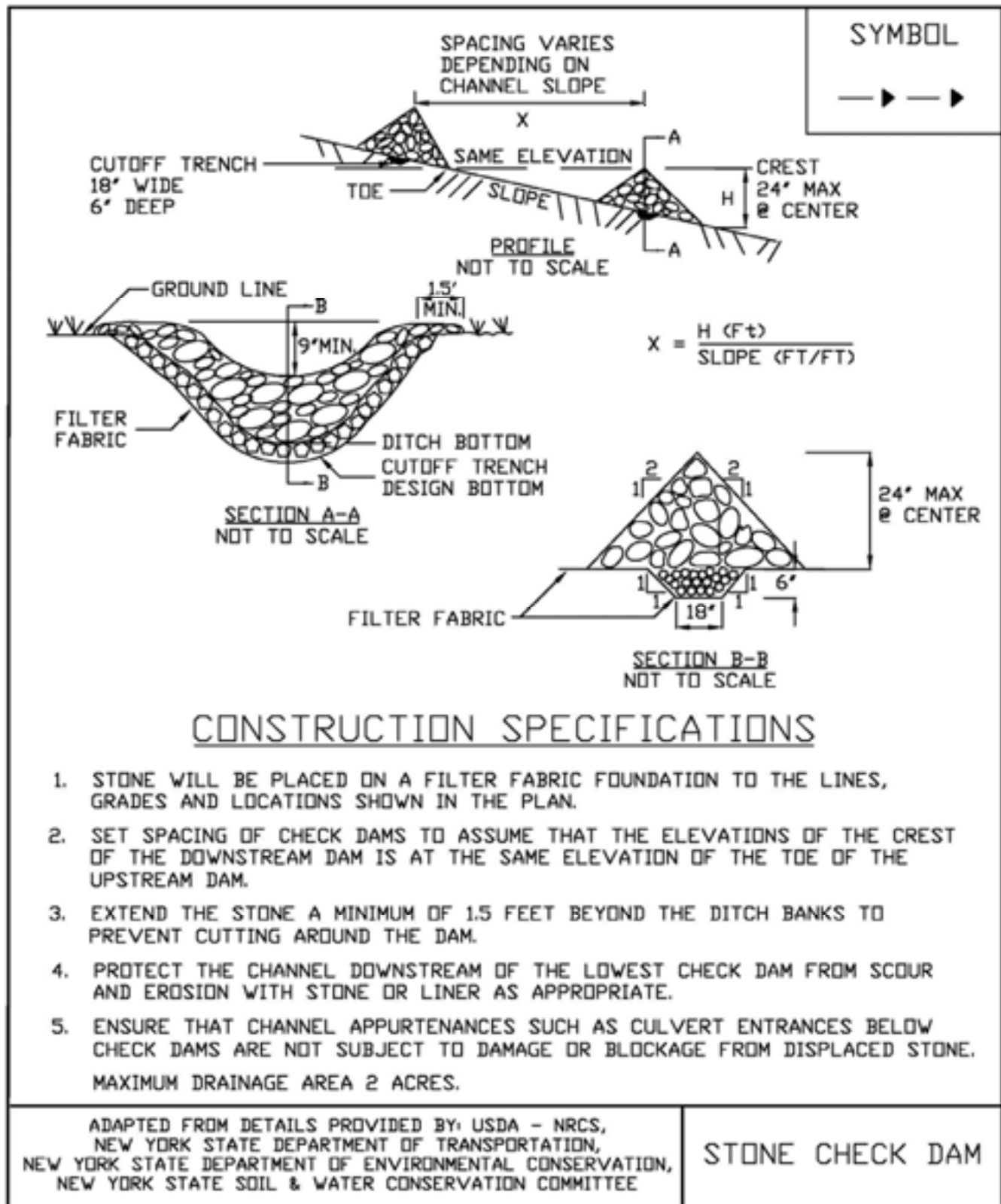
For filter sock or fiber roll check dams: The check dams will be anchored by staking the dam to the earth contact surface. The dam will extend to the top of the bank. The check dam will have a splash apron of NYS DOT #2 crushed stone extending a minimum 3 feet downstream from the dam and 1 foot up the sides of the channel. The compost and materials for a filter sock check dam shall meet the requirements shown in the standard for Compost Filter Sock on page 5.7.

Maintenance

The check dams should be inspected after each runoff event. Correct all damage immediately. If significant erosion has occurred between structures, a liner of stone or other suitable material should be installed in that portion of the channel or additional check dams added.

Remove sediment accumulated behind the dam as needed to allow channel to drain through the stone check dam and prevent large flows from carrying sediment over the dam.

Figure 3.1
Stone Check Dam Detail



STANDARD AND SPECIFICATIONS FOR DIVERSION



Definition & Scope

A drainage way of parabolic or trapezoidal cross-section with a supporting ridge on the lower side that is constructed across the slope to intercept and convey runoff to stable outlets at non-erosive velocities.

Conditions Where Practice Applies

Diversions are used where:

1. Runoff from higher areas has potential for damaging properties, causing erosion, or interfering with, or preventing the establishment of, vegetation on lower areas.
2. Surface and/or shallow subsurface flow is damaging sloping upland.
3. The length of slopes needs to be reduced so that soil loss will be kept to a minimum.

Diversions are only applicable below stabilized or protected areas. Avoid establishment on slopes greater than fifteen percent. Diversions should be used with caution on soils subject to slippage. Construction of diversions shall be in compliance with state and local drainage and water laws.

Design Criteria

Location

Diversion location shall be determined by considering outlet conditions, topography, land use, soil type, length of slope, seep planes (when seepage is a problem), and the development layout.

Capacity

Peak rates of runoff values used in determining the capacity requirements shall be calculated using the most current hydrologic data from the Northeast Regional Climate Center in an appropriate model.

The constructed diversion shall have capacity to carry, as a minimum, the peak discharge from a 10 year frequency rainfall event with freeboard of not less than 0.3 feet.

Diversions designed to protect homes, schools, industrial buildings, roads, parking lots, and comparable high-risk areas, and those designed to function in connection with other structures, shall have sufficient capacity to carry peak runoff expected from a storm frequency consistent with the hazard involved.

Cross Section

The diversion channel shall be parabolic or trapezoidal in shape. Parabolic Diversion design charts are provided in Tables 3.2, 3.3 and 3.4 on pages 3.10, 3.12 and 3.13. The diversion shall be designed to have stable side slopes. The side slopes shall not be steeper than 2:1 and shall be flat enough to ensure ease of maintenance of the diversion and its protective vegetative cover.

The ridge shall have a minimum width of four feet at the design water elevation; a minimum of 0.3 feet freeboard and a reasonable settlement factor shall be provided.

Velocity and Grade

The permissible velocity for the specified method of stabilization will determine the maximum grade. Maximum permissible velocities of flow for the stated conditions of stabilization shall be as shown in Table 3.1 on page 3.10 of this standard.

Diversions are not usually applicable below high sediment producing areas unless land treatment practices or structural measures, designed to prevent damaging accumulations of sediment in the channels, are installed with, or before, the diversions.

Outlets

Each diversion must have an adequate outlet. The outlet may be a grassed waterway, vegetated or paved area, grade stabilization structure, flow spreader, flow diffuser, stable watercourse, or subsurface drain outlet. In all cases, the outlet must convey runoff to a point where outflow will not cause damage. Vegetated outlets shall be installed before diversion construction, if needed, to ensure establishment of

vegetative cover in the outlet channel.

Stabilization

The design elevation of the water surface in the diversion shall not be lower than the design elevation of the water surface in the outlet at their junction when both are operating at design flow.

Vegetated diversions shall be stabilized in accordance with the following tables.

Table 3.1
Diversion Maximum Permissible Design Velocities Table

Soil Texture	Retardance and Cover	Permissible Velocity (ft / second) for Selected Channel Vegetation
Sand, Silt, Sandy loam, silty loam, loamy sand (ML, SM, SP, SW)	C-Kentucky 31 tall fescue and Kentucky bluegrass	3.0
	D-Annuals ¹ Small grain (rye, oats, barley, millet) Ryegrass	2.5
Silty clay loam, Sandy clay loam (ML-CL, SC)	C-Kentucky 31 tall fescue and Kentucky bluegrass	4.0
	D-Annuals ¹ Small grain (rye, oats, barley, millet) Ryegrass	3.5
Clay (CL)	C-Kentucky 31 tall fescue and Kentucky bluegrass	5.0
	D-Annuals ¹ Small grain (rye, oats, barley, millet) Ryegrass	4.0
¹ Annuals—Use only as temporary protection until permanent vegetation is established.		

Table 3.2 - Retardance Factors for Various Grasses and Legumes Table

Retardance	Cover	Condition
A	Reed canarygrass	Excellent stand, tall (average 36 inches)
B	Smooth brome grass	Good stand, mowed (average 12 to 15 inches)
	Tall fescue	Good stand, unmowed (average 18 inches)
	Grass-legume mixture—Timothy, smooth brome grass, or Orchard grass with birdsfoot trefoil	Good stand, uncut (average 20 inches)
	Reed canarygrass	Good stand, mowed (average 12 to 15 inches)
	Tall fescue, with birdsfoot trefoil or ladino clover	Good stand, uncut (average 18 inches)
C	Redtop	Good stand, headed (15 to 20 inches)
	Grass-legume mixture—summer (Orchard grass, redtop, Annual ryegrass, and ladino or white clover)	Good stand, uncut (6 to 8 inches)
	Kentucky bluegrass	Good stand, headed (6 to 12 inches)
D	Red fescue	Good stand, headed (12 to 18 inches)
	Grass-legume mixture—fall, spring (Orchard grass, redtop, Annual ryegrass, and white or ladino clover)	Good stand, uncut (4 to 5 inches)

Figure 3.4
Diversion Detail

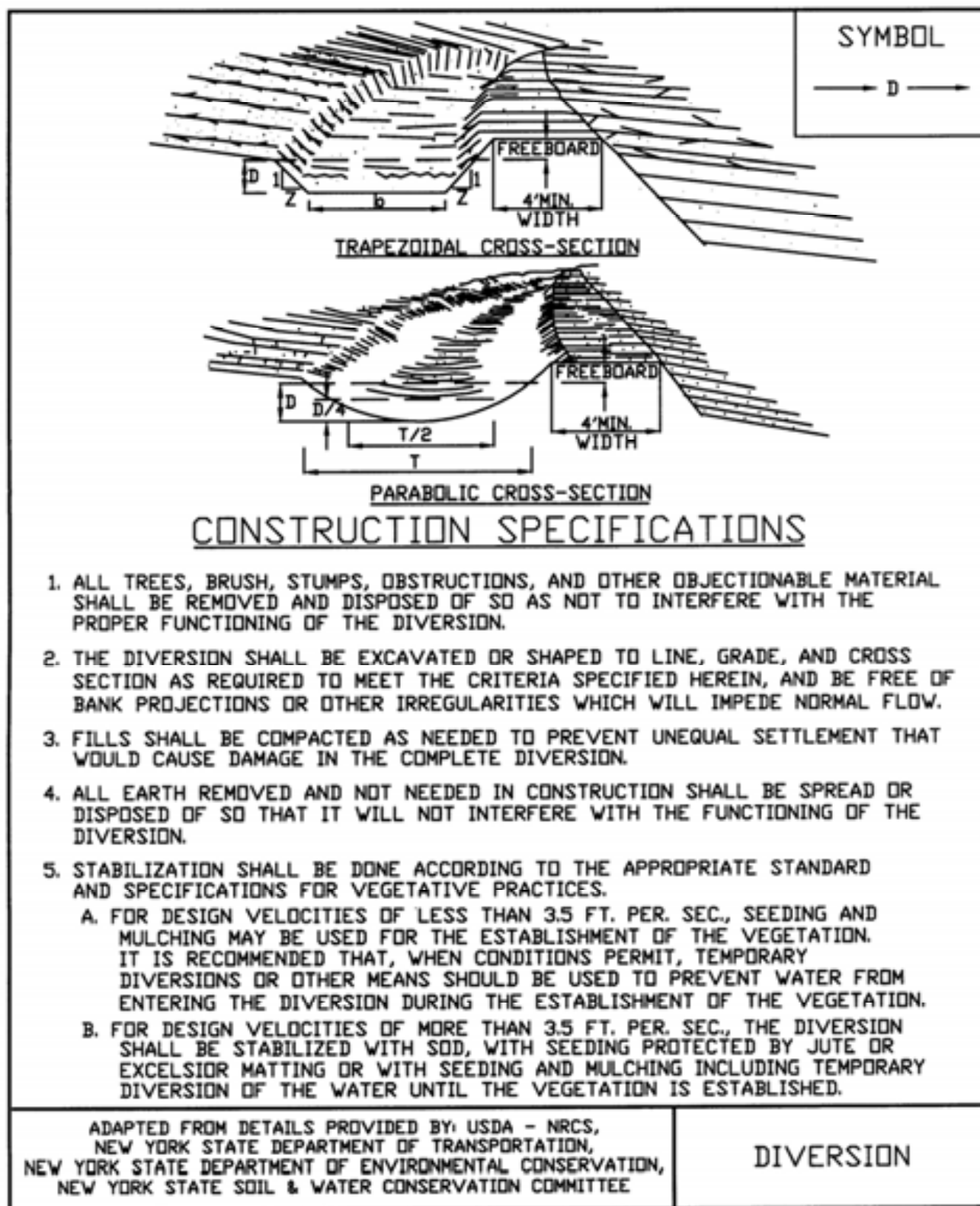


Table 3.3
Parabolic Diversion Design, Without Freeboard Tables - 1 (USDA- NRCS)

PARABOLIC DIVERSION DESIGN, WITHOUT FREEBOARD										RETARDANCE - D & C GRADE, %- 0.50
V ₁ Based on Permissible Velocity of the Soil With Retardance "D" Top Width, Depth & V ₂ Based on Retardance "C"										
Q	V ₁ = 2.0	V ₁ = 2.5	V ₁ = 3.0	V ₁ = 3.5	V ₁ = 4.0	V ₁ = 4.5	V ₁ = 5.0	V ₁ = 5.5	V ₁ = 6.0	
cfs	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂
15	9 1.6 1.6									
20	11 1.6 1.7									
25	14 1.6 1.7	9 1.9 2.1								
30	17 1.6 1.7	11 1.9 2.2								
35	20 1.6 1.7	12 1.9 2.3	8 2.2 2.5							
40	22 1.6 1.7	14 1.8 2.3	9 2.1 2.6							
45	25 1.5 1.7	16 1.8 2.3	11 2.1 2.7							
50	28 1.5 1.7	18 1.8 2.4	12 2.0 2.8							
55	31 1.5 1.7	19 1.8 2.4	13 2.0 2.8	10 2.4 3.2						
60	33 1.5 1.7	21 1.8 2.4	15 2.0 2.8	11 2.4 3.3						
65	36 1.5 1.8	23 1.8 2.4	16 2.0 2.8	12 2.4 3.3						
70	39 1.5 1.7	24 1.8 2.4	18 2.0 2.9	13 2.3 3.4						
75	42 1.5 1.8	26 1.8 2.4	19 2.0 2.9	14 2.3 3.4	11 2.7 3.7					
80	44 1.5 1.8	28 1.8 2.4	21 2.0 2.9	15 2.3 3.4	12 2.7 3.8					
90	50 1.5 1.8	31 1.8 2.4	24 2.0 2.9	17 2.3 3.4	13 2.7 3.8					
100	55 1.5 1.8	35 1.8 2.4	26 2.0 2.9	19 2.3 3.5	15 2.6 3.9	12 3.0 4.1				
...										

PARABOLIC DIVERSION DESIGN, WITHOUT FREEBOARD										RETARDANCE - D & C GRADE, %- 1.0
V ₁ Based on Permissible Velocity of the Soil With Retardance "D" Top Width, Depth & V ₂ Based on Retardance "C"										
Q	V ₁ = 2.0	V ₁ = 2.5	V ₁ = 3.0	V ₁ = 3.5	V ₁ = 4.0	V ₁ = 4.5	V ₁ = 5.0	V ₁ = 5.5	V ₁ = 6.0	
cfs	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂	T D V ₂
15	13 1.1 1.5	8 1.3 2.0								
20	18 1.1 1.5	11 1.3 2.1	8 1.5 2.6							
25	22 1.1 1.5	14 1.3 2.1	9 1.5 2.6	8 1.6 3.0						
30	27 1.1 1.5	17 1.3 2.1	11 1.5 2.7	9 1.6 3.0						
35	31 1.1 1.5	19 1.3 2.2	13 1.5 2.8	11 1.6 3.1	8 1.8 3.6					
40	35 1.1 1.5	22 1.3 2.1	15 1.6 2.8	12 1.6 3.1	9 1.8 3.7					
45	40 1.1 1.5	25 1.3 2.2	17 1.5 2.8	13 1.6 3.2	10 1.8 3.7					
50	44 1.1 1.5	28 1.3 2.2	19 1.6 2.8	15 1.6 3.2	11 1.8 3.7					
55	48 1.1 1.5	30 1.3 2.2	20 1.6 2.8	16 1.5 3.3	12 1.8 3.8					
60	53 1.1 1.5	33 1.3 2.2	22 1.6 2.8	18 1.5 3.3	14 1.7 3.8	10 2.0 4.3				
65	57 1.1 1.5	36 1.3 2.2	24 1.6 2.8	19 1.5 3.3	15 1.7 3.8	11 2.0 4.3				
70	61 1.1 1.5	38 1.3 2.2	26 1.6 2.9	21 1.5 3.3	16 1.7 3.9	12 2.0 4.4	9 2.2 4.7			
75	66 1.1 1.5	41 1.3 2.2	28 1.6 2.9	22 1.5 3.3	17 1.7 3.9	13 2.0 4.5	10 2.2 4.7			
80	70 1.1 1.5	44 1.3 2.2	29 1.6 2.9	24 1.5 3.3	18 1.7 3.9	14 2.0 4.5	11 2.2 4.7			
90	79 1.1 1.5	49 1.3 2.2	33 1.6 2.9	27 1.5 3.3	20 1.7 3.9	15 1.9 4.5	13 2.2 4.9	11 2.4 5.2		
100	87 1.1 1.5	55 1.3 2.2	37 1.6 2.9	29 1.5 3.3	22 1.7 3.9	17 1.9 4.5	14 2.2 4.9	12 2.4 5.3		

Parabolic Diversion Design, Without Freeboard Tables - 2 (USDA- NRCS)

[illegible]

PARABOLIC DIVERSION DESIGN, WITHOUT FREEBOARD															RETARDANCE - D & C GRADE, % - 2.0			
V ₁ Based on Permissible Velocity of the Soil With Retardance "D" Top Width, Depth & V ₂ Based on Retardance "C"																		
Q	V ₁ = 2.0		V ₁ = 2.5		V ₁ = 3.0		V ₁ = 3.5		V ₁ = 4.0		V ₁ = 4.5		V ₁ = 5.0		V ₁ = 5.5		V ₁ = 6.0	
cfs	T	D	V ₂	T	D	V ₂	T	D	V ₂	T	D	V ₂	T	D	V ₂	T	D	V ₂
15	21	0.8	1.3	13	0.9	1.9	9	1.0	2.4	7	1.2	2.9						
20	28	0.8	1.3	17	0.9	1.9	12	1.0	2.4	9	1.1	3.0						
25	35	0.8	1.3	21	0.9	1.9	15	1.0	2.4	11	1.1	3.0						
30	41	0.8	1.3	26	0.9	1.9	18	1.0	2.5	13	1.1	3.0						
35	48	0.8	1.4	30	0.9	1.9	22	1.0	2.4	15	1.1	3.1						
40	55	0.8	1.3	34	0.9	1.9	25	1.0	2.5	18	1.1	3.1						
45	62	0.8	1.4	38	0.9	1.9	28	1.0	2.5	20	1.1	3.1						
50	68	0.8	1.4	42	0.9	1.9	31	1.0	2.5	22	1.1	3.1						
55	75	0.8	1.4	46	0.9	1.9	34	1.0	2.5	24	1.1	3.1						
60	82	0.8	1.4	51	0.9	1.9	37	1.0	2.5	26	1.1	3.1						
65	88	0.8	1.4	55	0.9	1.9	40	1.0	2.5	28	1.1	3.1						
70	95	0.8	1.4	59	0.9	1.9	43	1.0	2.5	30	1.1	3.1						
75				63	0.9	1.9	46	1.0	2.5	32	1.1	3.2						
80				67	0.9	2.0	48	1.0	2.5	35	1.1	3.1						
90				75	0.9	2.0	54	1.0	2.5	39	1.1	3.2						
100				83	0.9	2.0	60	1.0	2.5	43	1.1	3.2						
													5	1.4	3.8			
										7	1.3	3.7	8	1.3	3.7			
										10	1.2	3.7	9	1.3	4.2			
										11	1.2	3.8	9	1.3	4.2			
										13	1.2	3.8	11	1.3	4.3			
										14	1.2	3.8	12	1.3	4.3			
										16	1.2	3.9	13	1.3	4.3			
										17	1.2	3.8	14	1.3	4.3			
										19	1.2	3.9	16	1.3	4.4			
										21	1.2	3.9	18	1.3	4.4			
										24	1.2	3.9	20	1.3	4.4			
										25	1.2	3.9	21	1.3	4.4			
										28	1.2	3.9	23	1.3	4.4			
										31	1.2	3.9	26	1.3	4.4			
													5	1.4	4.8			
										7	1.3	3.5	8	1.4	4.8			
										8	1.3	3.7	7	1.4	4.8			
										10	1.2	3.7	9	1.3	4.2			
										11	1.2	3.8	9	1.3	4.2			
										13	1.2	3.8	11	1.3	4.3			
										14	1.2	3.8	12	1.3	4.3			
										16	1.2	3.9	13	1.3	4.3			
										17	1.2	3.8	14	1.3	4.3			
										19	1.2	3.9	16	1.3	4.4			
										21	1.2	3.9	18	1.3	4.4			
										24	1.2	3.9	20	1.3	4.4			
										25	1.2	3.9	21	1.3	4.4			
										28	1.2	3.9	23	1.3	4.4			
										31	1.2	3.9	26	1.3	4.4			
													7	1.5	4.5			
										8	1.5	4.7	9	1.5	4.7			
										10	1.4	4.8	10	1.4	4.8			
										11	1.4	4.8	11	1.4	4.8			
										12	1.4	4.9	12	1.4	4.9			
										13	1.4	4.9	13	1.4	4.9			
										15	1.4	4.9	15	1.4	4.9			
										16	1.4	5.0	16	1.4	5.0			
										17	1.4	4.9	17	1.4	4.9			
										21	1.4	5.0	21	1.4	5.0			
													8	1.6	5.2			
										9	1.6	5.3	9	1.6	5.3			
										10	1.6	5.3	10	1.6	5.3			
										11	1.6	5.3	11	1.6	5.3			
										11	1.6	5.4	11	1.6	5.4			
										12	1.6	5.4	12	1.6	5.4			
										13	1.6	5.5	13	1.6	5.5			
										14	1.6	5.5	14	1.6	5.5			
										16	1.6	5.5	16	1.6	5.5			
										17	1.6	5.5	17	1.6	5.5			
										21	1.6	5.0	21	1.6	5.0			
													8	1.7	5.6			
										8	1.7	5.6	8	1.7	5.6			
										9	1.7	5.7	9	1.7	5.7			
										10	1.7	5.7	10	1.7	5.7			
										11	1.7	5.7	11	1.7	5.7			
										12	1.7	5.8	12	1.7	5.8			
										13	1.7	5.8	13	1.7	5.8			
										14	1.7	5.9	14	1.7	5.9			
										16	1.7	5.9	16	1.7	5.9			
										17	1.7	5.9	17	1.7	5.9			
										21	1.7	6.0	21	1.7	6.0			

STANDARD AND SPECIFICATIONS FOR ROCK OUTLET PROTECTION



Definition & Scope

A **permanent** section of rock protection placed at the outlet end of the culverts, conduits, or channels to reduce the depth, velocity, and energy of water, such that the flow will not erode the receiving downstream reach.

Conditions Where Practice Applies

This practice applies where discharge velocities and energies at the outlets of culverts, conduits, or channels are sufficient to erode the next downstream reach. This applies to:

1. Culvert outlets of all types.
2. Pipe conduits from all sediment basins, dry storm water ponds, and permanent type ponds.
3. New channels constructed as outlets for culverts and conduits.

Design Criteria

The design of rock outlet protection depends entirely on the location. Pipe outlet at the top of cuts or on slopes steeper than 10 percent, cannot be protected by rock aprons or riprap sections due to re-concentration of flows and high velocities encountered after the flow leaves the apron.

Many counties and state agencies have regulations and design procedures already established for dimensions, type and size of materials, and locations where outlet protection is required. Where these requirements exist, they shall be followed.

Tailwater Depth

The depth of tailwater immediately below the pipe outlet

must be determined for the design capacity of the pipe. If the tailwater depth is less than half the diameter of the outlet pipe, and the receiving stream is wide enough to accept divergence of the flow, it shall be classified as a Minimum Tailwater Condition; see Figure 3.16 on page 3.42 as an example. If the tailwater depth is greater than half the pipe diameter and the receiving stream will continue to confine the flow, it shall be classified as a Maximum Tailwater Condition; see Figure 3.17 on page 3.43 as an example. Pipes which outlet onto flat areas with no defined channel may be assumed to have a Minimum Tailwater Condition; see Figure 3.16 on page 3.42 as an example.

Apron Size

The apron length and width shall be determined from the curves according to the tailwater conditions:

Minimum Tailwater – Use Figure 3.16 on page 3.42

Maximum Tailwater – Use Figure 3.17 on page 3.43

If the pipe discharges directly into a well defined channel, the apron shall extend across the channel bottom and up the channel banks to an elevation one foot above the maximum tailwater depth or to the top of the bank, whichever is less.

The upstream end of the apron, adjacent to the pipe, shall have a width two (2) times the diameter of the outlet pipe, or conform to pipe end section if used.

Bottom Grade

The outlet protection apron shall be constructed with no slope along its length. There shall be no overfall at the end of the apron. The elevation of the downstream end of the apron shall be equal to the elevation of the receiving channel or adjacent ground.

Alignment

The outlet protection apron shall be located so that there are no bends in the horizontal alignment.

Materials

The outlet protection may be done using rock riprap, grouted riprap, or gabions. Outlets constructed on the bank of a stream or wetland shall not use grouted rip-rap, gabions or concrete.

Riprap shall be composed of a well-graded mixture of rock size so that 50 percent of the pieces, by weight, shall be larger than the d_{50} size determined by using the charts. A

well-graded mixture, as used herein, is defined as a mixture composed primarily of larger rock sizes, but with a sufficient mixture of other sizes to fill the smaller voids between the rocks. The diameter of the largest rock size in such a mixture shall be 1.5 times the d_{50} size.

Thickness

The minimum thickness of the riprap layer shall be 1.5 times the maximum rock diameter for d_{50} of 15 inches or less; and 1.2 times the maximum rock size for d_{50} greater than 15 inches. The following chart lists some examples:

D_{50} (inches)	d_{max} (inches)	Minimum Blanket Thick- ness (inches)
4	6	9
6	9	14
9	14	20
12	18	27
15	22	32
18	27	32
21	32	38
24	36	43

Rock Quality

Rock for riprap shall consist of field rock or rough unhewn quarry rock. The rock shall be hard and angular and of a quality that will not disintegrate on exposure to water or weathering. The specific gravity of the individual rocks shall be at least 2.5.

Filter

A filter is a layer of material placed between the riprap and the underlying soil surface to prevent soil movement into and through the riprap. Riprap shall have a filter placed under it in all cases.

A filter can be of two general forms: a gravel layer or a plastic filter cloth. The plastic filter cloth can be woven or non-woven monofilament yarns, and shall meet these base requirements: thickness 20-60 mils, grab strength 90-120 lbs; and shall conform to ASTM D-1777 and ASTM D-1682.

Gravel filter blanket, when used, shall be designed by comparing particle sizes of the overlying material and the base material. Design criteria are available in Standard and Specification for Anchored Slope and Channel Stabilization on page 4.7.

Gabions

Gabions shall be made of hexagonal triple twist mesh with heavily galvanized steel wire. The maximum linear dimension of the mesh opening shall not exceed 4 ½ inches and the area of the mesh opening shall not exceed 10 square inches.

Gabions shall be fabricated in such a manner that the sides, ends, and lid can be assembled at the construction site into a rectangular basket of the specified sizes. Gabions shall be of single unit construction and shall be installed according to manufacturer's recommendations.

The area on which the gabion is to be installed shall be graded as shown on the drawings. Foundation conditions shall be the same as for placing rock riprap, and filter cloth shall be placed under all gabions. Where necessary, key, or tie, the structure into the bank to prevent undermining of the main gabion structure.

Maintenance

Once a riprap outlet has been installed, the maintenance needs are very low. It should be inspected after high flows for evidence of scour beneath the riprap or for dislodged rocks. Repairs should be made immediately.

Design Procedure

1. Investigate the downstream channel to assure that nonerosive velocities can be maintained.
2. Determine the tailwater condition at the outlet to establish which curve to use.
3. Use the appropriate chart with the design discharge to determine the riprap size and apron length required. It is noted that references to pipe diameters in the charts are based on full flow. For other than full pipe flow, the parameters of depth of flow and velocity must be used to adjust the design discharges.
4. Calculate apron width at the downstream end if a flare section is to be employed.

Design Examples are demonstrated in Appendix B.

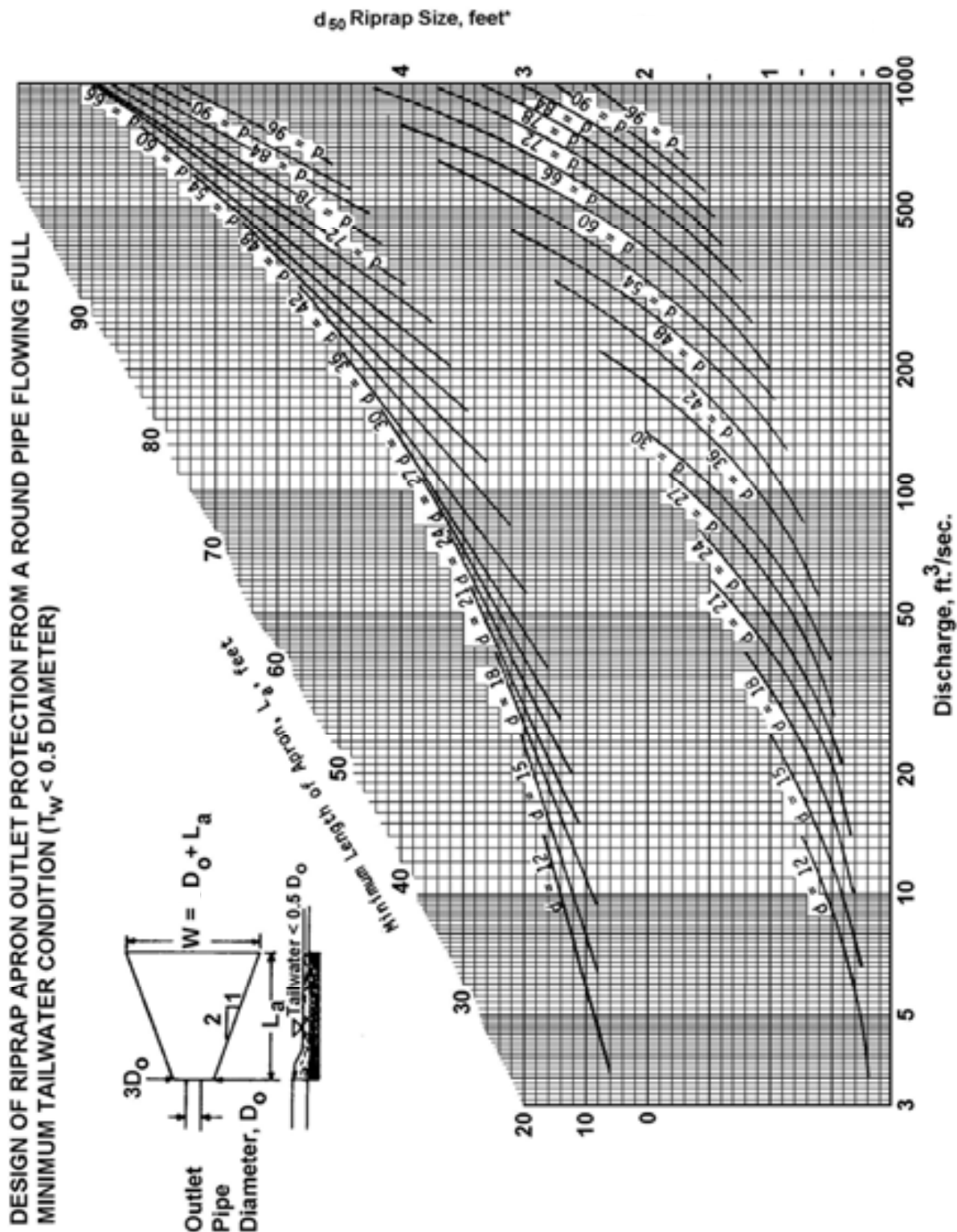
Construction Specifications

1. The subgrade for the filter, riprap, or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding undisturbed material.
2. The rock or gravel shall conform to the specified grad-

ing limits when installed respectively in the riprap or filter.

3. Filter cloth shall be protected from punching, cutting, or tearing. Any damage other than an occasional small hole shall be repaired by placing another piece of cloth over the damaged part or by completely replacing the cloth. All overlaps, whether for repairs or for joining two pieces of cloth shall be a minimum of one foot.
4. Rock for the riprap or gabion outlets may be placed by equipment. Both shall each be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The rock for riprap or gabion outlets shall be delivered and placed in a manner that will ensure that it is reasonably homogenous with the smaller rocks and spalls filling the voids between the larger rocks. Riprap shall be placed in a manner to prevent damage to the filter blanket or filter cloth. Hand placement will be required to the extent necessary to prevent damage to the permanent works.

Figure 3.16
Outlet Protection Design—Minimum Tailwater Condition Chart
(Design of Outlet Protection from a Round Pipe Flowing Full,
Minimum Tailwater Condition: $T_w < 0.5D_o$) (USDA - NRCS)



* For discharge velocities exceeding Maximum Allowable for Riprap indicated, increase d_{50} stone size and/or provide velocity reduction device.

Figure 3.17
Outlet Protection Design—Maximum Tailwater Condition Chart
(Design of Outlet Protection from a Round Pipe Flowing Full,
Maximum Tailwater Condition: $T_w \geq 0.5D_o$) (USDA - NRCS)

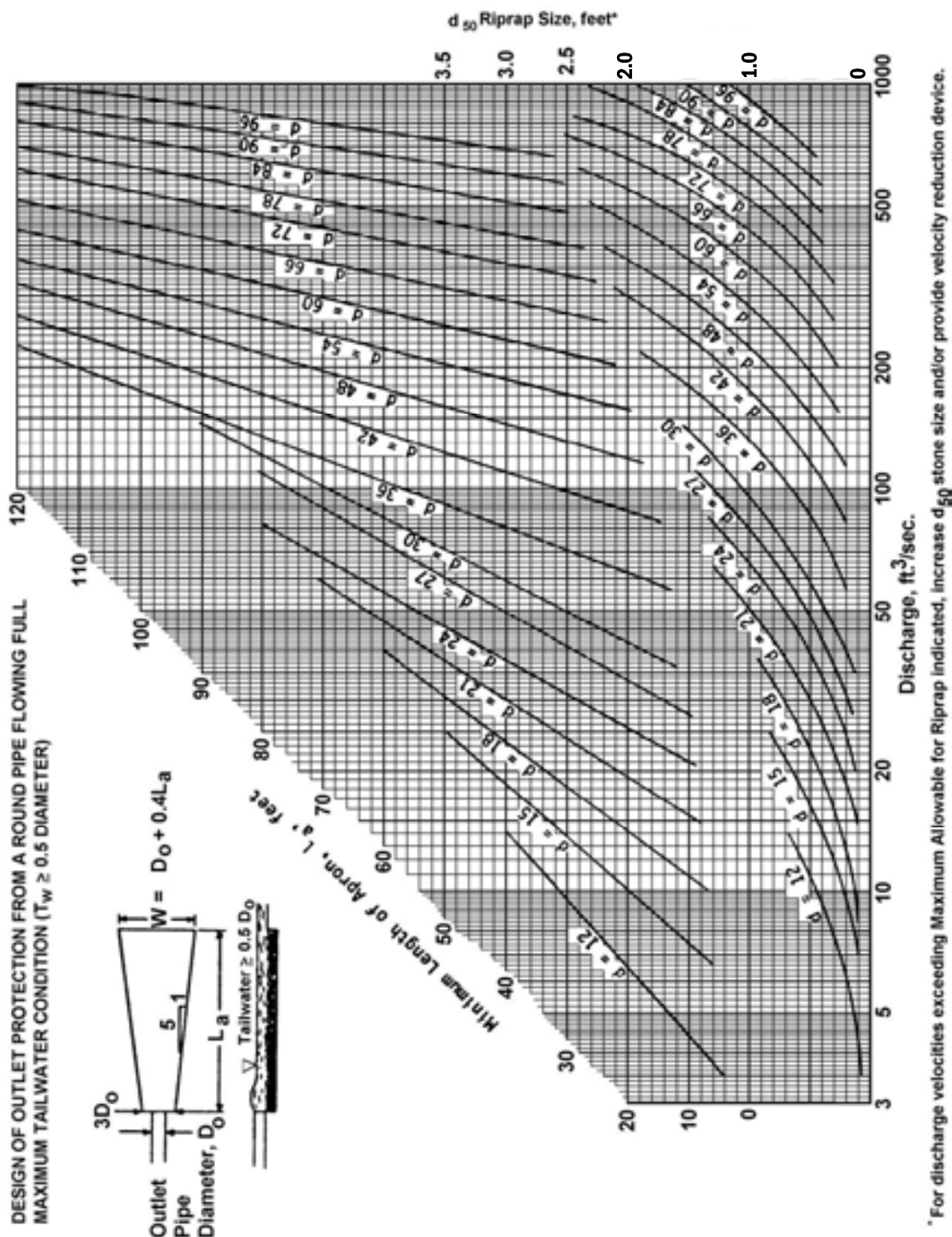


Figure 3.18
Riprap Outlet Protection Detail (1)

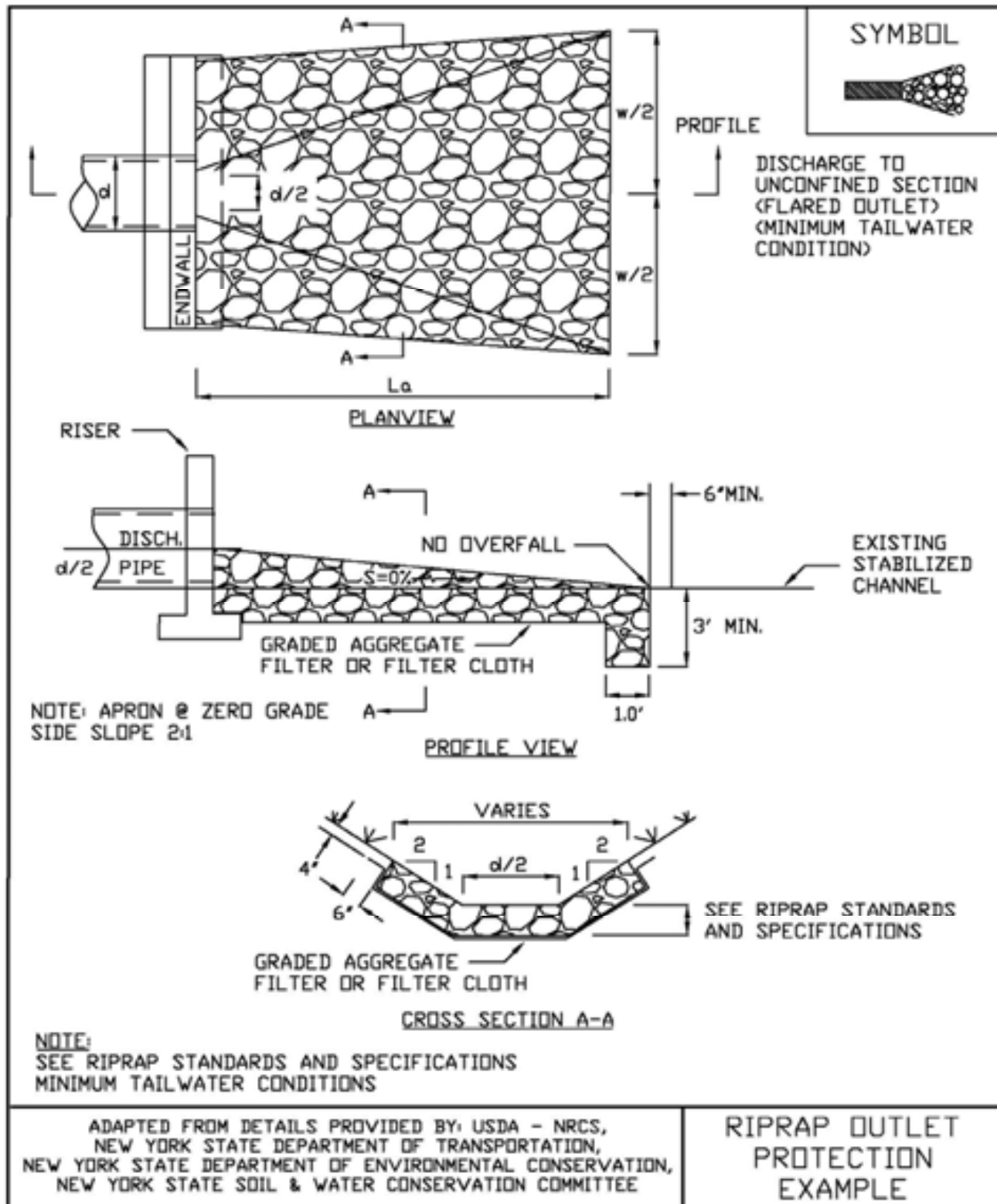


Figure 3.19
Riprap Outlet Protection Detail (2)

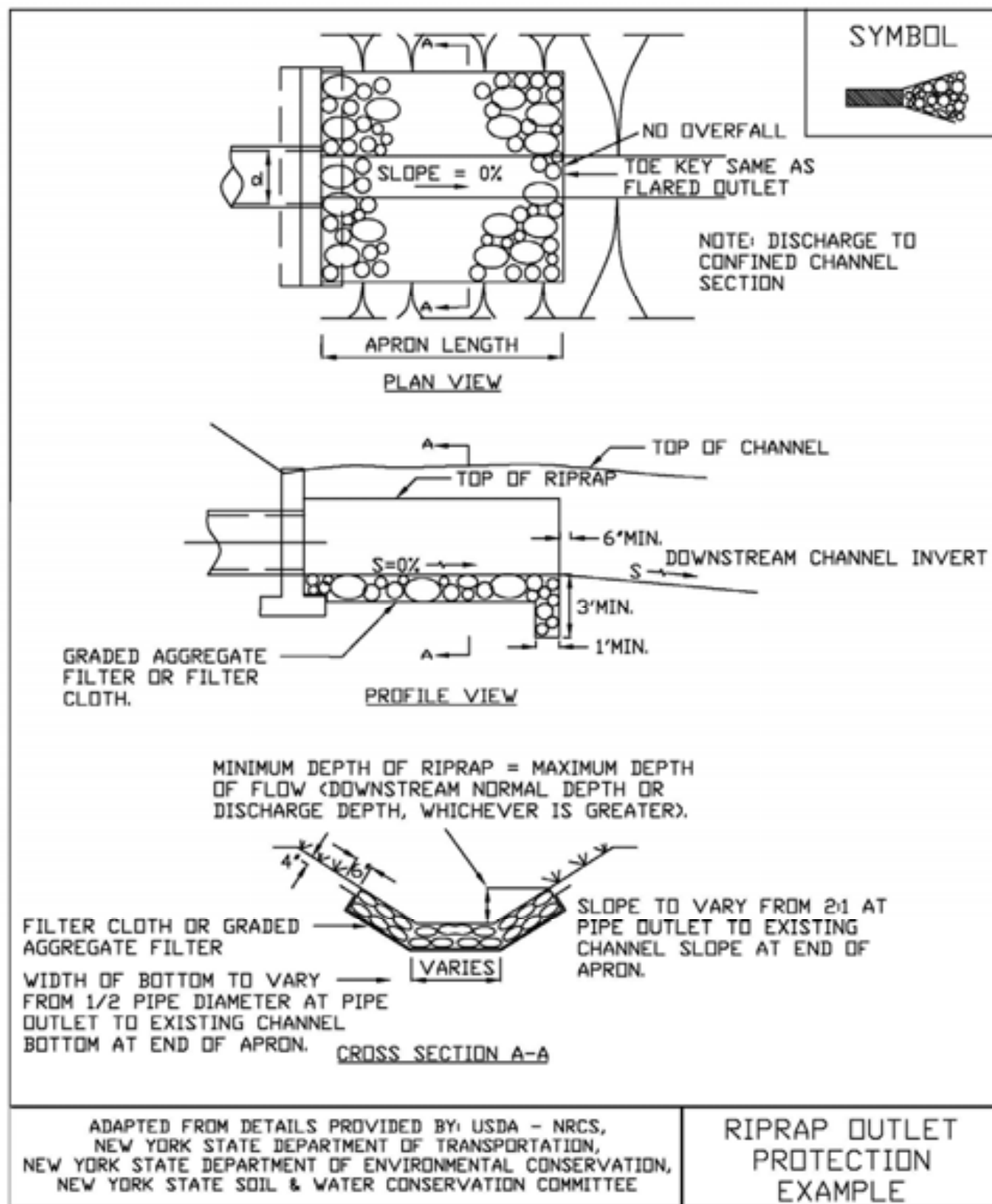
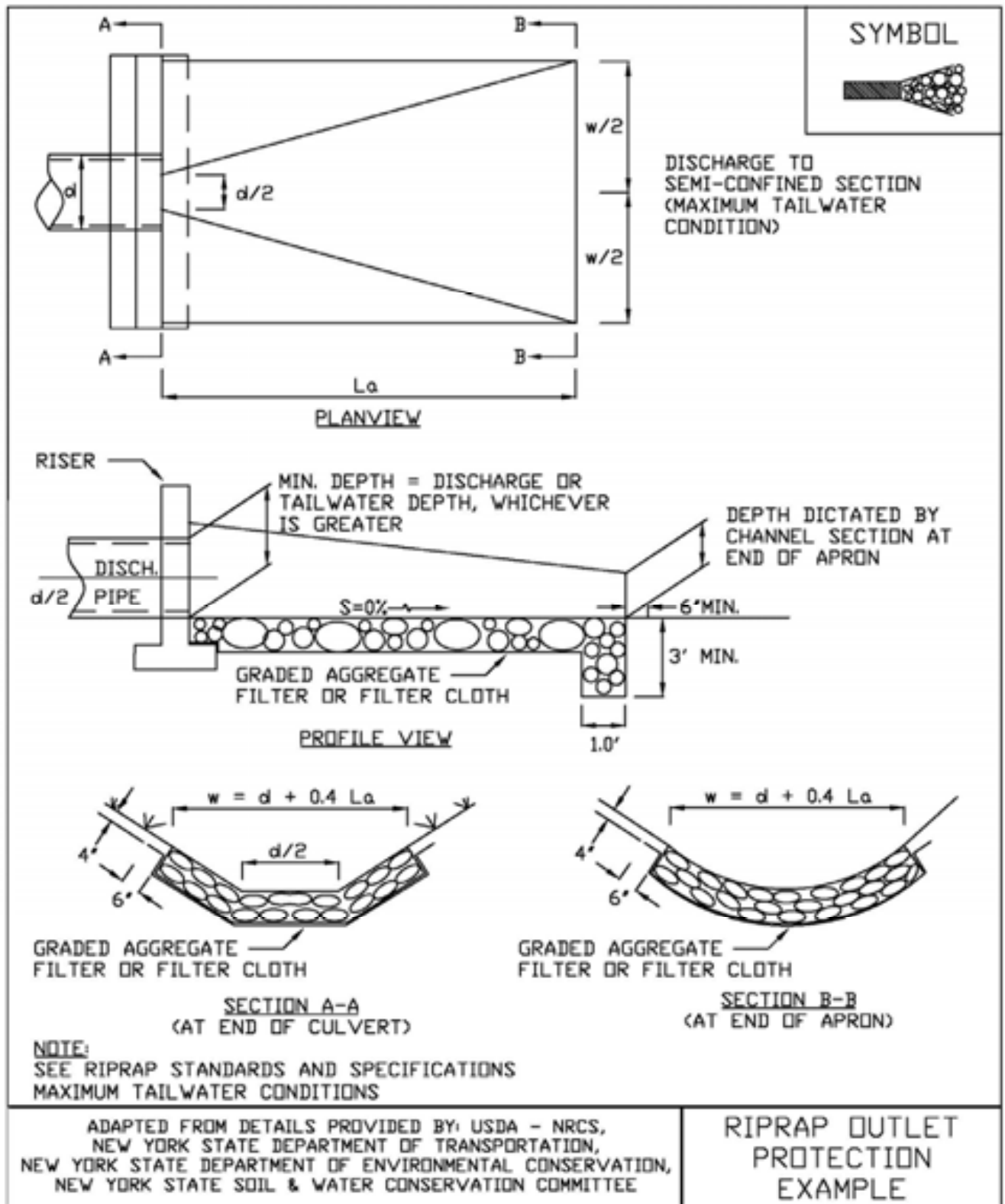


Figure 3.20
Riprap Outlet Protection Detail (3)



STANDARD AND SPECIFICATIONS FOR FERTILIZER APPLICATION



Definition & Scope

The **permanent** incorporation of fertilizer into the planting zone of the soil profile to provide nutrient amendments to the soil for vigorous support to plant and vegetation growth.

Conditions Where Practice Applies

This standard applies to all areas where permanent seeding, sodding, and plant establishment is required. All application of fertilizer shall be in accordance with Nutrient Runoff Law - ECL Article 17, Title 21. Phosphorus runoff poses a threat to water quality. Therefore, under New York Law, fertilizer containing phosphorus may only be applied to lawn or non-agricultural turf when:

1. A soil test indicates that additional phosphorus is needed for growth of that lawn or non-agricultural turf, or
2. The fertilizer is used for newly established lawn or non-agricultural turf during the first growing season.

For projects located within watersheds where enhanced phosphorus removal standards are required as part of its post-construction stormwater management plan, use of any fertilizer containing more than 0.67 percent phosphate (P_2O_5) content will be done only with a valid soil test demonstrating the need for that formulation.

Design Criteria

Fertilizer is sold with an analysis printed on the tag or bag shown as three numbers separated by a dash, such as 5-10-5. The first number is the percent of the total weight of the bag that is nitrogen (N), the second is the percent of

phosphate (phosphorus, P), and the third is the percent of potash (potassium, K). Other elements are sometimes included and are listed with these three basic components.

For example a 40 lb bag of 5-10-5 fertilizer contains 5% of 40 lbs of Nitrogen which equals 2 lbs. There is 10% of 40 lbs of phosphate (phosphorus) which equals 4 lbs, and there is 5% of potash (potassium), another 2 lbs., for a total of 8 lbs of active fertilizer in the 40 lb bag. The rest is filler to aid in spreading the material over the area to be treated.

Specify the design fertilizer mix and application rates based on the results of the soil tests.

Specifications

1. In no case shall fertilizer be applied between December 1 and April 1 annually.
2. Fertilizer shall not be spread within 20 feet of a surface water.
3. Any fertilizer falling or spilled into impervious surface areas such as parking lots, roadways, and sidewalks should be immediately contained and legally applied or placed in an appropriate container.
4. Incorporate the fertilizer, and lime if specified, into the top 2-4 inches of the topsoil or soil profile.
5. When applying fertilizer by hydro seeding care should be taken to apply mix only to seed bed areas at an appropriate flow rate to prevent erosion and spraying onto impervious areas.



STANDARD AND SPECIFICATIONS FOR LANDGRADING



Definition & Scope

Permanent reshaping of the existing land surface by grading in accordance with an engineering topographic plan and specification to provide for erosion control and vegetative establishment on disturbed, reshaped areas.

Design Criteria

The grading plan should be based upon the incorporation of building designs and street layouts that fit and utilize existing topography and desirable natural surrounding to avoid extreme grade modifications. Information submitted must provide sufficient topographic surveys and soil investigations to determine limitations that must be imposed on the grading operation related to slope stability, effect on adjacent properties and drainage patterns, measures for drainage and water removal, and vegetative treatment, etc.

Many municipalities and counties have regulations and design procedures already established for land grading and cut and fill slopes. Where these requirements exist, they shall be followed.

The plan must show existing and proposed contours of the area(s) to be graded. The plan shall also include practices for erosion control, slope stabilization, safe disposal of runoff water and drainage, such as waterways, lined ditches, reverse slope benches (include grade and cross section), grade stabilization structures, retaining walls, and surface and subsurface drains. The plan shall also include phasing of these practices. The following shall be incorporated into the plan:

1. Provisions shall be made to safely convey surface runoff to storm drains, protected outlets, or to stable water courses to ensure that surface runoff will not

damage slopes or other graded areas; see standards and specifications for Grassed Waterway, Diversion, or Grade Stabilization Structure.

2. Cut and fill slopes that are to be stabilized with grasses shall not be steeper than 2:1. When slopes exceed 2:1, special design and stabilization consideration are required and shall be adequately shown on the plans. (Note: Where the slope is to be mowed, the slope should be no steeper than 3:1, although 4:1 is preferred because of safety factors related to mowing steep slopes.)
3. Reverse slope benches or diversion shall be provided whenever the vertical interval (height) of any 2:1 slope exceeds 20 feet; for 3:1 slope it shall be increased to 30 feet and for 4:1 to 40 feet. Benches shall be located to divide the slope face as equally as possible and shall convey the water to a stable outlet. Soils, seeps, rock outcrops, etc., shall also be taken into consideration when designing benches.
 - A. Benches shall be a minimum of six feet wide to provide for ease of maintenance.
 - B. Benches shall be designed with a reverse slope of 6:1 or flatter to the toe of the upper slope and with a minimum of one foot in depth. Bench gradient to the outlet shall be between 2 percent and 3 percent, unless accompanied by appropriate design and computations.
 - C. The flow length within a bench shall not exceed 800 feet unless accompanied by appropriate design and computations; see Standard and Specifications for Diversion on page 3.9
4. Surface water shall be diverted from the face of all cut and/or fill slopes by the use of diversions, ditches and swales or conveyed downslope by the use of a designed structure, except where:
 - A. The face of the slope is or shall be stabilized and the face of all graded slopes shall be protected from surface runoff until they are stabilized.
 - B. The face of the slope shall not be subject to any concentrated flows of surface water such as from natural drainage ways, graded ditches, downspouts, etc.
 - C. The face of the slope will be protected by anchored stabilization matting, sod, gravel, riprap, or other stabilization method.

5. Cut slopes occurring in ripable rock shall be serrated as shown in Figure 4.9 on page 4.26. The serrations shall be made with conventional equipment as the excavation is made. Each step or serration shall be constructed on the contour and will have steps cut at nominal two-foot intervals with nominal three-foot horizontal shelves. These steps will vary depending on the slope ratio or the cut slope. The nominal slope line is 1 ½: 1. These steps will weather and act to hold moisture, lime, fertilizer, and seed thus producing a much quicker and longer-lived vegetative cover and better slope stabilization. Overland flow shall be diverted from the top of all serrated cut slopes and carried to a suitable outlet.
6. Subsurface drainage shall be provided where necessary to intercept seepage that would otherwise adversely affect slope stability or create excessively wet site conditions.
7. Slopes shall not be created so close to property lines as to endanger adjoining properties without adequately protecting such properties against sedimentation, erosion, slippage, settlement, subsidence, or other related damages.
8. Fill material shall be free of brush, rubbish, rocks, logs, stumps, building debris, and other objectionable material. It should be free of stones over two (2) inches in diameter where compacted by hand or mechanical tampers or over eight (8) inches in diameter where compacted by rollers or other equipment. Frozen material shall not be placed in the fill nor shall the fill material be placed on a frozen foundation.
9. Stockpiles, borrow areas, and spoil shall be shown on the plans and shall be subject to the provisions of this Standard and Specifications.
10. All disturbed areas shall be stabilized structurally or vegetatively in compliance with the Permanent Construction Area Planting Standard on page 4.42.
4. Areas to be filled shall be cleared, grubbed, and stripped of topsoil to remove trees, vegetation, roots, or other objectionable material.
5. Areas that are to be topsoiled shall be scarified to a minimum depth of four inches prior to placement of topsoil.
6. All fills shall be compacted as required to reduce erosion, slippage, settlement, subsidence, or other related problems. Fill intended to support buildings, structures, and conduits, etc., shall be compacted in accordance with local requirements or codes.
7. All fill shall be placed and compacted in layers not to exceed 9 inches in thickness.
8. Except for approved landfills or nonstructural fills, fill material shall be free of frozen particles, brush, roots, sod, or other foreign objectionable materials that would interfere with, or prevent, construction of satisfactory fills.
9. Frozen material or soft, mucky or highly compressible materials shall not be incorporated into fill slopes or structural fills.
10. Fill shall not be placed on saturated or frozen surfaces.
11. All benches shall be kept free of sediment during all phases of development.
12. Seeps or springs encountered during construction shall be handled in accordance with the Standard and Specification for Subsurface Drain on page 3.48 or other approved methods.
13. All graded areas shall be permanently stabilized immediately following finished grading.
14. Stockpiles, borrow areas, and spoil areas shall be shown on the plans and shall be subject to the provisions of this Standard and Specifications.

Construction Specifications

See Figures 4.9 and 4.10 for details.

1. All graded or disturbed areas, including slopes, shall be protected during clearing and construction in accordance with the erosion and sediment control plan until they are adequately stabilized.
2. All erosion and sediment control practices and measures shall be constructed, applied and maintained in accordance with the erosion and sediment control plan and these standards.
3. Topsoil required for the establishment of vegetation shall be stockpiled in amount necessary to complete finished grading of all exposed areas.



Figure 4.9
Typical Section of Serrated Cut Slope

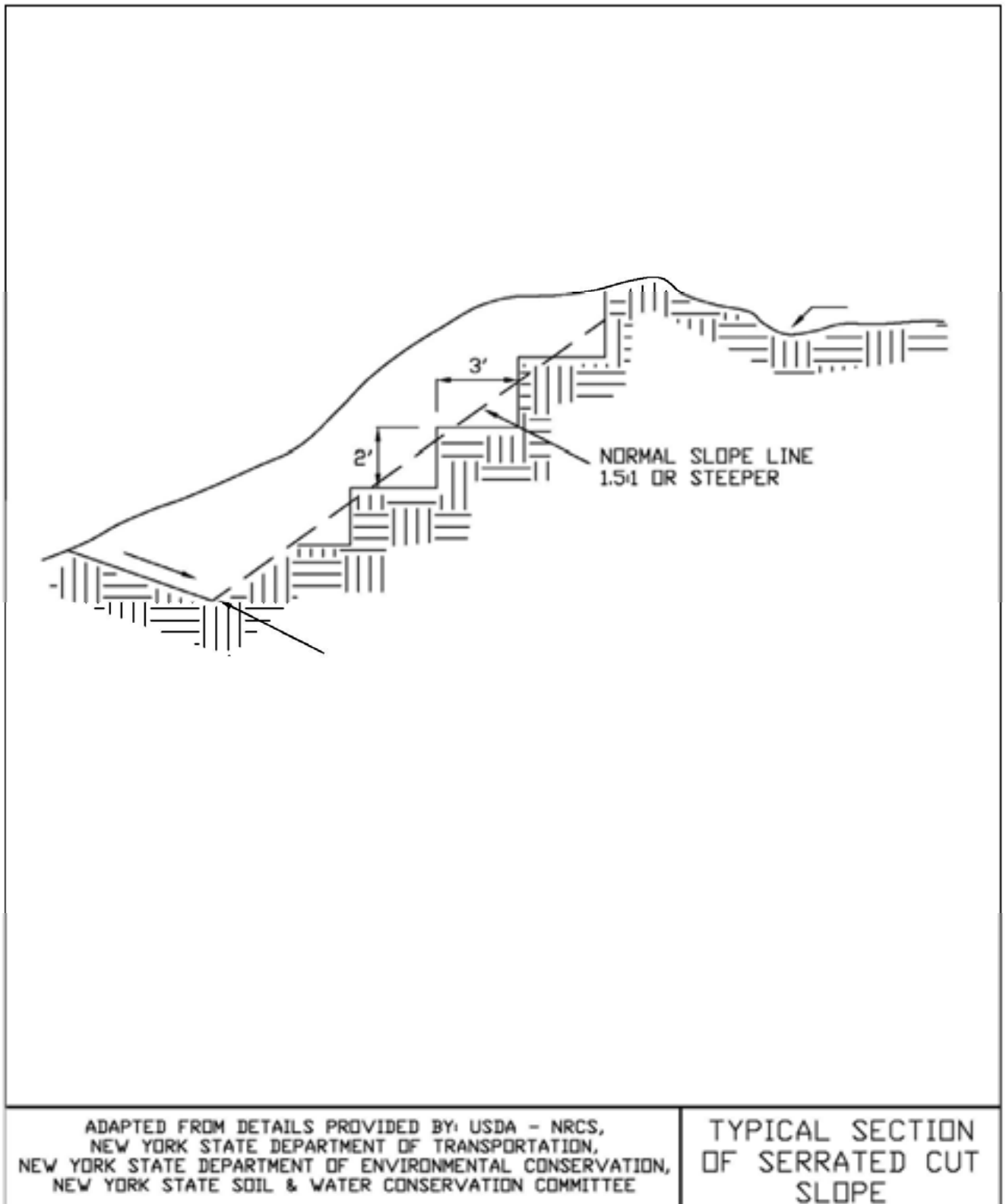


Figure 4.10
Landgrading

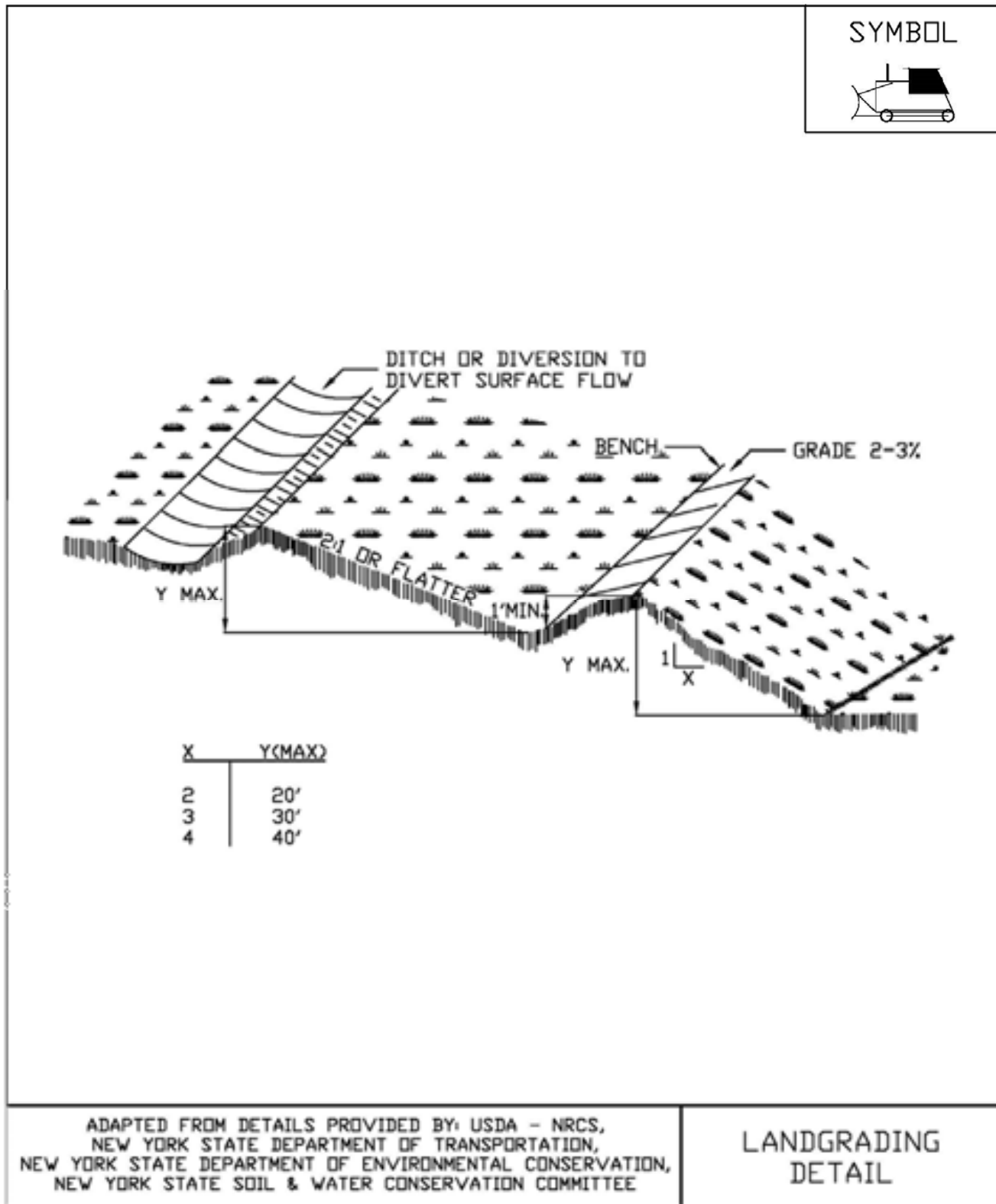


Figure 4.11
Landgrading - Construction Specifications

<u>CONSTRUCTION SPECIFICATIONS</u>	
1. ALL GRADED OR DISTURBED AREAS INCLUDING SLOPES SHALL BE PROTECTED DURING CLEARING AND CONSTRUCTION IN ACCORDANCE WITH THE APPROVED EROSION AND SEDIMENT CONTROL PLAN UNTIL THEY ARE PERMANENTLY STABILIZED. 2. ALL SEDIMENT CONTROL PRACTICES AND MEASURES SHALL BE CONSTRUCTED, APPLIED AND MAINTAINED IN ACCORDANCE WITH THE APPROVED EROSION AND SEDIMENT CONTROL PLAN. 3. TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN AMOUNT NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS. 4. AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED, AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL. 5. AREAS WHICH ARE TO BE TOPSOILED SHALL BE SCARIFIED TO A MINIMUM DEPTH OF FOUR INCHES PRIOR TO PLACEMENT OF TOPSOIL. 6. ALL FILLS SHALL BE COMPACTED AS REQUIRED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS. FILL INTENDED TO SUPPORT BUILDINGS, STRUCTURES AND CONDUITS, ETC. SHALL BE COMPACTED IN ACCORDANCE WITH LOCAL REQUIREMENTS OR CODES. 7. ALL FILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT TO EXCEED 9 INCHES IN THICKNESS. 8. EXCEPT FOR APPROVED LANDFILLS, FILL MATERIAL SHALL BE FREE OF FROZEN PARTICLES, BRUSH, ROOTS, SOD, OR OTHER FOREIGN OR OTHER OBJECTIONABLE MATERIALS THAT WOULD INTERFERE WITH OR PREVENT CONSTRUCTION OF SATISFACTORY FILLS. 9. FROZEN MATERIALS OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIALS SHALL NOT BE INCORPORATED IN FILLS. 10. FILL SHALL NOT BE PLACED ON SATURATED OR FROZEN SURFACES. 11. ALL BENCHES SHALL BE KEPT FREE OF SEDIMENT DURING ALL PHASES OF DEVELOPMENT. 12. SEEPS OR SPRINGS ENCOUNTERED DURING CONSTRUCTION SHALL BE HANDLED IN ACCORDANCE WITH THE STANDARD AND SPECIFICATION FOR SUBSURFACE DRAIN OR OTHER APPROVED METHOD. 13. ALL GRADED AREAS SHALL BE PERMANENTLY STABILIZED IMMEDIATELY FOLLOWING FINISHED GRADING. 14. STOCKPILES, BORROW AREAS AND SPOIL AREAS SHALL BE SHOWN ON THE PLANS AND SHALL BE SUBJECT TO THE PROVISIONS OF THIS STANDARD AND SPECIFICATION.	
ADAPTED FROM DETAILS PROVIDED BY: USDA - NRCS, NEW YORK STATE DEPARTMENT OF TRANSPORTATION, NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION, NEW YORK STATE SOIL & WATER CONSERVATION COMMITTEE	LANDGRADING SPECIFICATIONS

STANDARD AND SPECIFICATIONS FOR MULCHING



Definition and Scope

Applying coarse plant residue or chips, or other suitable materials, to cover the soil surface to provide initial erosion control while a seeding or shrub planting is establishing. Mulch will conserve moisture and modify the surface soil temperature and reduce fluctuation of both. Mulch will prevent soil surface crusting and aid in weed control. Mulch can also be used alone for temporary stabilization in non-growing months. Use of stone as a mulch could be more permanent and should not be limited to non-growing months.

Conditions Where Practice Applies

On soils subject to erosion and on new seedlings and shrub plantings. Mulch is useful on soils with low infiltration rates by retarding runoff.

Criteria

Site preparation prior to mulching requires the installation of necessary erosion control or water management practices and drainage systems.

Slope, grade and smooth the site to fit needs of selected mulch products.

Remove all undesirable stones and other debris to meet the needs of the anticipated land use and maintenance required.

Apply mulch after soil amendments and planting is accomplished or simultaneously if hydroseeding is used.

Select appropriate mulch material and application rate or material needs. Hay mulch shall not be used in wetlands or in areas of permanent seeding. Clean straw mulch is preferred alternative in wetland application. Determine local availability.

Select appropriate mulch anchoring material.

NOTE: The best combination for grass/legume establishment is straw (cereal grain) mulch applied at 2 ton/acre (90 lbs./1000sq.ft.) and anchored with wood fiber mulch (hydromulch) at 500 – 750 lbs./acre (11 – 17 lbs./1000 sq. ft.). The wood fiber mulch must be applied through a hydroseeder immediately after mulching.



Table 4.2
Guide to Mulch Materials, Rates, and Uses

Mulch Material	Quality Standards	per 1000 Sq. Ft.	per Acre	Depth of Application	Remarks
Wood chips or shavings	Air-dried. Free of objectionable coarse material	500-900 lbs.	10-20 tons	2-7"	Used primarily around shrub and tree plantings and recreation trails to inhibit weed competition. Resistant to wind blowing. Decomposes slowly.
Wood fiber cellulose (partly digested wood fibers)	Made from natural wood usually with green dye and dispersing agent	50 lbs.	2,000 lbs.	—	Apply with hydromulcher. No tie down required. Less erosion control provided than 2 tons of hay or straw.
Gravel, Crushed Stone or Slag	Washed; Size 2B or 3A—1 1/2"	9 cu. yds.	405 cu. yds.	3"	Excellent mulch for short slopes and around plants and ornaments. Use 2B where subject to traffic. (Approximately 2,000 lbs./cu. yd.). Frequently used over filter fabric for better weed control.
Hay or Straw	Air-dried; free of undesirable seeds & coarse materials	90-100 lbs. 2-3 bales	2 tons (100-120 bales)	cover about 90% surface	Use small grain straw where mulch is maintained for more than three months. Subject to wind blowing unless anchored. Most commonly used mulching material. Provides the best micro-environment for germinating seeds.
Jute twisted yarn	Undyed, unbleached plain weave. Warp 78 ends/yd., Weft 41 ends/yd. 60-90 lbs./roll	48" x 50 yds. or 48" x 75 yds.	—	—	Use without additional mulch. Tie down as per manufacturers specifications. Good for center line of concentrated water flow.
Excelsior wood fiber mats	Interlocking web of excelsior fibers with photodegradable plastic netting	4' x 112.5' or 8' x 112.5'.	—	—	Use without additional mulch. Excellent for seeding establishment. Anchor as per manufacturers specifications. Approximately 72 lbs./roll for excelsior with plastic on both sides. Use two sided plastic for centerline of waterways.
Straw or coconut fiber, or combination	Photodegradable plastic net on one or two sides	Most are 6.5 ft. x 3.5 ft.	81 rolls	—	Designed to tolerate higher velocity water flow, centerlines of waterways, 60 sq. yds. per roll.

Table 4.3
Mulch Anchoring Guide

Anchoring Method or Material	Kind of Mulch to be Anchored	How to Apply
1. Peg and Twine	Hay or straw	After mulching, divide areas into blocks approximately 1 sq. yd. in size. Drive 4-6 pegs per block to within 2" to 3" of soil surface. Secure mulch to surface by stretching twine between pegs in criss-cross pattern on each block. Secure twine around each peg with 2 or more tight turns. Drive pegs flush with soil. Driving stakes into ground tightens the twine.
2. Mulch netting	Hay or straw	Staple the light-weight paper, jute, wood fiber, or plastic nettings to soil surface according to manufacturer's recommendations. Should be biodegradable. Most products are not suitable for foot traffic.
3. Wood cellulose fiber	Hay or straw	Apply with hydroseeder immediately after mulching. Use 500 lbs. wood fiber per acre. Some products contain an adhesive material ("tackifier"), possibly advantageous.
4. Mulch anchoring tool	Hay or straw	Apply mulch and pull a mulch anchoring tool (blunt, straight discs) over mulch as near to the contour as possible. Mulch material should be "tucked" into soil surface about 3".
5. Tackifier	Hay or straw	Mix and apply polymeric and gum tackifiers according to manufacturer's instructions. Avoid application during rain. A 24-hour curing period and a soil temperature higher than 45 ⁰ Fahrenheit are required.

STANDARD AND SPECIFICATIONS FOR RETAINING WALLS



Definition & Scope

A **permanent** structural wall constructed and located to prevent soil movement by retaining soil in place and preventing slope failures and movement of material down steep slopes.

Conditions Where Practice Applies

A retaining wall may be used where site constraints will not allow slope shaping and seeding to stabilize an area. Slope areas that demonstrate seepage problems or experience erosive conditions at the toe can utilize retaining walls to help stabilize these areas. Retaining walls can be built from mortared block or stone, cast-in-place concrete, railroad ties, gabions, and more recently, precast concrete modular units and segmented walls that form a gravity retaining wall (see Figure 4.16 and 4.17). These precast units allow for ease and quickness of installation while their granular backfill provides drainage. Selection of materials and type of wall should be based on hazard potential, load conditions, soil parameters, groundwater conditions, site constraints, and aesthetics.

Design Criteria

The design of any retaining wall structure must address the aspects of foundation bearing capacity, sliding, overturning, drainage and loading systems. **These are complex systems that should be designed by a licensed professional engineer.**

Bearing Capacity – A minimum factor of safety of 1.5 should be maintained as the ratio of the ultimate bearing capacity to the designed unit loading. Spread footers and

other methods may be used to meet factor requirements.

Sliding – A minimum factor of 2.0 should be maintained against sliding. This factor can be reduced to 1.5 when passive pressures on the front of the wall are ignored.

Overturning – A minimum factor of safety of 1.5 should be used as the ratio of the resisting moment (that which tends to keep the wall in place) to the overturning moment.

Drainage – Unless adequate provisions are made to control both surface and groundwater behind the retaining wall, a substantial increase in active pressures tending to slide or overturn the wall will result. When backfill is sloped down to a retaining wall, surface drainage should be provided. Drainage systems with adequate outlets should be provided behind retaining walls that are placed in cohesive soils. Drains should be graded or protected by filters so soil material will not move through the drainfill.

Load systems – Several different loads or combination of loads need to be considered when designing a retaining wall. The minimum load is the level backfill that the wall is being constructed to retain. Its unit weight will vary depending on its composition.

Additional loads such as line loads, surcharge loads, or slope fills, will add to make the composite design load system for the wall.

Construction Specifications

Concrete Walls

1. Foundation will be prepared by excavating to the lines and grades shown on the drawings and removing all objectionable material.
2. Subgrade will be compacted and kept moist at least 2 hours prior to placement of concrete.
3. Steel reinforcing will be in accordance with the schedule on the drawings and kept free of rust, scale, or dirt.
4. Exposed edges will be chamfered $\frac{3}{4}$ inches.
5. Drainfill will meet the gradations shown on the drawings.

6. Weep holes will be provided as drain outlets as shown on the drawings.



7. Concrete will be poured and cured in accordance with American Concrete Institute (ACI) specifications.

Precast Units

1. Foundation will be prepared by excavating to the lines and grades shown on the drawings.
2. Subgrade will be compacted and trimmed to receive the leveling beam.
3. Precast units will be placed in accordance with the manufacturers recommendation.
4. Granular fill placed in the precast bins shall be placed in 3-foot lifts, leveled off and compacted with a plate vibrator.

Segmented Walls

1. Foundation will be prepared by excavating to the lines and grades shown on the drawings.
2. Sub-grade will be compacted and screeded to form the base for the first course of wall units.
3. Units will be placed in accordance with the manufacturers recommendations, with each succeeding lift anchored and pinned as specified.
4. Granular fill will be placed behind the segmented wall to provide drainage. It shall be compacted with a plate vibrator. A drainage outlet will be provided as specified on the construction drawings.

Gabions

1. Foundation will be prepared by excavating to the lines and grades shown on the drawings.
2. Subgrade will be compacted and leveled to receive first layer of gabions. The first row will be keyed into the existing grade at the toe, a minimum of 1.5 feet.
3. Gabions will be placed according to the manufacturers recommendations.
4. Gabions will be filled with stone or crushed rock from 4 to 8 inches in diameter.



Non-Mortared Stone Walls

1. Foundation will be prepared by excavating to the lines and grade shown on the drawings.
2. Subgrade will be compacted and leveled to receive monolithic stone. First row will be placed 1.0 feet below design toe elevation.
3. Stone will be placed horizontally with long dimension parallel to face of wall except at return ends.
4. Maximum of 3 lifts of stone each approximately 2' thick without pinning. Where stones do not fit in good contact, pinning with two steel #8 re-bar dowels is required.
5. Backside of stone will be filled with a minimum of 2' of #1 and #2 stone between filter fabric against parent soil and rock to provide drainage.



Figure 4.16
Typical Retaining Wall Examples
(Schematic only - not to be used for design)

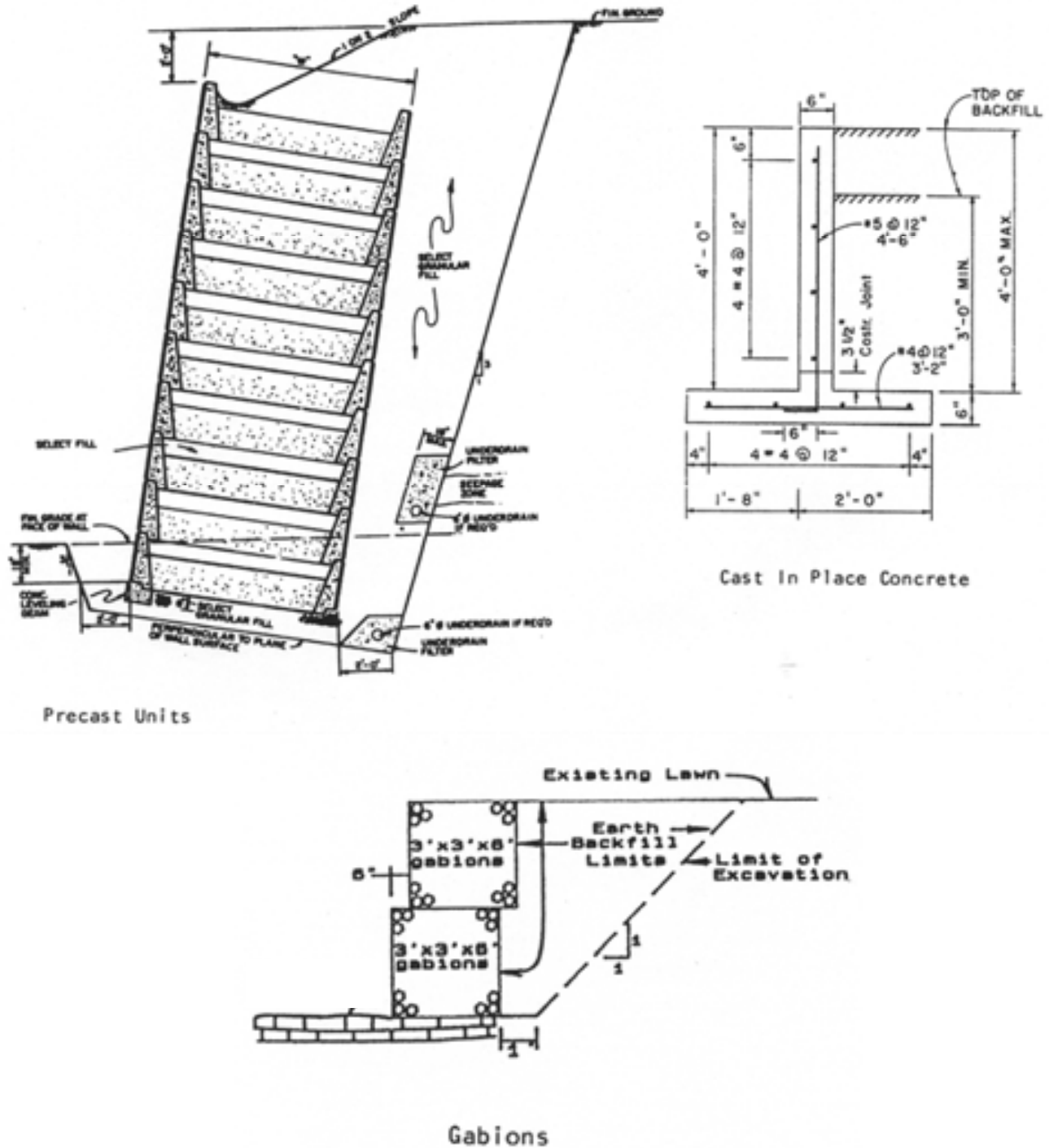
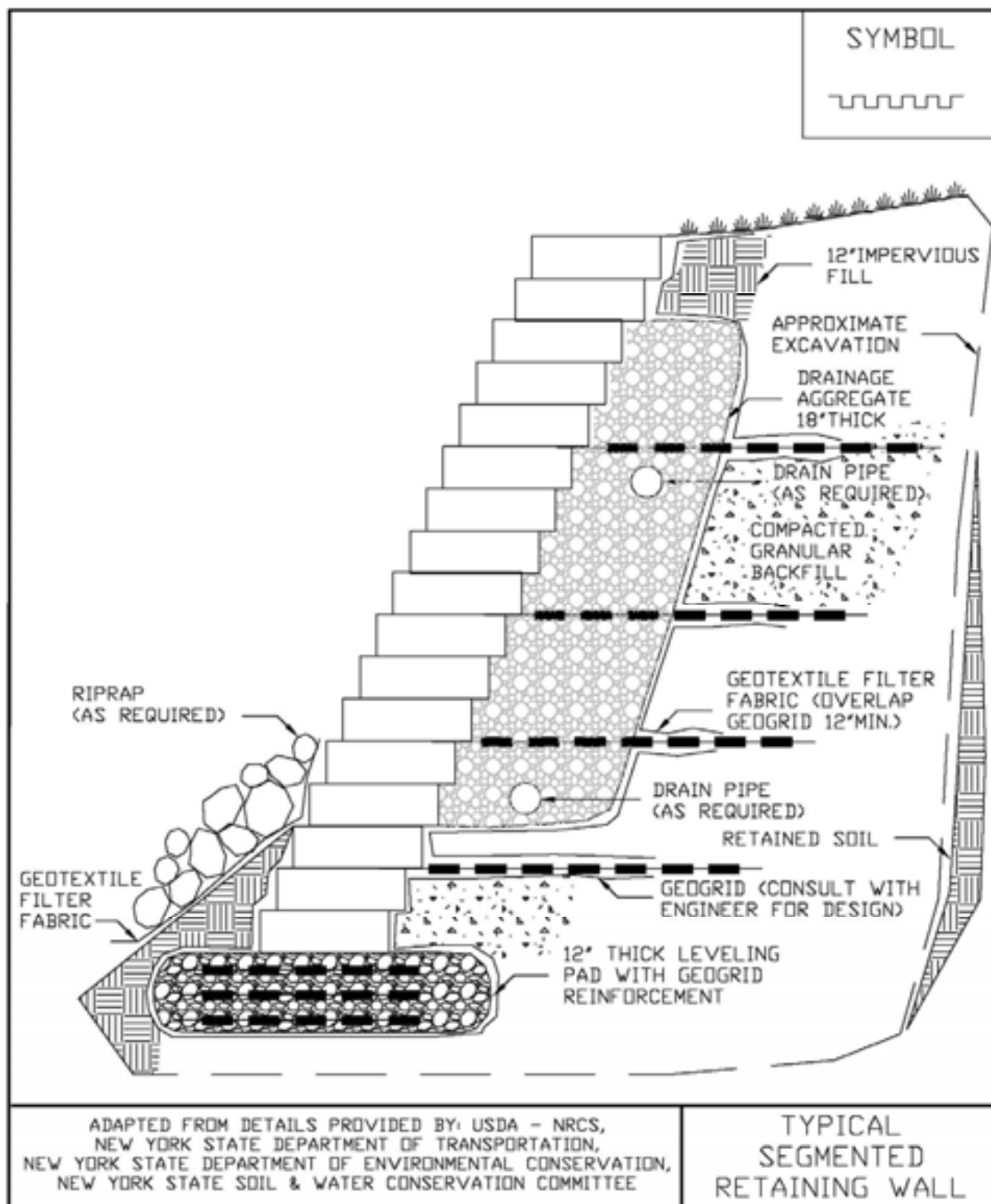


Figure 4.17
Typical Segmented Retaining Wall Example
(Schematic only - not to be used for design)



STANDARD AND SPECIFICATIONS FOR TOPSOILING



Definition & Scope

Spreading a specified quality and quantity of topsoil materials on graded or constructed subsoil areas to provide acceptable plant cover growing conditions, thereby reducing erosion; to reduce irrigation water needs; and to reduce the need for nitrogen fertilizer application.

Conditions Where Practice Applies

Topsoil is applied to subsoils that are droughty (low available moisture for plants), stony, slowly permeable, salty or extremely acid. It is also used to backfill around shrub and tree transplants. This standard does not apply to wetland soils.

Design Criteria

1. Preserve existing topsoil in place where possible, thereby reducing the need for added topsoil.
2. Conserve by stockpiling topsoil and friable fine textured subsoils that must be stripped from the excavated site and applied after final grading where vegetation will be established. Topsoil stockpiles must be stabilized. Stockpile surfaces can be stabilized by vegetation, geotextile or plastic covers. This can be aided by orientating the stockpile lengthwise into prevailing winds.
3. Refer to USDA Natural Resource Conservation Service soil surveys or soil interpretation record sheets for further soil texture information for selecting appropriate design topsoil depths.

Site Preparation

1. As needed, install erosion and sediment control practices such as diversions, channels, sediment traps, and stabilizing measures, or maintain if already installed.
2. Complete rough grading and final grade, allowing for depth of topsoil to be added.
3. Scarify all compact, slowly permeable, medium and fine textured subsoil areas. Scarify at approximately right angles to the slope direction in soil areas that are steeper than 5 percent. Areas that have been overly compacted shall be decompacted in accordance with the Soil Restoration Standard.
4. Remove refuse, woody plant parts, stones over 3 inches in diameter, and other litter.

Topsoil Materials

1. Topsoil shall have at least 6 percent by weight of fine textured stable organic material, and no greater than 20 percent. Muck soil shall not be considered topsoil.
2. Topsoil shall have not less than 20 percent fine textured material (passing the NO. 200 sieve) and not more than 15 percent clay.
3. Topsoil treated with soil sterilants or herbicides shall be so identified to the purchaser.
4. Topsoil shall be relatively free of stones over 1 1/2 inches in diameter, trash, noxious weeds such as nut sedge and quackgrass, and will have less than 10 percent gravel.
5. Topsoil containing soluble salts greater than 500 parts per million shall not be used.
6. Topsoil may be manufactured as a mixture of a mineral component and organic material such as compost.

Application and Grading

1. Topsoil shall be distributed to a uniform depth over the area. It shall not be placed when it is partly frozen, muddy, or on frozen slopes or over ice, snow, or standing water puddles.
2. Topsoil placed and graded on slopes steeper than 5 percent shall be promptly fertilized, seeded, mulched, and stabilized by "tracking" with suitable equipment.
3. Apply topsoil in the amounts shown in Table 4.7 below:

Table 4.7 - Topsoil Application Depth		
Site Conditions	Intended Use	Minimum Topsoil Depth
1. Deep sand or loamy sand	Mowed lawn	6 in.
	Tall legumes, unmowed	2 in.
	Tall grass, unmowed	1 in.
2. Deep sandy loam	Mowed lawn	5 in.
	Tall legumes, unmowed	2 in.
	Tall grass, unmowed	none
3. Six inches or more: silt loam, clay loam, loam, or silt	Mowed lawn	4 in.
	Tall legumes, unmowed	1 in.
	Tall grass, unmowed	1 in.

STANDARD AND SPECIFICATIONS FOR TREES, SHRUBS, AND VINES



Definition & Scope

Establishing trees, shrubs, and vines or selectively reducing stand density and trimming woody plants to protect the soil and plant resources, improve an area for recreation and increase the attractiveness and usefulness of areas.

Conditions Where Practice Applies

On any area planned for recreation or landscape use such as yard areas, leisure areas, picnic areas, and park lands providing outdoor recreational opportunities.

Criteria and Specifications

1. Planting nursery stock

A. Select species to serve the intended purpose. See Appendix G, Table G.1, "Trees Suitable for Landscape and Conservation Plantings in New York." Where planting of trees is to be done in recreation areas, use those species resistant to compaction listed in Table G.2, "Susceptibility of Tree Species to Compaction" whenever possible.

B. Plant Materials

1) Plants shall conform to the species, variety, size, number, and conditions as stated in a conservation plan or on a plant list shown on landscape drawings. "American Standard for Nursery Stock," by American Association of Nurserymen, shall be used to develop the plant list for landscape drawings and to check quality of plant materials.

2) Durable, legible labels with the scientific and common name and cultivar shall be securely

attached to plants, bundles of seedlings, containers, and/or flats.

C. Plant Protection

Prior to delivery, the trunk, branches, and foliage of the plants shall be sprayed with non-toxic antidesiccant, applied according to the manufacturer's recommendations. This does not apply to state nursery seedlings.

D. Planting Time

Deciduous trees and shrubs: April 1 to June 1 and October 15 to December 15. Evergreen trees and shrubs: April 1 to June 1 and September 1 to November 15.

E. Spacing

Plant all trees and shrubs well back from buildings to allow for mature crown size. The following are guides for planning:

Large Trees	50-60 feet apart
Small Trees	20-30 feet apart
Columnar Species	6-8 feet apart
Hedges	1-4 feet apart
Shrubs	For clumps, plan spacing so mature shrubs will be touching or overlapping by only 1 or 2 feet

F. Site Preparation

1) Individual sites for planting seedlings can be prepared by scalping the sod away from a four foot square area where the seedling is to be planted.

2) All planting beds shall be cultivated to a depth of 8 inches, or chemically treated for weed control. Remove objectionable objects that will interfere with maintenance of site.

G. Planting

1) Plants shall be located as shown on plans and/or drawings and, where necessary, located on the site by stakes, flags or other means.

2) Prior to planting, remove galvanized wire basket securing root ball, untie and roll down burlap covering from around the stem.

3) The plants shall be set upright in holes as illustrated in Figure G.1 in Appendix G.

4) All plants shall be thoroughly watered on the same day of planting. Plants that have settled shall be reset to grade.

H. Wrapping

Immediately after planting, wrap deciduous tree trunks from the bottom to the first limb with a 4 inch wide bituminous impregnated, insect resistant tape or paper manufactured for that purpose. Tie with jute (bag strings) at top and bottom. The wrap should be removed per nursery recommendations.

I. Mulching

Mulch the disturbed area around individual trees and shrubs with a 2-3" layer of wood chips. Pull wood chips 1 inch away from the base of shrubs to avoid fungus development.

J. Pruning

After planting, prune to remove injured twigs and branches. The natural shape of the plant should not be changed.

K. Cleanup and Maintenance

1) After all work is complete, all excess soil, peat moss, debris, etc., shall be removed from the site.

2) Water plants two weeks after planting. For two years, water plants every two weeks during dry periods, which exceed three weeks without a good soaking rain, or water as needed in accordance with local conditions. Shrubs may require 5 to 10 gallons and trees, 20 to 30 gallons for each watering.

3) Remove trunk wrap per nursery recommendation.

2. Transplanting "Wild" Stock

Successful transplanting of wild stock will require heavy equipment and considerable labor as a large weight of soil must be moved with the roots.

A. Select trees and shrubs with good form and full crowns.

B. Transplant only when plants are dormant and soil is moist. Wrap soil ball with burlap to prevent soil from separating from roots.

C. Table 4.8 shows minimum diameter and

approximate weight of soil ball that must be moved with each size plant.

D. Plant and maintain as described above for nursery stock.

PRUNING AND THINNING

Use	Cleared Width Each Side of Trail Tread (ft.)	Cleared Height (ft.)
<u>TRAILS</u>		
Hiking	1	8
Bicycle	2	10
Motorbike	2	10
Horse	2	12
X-Country Ski	Total: 3-12	12 ¹
Snowmobile	Total: 6-12	12 ¹
<u>PICNIC & CAMPING AREAS</u>		
Campfire/Grill	10 ft. diam.	15
¹ Includes allowance for snow depth and snow load on branches		

1. Pruning

A. Remove trees, limbs, and limb stubs to the above widths and heights specified for the intended use.

B. Remove dead, diseased, or dying limbs that may fall.

C. Do not remove more than one-third of the live crown of a tree in a year.

D. Cut limbs flush to the branch bark ridge.

E. Use the 3 or 4 cut pruning method on all branches over 2 inches in diameter: First cut about one-third the way through the underside of the limb (about 6-12 inches from the tree trunk). Then (approximately an inch further out) make a second cut through the limb from the upper side. When the branch is removed, there is no splintering of the main tree trunk. Remove the stub. If the branch is larger than 5-6 inches in diameter, use the four cut system. Cuts 1 and 2 remain the same and cut 3 should be from the underside of the limb, on the outside of the branch collar. Cut 4 should be from the top and in alignment with the 3rd cut. Cut 3 should be 1/4 to 1/3 the way through the limb. This will prevent the bark from peeling down the trunk. Do not paint the cut surface.

2. Thinning

- A. Remove dead, diseased, dying, poorly anchored, or ice damaged trees that pose a hazard to recreationists or that interfere with intended use.
- B. To maintain grass cover in a wooded area, thin according to formula $D \times 3$ (average diameter of the trunk of overstory trees, in inches, times three—the answer is the spacing between trees to be left, in feet). For example, for trees with average diameter of 6 inches, spacing after thinning should leave trees 18 feet apart on average. Crown cover after thinning should be about 50 percent.
- C. Selectively thin as needed to favor those trees that are most “resistant” to compaction around their roots. See Table G.2, “Susceptibility of Tree Species to Compaction” in Appendix G. If the soil on the site is naturally well drained, those species in the “intermediate” group may also be favored.

Table 4.8
Size and Weight of Earth Ball Required to Transplant Wild Stock

Caliper ¹ (Inches)	Shade Trees (Maple, Ash, Oak, Birch, etc.)		Small Trees & Shrubs (Crabapple, Thornapple, Viburnum, Dogwood, etc.)		
	Minimum Diameter Ball (Inches)	Weight of Ball (lbs.)	Up to 6 ft. Height — 6 ft. and Caliper ¹	Minimum Diameter Ball (Inches)	Weight of Ball (lbs.)
1/2	14	88	2	12	55
3/4	16	130	3	14	88
1	18	186	4	16	130
1-1/4	20	227	5	18	186
1-1/2	22	302	3/4	18	186
1-3/4	24	390	1	20	227
2	28	621	1-1/2	22	302
3	32	836	1-3/4	24	390
3-1/2	38	1,400	2	28	621
4	42	1,887	2-1/2	32	836
			3	38	1,400

¹Caliper is a diameter measurement of trees at a height of 6 inches above the ground.

STANDARD AND SPECIFICATIONS FOR SEDIMENT DIKE



Definition & Scope

A **temporary** earth dike with an excavated trench on the upslope toe placed across a slope to capture sediment laden flow from small disturbed drainage areas and allowing sediment to settle out by ponding.

Condition Where Practice Applies

This practice can be used on slopes and in areas where it is difficult to place and maintain silt fence. This practice acts as a smaller sediment trap for linear type applications. This practice will handle sheet and rill erosion for small tributary areas.

Design Criteria

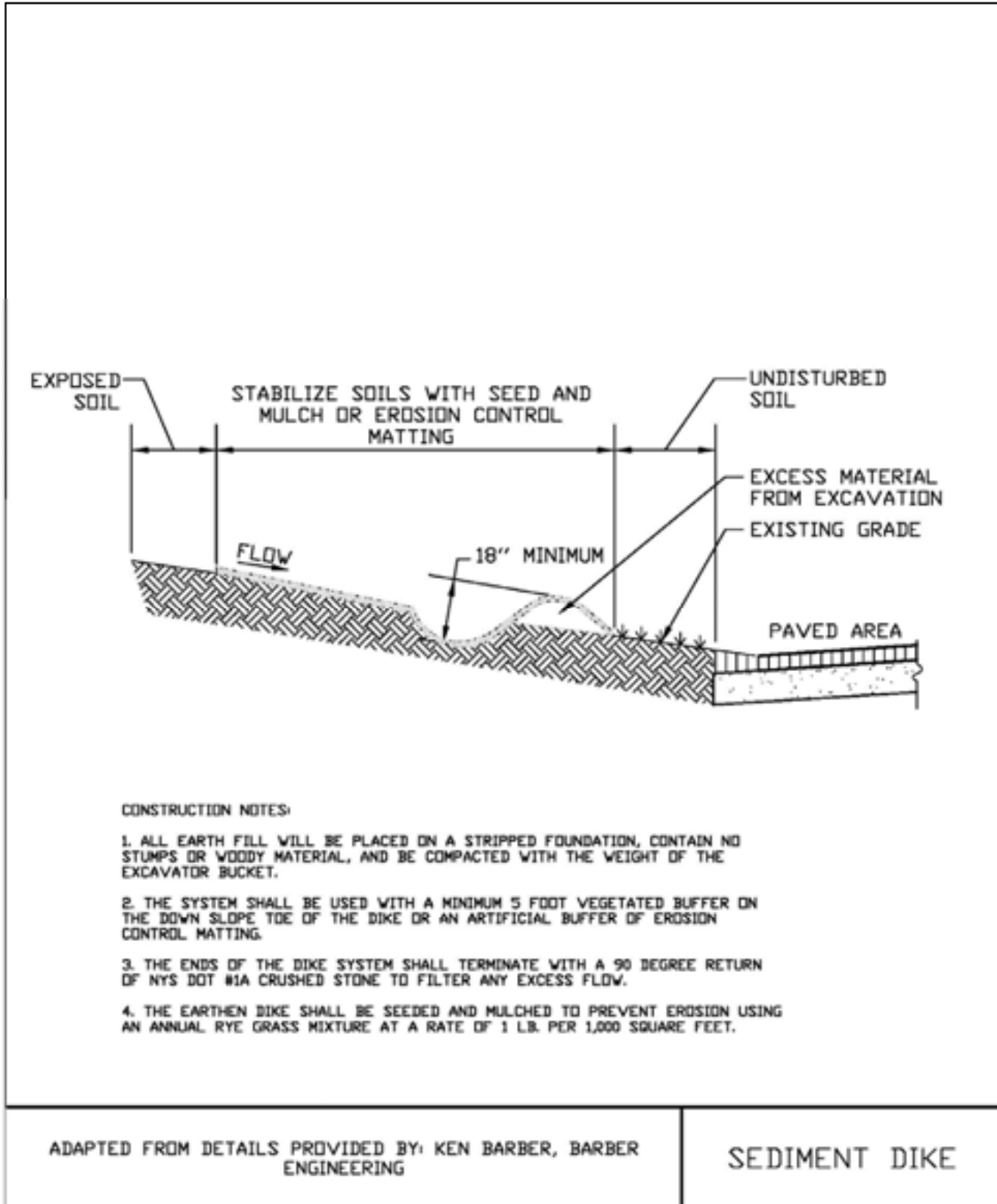
1. The earth dike will be a maximum of 2 feet high with a 2 foot top width and 2:1 side slopes.
2. All earth fill will be placed on a stripped foundation, contain no stumps or woody material, and be compacted with the weight of the excavator bucket.
3. The interior capture trench will be a minimum 1.5 feet deep, 2 feet wide, with 1:1 side slopes and with a near level bottom. In areas where linear slopes exceed 4% the sediment dike system shall be segmented to maintain capture volume and ponding.
4. The system shall be used with a minimum 5 foot vegetated buffer on the down slope toe of the dike or an artificial buffer of erosion control matting.
5. The maximum ponding depth behind the dike shall be 1/2 the height of the constructed dike at its lowest elevation.

6. The ends of the dike system shall terminate with a 90° return of NYS DOT #1A crushed stone to filter any excess flow.
7. The maximum drainage area tributary to this practice shall not exceed 0.5 acres per 100 feet of dike, for slopes less than 10%. For slopes greater than 10%, the drainage area shall be 0.25 acres per 100 feet of dike.
8. The earthen dike shall be seeded and mulched to prevent erosion using an annual rye grass mixture at a rate of 1 lb. per 1,000 square feet.

Maintenance

1. No traffic will be allowed on the dike.
2. Dike system will be inspected weekly and after each runoff event.
3. Sediment in the system will be removed when the interior trench has filled to 75% capacity.
4. Sediment will be disposed of on-site as specified in the Erosion and Sediment Control Plan.
5. Upon stabilization of the tributary drainage area, the trench will be filled, excess dike fill removed, and the area graded and stabilized in accordance with the Erosion and Sediment Control Plan.

Figure 5.23
Sediment Dike



STANDARD AND SPECIFICATIONS FOR SEDIMENT TRAP



Definition & Scope

A **temporary** sediment control device formed by excavation and/or embankment to intercept sediment-laden runoff and trap the sediment in order to protect drainageways, properties, and rights-of-way below the sediment trap from sedimentation.

Conditions Where Practice Applies

A sediment trap is usually installed in a drainageway, at a storm drain inlet, or other points of collection from a disturbed area for one construction season.

Sediment traps should be used to artificially break up the natural drainage area into smaller sections where a larger device (sediment basin) would be less effective.

Design Criteria

If the drainage area to the proposed trap location exceeds 5 acres, or the trap is in place beyond one construction season, or any of the additional design criteria presented here cannot be met, a full Sediment Basin must be used. See Standard and Specification for Sediment Basin on page 5.19.

Drainage Area

The maximum drainage area for all sediment traps shall be 5 acres.

Location

Sediment traps shall be located so that they can be installed prior to grading or filling in the drainage area they are to protect. Traps must **not be located any closer than 20 feet** from a proposed building foundation if the trap is to func-

tion during building construction. Locate traps to obtain maximum storage benefit from the terrain and for ease of cleanout and disposal of the trapped sediment.

Trap Size

The volume of a sediment trap as measured at the elevation of the crest of the outlet shall be at least 3,600 cubic feet per acre of drainage area. A minimum length to width ratio of 2:1 should be provided. The volume of a constructed trap shall be calculated using standard mathematical procedures. The volume of a natural sediment trap may be approximated by the equation: Volume (cu.ft.) = 0.4 x surface area (sq.ft.) x maximum depth (ft.).

Trap Cleanout

Sediment shall be removed and the trap restored to the original dimensions when the sediment has accumulated to $\frac{1}{2}$ of the design depth of traps I-II, and $\frac{1}{3}$ the depth for trap III. Sediment removed from the trap shall be deposited in a protected area and in such a manner that it will not erode.

Embankment

All earth embankments for sediment traps shall not exceed five (5) feet in height as measured at the low point of the original ground along the centerline of the embankment. Embankments shall have a minimum four (4) foot wide top and side slopes of 2:1 or flatter. The embankment shall be compacted by traversing with equipment while it is being constructed. The embankment shall be stabilized with seed and mulch as soon as it is completed.

The elevation of the top of any dike directing water to any sediment trap will equal or exceed the maximum height of the outlet structure along the entire length of the trap.

Excavation

All excavation operations shall be carried out in such a manner that erosion and water pollution shall be minimal. Excavated portions of sediment traps shall have 1:1 or flatter slopes.

Outlet

The outlet shall be designed, constructed, and maintained in such a manner that sediment does not leave the trap and that erosion at or below the outlet does not occur.

Sediment traps must outlet onto stabilized (preferable undisturbed) ground, into a watercourse, stabilized channel, or into a storm drain system. Distance between inlet and outlet should be maximized to the longest length practicable.

All traps must be seeded and mulched immediately after construction.

Trap Details Needed on Erosion and Sediment Control Plans

Each trap shall be delineated on the plans in such a manner that it will not be confused with any other features. Each trap on a plan shall indicate all the information necessary to properly construct and maintain the structure. If the drawings are such that this information cannot be delineated on the drawings, then a table shall be developed. If a table is developed, then each trap on a plan shall have a number and the numbers shall be consecutive.

The following information shall be shown for each trap in a summary table format on the plans.

1. Trap number
2. Type of trap
3. Drainage area
4. Storage required
5. Storage provided (if applicable)
6. Outlet length or pipe sizes
7. Storage depth below outlet or cleanout elevation
8. Embankment height and elevation (if applicable)

Type of Sediment Traps

There are three (3) specific types of sediment traps which vary according to their function, location, or drainage area.

- I. Pipe Outlet Sediment Trap
- II. Stone Outlet Sediment Trap
- III. Compost Filter Sock Sediment Trap

I. Pipe Outlet Sediment Trap

A Pipe Outlet Sediment Trap consists of a trap formed by embankment or excavation. The outlet for the trap is through a perforated riser and a pipe through the embankment. The outlet pipe and riser shall be made of steel, corrugated metal or other suitable material. The top of the embankment shall be at least 1 ½ feet above the crest of the riser. The preferred method of dewatering the sediment trap is by surface skimmer. See Dewatering Device Standard, page 5.10. If the riser alone is used for dewatering, the top 2/3 of the riser shall be perforated with one (1) inch nominal diameter holes or slits spaced six (6) inches vertically and horizontally placed in the concave portion of the corrugated pipe.

No holes or slits will be allowed within six (6) inches of the top of the horizontal barrel. All pipe connections shall be watertight. The riser shall be wrapped with ½ to ¾ inch hardware cloth wire then wrapped with filter cloth with a sieve size between #40-80 and secured with strapping or connecting band at the top and bottom of the cloth. The

cloth shall cover an area at least six (6) inches above the highest hole and six (6) inches below the lowest hole. The top of the riser pipe shall not be covered with filter cloth. The riser shall have a base with sufficient weight to prevent flotation of the riser. Two approved bases are:

1. A concrete base 12 in. thick with the riser embedded 9 in. into the concrete base, or
2. One quarter inch, minimum, thick steel plate attached to the riser by a continuous weld around the circumference of the riser to form a watertight connection. The plate shall have 2.5 feet of stone, gravel, or earth placed on it to prevent flotation. In either case, each side of the square base measurement shall be the riser diameter plus 24 inches.

Pipe outlet sediment traps shall be limited to a five (5) acre maximum drainage area. Pipe outlet sediment trap is interchangeable in the field with stone outlet provided that these sediment traps are constructed in accordance with the detail and specifications for that trap.

Select pipe diameter from the following table:
See details for Pipe Outlet Sediment Trap ST-I in Figure 5.25 and 5.26 on pages 5.49 and 5.50.

Optional sediment trap dewatering devices are shown on Figure 5.29 on Page 5.53.

Minimum Sizes

Barrel Diameter ¹ (in.)	Riser Diameter ¹ (in.)	Maximum Drainage Area (ac.)
12	15	1
15	18	2
18	21	3
21	24	4
21	27	5

¹ Barrel diameter may be same size as riser diameter



II. Stone Outlet Sediment Trap

A Stone Outlet Sediment Trap consists of a trap formed by an embankment or excavation. The outlet of this trap is over a stone section placed on level ground. The minimum length (feet) of the outlet shall be equal to four (4) times the drainage area (acres).

Required storage shall be 3,600 cubic feet per acre of drainage area.

The outlet crest (top of stone in weir section) shall be level, at least one (1) foot below top of embankment and no more than one (1) foot above ground beneath the outlet. Stone used in the outlet shall be small riprap (4 in. x 8 in.). To provide more efficient trapping effect, a layer of filter cloth should be embedded one (1) foot back into the upstream face of the outlet stone or a one (1) foot thick layer of two (2) inch or finer aggregate shall be placed on the upstream face of the outlet.

Stone Outlet Sediment Traps may be interchangeable in the field with pipe outlet sediment traps provided they are constructed in accordance with the detail and specifications for those traps. Stone outlet sediment traps shall be limited to a five (5) acre maximum drainage area.

See details for Stone Outlet Sediment Trap ST-II in Figure 5.27 on page 5.51



III. Compost Sock Sediment Trap

A compost sock sediment trap consists of a trap formed by creating an enclosure of geotextile mesh tubes filled with a compost filter media. These traps are used in locations where there is no opportunity to direct runoff into larger traps or well vegetated areas. This could occur at site entrances and access points or in tight areas due to construction boundary limits.

Surface runoff can be directed to the trap with standard conveyance practices. Groundwater or surface ponding in low areas can be pumped into the compost sock sediment trap with appropriate energy dissipation at the pump outlet to prevent scour.

Design criteria for Compost Sock Sediment Trap

1. The maximum drainage area tributary to the trap shall be 5 acres.
2. The minimum settled height above ground shall be 2.0 feet formed by staking 3 compost filter socks in a pyramid as shown in Figure 5.28 on page 5.52.
3. The storage volume provided in the compost sock sediment trap shall be 3,600 cubic feet per tributary drainage acre.
4. If necessary, additional storage area can be created by excavating a sump 1 foot deep beginning at least 5 feet away from the inside sock.
5. All compost filter sock materials, mesh, and compost, will meet the material specifications listed in the Compost Filter Sock standard. No spillway is required.
6. Compost filter sock sediment traps shall be inspected weekly and after every rainfall event. Sediment shall be removed when it reaches one third, $\frac{1}{3}$, the height of the trap.
7. The maximum limit of use for a compost sock sediment trap is one (1) year. The existing trap shall be replaced if there is a need for a trap beyond that time limit.
8. Upon completion of the work, the compost sock sediment trap shall be removed. The compost within the socks may be used during cleanup as a vegetative growth medium in accordance with the site stabilization plan.



Figure 5.25
Pipe Outlet Sediment Trap: ST-I

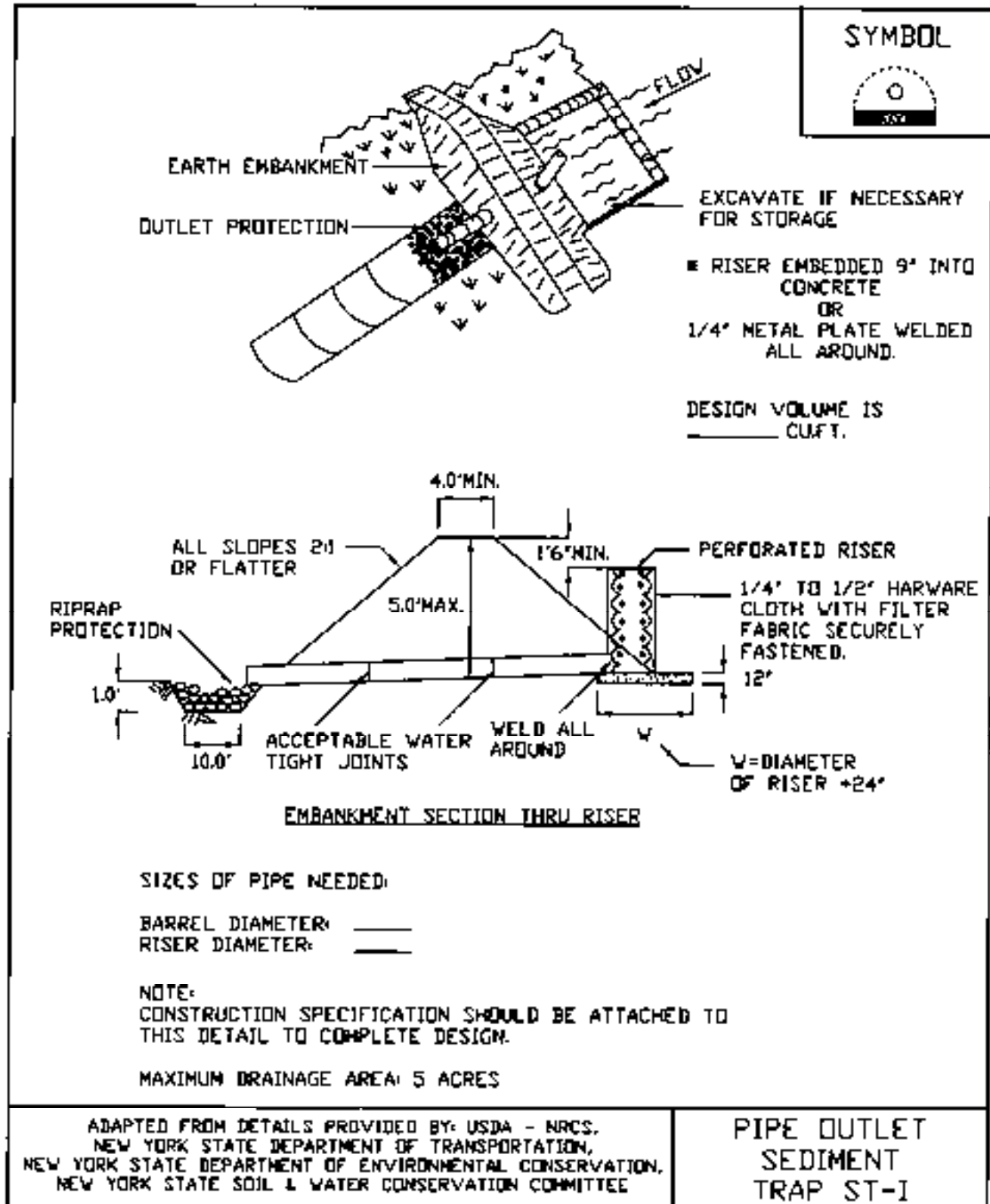


Figure 5.26
Pipe Outlet Sediment Trap: ST-I - Construction Specifications

<u>CONSTRUCTION SPECIFICATIONS</u>	SYMBOL 
1. AREA UNDER EMBANKMENT SHALL BE CLEARED, GRUBBED AND STRIPPED OF ANY VEGETATION AND ROOT MAT. THE POOL AREA SHALL BE CLEARED. 2. THE FILL MATERIAL FOR THE EMBANKMENT SHALL BE FREE OF ROOTS OR OTHER WOODY VEGETATION AS WELL AS OVER-SIZED STONES, ROCKS, ORGANIC MATERIAL, OR OTHER OBJECTIONABLE MATERIAL. THE EMBANKMENT SHALL BE COMPACTED BY TRAVERSING WITH EQUIPMENT WHILE IT IS BEING CONSTRUCTED. 3. VOLUME OF SEDIMENT STORAGE SHALL BE 3600 CUBIC FEET PER ACRE OF CONTRIBUTORY DRAINAGE. 4. SEDIMENT SHALL BE REMOVED AND TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO 1/2 THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND STABILIZED. 5. THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS NEEDED. 6. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER THAT EROSION AND SEDIMENT ARE CONTROLLED. 7. THE STRUCTURE SHALL BE REMOVED AND AREA STABILIZED WHEN THE DRAINAGE AREA HAS BEEN PROPERLY STABILIZED. 8. ALL FILL SLOPES SHALL BE 2:1 OR FLATTER; CUT SLOPES 1:1 OR FLATTER. 9. ALL PIPE CONNECTIONS SHALL BE WATERTIGHT. 10. THE TOP 2/3 OF THE RISER SHALL BE PERFORATED WITH ONE (1) INCH DIAMETER HOLES OR SLITS SPACED SIX (6) INCHES VERTICALLY AND HORIZONTALLY AND PLACED IN THE CONCAVE PORTION OF PIPE. NO HOLES WILL BE ALLOWED WITHIN SIX (6) INCHES OF THE HORIZONTAL BARREL. 11. THE RISER SHALL BE WRAPPED WITH 1/4 TO 1/2 INCH HARDWARE CLOTH WIRE THEN WRAPPED WITH FILTER CLOTH (HAVING AN EQUIVALENT SIEVE SIZE OF 40-80). THE FILTER CLOTH SHALL EXTEND SIX (6) INCHES ABOVE THE HIGHEST HOLE AND SIX (6) INCHES BELOW THE LOWEST HOLE. WHERE ENDS OF THE FILTER CLOTH COME TOGETHER, THEY SHALL BE OVER-LAPPED, FOLDED AND STAPLED TO PREVENT BYPASS. 12. STRAPS OR CONNECTING BANDS SHALL BE USED TO HOLD THE FILTER CLOTH AND WIRE FABRIC IN PLACE. THEY SHALL BE PLACED AT THE TOP AND BOTTOM OF THE CLOTH. 13. FILL MATERIAL AROUND THE PIPE SPILLWAY SHALL BE HAND COMPACTED IN FOUR (4) INCH LAYERS. A MINIMUM OF TWO (2) FEET OF HAND COMPACTED BACKFILL SHALL BE PLACED OVER THE PIPE SPILLWAY BEFORE CROSSING IT WITH CONSTRUCTION EQUIPMENT. 14. THE RISER SHALL BE ANCHORED WITH EITHER A CONCRETE BASE OR STEEL PLATE BASE TO PREVENT FLOTATION. FOR CONCRETE BASE THE DEPTH SHALL BE TWELVE (12) INCHES WITH THE RISER EMBEDDED NINE (9) INCHES. A 1/4 INCH MINIMUM THICKNESS STEEL PLATE SHALL BE ATTACHED TO THE RISER BY A CONTINUOUS WELD AROUND THE BOTTOM TO FORM A WATERTIGHT CONNECTION AND THEN PLACE TWO (2) FEET OF STONE, GRAVEL, OR TAMPED EARTH ON THE PLATE.	
ADAPTED FROM DETAILS PROVIDED BY: USDA - NRCS, NEW YORK STATE DEPARTMENT OF TRANSPORTATION, NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION, NEW YORK STATE SOIL & WATER CONSERVATION COMMITTEE	PIPE OUTLET SEDIMENT TRAP ST-I

Figure 5.27
Stone Outlet Sediment Trap: ST-II

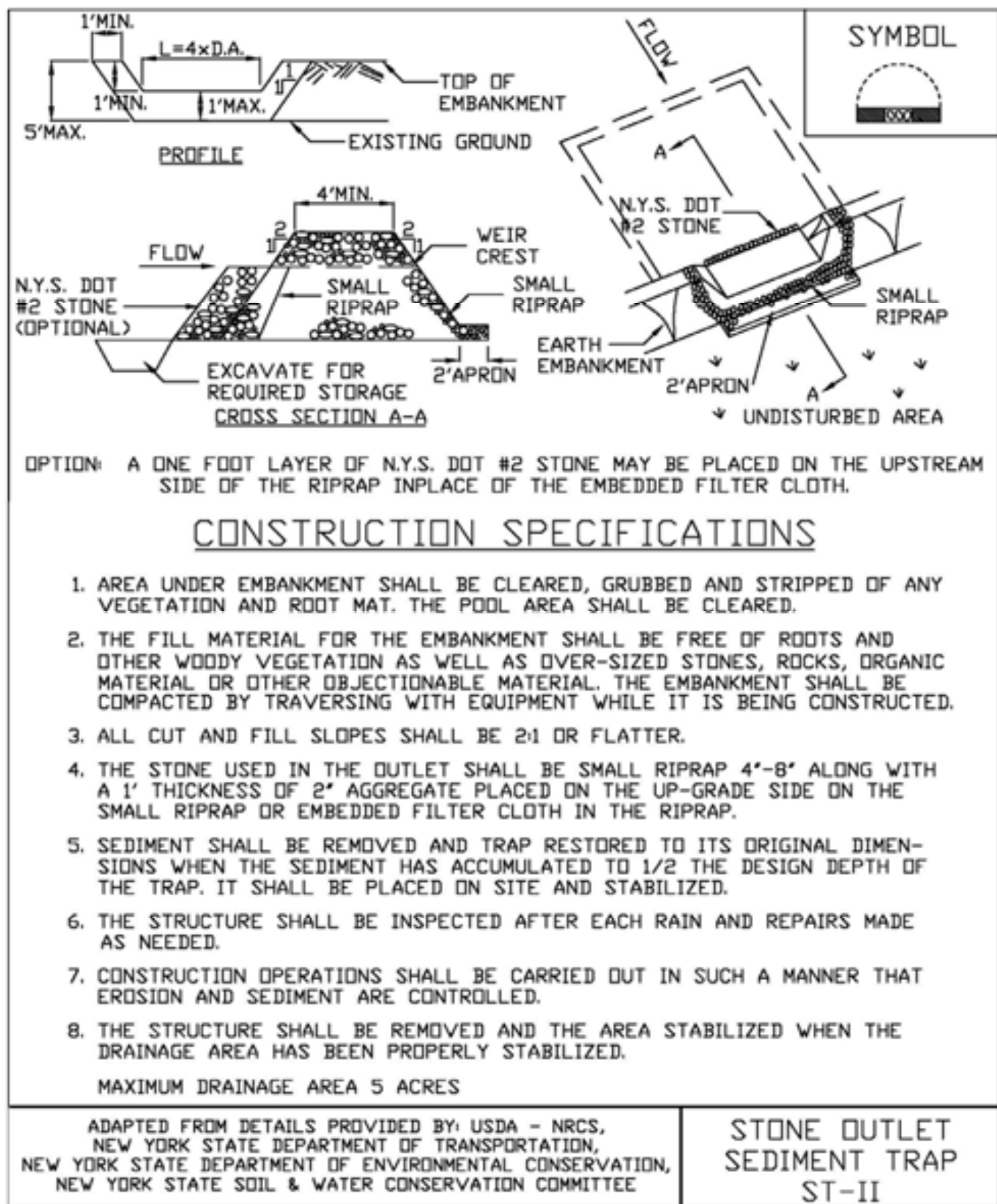
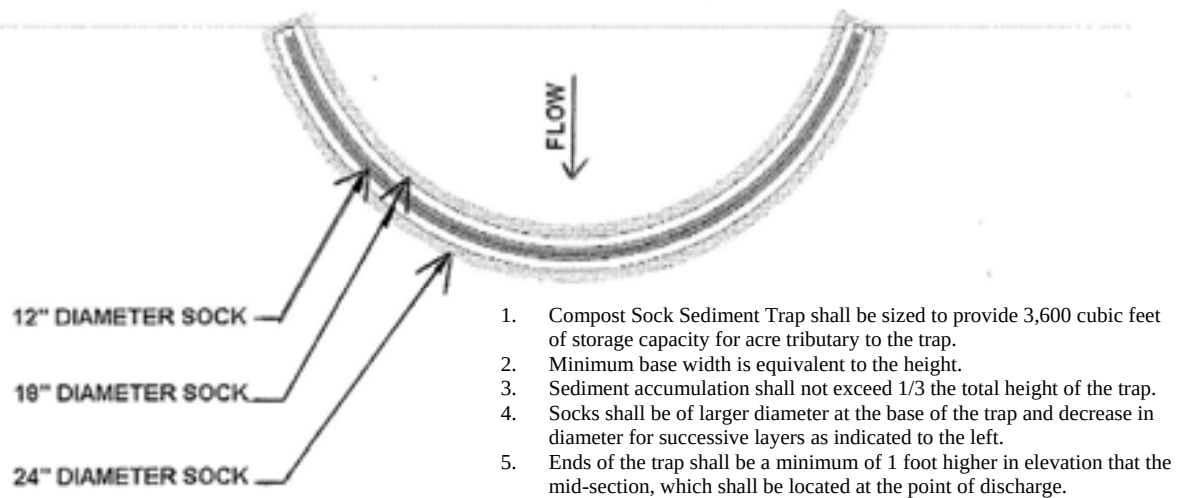
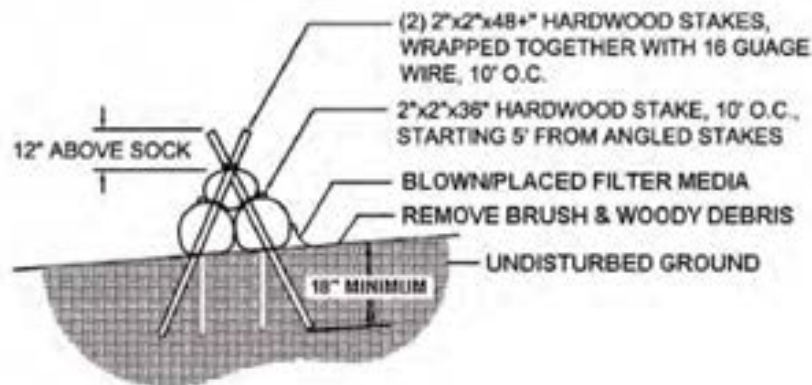


Figure 5.28 Compost Filter Sock Sediment Trap: ST-III

Plan View



Staking Detail

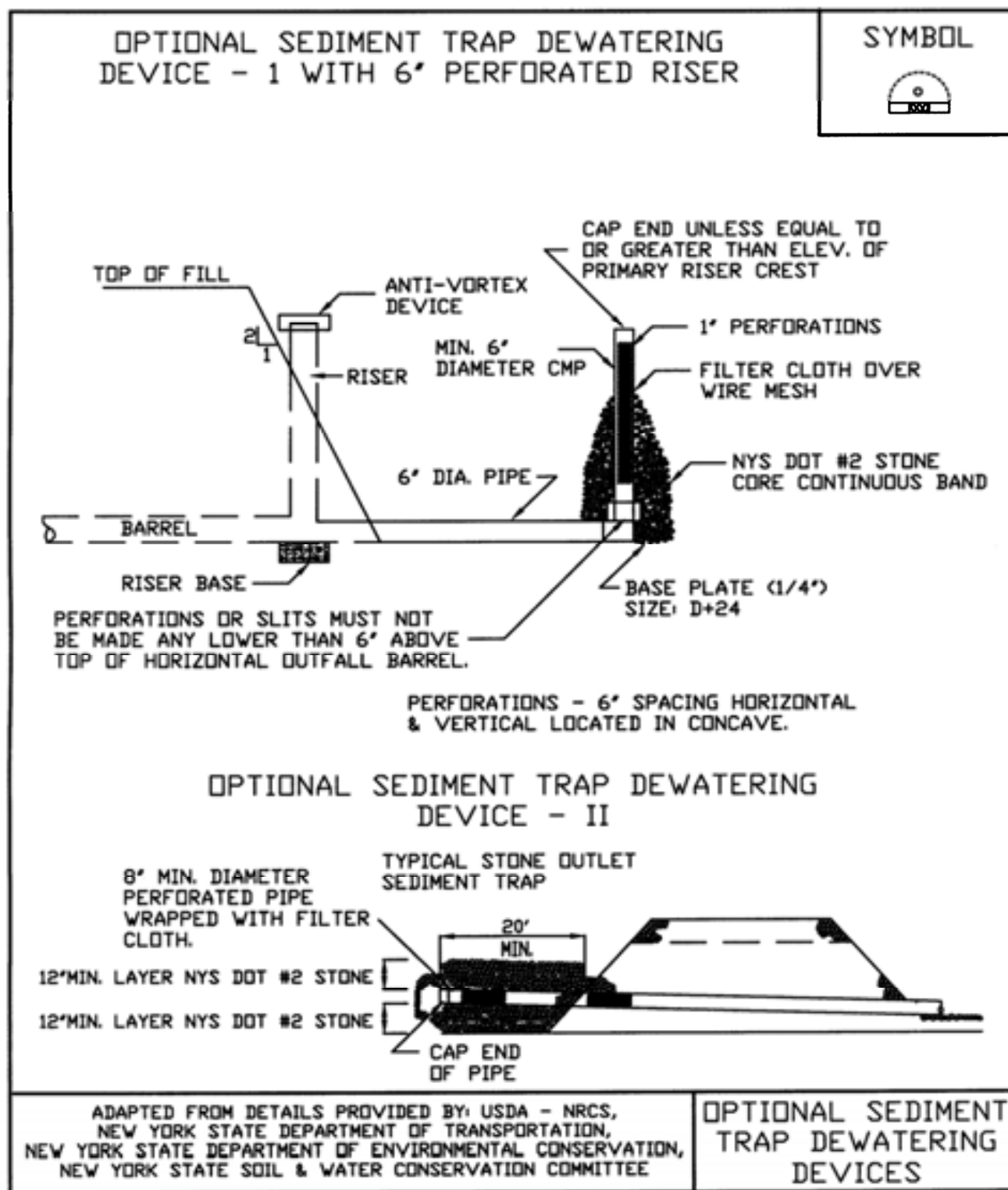


* Figures adapted from Filtrex

Specifications:

1. Sock infill and filter media material shall meet the standards of Table 5.1 on page 5.8 . Compost shall meet the compost filter sock standard of Table 5.2 on page 5.8.
2. Compost sock sediment traps shall not exceed three socks in height and shall be stacked in pyramidal form as shown above. Minimum trap height is one 24 inch diameter sock. Additional storage may be provided by means of an excavated sump 12 inches deep extending 1 to 3 feet upslope of the socks along the lower side of the trap.
3. Compost sock sediment traps shall provide 3,600 cubic feet storage capacity with 12 inches of freeboard for each tributary drainage acreage. (See manufacturer for anticipated settlement.)
4. The maximum tributary drainage area is 5.0 acres. Since compost socks are "flow-through," no spillway is required.
5. Compost sock sediment traps shall be inspected weekly and after each runoff event. Sediment shall be removed when it reaches 1/3 the height of the socks.
6. Photodegradable and biodegradable socks shall not be used for more than 1 year.

Figure 5.29
Optional Sediment Trap Dewatering Devices
for Traps with <5 Acres Drainage Area



STANDARD AND SPECIFICATIONS FOR SILT FENCE



Definition & Scope

A **temporary** barrier of geotextile fabric installed on the contours across a slope used to intercept sediment laden runoff from small drainage areas of disturbed soil by temporarily ponding the sediment laden runoff allowing settling to occur. The maximum period of use is limited by the ultraviolet stability of the fabric (approximately one year).

Conditions Where Practice Applies

A silt fence may be used subject to the following conditions:

1. Maximum allowable slope length and fence length will not exceed the limits shown in the Design Criteria for the specific type of silt fence used ; and
2. Maximum ponding depth of 1.5 feet behind the fence; and
3. Erosion would occur in the form of sheet erosion; and
4. There is no concentration of water flowing to the barrier; and
5. Soil conditions allow for proper keying of fabric, or other anchorage, to prevent blowouts.

Design Criteria

1. Design computations are not required for installations of 1 month or less. Longer installation periods should be designed for expected runoff.
2. All silt fences shall be placed as close to the disturbed area as possible, but at least 10 feet from the toe of a slope steeper than 3H:1V, to allow for maintenance and

roll down. The area beyond the fence must be undisturbed or stabilized.

3. The type of silt fence specified for each location on the plan shall not exceed the maximum slope length and maximum fence length requirements shown in the following table:

		Slope Length/Fence Length (ft.)		
Slope	Steepness	Standard	Reinforced	Super
<2%	< 50:1	300/1500	N/A	N/A
2-10%	50:1 to 10:1	125/1000	250/2000	300/2500
10-20%	10:1 to 5:1	100/750	150/1000	200/1000
20-33%	5:1 to 3:1	60/500	80/750	100/1000
33-50%	3:1 to 2:1	40/250	70/350	100/500
>50%	> 2:1	20/125	30/175	50/250

Standard Silt Fence (SF) is fabric rolls stapled to wooden stakes driven 16 inches in the ground.

Reinforced Silt Fence (RSF) is fabric placed against welded wire fabric with anchored steel posts driven 16 inches in the ground.

Super Silt Fence (SSF) is fabric placed against chain link fence as support backing with posts driven 3 feet in the ground.

4. Silt fence shall be removed as soon as the disturbed area has achieved final stabilization.

The silt fence shall be installed in accordance with the appropriate details. Where ends of filter cloth come together, they shall be overlapped, folded and stapled to prevent sediment bypass. Butt joints are not acceptable. A detail of the silt fence shall be shown on the plan. See Figure 5.30 on page 5.56 for Reinforced Silt Fence as an example of details to be provided.

Criteria for Silt Fence Materials

1. Silt Fence Fabric: The fabric shall meet the following specifications unless otherwise approved by the appropriate erosion and sediment control plan approval authority. Such approval shall not constitute statewide acceptance.

Fabric Properties	Minimum Acceptable Value	Test Method
Grab Tensile Strength (lbs)	110	ASTM D 4632
Elongation at Failure (%)	20	ASTM D 4632
Mullen Burst Strength (PSI)	300	ASTM D 3786
Puncture Strength (lbs)	60	ASTM D 4833
Minimum Trapezoidal Tear Strength (lbs)	50	ASTM D 4533
Flow Through Rate (gal/min/sf)	25	ASTM D 4491
Equivalent Opening Size	40-80	US Std Sieve ASTM D 4751
Minimum UV Residual (%)	70	ASTM D 4355

Super Silt Fence

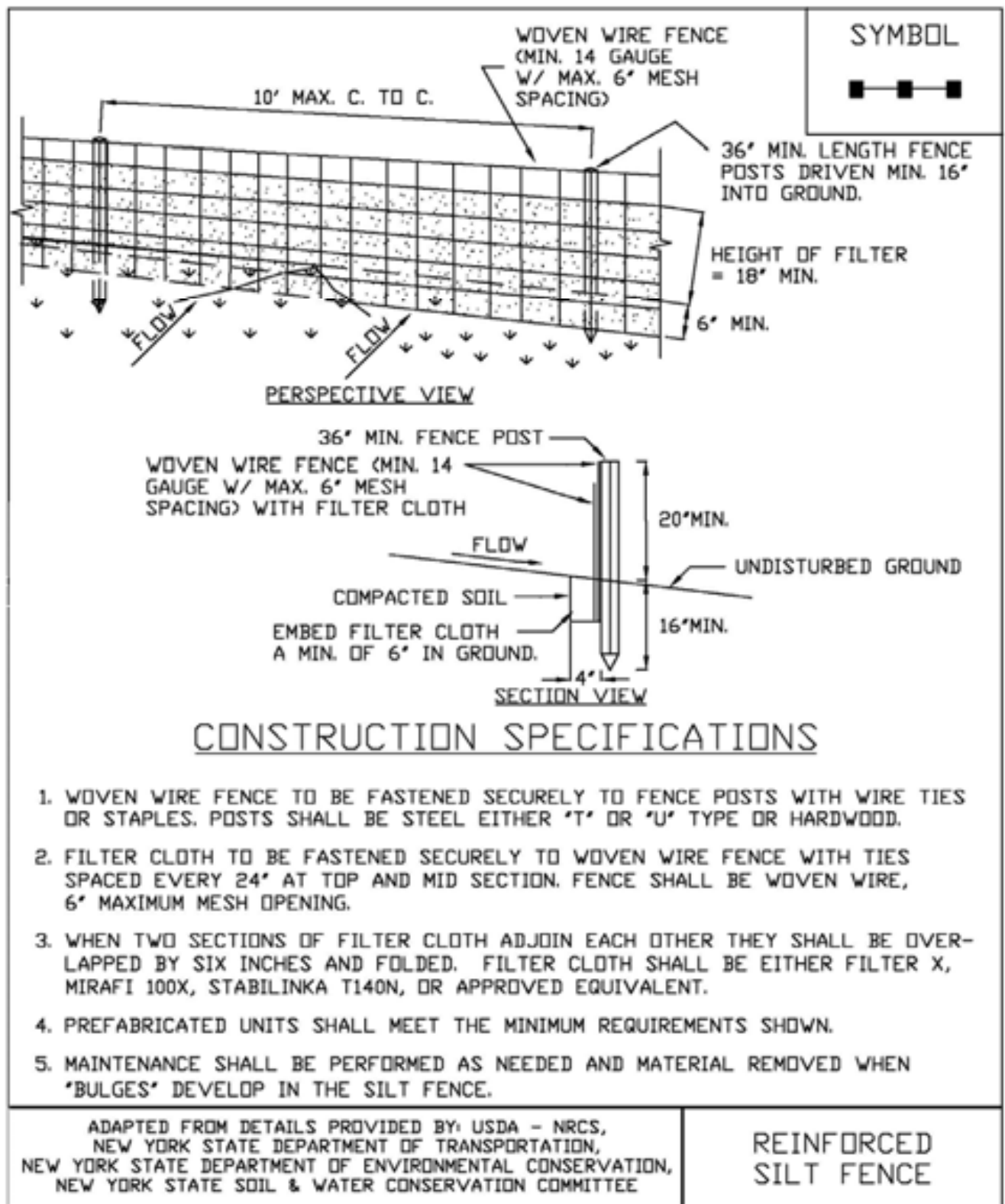


2. Fence Posts (for fabricated units): The length shall be a minimum of 36 inches long. Wood posts will be of sound quality hardwood with a minimum cross sectional area of 3.5 square inches. Steel posts will be standard T and U section weighing not less than 1.00 pound per linear foot. Posts for super silt fence shall be standard chain link fence posts.
3. Wire Fence for reinforced silt fence: Wire fencing shall be a minimum 14 gage with a maximum 6 in. mesh opening, or as approved.
4. Prefabricated silt fence is acceptable as long as all material specifications are met.

Reinforced Silt Fence



**Figure 5.30
Reinforced Silt Fence**



STANDARD AND SPECIFICATIONS FOR STORM DRAIN INLET PROTECTION



Definition & Scope

A **temporary** barrier with low permeability, installed around inlets in the form of a fence, berm or excavation around an opening, detaining water and thereby reducing the sediment content of sediment laden water by settling thus preventing heavily sediment laden water from entering a storm drain system.

Conditions Where Practice Applies

This practice shall be used where the drainage area to an inlet is disturbed, it is not possible to temporarily divert the storm drain outfall into a trapping device, and watertight blocking of inlets is not advisable. **It is not to be used in place of sediment trapping devices.** This practice shall be used with an upstream buffer strip if placed at a storm drain inlet on a paved surface. It may be used in conjunction with storm drain diversion to help prevent siltation of pipes installed with low slope angle.

Types of Storm Drain Inlet Practices

There are five (5) specific types of storm drain inlet protection practices that vary according to their function, location, drainage area, and availability of materials:

- I. Excavated Drop Inlet Protection
- II. Fabric Drop Inlet Protection
- III. Stone & Block Drop Inlet Protection
- IV. Paved Surface Inlet Protection
- V. Manufactured Insert Inlet Protection

Design Criteria

Drainage Area – The drainage area for storm drain inlets shall not exceed one acre. Erosion control/temporary stabilization measures must be implemented on the disturbed

drainage area tributary to the inlet. The crest elevations of these practices shall provide storage and minimize bypass flow.

Type I – Excavated Drop Inlet Protection

This practice is generally used during initial overlot grading after the storm drain trunk line is installed.

Limit the drainage area to the inlet device to 1 acre. Excavated side slopes shall be no steeper than 2:1. The minimum depth shall be 1 foot and the maximum depth 2 feet as measured from the crest of the inlet structure. Shape the excavated basin to fit conditions with the longest dimension oriented toward the longest inflow area to provide maximum trap efficiency. The capacity of the excavated basin should be established to contain 900 cubic feet per acre of disturbed area. Weep holes, protected by fabric and stone, should be provided for draining the temporary pool.

Inspect and clean the excavated basin after every storm. Sediment should be removed when 50 percent of the storage volume is achieved. This material should be incorporated into the site in a stabilized manner.

Type II – Fabric Drop Inlet Protection



This practice is generally used during final elevation grading phases after the storm drain system is completed.

Limit the drainage area to 1 acre per inlet device. Land area slope immediately surrounding this device should not exceed 1 percent. The maximum height of the fabric above the inlet crest shall not exceed 1.5 feet unless reinforced.

The top of the barrier should be maintained to allow overflow to drop into the drop inlet and not bypass the inlet to

unprotected lower areas. Support stakes for fabric shall be a minimum of 3 feet long, spaced a maximum 3 feet apart. They should be driven close to the inlet so any overflow drops into the inlet and not on the unprotected soil. Improved performance and sediment storage volume can be obtained by excavating the area.

Inspect the fabric barrier after each rain event and make repairs as needed. Remove sediment from the pool area as necessary with care not to undercut or damage the filter fabric. Upon stabilization of the drainage area, remove all materials and unstable sediment and dispose of properly. Bring the adjacent area of the drop inlet to grade, smooth and compact and stabilize in the appropriate manner to the site.

Type III – Stone and Block Drop Inlet Protection

This practice is generally used during the initial and intermediate overlot grading of a construction site.

Limit the drainage area to 1 acre at the drop inlet. The stone barrier should have a minimum height of 1 foot and a maximum height of 2 feet. Do not use mortar. The height should be limited to prevent excess ponding and bypass flow.

Recess the first course of blocks at least 2 inches below the crest opening of the storm drain for lateral support. Subsequent courses can be supported laterally if needed by placing a 2x4 inch wood stud through the block openings perpendicular to the course. The bottom row should have a few blocks oriented so flow can drain through the block to dewater the basin area.

The stone should be placed just below the top of the blocks on slopes of 2:1 or flatter. Place hardware cloth of wire mesh with ½ inch openings over all block openings to hold stone in place.

As an optional design, the concrete blocks may be omitted and the entire structure constructed of stone, ringing the outlet (“doughnut”). The stone should be kept at a 3:1 slope toward the inlet to keep it from being washed into the inlet. A level area 1 foot wide and four inches below the crest will further prevent wash. Stone on the slope toward the inlet should be at least 3 inches in size for stability and 1 inch or smaller away from the inlet to control flow rate. The elevation of the top of the stone crest must be maintained 6 inches lower than the ground elevation down slope from the inlet to ensure that all storm flows pass over the stone into the storm drain and not past the structure. Temporary diking should be used as necessary to prevent bypass flow.

The barrier should be inspected after each rain event and repairs made where needed. Remove sediment as necessary to provide for accurate storage volume for subsequent rains. Upon stabilization of contributing drainage area, remove all

materials and any unstable soil and dispose of properly.

Bring the disturbed area to proper grade, smooth, compact and stabilize in a manner appropriate to the site.

Type IV – Paved Surface Inlet Protection



This practice is generally used after pavement construction has been done while final grading and soil stabilization is occurring. These practices should be used with upstream buffer strips in linear construction applications, and with temporary surface stabilization for overlot areas, to reduce the sediment load at the practice. This practice includes sand bags, compost filter socks, geo-tubes filled with ballast, and manufactured surface barriers. Pea gravel can also be used in conjunction with these practices to improve performance. When the inlet is not at a low point, and is offset from the pavement or gutter line, protection should be selected and installed so that flows are not diverted around the inlet.



The drainage area should be limited to 1 acre at the drain inlet. All practices will be placed at the inlet perimeter or beyond to maximize the flow capacity of the inlet. Practices shall be weighted, braced, tied, or otherwise anchored to prevent movement or shifting of location on paved surfaces. Traffic safety shall be integrated with the use of this practice. All practices should be marked with traffic safety cones as appropriate. Structure height shall not cause flooding or by-pass flow that would cause additional erosion.

The structure should be inspected after every storm event. Any sediment should be removed and disposed of on the site. Any broken or damaged components should be replaced. Check all materials for proper anchorage and secure as necessary.

Type V - Manufactured Insert Inlet Protection



The drainage area shall be limited to 1 acre at the drain inlet. All inserts will be installed and anchored in accordance with the manufacturers recommendations and design details. The fabric portion of the structure will equal or exceed the performance standard for the silt fence fabric. The inserts will be installed to preserve a minimum of 50 percent of the open, unobstructed design flow area of the storm drain inlet opening to maintain capacity for storm events.

Figure 5.31
Excavated Drop Inlet Protection

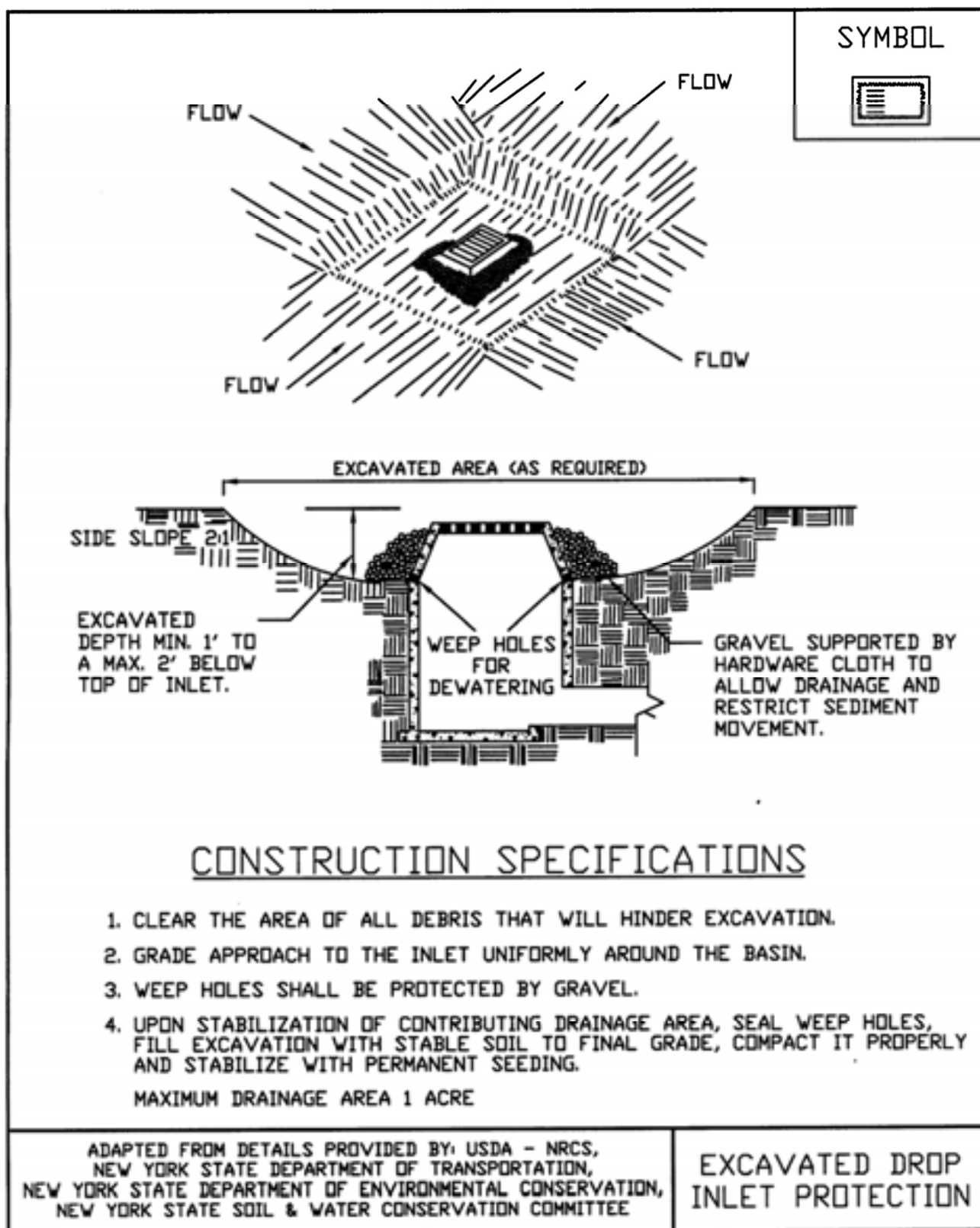


Figure 5.32
Fabric Drop Inlet Protection

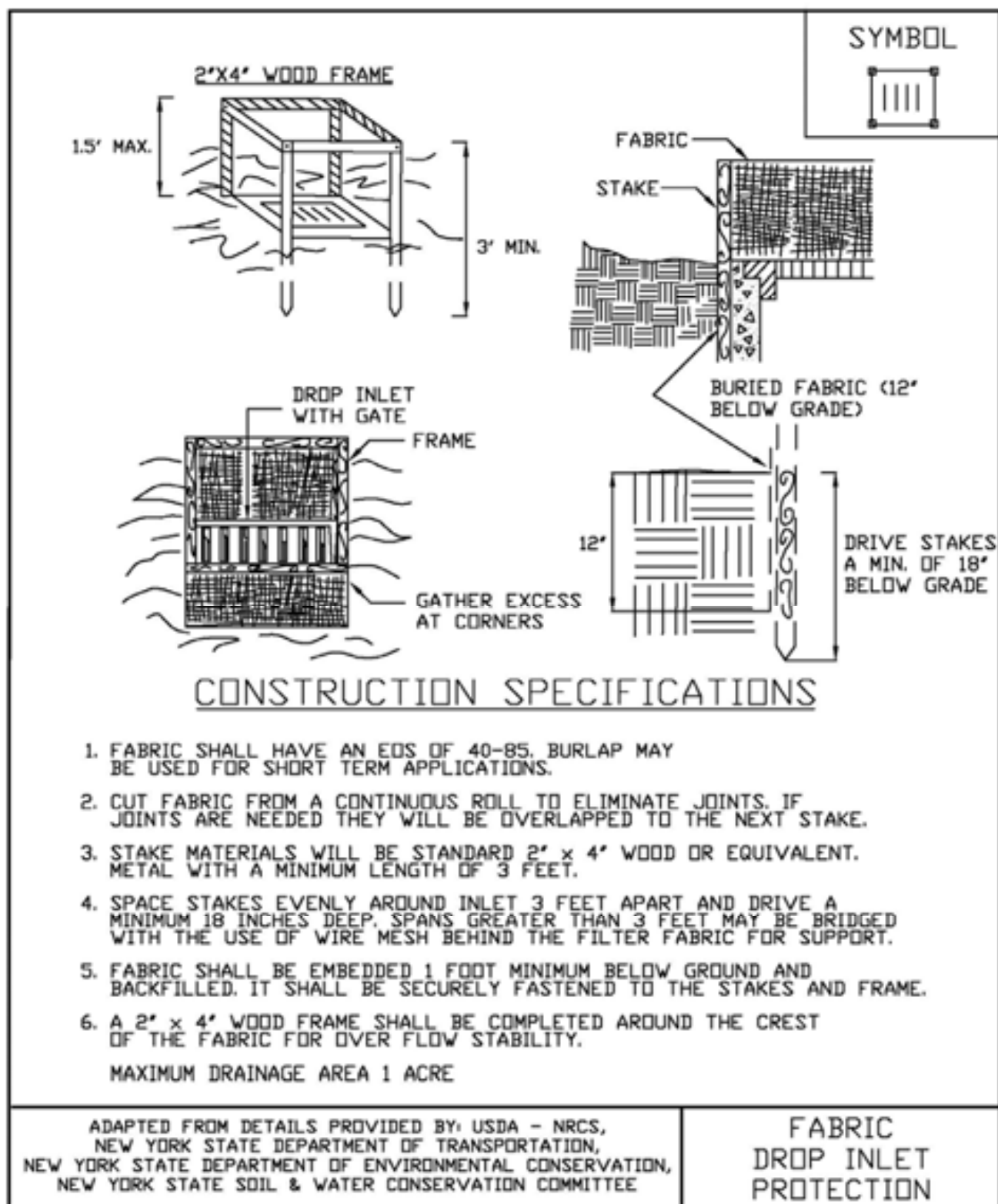
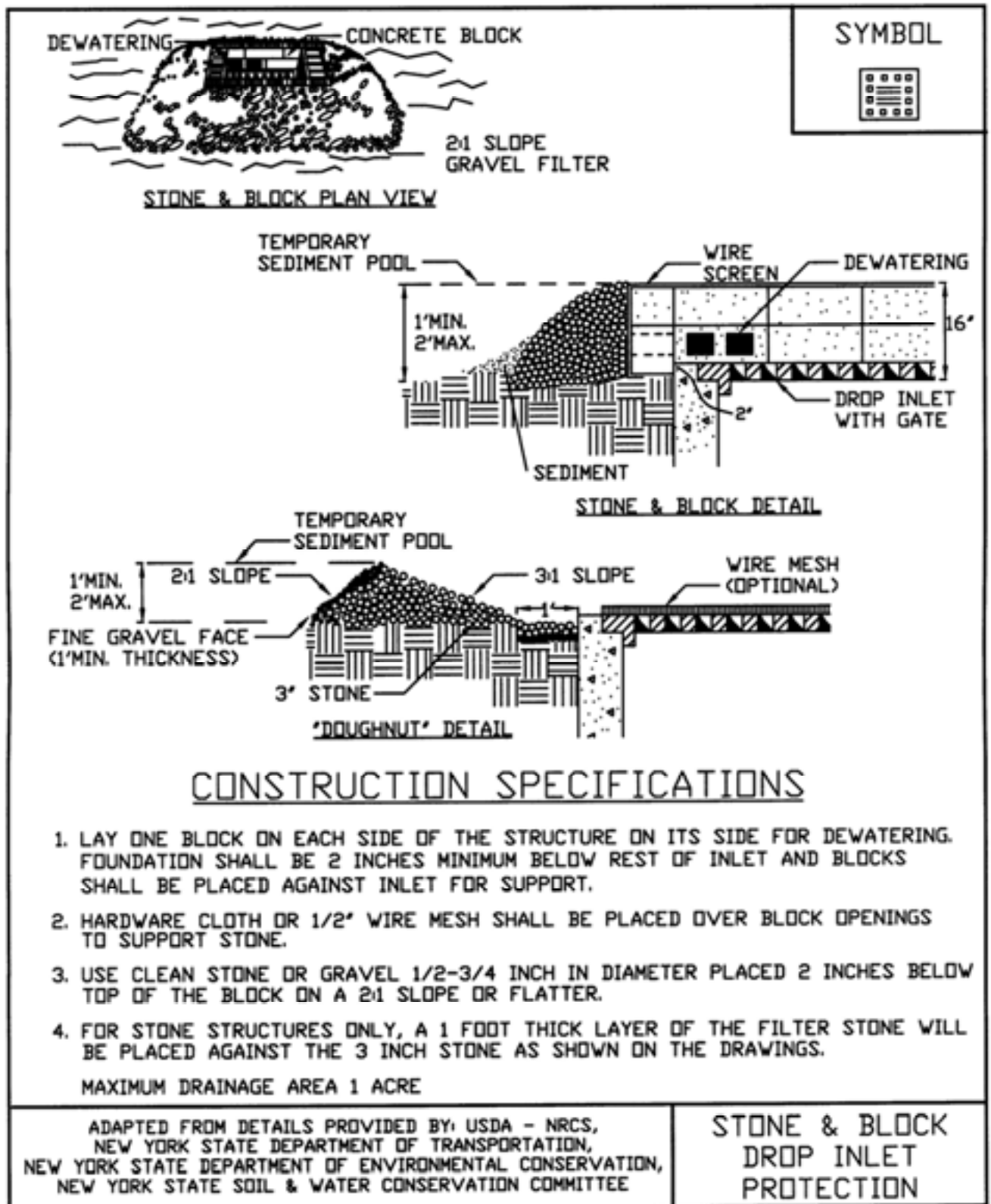


Figure 5.33
Stone & Block Drop Inlet Protection



STANDARD AND SPECIFICATIONS FOR DEWATERING SUMP PIT



Discharge of turbid water pumped from the standpipe should be to a sediment trap, sediment basin, filter bag or stabilized area, such as a filter strip. If water from the sump pit will be pumped directly to a storm drain system, filter cloth with an equivalent sieve size between 40-80 should be wrapped around the standpipe to ensure clean water discharge. It is recommended that $\frac{1}{4}$ to $\frac{1}{2}$ inch hardware cloth be wrapped around and secured to the standpipe prior to attaching the filter cloth. This will increase the rate of water seepage into the standpipe.

Definition & Scope

A **temporary** pit which is constructed using pipe and stone for pumping excessive water from excavations to a suitable discharge area.

Conditions Where Practice Applies

Sump pits are constructed when water collects during the excavation phase of construction. This practice is particularly useful in urban areas during excavation for building foundations. It may also be necessary during construction activities that encounter high ground water tables in floodplain locations.

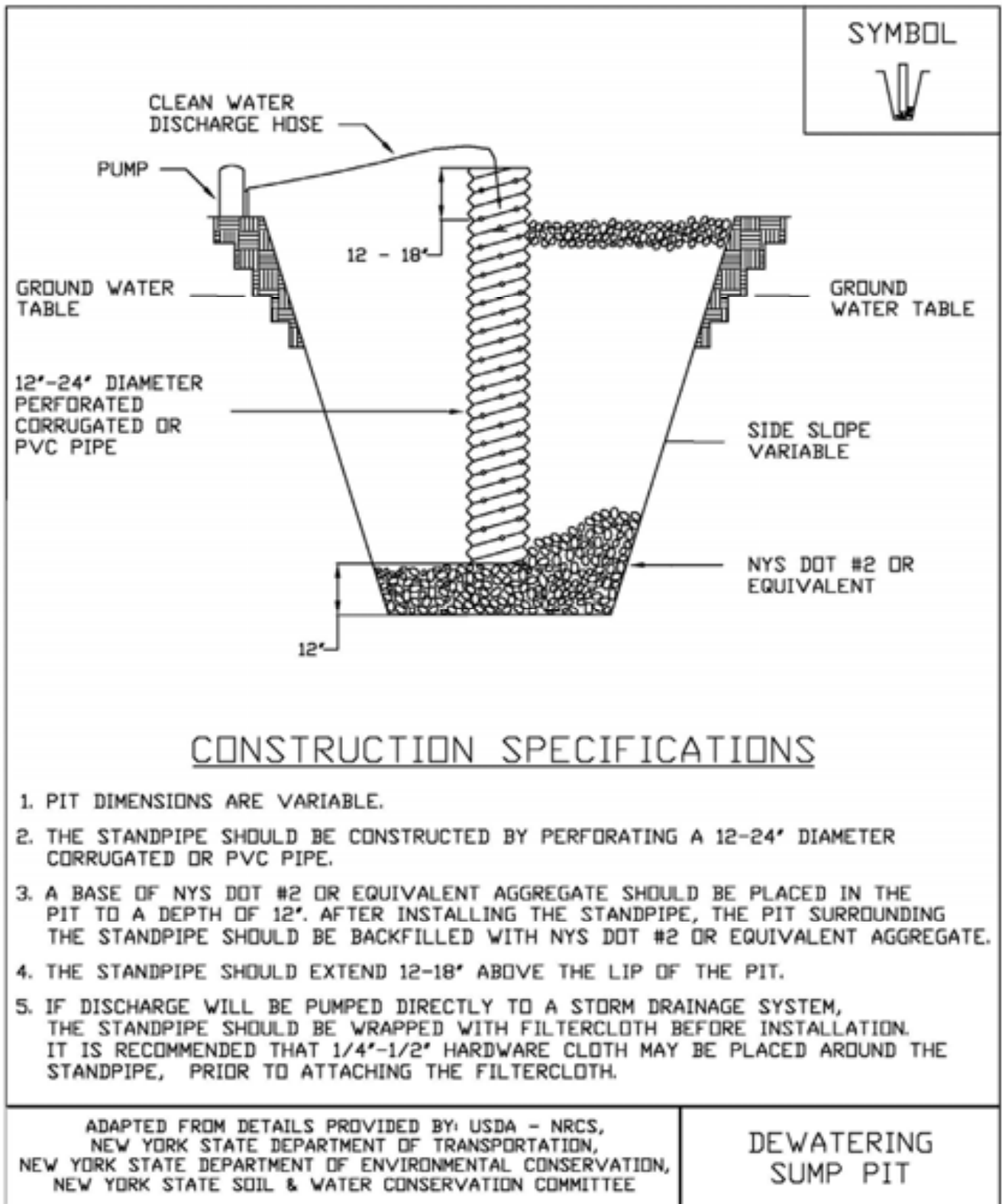
Design Criteria

The number of sump pits and their locations shall be determined by the contractor/engineer. A design is not required, but construction should conform to the general criteria outlined on Figure 3.3 on page 3.8.

A perforated vertical standpipe is placed in the center of the pit and surrounded with a stone screening material to collect filtered water. Water is then pumped from the center of the pipe to a suitable discharge area.



Figure 3.3
Dewatering Sump Pit Detail



STANDARD AND SPECIFICATIONS FOR ANCHORED STABILIZATION MATTING



Definition and Scope

A **temporary** or **permanent** protective covering placed on a prepared, seeded planting area that is anchored in place by staples or other means to aid in controlling erosion by absorbing rain splash energy and withstand overland flow as well as provide a microclimate to protect and promote seed establishment.

Conditions Where Practice Applies

Anchored stabilization mats are required for seeded earthen slopes steeper than 3 horizontal to 1 vertical; in vegetated channels where the velocity of the design flow exceeds the allowable velocity for vegetation alone (usually greater than 5 feet per second); on streambanks and shorelines where moving water is likely to erode newly seeded or planted areas; and in areas where wind prevents standard mulching with straw. This standard does not apply to slopes stabilized with sod, rock riprap or hard armor material.

Design Criteria

Slope Applications - Anchored stabilization mats for use on slopes are primarily used as mulch blankets where the mesh material is within the blanket or as a netting over previously placed mulch. These stabilization mats are NOT effective in preventing slope failures.

1. Required on all slopes steeper than 3:1
2. Matting will be designed for proper longevity need and strength based on intended use.
3. All installation details and directions will be included on the site erosion and sediment control plan and will follow manufactures specifications.

Channel Applications - Anchored stabilization mats, for use in supporting vegetation in flow channels, are generally a non-degradable, three dimensional plastic structure which can be filled with soil prior to planting. This structure provides a medium for root growth where the matting and roots become intertwined forming a continuous anchor for the vegetated lining.

1. Channel stabilization shall be based on the tractive force method.
2. For maximum design shear stresses less than 2 pounds per square foot, a temporary or bio-degradable mat may be used.
3. The design of the final matting shall be based on the mats ability to resist the tractive shear stress at bank full flow.
4. The installation details and procedures shall be included on the site erosion and sediment control plan and will follow manufacturers specifications.



Construction Specifications

1. Prepare soil before installing matting by smoothing the surface, removing debris and large stone, and applying lime, fertilizer and seed. Refer to manufacturers installation details.
2. Begin at the top of the slope by anchoring the mat in a 6" deep x 6" wide trench. Backfill and compact the trench after stapling.
3. In channels or swales, begin at the downslope end, anchoring the mat at the bottom and top ends of the blanket. When another roll is needed, the upslope roll

should overlay the lower layer, shingle style, so that channel flows do not peel back the material.

4. Roll the mats down a slope with a minimum 4" overlap. Roll center mat in a channel in direction of water flow on bottom of the channel. Do not stretch blankets. Blankets shall have good continuous contact with the underlying soil throughout its entire length.
5. Place mats end over end (shingle style) with a 6" overlap, use a double row of staggered staples 4" apart to secure mats.
6. Full length edge of mats at top of side slopes must be anchored in 6" deep x 6" wide trench; backfill and compact the trench after stapling.
7. Mats on side slopes of a channel must be overlapped 4" over the center mat and stapled.
8. In high flow channel applications, a staple check slot is recommended at 30 to 40 foot intervals. Use a row of staples 4" apart over entire width of the channel. Place a second row 4" below the first row in a staggered pattern.
9. The terminal end of the mats must be anchored in a 6"x6" wide trench. Backfill and compact the trench after stapling.
10. Stapling and anchoring of blanket shall be done in accordance with the manufactures recommendations.

Maintenance

Blanketed areas shall be inspected weekly and after each runoff event until perennial vegetation is established to a minimum uniform 80% coverage throughout the blanketed area. Damaged or displaced blankets shall be restored or replaced within 2 calendar days.

STANDARD AND SPECIFICATIONS FOR ARMORED SLOPE AND CHANNEL STABILIZATION



Definition & Scope

A **permanent** layer of stone designed to protect and stabilize areas subject to erosion by protecting the soil surface from rain splash, sheet flow, rill and gully erosion and channel erosion. It can also be used to improve the stability of soil slopes that are subject to seepage or have poor soil structure.

Conditions Where Practice Applies

Riprap is used for cut and fill slopes subject to seepage, erosion, or weathering, particularly where conditions prohibit the establishment of vegetation. Riprap is also used for channel side slopes and bottoms, temporary dewatering diversion channels where the flow velocities exceed 6 feet/second, grade sills, on shorelines subject to erosion, and at inlets and outlets to culverts, bridges, slope drains, grade stabilization structures, and storm drains.

Slope Stabilization Design Criteria

Gradation – Riprap shall be a well-graded mixture with 50% by weight larger than the specified design size. The diameter of the largest stone size in such a mixture should be 1.5 times the d_{50} size with smaller sizes grading down to 1 inch. The designer should select the size or sizes that equal or exceed that minimum size based on riprap gradations commercially available in the area.

Thickness – The minimum layer thickness shall be 1.5 times the maximum stone diameter, but in no case less than 6 inches.

Quality – Stone for riprap shall be hard, durable field or quarry materials. They shall be angular and not subject to breaking down when exposed to water or weathering. The specific gravity shall be at least 2.5.

Size – The sizes of stones used for riprap protection are determined by purpose and specific site conditions:

1. Slope Stabilization – Riprap stone for slope stabilization not subject to flowing water or wave action shall be sized for the proposed grade. The gradient of the slope to be stabilized shall be less than the natural angle of repose of the stone selected. Angles of repose of riprap stones may be estimated from Figure 4.1.

Riprap used for surface stabilization of slopes does not add significant resistance to sliding or slope failure and should not be considered a retaining wall. Slopes approaching 1.5:1 may require special stability analysis. The inherent stability of the soil must be satisfactory before riprap is used for surface stabilization.

2. Channel Stabilization - Design criteria for sizing stone for stability of channel side slopes are presented under Channel Stabilization Design Criteria on page 4.10.
2. Outlet Protection – Design criteria for sizing stone and determining dimensions of riprap aprons are presented in Standards and Specifications for Rock Outlet Protection on page 3.39.

Filter Blanket – A filter blanket is a layer of material placed between the riprap and the underlying soil to prevent soil movement into or through the riprap. A suitable filter may consist of a well-graded gravel or sand-gravel layer or a synthetic filter fabric manufactured for this purpose. The design of a gravel filter blanket is based on the ratio of particle size in the overlying filter material to that of the base material in accordance with the criteria below. Multiple layers may be designed to affect a proper filter if necessary.

A gravel filter blanket should have the following relationship for a stable design:

$$\frac{d_{15} \text{ filter}}{d_{85} \text{ base}} \leq 5$$

$$5 < \frac{d_{15} \text{ filter}}{d_{50} \text{ base}} \leq 40$$

and

$$\frac{d_{30} \text{ filter}}{d_{30} \text{ base}} \leq 40$$

Filter refers to the overlying material while base refers to the underlying material. These relationships must hold between the base and filter and the filter and riprap to prevent migration of material. In some cases, more than one filter may be needed. Each filter layer should be a minimum of 6 inches thick, unless an acceptable filter fabric is used.

A synthetic filter fabric may be used with or in place of gravel filters. The following particle size relationships should exist:

1. Filter fabric covering a base containing 50% or less by weight of fine particles (#200 sieve size):

A.
$$\frac{d_{85} \text{ base (mm)}}{\text{EOS} \times \text{filter fabric (mm)}} > 1$$

- B. total open area of filter fabric should not exceed 36%

2. Filter fabric covering other soils:

- A. EOS is no larger than 0.21 mm (#70 sieve size)

- B. total open area of filter fabric should not exceed 10%

*EOS – Equivalent opening size compared to a U.S. standard sieve size.

No filter fabric should have less than 4% open area or an EOS less than U.S. Standard Sieve #100 (0.15 mm). The permeability of the fabric must be greater than that of the soil. The fabric may be made of woven or nonwoven monofilament yarns and should meet the following minimum requirements:

Thickness 20-60 mils

grab strength 90-120 lbs.

conform to ASTM D-1682 or ASTM D-177

Filter blankets should always be provided where seepage is significant or where flow velocity and duration of flow or turbulence may cause underlying soil particles to move through the riprap.

Construction Specifications

Subgrade Preparation – Prepare the subgrade for riprap and filter to the required lines and grades shown on the plans. Compact any fill required in the subgrade to a density approximating that of the undisturbed material or overfill depressions with riprap. Remove brush, trees, stumps, and other objectionable material. Cut the subgrade sufficiently deep so that the finished grade of the riprap will be at the

elevation of the surrounding area. Channels shall be excavated sufficiently to allow placement of the riprap in a manner such that the finished inside dimensions and grade of the riprap meet design specifications.

Sand and gravel filter blanket – Place the filter blanket immediately after the ground foundation is prepared. For gravel, spread filter stone in a uniform layer to the specified depth. Where more than one layer of filter material is used, spread the layers with minimal mixing.

Synthetic filter fabric – Place the cloth directly on the prepared foundation. Overlap the edges by at least 2 feet, and space the anchor pins every 3 feet along the overlap. Bury the upper and lower ends of the cloth a minimum of 12 inches below ground. Take precautions not to damage the cloth by dropping the riprap. If damage occurs, remove the riprap and repair the sheet by adding another layer of filter fabric with a minimum overlap of 12 inches around the damaged area. Where large stones are to be placed, a 4-inch layer of fine sand or gravel is recommended to protect the filter cloth. Filter fabric is not recommended as a filter on slopes steeper than 2 horizontal to 1 vertical.

Stone placement – Placement of the riprap shall follow immediately after placement of the filter. Place riprap so that it forms dense, well-graded mass of stone with a minimum of voids. The desired distribution of stones throughout the mass may be obtained by selective loading at the quarry and controlled dumping during final placement. Place riprap to its full thickness in one operation. Do not place riprap by dumping through chutes or other methods that cause segregation of stone sizes. Be careful not to dislodge the underlying base or filter when placing the stones.

The toe of the riprap shall be keyed into a stable foundation at its base as shown in Figure 4.2 - Typical Riprap Slope Protection Detail. The toe should be excavated to a depth of 2.0 feet. The design thickness of the riprap shall extend a minimum of 3 feet horizontally from the slope. The finished slope should be free of pockets of small stone or clusters of large stones. Hand placing may be necessary to achieve proper distribution of stone sizes to produce a relatively smooth, uniform surface. The finished grade of the riprap should blend with the surrounding area.

Maintenance

Riprap shall be inspected periodically for scour or dislodged stones. Control weed and brush growth as needed.

Figure 4.1
Angles of Repose of Riprap Stones (FHWA)

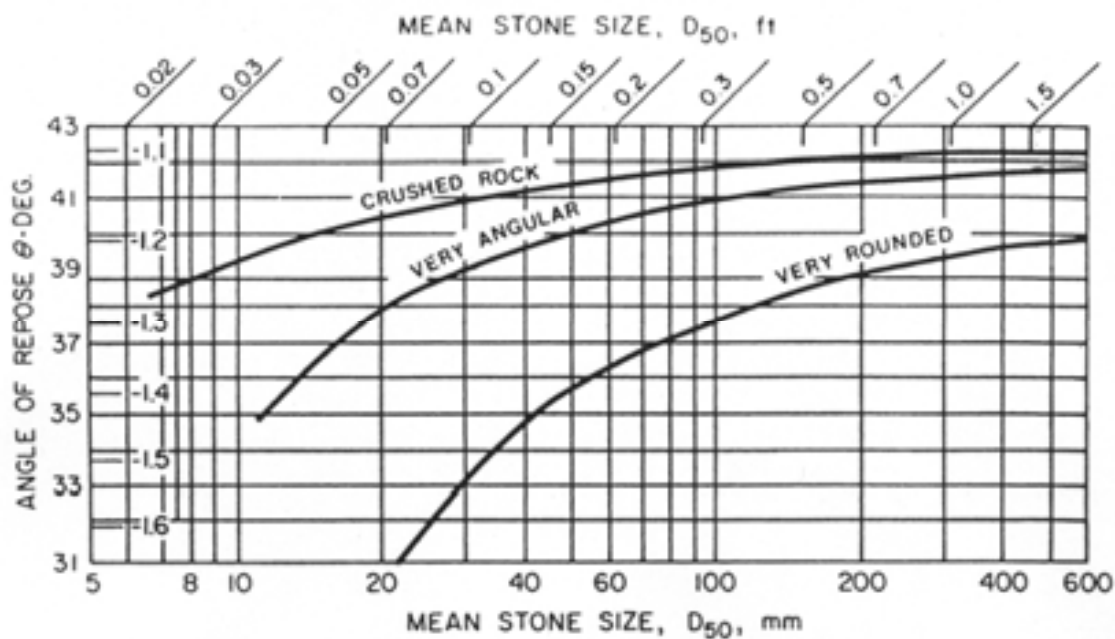
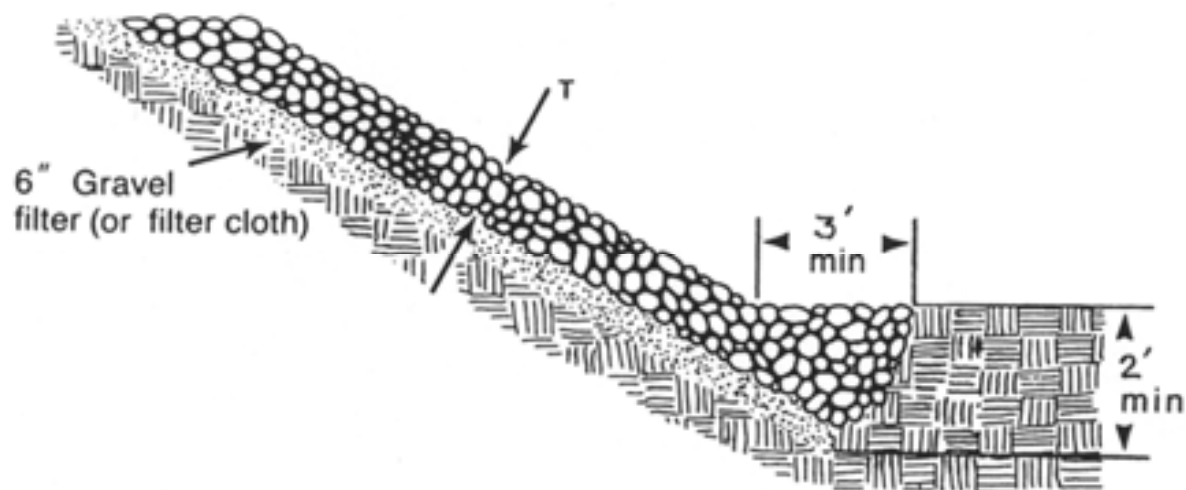


Figure 4.2
Typical Riprap Slope Protection Detail





Channel Stabilization Design Criteria

1. Since each channel is unique, measures for structural channel stabilization should be installed according to a design based on specific site conditions.
2. The plan and profile of the design reach should approximate a naturally stable channel from the project area, based on a stable “reference reach” for the subject channel type.
3. Develop designs according to the following principles:
 - Make protective measures compatible with other channel modifications planned or being carried out in the channel reaches.
 - Whenever excavation and re-shaping work is proposed within channels, the design should provide functional channel dimensions and geometry at each section. Work proposed within a stream channel may require permits from the NYS DEC and US Army Corps of Engineers.
 - Use the design velocity of the peak discharge of the 10-year storm or bankfull discharge, whichever is less. Structural measures should be capable of withstanding greater flows without serious damage.
 - Ensure that the channel bottom is stable or stabilized by structural means before installing any permanent slope protection.
 - Channel stabilization should begin at a stable location and end at a stable point along the bank.
 - Changes in alignment should not be done without a complete analysis of the environmental and stability effects on the entire system.
 - Provisions should be made to maintain and improve fish and wildlife habitat. For example, restoring lost vegetation will provide valuable shade, food, and/or cover.
 - Ensure that all requirements of state law and all permit requirements of local, state, and federal agencies are met.

Construction Specifications

Riprap – Riprap is the most commonly used material to structurally stabilize a channel. While riprap will provide the structural stabilization necessary, the side slope can be enhanced with vegetative material to slow the velocity of water, filter debris, and enhance habitat. See Principles of Biotechnical Practices on page 4.1, for more information.

1. Side slope – slopes shall be graded to 2:1 or flatter prior to placing bedding, filter fabric, or riprap.
2. Filter – filters should be placed between the base material and the riprap and meet the requirements of criteria listed pages 4.7 and 4.8.
3. Gradation – The gradation of the riprap is dependent on the velocity expected against the bank for the design conditions. See Table 4.1 on page 4.12. Once the velocity is known, gradation can be selected from the table for the appropriate class of rock. Note, this table was developed for a 2:1 slope; if the slope steepens to 1.5:1 the gradations should be increased 20%. The riprap should extend 2 feet below the channel bottom and be keyed into the side slope both at the upstream end and downstream end of the proposed work or reach.

See Figure 4.3 on page 4.13 for details.

Reinforced Concrete - Is often used to armor eroding sections of flow channel by constructing walls, bulk heads, or stabilize bank linings in urban areas for redevelopment work. Provide positive drainage behind these structures to relieve uplift pressures.



Grid Pavers – Modular concrete units with or without void areas can be used to stabilize flow channel. Units with void areas can allow the establishment of vegetation. These structures may be obtained in a variety of shapes (Figure 4.4) or they may be formed and poured in place. Maintain design and installation in accordance with manufacturer's instructions.



Revetment – Structural support or armoring to protect an embankment from erosion. Riprap and gabions are commonly used. Also used is a hollow fabric mattress with cells that receive a concrete mixture. Any revetment should be installed to a depth below the anticipated channel degradation and into the channel bed as necessary to provide stability.



Modular Pre-Cast Units – Interlocking modular precast units of different sizes, shapes, heights, and depths, have been developed for a wide variety of applications. They provide vertical support in tight areas as well as durability. Many types are available with textured surfaces. They also act as gravity retaining walls. They should be designed and installed in accordance with the manufacturer's recommendations (Figure 4.4). All areas disturbed by construction should be stabilized as soon as the structural measures are complete.



Maintenance

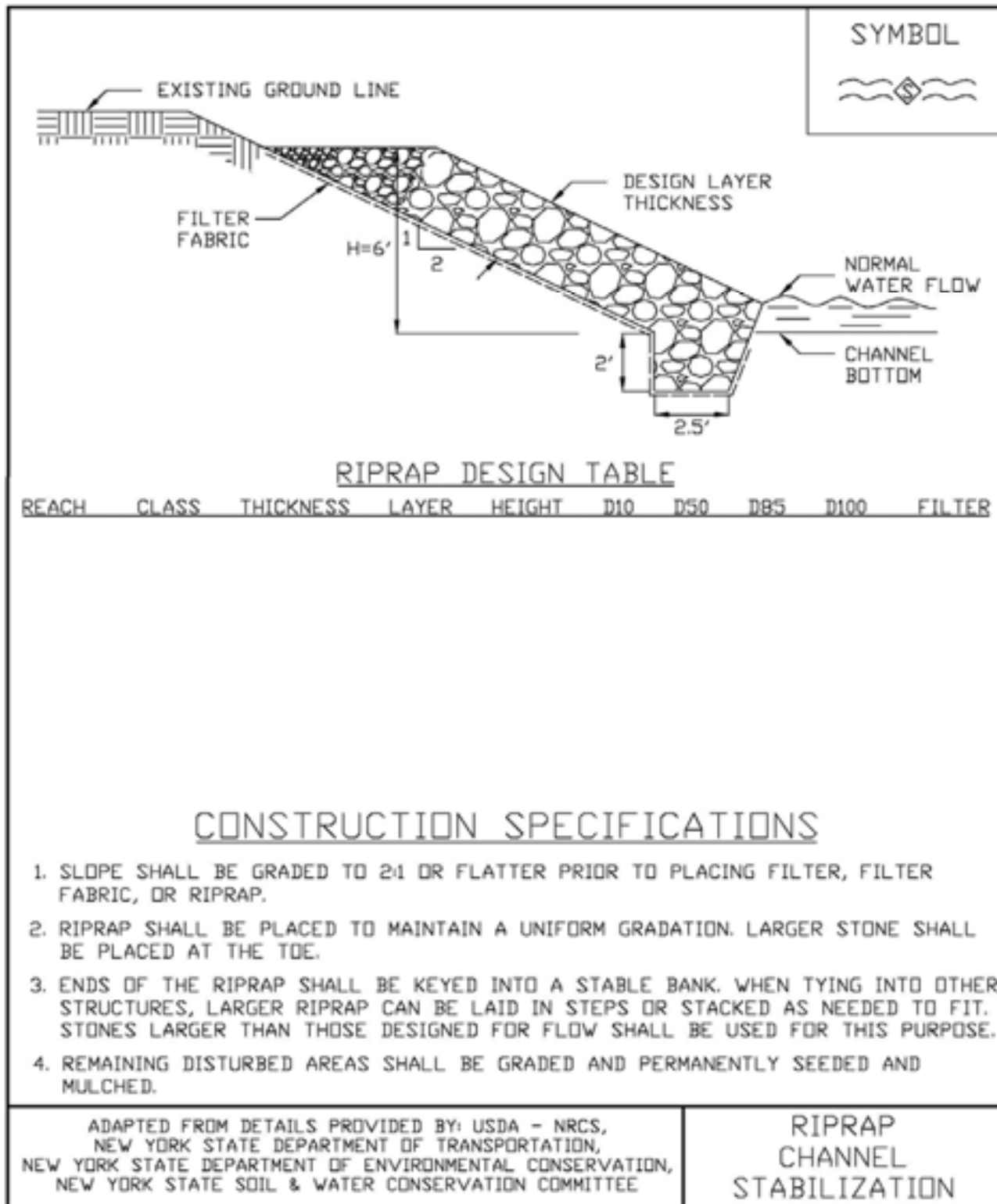
Check stabilized flow channel sections after every high-water event, and make any needed repairs immediately to prevent any further damage or unraveling of the existing work.

Table 4.1 - Riprap Gradations for Channel Stabilization

Class	Layer Thickness (in.)	Max. Velocity (ft/s)	Wave Height (ft.)		PERCENT FINER BY WEIGHT											
					D ₁₀			D ₅₀			D ₈₅			D ₁₀₀		
					Wt. (lbs.)	d _o (in.)	d _□ (in.)	Wt. (lbs.)	d _o (in.)	d _□ (in.)	Wt. (lbs.)	d _o (in.)	d _□ (in.)	Wt. (lbs.)	d _o (in.)	d _□ (in.)
I	18	8.5	-		5	5	4	50	10	8	100	13	10	150	15	12
II	18	10	-		17	7	6	170	15	12	340	19	15	500	22	18
III	24	12	2		46	10	8	460	21	17	920	26	21	1400	30	24
IV	36	14	3		150	15	12	1500	30	25	3000	39	32	4500	47	36
V	48	17	4.8		370	20	16	3700	42	34	7400	53	43	11,000	60	49

d_o = gravel material d_□ = angular rock riprap
Wt = weight in pounds

Figure 4.3
Riprap Channel Stabilization



STABLE BOTTOM

FILTER CLOTH

2 BLOCKS WIDE

GRID PAVERS

EXISTING CHANNEL BOTTOM

10' HIGH WALL

2' MIN.

1:1

6.0'

6.0'

2.0'

6"

PRE-CAST MODULAR UNITS

EST. 120 CY ROCK RIPRAP (MEDIUM STONE FILL)

EXCAVATION LINE

BACKFILL MATERIAL

Interlocking Blocks

CHANNEL STABILIZATION METHODS

ADAPTED FROM DETAILS PROVIDED BY: USDA - NRCS,
NEW YORK STATE DEPARTMENT OF TRANSPORTATION,
NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION,
NEW YORK STATE SOIL & WATER CONSERVATION COMMITTEE

STANDARD AND SPECIFICATIONS FOR DEWATERING DEVICE



Definition & Scope

An appurtenance to a sediment trapping structure such as a basin or trap that allows sediment laden water to pond allowing sediment to settle out while removing relatively clean water to a suitable, stable outlet.

Condition Where Practice Applies

Dewatering devices are appropriate where the discharge from a trap or basin will be by gravity flow through a riser and pipe outlet system. The skimmer dewatering device is the preferred option. A fixed pipe dewatering device, configured as a perforated vertical riser surrounded by filter fabric and stone material is an alternate option for small structures.

Design Criteria

Skimmer Device

1. Skimmers must be designed so as to float just beneath the water surface to remove the least sediment laden water effectively.
2. Skimmer shall be constructed with a 4 foot long flexible pipe elbow to allow for vertical movement of the skimmer for its designated range of operation.
3. The designer will provide a table that shows all required dimensions for the skimmer. An example of this table is shown in Figure 5.4 on page 5.12. See design example in Appendix B.
4. The skimmer will be provided with vertical travel guides and a resting stone pad set at the appropriate design elevation.

5. The orifice plate will be at the “T” intersection of the perforated skimmer section with the non-perforated extension arm.

Riser-Pipe Device

1. The riser-pipe device is constructed as a fixed rigid structure with a larger diameter pipe as the vertical riser connected to a smaller diameter horizontal pipe barrel.
2. The joint of these two conduits will be anchored by means of a concrete block or welded steel plate to prevent flotation.
3. The riser will be perforated above the bottom of the dewatering zone elevation and wrapped with a geotextile filter fabric to filter out sediment.
4. The filter fabric shall be covered with stone graded as NYSDOT #1, #2, or a blend of both, to protect the fabric from deterioration.
5. An orifice plate shall be placed in the riser at the bottom of the dewatering zone elevation to control the dewatering rate.

Dewatering Drawdown

As a minimum, sediment traps and basins should have their temporary storage dewatered over a 48 hour period to maximize sediment retention. If the soils disturbed within the drainage area will have 60% - 80% fines the settling time should be increased to 4 days. Soils containing greater than 80% fines will need longer settling times but in no case longer than 7 days to maintain the hydraulic performance of the basin for recurring runoff events.

1. Skimmer orifices may be sized by using the design chart shown in Figure 5.3 on page 5.11.
2. Riser-pipe orifice sizes may be approximated by the following formula:

$$A_0 = \frac{A_s \times 2h^{0.5}}{T \times C_d \times 20,428}$$

Where:

A_0 = Areas of the dewatering orifice (ft²)

A_s = Surface area of the basin/trap (ft²)

h = head of water above the orifice (ft)

C_d = 0.6 (contraction coefficient of an orifice)

T = Detention time needed to dewater basin (48 hours minimum)

Therefore, the minimum A_o formula for 48 hrs. reduces to:

$$A_o = \frac{A_r \times 2h^{0.5}}{588,326}$$

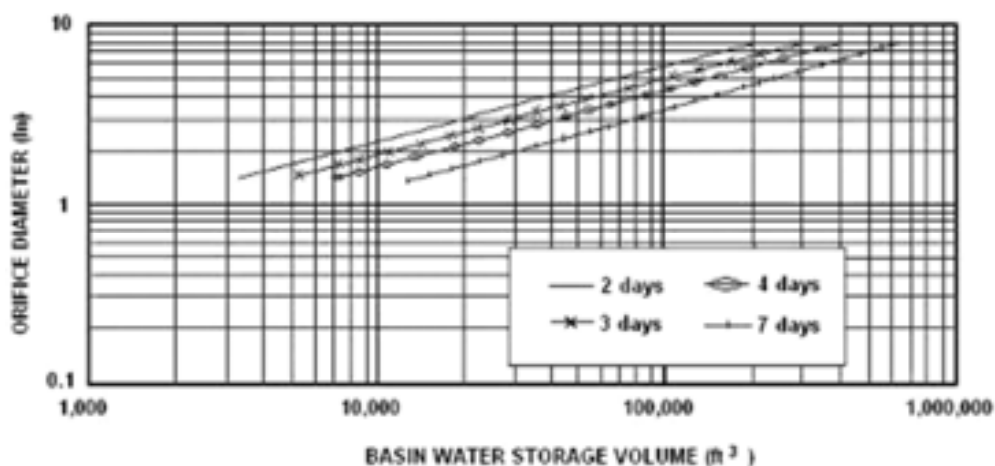
Material Specifications

1. Skimmer Devices - These devices shall be constructed with Schedule 40 PVC pipe with diameters of 4 to 6 inches. The flexible arm shall be equal diameter of non-perforated, corrugated, plastic tubing.
2. Riser-pipe Devices - These devices shall be constructed of Schedule 40 PVC if plastic pipe is used or galvanized corrugated steel or aluminum pipe. The minimum diameter shall be 6 inches if the device is used in conjunction with another permanent riser. All perforations will be at the interior of the corrugations.

Maintenance

1. Dewatering devices shall be inspected weekly and after each runoff event.
2. Filter fabric or media will be replaced as needed.
3. Any malfunctioning skimmer or its components shall be repaired or replaced within 24 hours of inspection notification.
4. Sediment shall be removed from the system when it reaches the level marked in a sediment cleanout stake or the top of the skimmer landing area.
5. The structure shall only be removed when the tributary area has been properly stabilized.

Figure 5.3 - Skimmer Orifice Design Chart

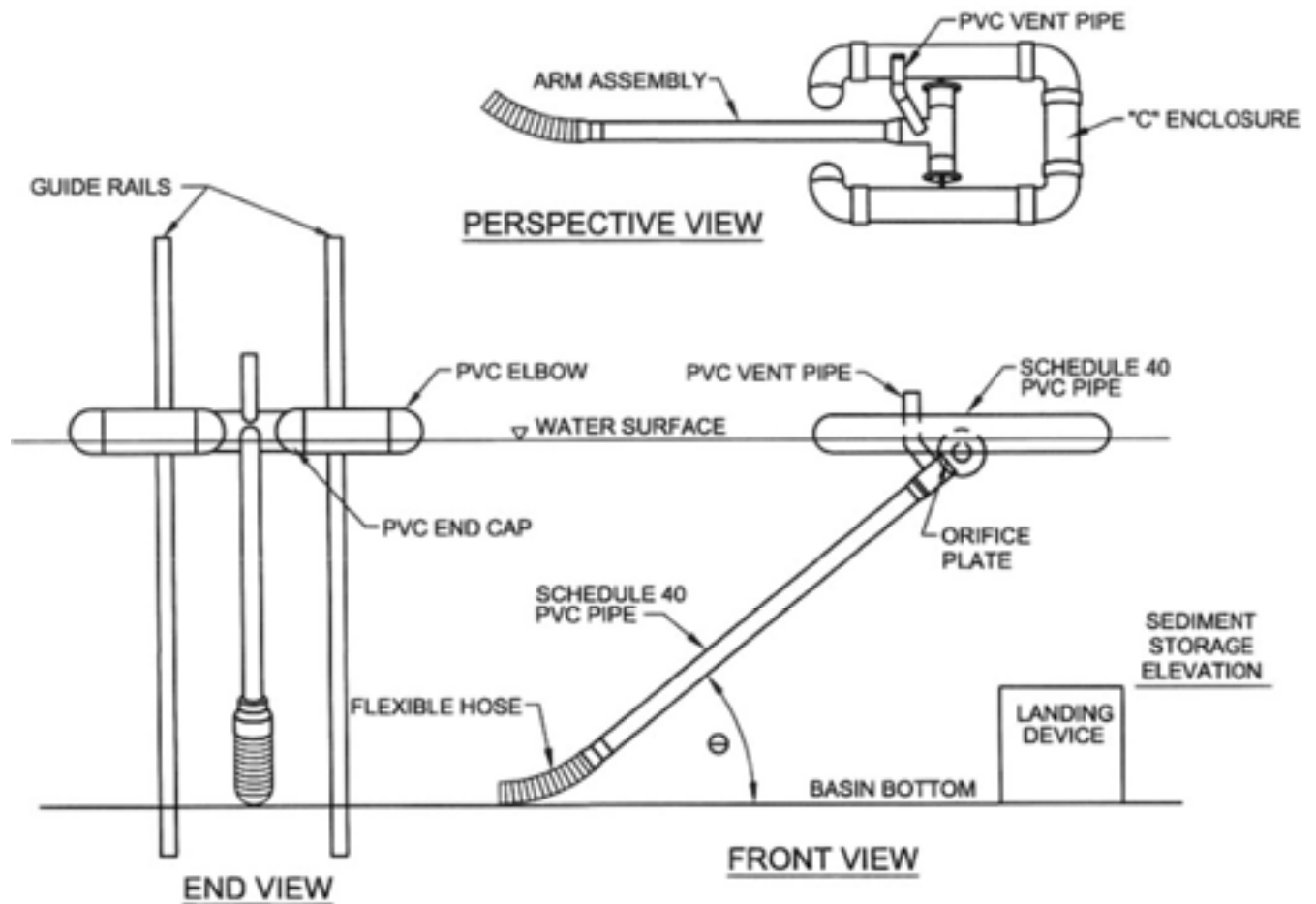


* Figure adapted from Penn State Agricultural and Biological Fact Sheet F-253

Notes:

1. Figure 5.3 is for use in designing the orifice plate for the skimmer shown in Figure 5.4. It assumes 3" to 5" head (depending upon the size of the skimmer). The required head for use of Figure 5.3 varies as follows: For a skimmer with a dewatering tube $\leq 2 \frac{1}{2}$ " diameter, use a 2" head. For a 3" diameter tube, use a 2.5" head; 4" tube, use 3.3" head, 5" tube use 4" head, and 6" diameter tube use 5" head.
2. Find the vertical line representing the basin's dewatering zone volume. At the intersection of the vertical line with the desired dewatering time, read horizontally to the left to find the required skimmer orifice diameter.

Figure 5.4
Skimmer Dewatering Device



* Figure adapted from Penn State Agricultural and Biological Fact Sheet F-253

Basin No.	Water Surface Elevation (ft.)	Arm Length* (ft.)	Arm Dia. (in.)	Orifice Size** (in.)	Top of Landing Device Elevation (ft.)	Flexible Hose Length (in.)	Flexible Hose Attachment Elevation (ft.)

* Minimum Arm length = Full design storage depth x 1.414 (for 45 degree angle)
 ** Must be equal to or less than arm diameter

Skimmer Construction Notes

1. Pipe flotation section shall be solvent welded to ensure an airtight assembly. The contractor is required to conduct a test to check for leaks prior to installation.
2. Skimmer section shall have 12 rows of 1/2" diameter holes, 1 1/4" on center. If additional filtration is necessary, the filtering media shall consist of a Type GD-II geotextile fabric wrapped around the perforated portion of the skimmer and attached with plastic snap ties, bands, etc.
3. Flexible pipe shall be inserted into solid pipe and fastened with 2 #8 wood screws.
4. At a minimum, the structure shall be inspected after each rain and repairs made as needed. If vandalism is a problem, more frequent inspection may be necessary.
5. Construction operations shall be carried out in such a manner that erosion and water pollution are minimized.
6. The structure shall only be removed when the contributing drainage area has been properly stabilized.

Materials

(Note: materials for a 4" diameter arm assembly)

1. Solid Pipe - 4" Schedule 40 PVC
2. Perforated Pipe - 4" Schedule 40 PVC
3. 90° Tee (1 each) - 4" Schedule 40 PVC
4. 90° Elbow (4 each) - 4" Schedule 40 PVC
5. Cap (2 each) - 4" Schedule 40 PVC, solid
6. Flexible pipe - 4" Corrugated Plastic Tubing (non-perforated)

Figure 5.5
Riser Pipe Dewatering Device

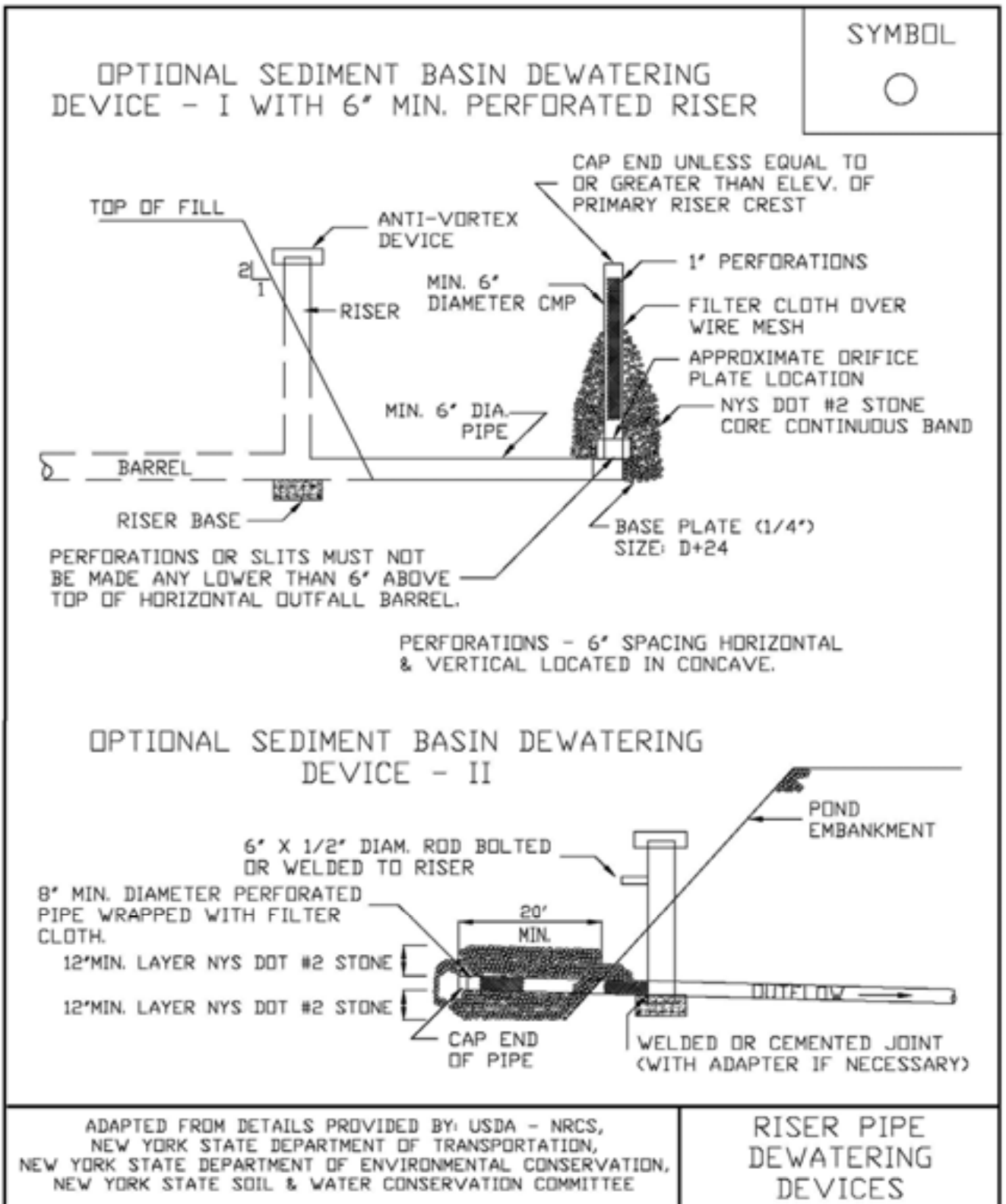


Figure 5.6

Riser Pipe Dewatering Device Construction Notes

Riser Pipe Construction Notes

1. Standpipe and connector pipe shall be a minimum of 6 inches diameter.
2. Metal pipe may be galvanized steel or aluminum; plastic pipe may be Schedule 40 PVC or HDPP.
3. Construction operations shall be carried out in such a manner that erosion and water pollution are minimized.
4. The structure shall only be removed when the contributing drainage area has been properly stabilized.
5. All pipe connections shall be watertight. The lower portion of the standpipe, at a point above the barrel connection, shall be fitted with an internal orifice plate sized to release the volume of the basin no sooner than 48 hours.
6. The top 2/3 of the standpipe shall be perforated with 1 inch diameter hole or slit spaced 6 inches vertically and horizontally and placed in the concave portion of the pipe. No holes will be allowed within 6 inches of the horizontal connector pipe.
7. The riser shall be wrapped with a Type GD-II geotextile fabric. The fabric shall extend 6 inches above the highest hole and 6" below the lowest hole. Where ends of fabric come together, they shall be overlapped, folded and stapled to prevent bypass.
8. Straps or connecting bands shall be used to hold the fabric and wire mesh (as needed) in place. They shall be placed at the top and bottom of the cloth.
9. The standpipe shall be anchored with either concrete base or steel plate base to prevent flotation. Concrete bases shall be 12 inches thick with the standpipe embedded nine inches. Steel plate bases will be 1/4 inch minimum thickness attached to the standpipe by a continuous weld around the bottom to form a watertight connection. The plate shall have 2.5 feet of stone, gravel or tamped earth placed on it.
10. The perforated standpipe shall be surrounded by NYSDOT #1 or #2 stone or a blend of both to protect the filter fabric.

STANDARD AND SPECIFICATIONS FOR GEOTEXTILE FILTER BAG



Definition & Scope

A **temporary** portable device through which sediment laden water is pumped to trap and retain sediment prior to its discharge to drainageways or off-site.

Condition Where Practice Applies

On sites where space is limited such as urban construction or linear projects (e.g. roads and utility work) where rights-of-way are limited and larger de-silting practices are impractical.

Design Criteria

1. Location - The portable filter bag should be located to minimize interference with construction activities and pedestrian traffic. It should also be placed in a location that is vegetated, relatively level, and provides for ease of access by heavy equipment, cleanout, disposal of trapped sediment, and proper release of filtered water.

The filter bag shall also be placed at least 50 feet from all wetlands, streams or other surface waters.

2. Size - Geotextile filter bag shall be sized in accordance with the manufacturers recommendations based on the pump discharge rate.

Materials and Installation

1. The geotextile material will have the following attributes:

Minimum Grab Tensile Strength	200 lbs.
Minimum Grab Tensile Elongation	50 %
Minimum Trapezoid Tear Strength	80 lbs.
Mullen Burst Strength	380 psi
Minimum Puncture Strength	130 lbs
Apparent Opening Size	40 - 80 US sieve
Minimum UV Resistance	70%
Minimum Flow Thru Rate	70 gpm/sq ft

2. The bag shall be sewn with a double needle machine using high strength thread, double stitched "Joe" type capable of minimum roll strength of 100 lbs/inch (ASTM D4884).
3. The geotextile filter bag shall have an opening large enough to accommodate a 4 inch diameter discharge hose with an attached strap to tie off the bag to the hose to prevent back flow.
4. The geotextile shall be placed on a gravel bed 2 inches thick, a straw mat 4 inches thick, or a vegetated filter strip to allow water to flow out of the bag in all directions.

Maintenance

1. The geotextile filter bag is considered full when remaining bag flow area has been reduced by 75%. At this point, it should be replaced with a new bag.
2. Disposal may be accomplished by removing the bag to an appropriate designated upland area, cut open, remove the geotextile for disposal, and spread sediment contents and seeded and mulched according to the vegetative plan.

STANDARD AND SPECIFICATIONS FOR TEMPORARY CONSTRUCTION AREA SEEDING



Definition & Scope

Providing temporary erosion control protection to disturbed areas and/or localized critical areas for an interim period by covering all bare ground that exists as a result of construction activities or a natural event. Critical areas may include but are not limited to steep excavated cut or fill slopes and any disturbed, denuded natural slopes subject to erosion.

Conditions Where Practice Applies

Temporary seedings may be necessary on construction sites to protect an area, or section, where final grading is complete, when preparing for winter work shutdown, or to provide cover when permanent seedings are likely to fail due to mid-summer heat and drought. The intent is to provide temporary protective cover during temporary shutdown of construction and/or while waiting for optimal planting time.

Criteria

Water management practices must be installed as appropriate for site conditions. The area must be rough graded and slopes physically stable. Large debris and rocks are usually removed. Seedbed must be seeded within 24 hours of disturbance or scarification of the soil surface will be necessary prior to seeding.

Fertilizer or lime are not typically used for temporary seedings.

IF: Spring or summer or early fall, then seed the area with ryegrass (annual or perennial) at 30 lbs. per acre (Approximately 0.7 lb./1000 sq. ft. or use 1 lb./1000 sq. ft.).

IF: Late fall or early winter, then seed Certified 'Aroostook' winter rye (cereal rye) at 100 lbs. per acre (2.5 lbs./1000 sq. ft.).

Any seeding method may be used that will provide uniform application of seed to the area and result in relatively good soil to seed contact.

Mulch the area with hay or straw at 2 tons/acre (approx. 90 lbs./1000 sq. ft. or 2 bales). Quality of hay or straw mulch allowable will be determined based on long term use and visual concerns. Mulch anchoring will be required where wind or areas of concentrated water are of concern. Wood fiber hydromulch or other sprayable products approved for erosion control (nylon web or mesh) may be used if applied according to manufacturers' specification. Caution is advised when using nylon or other synthetic products. They may be difficult to remove prior to final seeding and can be a hazard to young wildlife species.

Description

Stockpile management includes measures to minimize erosion and sediment transport from soil stockpiles.

Appropriate Uses

Stockpile management should be used when soils or other erodible materials are stored at the construction site. Special attention should be given to stockpiles in close proximity to natural or manmade storm systems.



Photograph SP-1. A topsoil stockpile that has been partially revegetated and is protected by silt fence perimeter control.

Design and Installation

Locate stockpiles away from all drainage system components including storm sewer inlets. Where practical, choose stockpile locations that that will remain undisturbed for the longest period of time as the phases of construction progress. Place sediment control BMPs around the perimeter of the stockpile, such as sediment control logs, rock socks, silt fence, straw bales and sand bags. See Detail SP-1 for guidance on proper establishment of perimeter controls around a stockpile. For stockpiles in active use, provide a stabilized designated access point on the upgradient side of the stockpile.

Stabilize the stockpile surface with surface roughening, temporary seeding and mulching, erosion control blankets, or soil binders. Soils stockpiled for an extended period (typically for more than 60 days) should be seeded and mulched with a temporary grass cover once the stockpile is placed (typically within 14 days). Use of mulch only or a soil binder is acceptable if the stockpile will be in place for a more limited time period (typically 30-60 days). Timeframes for stabilization of stockpiles noted in this fact sheet are "typical" guidelines. Check permit requirements for specific federal, state, and/or local requirements that may be more prescriptive.

Stockpiles should not be placed in streets or paved areas unless no other practical alternative exists. See the Stabilized Staging Area Fact Sheet for guidance when staging in roadways is unavoidable due to space or right-of-way constraints. For paved areas, rock socks must be used for perimeter control and all inlets with the potential to receive sediment from the stockpile (even from vehicle tracking) must be protected.

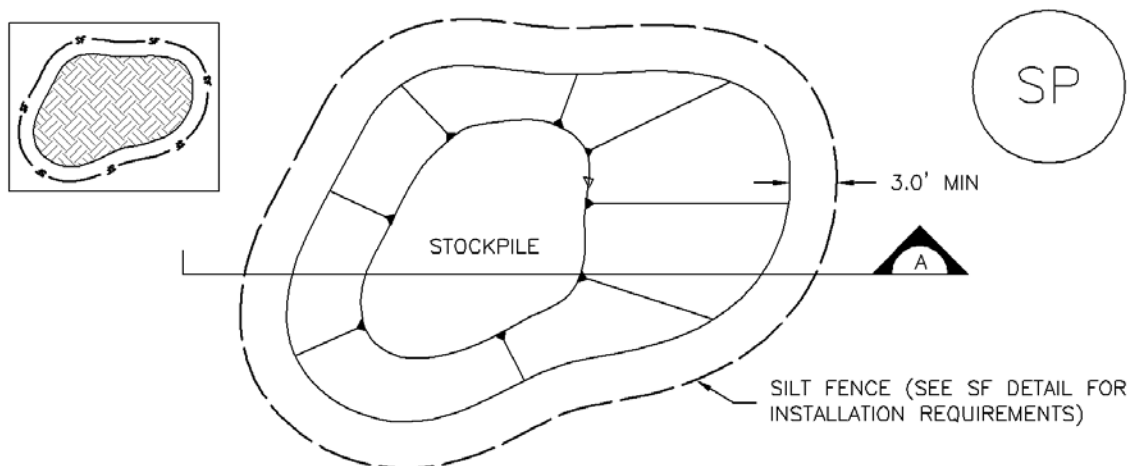
Maintenance and Removal

Inspect perimeter controls and inlet protection in accordance with their respective BMP Fact Sheets. Where seeding, mulch and/or soil binders are used, reseeding or reapplication of soil binder may be necessary.

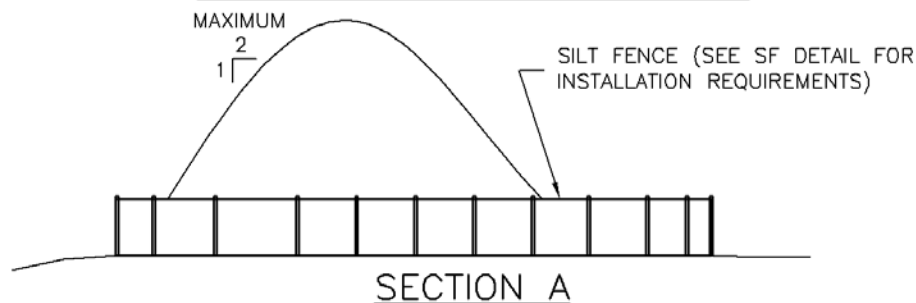
When temporary removal of a perimeter BMP is necessary to access a stockpile, ensure BMPs are reinstalled in accordance with their respective design detail section.

Stockpile Management	
Functions	
Erosion Control	Yes
Sediment Control	Yes
Site/Material Management	Yes

When the stockpile is no longer needed, properly dispose of excess materials and revegetate or otherwise stabilize the ground surface where the stockpile was located.



STOCKPILE PROTECTION PLAN



SECTION A

SP-1. STOCKPILE PROTECTION

STOCKPILE PROTECTION INSTALLATION NOTES

1. SEE PLAN VIEW FOR:
 -LOCATION OF STOCKPILES.
 -TYPE OF STOCKPILE PROTECTION.
2. INSTALL PERIMETER CONTROLS IN ACCORDANCE WITH THEIR RESPECTIVE DESIGN DETAILS. SILT FENCE IS SHOWN IN THE STOCKPILE PROTECTION DETAILS; HOWEVER, OTHER TYPES OF PERIMETER CONTROLS INCLUDING SEDIMENT CONTROL LOGS OR ROCK SOCKS MAY BE SUITABLE IN SOME CIRCUMSTANCES. CONSIDERATIONS FOR DETERMINING THE APPROPRIATE TYPE OF PERIMETER CONTROL FOR A STOCKPILE INCLUDE WHETHER THE STOCKPILE IS LOCATED ON A PERVIOUS OR IMPERVIOUS SURFACE, THE RELATIVE HEIGHTS OF THE PERIMETER CONTROL AND STOCKPILE, THE ABILITY OF THE PERIMETER CONTROL TO CONTAIN THE STOCKPILE WITHOUT FAILING IN THE EVENT THAT MATERIAL FROM THE STOCKPILE SHIFTS OR SLUMPS AGAINST THE PERIMETER, AND OTHER FACTORS.
3. STABILIZE THE STOCKPILE SURFACE WITH SURFACE ROUGHENING, TEMPORARY SEEDING AND MULCHING, EROSION CONTROL BLANKETS, OR SOIL BINDERS. SOILS STOCKPILED FOR AN EXTENDED PERIOD (TYPICALLY FOR MORE THAN 60 DAYS) SHOULD BE SEEDDED AND MULCHED WITH A TEMPORARY GRASS COVER ONCE THE STOCKPILE IS PLACED (TYPICALLY WITHIN 14 DAYS). USE OF MULCH ONLY OR A SOIL BINDER IS ACCEPTABLE IF THE STOCKPILE WILL BE IN PLACE FOR A MORE LIMITED TIME PERIOD (TYPICALLY 30-60 DAYS).
4. FOR TEMPORARY STOCKPILES ON THE INTERIOR PORTION OF A CONSTRUCTION SITE, WHERE OTHER DOWNGRADIENT CONTROLS, INCLUDING PERIMETER CONTROL, ARE IN PLACE, STOCKPILE PERIMETER CONTROLS MAY NOT BE REQUIRED.

STOCKPILE PROTECTION MAINTENANCE NOTES

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.

2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.

3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.

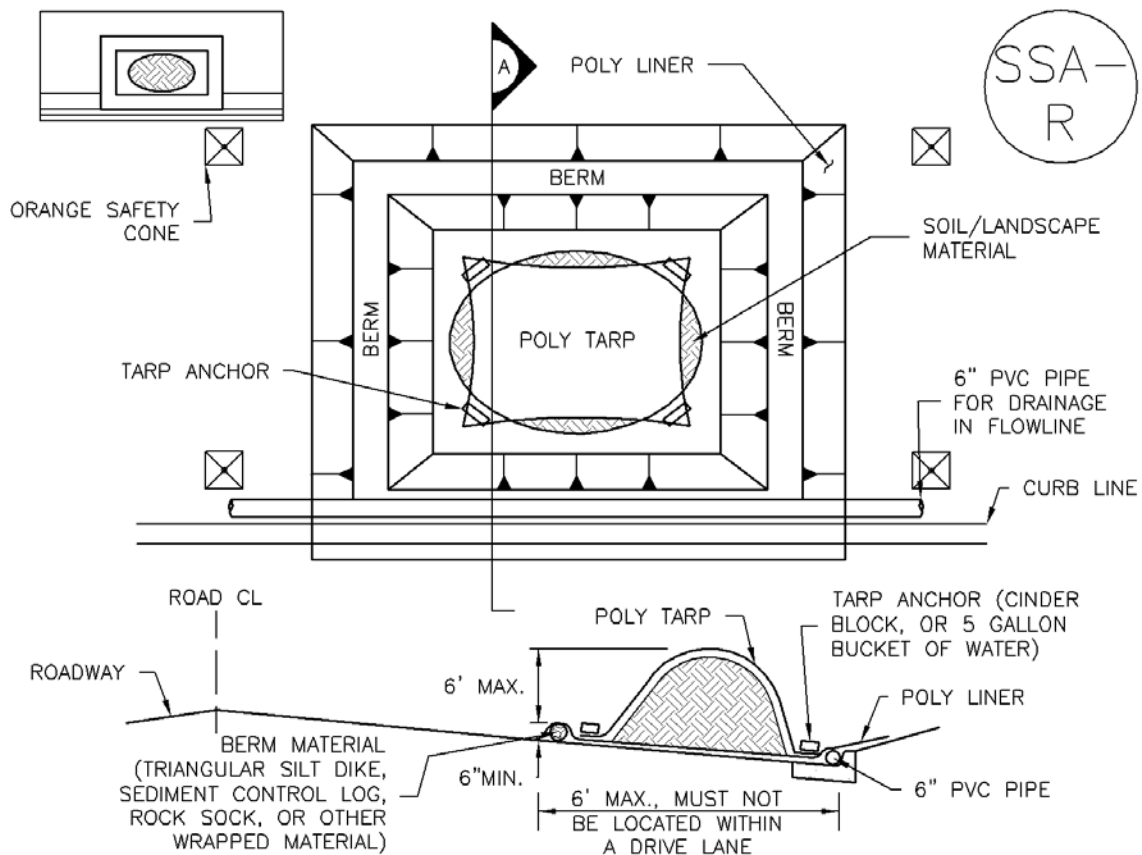
STOCKPILE PROTECTION MAINTENANCE NOTES

4. IF PERIMETER PROTECTION MUST BE MOVED TO ACCESS SOIL STOCKPILE, REPLACE PERIMETER CONTROLS BY THE END OF THE WORKDAY.

5. STOCKPILE PERIMETER CONTROLS CAN BE REMOVED ONCE ALL THE MATERIAL FROM THE STOCKPILE HAS BEEN USED.

(DETAILS ADAPTED FROM PARKER, COLORADO, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.



SP-2. MATERIALS STAGING IN ROADWAY

MATERIALS STAGING IN ROADWAYS INSTALLATION NOTES

1. SEE PLAN VIEW FOR
 - LOCATION OF MATERIAL STAGING AREA(S).
 - CONTRACTOR MAY ADJUST LOCATION AND SIZE OF STAGING AREA WITH APPROVAL FROM THE LOCAL JURISDICTION.
2. FEATURE MUST BE INSTALLED PRIOR TO EXCAVATION, EARTHWORK OR DELIVERY OF MATERIALS.
3. MATERIALS MUST BE STATIONED ON THE POLY LINER. ANY INCIDENTAL MATERIALS DEPOSITED ON PAVED SECTION OR ALONG CURB LINE MUST BE CLEANED UP PROMPTLY.
4. POLY LINER AND TARP COVER SHOULD BE OF SIGNIFICANT THICKNESS TO PREVENT DAMAGE OR LOSS OF INTEGRITY.
5. SAND BAGS MAY BE SUBSTITUTED TO ANCHOR THE COVER TARP OR PROVIDE BERMING UNDER THE BASE LINER.
6. FEATURE IS NOT INTENDED FOR USE WITH WET MATERIAL THAT WILL BE DRAINING AND/OR SPREADING OUT ON THE POLY LINER OR FOR DEMOLITION MATERIALS.
7. THIS FEATURE CAN BE USED FOR:
 - UTILITY REPAIRS.
 - WHEN OTHER STAGING LOCATIONS AND OPTIONS ARE LIMITED.
 - OTHER LIMITED APPLICATION AND SHORT DURATION STAGING.

MATERIALS STAGING IN ROADWAY MAINTENANCE NOTES

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. INSPECT PVC PIPE ALONG CURB LINE FOR CLOGGING AND DEBRIS. REMOVE OBSTRUCTIONS PROMPTLY.
5. CLEAN MATERIAL FROM PAVED SURFACES BY SWEEPING OR VACUUMING.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM AURORA, COLORADO)

APPENDIX H

PIPE CALCULATIONS

STORM PIPE CAPACITY CALCULATIONS FOR PROPOSED CONDITIONS

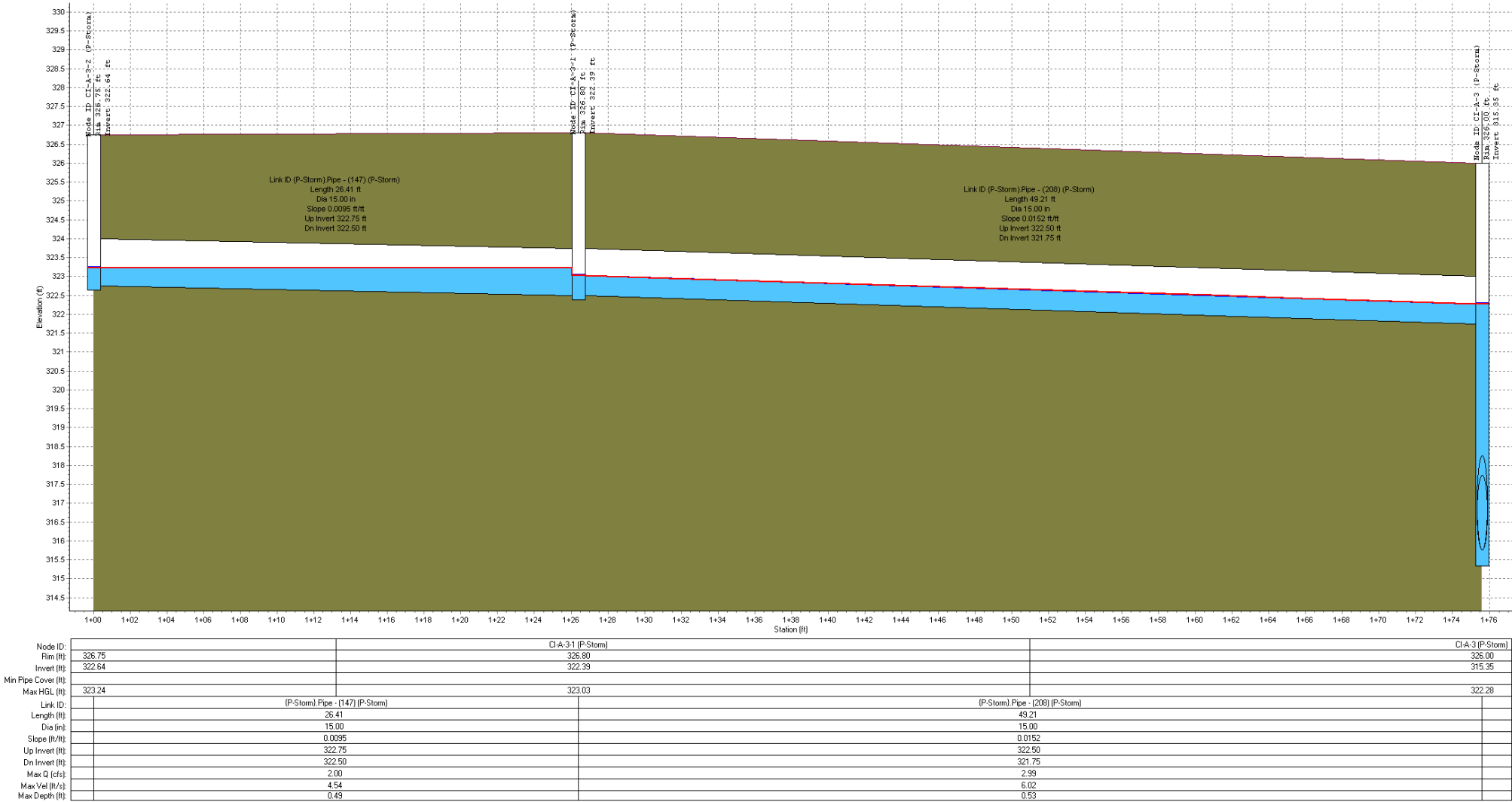
25 YEAR STORM

From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Pipe Diameter or Height	Manning's Roughness	Peak Flow	Max Flow Velocity	Design Flow Capacity	Max Flow Depth / Total Depth Ratio
		(ft)	(ft)	(ft)	(%)	(inches)		(cfs)	(ft/sec)	(cfs)	
CI-A-1-1	WQS-A-1	42.54	315.50	315.25	0.5900	15.000	0.0130	3.70	4.43	4.95	0.65
CI-A-3-2	CI-A-3-1	26.41	322.75	322.50	0.9500	15.000	0.0130	2.00	4.54	6.29	0.39
CI-A-2-1	CI-A-2	26.00	317.50	317.25	0.9600	15.000	0.0130	1.21	3.97	6.33	0.30
CI-A-4-1A	CI-A-4-1	26.00	321.40	321.15	0.9600	15.000	0.0130	2.63	4.92	6.33	0.45
CI-A-5	CI-A-4	30.55	324.00	323.75	0.8200	18.000	0.0130	0.95	3.44	9.50	0.21
CI-C-5-1	CI-C-5	26.01	332.95	332.40	2.1100	15.000	0.0130	1.02	5.02	9.39	0.22
CI-C-2-3-1	CI-C-2-3	26.00	327.75	327.50	0.9600	24.000	0.0130	16.62	7.75	22.18	0.65
CI-C-4	CI-C-3	26.00	327.00	326.85	0.5800	24.000	0.0130	10.31	5.71	17.18	0.56
CI-C-2-5	CI-C-2-4	26.00	330.45	330.15	1.1500	15.000	0.0130	4.85	6.11	6.94	0.62
CI-E-7-2	CI-E-7-1	29.06	329.35	329.05	1.0300	15.000	0.0130	2.82	5.13	6.56	0.46
CI-A-4-1	CI-A-4	141.89	317.50	316.33	0.8200	24.000	0.0130	6.71	5.86	20.54	0.39
CI-E-3-1	CI-E-3	26.00	320.43	320.30	0.5000	15.000	0.0130	3.06	4.00	4.57	0.60
CI-E-3-3	CI-E-3-2	25.76	320.85	320.55	1.1600	15.000	0.0130	0.07	1.79	6.97	0.07
CI-E-4-2	CI-E-4-1	26.00	323.35	323.05	1.1500	15.000	0.0130	1.74	4.70	6.94	0.34
CI-F-9	CI-F-8	26.18	326.60	326.30	1.1500	15.000	0.0130	0.77	3.71	6.92	0.23
CI-F-7-1	CI-F-7	26.00	317.25	317.00	0.9600	15.000	0.0130	1.39	4.13	6.33	0.32
CI-F-6-1	CI-F-6	26.65	309.85	309.55	1.1300	15.000	0.0130	1.45	4.43	6.85	0.31
CI-F-5-1	CI-F-5	26.00	304.70	304.40	1.1500	15.000	0.0130	7.46	6.47	6.94	1.00
CI-F-3-1	CI-F-3	26.00	300.60	300.45	0.5800	15.000	0.0130	3.88	4.43	4.91	0.67
CI-F-2-1	CI-F-2	26.00	298.60	298.30	1.1500	15.000	0.0130	5.94	6.35	6.94	0.71
CI-F-2-2	CI-F-2-2A	26.00	305.10	304.80	1.1500	15.000	0.0130	0.27	2.72	6.94	0.13
CI-F-2-3	CI-F-2-3A	26.00	314.85	314.55	1.1500	15.000	0.0130	0.47	3.21	6.94	0.18
CI-GG-4-2-1	CI-GG-4-2	26.00	322.85	322.55	1.1500	15.000	0.0130	1.38	4.40	6.94	0.30
CI-GG-4-4	CI-GG-4-3	25.97	336.85	335.55	5.0100	18.000	0.0130	23.38	15.16	23.50	0.82
CI-GG-4-5-1	CI-GG-4-5	26.00	356.25	356.00	0.9500	15.000	0.0130	1.42	4.14	6.28	0.32
CI-GG-4-6-1	CI-GG-4-6	26.00	366.00	365.75	0.9600	15.000	0.0130	0.22	2.43	6.33	0.13
CI-GG-4-8-1	CI-GG-4-8	26.00	364.07	363.30	2.9600	15.000	0.0130	3.57	8.05	11.12	0.39
MH-A-4-2	CI-A-4-1	62.60	317.85	317.50	0.5700	15.000	0.0130	3.40	4.30	4.86	0.62
CI-A-4-3	MH-A-4-2	39.00	318.25	317.85	1.0200	15.000	0.0130	2.81	5.12	6.51	0.46
CI-D-11	CI-D-10	26.00	367.50	367.25	0.9600	15.000	0.0130	1.63	4.32	6.33	0.35
CI-D-9-1	CI-D-9	26.00	362.00	361.75	0.9600	15.000	0.0130	1.40	4.14	6.33	0.32
CI-D-8-1-1	CI-D-8-1	26.00	357.24	356.90	1.3200	15.000	0.0130	0.59	3.61	7.41	0.19
CI-D-7-1	CI-D-7	26.00	352.50	352.25	0.9600	18.000	0.0130	2.30	4.72	10.30	0.32
CI-A-4	CI-A-3	168.67	316.33	315.75	0.3400	24.000	0.0130	8.61	4.52	13.27	0.59
CI-H-4-1	CI-H-4	25.91	350.35	349.00	5.2100	15.000	0.0130	7.25	11.94	14.75	0.50
CI-H-3-1	CI-H-3-2	25.97	357.75	357.50	0.9600	15.000	0.0130	1.10	3.88	6.34	0.28
CI-H-5-1	CI-H-5	26.02	355.35	354.25	4.2300	15.000	0.0130	2.13	7.93	13.28	0.27
DI-J-5	DI-J-4	122.94	309.00	308.00	0.8100	15.000	0.0130	2.15	4.40	5.83	0.42
DI-J-4	DI-J-3	120.02	308.00	306.75	1.0400	15.000	0.0130	2.85	5.18	6.59	0.46
DI-J-3	DI-J-2	117.34	306.75	300.50	5.3300	15.000	0.0130	3.39	9.83	14.91	0.32
DI-J-2	WQS-J-1	82.91	300.50	300.25	0.3000	18.000	0.0130	4.18	3.56	5.77	0.63
DI-J-7	DI-J-6	180.17	334.00	311.00	12.7700	15.000	0.0130	0.89	9.10	23.08	0.13
DI-J-6	DI-J-5	156.98	311.00	309.00	1.2700	15.000	0.0130	1.38	4.58	7.29	0.29
CI-E-3-2	CI-E-3	161.98	320.55	319.40	0.7100	15.000	0.0130	0.13	1.83	5.44	0.11
CI-B-5	CI-B-4	26.00	330.25	330.00	0.9600	15.000	0.0130	1.50	4.22	6.33	0.33
CI-B-4	CI-B-3	129.00	330.00	328.00	1.5500	15.000	0.0130	2.15	5.57	8.04	0.35
CI-H-5	CI-H-4	204.65	354.25	349.00	2.5700	15.000	0.0130	2.96	7.30	10.35	0.37
CI-H-3-2	CI-H-3	128.18	357.50	350.00	5.8500	15.000	0.0130	2.38	9.23	15.63	0.26
CI-H-3	DMH-H-2	163.30	348.00	332.00	9.8000	18.000	0.0130	15.28	18.27	32.88	0.48
WQS-H-2-1	ES-H-1	34.90	300.25	300.00	0.7200	24.000	0.0130	15.28	6.77	19.15	0.68
DMH-H-2	WQS-H-2-1	166.82	326.00	300.25	15.4400	18.000	0.0130	15.27	21.60	41.27	0.42
CI-H-4	CI-H-3	79.17	349.00	348.00	1.2600	18.000	0.0130	11.52	7.63	11.81	0.80
CI-I-3	MH-I-2	61.08	317.39	315.85	2.5200	18.000	0.0130	14.05	10.61	16.69	0.70
WQS-J-1	ES-I-1	12.32	300.25	300.00	2.0300	24.000	0.0130	18.21	10.56	32.22	0.54

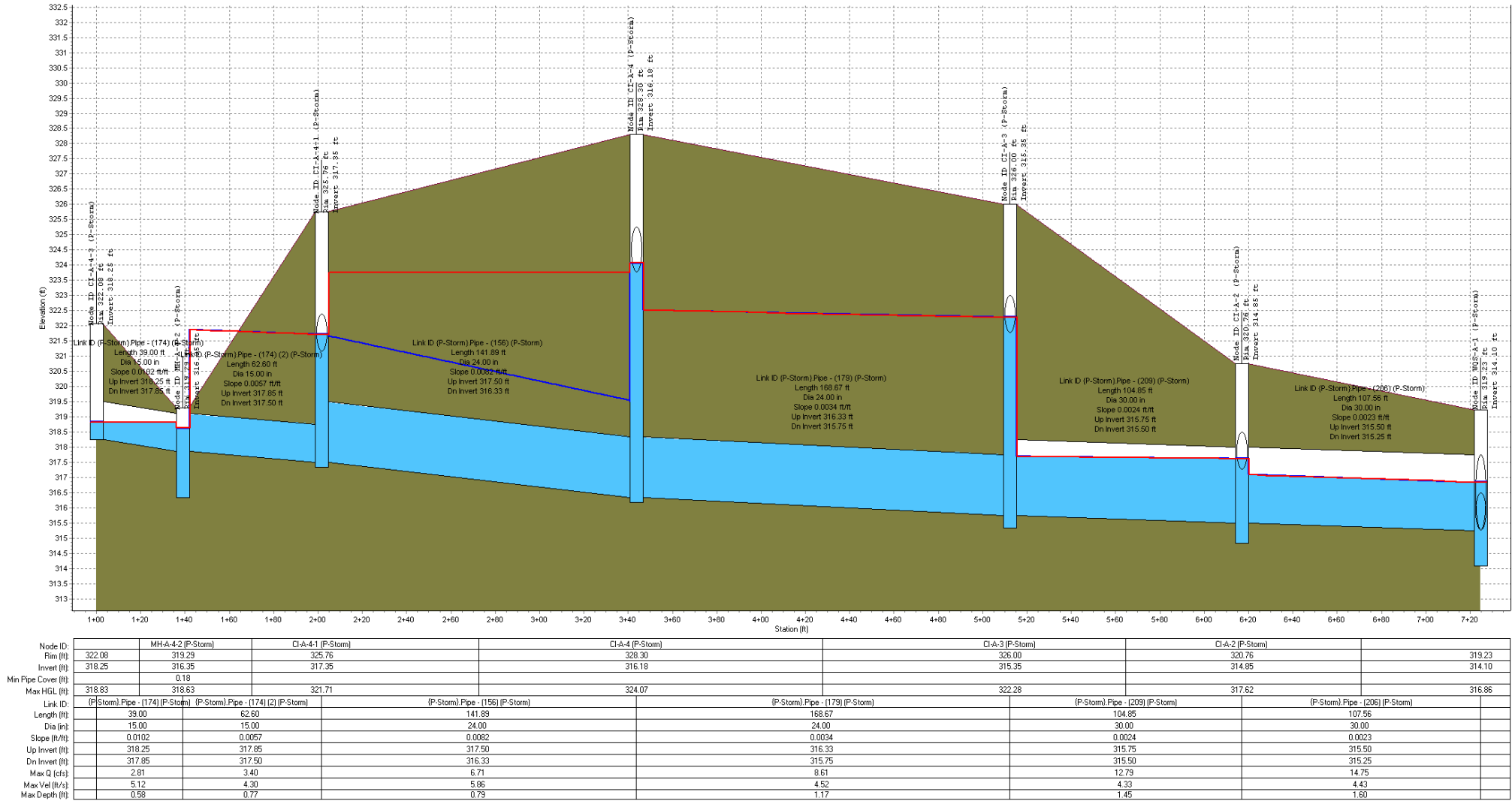
MH-I-2	WQS-J-1	119.35	313.00	300.25	10.6800	18.000	0.0130	14.08	18.48	34.33	0.45
CI-I-4-1	CI-I-4	26.00	324.15	324.00	0.5800	15.000	0.0130	2.83	4.14	4.91	0.55
CI-I-6-1	CI-I-6	26.03	342.65	342.40	0.9600	15.000	0.0130	0.80	3.53	6.33	0.24
CI-I-8-1	CI-I-8	26.00	355.35	355.00	1.3500	15.000	0.0130	0.11	2.23	7.49	0.09
MH-I-9-7	CI-I-6	101.39	349.05	342.40	6.5600	15.000	0.0130	1.91	8.98	16.54	0.23
CI-I-8	MH-I-9-7	91.28	355.00	349.05	6.5200	15.000	0.0130	1.90	8.96	16.50	0.23
MH-I-9-5	CI-I-4	116.96	333.30	322.00	9.6600	15.000	0.0130	3.53	12.30	20.08	0.28
CI-I-6	MH-I-9-5	95.66	342.40	333.30	9.5100	15.000	0.0130	3.54	12.25	19.92	0.29
CI-I-4	CI-I-3	200.11	322.00	317.39	2.3000	15.000	0.0130	6.93	8.67	9.80	0.62
CI-I-9	CI-I-8	35.63	356.50	355.35	3.2300	15.000	0.0130	1.57	6.61	11.61	0.25
CI-A-2	WQS-A-1	107.56	315.50	315.25	0.2300	30.000	0.0130	14.75	4.43	19.78	0.64
WQS-A-1	EX-DMH	41.00	315.25	315.00	0.6100	30.000	0.0130	19.05	6.80	32.03	0.56
CI-A-3-1	CI-A-3	49.21	322.50	321.75	1.5200	15.000	0.0130	2.99	6.02	7.98	0.42
CI-A-3	CI-A-2	104.85	315.75	315.50	0.2400	30.000	0.0130	12.79	4.33	20.03	0.58
CI-B-3-1	CI-B-3	29.66	337.00	336.75	0.8400	15.000	0.0130	0.10	1.82	5.93	0.09
OCS-1	DMH-A-7	188.56	345.00	336.00	4.7700	18.000	0.0130	0.00	0.00	22.95	0.00
CI-B-3	WQS-B-2	180.68	328.00	326.00	1.1100	15.000	0.0130	2.67	5.23	6.80	0.44
WQS-B-2	ES-B-1	225.51	326.00	325.00	0.4400	15.000	0.0130	2.65	3.73	4.30	0.57
CI-D-8-1	CI-D-8	54.86	356.90	356.20	1.2800	15.000	0.0130	5.62	6.56	7.30	0.66
MH-D-7-2	CI-D-7	99.72	354.64	352.50	2.1400	24.000	0.0130	17.69	10.72	33.10	0.52
CI-D-8	MH-D-7-2	71.84	356.20	354.64	2.1800	24.000	0.0130	17.41	10.74	33.39	0.51
CI-D-10	CI-D-9	199.21	367.25	361.25	3.0100	15.000	0.0130	3.72	8.24	11.21	0.40
CI-D-9	CI-D-8	138.51	361.25	356.20	3.6500	15.000	0.0130	9.07	11.00	12.33	0.64
CI-D-8-2	CI-D-8-1	116.49	359.50	356.90	2.2300	15.000	0.0130	3.44	7.20	9.65	0.41
CI-D-6-1	CI-D-6	38.15	356.35	355.75	1.5700	15.000	0.0130	1.92	5.40	8.10	0.33
CI-D-7	CI-D-6	140.51	352.25	350.75	1.0700	24.000	0.0130	22.40	8.51	23.37	0.78
CI-D-4-1	CI-D-4	26.00	353.25	353.00	0.9600	15.000	0.0130	1.42	4.16	6.33	0.32
CI-D-2-1	WQS-D-2	26.01	346.75	346.00	2.8800	15.000	0.0130	1.29	5.98	10.97	0.23
WQS-D-2	ES-D-1	195.69	346.00	345.00	0.5100	30.000	0.0130	30.63	7.01	29.32	0.93
CI-D-6	CI-D-5	74.85	350.75	350.00	1.0000	30.000	0.0130	24.56	8.73	41.06	0.56
CI-D-5	CI-D-4	130.93	350.00	348.50	1.1500	30.000	0.0130	24.64	9.20	43.90	0.54
MH-D-3	WQS-D-2	119.21	347.25	346.00	1.0500	30.000	0.0130	27.57	9.13	42.00	0.59
CI-D-4	MH-D-3	88.36	348.50	347.25	1.4100	30.000	0.0130	27.31	10.21	48.79	0.54
CI-C-2-4	CI-C-2-3	196.21	330.15	327.50	1.3500	15.000	0.0130	7.40	7.05	7.51	0.81
CI-C-2-3	CI-C-2-2	161.70	327.25	326.55	0.4300	30.000	0.0130	26.39	6.28	26.99	0.80
CI-C-5	CI-C-4	35.36	332.40	332.15	0.7100	15.000	0.0130	5.21	5.05	5.43	0.79
CI-C-3	WQS-C-2	202.15	326.65	325.25	0.6900	24.000	0.0130	13.84	6.58	18.83	0.64
WQS-C-2	ES-C-1	45.79	325.25	325.00	0.5500	36.000	0.0130	40.74	7.80	49.29	0.69
CI-C-7	CI-C-6	26.00	338.65	338.40	0.9600	15.000	0.0130	1.90	4.51	6.33	0.38
CI-C-6	CI-C-5	165.85	338.40	332.40	3.6200	15.000	0.0130	3.23	8.47	12.29	0.35
CI-GG-4-8	CI-GG-4-7	168.65	363.30	360.35	1.7500	18.000	0.0130	13.23	9.00	13.89	0.78
CI-GG-4-7	CI-GG-4-6	60.98	360.35	358.35	3.2800	18.000	0.0130	15.17	11.96	19.02	0.68
CI-GG-4-6	CI-GG-4-5	195.50	358.35	351.05	3.7300	18.000	0.0130	16.60	12.84	20.30	0.69
CI-GG-4-5	CI-GG-4-4	196.02	351.05	336.85	7.2400	18.000	0.0130	19.19	17.19	28.27	0.60
CI-GG-4-3	CI-GG-4-2	193.77	335.55	322.55	6.7100	18.000	0.0130	24.42	17.42	27.21	0.74
DMH-A-7	DMH-A-6	117.89	336.00	335.00	0.8500	18.000	0.0130	0.00	0.00	9.67	0.00
CI-F-2-3A	CI-F-2-2A	119.21	314.55	304.80	8.1800	15.000	0.0130	0.83	7.66	18.47	0.14
CI-F-2-2A	CI-F-2	178.07	304.80	295.00	5.5000	15.000	0.0130	2.39	9.04	15.15	0.27
DMH-A-6	CI-A-5	142.43	335.00	324.00	7.7200	18.000	0.0130	0.00	0.00	29.19	0.00
CI-G-5-1	CI-G-5	26.00	325.85	325.50	1.3500	15.000	0.0130	0.85	4.06	7.49	0.23
DMH-G-2	ES-G-1	107.14	319.05	318.75	0.2800	36.000	0.0130	37.60	5.84	35.29	1.00
WQS-G-3	DMH-G-2	159.04	319.75	319.05	0.4400	36.000	0.0130	47.04	7.30	44.25	1.00
CI-G-6-1	CI-G-6	26.00	337.50	337.25	0.9600	15.000	0.0130	0.54	3.15	6.33	0.20
CI-G-6	CI-G-5	198.54	337.00	325.50	5.7900	15.000	0.0130	12.98	14.32	15.55	0.70
CI-G-5	DMH-G-4	107.84	325.50	319.90	5.1900	18.000	0.0130	20.52	15.26	23.94	0.72
DMH-G-4	WQS-G-3	22.77	319.90	319.75	0.6600	24.000	0.0130	19.73	6.71	18.36	1.00
CI-G-8	CI-G-7	25.80	349.70	349.40	1.1600	15.000	0.0130	0.85	3.85	6.97	0.24
CI-G-7	CI-G-6	198.67	348.60	337.00	5.8400	15.000	0.0130	6.25	12.08	15.61	0.44
CI-GG-4-2	CI-G-4-1	99.16	322.55	320.75	1.8200	24.000	0.0130	26.56	10.93	30.48	0.72
CI-F-8	CI-F-7	189.90	326.30	317.00	4.9000	15.000	0.0130	3.52	9.67	14.30	0.34
CI-F-7	CI-F-6	219.12	317.00	308.00	4.1100	15.000	0.0130	7.03	10.89	13.09	0.52
CI-F-6	CI-F-5	201.23	308.00	302.85	2.5600	18.000	0.0130	9.66	9.86	16.80	0.54
MH-F-4	CI-F-3	93.96	296.48	296.00	0.5100	30.000	0.0130	17.26	6.22	29.35	0.55

CI-F-5	MH-F-4	130.46	297.00	296.48	0.4000	30.000	0.0130	17.29	5.65	25.87	0.60
CI-F-3	CI-F-2	154.00	296.00	295.00	0.6500	30.000	0.0130	21.41	7.16	33.05	0.59
WQS F-1-1	ES-F-1	32.49	294.00	290.00	12.3100	30.000	0.0130	30.18	23.19	143.91	0.31
CI-F-2	WQS F-1-1	21.50	295.00	294.00	4.6500	30.000	0.0130	30.19	16.30	88.46	0.40
CI-E-10	CI-E-9	26.00	344.85	344.55	1.1500	15.000	0.0130	1.72	4.68	6.94	0.34
CI-E-9-1	CI-E-8	25.96	330.85	330.55	1.1600	15.000	0.0130	6.71	6.50	6.94	0.80
CI-E-6-1	CI-E-6	26.00	326.85	326.55	1.1500	15.000	0.0130	1.26	4.28	6.94	0.29
CI-E-8	DMH-E-7	122.29	329.50	328.25	1.0200	24.000	0.0130	12.19	7.43	22.87	0.52
DMH-E-7	CI-E-6	77.48	328.25	326.55	2.1900	24.000	0.0130	16.64	10.65	33.51	0.50
CI-E-9	CI-E-8	192.00	344.55	330.55	7.2900	15.000	0.0130	3.47	11.12	17.44	0.30
CI-E-5-1	CI-E-5	26.00	322.90	322.60	1.1500	15.000	0.0130	0.66	3.57	6.94	0.21
CI-E-4	CI-E-3	130.98	321.75	319.40	1.7900	24.000	0.0130	30.08	11.03	30.30	0.81
CI-E-5	CI-E-4	82.24	322.60	321.75	1.0300	24.000	0.0130	22.62	8.37	23.00	0.81
CI-E-4-1	CI-E-4	78.72	323.05	321.75	1.6500	15.000	0.0130	6.82	7.56	8.30	0.69
WQS-E-2	ES-E-1	48.06	319.34	319.00	0.7100	30.000	0.0130	30.10	7.93	34.50	0.72
CI-E-3	WQS-E-2	12.72	319.40	319.34	0.4700	30.000	0.0130	30.02	6.58	28.17	1.00
CI-E-6	CI-E-5	192.60	326.55	322.60	2.0500	24.000	0.0130	20.07	10.87	32.40	0.57
OCS-2	ES-2	250.57	325.00	322.00	1.2000	30.000	0.0130	0.00	0.00	44.88	0.00
OCS-2A	ES-2A	235.43	319.50	319.25	0.1100	18.000	0.0130	0.00	0.00	3.42	0.00
OCS-3A	ES-3A	198.72	310.00	296.00	7.0500	15.000	0.0130	0.00	0.00	17.15	0.00
OCS-3	ES-3	64.37	290.00	289.00	1.5500	15.000	0.0130	0.00	0.00	8.05	0.00
CI-E-7-1	DMH-E-7	40.48	329.05	328.25	1.9800	15.000	0.0130	4.18	7.23	9.08	0.48
CI-C-2-2	DMH-C-2-1	189.03	326.55	325.75	0.4200	30.000	0.0130	27.91	6.32	26.68	0.92
DMH-C-2-1	WQS-C-2	121.77	325.75	325.25	0.4100	36.000	0.0130	27.52	6.48	42.74	0.59
DI-5-1	OCS-5 (NYLOPLAST)	49.00	359.95	359.90	0.1000	24.000	0.0130	0.09	0.78	7.23	0.08
CI-C-3-1	CI-C-3	280.99	337.22	326.65	3.7600	15.000	0.0130	1.73	7.22	12.53	0.25
CI-C-2-4-1	CI-C-2-4	171.14	333.12	330.15	1.7400	15.000	0.0130	1.22	4.96	8.51	0.26
CI-E-4-3	CI-E-4-1	134.18	325.82	323.05	2.0600	15.000	0.0130	3.96	7.29	9.28	0.46
CI-F-8-1	CI-F-8	91.41	328.26	326.30	2.1500	15.000	0.0130	1.54	5.68	9.46	0.27
CI-G-7-1	CI-G-7	151.25	358.18	348.60	6.3400	15.000	0.0130	2.53	9.66	16.26	0.27
CI-E-9-2	CI-E-9	152.44	358.40	344.55	9.0900	15.000	0.0130	0.74	7.64	19.47	0.13
CI-GG-4-7-1	CI-GG-4-7	145.17	365.93	362.05	2.6700	15.000	0.0130	1.50	6.11	10.56	0.25
CI-GG-4-8-2	CI-GG-4-8	134.75	369.23	364.05	3.8500	15.000	0.0130	5.15	9.79	12.67	0.44
CI-D-10-1	CI-D-10	112.31	370.76	367.25	3.1300	15.000	0.0130	0.81	5.40	11.43	0.18
CI-D-8-3	CI-D-8-2	70.56	360.65	359.44	1.7100	15.000	0.0130	2.74	6.17	8.45	0.39
CI-I-10	CI-I-9	174.43	359.50	356.50	1.7200	15.000	0.0130	1.30	5.03	8.47	0.26
CI-C-6-1	CI-C-6	99.51	343.26	338.40	4.8900	15.000	0.0130	0.66	5.96	14.28	0.15
MH-C-2-4-2	CI-C-2-4-1	91.07	334.06	333.12	1.0300	12.000	0.0130	0.61	3.45	3.62	0.28
CI-G-4-1	CI-G-4-1-1	26.00	320.75	320.25	1.9200	24.000	0.0130	29.82	11.36	31.37	0.78
CI-G-4-1-1	WQS-G-3	25.71	320.25	319.75	1.9400	24.000	0.0130	30.43	11.43	31.55	0.79

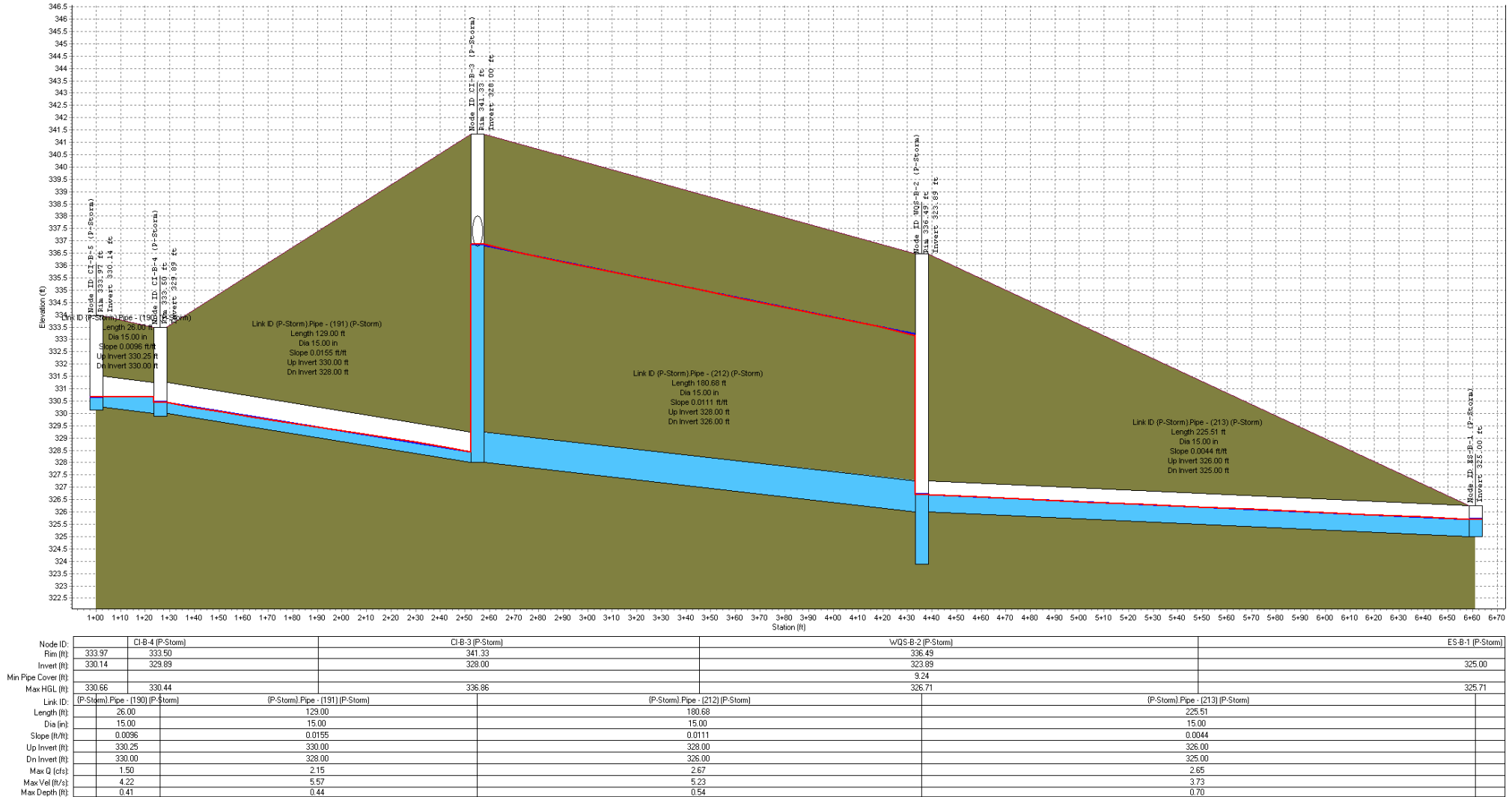
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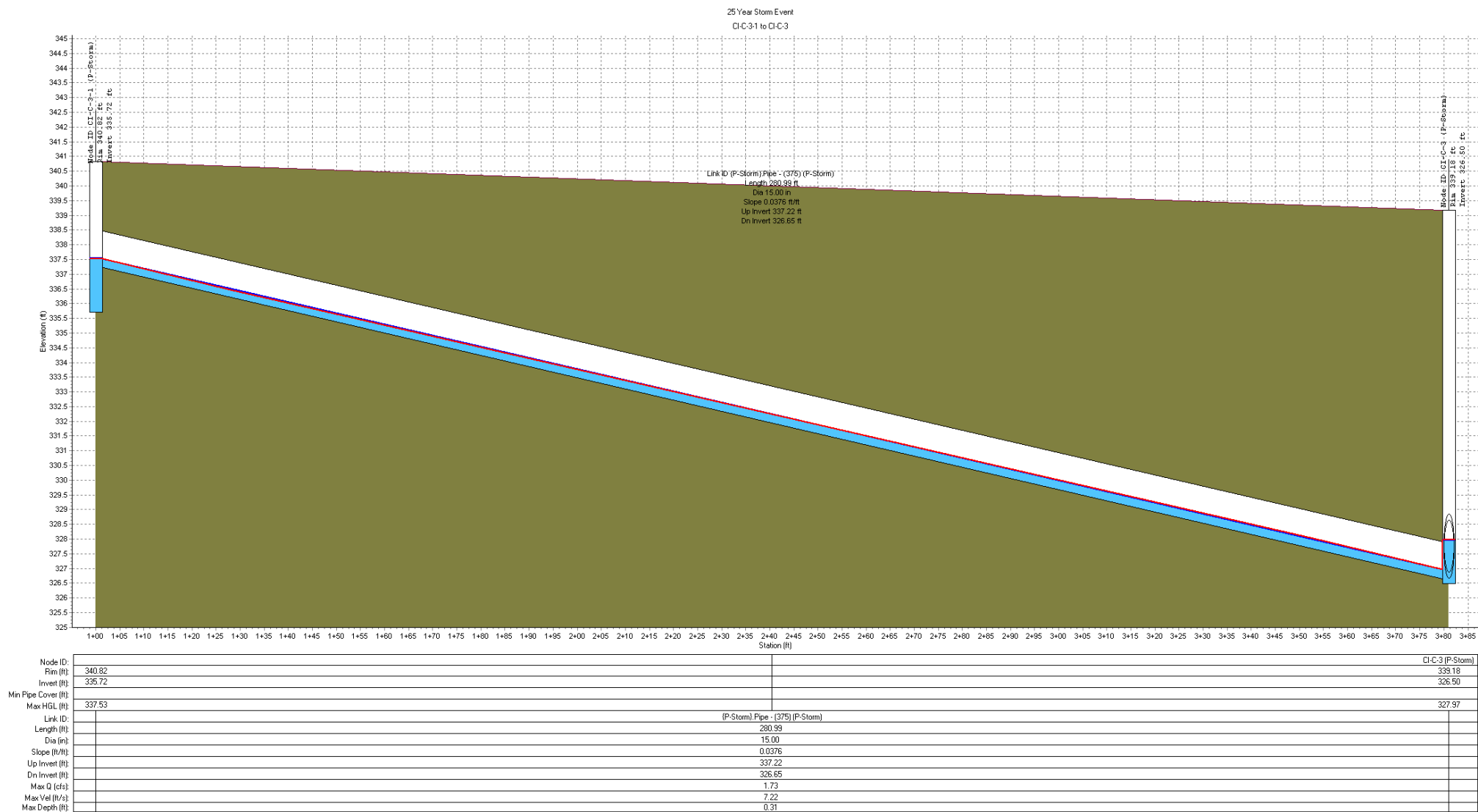


25 Year Storm Event
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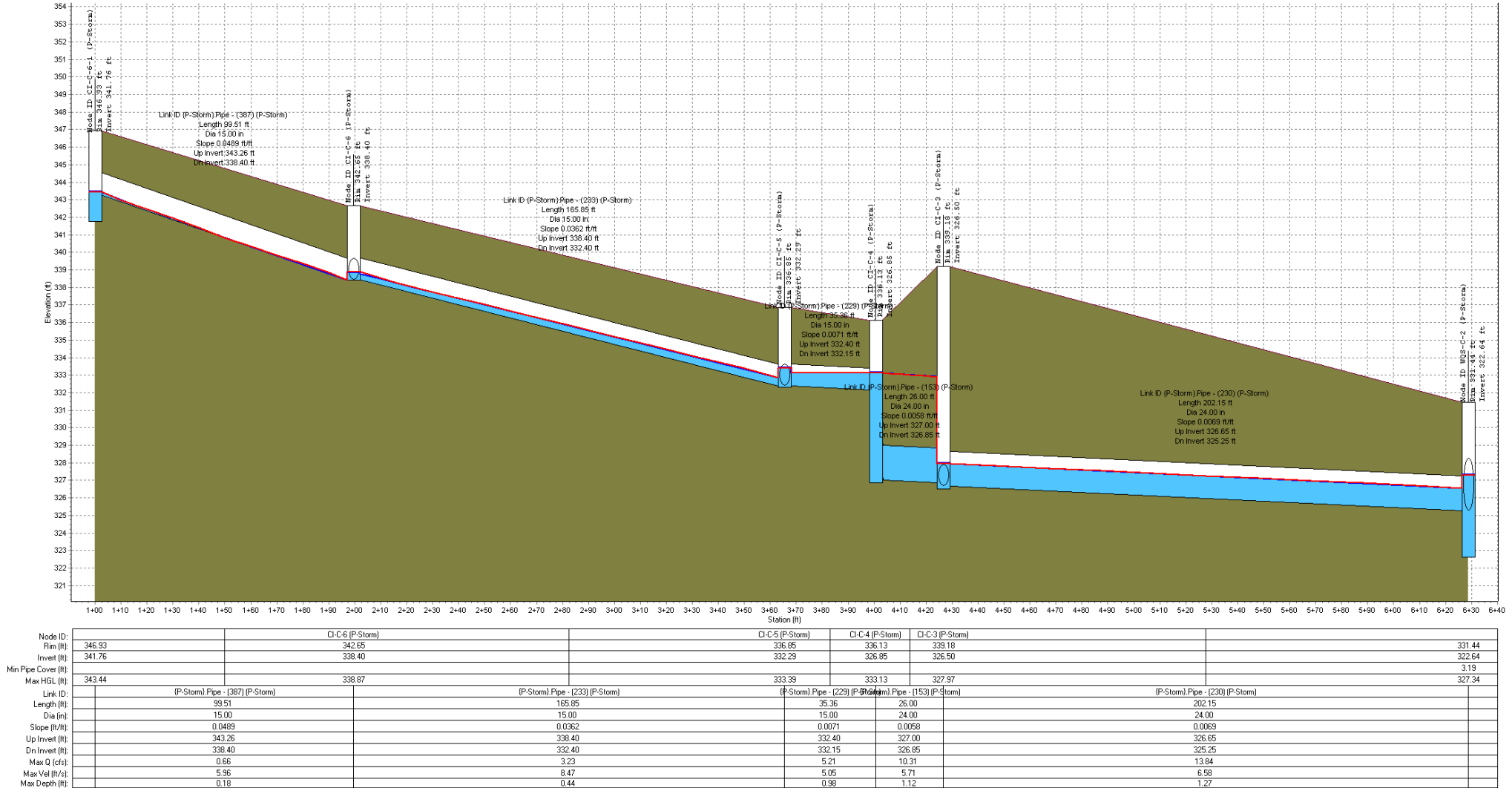


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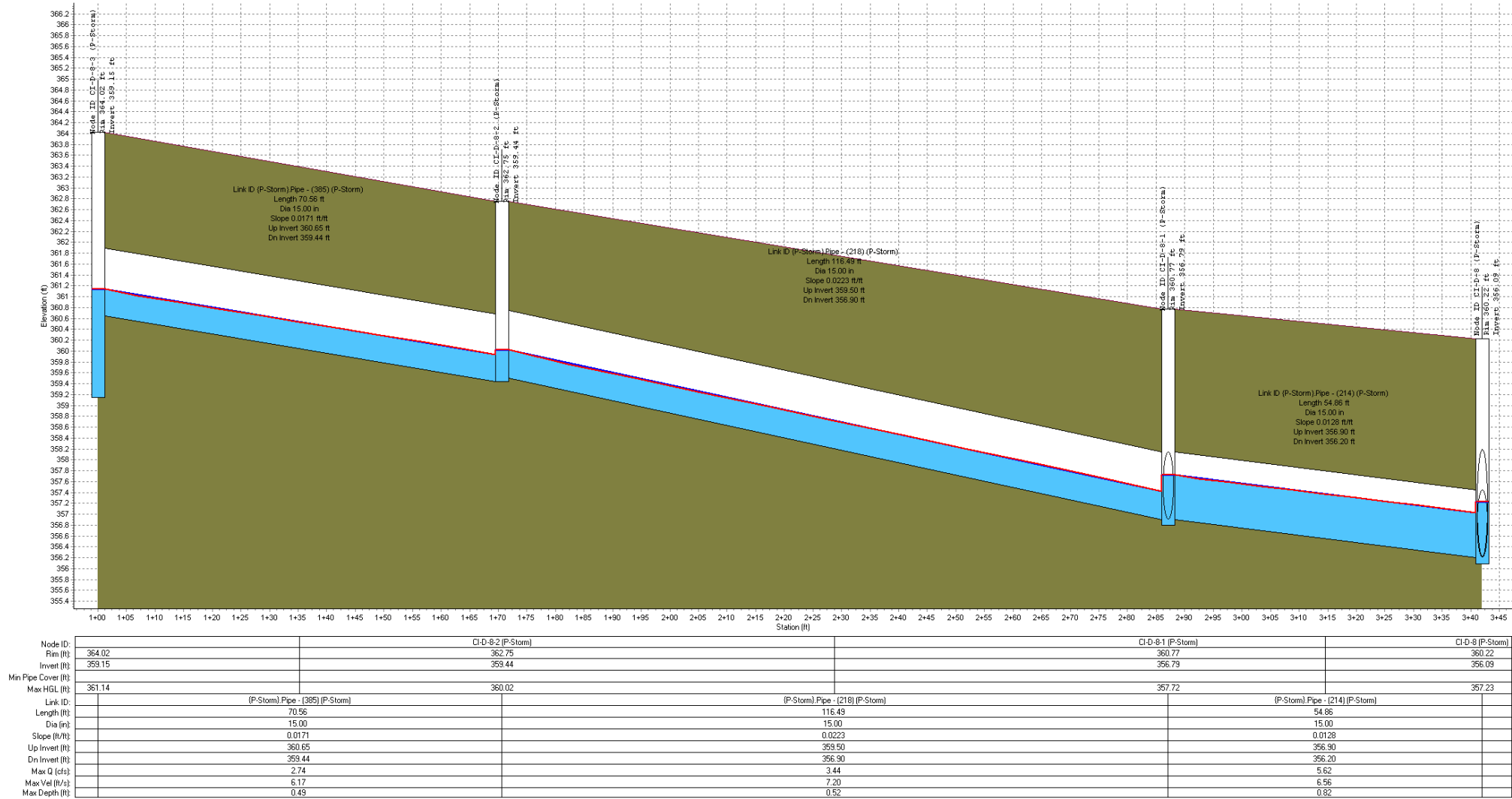




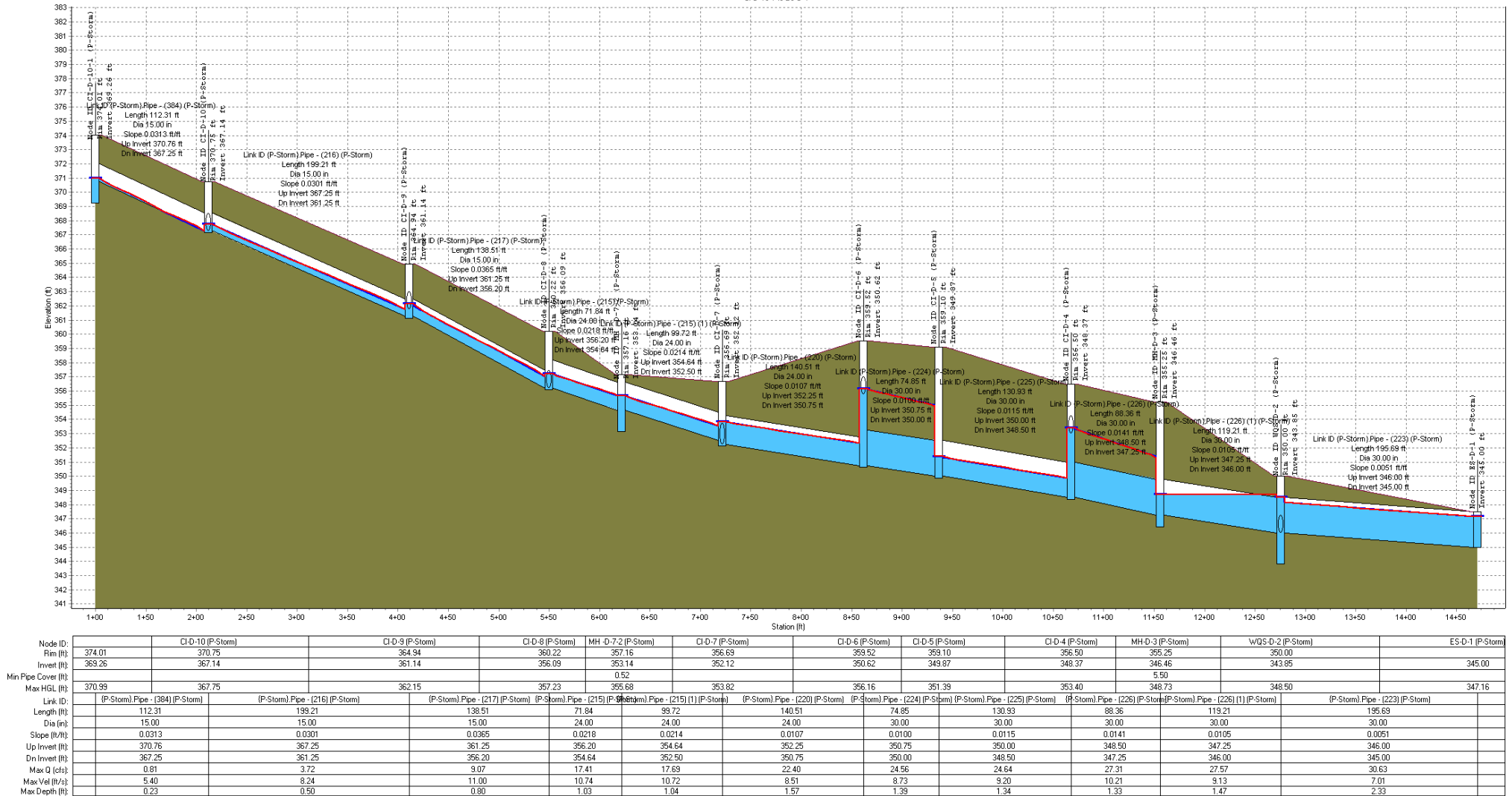
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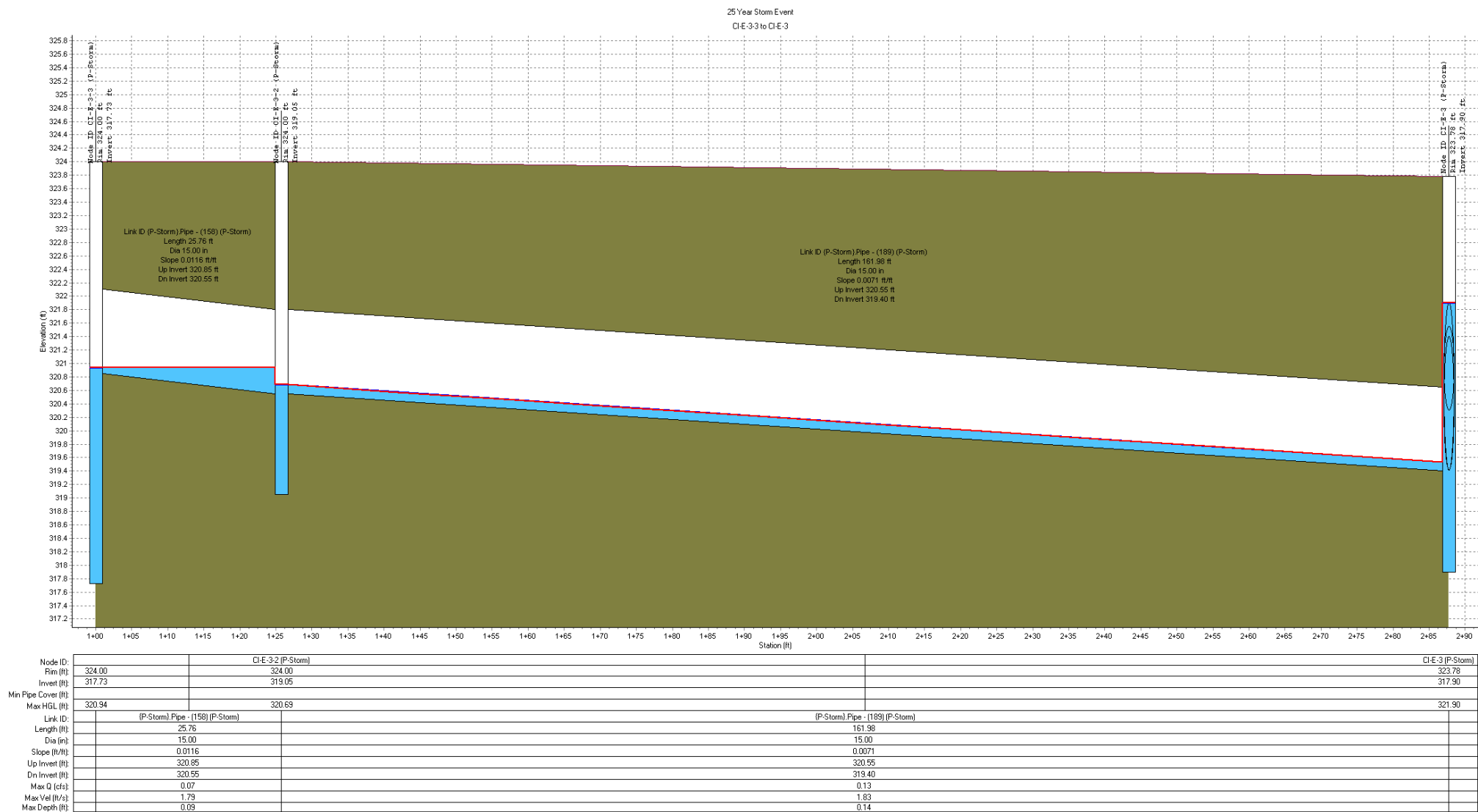


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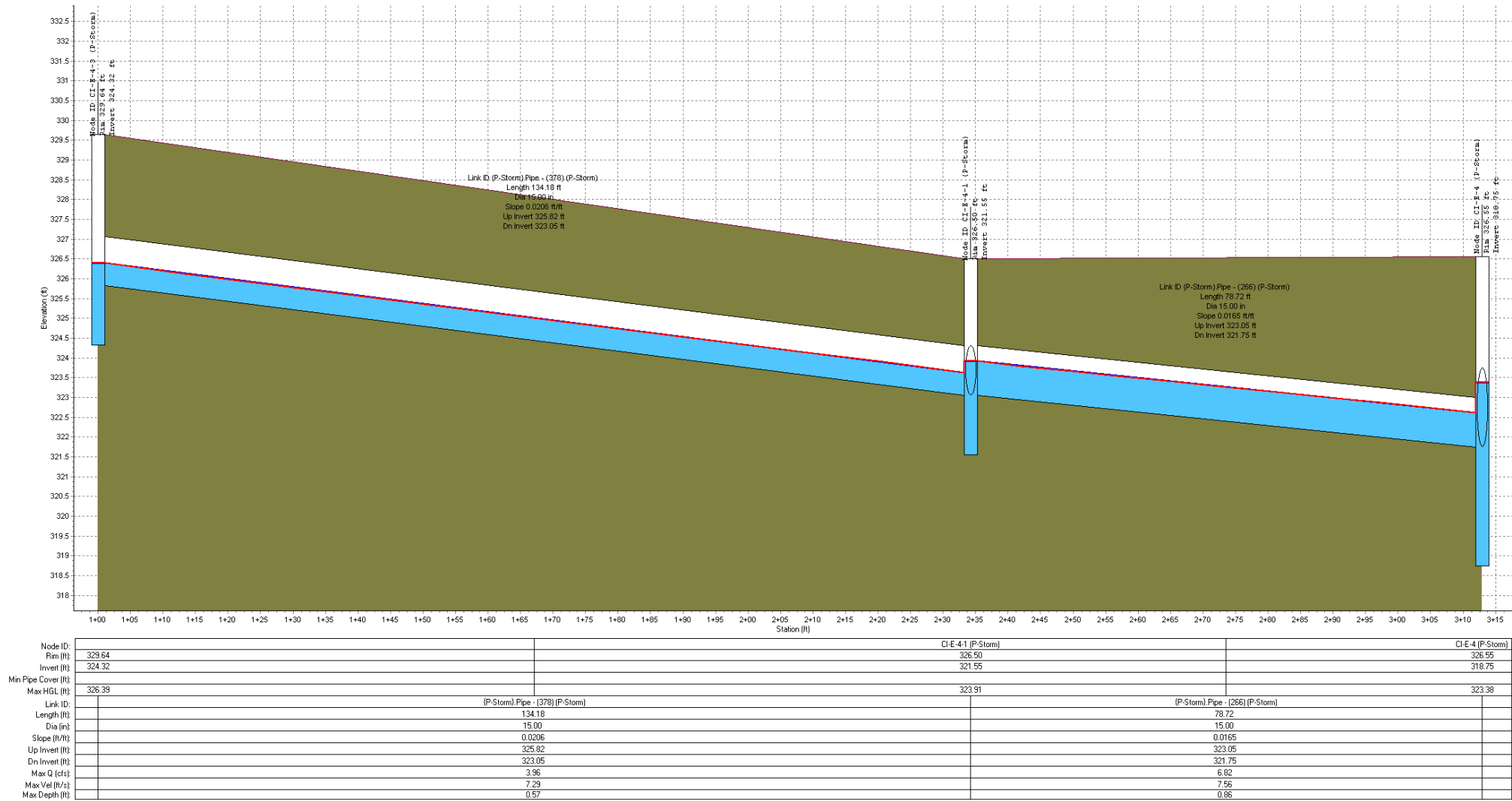


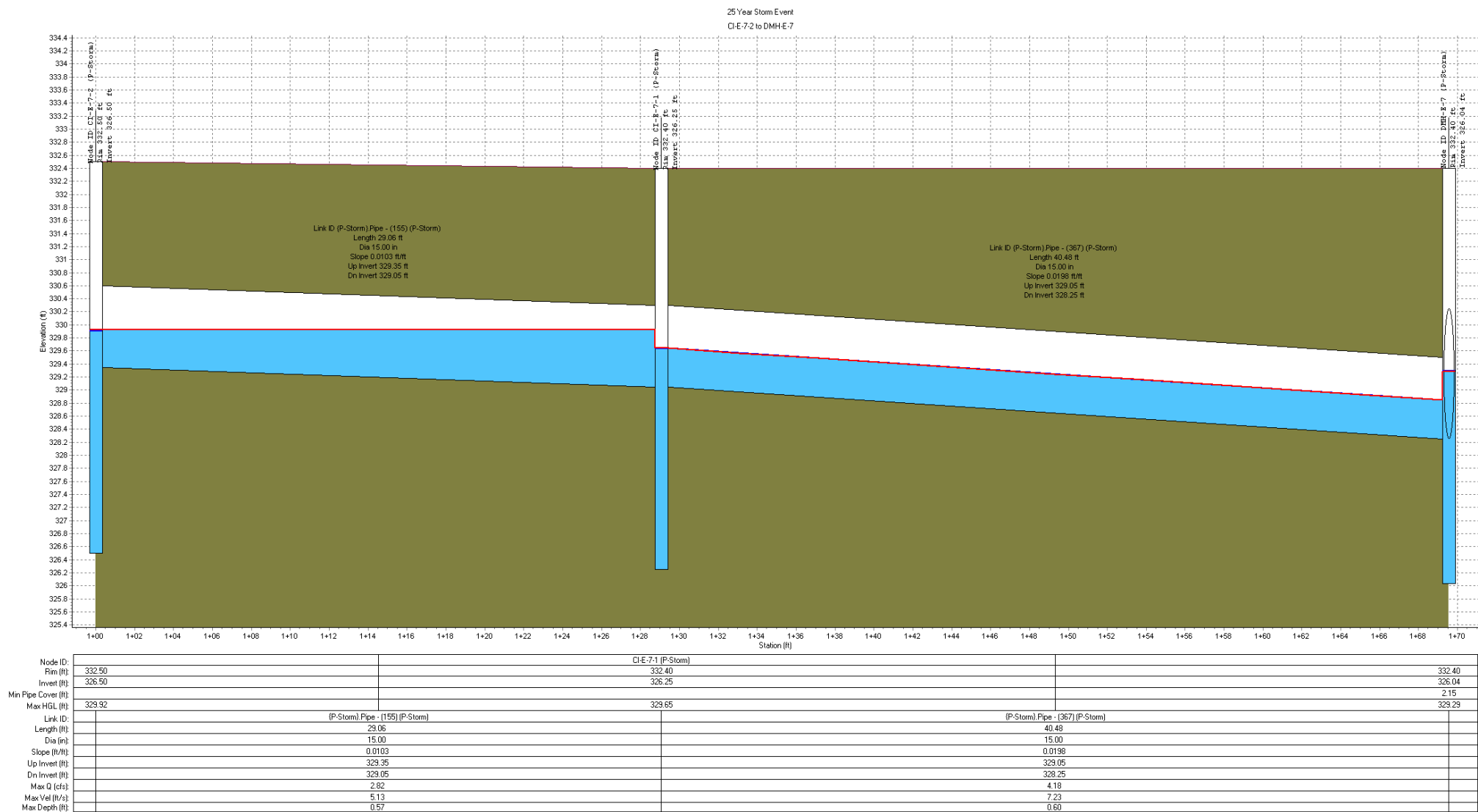
25 Year Storm Event
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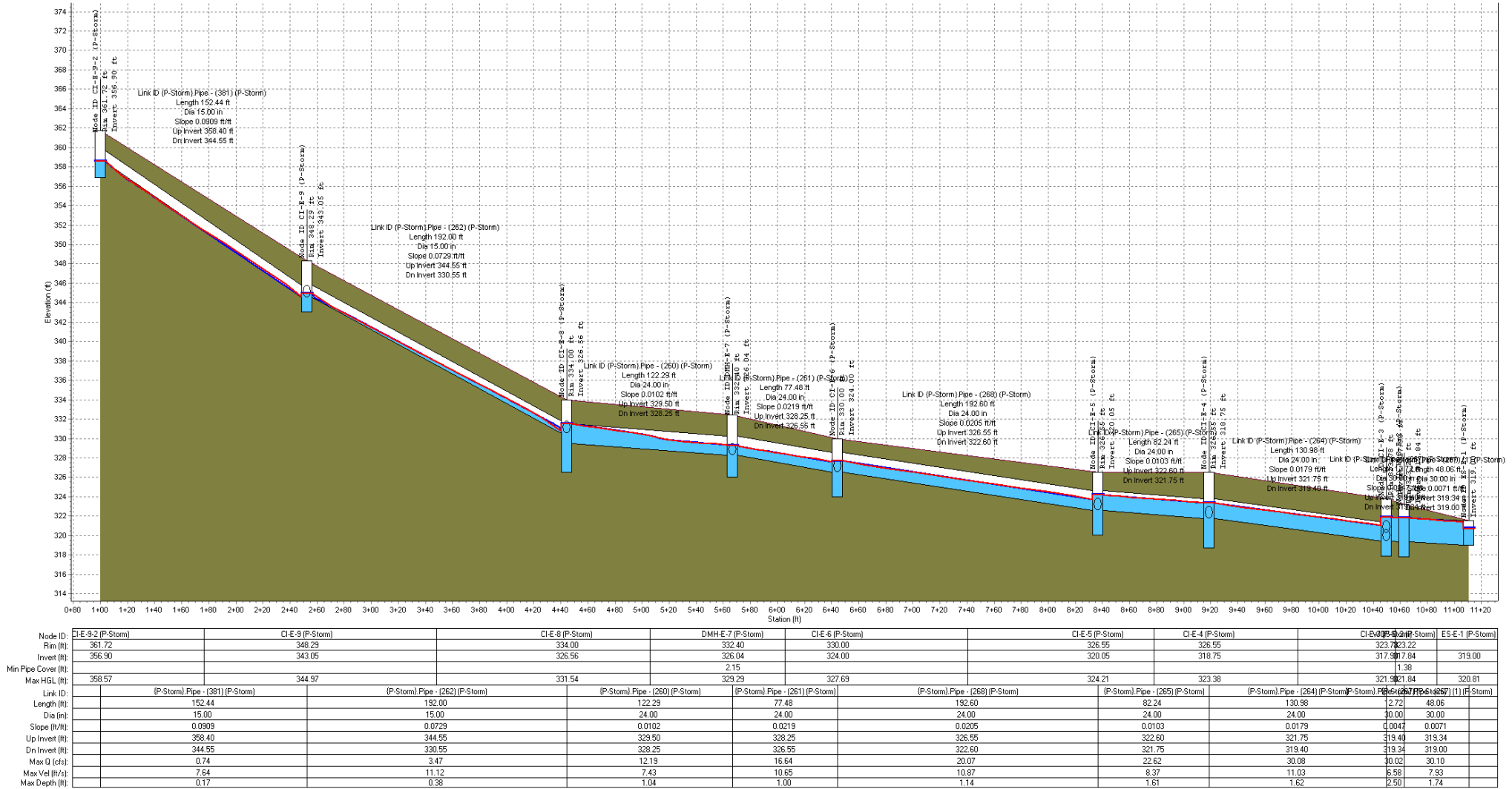


25 Year Storm Event
CI-E-3 to CI-E-4

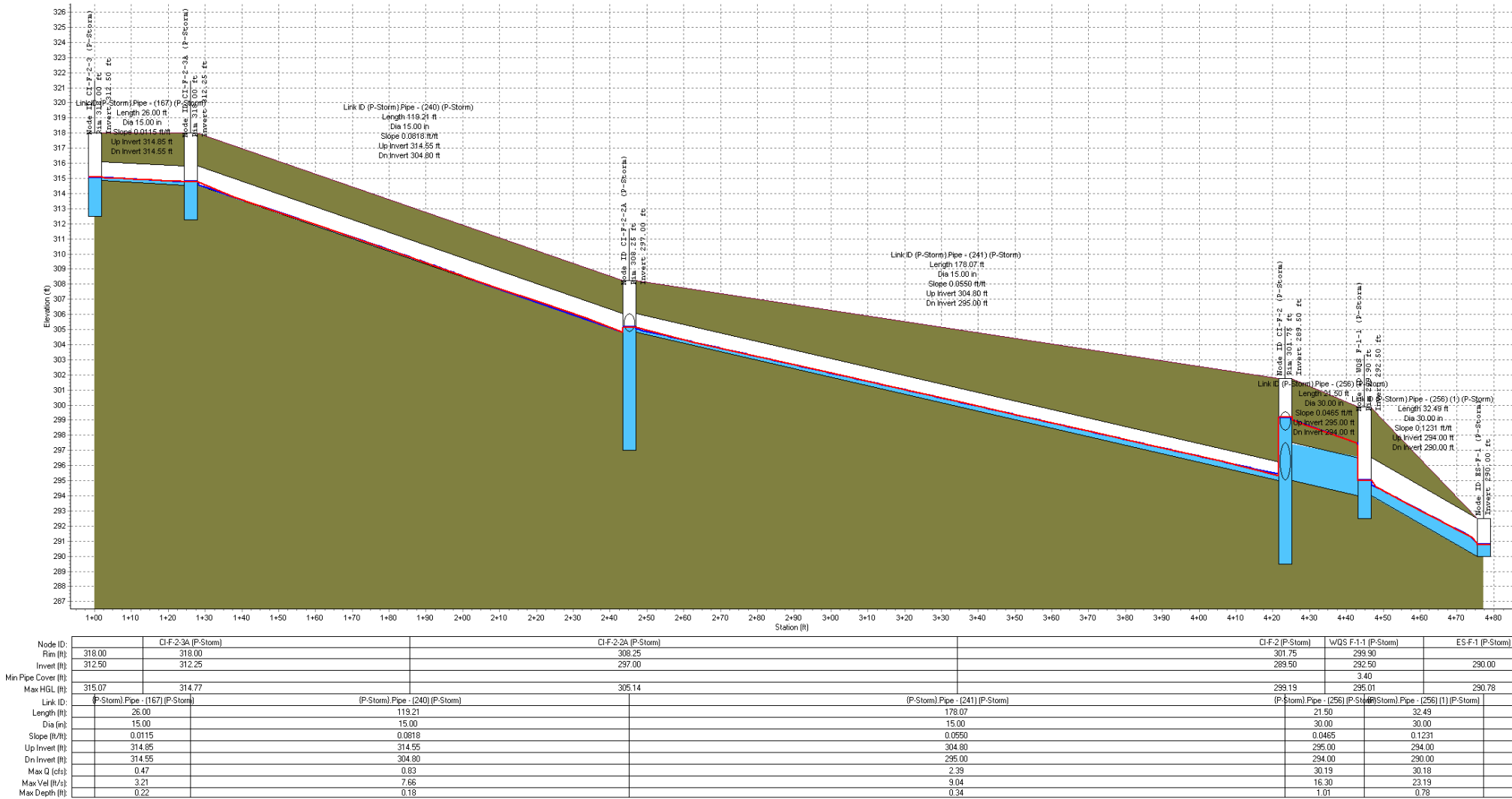




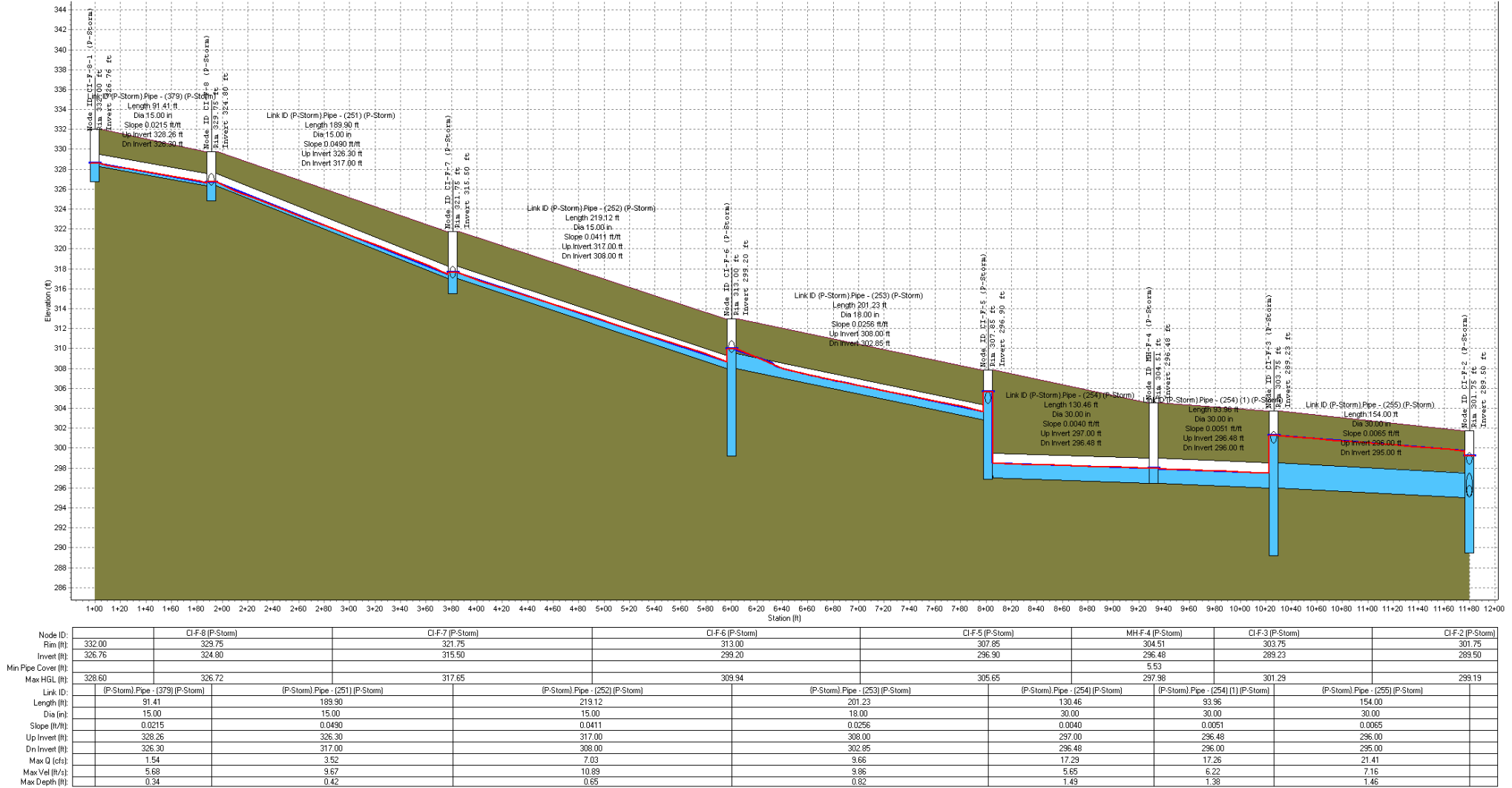
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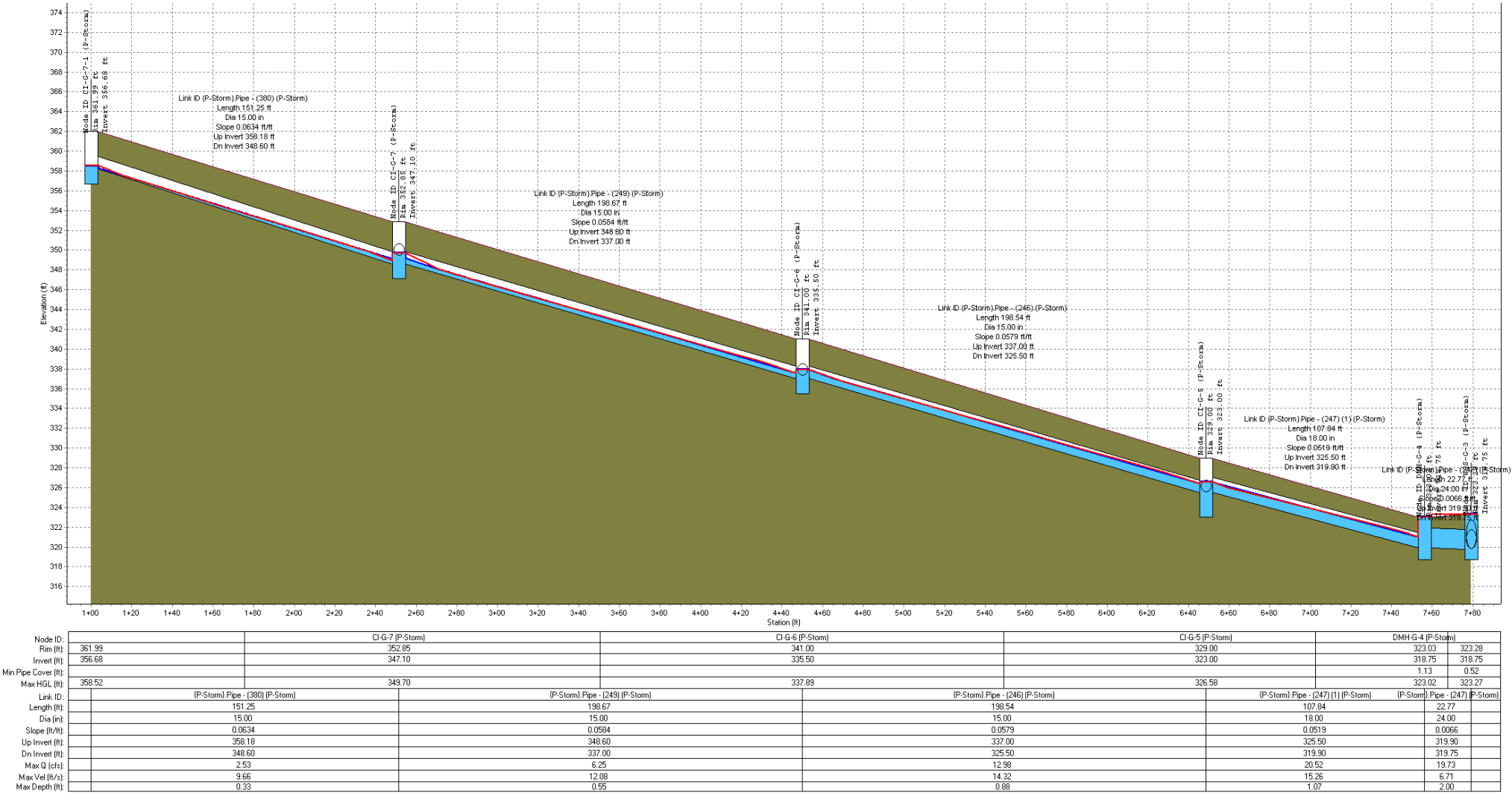
25 Year Storm Event
CIF-2-3 to ES-F1



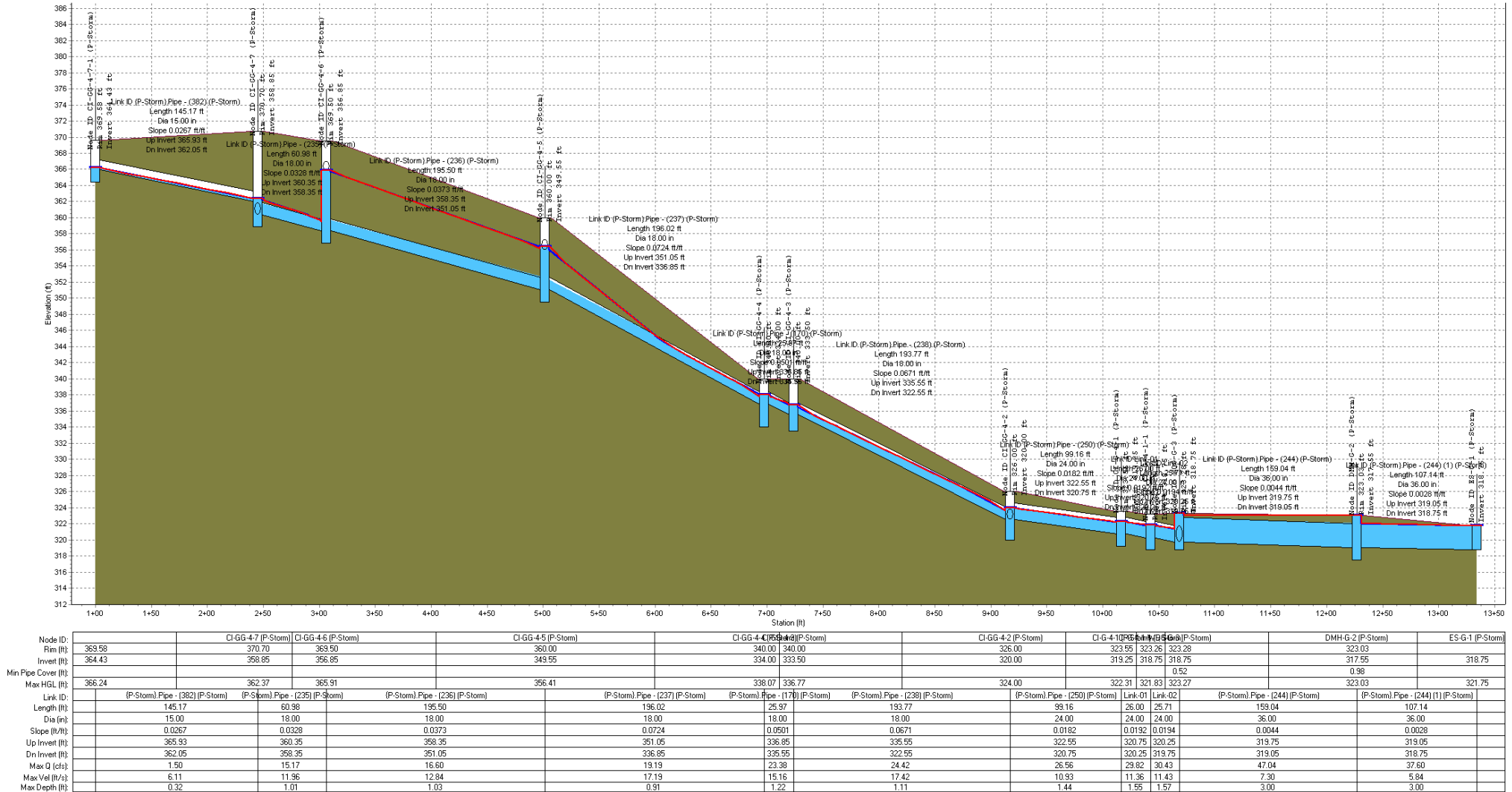
25 Year Storm Event
C1-F-1 to C1-F-2

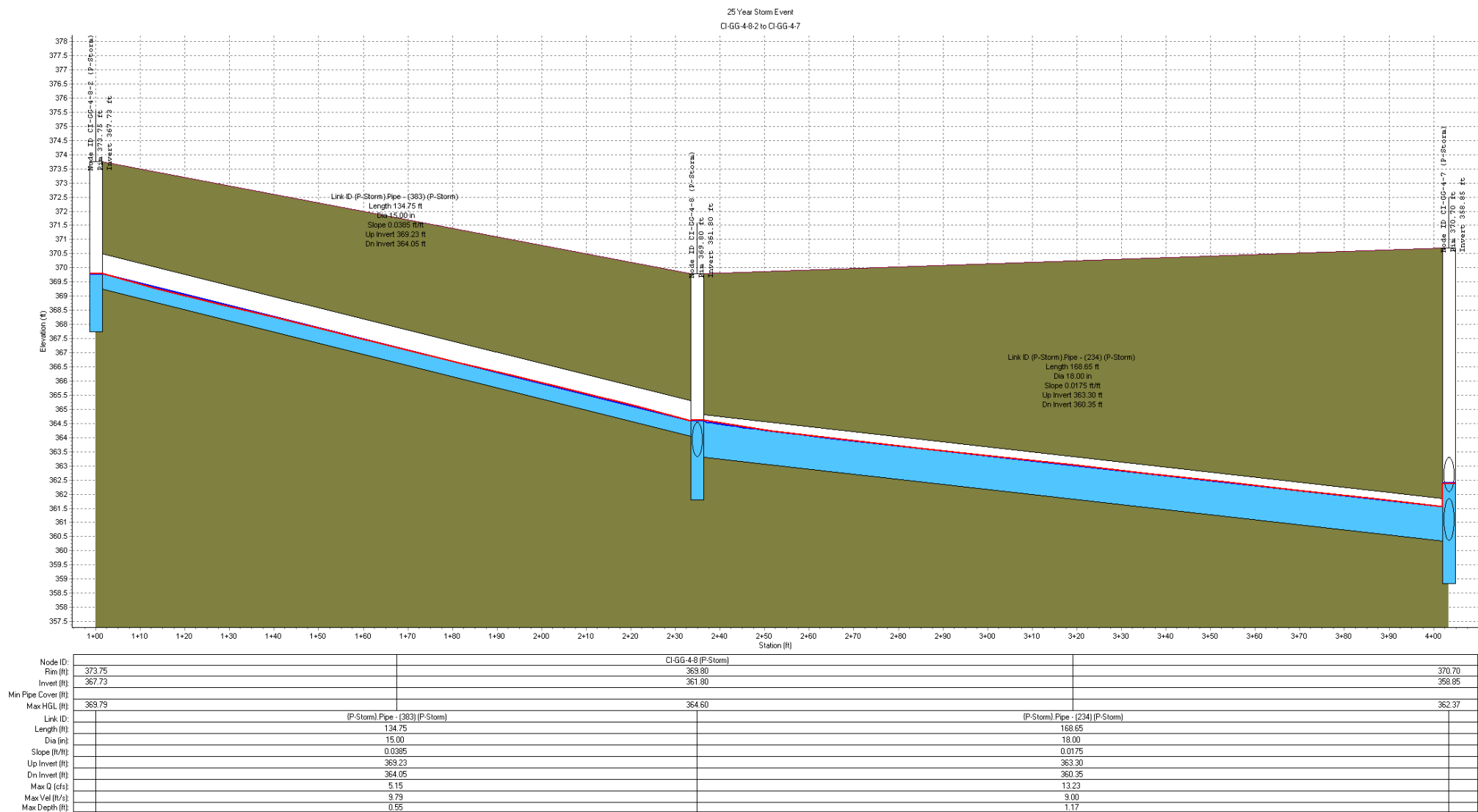


25 Year Storm Event
CI-G-7.1 to WQS-G-3

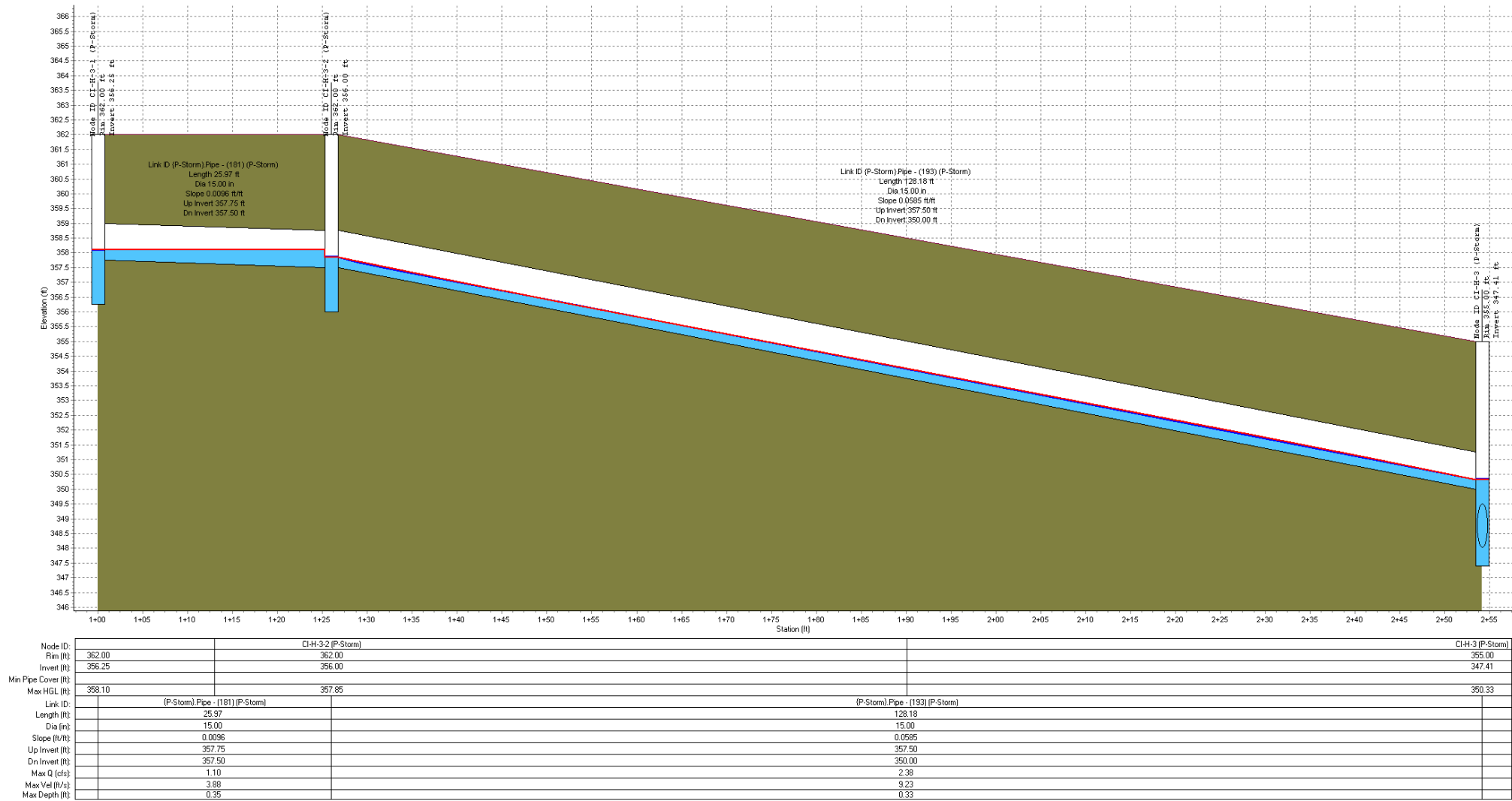


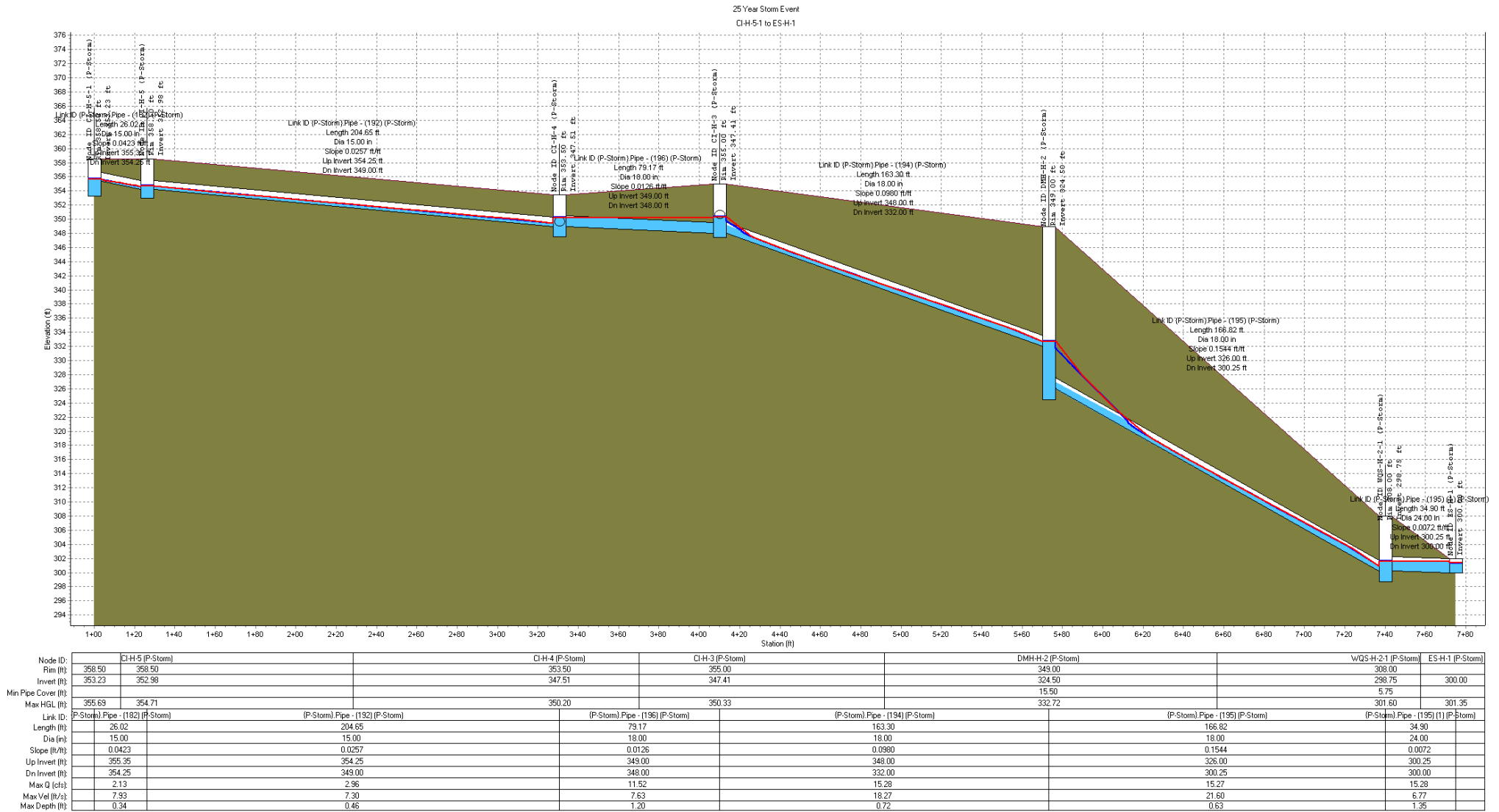
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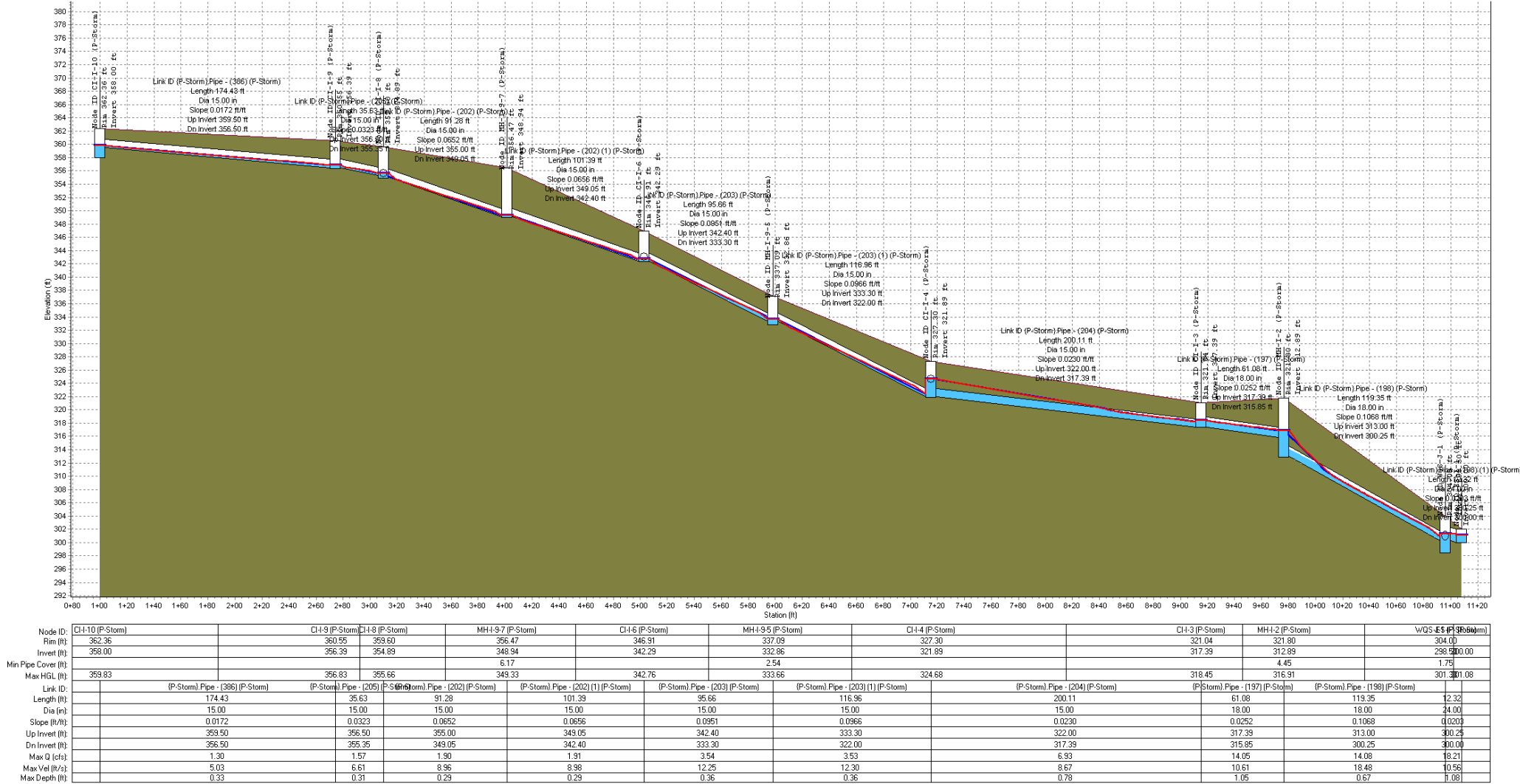


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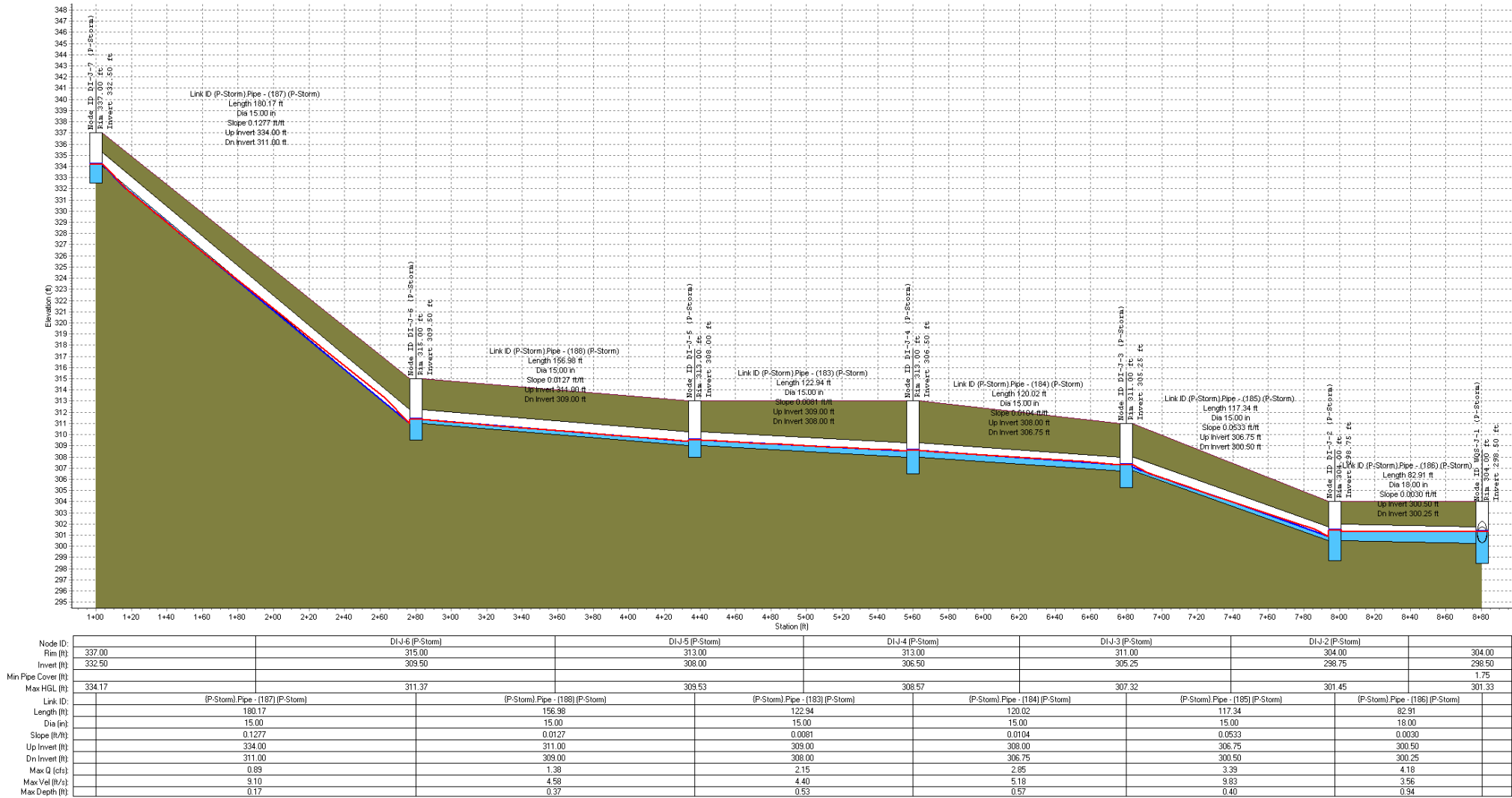




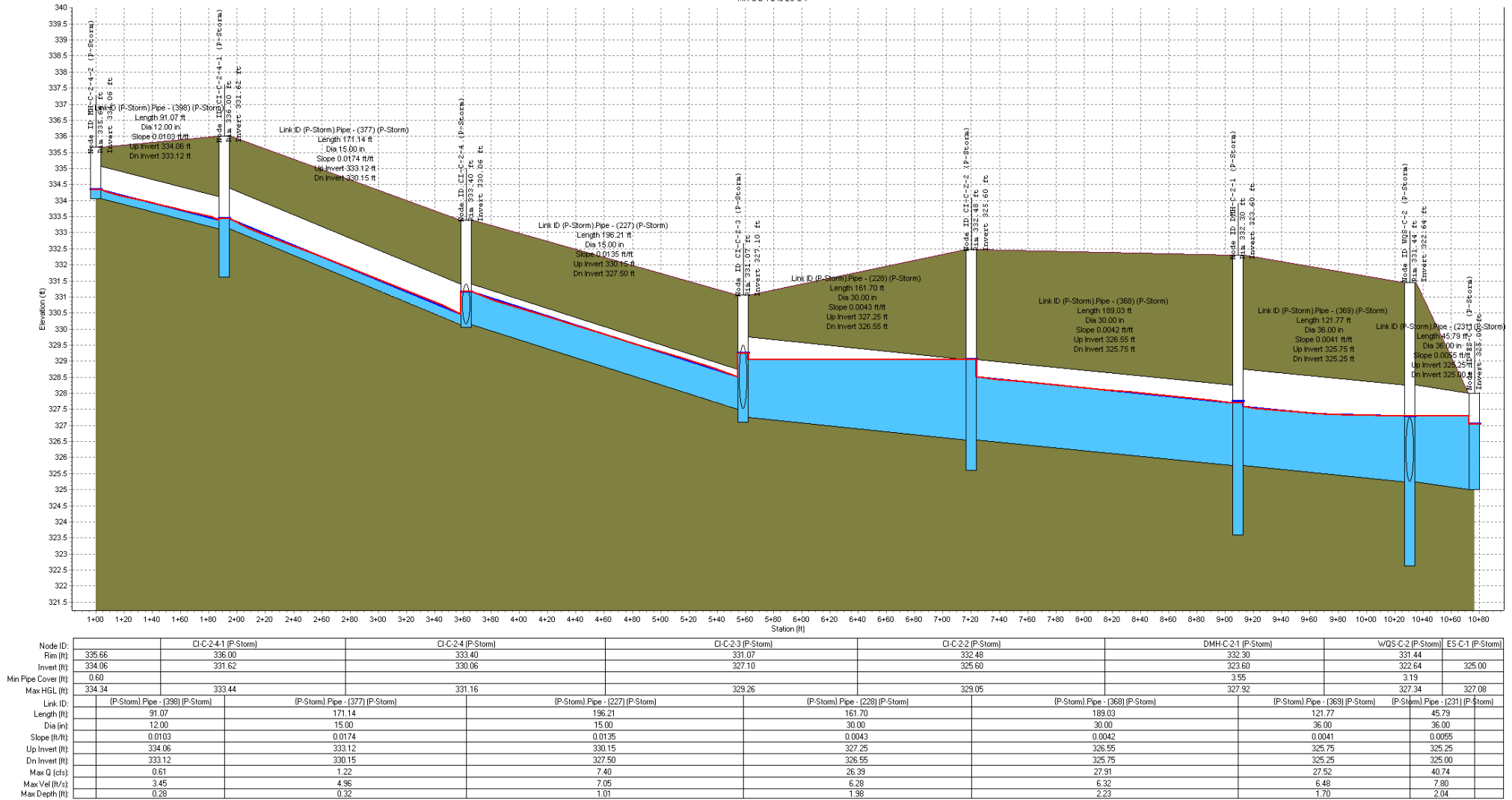
25 Year Storm Event
CH-10 to ES-1-1



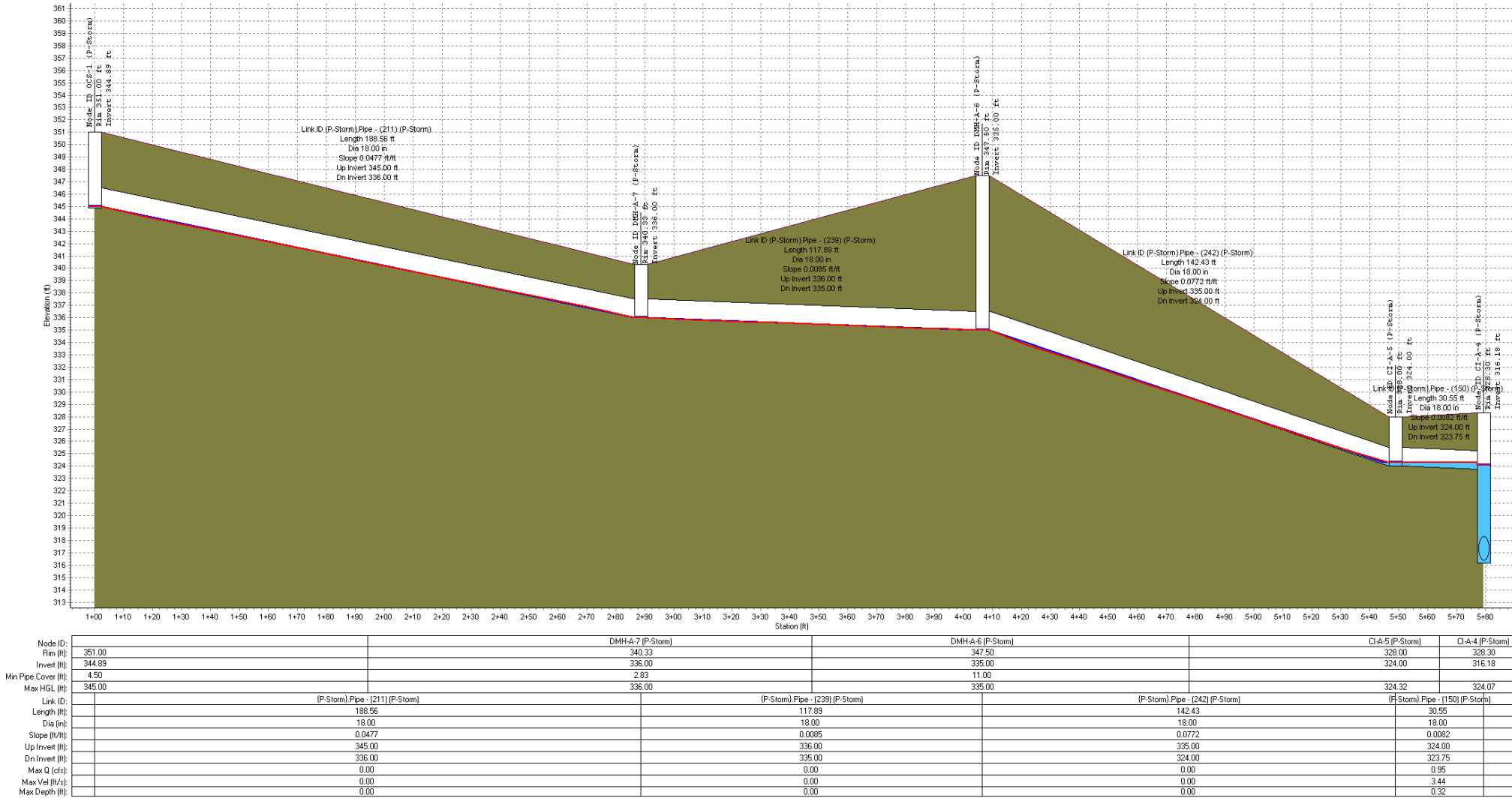
25 Year Storm Event
DI-J-7 to WQS-J-1



25 Year Storm Event
MHC-2.4.2 to ES-C-1



25 Year Storm Event
OCS-1 to CIA-4



APPENDIX I

OWNER/OPERATOR CERTIFICATION



Department of
Environmental
Conservation

Owner/Operator Certification Form

SPDES General Permit For Stormwater Discharges From Construction Activity (GP-0-20-001)

Project/Site Name: Elmwood Preserve - 850 Dobbs Ferry Road, Town of Greenburgh, New York

eNOI Submission Number: HPP-1PZQ-JV6M5

eNOI Submitted by: ☐ Owner/Operator ☒ SWPPP Preparer ☐ Other

Certification Statement - Owner/Operator

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Owner/Operator First Name

M.I. Last Name

Signature

Date

APPENDIX J

SWPPP PREPARER CERTIFICATION



Department of
Environmental
Conservation

SWPPP Preparer Certification Form

*SPDES General Permit for Stormwater
Discharges From Construction Activity
(GP-0-20-001)*

Project Site Information

Project/Site Name

Elmwood Preserve - 850 Dobbs Ferry Road, Town of Greenburgh, New York

Owner/Operator Information

Owner/Operator (Company Name/Private Owner/Municipality Name)

Ridgewood Elmwood Owner, LLC

Certification Statement – SWPPP Preparer

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Frederick

First name

MI

Bohlander

Last Name

Signature

A handwritten signature in black ink, appearing to read "Frederick Bohlander", written over a light blue grid background.

11/08/2022

Date

APPENDIX K

***SPDES GENERAL PERMIT
No. GP-0-02-001***



Department of
Environmental
Conservation

NEW YORK STATE
DEPARTMENT OF ENVIRONMENTAL CONSERVATION

SPDES GENERAL PERMIT
FOR STORMWATER DISCHARGES

From

CONSTRUCTION ACTIVITY

Permit No. GP- 0-20-001

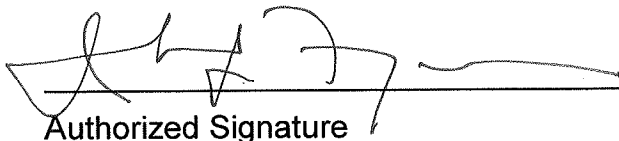
Issued Pursuant to Article 17, Titles 7, 8 and Article 70
of the Environmental Conservation Law

Effective Date: January 29, 2020

Expiration Date: January 28, 2025

John J. Ferguson

Chief Permit Administrator


Authorized Signature

1-23-20
Date

Address: NYS DEC
Division of Environmental Permits
625 Broadway, 4th Floor
Albany, N.Y. 12233-1750

PREFACE

Pursuant to Section 402 of the Clean Water Act (“CWA”), stormwater *discharges* from certain *construction activities* are unlawful unless they are authorized by a *National Pollutant Discharge Elimination System (“NPDES”)* permit or by a state permit program. New York administers the approved State Pollutant Discharge Elimination System (SPDES) program with permits issued in accordance with the New York State Environmental Conservation Law (ECL) Article 17, Titles 7, 8 and Article 70.

An *owner or operator* of a *construction activity* that is eligible for coverage under this permit must obtain coverage prior to the *commencement of construction activity*. Activities that fit the definition of “*construction activity*”, as defined under 40 CFR 122.26(b)(14)(x), (15)(i), and (15)(ii), constitute construction of a *point source* and therefore, pursuant to ECL section 17-0505 and 17-0701, the *owner or operator* must have coverage under a SPDES permit prior to *commencing construction activity*. The *owner or operator* cannot wait until there is an actual *discharge* from the *construction site* to obtain permit coverage.

***Note: The italicized words/phrases within this permit are defined in Appendix A.**

**NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION
SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES FROM
CONSTRUCTION ACTIVITIES**

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Part 1. PERMIT COVERAGE AND LIMITATIONS

A. Permit Application

This permit authorizes stormwater *discharges* to *surface waters of the State* from the following *construction activities* identified within 40 CFR Parts 122.26(b)(14)(x), 122.26(b)(15)(i) and 122.26(b)(15)(ii), provided all of the eligibility provisions of this permit are met:

1. *Construction activities* involving soil disturbances of one (1) or more acres; including disturbances of less than one acre that are part of a *larger common plan of development or sale* that will ultimately disturb one or more acres of land; excluding *routine maintenance activity* that is performed to maintain the original line and grade, hydraulic capacity or original purpose of a facility;
2. *Construction activities* involving soil disturbances of less than one (1) acre where the Department has determined that a *SPDES* permit is required for stormwater *discharges* based on the potential for contribution to a violation of a *water quality standard* or for significant contribution of *pollutants* to *surface waters of the State*.
3. *Construction activities* located in the watershed(s) identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

B. Effluent Limitations Applicable to Discharges from Construction Activities

Discharges authorized by this permit must achieve, at a minimum, the effluent limitations in Part I.B.1. (a) – (f) of this permit. These limitations represent the degree of effluent reduction attainable by the application of best practicable technology currently available.

1. Erosion and Sediment Control Requirements - The *owner or operator* must select, design, install, implement and maintain control measures to *minimize* the *discharge of pollutants* and prevent a violation of the *water quality standards*. The selection, design, installation, implementation, and maintenance of these control measures must meet the non-numeric effluent limitations in Part I.B.1.(a) – (f) of this permit and be in accordance with the New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, using sound engineering judgment. Where control measures are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must include in the *Stormwater Pollution Prevention Plan* (“SWPPP”) the reason(s) for the

deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

- a. **Erosion and Sediment Controls.** Design, install and maintain effective erosion and sediment controls to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such controls must be designed, installed and maintained to:
- (i) *Minimize* soil erosion through application of runoff control and soil stabilization control measure to *minimize pollutant discharges*;
 - (ii) Control stormwater *discharges*, including both peak flowrates and total stormwater volume, to *minimize* channel and *streambank* erosion and scour in the immediate vicinity of the *discharge* points;
 - (iii) *Minimize* the amount of soil exposed during *construction activity*;
 - (iv) *Minimize* the disturbance of *steep slopes*;
 - (v) *Minimize* sediment *discharges* from the site;
 - (vi) Provide and maintain *natural buffers* around surface waters, direct stormwater to vegetated areas and maximize stormwater infiltration to reduce *pollutant discharges*, unless *infeasible*;
 - (vii) *Minimize* soil compaction. Minimizing soil compaction is not required where the intended function of a specific area of the site dictates that it be compacted;
 - (viii) Unless *infeasible*, preserve a sufficient amount of topsoil to complete soil restoration and establish a uniform, dense vegetative cover; and
 - (ix) *Minimize* dust. On areas of exposed soil, *minimize* dust through the appropriate application of water or other dust suppression techniques to control the generation of pollutants that could be discharged from the site.
- b. **Soil Stabilization.** In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within fourteen (14) days from the date the current soil disturbance activity ceased. For construction sites that *directly discharge* to one of the 303(d) segments

listed in Appendix E or is located in one of the watersheds listed in Appendix C, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. See Appendix A for definition of *Temporarily Ceased*.

- c. **Dewatering.** *Discharges from dewatering activities, including discharges from dewatering of trenches and excavations, must be managed by appropriate control measures.*
- d. **Pollution Prevention Measures.** Design, install, implement, and maintain effective pollution prevention measures to *minimize the discharge of pollutants* and prevent a violation of the *water quality standards*. At a minimum, such measures must be designed, installed, implemented and maintained to:
 - (i) *Minimize the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other wash waters. This applies to washing operations that use clean water only. Soaps, detergents and solvents cannot be used;*
 - (ii) *Minimize the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, detergents, sanitary waste, hazardous and toxic waste, and other materials present on the site to precipitation and to stormwater. Minimization of exposure is not required in cases where the exposure to precipitation and to stormwater will not result in a discharge of pollutants, or where exposure of a specific material or product poses little risk of stormwater contamination (such as final products and materials intended for outdoor use) ; and*
 - (iii) *Prevent the discharge of pollutants from spills and leaks and implement chemical spill and leak prevention and response procedures.*
- e. **Prohibited Discharges.** The following *discharges* are prohibited:
 - (i) *Wastewater from washout of concrete;*
 - (ii) *Wastewater from washout and cleanout of stucco, paint, form release oils, curing compounds and other construction materials;*

- (iii) Fuels, oils, or other *pollutants* used in vehicle and equipment operation and maintenance;
 - (iv) Soaps or solvents used in vehicle and equipment washing; and
 - (v) Toxic or hazardous substances from a spill or other release.
- f. Surface Outlets. When discharging from basins and impoundments, the outlets shall be designed, constructed and maintained in such a manner that sediment does not leave the basin or impoundment and that erosion at or below the outlet does not occur.

C. Post-construction Stormwater Management Practice Requirements

1. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must select, design, install, and maintain the practices to meet the *performance criteria* in the New York State Stormwater Management Design Manual (“Design Manual”), dated January 2015, using sound engineering judgment. Where post-construction stormwater management practices (“SMPs”) are not designed in conformance with the *performance criteria* in the Design Manual, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.
2. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must design the practices to meet the applicable *sizing criteria* in Part I.C.2.a., b., c. or d. of this permit.

a. Sizing Criteria for New Development

- (i) Runoff Reduction Volume (“RRv”): Reduce the total Water Quality Volume (“WQv”) by application of RR techniques and standard SMPs with RRv capacity. The total WQv shall be calculated in accordance with the criteria in Section 4.2 of the Design Manual.
- (ii) Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the criteria in Part I.C.2.a.(i) of this permit due to site limitations shall direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv shall be documented in the SWPPP.

For each impervious area that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed impervious areas be less than the Minimum RRv as calculated using the criteria in Section 4.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume (“Cpv”): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site discharges directly to tidal waters, or fifth order or larger streams.
- (iv) *Overbank* Flood Control Criteria (“Qp”): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 - (1) the site discharges directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria (“Qf”): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site discharges directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

b. Sizing Criteria for New Development in Enhanced Phosphorus Removal Watershed

- (i) Runoff Reduction Volume (RRv): Reduce the total Water Quality Volume (WQv) by application of RR techniques and standard SMPs with RRv capacity. The total WQv is the runoff volume from the 1-year, 24 hour design storm over the post-developed watershed and shall be

calculated in accordance with the criteria in Section 10.3 of the Design Manual.

- (ii) Minimum RRv and Treatment of Remaining Total WQv: *Construction activities* that cannot meet the criteria in Part I.C.2.b.(i) of this permit due to *site limitations* shall direct runoff from all newly constructed *impervious areas* to a RR technique or standard SMP with RRv capacity unless *infeasible*. The specific *site limitations* that prevent the reduction of 100% of the WQv shall be documented in the SWPPP. For each *impervious area* that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered *infeasible*.

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 10.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume (Cpv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site *discharges* directly to tidal waters, or fifth order or larger streams.
- (iv) Overbank Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak *discharge* rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak *discharge* rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

c. Sizing Criteria for Redevelopment Activity

- (i) Water Quality Volume (WQv): The WQv treatment objective for *redevelopment activity* shall be addressed by one of the following options. *Redevelopment activities* located in an Enhanced Phosphorus Removal Watershed (see Part III.B.3. and Appendix C of this permit) shall calculate the WQv in accordance with Section 10.3 of the Design Manual. All other *redevelopment activities* shall calculate the WQv in accordance with Section 4.2 of the Design Manual.
 - (1) Reduce the existing *impervious cover* by a minimum of 25% of the total disturbed, *impervious area*. The Soil Restoration criteria in Section 5.1.6 of the Design Manual must be applied to all newly created pervious areas, or
 - (2) Capture and treat a minimum of 25% of the WQv from the disturbed, *impervious area* by the application of standard SMPs; or reduce 25% of the WQv from the disturbed, *impervious area* by the application of RR techniques or standard SMPs with RRV capacity., or
 - (3) Capture and treat a minimum of 75% of the WQv from the disturbed, *impervious area* as well as any additional runoff from tributary areas by application of the alternative practices discussed in Sections 9.3 and 9.4 of the Design Manual., or
 - (4) Application of a combination of 1, 2 and 3 above that provide a weighted average of at least two of the above methods. Application of this method shall be in accordance with the criteria in Section 9.2.1(B) (IV) of the Design Manual.

If there is an existing post-construction stormwater management practice located on the site that captures and treats runoff from the *impervious area* that is being disturbed, the WQv treatment option selected must, at a minimum, provide treatment equal to the treatment that was being provided by the existing practice(s) if that treatment is greater than the treatment required by options 1 – 4 above.

- (ii) Channel Protection Volume (Cpv): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iii) Overbank Flood Control Criteria (Qp): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iv) Extreme Flood Control Criteria (Qf): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site

d. Sizing Criteria for Combination of Redevelopment Activity and New Development

Construction projects that include both New Development and Redevelopment Activity shall provide post-construction stormwater management controls that meet the sizing criteria calculated as an aggregate of the Sizing Criteria in Part I.C.2.a. or b. of this permit for the New Development portion of the project and Part I.C.2.c of this permit for Redevelopment Activity portion of the project.

D. Maintaining Water Quality

The Department expects that compliance with the conditions of this permit will control *discharges* necessary to meet applicable *water quality standards*. It shall be a violation of the *ECL* for any discharge to either cause or contribute to a violation of *water quality standards* as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York, such as:

1. There shall be no increase in turbidity that will cause a substantial visible contrast to natural conditions;
2. There shall be no increase in suspended, colloidal or settleable solids that will cause deposition or impair the waters for their best usages; and
3. There shall be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

If there is evidence indicating that the stormwater *discharges* authorized by this permit are causing, have the reasonable potential to cause, or are contributing to a violation of the *water quality standards*; the *owner or operator* must take appropriate corrective action in accordance with Part IV.C.5. of this general permit and document in accordance with Part IV.C.4. of this general permit. To address the *water quality standard* violation the *owner or operator* may need to provide additional information, include and implement appropriate controls in the SWPPP to correct the problem, or obtain an individual SPDES permit.

If there is evidence indicating that despite compliance with the terms and conditions of this general permit it is demonstrated that the stormwater *discharges* authorized by this permit are causing or contributing to a violation of *water quality standards*, or if the Department determines that a modification of the permit is necessary to prevent a violation of *water quality standards*, the authorized *discharges* will no longer be eligible for coverage under this permit. The Department may require the *owner or operator* to obtain an individual SPDES permit to continue discharging.

E. Eligibility Under This General Permit

1. This permit may authorize all *discharges* of stormwater from *construction activity to surface waters of the State* and *groundwaters* except for ineligible *discharges* identified under subparagraph F. of this Part.
2. Except for non-stormwater *discharges* explicitly listed in the next paragraph, this permit only authorizes stormwater *discharges*; including stormwater runoff, snowmelt runoff, and surface runoff and drainage, from *construction activities*.
3. Notwithstanding paragraphs E.1 and E.2 above, the following non-stormwater discharges are authorized by this permit: those listed in 6 NYCRR 750-1.2(a)(29)(vi), with the following exception: “Discharges from firefighting activities are authorized only when the firefighting activities are emergencies/unplanned”; waters to which other components have not been added that are used to control dust in accordance with the SWPPP; and uncontaminated *discharges* from *construction site* de-watering operations. All non-stormwater discharges must be identified in the SWPPP. Under all circumstances, the *owner or operator* must still comply with *water quality standards* in Part I.D of this permit.
4. The *owner or operator* must maintain permit eligibility to *discharge* under this permit. Any *discharges* that are not compliant with the eligibility conditions of this permit are not authorized by the permit and the *owner or operator* must either apply for a separate permit to cover those ineligible *discharges* or take steps necessary to make the *discharge* eligible for coverage.

F. Activities Which Are Ineligible for Coverage Under This General Permit

All of the following are **not** authorized by this permit:

1. *Discharges after construction activities* have been completed and the site has undergone *final stabilization*;
2. *Discharges* that are mixed with sources of non-stormwater other than those expressly authorized under subsection E.3. of this Part and identified in the SWPPP required by this permit;
3. *Discharges* that are required to obtain an individual SPDES permit or another SPDES general permit pursuant to Part VII.K. of this permit;
4. *Construction activities or discharges from construction activities* that may adversely affect an *endangered or threatened species* unless the *owner or*

operator has obtained a permit issued pursuant to 6 NYCRR Part 182 for the project or the Department has issued a letter of non-jurisdiction for the project. All documentation necessary to demonstrate eligibility shall be maintained on site in accordance with Part II.D.2 of this permit;

5. *Discharges* which either cause or contribute to a violation of *water quality standards* adopted pursuant to the *ECL* and its accompanying regulations;
6. *Construction activities* for residential, commercial and institutional projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing *impervious cover*; and
 - c. Which disturb one (1) or more acres of land designated on the current United States Department of Agriculture (“USDA”) Soil Survey as Soil Slope Phase “D”, (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase “E” or “F” (regardless of the map unit name), or a combination of the three designations.
7. *Construction activities* for linear transportation projects and linear utility projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing *impervious cover*; and
 - c. Which disturb two (2) or more acres of land designated on the current USDA Soil Survey as Soil Slope Phase “D” (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase “E” or “F” (regardless of the map unit name), or a combination of the three designations.

8. *Construction activities* that have the potential to affect an *historic property*, unless there is documentation that such impacts have been resolved. The following documentation necessary to demonstrate eligibility with this requirement shall be maintained on site in accordance with Part II.D.2 of this permit and made available to the Department in accordance with Part VII.F of this permit:
- a. Documentation that the *construction activity* is not within an archeologically sensitive area indicated on the sensitivity map, and that the *construction activity* is not located on or immediately adjacent to a property listed or determined to be eligible for listing on the National or State Registers of Historic Places, and that there is no new permanent building on the *construction site* within the following distances from a building, structure, or object that is more than 50 years old, or if there is such a new permanent building on the *construction site* within those parameters that NYS Office of Parks, Recreation and Historic Preservation (OPRHP), a Historic Preservation Commission of a Certified Local Government, or a qualified preservation professional has determined that the building, structure, or object more than 50 years old is not historically/archeologically significant.
 - 1-5 acres of disturbance - 20 feet
 - 5-20 acres of disturbance - 50 feet
 - 20+ acres of disturbance - 100 feet, or
 - b. DEC consultation form sent to OPRHP, and copied to the NYS DEC Agency Historic Preservation Officer (APO), and
 - (i) the State Environmental Quality Review (SEQR) Environmental Assessment Form (EAF) with a negative declaration or the Findings Statement, with documentation of OPRHP's agreement with the resolution; or
 - (ii) documentation from OPRHP that the *construction activity* will result in No Impact; or
 - (iii) documentation from OPRHP providing a determination of No Adverse Impact; or
 - (iv) a Letter of Resolution signed by the owner/operator, OPRHP and the DEC APO which allows for this *construction activity* to be eligible for coverage under the general permit in terms of the State Historic Preservation Act (SHPA); or
 - c. Documentation of satisfactory compliance with Section 106 of the National Historic Preservation Act for a coterminous project area:

- (i) No Affect
- (ii) No Adverse Affect
- (iii) Executed Memorandum of Agreement, or

d. Documentation that:

- (i) SHPA Section 14.09 has been completed by NYS DEC or another state agency.

9. *Discharges from construction activities* that are subject to an existing SPDES individual or general permit where a SPDES permit for *construction activity* has been terminated or denied; or where the *owner or operator* has failed to renew an expired individual permit.

Part II. PERMIT COVERAGE

A. How to Obtain Coverage

1. An *owner or operator* of a *construction activity* that is not subject to the requirements of a regulated, traditional land use control MS4 must first prepare a SWPPP in accordance with all applicable requirements of this permit and then submit a completed Notice of Intent (NOI) to the Department to be authorized to discharge under this permit.
2. An *owner or operator* of a *construction activity* that is subject to the requirements of a *regulated, traditional land use control MS4* must first prepare a SWPPP in accordance with all applicable requirements of this permit and then have the SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department. The *owner or operator* shall have the "MS4 SWPPP Acceptance" form signed in accordance with Part VII.H., and then submit that form along with a completed NOI to the Department.
3. The requirement for an *owner or operator* to have its SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department does not apply to an *owner or operator* that is obtaining permit coverage in accordance with the requirements in Part II.F. (Change of Owner or Operator) or where the *owner or operator* of the *construction activity* is the *regulated, traditional land use control MS4*. This exemption does not apply to *construction activities* subject to the New York City Administrative Code.

B. Notice of Intent (NOI) Submittal

1. Prior to December 21, 2020, an owner or operator shall use either the electronic (eNOI) or paper version of the NOI that the Department prepared. Both versions of the NOI are located on the Department's website (<http://www.dec.ny.gov/>). The paper version of the NOI shall be signed in accordance with Part VII.H. of this permit and submitted to the following address:

**NOTICE OF INTENT
NYS DEC, Bureau of Water Permits
625 Broadway, 4th Floor
Albany, New York 12233-3505**

2. Beginning December 21, 2020 and in accordance with EPA's 2015 NPDES Electronic Reporting Rule (40 CFR Part 127), the *owner or operator* must submit the NOI electronically using the *Department's* online NOI.
3. The *owner or operator* shall have the SWPPP preparer sign the "SWPPP Preparer Certification" statement on the NOI prior to submitting the form to the Department.
4. As of the date the NOI is submitted to the Department, the *owner or operator* shall make the NOI and SWPPP available for review and copying in accordance with the requirements in Part VII.F. of this permit.

C. Permit Authorization

1. An *owner or operator* shall not *commence construction activity* until their authorization to *discharge* under this permit goes into effect.
2. Authorization to *discharge* under this permit will be effective when the *owner or operator* has satisfied all of the following criteria:
 - a. project review pursuant to the State Environmental Quality Review Act ("SEQRA") have been satisfied, when SEQRA is applicable. See the Department's website (<http://www.dec.ny.gov/>) for more information,
 - b. where required, all necessary Department permits subject to the *Uniform Procedures Act* ("UPA") (see 6 NYCRR Part 621), or the equivalent from another New York State agency, have been obtained, unless otherwise notified by the Department pursuant to 6 NYCRR 621.3(a)(4). *Owners or operators of construction activities* that are required to obtain UPA permits

must submit a preliminary SWPPP to the appropriate DEC Permit Administrator at the Regional Office listed in Appendix F at the time all other necessary *UPA* permit applications are submitted. The preliminary SWPPP must include sufficient information to demonstrate that the *construction activity* qualifies for authorization under this permit,

- c. the final SWPPP has been prepared, and
 - d. a complete NOI has been submitted to the Department in accordance with the requirements of this permit.
3. An *owner or operator* that has satisfied the requirements of Part II.C.2 above will be authorized to *discharge* stormwater from their *construction activity* in accordance with the following schedule:
- a. For *construction activities* that are not subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives a complete electronic version of the NOI (eNOI) for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.; or
 - (ii) Sixty (60) business days from the date the Department receives a complete NOI (electronic or paper version) for *construction activities* with a SWPPP that has not been prepared in conformance with the design criteria in technical standard referenced in Part III.B.1. or, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C., the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, or;
 - (iii) Ten (10) business days from the date the Department receives a complete paper version of the NOI for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.

- b. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives both a complete electronic version of the NOI (eNOI) and signed “MS4 SWPPP Acceptance” form, or
 - (ii) Ten (10) business days from the date the Department receives both a complete paper version of the NOI and signed “MS4 SWPPP Acceptance” form.
- 4. Coverage under this permit authorizes stormwater *discharges* from only those areas of disturbance that are identified in the NOI. If an *owner or operator* wishes to have stormwater *discharges* from future or additional areas of disturbance authorized, they must submit a new NOI that addresses that phase of the development, unless otherwise notified by the Department. The *owner or operator* shall not *commence construction activity* on the future or additional areas until their authorization to *discharge* under this permit goes into effect in accordance with Part II.C. of this permit.

D. General Requirements For Owners or Operators With Permit Coverage

- 1. The *owner or operator* shall ensure that the provisions of the SWPPP are implemented from the *commencement of construction activity* until all areas of disturbance have achieved *final stabilization* and the Notice of Termination (“NOT”) has been submitted to the Department in accordance with Part V. of this permit. This includes any changes made to the SWPPP pursuant to Part III.A.4. of this permit.
- 2. The *owner or operator* shall maintain a copy of the General Permit (GP-0-20-001), NOI, *NOI Acknowledgment Letter*, SWPPP, MS4 SWPPP Acceptance form, inspection reports, responsible contractor’s or subcontractor’s certification statement (see Part III.A.6.), and all documentation necessary to demonstrate eligibility with this permit at the *construction site* until all disturbed areas have achieved *final stabilization* and the NOT has been submitted to the Department. The documents must be maintained in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection.
- 3. The *owner or operator* of a *construction activity* shall not disturb greater than five (5) acres of soil at any one time without prior written authorization from the Department or, in areas under the jurisdiction of a *regulated, traditional land*

use control MS4, the regulated, traditional land use control MS4 (provided the regulated, traditional land use control MS4 is not the owner or operator of the construction activity). At a minimum, the owner or operator must comply with the following requirements in order to be authorized to disturb greater than five (5) acres of soil at any one time:

- a. The *owner or operator* shall have a *qualified inspector* conduct **at least** two (2) site inspections in accordance with Part IV.C. of this permit every seven (7) calendar days, for as long as greater than five (5) acres of soil remain disturbed. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
 - b. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. The soil stabilization measures selected shall be in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016.
 - c. The *owner or operator* shall prepare a phasing plan that defines maximum disturbed area per phase and shows required cuts and fills.
 - d. The *owner or operator* shall install any additional site-specific practices needed to protect water quality.
 - e. The *owner or operator* shall include the requirements above in their SWPPP.
4. In accordance with statute, regulations, and the terms and conditions of this permit, the Department may suspend or revoke an *owner's or operator's* coverage under this permit at any time if the Department determines that the SWPPP does not meet the permit requirements or consistent with Part VII.K..
 5. Upon a finding of significant non-compliance with the practices described in the SWPPP or violation of this permit, the Department may order an immediate stop to all activity at the site until the non-compliance is remedied. The stop work order shall be in writing, describe the non-compliance in detail, and be sent to the *owner or operator*.
 6. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*, the *owner or operator* shall notify the

regulated, traditional land use control MS4 in writing of any planned amendments or modifications to the post-construction stormwater management practice component of the SWPPP required by Part III.A. 4. and 5. of this permit. Unless otherwise notified by the *regulated, traditional land use control MS4*, the *owner or operator* shall have the SWPPP amendments or modifications reviewed and accepted by the *regulated, traditional land use control MS4* prior to commencing construction of the post-construction stormwater management practice.

E. Permit Coverage for Discharges Authorized Under GP-0-15-002

1. Upon renewal of SPDES General Permit for Stormwater Discharges from *Construction Activity* (Permit No. GP-0-15-002), an *owner or operator* of a *construction activity* with coverage under GP-0-15-002, as of the effective date of GP- 0-20-001, shall be authorized to *discharge* in accordance with GP- 0-20-001, unless otherwise notified by the Department.

An *owner or operator* may continue to implement the technical/design components of the post-construction stormwater management controls provided that such design was done in conformance with the technical standards in place at the time of initial project authorization. However, they must comply with the other, non-design provisions of GP-0-20-001.

F. Change of Owner or Operator

1. When property ownership changes or when there is a change in operational control over the construction plans and specifications, the original *owner or operator* must notify the new *owner or operator*, in writing, of the requirement to obtain permit coverage by submitting a NOI with the Department. For *construction activities* subject to the requirements of a *regulated, traditional land use control MS4*, the original *owner or operator* must also notify the MS4, in writing, of the change in ownership at least 30 calendar days prior to the change in ownership.
2. Once the new *owner or operator* obtains permit coverage, the original *owner or operator* shall then submit a completed NOT with the name and permit identification number of the new *owner or operator* to the Department at the address in Part II.B.1. of this permit. If the original *owner or operator* maintains ownership of a portion of the *construction activity* and will disturb soil, they must maintain their coverage under the permit.
3. Permit coverage for the new *owner or operator* will be effective as of the date the Department receives a complete NOI, provided the original *owner or*

operator was not subject to a sixty (60) business day authorization period that has not expired as of the date the Department receives the NOI from the new *owner or operator*.

Part III. STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

A. General SWPPP Requirements

1. A SWPPP shall be prepared and implemented by the *owner or operator* of each *construction activity* covered by this permit. The SWPPP must document the selection, design, installation, implementation and maintenance of the control measures and practices that will be used to meet the effluent limitations in Part I.B. of this permit and where applicable, the post-construction stormwater management practice requirements in Part I.C. of this permit. The SWPPP shall be prepared prior to the submittal of the NOI. The NOI shall be submitted to the Department prior to the *commencement of construction activity*. A copy of the completed, final NOI shall be included in the SWPPP.
2. The SWPPP shall describe the erosion and sediment control practices and where required, post-construction stormwater management practices that will be used and/or constructed to reduce the *pollutants* in stormwater *discharges* and to assure compliance with the terms and conditions of this permit. In addition, the SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater *discharges*.
3. All SWPPPs that require the post-construction stormwater management practice component shall be prepared by a *qualified professional* that is knowledgeable in the principles and practices of stormwater management and treatment.
4. The *owner or operator* must keep the SWPPP current so that it at all times accurately documents the erosion and sediment controls practices that are being used or will be used during construction, and all post-construction stormwater management practices that will be constructed on the site. At a minimum, the *owner or operator* shall amend the SWPPP, including construction drawings:
 - a. whenever the current provisions prove to be ineffective in minimizing *pollutants* in stormwater *discharges* from the site;

- b. whenever there is a change in design, construction, or operation at the *construction site* that has or could have an effect on the *discharge* of *pollutants*;
 - c. to address issues or deficiencies identified during an inspection by the *qualified inspector*, the Department or other regulatory authority; and
 - d. to document the final construction conditions.
5. The Department may notify the *owner or operator* at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. The notification shall be in writing and identify the provisions of the SWPPP that require modification. Within fourteen (14) calendar days of such notification, or as otherwise indicated by the Department, the *owner or operator* shall make the required changes to the SWPPP and submit written notification to the Department that the changes have been made. If the *owner or operator* does not respond to the Department's comments in the specified time frame, the Department may suspend the *owner's or operator's* coverage under this permit or require the *owner or operator* to obtain coverage under an individual SPDES permit in accordance with Part II.D.4. of this permit.
6. Prior to the *commencement of construction activity*, the *owner or operator* must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The *owner or operator* shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the *trained contractor*. The *owner or operator* shall ensure that at least one *trained contractor* is on site on a daily basis when soil disturbance activities are being performed.

The *owner or operator* shall have each of the contractors and subcontractors identified above sign a copy of the following certification statement below before they commence any *construction activity*:

"I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the *qualified inspector* during a site inspection. I also understand that the *owner or operator* must comply with

the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater *discharges* from *construction activities* and that it is unlawful for any person to cause or contribute to a violation of *water quality standards*. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

In addition to providing the certification statement above, the certification page must also identify the specific elements of the SWPPP that each contractor and subcontractor will be responsible for and include the name and title of the person providing the signature; the name and title of the *trained contractor* responsible for SWPPP implementation; the name, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification statement is signed. The *owner or operator* shall attach the certification statement(s) to the copy of the SWPPP that is maintained at the *construction site*. If new or additional contractors are hired to implement measures identified in the SWPPP after construction has commenced, they must also sign the certification statement and provide the information listed above.

7. For projects where the Department requests a copy of the SWPPP or inspection reports, the *owner or operator* shall submit the documents in both electronic (PDF only) and paper format within five (5) business days, unless otherwise notified by the Department.

B. Required SWPPP Contents

1. Erosion and sediment control component - All SWPPPs prepared pursuant to this permit shall include erosion and sediment control practices designed in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Where erosion and sediment control practices are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must demonstrate *equivalence* to the technical standard. At a minimum, the erosion and sediment control component of the SWPPP shall include the following:
 - a. Background information about the scope of the project, including the location, type and size of project

- b. A site map/construction drawing(s) for the project, including a general location map. At a minimum, the site map shall show the total site area; all improvements; areas of disturbance; areas that will not be disturbed; existing vegetation; on-site and adjacent off-site surface water(s); floodplain/floodway boundaries; wetlands and drainage patterns that could be affected by the *construction activity*; existing and final contours ; locations of different soil types with boundaries; material, waste, borrow or equipment storage areas located on adjacent properties; and location(s) of the stormwater *discharge(s)*;
- c. A description of the soil(s) present at the site, including an identification of the Hydrologic Soil Group (HSG);
- d. A construction phasing plan and sequence of operations describing the intended order of *construction activities*, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance;
- e. A description of the minimum erosion and sediment control practices to be installed or implemented for each *construction activity* that will result in soil disturbance. Include a schedule that identifies the timing of initial placement or implementation of each erosion and sediment control practice and the minimum time frames that each practice should remain in place or be implemented;
- f. A temporary and permanent soil stabilization plan that meets the requirements of this general permit and the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, for each stage of the project, including initial land clearing and grubbing to project completion and achievement of *final stabilization*;
- g. A site map/construction drawing(s) showing the specific location(s), size(s), and length(s) of each erosion and sediment control practice;
- h. The dimensions, material specifications, installation details, and operation and maintenance requirements for all erosion and sediment control practices. Include the location and sizing of any temporary sediment basins and structural practices that will be used to divert flows from exposed soils;
- i. A maintenance inspection schedule for the contractor(s) identified in Part III.A.6. of this permit, to ensure continuous and effective operation of the erosion and sediment control practices. The maintenance inspection

schedule shall be in accordance with the requirements in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016;

- j. A description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a *pollutant* source in the stormwater *discharges*;
 - k. A description and location of any stormwater *discharges* associated with industrial activity other than construction at the site, including, but not limited to, stormwater *discharges* from asphalt plants and concrete plants located on the *construction site*; and
 - l. Identification of any elements of the design that are not in conformance with the design criteria in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Include the reason for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.
2. Post-construction stormwater management practice component – The *owner or operator* of any construction project identified in Table 2 of Appendix B as needing post-construction stormwater management practices shall prepare a SWPPP that includes practices designed in conformance with the applicable *sizing criteria* in Part I.C.2.a., c. or d. of this permit and the *performance criteria* in the technical standard, New York State Stormwater Management Design Manual dated January 2015

Where post-construction stormwater management practices are not designed in conformance with the *performance criteria* in the technical standard, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

The post-construction stormwater management practice component of the SWPPP shall include the following:

- a. Identification of all post-construction stormwater management practices to be constructed as part of the project. Include the dimensions, material specifications and installation details for each post-construction stormwater management practice;

- b. A site map/construction drawing(s) showing the specific location and size of each post-construction stormwater management practice;
- c. A Stormwater Modeling and Analysis Report that includes:
 - (i) Map(s) showing pre-development conditions, including watershed/subcatchments boundaries, flow paths/routing, and design points;
 - (ii) Map(s) showing post-development conditions, including watershed/subcatchments boundaries, flow paths/routing, design points and post-construction stormwater management practices;
 - (iii) Results of stormwater modeling (i.e. hydrology and hydraulic analysis) for the required storm events. Include supporting calculations (model runs), methodology, and a summary table that compares pre and post-development runoff rates and volumes for the different storm events;
 - (iv) Summary table, with supporting calculations, which demonstrates that each post-construction stormwater management practice has been designed in conformance with the *sizing criteria* included in the Design Manual;
 - (v) Identification of any *sizing criteria* that is not required based on the requirements included in Part I.C. of this permit; and
 - (vi) Identification of any elements of the design that are not in conformance with the *performance criteria* in the Design Manual. Include the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the Design Manual;
- d. Soil testing results and locations (test pits, borings);
- e. Infiltration test results, when required; and
- f. An operations and maintenance plan that includes inspection and maintenance schedules and actions to ensure continuous and effective operation of each post-construction stormwater management practice. The plan shall identify the entity that will be responsible for the long term operation and maintenance of each practice.

3. Enhanced Phosphorus Removal Standards - All construction projects identified in Table 2 of Appendix B that are located in the watersheds identified in Appendix C shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the applicable *sizing criteria* in Part I.C.2. b., c. or d. of this permit and the *performance criteria*, Enhanced Phosphorus Removal Standards included in the Design Manual. At a minimum, the post-construction stormwater management practice component of the SWPPP shall include items 2.a - 2.f. above.

C. Required SWPPP Components by Project Type

Unless otherwise notified by the Department, *owners or operators of construction activities* identified in Table 1 of Appendix B are required to prepare a SWPPP that only includes erosion and sediment control practices designed in conformance with Part III.B.1 of this permit. *Owners or operators of the construction activities* identified in Table 2 of Appendix B shall prepare a SWPPP that also includes post-construction stormwater management practices designed in conformance with Part III.B.2 or 3 of this permit.

Part IV. INSPECTION AND MAINTENANCE REQUIREMENTS

A. General Construction Site Inspection and Maintenance Requirements

1. The *owner or operator* must ensure that all erosion and sediment control practices (including pollution prevention measures) and all post-construction stormwater management practices identified in the SWPPP are inspected and maintained in accordance with Part IV.B. and C. of this permit.
2. The terms of this permit shall not be construed to prohibit the State of New York from exercising any authority pursuant to the ECL, common law or federal law, or prohibit New York State from taking any measures, whether civil or criminal, to prevent violations of the laws of the State of New York or protect the public health and safety and/or the environment.

B. Contractor Maintenance Inspection Requirements

1. The *owner or operator* of each *construction activity* identified in Tables 1 and 2 of Appendix B shall have a *trained contractor* inspect the erosion and sediment control practices and pollution prevention measures being implemented within the active work area daily to ensure that they are being maintained in effective operating condition at all times. If deficiencies are identified, the contractor shall

begin implementing corrective actions within one business day and shall complete the corrective actions in a reasonable time frame.

2. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *trained contractor* can stop conducting the maintenance inspections. The *trained contractor* shall begin conducting the maintenance inspections in accordance with Part IV.B.1. of this permit as soon as soil disturbance activities resume.
3. For construction sites where soil disturbance activities have been shut down with partial project completion, the *trained contractor* can stop conducting the maintenance inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational.

C. Qualified Inspector Inspection Requirements

The *owner or operator* shall have a *qualified inspector* conduct site inspections in conformance with the following requirements:

[Note: The *trained contractor* identified in Part III.A.6. and IV.B. of this permit **cannot** conduct the *qualified inspector* site inspections unless they meet the *qualified inspector* qualifications included in Appendix A. In order to perform these inspections, the *trained contractor* would have to be a:

- licensed Professional Engineer,
 - Certified Professional in Erosion and Sediment Control (CPESC),
 - New York State Erosion and Sediment Control Certificate Program holder
 - Registered Landscape Architect, or
 - someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity].
1. A *qualified inspector* shall conduct site inspections for all *construction activities* identified in Tables 1 and 2 of Appendix B, with the exception of:
 - a. the construction of a single family residential subdivision with 25% or less *impervious cover* at total site build-out that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located

in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;

- b. the construction of a single family home that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;
 - c. construction on agricultural property that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres; and
 - d. *construction activities* located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
2. Unless otherwise notified by the Department, the *qualified inspector* shall conduct site inspections in accordance with the following timetable:
- a. For construction sites where soil disturbance activities are on-going, the *qualified inspector* shall conduct a site inspection at least once every seven (7) calendar days.
 - b. For construction sites where soil disturbance activities are on-going and the *owner or operator* has received authorization in accordance with Part II.D.3 to disturb greater than five (5) acres of soil at any one time, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
 - c. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *qualified inspector* shall conduct a site inspection at least once every thirty (30) calendar days. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4*, the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to reducing the frequency of inspections.

- d. For construction sites where soil disturbance activities have been shut down with partial project completion, the *qualified inspector* can stop conducting inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4*, the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to the shutdown. If soil disturbance activities are not resumed within 2 years from the date of shutdown, the *owner or operator* shall have the *qualified inspector* perform a final inspection and certify that all disturbed areas have achieved *final stabilization*, and all temporary, structural erosion and sediment control measures have been removed; and that all post-construction stormwater management practices have been constructed in conformance with the SWPPP by signing the “*Final Stabilization*” and “*Post-Construction Stormwater Management Practice*” certification statements on the NOT. The *owner or operator* shall then submit the completed NOT form to the address in Part II.B.1 of this permit.
 - e. For construction sites that directly *discharge* to one of the 303(d) segments listed in Appendix E or is located in one of the watersheds listed in Appendix C, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
3. At a minimum, the *qualified inspector* shall inspect all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness, all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP, all areas of disturbance that have not achieved *final stabilization*, all points of *discharge* to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site*, and all points of *discharge* from the *construction site*.
 4. The *qualified inspector* shall prepare an inspection report subsequent to each and every inspection. At a minimum, the inspection report shall include and/or address the following:

- a. Date and time of inspection;
- b. Name and title of person(s) performing inspection;
- c. A description of the weather and soil conditions (e.g. dry, wet, saturated) at the time of the inspection;
- d. A description of the condition of the runoff at all points of *discharge* from the *construction site*. This shall include identification of any *discharges* of sediment from the *construction site*. Include *discharges* from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow;
- e. A description of the condition of all natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site* which receive runoff from disturbed areas. This shall include identification of any *discharges* of sediment to the surface waterbody;
- f. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance;
- g. Identification of all erosion and sediment control practices and pollution prevention measures that were not installed properly or are not functioning as designed and need to be reinstalled or replaced;
- h. Description and sketch of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection;
- i. Current phase of construction of all post-construction stormwater management practices and identification of all construction that is not in conformance with the SWPPP and technical standards;
- j. Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the post-construction stormwater management practice(s);
- k. Identification and status of all corrective actions that were required by previous inspection; and

- I. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The *qualified inspector* shall also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.
5. Within one business day of the completion of an inspection, the *qualified inspector* shall notify the *owner or operator* and appropriate contractor or subcontractor identified in Part III.A.6. of this permit of any corrective actions that need to be taken. The contractor or subcontractor shall begin implementing the corrective actions within one business day of this notification and shall complete the corrective actions in a reasonable time frame.
6. All inspection reports shall be signed by the *qualified inspector*. Pursuant to Part II.D.2. of this permit, the inspection reports shall be maintained on site with the SWPPP.

Part V. TERMINATION OF PERMIT COVERAGE

A. Termination of Permit Coverage

1. An *owner or operator* that is eligible to terminate coverage under this permit must submit a completed NOT form to the address in Part II.B.1 of this permit. The NOT form shall be one which is associated with this permit, signed in accordance with Part VII.H of this permit.
2. An *owner or operator* may terminate coverage when one or more the following conditions have been met:
 - a. Total project completion - All *construction activity* identified in the SWPPP has been completed; and all areas of disturbance have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all post-construction stormwater management practices have been constructed in conformance with the SWPPP and are operational;

- b. Planned shutdown with partial project completion - All soil disturbance activities have ceased; and all areas disturbed as of the project shutdown date have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational;
 - c. A new *owner or operator* has obtained coverage under this permit in accordance with Part II.F. of this permit.
 - d. The *owner or operator* obtains coverage under an alternative SPDES general permit or an individual SPDES permit.
3. For *construction activities* meeting subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *qualified inspector* perform a final site inspection prior to submitting the NOT. The *qualified inspector* shall, by signing the “*Final Stabilization*” and “Post-Construction Stormwater Management Practice certification statements on the NOT, certify that all the requirements in Part V.A.2.a. or b. of this permit have been achieved.
4. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4* and meet subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *regulated, traditional land use control MS4* sign the “MS4 Acceptance” statement on the NOT in accordance with the requirements in Part VII.H. of this permit. The *regulated, traditional land use control MS4* official, by signing this statement, has determined that it is acceptable for the *owner or operator* to submit the NOT in accordance with the requirements of this Part. The *regulated, traditional land use control MS4* can make this determination by performing a final site inspection themselves or by accepting the *qualified inspector’s* final site inspection certification(s) required in Part V.A.3. of this permit.
5. For *construction activities* that require post-construction stormwater management practices and meet subdivision 2a. of this Part, the *owner or operator* must, prior to submitting the NOT, ensure one of the following:
- a. the post-construction stormwater management practice(s) and any right-of-way(s) needed to maintain such practice(s) have been deeded to the municipality in which the practice(s) is located,

- b. an executed maintenance agreement is in place with the municipality that will maintain the post-construction stormwater management practice(s),
- c. for post-construction stormwater management practices that are privately owned, the *owner or operator* has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the *owner or operator's* deed of record,
- d. for post-construction stormwater management practices that are owned by a public or private institution (e.g. school, university, hospital), government agency or authority, or public utility; the *owner or operator* has policy and procedures in place that ensures operation and maintenance of the practices in accordance with the operation and maintenance plan.

Part VI. REPORTING AND RETENTION RECORDS

A. Record Retention

The *owner or operator* shall retain a copy of the NOI, NOI Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form and any inspection reports that were prepared in conjunction with this permit for a period of at least five (5) years from the date that the Department receives a complete NOT submitted in accordance with Part V. of this general permit.

B. Addresses

With the exception of the NOI, NOT, and MS4 SWPPP Acceptance form (which must be submitted to the address referenced in Part II.B.1 of this permit), all written correspondence requested by the Department, including individual permit applications, shall be sent to the address of the appropriate DOW Water (SPDES) Program contact at the Regional Office listed in Appendix F.

Part VII. STANDARD PERMIT CONDITIONS

A. Duty to Comply

The *owner or operator* must comply with all conditions of this permit. All contractors and subcontractors associated with the project must comply with the terms of the SWPPP. Any non-compliance with this permit constitutes a violation of the Clean Water

Act (CWA) and the ECL and is grounds for an enforcement action against the *owner or operator* and/or the contractor/subcontractor; permit revocation, suspension or modification; or denial of a permit renewal application. Upon a finding of significant non-compliance with this permit or the applicable SWPPP, the Department may order an immediate stop to all *construction activity* at the site until the non-compliance is remedied. The stop work order shall be in writing, shall describe the non-compliance in detail, and shall be sent to the *owner or operator*.

If any human remains or archaeological remains are encountered during excavation, the *owner or operator* must immediately cease, or cause to cease, all *construction activity* in the area of the remains and notify the appropriate Regional Water Engineer (RWE). *Construction activity* shall not resume until written permission to do so has been received from the RWE.

B. Continuation of the Expired General Permit

This permit expires five (5) years from the effective date. If a new general permit is not issued prior to the expiration of this general permit, an *owner or operator* with coverage under this permit may continue to operate and *discharge* in accordance with the terms and conditions of this general permit, if it is extended pursuant to the State Administrative Procedure Act and 6 NYCRR Part 621, until a new general permit is issued.

C. Enforcement

Failure of the *owner or operator*, its contractors, subcontractors, agents and/or assigns to strictly adhere to any of the permit requirements contained herein shall constitute a violation of this permit. There are substantial criminal, civil, and administrative penalties associated with violating the provisions of this permit. Fines of up to \$37,500 per day for each violation and imprisonment for up to fifteen (15) years may be assessed depending upon the nature and degree of the offense.

D. Need to Halt or Reduce Activity Not a Defense

It shall not be a defense for an *owner or operator* in an enforcement action that it would have been necessary to halt or reduce the *construction activity* in order to maintain compliance with the conditions of this permit.

E. Duty to Mitigate

The *owner or operator* and its contractors and subcontractors shall take all reasonable steps to *minimize* or prevent any *discharge* in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

F. Duty to Provide Information

The *owner or operator* shall furnish to the Department, within a reasonable specified time period of a written request, all documentation necessary to demonstrate eligibility and any information to determine compliance with this permit or to determine whether cause exists for modifying or revoking this permit, or suspending or denying coverage under this permit, in accordance with the terms and conditions of this permit. The NOI, SWPPP and inspection reports required by this permit are public documents that the *owner or operator* must make available for review and copying by any person within five (5) business days of the *owner or operator* receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.

G. Other Information

When the *owner or operator* becomes aware that they failed to submit any relevant facts, or submitted incorrect information in the NOI or in any of the documents required by this permit, or have made substantive revisions to the SWPPP (e.g. the scope of the project changes significantly, the type of post-construction stormwater management practice(s) changes, there is a reduction in the sizing of the post-construction stormwater management practice, or there is an increase in the disturbance area or *impervious area*), which were not reflected in the original NOI submitted to the Department, they shall promptly submit such facts or information to the Department using the contact information in Part II.A. of this permit. Failure of the *owner or operator* to correct or supplement any relevant facts within five (5) business days of becoming aware of the deficiency shall constitute a violation of this permit.

H. Signatory Requirements

1. All NOIs and NOTs shall be signed as follows:
 - a. For a corporation these forms shall be signed by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means:

- (i) a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy or decision-making functions for the corporation; or
 - (ii) the manager of one or more manufacturing, production or operating facilities, provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;
 - b. For a partnership or sole proprietorship these forms shall be signed by a general partner or the proprietor, respectively; or
 - c. For a municipality, State, Federal, or other public agency these forms shall be signed by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes:
 - (i) the chief executive officer of the agency, or
 - (ii) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).
2. The SWPPP and other information requested by the Department shall be signed by a person described in Part VII.H.1. of this permit or by a duly authorized representative of that person. A person is a duly authorized representative only if:
- a. The authorization is made in writing by a person described in Part VII.H.1. of this permit;
 - b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of plant manager, operator of a well or a well field,

superintendent, position of *equivalent* responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position) and,

- c. The written authorization shall include the name, title and signature of the authorized representative and be attached to the SWPPP.
3. All inspection reports shall be signed by the *qualified inspector* that performs the inspection.
4. The MS4 SWPPP Acceptance form shall be signed by the principal executive officer or ranking elected official from the *regulated, traditional land use control MS4*, or by a duly authorized representative of that person.

It shall constitute a permit violation if an incorrect and/or improper signatory authorizes any required forms, SWPPP and/or inspection reports.

I. Property Rights

The issuance of this permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of Federal, State or local laws or regulations. *Owners or operators* must obtain any applicable conveyances, easements, licenses and/or access to real property prior to *commencing construction activity*.

J. Severability

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit shall not be affected thereby.

K. Requirement to Obtain Coverage Under an Alternative Permit

1. The Department may require any owner or operator authorized by this permit to apply for and/or obtain either an individual SPDES permit or another SPDES general permit. When the Department requires any discharger authorized by a general permit to apply for an individual SPDES permit, it shall notify the discharger in writing that a permit application is required. This notice shall

include a brief statement of the reasons for this decision, an application form, a statement setting a time frame for the owner or operator to file the application for an individual SPDES permit, and a deadline, not sooner than 180 days from owner or operator receipt of the notification letter, whereby the authorization to discharge under this general permit shall be terminated. Applications must be submitted to the appropriate Permit Administrator at the Regional Office. The Department may grant additional time upon demonstration, to the satisfaction of the Department, that additional time to apply for an alternative authorization is necessary or where the Department has not provided a permit determination in accordance with Part 621 of this Title.

2. When an individual SPDES permit is issued to a discharger authorized to *discharge* under a general SPDES permit for the same *discharge(s)*, the general permit authorization for outfalls authorized under the individual SPDES permit is automatically terminated on the effective date of the individual permit unless termination is earlier in accordance with 6 NYCRR Part 750.

L. Proper Operation and Maintenance

The *owner or operator* shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the *owner or operator* to achieve compliance with the conditions of this permit and with the requirements of the SWPPP.

M. Inspection and Entry

The *owner or operator* shall allow an authorized representative of the Department, EPA, applicable county health department, or, in the case of a *construction site* which *discharges* through an *MS4*, an authorized representative of the *MS4* receiving the discharge, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the owner's or operator's premises where a regulated facility or activity is located or conducted or where records must be kept under the conditions of this permit;
2. Have access to and copy at reasonable times, any records that must be kept under the conditions of this permit; and

3. Inspect at reasonable times any facilities or equipment (including monitoring and control equipment), practices or operations regulated or required by this permit.
4. Sample or monitor at reasonable times, for purposes of assuring permit compliance or as otherwise authorized by the Act or ECL, any substances or parameters at any location.

N. Permit Actions

This permit may, at any time, be modified, suspended, revoked, or renewed by the Department in accordance with 6 NYCRR Part 621. The filing of a request by the *owner or operator* for a permit modification, revocation and reissuance, termination, a notification of planned changes or anticipated noncompliance does not limit, diminish and/or stay compliance with any terms of this permit.

O. Definitions

Definitions of key terms are included in Appendix A of this permit.

P. Re-Opener Clause

1. If there is evidence indicating potential or realized impacts on water quality due to any stormwater discharge associated with construction activity covered by this permit, the owner or operator of such discharge may be required to obtain an individual permit or alternative general permit in accordance with Part VII.K. of this permit or the permit may be modified to include different limitations and/or requirements.
2. Any Department initiated permit modification, suspension or revocation will be conducted in accordance with 6 NYCRR Part 621, 6 NYCRR 750-1.18, and 6 NYCRR 750-1.20.

Q. Penalties for Falsification of Forms and Reports

In accordance with 6NYCRR Part 750-2.4 and 750-2.5, any person who knowingly makes any false material statement, representation, or certification in any application, record, report or other document filed or required to be maintained under this permit, including reports of compliance or noncompliance shall, upon conviction, be punished in accordance with ECL §71-1933 and or Articles 175 and 210 of the New York State Penal Law.

R. Other Permits

Nothing in this permit relieves the *owner or operator* from a requirement to obtain any other permits required by law.

APPENDIX A – Acronyms and Definitions

Acronyms

APO – Agency Preservation Officer
BMP – Best Management Practice
CPESC – Certified Professional in Erosion and Sediment Control
Cpv – Channel Protection Volume
CWA – Clean Water Act (or the Federal Water Pollution Control Act, 33 U.S.C. §1251 et seq)
DOW – Division of Water
EAF – Environmental Assessment Form
ECL - Environmental Conservation Law
EPA – U. S. Environmental Protection Agency
HSG – Hydrologic Soil Group
MS4 – Municipal Separate Storm Sewer System
NOI – Notice of Intent
NOT – Notice of Termination
NPDES – National Pollutant Discharge Elimination System
OPRHP – Office of Parks, Recreation and Historic Places
Qf – Extreme Flood
Qp – Overbank Flood
RRv – Runoff Reduction Volume
RWE – Regional Water Engineer
SEQR – State Environmental Quality Review
SEQRA - State Environmental Quality Review Act
SHPA – State Historic Preservation Act
SPDES – State Pollutant Discharge Elimination System
SWPPP – Stormwater Pollution Prevention Plan
TMDL – Total Maximum Daily Load
UPA – Uniform Procedures Act
USDA – United States Department of Agriculture
WQv – Water Quality Volume

Definitions

All definitions in this section are solely for the purposes of this permit.

Agricultural Building – a structure designed and constructed to house farm implements, hay, grain, poultry, livestock or other horticultural products; excluding any structure designed, constructed or used, in whole or in part, for human habitation, as a place of employment where agricultural products are processed, treated or packaged, or as a place used by the public.

Agricultural Property – means the land for construction of a barn, *agricultural building*, silo, stockyard, pen or other structural practices identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State” prepared by the Department in cooperation with agencies of New York Nonpoint Source Coordinating Committee (dated June 2007).

Alter Hydrology from Pre to Post-Development Conditions - means the post-development peak flow rate(s) has increased by more than 5% of the pre-developed condition for the design storm of interest (e.g. 10 yr and 100 yr).

Combined Sewer - means a sewer that is designed to collect and convey both “sewage” and “stormwater”.

Commence (Commencement of) Construction Activities - means the initial disturbance of soils associated with clearing, grading or excavation activities; or other construction related activities that disturb or expose soils such as demolition, stockpiling of fill material, and the initial installation of erosion and sediment control practices required in the SWPPP. See definition for “*Construction Activity(ies)*” also.

Construction Activity(ies) - means any clearing, grading, excavation, filling, demolition or stockpiling activities that result in soil disturbance. Clearing activities can include, but are not limited to, logging equipment operation, the cutting and skidding of trees, stump removal and/or brush root removal. Construction activity does not include routine maintenance that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility.

Construction Site – means the land area where *construction activity(ies)* will occur. See definition for “*Commence (Commencement of) Construction Activities*” and “*Larger Common Plan of Development or Sale*” also.

Dewatering – means the act of draining rainwater and/or groundwater from building foundations, vaults or excavations/trenches.

Direct Discharge (to a specific surface waterbody) - means that runoff flows from a *construction site* by overland flow and the first point of discharge is the specific surface waterbody, or runoff flows from a *construction site* to a separate storm sewer system

and the first point of discharge from the separate storm sewer system is the specific surface waterbody.

Discharge(s) - means any addition of any pollutant to waters of the State through an outlet or *point source*.

Embankment – means an earthen or rock slope that supports a road/highway.

Endangered or Threatened Species – see 6 NYCRR Part 182 of the Department’s rules and regulations for definition of terms and requirements.

Environmental Conservation Law (ECL) - means chapter 43-B of the Consolidated Laws of the State of New York, entitled the Environmental Conservation Law.

Equivalent (Equivalence) – means that the practice or measure meets all the performance, longevity, maintenance, and safety objectives of the technical standard and will provide an equal or greater degree of water quality protection.

Final Stabilization - means that all soil disturbance activities have ceased and a uniform, perennial vegetative cover with a density of eighty (80) percent over the entire pervious surface has been established; or other equivalent stabilization measures, such as permanent landscape mulches, rock rip-rap or washed/crushed stone have been applied on all disturbed areas that are not covered by permanent structures, concrete or pavement.

General SPDES permit - means a SPDES permit issued pursuant to 6 NYCRR Part 750-1.21 and Section 70-0117 of the ECL authorizing a category of discharges.

Groundwater(s) - means waters in the saturated zone. The saturated zone is a subsurface zone in which all the interstices are filled with water under pressure greater than that of the atmosphere. Although the zone may contain gas-filled interstices or interstices filled with fluids other than water, it is still considered saturated.

Historic Property – means any building, structure, site, object or district that is listed on the State or National Registers of Historic Places or is determined to be eligible for listing on the State or National Registers of Historic Places.

Impervious Area (Cover) - means all impermeable surfaces that cannot effectively infiltrate rainfall. This includes paved, concrete and gravel surfaces (i.e. parking lots, driveways, roads, runways and sidewalks); building rooftops and miscellaneous impermeable structures such as patios, pools, and sheds.

Infeasible – means not technologically possible, or not economically practicable and achievable in light of best industry practices.

Larger Common Plan of Development or Sale - means a contiguous area where multiple separate and distinct *construction activities* are occurring, or will occur, under one plan. The term “plan” in “larger common plan of development or sale” is broadly defined as any announcement or piece of documentation (including a sign, public notice or hearing, marketing plan, advertisement, drawing, permit application, State Environmental Quality Review Act (SEQRA) environmental assessment form or other documents, zoning request, computer design, etc.) or physical demarcation (including boundary signs, lot stakes, surveyor markings, etc.) indicating that *construction activities* may occur on a specific plot.

For discrete construction projects that are located within a larger common plan of development or sale that are at least 1/4 mile apart, each project can be treated as a separate plan of development or sale provided any interconnecting road, pipeline or utility project that is part of the same “common plan” is not concurrently being disturbed.

Minimize – means reduce and/or eliminate to the extent achievable using control measures (including best management practices) that are technologically available and economically practicable and achievable in light of best industry practices.

Municipal Separate Storm Sewer (MS4) - a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains):

- (i) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA that discharges to surface waters of the State;
- (ii) Designed or used for collecting or conveying stormwater;
- (iii) Which is not a *combined sewer*; and
- (iv) Which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2.

National Pollutant Discharge Elimination System (NPDES) - means the national system for the issuance of wastewater and stormwater permits under the Federal Water Pollution Control Act (Clean Water Act).

Natural Buffer – means an undisturbed area with natural cover running along a surface water (e.g. wetland, stream, river, lake, etc.).

New Development – means any land disturbance that does not meet the definition of Redevelopment Activity included in this appendix.

New York State Erosion and Sediment Control Certificate Program – a certificate program that establishes and maintains a process to identify and recognize individuals who are capable of developing, designing, inspecting and maintaining erosion and sediment control plans on projects that disturb soils in New York State. The certificate program is administered by the New York State Conservation District Employees Association.

NOI Acknowledgment Letter - means the letter that the Department sends to an owner or operator to acknowledge the Department's receipt and acceptance of a complete Notice of Intent. This letter documents the owner's or operator's authorization to discharge in accordance with the general permit for stormwater discharges from *construction activity*.

Nonpoint Source - means any source of water pollution or pollutants which is not a discrete conveyance or *point source* permitted pursuant to Title 7 or 8 of Article 17 of the Environmental Conservation Law (see ECL Section 17-1403).

Overbank –means flow events that exceed the capacity of the stream channel and spill out into the adjacent floodplain.

Owner or Operator - means the person, persons or legal entity which owns or leases the property on which the *construction activity* is occurring; an entity that has operational control over the construction plans and specifications, including the ability to make modifications to the plans and specifications; and/or an entity that has day-to-day operational control of those activities at a project that are necessary to ensure compliance with the permit conditions.

Performance Criteria – means the design criteria listed under the “Required Elements” sections in Chapters 5, 6 and 10 of the technical standard, New York State Stormwater Management Design Manual, dated January 2015. It does not include the Sizing Criteria (i.e. WQv, RRv, Cpv, Qp and Qf) in Part I.C.2. of the permit.

Point Source - means any discernible, confined and discrete conveyance, including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, container, rolling stock, concentrated animal feeding operation, vessel or other floating craft, or landfill leachate collection system from which *pollutants* are or may be discharged.

Pollutant - means dredged spoil, filter backwash, solid waste, incinerator residue, sewage, garbage, sewage sludge, munitions, chemical wastes, biological materials, radioactive materials, heat, wrecked or discarded equipment, rock, sand and industrial, municipal, agricultural waste and ballast discharged into water; which may cause or might reasonably be expected to cause pollution of the waters of the state in contravention of the standards or guidance values adopted as provided in 6 NYCRR Parts 700 et seq .

Qualified Inspector - means a person that is knowledgeable in the principles and practices of erosion and sediment control, such as a licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder or other Department endorsed individual(s).

It can also mean someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided that person has training in the principles and practices of erosion and sediment control. Training in the principles and practices of erosion and sediment control means that the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect shall receive four (4) hours of training every three (3) years.

It can also mean a person that meets the *Qualified Professional* qualifications in addition to the *Qualified Inspector* qualifications.

Note: Inspections of any post-construction stormwater management practices that include structural components, such as a dam for an impoundment, shall be performed by a licensed Professional Engineer.

Qualified Professional - means a person that is knowledgeable in the principles and practices of stormwater management and treatment, such as a licensed Professional Engineer, Registered Landscape Architect or other Department endorsed individual(s). Individuals preparing SWPPPs that require the post-construction stormwater management practice component must have an understanding of the principles of hydrology, water quality management practice design, water quantity control design, and, in many cases, the principles of hydraulics. All components of the SWPPP that involve the practice of engineering, as defined by the NYS Education Law (see Article 145), shall be prepared by, or under the direct supervision of, a professional engineer licensed to practice in the State of New York.

Redevelopment Activity(ies) – means the disturbance and reconstruction of existing impervious area, including impervious areas that were removed from a project site within five (5) years of preliminary project plan submission to the local government (i.e. site plan, subdivision, etc.).

Regulated, Traditional Land Use Control MS4 - means a city, town or village with land use control authority that is authorized to discharge under New York State DEC's

SPDES General Permit For Stormwater Discharges from Municipal Separate Stormwater Sewer Systems (MS4s) or the City of New York's Individual SPDES Permit for their Municipal Separate Storm Sewer Systems (NY-0287890).

Routine Maintenance Activity - means *construction activity* that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility, including, but not limited to:

- Re-grading of gravel roads or parking lots,
- Cleaning and shaping of existing roadside ditches and culverts that maintains the approximate original line and grade, and hydraulic capacity of the ditch,
- Cleaning and shaping of existing roadside ditches that does not maintain the approximate original grade, hydraulic capacity and purpose of the ditch if the changes to the line and grade, hydraulic capacity or purpose of the ditch are installed to improve water quality and quantity controls (e.g. installing grass lined ditch),
- Placement of aggregate shoulder backing that stabilizes the transition between the road shoulder and the ditch or *embankment*,
- Full depth milling and filling of existing asphalt pavements, replacement of concrete pavement slabs, and similar work that does not expose soil or disturb the bottom six (6) inches of subbase material,
- Long-term use of equipment storage areas at or near highway maintenance facilities,
- Removal of sediment from the edge of the highway to restore a previously existing sheet-flow drainage connection from the highway surface to the highway ditch or *embankment*,
- Existing use of Canal Corp owned upland disposal sites for the canal, and
- Replacement of curbs, gutters, sidewalks and guide rail posts.

Site limitations – means site conditions that prevent the use of an infiltration technique and or infiltration of the total WQv. Typical site limitations include: seasonal high groundwater, shallow depth to bedrock, and soils with an infiltration rate less than 0.5 inches/hour. The existence of site limitations shall be confirmed and documented using actual field testing (i.e. test pits, soil borings, and infiltration test) or using information from the most current United States Department of Agriculture (USDA) Soil Survey for the County where the project is located.

Sizing Criteria – means the criteria included in Part I.C.2 of the permit that are used to size post-construction stormwater management control practices. The criteria include; Water Quality Volume (WQv), Runoff Reduction Volume (RRv), Channel Protection Volume (Cpv), *Overbank Flood* (Qp), and *Extreme Flood* (Qf).

State Pollutant Discharge Elimination System (SPDES) - means the system established pursuant to Article 17 of the ECL and 6 NYCRR Part 750 for issuance of permits authorizing discharges to the waters of the state.

Steep Slope – means land area designated on the current United States Department of Agriculture (“USDA”) Soil Survey as Soil Slope Phase “D”, (provided the map unit name is inclusive of slopes greater than 25%) , or Soil Slope Phase E or F, (regardless of the map unit name), or a combination of the three designations.

Streambank – as used in this permit, means the terrain alongside the bed of a creek or stream. The bank consists of the sides of the channel, between which the flow is confined.

Stormwater Pollution Prevention Plan (SWPPP) – means a project specific report, including construction drawings, that among other things: describes the construction activity(ies), identifies the potential sources of pollution at the *construction site*; describes and shows the stormwater controls that will be used to control the pollutants (i.e. erosion and sediment controls; for many projects, includes post-construction stormwater management controls); and identifies procedures the *owner or operator* will implement to comply with the terms and conditions of the permit. See Part III of the permit for a complete description of the information that must be included in the SWPPP.

Surface Waters of the State - shall be construed to include lakes, bays, sounds, ponds, impounding reservoirs, springs, rivers, streams, creeks, estuaries, marshes, inlets, canals, the Atlantic ocean within the territorial seas of the state of New York and all other bodies of surface water, natural or artificial, inland or coastal, fresh or salt, public or private (except those private waters that do not combine or effect a junction with natural surface waters), which are wholly or partially within or bordering the state or within its jurisdiction. Waters of the state are further defined in 6 NYCRR Parts 800 to 941.

Temporarily Ceased – means that an existing disturbed area will not be disturbed again within 14 calendar days of the previous soil disturbance.

Temporary Stabilization - means that exposed soil has been covered with material(s) as set forth in the technical standard, New York Standards and Specifications for Erosion and Sediment Control, to prevent the exposed soil from eroding. The materials can include, but are not limited to, mulch, seed and mulch, and erosion control mats (e.g. jute twisted yarn, excelsior wood fiber mats).

Total Maximum Daily Loads (TMDLs) - A TMDL is the sum of the allowable loads of a single pollutant from all contributing point and *nonpoint sources*. It is a calculation of the maximum amount of a pollutant that a waterbody can receive on a daily basis and still meet *water quality standards*, and an allocation of that amount to the pollutant's sources. A TMDL stipulates wasteload allocations (WLAs) for *point source* discharges, load allocations (LAs) for *nonpoint sources*, and a margin of safety (MOS).

Trained Contractor - means an employee from the contracting (construction) company, identified in Part III.A.6., that has received four (4) hours of Department endorsed

training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the *trained contractor* shall receive four (4) hours of training every three (3) years.

It can also mean an employee from the contracting (construction) company, identified in Part III.A.6., that meets the *qualified inspector* qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity).

The *trained contractor* is responsible for the day to day implementation of the SWPPP.

Uniform Procedures Act (UPA) Permit - means a permit required under 6 NYCRR Part 621 of the Environmental Conservation Law (ECL), Article 70.

Water Quality Standard - means such measures of purity or quality for any waters in relation to their reasonable and necessary use as promulgated in 6 NYCRR Part 700 et seq.

APPENDIX B – Required SWPPP Components by Project Type

Table 1
Construction Activities that Require the Preparation of a SWPPP That Only
Includes Erosion and Sediment Controls

The following construction activities that involve soil disturbances of one (1) or more acres of land, but less than five (5) acres: • Single family home <u>not</u> located in one of the watersheds listed in Appendix C or <u>not directly discharging</u> to one of the 303(d) segments listed in Appendix E • Single family residential subdivisions with 25% or less impervious cover at total site build-out and <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E • Construction of a barn or other <i>agricultural building</i>, silo, stock yard or pen.
The following construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land: All construction activities located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
The following construction activities that involve soil disturbances of one (1) or more acres of land: • Installation of underground, linear utilities; such as gas lines, fiber-optic cable, cable TV, electric, telephone, sewer mains, and water mains • Environmental enhancement projects, such as wetland mitigation projects, stormwater retrofits and stream restoration projects • Pond construction • Linear bike paths running through areas with vegetative cover, including bike paths surfaced with an impervious cover • Cross-country ski trails and walking/hiking trails • Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are not part of residential, commercial or institutional development; • Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that include incidental shoulder or curb work along an existing highway to support construction of the sidewalk, bike path or walking path. • Slope stabilization projects • Slope flattening that changes the grade of the site, but does not significantly change the runoff characteristics

**Table 1 (Continued) CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP
THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS**

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Spoil areas that will be covered with vegetation
- Vegetated open space projects (i.e. recreational parks, lawns, meadows, fields, downhill ski trails) excluding projects that *alter hydrology from pre to post development* conditions,
- Athletic fields (natural grass) that do not include the construction or reconstruction of *impervious area* and do not *alter hydrology from pre to post development* conditions
- Demolition project where vegetation will be established, and no redevelopment is planned
- Overhead electric transmission line project that does not include the construction of permanent access roads or parking areas surfaced with *impervious cover*
- Structural practices as identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State”, excluding projects that involve soil disturbances of greater than five acres and construction activities that include the construction or reconstruction of impervious area
- Temporary access roads, median crossovers, detour roads, lanes, or other temporary impervious areas that will be restored to pre-construction conditions once the construction activity is complete

Table 2
CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES
POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Single family home located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family home that disturbs five (5) or more acres of land
- Single family residential subdivisions located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family residential subdivisions that involve soil disturbances of between one (1) and five (5) acres of land with greater than 25% impervious cover at total site build-out
- Single family residential subdivisions that involve soil disturbances of five (5) or more acres of land, and single family residential subdivisions that involve soil disturbances of less than five (5) acres that are part of a larger common plan of development or sale that will ultimately disturb five or more acres of land
- Multi-family residential developments; includes duplexes, townhomes, condominiums, senior housing complexes, apartment complexes, and mobile home parks
- Airports
- Amusement parks
- Breweries, cideries, and wineries, including establishments constructed on agricultural land
- Campgrounds
- Cemeteries that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development conditions*
- Commercial developments
- Churches and other places of worship
- Construction of a barn or other *agricultural building* (e.g. silo) and structural practices as identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State" that include the construction or reconstruction of *impervious area*, excluding projects that involve soil disturbances of less than five acres.
- Golf courses
- Institutional development; includes hospitals, prisons, schools and colleges
- Industrial facilities; includes industrial parks
- Landfills
- Municipal facilities; includes highway garages, transfer stations, office buildings, POTW's, water treatment plants, and water storage tanks
- Office complexes
- Playgrounds that include the construction or reconstruction of impervious area
- Sports complexes
- Racetracks; includes racetracks with earthen (dirt) surface
- Road construction or reconstruction, including roads constructed as part of the construction activities listed in Table 1

Table 2 (Continued)

**CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES
POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES**

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Parking lot construction or reconstruction, including parking lots constructed as part of the construction activities listed in Table 1
- Athletic fields (natural grass) that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development* conditions
- Athletic fields with artificial turf
- Permanent access roads, parking areas, substations, compressor stations and well drilling pads, surfaced with *impervious cover*, and constructed as part of an over-head electric transmission line project, wind-power project, cell tower project, oil or gas well drilling project, sewer or water main project or other linear utility project
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a residential, commercial or institutional development
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a highway construction or reconstruction project
- All other construction activities that include the construction or reconstruction of *impervious area* or *alter the hydrology from pre to post development* conditions, and are not listed in Table 1

APPENDIX C – Watersheds Requiring Enhanced Phosphorus Removal

Watersheds where *owners or operators* of construction activities identified in Table 2 of Appendix B must prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the Enhanced Phosphorus Removal Standards included in the technical standard, New York State Stormwater Management Design Manual (“Design Manual”).

- Entire New York City Watershed located east of the Hudson River - Figure 1
- Onondaga Lake Watershed - Figure 2
- Greenwood Lake Watershed -Figure 3
- Oscawana Lake Watershed – Figure 4
- Kinderhook Lake Watershed – Figure 5

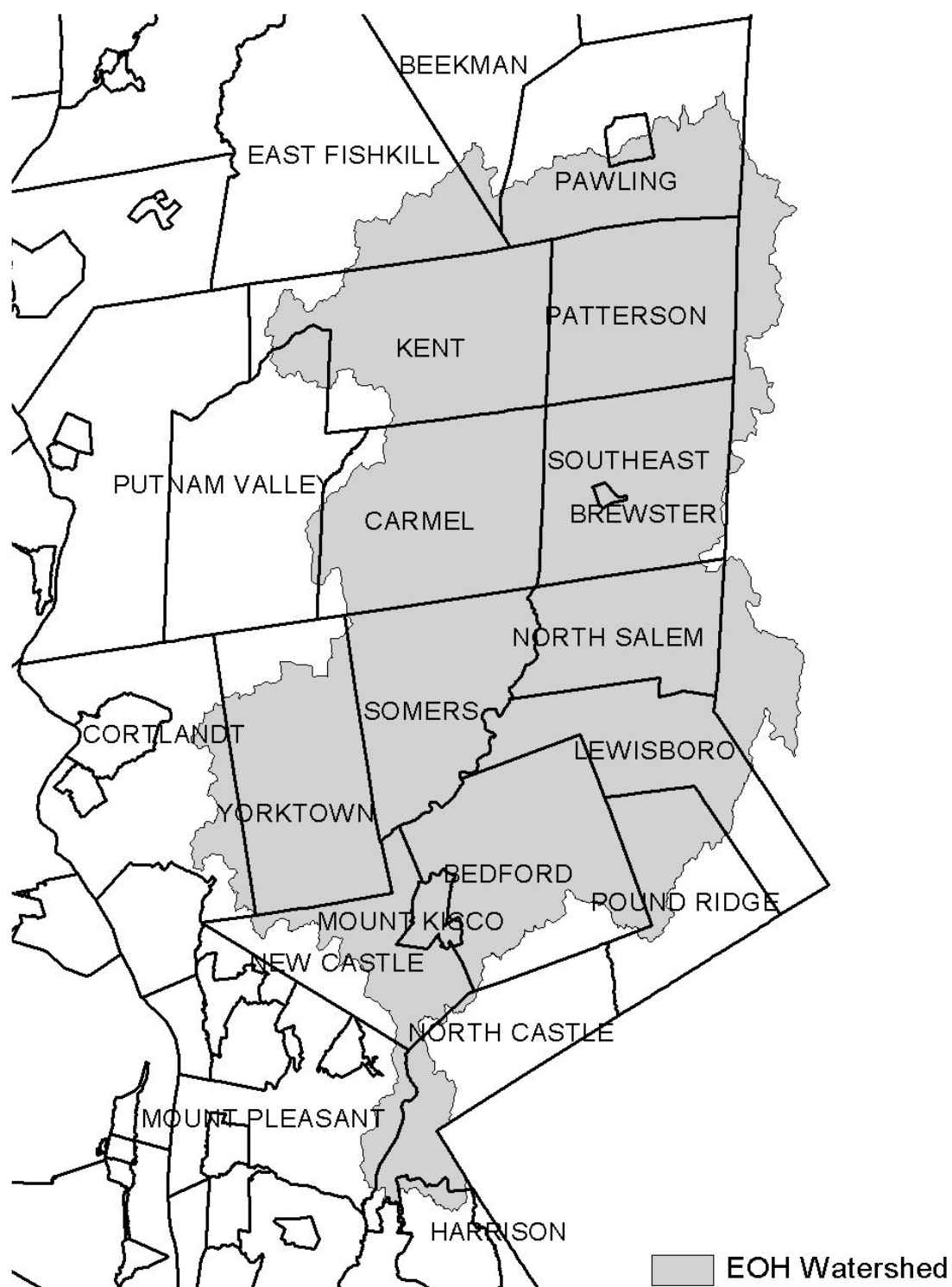
Figure 1 - New York City Watershed East of the Hudson

Figure 2 - Onondaga Lake Watershed

Figure 3 - Greenwood Lake Watershed

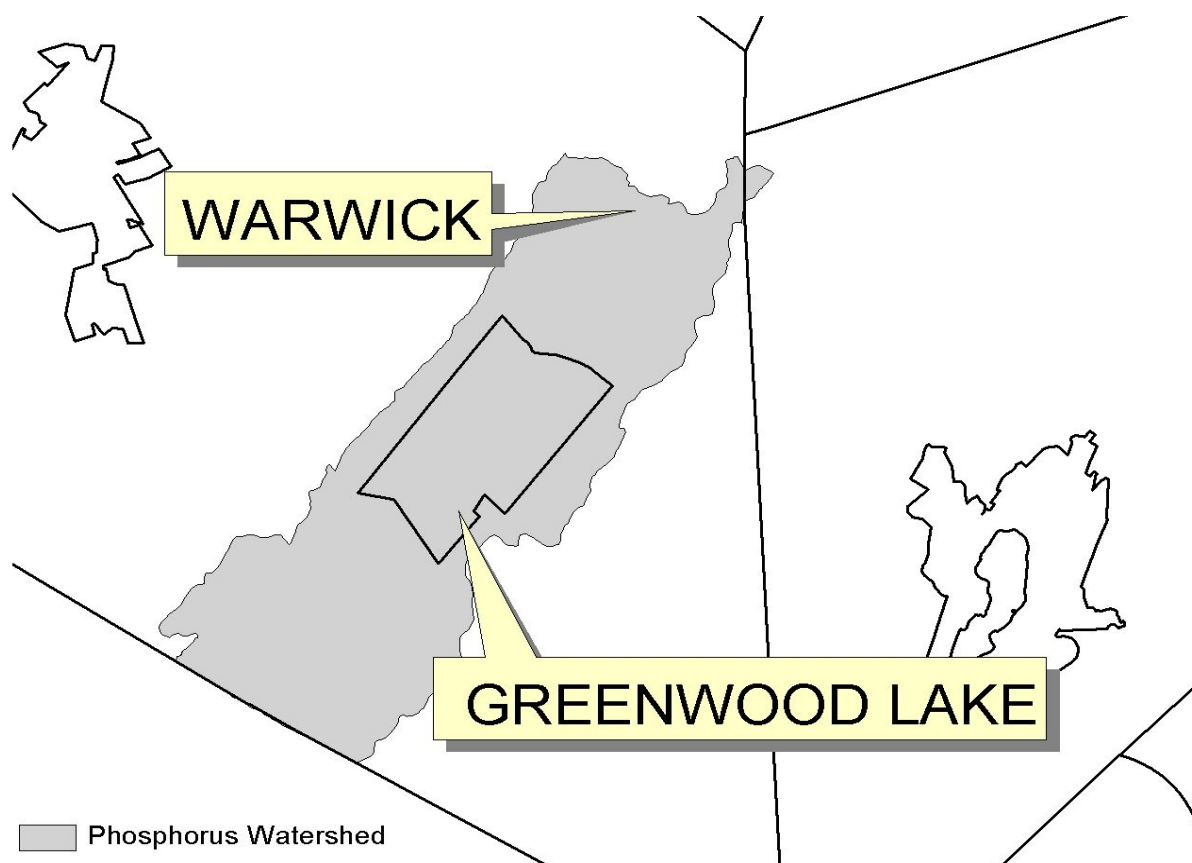


Figure 4 - Oscawana Lake Watershed

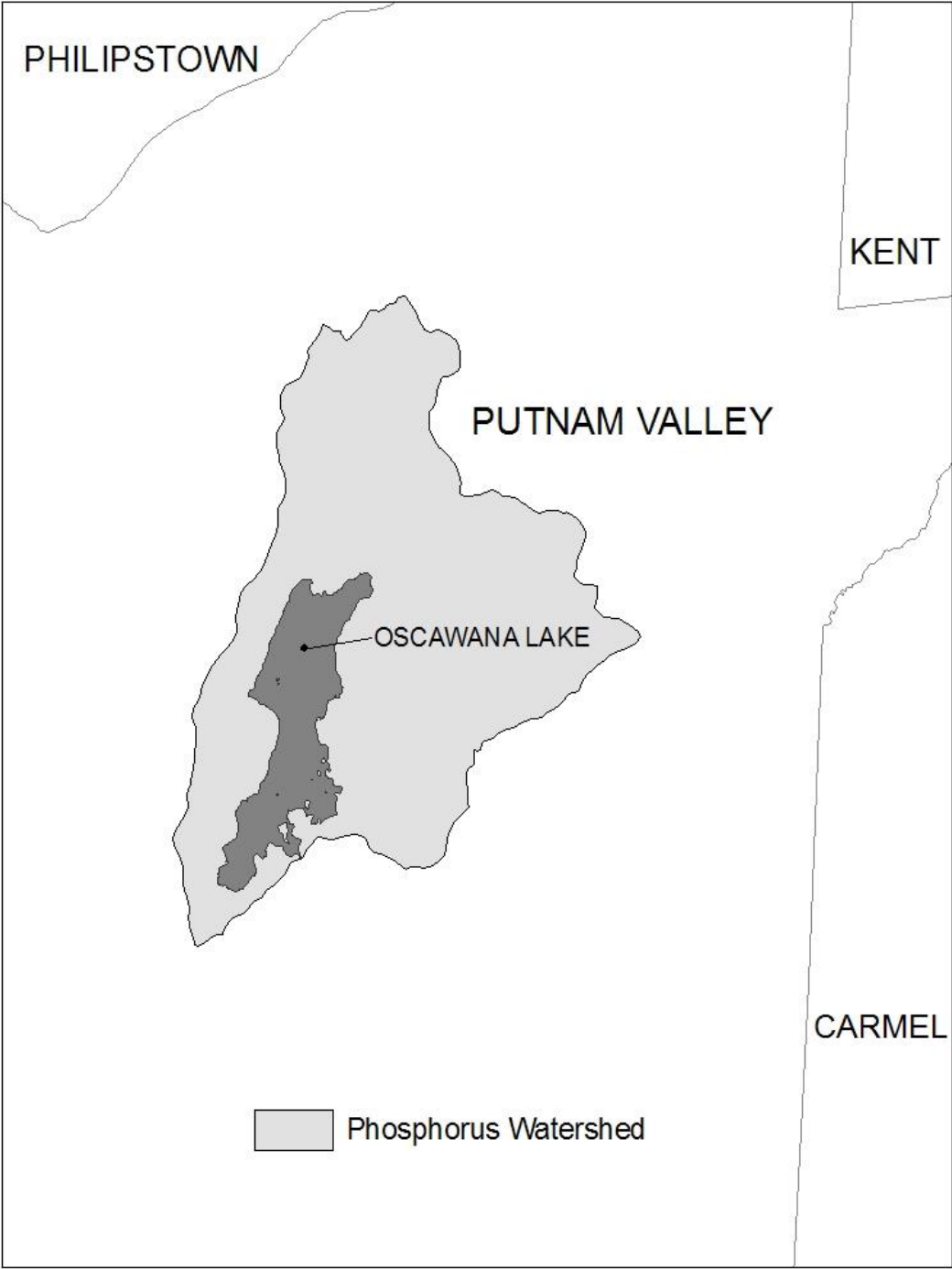
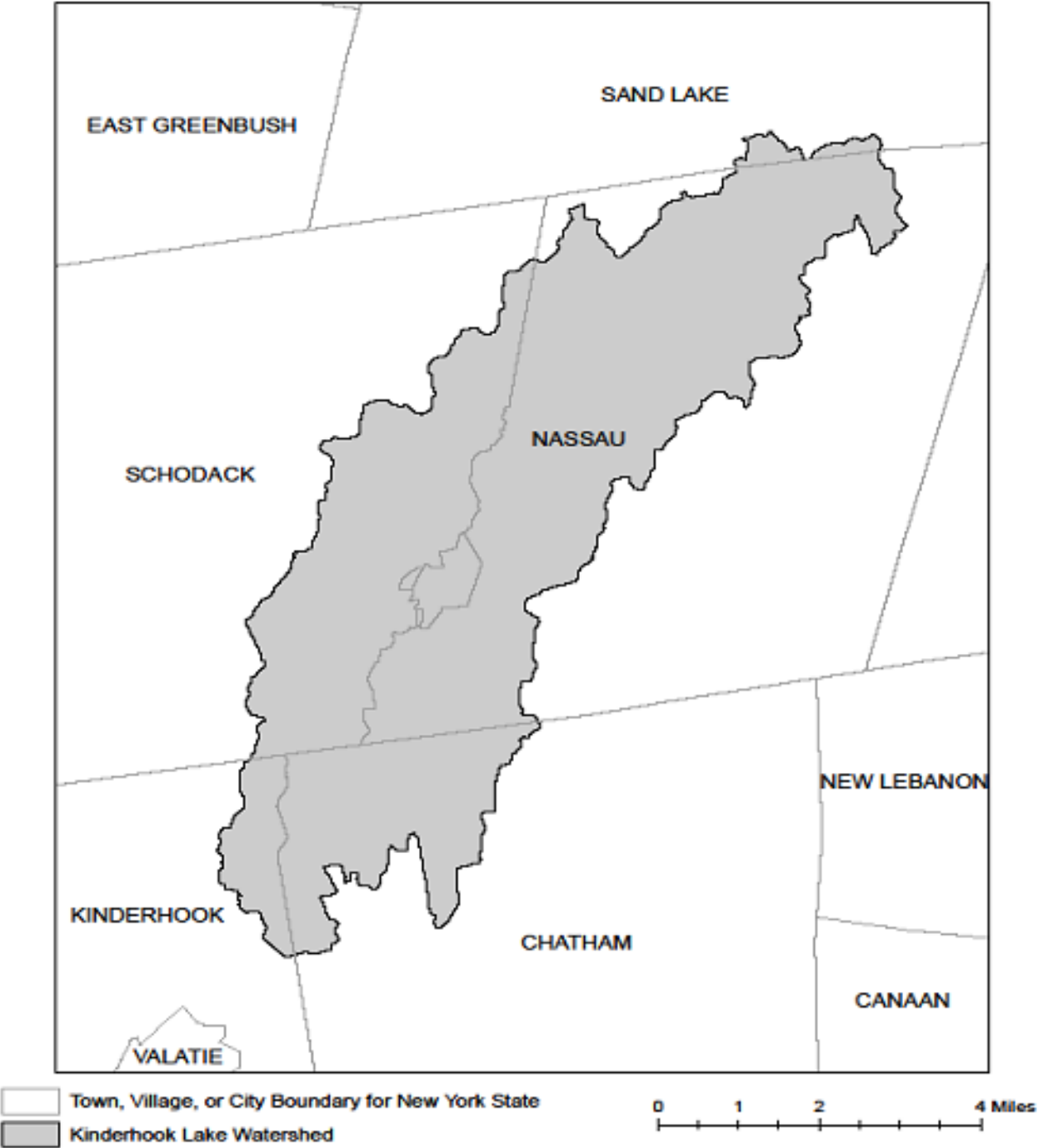


Figure 5 - Kinderhook Lake Watershed



APPENDIX D – Watersheds with Lower Disturbance Threshold

Watersheds where *owners or operators* of construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land must obtain coverage under this permit.

Entire New York City Watershed that is located east of the Hudson River - See Figure 1 in Appendix C
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APPENDIX E – 303(d) Segments Impaired by Construction Related Pollutant(s)

List of 303(d) segments impaired by pollutants related to *construction activity* (e.g. silt, sediment or nutrients). The list was developed using "The Final New York State 2016 Section 303(d) List of Impaired Waters Requiring a TMDL/Other Strategy" dated November 2016. *Owners or operators* of single family home and single family residential subdivisions with 25% or less total impervious cover at total site build-out that involve soil disturbances of one or more acres of land, but less than 5 acres, and *directly discharge* to one of the listed segments below shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the New York State Stormwater Management Design Manual ("Design Manual"), dated January 2015.

COUNTY	WATERBODY	POLLUTANT
Albany	Ann Lee (Shakers) Pond, Stump Pond	Nutrients
Albany	Basic Creek Reservoir	Nutrients
Allegany	Amity Lake, Saunders Pond	Nutrients
Bronx	Long Island Sound, Bronx	Nutrients
Bronx	Van Cortlandt Lake	Nutrients
Broome	Fly Pond, Deer Lake, Sky Lake	Nutrients
Broome	Minor Tribs to Lower Susquehanna (north)	Nutrients
Broome	Whitney Point Lake/Reservoir	Nutrients
Cattaraugus	Allegheny River/Reservoir	Nutrients
Cattaraugus	Beaver (Alma) Lake	Nutrients
Cattaraugus	Case Lake	Nutrients
Cattaraugus	Linlyco/Club Pond	Nutrients
Cayuga	Duck Lake	Nutrients
Cayuga	Little Sodus Bay	Nutrients
Chautauqua	Bear Lake	Nutrients
Chautauqua	Chadakoin River and tribs	Nutrients
Chautauqua	Chautauqua Lake, North	Nutrients
Chautauqua	Chautauqua Lake, South	Nutrients
Chautauqua	Findley Lake	Nutrients
Chautauqua	Hulburt/Clymer Pond	Nutrients
Clinton	Great Chazy River, Lower, Main Stem	Silt/Sediment
Clinton	Lake Champlain, Main Lake, Middle	Nutrients
Clinton	Lake Champlain, Main Lake, North	Nutrients
Columbia	Kinderhook Lake	Nutrients
Columbia	Robinson Pond	Nutrients
Cortland	Dean Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Dutchess	Fall Kill and tribs	Nutrients
Dutchess	Hillside Lake	Nutrients
Dutchess	Wappingers Lake	Nutrients
Dutchess	Wappingers Lake	Silt/Sediment
Erie	Beeman Creek and tribs	Nutrients
Erie	Ellicott Creek, Lower, and tribs	Silt/Sediment
Erie	Ellicott Creek, Lower, and tribs	Nutrients
Erie	Green Lake	Nutrients
Erie	Little Sister Creek, Lower, and tribs	Nutrients
Erie	Murder Creek, Lower, and tribs	Nutrients
Erie	Rush Creek and tribs	Nutrients
Erie	Scajaquada Creek, Lower, and tribs	Nutrients
Erie	Scajaquada Creek, Middle, and tribs	Nutrients
Erie	Scajaquada Creek, Upper, and tribs	Nutrients
Erie	South Branch Smoke Cr, Lower, and tribs	Silt/Sediment
Erie	South Branch Smoke Cr, Lower, and tribs	Nutrients
Essex	Lake Champlain, Main Lake, South	Nutrients
Essex	Lake Champlain, South Lake	Nutrients
Essex	Willsboro Bay	Nutrients
Genesee	Bigelow Creek and tribs	Nutrients
Genesee	Black Creek, Middle, and minor tribs	Nutrients
Genesee	Black Creek, Upper, and minor tribs	Nutrients
Genesee	Bowen Brook and tribs	Nutrients
Genesee	LeRoy Reservoir	Nutrients
Genesee	Oak Orchard Cr, Upper, and tribs	Nutrients
Genesee	Tonawanda Creek, Middle, Main Stem	Nutrients
Greene	Schoharie Reservoir	Silt/Sediment
Greene	Sleepy Hollow Lake	Silt/Sediment
Herkimer	Steele Creek tribs	Silt/Sediment
Herkimer	Steele Creek tribs	Nutrients
Jefferson	Moon Lake	Nutrients
Kings	Hendrix Creek	Nutrients
Kings	Prospect Park Lake	Nutrients
Lewis	Mill Creek/South Branch, and tribs	Nutrients
Livingston	Christie Creek and tribs	Nutrients
Livingston	Conesus Lake	Nutrients
Livingston	Mill Creek and minor tribs	Silt/Sediment
Monroe	Black Creek, Lower, and minor tribs	Nutrients
Monroe	Buck Pond	Nutrients
Monroe	Cranberry Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Monroe	Lake Ontario Shoreline, Western	Nutrients
Monroe	Long Pond	Nutrients
Monroe	Mill Creek and tribs	Nutrients
Monroe	Mill Creek/Blue Pond Outlet and tribs	Nutrients
Monroe	Minor Tribs to Irondequoit Bay	Nutrients
Monroe	Rochester Embayment - East	Nutrients
Monroe	Rochester Embayment - West	Nutrients
Monroe	Shipbuilders Creek and tribs	Nutrients
Monroe	Thomas Creek/White Brook and tribs	Nutrients
Nassau	Beaver Lake	Nutrients
Nassau	Camaans Pond	Nutrients
Nassau	East Meadow Brook, Upper, and tribs	Silt/Sediment
Nassau	East Rockaway Channel	Nutrients
Nassau	Grant Park Pond	Nutrients
Nassau	Hempstead Bay	Nutrients
Nassau	Hempstead Lake	Nutrients
Nassau	Hewlett Bay	Nutrients
Nassau	Hog Island Channel	Nutrients
Nassau	Long Island Sound, Nassau County Waters	Nutrients
Nassau	Massapequa Creek and tribs	Nutrients
Nassau	Milburn/Parsonage Creeks, Upp, and tribs	Nutrients
Nassau	Reynolds Channel, west	Nutrients
Nassau	Tidal Tribs to Hempstead Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Silt/Sediment
Nassau	Tribs to Smith/Halls Ponds	Nutrients
Nassau	Woodmere Channel	Nutrients
New York	Harlem Meer	Nutrients
New York	The Lake in Central Park	Nutrients
Niagara	Bergholtz Creek and tribs	Nutrients
Niagara	Hyde Park Lake	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Oneida	Ballou, Nail Creeks and tribs	Nutrients
Onondaga	Harbor Brook, Lower, and tribs	Nutrients
Onondaga	Ley Creek and tribs	Nutrients
Onondaga	Minor Tribs to Onondaga Lake	Nutrients
Onondaga	Ninemile Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Middle, and tribs	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Onondaga	Onondaga Lake, northern end	Nutrients
Onondaga	Onondaga Lake, southern end	Nutrients
Ontario	Great Brook and minor tribs	Silt/Sediment
Ontario	Great Brook and minor tribs	Nutrients
Ontario	Hemlock Lake Outlet and minor tribs	Nutrients
Ontario	Honeoye Lake	Nutrients
Orange	Greenwood Lake	Nutrients
Orange	Monhagen Brook and tribs	Nutrients
Orange	Orange Lake	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Oswego	Lake Neatahwanta	Nutrients
Oswego	Pleasant Lake	Nutrients
Putnam	Bog Brook Reservoir	Nutrients
Putnam	Boyd Corners Reservoir	Nutrients
Putnam	Croton Falls Reservoir	Nutrients
Putnam	Diverting Reservoir	Nutrients
Putnam	East Branch Reservoir	Nutrients
Putnam	Lake Carmel	Nutrients
Putnam	Middle Branch Reservoir	Nutrients
Putnam	Oscawana Lake	Nutrients
Putnam	Palmer Lake	Nutrients
Putnam	West Branch Reservoir	Nutrients
Queens	Bergen Basin	Nutrients
Queens	Flushing Creek/Bay	Nutrients
Queens	Jamaica Bay, Eastern, and tribs (Queens)	Nutrients
Queens	Kissena Lake	Nutrients
Queens	Meadow Lake	Nutrients
Queens	Willow Lake	Nutrients
Rensselaer	Nassau Lake	Nutrients
Rensselaer	Snyders Lake	Nutrients
Richmond	Grasmere Lake/Bradys Pond	Nutrients
Rockland	Congers Lake, Swartout Lake	Nutrients
Rockland	Rockland Lake	Nutrients
Saratoga	Ballston Lake	Nutrients
Saratoga	Dwaas Kill and tribs	Silt/Sediment
Saratoga	Dwaas Kill and tribs	Nutrients
Saratoga	Lake Lonely	Nutrients
Saratoga	Round Lake	Nutrients
Saratoga	Tribs to Lake Lonely	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Schenectady	Collins Lake	Nutrients
Schenectady	Duane Lake	Nutrients
Schenectady	Mariaville Lake	Nutrients
Schoharie	Engleville Pond	Nutrients
Schoharie	Summit Lake	Nutrients
Seneca	Reeder Creek and tribs	Nutrients
St.Lawrence	Black Lake Outlet/Black Lake	Nutrients
St.Lawrence	Fish Creek and minor tribs	Nutrients
Steuben	Smith Pond	Nutrients
Suffolk	Agawam Lake	Nutrients
Suffolk	Big/Little Fresh Ponds	Nutrients
Suffolk	Canaan Lake	Silt/Sediment
Suffolk	Canaan Lake	Nutrients
Suffolk	Flanders Bay, West/Lower Sawmill Creek	Nutrients
Suffolk	Fresh Pond	Nutrients
Suffolk	Great South Bay, East	Nutrients
Suffolk	Great South Bay, Middle	Nutrients
Suffolk	Great South Bay, West	Nutrients
Suffolk	Lake Ronkonkoma	Nutrients
Suffolk	Long Island Sound, Suffolk County, West	Nutrients
Suffolk	Mattituck (Marratooka) Pond	Nutrients
Suffolk	Meetinghouse/Terrys Creeks and tribs	Nutrients
Suffolk	Mill and Seven Ponds	Nutrients
Suffolk	Millers Pond	Nutrients
Suffolk	Moriches Bay, East	Nutrients
Suffolk	Moriches Bay, West	Nutrients
Suffolk	Peconic River, Lower, and tidal tribs	Nutrients
Suffolk	Quantuck Bay	Nutrients
Suffolk	Shinnecock Bay and Inlet	Nutrients
Suffolk	Tidal tribs to West Moriches Bay	Nutrients
Sullivan	Bodine, Montgomery Lakes	Nutrients
Sullivan	Davies Lake	Nutrients
Sullivan	Evens Lake	Nutrients
Sullivan	Pleasure Lake	Nutrients
Tompkins	Cayuga Lake, Southern End	Nutrients
Tompkins	Cayuga Lake, Southern End	Silt/Sediment
Tompkins	Owasco Inlet, Upper, and tribs	Nutrients
Ulster	Ashokan Reservoir	Silt/Sediment
Ulster	Esopus Creek, Upper, and minor tribs	Silt/Sediment
Warren	Hague Brook and tribs	Silt/Sediment

303(d) Segments Impaired by Construction Related Pollutant(s)

Warren	Huddle/Finkle Brooks and tribs	Silt/Sediment
Warren	Indian Brook and tribs	Silt/Sediment
Warren	Lake George	Silt/Sediment
Warren	Tribs to L.George, Village of L George	Silt/Sediment
Washington	Cossayuna Lake	Nutrients
Washington	Lake Champlain, South Bay	Nutrients
Washington	Tribs to L.George, East Shore	Silt/Sediment
Washington	Wood Cr/Champlain Canal and minor tribs	Nutrients
Wayne	Port Bay	Nutrients
Westchester	Amawalk Reservoir	Nutrients
Westchester	Blind Brook, Upper, and tribs	Silt/Sediment
Westchester	Cross River Reservoir	Nutrients
Westchester	Lake Katonah	Nutrients
Westchester	Lake Lincolndale	Nutrients
Westchester	Lake Meahagh	Nutrients
Westchester	Lake Mohegan	Nutrients
Westchester	Lake Shenorock	Nutrients
Westchester	Long Island Sound, Westchester (East)	Nutrients
Westchester	Mamaroneck River, Lower	Silt/Sediment
Westchester	Mamaroneck River, Upper, and minor tribs	Silt/Sediment
Westchester	Muscoot/Upper New Croton Reservoir	Nutrients
Westchester	New Croton Reservoir	Nutrients
Westchester	Peach Lake	Nutrients
Westchester	Reservoir No.1 (Lake Isle)	Nutrients
Westchester	Saw Mill River, Lower, and tribs	Nutrients
Westchester	Saw Mill River, Middle, and tribs	Nutrients
Westchester	Sheldrake River and tribs	Silt/Sediment
Westchester	Sheldrake River and tribs	Nutrients
Westchester	Silver Lake	Nutrients
Westchester	Teatown Lake	Nutrients
Westchester	Titicus Reservoir	Nutrients
Westchester	Truesdale Lake	Nutrients
Westchester	Wallace Pond	Nutrients
Wyoming	Java Lake	Nutrients
Wyoming	Silver Lake	Nutrients

APPENDIX F – List of NYS DEC Regional Offices

<u>Region</u>	<u>COVERING THE FOLLOWING COUNTIES:</u>	<u>DIVISION OF ENVIRONMENTAL PERMITS (DEP) PERMIT ADMINISTRATORS</u>	<u>DIVISION OF WATER (DOW) WATER (SPDES) PROGRAM</u>
1	NASSAU AND SUFFOLK	50 CIRCLE ROAD STONY BROOK, NY 11790 TEL. (631) 444-0365	50 CIRCLE ROAD STONY BROOK, NY 11790-3409 TEL. (631) 444-0405
2	BRONX, KINGS, NEW YORK, QUEENS AND RICHMOND	1 HUNTERS POINT PLAZA, 47-40 21ST ST. LONG ISLAND CITY, NY 11101-5407 TEL. (718) 482-4997	1 HUNTERS POINT PLAZA, 47-40 21ST ST. LONG ISLAND CITY, NY 11101-5407 TEL. (718) 482-4933
3	DUTCHESS, ORANGE, PUTNAM, ROCKLAND, SULLIVAN, ULSTER AND WESTCHESTER	21 SOUTH PUTT CORNERS ROAD NEW PALTZ, NY 12561-1696 TEL. (845) 256-3059	100 HILLSIDE AVENUE, SUITE 1W WHITE PLAINS, NY 10603 TEL. (914) 428 - 2505
4	ALBANY, COLUMBIA, DELAWARE, GREENE, MONTGOMERY, OTSEGO, RENSSELAER, SCHENECTADY AND SCHOHARIE	1150 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL. (518) 357-2069	1130 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL. (518) 357-2045
5	CLINTON, ESSEX, FRANKLIN, FULTON, HAMILTON, SARATOGA, WARREN AND WASHINGTON	1115 STATE ROUTE 86, Po Box 296 RAY BROOK, NY 12977-0296 TEL. (518) 897-1234	232 GOLF COURSE ROAD WARRENSBURG, NY 12885-1172 TEL. (518) 623-1200
6	HERKIMER, JEFFERSON, LEWIS, ONEIDA AND ST. LAWRENCE	STATE OFFICE BUILDING 317 WASHINGTON STREET WATERTOWN, NY 13601-3787 TEL. (315) 785-2245	STATE OFFICE BUILDING 207 GENESEE STREET UTICA, NY 13501-2885 TEL. (315) 793-2554
7	BROOME, CAYUGA, CHENANGO, CORTLAND, MADISON, ONONDAGA, OSWEGO, TIOGA AND TOMPKINS	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7438	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7500
8	CHEMUNG, GENESEE, LIVINGSTON, MONROE, ONTARIO, ORLEANS, SCHUYLER, SENECA, STEUBEN, WAYNE AND YATES	6274 EAST AVON-LIMA ROADAVON, NY 14414-9519 TEL. (585) 226-2466	6274 EAST AVON-LIMA RD. AVON, NY 14414-9519 TEL. (585) 226-2466
9	ALLEGANY, CATTARAUGUS, CHAUTAUQUA, ERIE, NIAGARA AND WYOMING	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7165	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7070

APPENDIX L

NOTICE OF INTENT

NOI for coverage under Stormwater General Permit for Construction Activity

version 1.35

(Submission #: HPP-1PZQ-JV6M5, version 1)

Details

Submitted 11/8/2022 (0 days ago) by Frederick Bohlander

Alternate Identifier Elmwood Preserve

Submission ID HPP-1PZQ-JV6M5

Submission Reason New

Status Submitted

Active Steps Under Review , Under Review

Form Input

Owner/Operator Information

Owner/Operator Name (Company/Private Owner/Municipality/Agency/Institution, etc.)

Ridgewood Elmwood Owner, LLC

Owner/Operator Contact Person Last Name (NOT CONSULTANT)

Brett

Owner/Operator Contact Person First Name

Owings

Owner/Operator Mailing Address

25A Hanover Road, Suite 310

City

Florham Park

State

New Jersey

Zip

07932

Phone

973-593-0003

Email

bowings@ridgewoodrep.com

Federal Tax ID

82-1881284

Project Location**Project/Site Name**

Elmwood Preserve

Street Address (Not P.O. Box)

850 Dobbs Ferry Road

Side of Street

North

City/Town/Village (THAT ISSUES BUILDING PERMIT)

Town of Greenburgh

State

NY

Zip

10607

DEC Region

3

County

WESTCHESTER

Name of Nearest Cross Street

Sprain Brook Parkway

Distance to Nearest Cross Street (Feet)

500

Project In Relation to Cross Street

West

Tax Map Numbers Section-Block-Parcel

7530-320-1.SG & 1.SE

Tax Map Numbers

7530-320-1.SG & 1.SE

1. Coordinates

Provide the Geographic Coordinates for the project site. The two methods are:

- Navigate to the project location on the map (below) and click to place a marker and obtain the XY coordinates.
- The "Find Me" button will provide the lat/long for the person filling out this form. Then pan the map to the correct location and click the map to place a marker and obtain the XY coordinates.

Navigate to your location and click on the map to get the X,Y coordinates

41.0350172,-73.82262209999999

Project Details**2. What is the nature of this project?**

Redevelopment with increase in impervious area

3. Select the predominant land use for both pre and post development conditions.**Pre-Development Existing Landuse**

Recreational/Sports Field

Post-Development Future Land Use

Single Family Subdivision (Please answer 3a)

3a. If Single Family Subdivision was selected in question 3, enter the number of subdivision lots.125

4. In accordance with the larger common plan of development or sale, enter the total project site acreage, the acreage to be disturbed and the future impervious area (acreage)within the disturbed area.

*** ROUND TO THE NEAREST TENTH OF AN ACRE. ***

Total Site Area (acres)

106.8

Total Area to be Disturbed (acres)

88.0

Existing Impervious Area to be Disturbed (acres)

9.95

Future Impervious Area Within Disturbed Area (acres)

20.22

5. Do you plan to disturb more than 5 acres of soil at any one time?

Yes

6. Indicate the percentage (%) of each Hydrologic Soil Group(HSG) at the site.**A (%)**

0

B (%)

67

C (%)

30

D (%)

3

7. Is this a phased project?

No

8. Enter the planned start and end dates of the disturbance activities.**Start Date**

04/03/2023

End Date

10/25/2024

9. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge.

Rum Brook

9a. Type of waterbody identified in question 9?

Stream/Creek Off Site

Other Waterbody Type Off Site Description

NONE PROVIDED

9b. If "wetland" was selected in 9A, how was the wetland identified?

NONE PROVIDED

10. Has the surface waterbody(ies in question 9 been identified as a 303(d) segment in Appendix E of GP-0-20-001?

No

11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001?

No

12. Is the project located in one of the watershed areas associated with AA and AA-S classified waters?

No

If No, skip question 13.

13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as D (provided the map unit name is inclusive of slopes greater than 25%), E or F on the USDA Soil Survey?

NONE PROVIDED

If Yes, what is the acreage to be disturbed?

NONE PROVIDED

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area?

No

15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?

Yes

16. What is the name of the municipality/entity that owns the separate storm sewer system?

Westchester County

17. Does any runoff from the site enter a sewer classified as a Combined Sewer?

No

18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?

No

19. Is this property owned by a state authority, state agency, federal government or local government?

No

20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.)

No

Required SWPPP Components

21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?

Yes

22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)?

Yes

If you answered No in question 22, skip question 23 and the Post-construction Criteria and Post-construction SMP Identification sections.

23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual?

Yes

24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:
Professional Engineer (P.E.)

SWPPP Preparer

JMC, PLLC

Contact Name (Last, Space, First)

Bohlander, Frederick

Mailing Address

120 Bedford Road

City

Armonk

State

New York

Zip

10504

Phone

914-273-5225

Email

rbohlander@jmcpllc.com

Download SWPPP Preparer Certification Form

Please take the following steps to prepare and upload your preparer certification form:

1) Click on the link below to download a blank certification form

- 2) The certified SWPPP preparer should sign this form
- 3) Scan the signed form
- 4) Upload the scanned document

[Download SWPPP Preparer Certification Form](#)

Please upload the SWPPP Preparer Certification

APPENDIX L-SWPPP Preparer Certification Form (GP-0-20-001).pdf - 11/08/2022

02:34 PM

Comment

NONE PROVIDED

Erosion & Sediment Control Criteria

25. Has a construction sequence schedule for the planned management practices been prepared?

Yes

26. Select all of the erosion and sediment control practices that will be employed on the project site:

Temporary Structural

Dust Control
Sediment Basin
Sediment Traps
Silt Fence
Stabilized Construction Entrance
Storm Drain Inlet Protection
Temporary Swale

Biotechnical

None

Vegetative Measures

Mulching
Protecting Vegetation
Seeding
Sodding
Temporary Swale
Topsoiling

Permanent Structural

Land Grading
Retaining Wall
Rock Outlet Protection

Other

NONE PROVIDED

Post-Construction Criteria

*** IMPORTANT: Completion of Questions 27-39 is not required if response to Question 22 is No.**

27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.

Preservation of Undisturbed Area
Preservation of Buffers
Reduction of Clearing and Grading
Roadway Reduction

27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).

All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).

28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout). (Acre-feet)

2.74

29. Post-construction SMP Identification

Use the Post-construction SMP Identification section to identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity that were used to reduce the Total WQv Required (#28).

Identify the SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use the Post-Construction SMP Identification section to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29. (acre-feet)

2.26

31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28)?

No

If Yes, go to question 36. If No, go to question 32.

32. Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P) (0.95) (Ai) / 12, Ai=(s) (Aic)] (acre-feet)

0.88

32a. Is the Total RRv provided (#30) greater than or equal to the Minimum RRv Required (#32)?

Yes

If Yes, go to question 33.

Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the SWPPP.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

33. SMPs

Use the Post-construction SMP Identification section to identify the Standard SMPs and, if applicable, the Alternative SMPs to be used to treat the remaining total WQv (=Total WQv Required in #28 - Total RRv Provided in #30).

Also, provide the total impervious area that contributes runoff to each practice selected.

NOTE: Use the Post-construction SMP Identification section to identify the SMPs used on Redevelopment projects.

33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question #29. (acre-feet)

0.67

Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - provided by the practice. (See Table 3.5 in Design Manual)

34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a).

2.93

35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)?

Yes

If Yes, go to question 36.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

36. Provide the total Channel Protection Storage Volume (CPv required and provided or select waiver (#36a), if applicable.**CPv Required (acre-feet)**

2.74

CPv Provided (acre-feet)

2.93

36a. The need to provide channel protection has been waived because:

NONE PROVIDED

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (#37a), if applicable.**Overbank Flood Control Criteria (Qp)****Pre-Development (CFS)**

108.53

Post-Development (CFS)

71.03

Total Extreme Flood Control Criteria (Qf)**Pre-Development (CFS)**

351.76

Post-Development (CFS)

243.41

37a. The need to meet the Qp and Qf criteria has been waived because:

NONE PROVIDED

38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed?

Yes

If Yes, Identify the entity responsible for the long term Operation and Maintenance

NONE PROVIDED

39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). (See question #32a) This space can also be used for other pertinent project information.

NONE PROVIDED

Post-Construction SMP Identification**Runoff Reduction (RR) Techniques, Standard Stormwater Management Practices (SMPs) and Alternative SMPs**

Identify the Post-construction SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

RR Techniques (Area Reduction)

Round to the nearest tenth

Total Contributing Acres for Conservation of Natural Area (RR-1)

0

Total Contributing Impervious Acres for Conservation of Natural Area (RR-1)

0

Total Contributing Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)

0

Total Contributing Impervious Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)

0

Total Contributing Acres for Tree Planting/Tree Pit (RR-3)

0

Total Contributing Impervious Acres for Tree Planting/Tree Pit (RR-3)

0

Total Contributing Acres for Disconnection of Rooftop Runoff (RR-4)

0

RR Techniques (Volume Reduction)

Total Contributing Impervious Acres for Disconnection of Rooftop Runoff (RR-4)

0

Total Contributing Impervious Acres for Vegetated Swale (RR-5)

0

Total Contributing Impervious Acres for Rain Garden (RR-6)

0

Total Contributing Impervious Acres for Stormwater Planter (RR-7)

0

Total Contributing Impervious Acres for Rain Barrel/Cistern (RR-8)

0

Total Contributing Impervious Acres for Porous Pavement (RR-9)

0

Total Contributing Impervious Acres for Green Roof (RR-10)

0

Standard SMPs with RRv Capacity

Total Contributing Impervious Acres for Infiltration Trench (I-1)

0

Total Contributing Impervious Acres for Infiltration Basin (I-2)

14.74

Total Contributing Impervious Acres for Dry Well (I-3)

0

Total Contributing Impervious Acres for Underground Infiltration System (I-4)

0

Total Contributing Impervious Acres for Bioretention (F-5)

0

Total Contributing Impervious Acres for Dry Swale (O-1)

0

Standard SMPs

Total Contributing Impervious Acres for Micropool Extended Detention (P-1)

0

Total Contributing Impervious Acres for Wet Pond (P-2)

0

Total Contributing Impervious Acres for Wet Extended Detention (P-3)

2.72

Total Contributing Impervious Acres for Multiple Pond System (P-4)

0

Total Contributing Impervious Acres for Pocket Pond (P-5)

0

Total Contributing Impervious Acres for Surface Sand Filter (F-1)

0

Total Contributing Impervious Acres for Underground Sand Filter (F-2)

0

Total Contributing Impervious Acres for Perimeter Sand Filter (F-3)

0

Total Contributing Impervious Acres for Organic Filter (F-4)

0

Total Contributing Impervious Acres for Shallow Wetland (W-1)

0

Total Contributing Impervious Acres for Extended Detention Wetland (W-2)

0

Total Contributing Impervious Acres for Pond/Wetland System (W-3)

0

Total Contributing Impervious Acres for Pocket Wetland (W-4)

0

Total Contributing Impervious Acres for Wet Swale (O-2)

0

Alternative SMPs (DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY)

Total Contributing Impervious Area for Hydrodynamic

4.92

Total Contributing Impervious Area for Wet Vault

0

Total Contributing Impervious Area for Media Filter

0

"Other" Alternative SMP?

0

Total Contributing Impervious Area for "Other"

0

Provide the name and manufacturer of the alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment.

Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.

Manufacturer of Alternative SMP

Contech

Name of Alternative SMP

Cascade unit 5, 6, 8, 10

Other Permits

40. Identify other DEC permits, existing and new, that are required for this project/facility.

Individual SPDES

If SPDES Multi-Sector GP, then give permit ID

NONE PROVIDED

If Other, then identify

NONE PROVIDED

41. Does this project require a US Army Corps of Engineers Wetland Permit?

No

If "Yes," then indicate Size of Impact, in acres, to the nearest tenth

NONE PROVIDED

42. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned.

NONE PROVIDED

MS4 SWPPP Acceptance

43. Is this project subject to the requirements of a regulated, traditional land use control MS4?

Yes - Please attach the MS4 Acceptance form below

If No, skip question 44**44. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?**

No

MS4 SWPPP Acceptance Form Download

Download form from the link below. Complete, sign, and upload.

[MS4 SWPPP Acceptance Form](#)

MS4 Acceptance Form Upload

MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance Form.pdf -

11/08/2022 05:39 PM

Comment

NONE PROVIDED

Owner/Operator Certification

Owner/Operator Certification Form Download

Download the certification form by clicking the link below. Complete, sign, scan, and upload the form.

[Owner/Operator Certification Form \(PDF, 45KB\)](#)

Upload Owner/Operator Certification Form

SWPPP Preparer Certification Form.pdf - 11/08/2022 05:42 PM

Comment

NONE PROVIDED

Attachments

Date	Attachment Name	Context	User
11/8/2022 5:42 PM	SWPPP Preparer Certification Form.pdf	Attachment	Frederick Bohlander
11/8/2022 5:39 PM	MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance Form.pdf	Attachment	Frederick Bohlander
11/8/2022 2:34 PM	APPENDIX L-SWPPP Preparer Certification Form (GP-0-20-001).pdf	Attachment	Frederick Bohlander

Status History

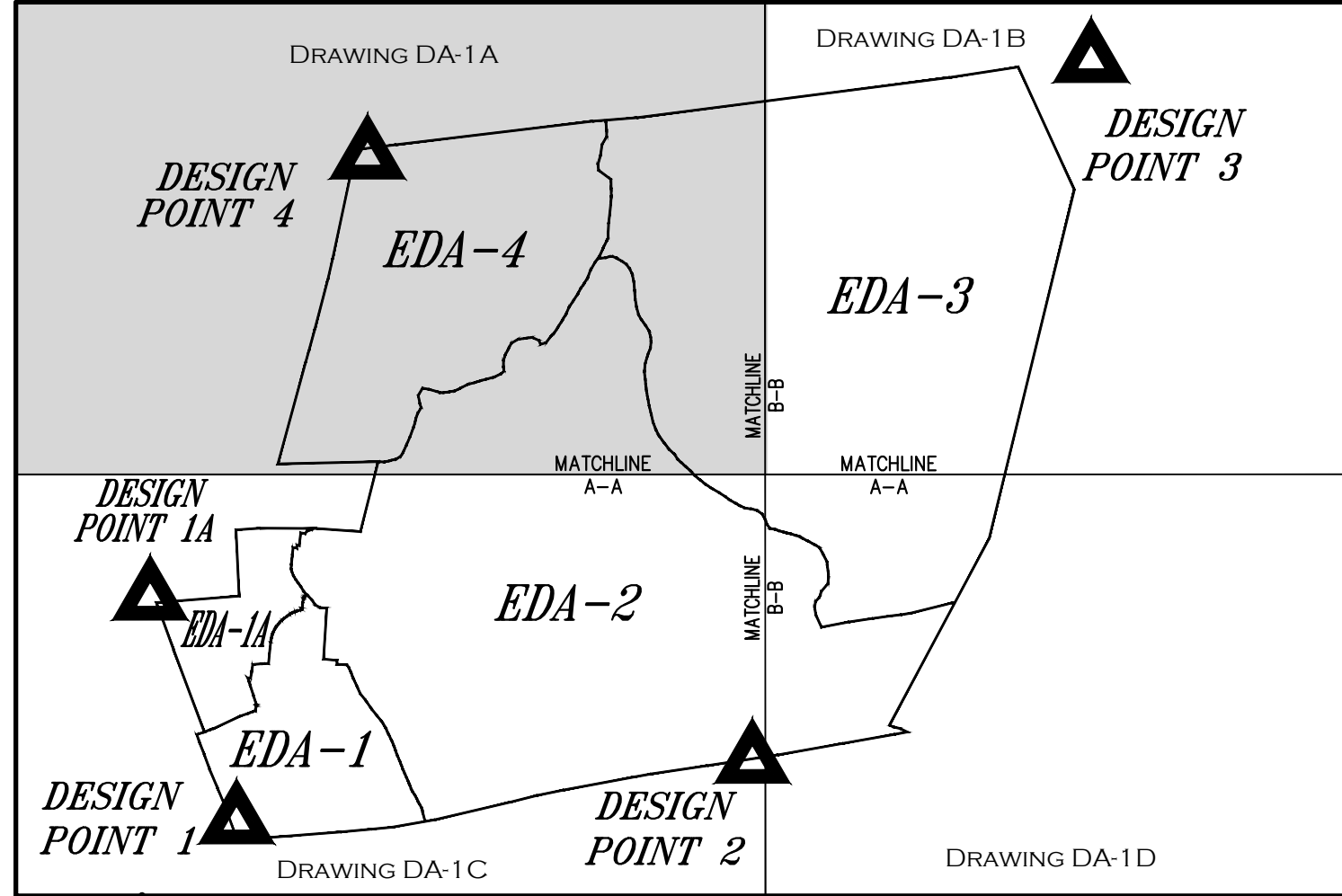
	User	Processing Status
11/8/2022 1:34:57 PM	Frederick Bohlander	Draft
11/8/2022 5:42:44 PM	Frederick Bohlander	Submitting
11/8/2022 5:42:47 PM	Frederick Bohlander	Submitted

Processing Steps

Step Name	Assigned To/Completed By	Date Completed
Form Submitted	Frederick Bohlander	11/8/2022 5:42:47 PM
Under Review	DAVID GASPER	
Under Review	Daniel von Schilgen	

APPENDIX M

DRAWINGS



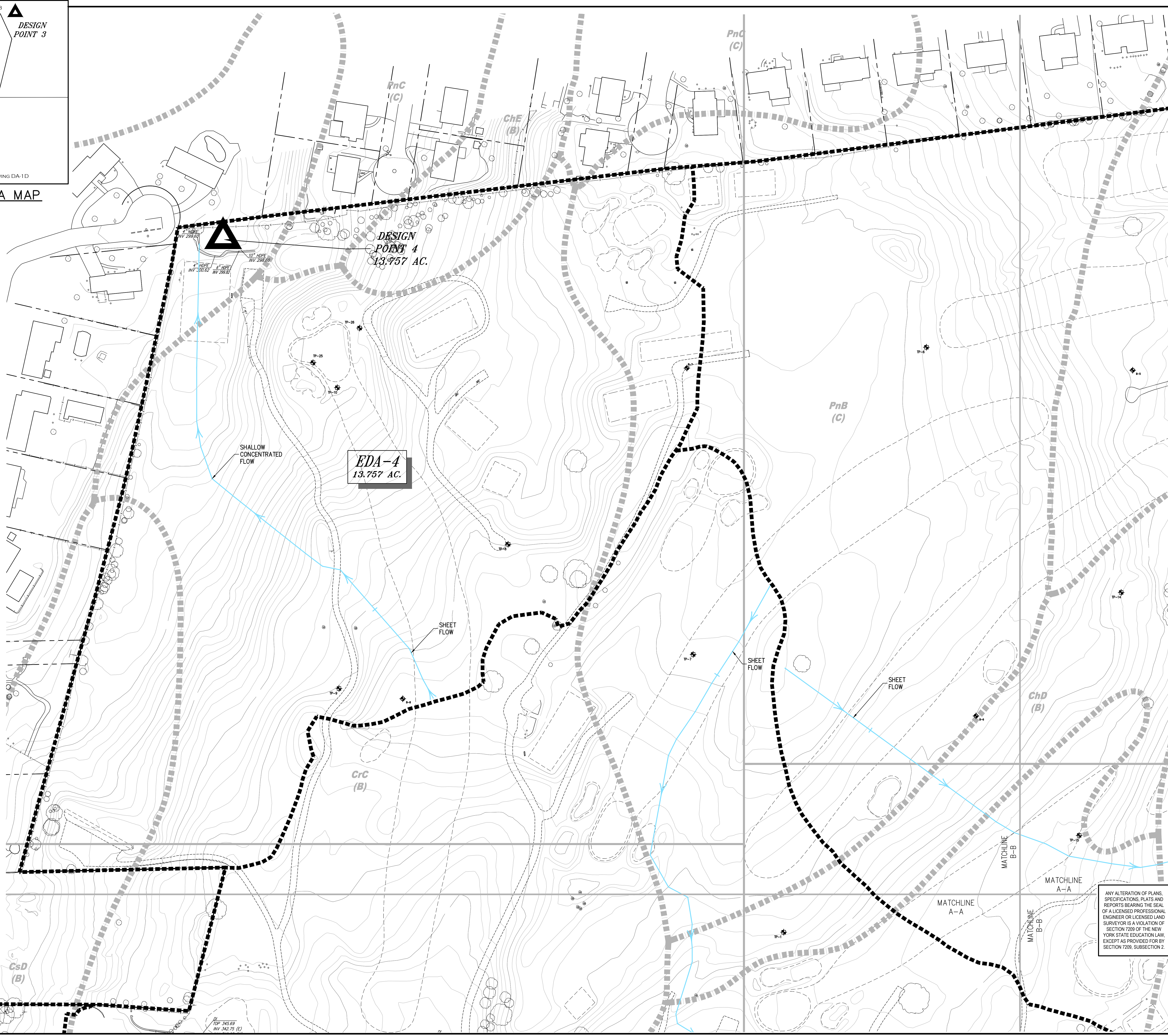
OVERALL DRAINAGE AREA MAP

LEGEND

- EXISTING PROPERTY LINE
- ADJACENT PROPERTY LINE
- EXISTING EASEMENT LINE
- EXISTING BUILDING LINE
- EXISTING PAVEMENT EDGE
- EXISTING CURB LINE
- EXISTING CONTOUR
- EXISTING INDEX CONTOUR
- EXISTING WATER COURSE
- EXISTING WETLANDS
- 100' WETLAND / WATER COURSE BUFFER
- EXISTING STONE WALL
- EXISTING RETAINING WALL
- EXISTING GUIDE RAIL
- EXISTING FENCE
- EXISTING TREE AND DESIGNATION
- EXISTING TREE LINE
- EXISTING STORM DRAIN LINE AND SIZE
- EXISTING SANITARY LINE AND SIZE
- EXISTING DRAIN INLET
- EXISTING MANHOLE
- EXISTING FIRE HYDRANT
- EXISTING UTILITY POLE
- EXISTING LIGHT POLE
- EXISTING SIGN
- BORING LOCATION AND DESIGNATION
- TEST PIT LOCATION AND DESIGNATION
- WATERSHED BOUNDARY LINE
- LIMIT OF SOIL GROUPS LINE
- FLOW PATH LINE
- SOIL DESIGNATION AND HYDROLOGIC SOIL GROUP

- NOTES:**
- EXISTING CONDITIONS DEPICTED ON THIS PLAN HAVE BEEN TAKEN FROM SURVEY TITLED, "TOPOGRAPHIC SURVEY," PREPARED BY JMC, PLLC, DATED DECEMBER 2, 2018.
 - WETLANDS WERE EVALUATED IN A REPORT TITLED "POND AND WATERCOURSE FUNCTIONAL EVALUATION REPORT" DATED DECEMBER 10, 2019.
 - GEOTECHNICAL BORING/TEST PIT LOCATIONS DEPICTED ON THIS PLAN WERE TAKEN FROM THE GEOTECHNICAL REPORT ENTITLED, "BORING & TEST PIT LOCATION PLAN", DATED 08/14/2018, PREPARED BY CARLIN-SIMPSON AND ASSOCIATES.

SOIL TYPE TABLE		
DESIGNATION	HYDROLOGIC GROUP	DESCRIPTION
ChC	B	CHARLTON FINE SANDY LOAM, 8 TO 15 PERCENT SLOPES
ChB	B	CHARLTON FINE SANDY LOAM, 3 TO 8 PERCENT SLOPES
UIC	N/A	URBAN LAND-CHARLTON-CHATTFIELD COMPLEX, ROLLING, VERY ROCKY
PnC	C	PAXTON FINE SANDY LOAM, 8 TO 15 PERCENT SLOPES
ChE	B	CHARLTON LOAM, 25 TO 35 PERCENT SLOPES
PnB	C	PAXTON FINE SANDY LOAM, 3 TO 8 PERCENT SLOPES
WdC	C/D	WOODBIDGE LOAM, 8 TO 15 PERCENT SLOPES
SuB	B	SUTTON LOAM, 3 TO 8 PERCENT SLOPES
Ub	B	UDORTHENTS, SMOOTHED
LeB	A/D	LEICESTER LOAM, 2 TO 8 PERCENT SLOPES, VERY STONY
W	N/A	WATER
Uc	A/D	UDORTHENTS, WET SUBSTRATUM
Uf	N/A	URBAN LAND
CcC	B	CHARLTON-CHATTFIELD COMPLEX, 0 TO 15 PERCENT SLOPES, VERY ROCKY
CdD	B	CHATTFIELD-CHARLTON COMPLEX, 15 TO 35 PERCENT SLOPES, VERY ROCKY



EXISTING DRAINAGE AREA MAP

ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

RIDGEWOOD ELMWOOD OWNER, LLC.
25A HANOVER ROAD, SUITE 310
FLORHAM PARK, NJ 07932

JMC
JMC Planning, Engineering, Landscape Architecture & Land Surveying, PLLC
JMC Site Development Consultants, LLC
John Meyer Consulting, Inc.
120 BEDFORD ROAD • ARMONK, NY 10504
voice 914.273.5225 • fax 914.273.2102
www.jmcpllc.com

DA-1A

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ANY ALTERATION OF PLANS, SPECIFICATIONS, PLATS AND REPORTS BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LICENSED LAND SURVEYOR IS A VIOLATION OF SECTION 7209 OF THE NEW YORK STATE EDUCATION LAW, EXCEPT AS PROVIDED FOR BY SECTION 7209, SUBSECTION 2.

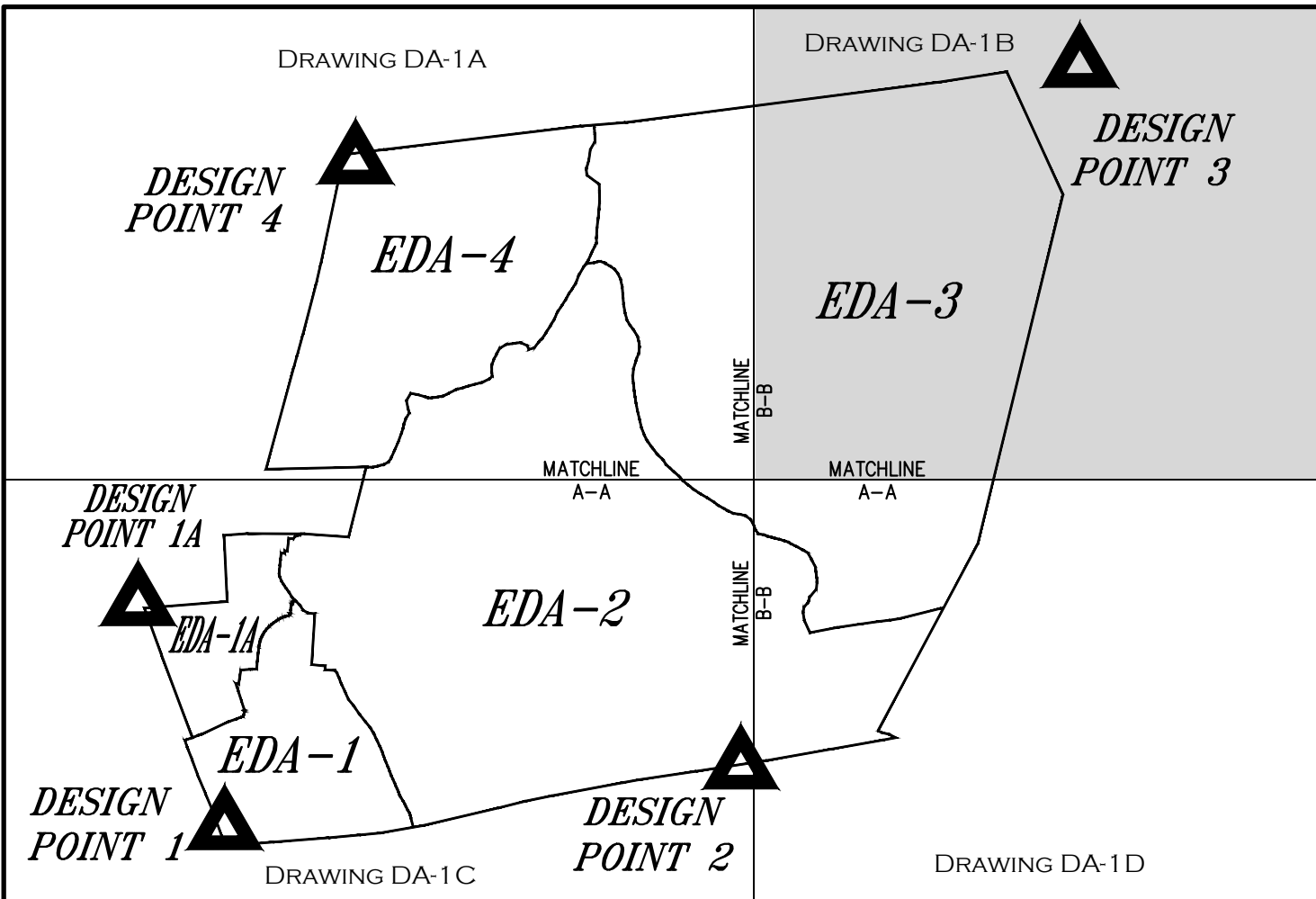
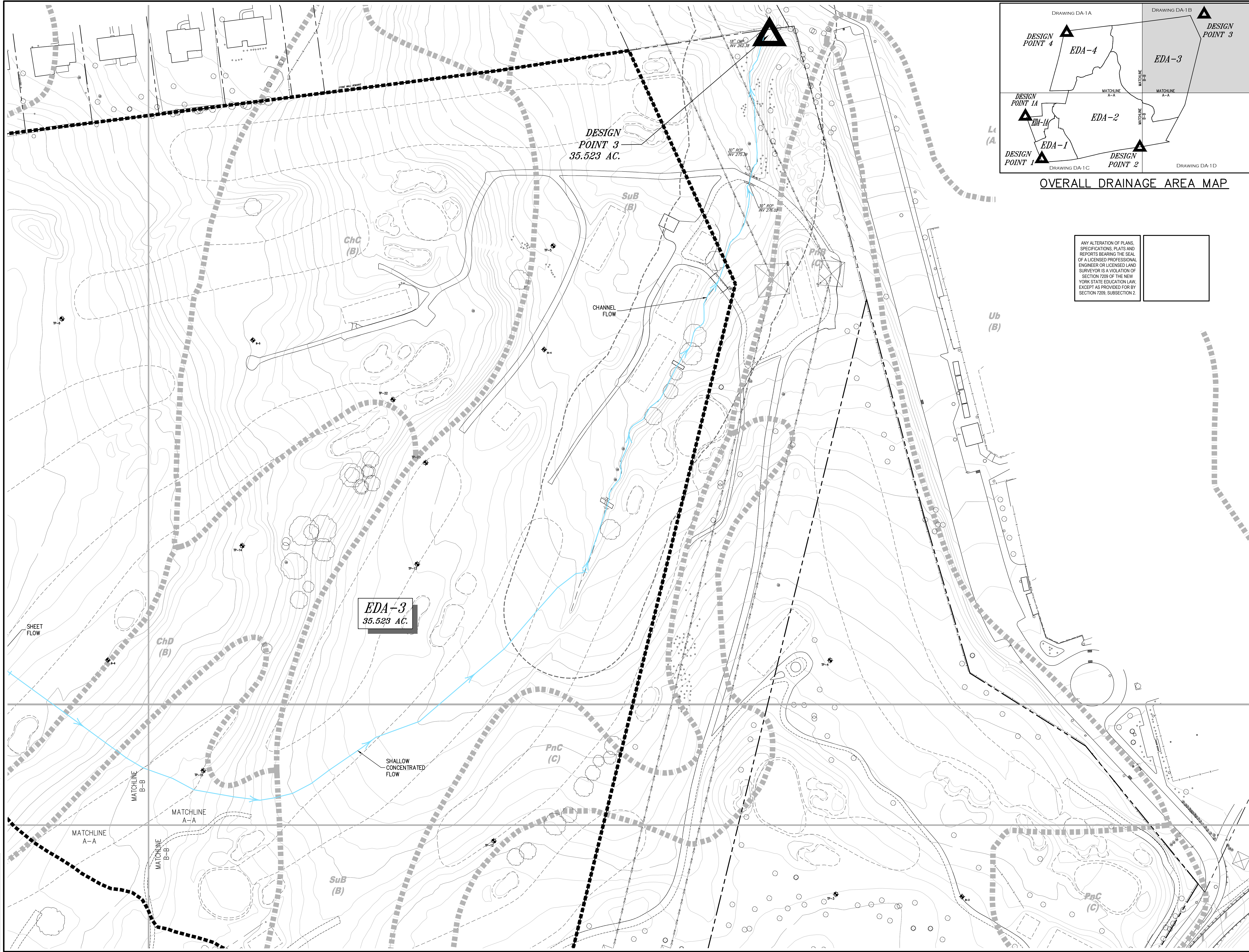
Revision

No.	Revision	Date	By
1.	DRAINAGE REVISIONS	08/13/2020	RB
2.	DRAINAGE REVISIONS	06/24/2021	RB
3.	DRAINAGE REVISIONS	10/15/2021	RB
4.	LAYOUT UPDATES	10/26/2022	RB
5.	DRAINAGE REVISIONS	05/02/2023	RB
6.	DRAINAGE REVISIONS	06/09/2023	RB
7.	DRAINAGE REVISIONS	08/07/2023	RB

Drawn: EOK **Approved:** DV
Scale: 1" = 50'
Date: 04/02/2020
Project No: 170999
1709-16A-WA EDA-1 EDA-1

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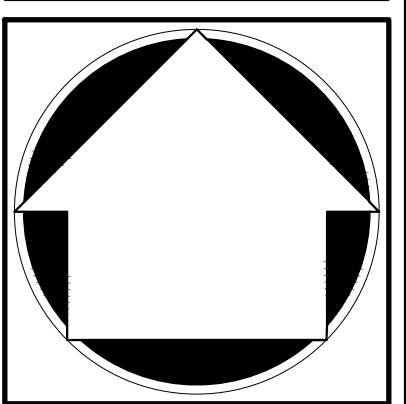
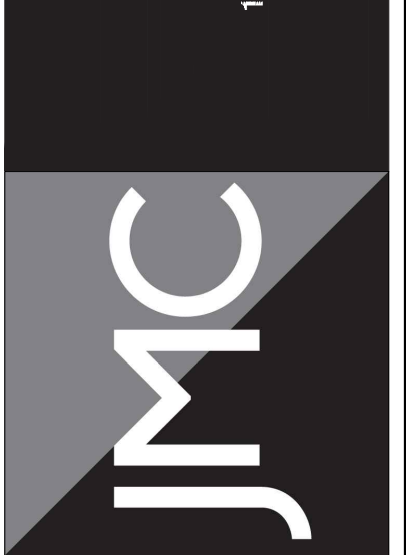
OVERALL DRAINAGE AREA MAP

ANY ALTERATION OF PLANS, SPECIFICATIONS, PLATS AND REPORTS BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LICENSED LAND SURVEYOR IS A VIOLATION OF SECTION 7209 OF THE NEW YORK STATE EDUCATION LAW, EXCEPT AS PROVIDED FOR BY SECTION 7209, SUBSECTION 2.

No.	Revision	Date	By
1.	DRAINAGE REVISIONS	06/13/2020	RB
2.	DRAINAGE REVISIONS	06/24/2021	RB
3.	DRAINAGE REVISIONS	10/15/2021	RB
4.	LAYOUT UPDATES	10/26/2022	RB
5.	DRAINAGE REVISIONS	05/02/2023	RB
6.	DRAINAGE REVISIONS	06/09/2023	RB
7.	DRAINAGE REVISIONS	06/01/2023	RB

OWNER: RIDGEWOOD ELMWOOD OWNER, LLC.
25A HANOVER ROAD, SUITE 310
FLORHAM PARK, NJ 07932

JMC Planning, Engineering, Landscape Architecture & Land Surveying, PLLC
JMC Site Development Consultants, LLC
John Meyer Consulting, Inc.
120 BEDFORD ROAD • ARMONK, NY 10504
voice 914.273.5225 • fax 914.273.2192
www.jmcpllc.com

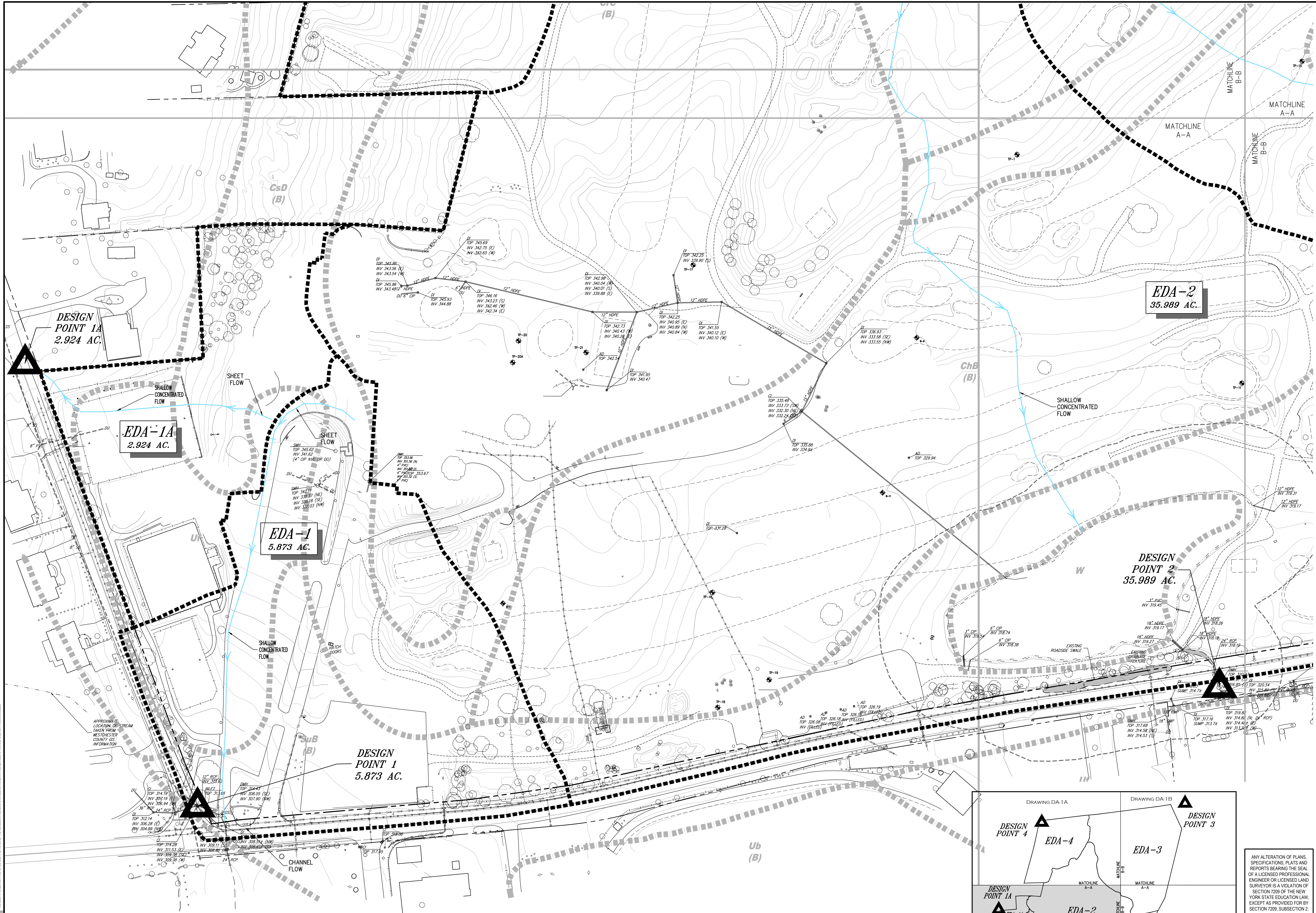


EXISTING DRAINAGE AREA MAP
ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

Drawn: EGK	Approved: DV
Scale: 1" = 50'	
Date: 04/02/2020	
Project No: 17099	
1709-DRAINAGE	EDA-2
EDA-1	EDA-3
EDA-4	

DA-1B

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6.	DRAINAGE REVISIONS	06/18/2023	RB
7.	DRAINAGE REVISIONS	08/01/2023	RB

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EXISTING DRAINAGE AREA MAP
ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

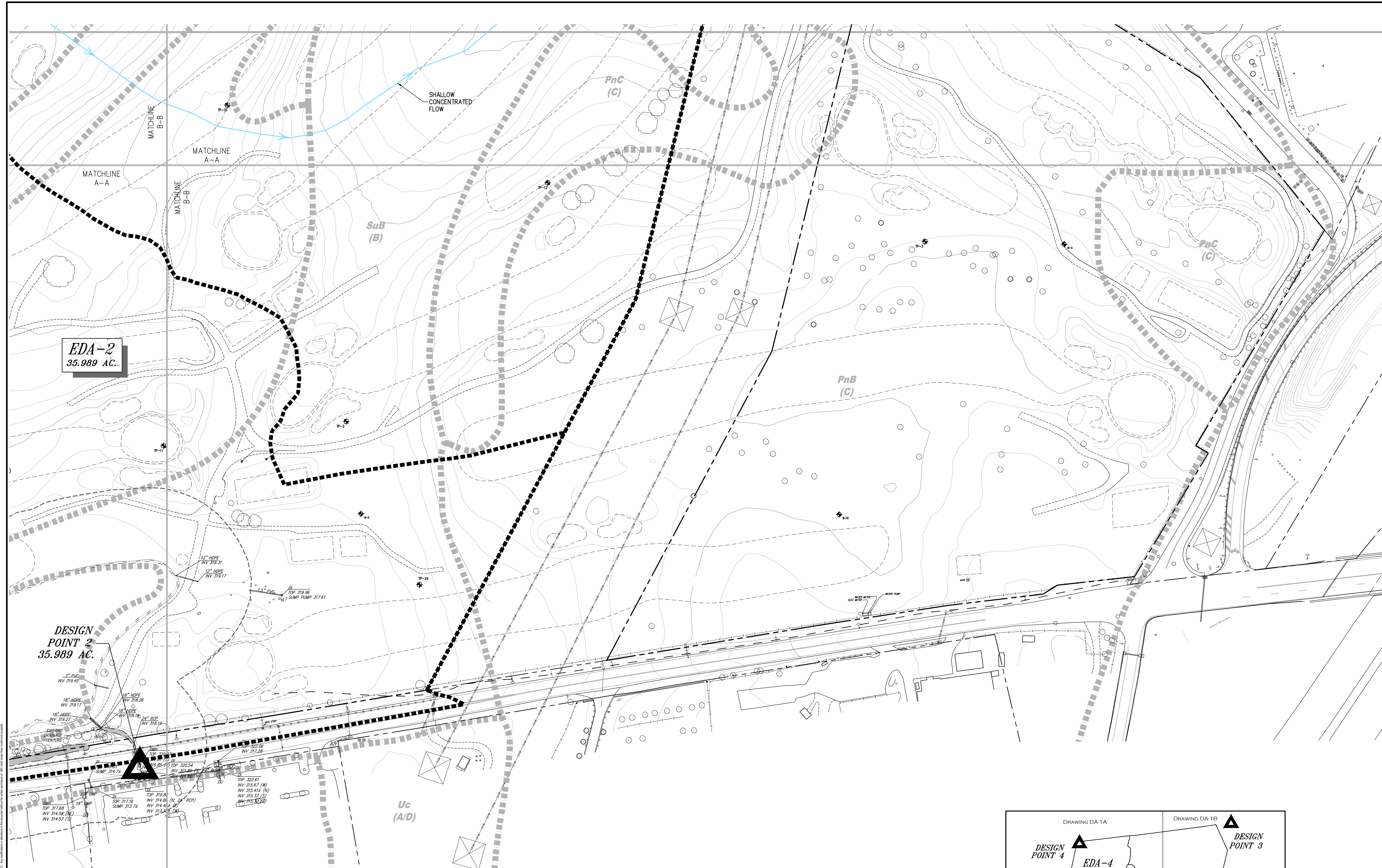
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Drawn	EOK	Approved	DV
Scale:	1" = 50'		
Date:	04/02/2020		
Project No:	170999		
TOWN-DRAWN	EDA-3	EDA-3	EDA-3

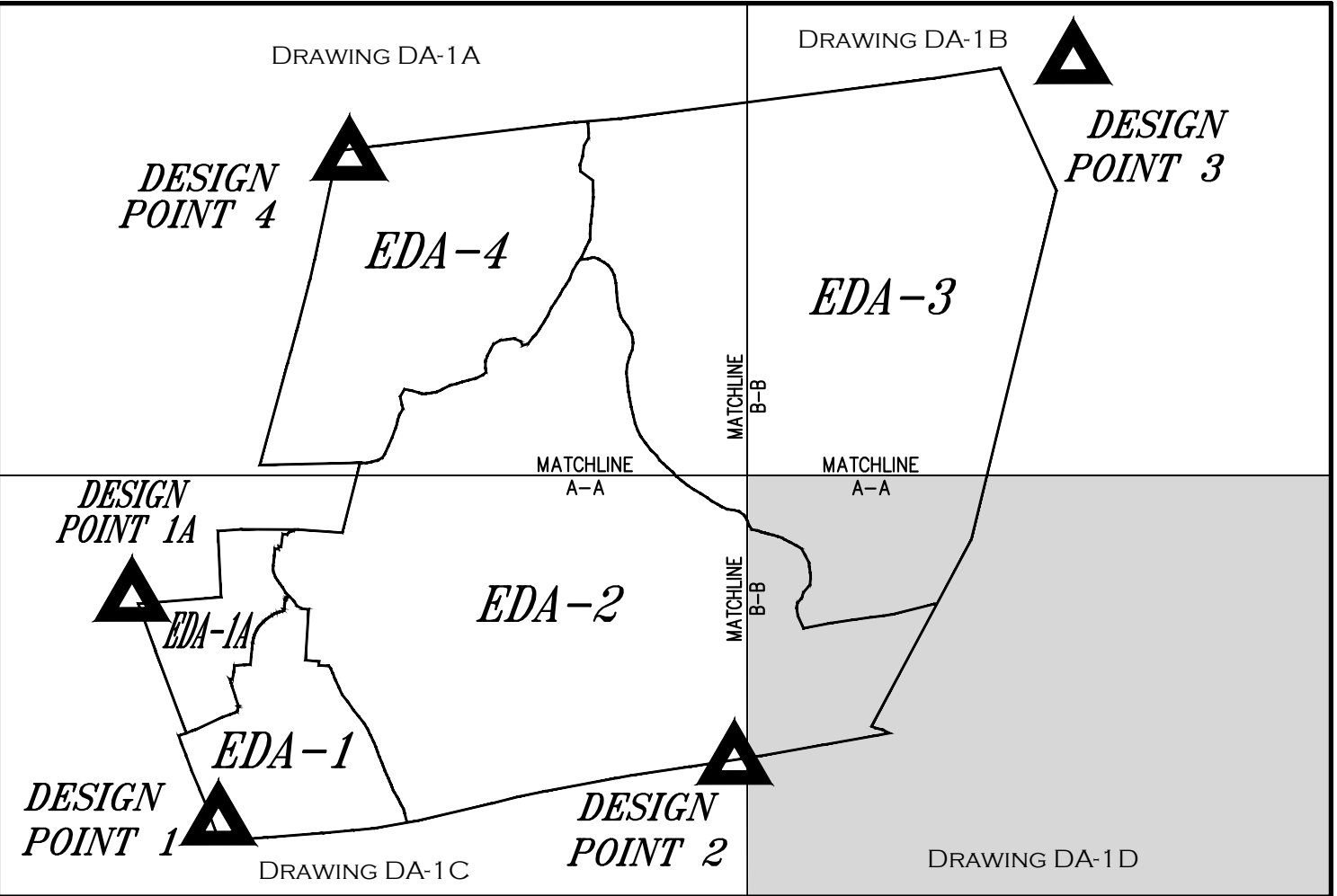
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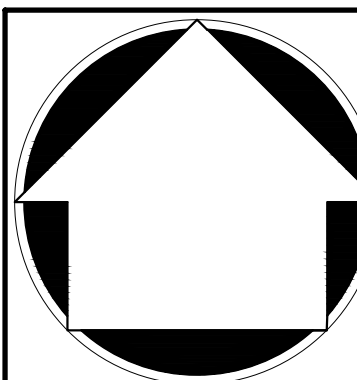
OVERALL
DRAINAGE
AREA MAP



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Project No:	17099		
1709-DRAINAGE	EDA-4	EDA-1	

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EXISTING DRAINAGE
AREA MAP
ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK



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LEGEND

EXISTING PROPERTY LINE

ADJACENT PROPERTY LINE

EXISTING EASEMENT LINE

EXISTING BUILDING LINE

EXISTING PAVEMENT EDGE

EXISTING CURB LINE

EXISTING CONTOUR

EXISTING INDEX CONTOUR

EXISTING WATER COURSE

EXISTING WETLANDS

100' WETLAND / WATER COURSE BUFFER

EXISTING STONE WALL

EXISTING RETAINING WALL

EXISTING GUIDE RAIL

EXISTING FENCE

EXISTING TREE AND DESIGNATION

EXISTING TREE LINE

EXISTING STORM DRAIN LINE AND SIZE

EXISTING SANITARY LINE AND SIZE

EXISTING DRAIN INLET

EXISTING MANHOLE

EXISTING FIRE HYDRANT

EXISTING UTILITY POLE

EXISTING LIGHT POLE

EXISTING SIGN

PROPOSED FINISHED GRADE

PROPOSED SPOT GRADE

PROPOSED SANITARY SEWER MANHOLE

PROPOSED STORM DRAIN MANHOLE

PROPOSED TYPE C DRAIN INLET

PROPOSED TYPE D DRAIN INLET

PROPOSED TRENCH DRAIN

PROPOSED STORMWATER BASIN

PROPOSED CLEAFOUT

PROPOSED HYDRANT

PROPOSED STORM DRAIN LINE & SIZE

PROPOSED RETAINING WALL (DESIGN BY OTHERS)

BORING LOCATION AND DESIGNATION

TEST PIT LOCATION AND DESIGNATION

WATERSHED BOUNDARY LINE

LIMIT OF SOIL GROUPS LINE

FLOW PATH LINE

SOIL DESIGNATION AND HYDROLOGIC SOIL GROUP

191.00'

● MH

● DI

● CI

● DP-2

● TP-14

15' HDPE

PhC (C)

191.00'

● MH

● DI

● CI

● DP-2

● TP-14

15' HDPE

PhC (C)

NOTES

1. EXISTING CONDITIONS DEPICTED ON THIS PLAN HAVE BEEN TAKEN FROM SURVEY TITLED, "TOPOGRAPHIC SURVEY," PREPARED BY JMC, PLLC, DATED DECEMBER 2, 2019.

2. WETLANDS WERE EVALUATED IN A REPORT TITLED "POND AND WATERCOURSE FUNCTIONAL EVALUATION REPORT" DATED DECEMBER 10, 2019.

3. GEOTECHNICAL BORING/TEST PIT LOCATIONS DEPICTED ON THIS PLAN WERE TAKEN FROM THE GEOTECHNICAL REPORT ENTITLED, "BORING & TEST PIT LOCATION PLAN," DATED 08/14/2018, PREPARED BY CARLIN-SIMPSON AND ASSOCIATES.

SOIL TYPE TABLE		
DESIGNATION	HYDROLOGIC GROUP	DESCRIPTION
ChC	B	CHARLTON FINE SANDY LOAM, 8 TO 15 PERCENT SLOPES
ChB	B	CHARLTON FINE SANDY LOAM, 3 TO 8 PERCENT SLOPES
UIC	N/A	URBAN LAND-CHARLTON-CHATELAIN COMPLEX, ROLLING, VERY ROCKY
PhC	C	PAXTON FINE SANDY LOAM, 8 TO 15 PERCENT SLOPES
ChE	B	CHARLTON LOAM, 25 TO 35 PERCENT SLOPES
PhB	C	PAXTON FINE SANDY LOAM, 3 TO 8 PERCENT SLOPES
Woc	C/D	WOODBRIDGE LOAM, 8 TO 15 PERCENT SLOPES
SuB	B	SUTTON LOAM, 3 TO 8 PERCENT SLOPES
Ub	B	UDORTHENTS, SMOOTHED
LeB	A/D	LEICESTER LOAM, 2 TO 8 PERCENT SLOPES, VERY STONY
W	N/A	WATER
Uc	A/D	UDORTHENTS, WET SUBSTRATUM
Uf	N/A	URBAN LAND
CrC	B	CHARLTON-CHATELAIN COMPLEX, 0 TO 15 PERCENT SLOPES, VERY ROCKY
Cad	B	CHATELAIN-CHARLTON COMPLEX, 15 TO 35 PERCENT SLOPES, VERY ROCKY

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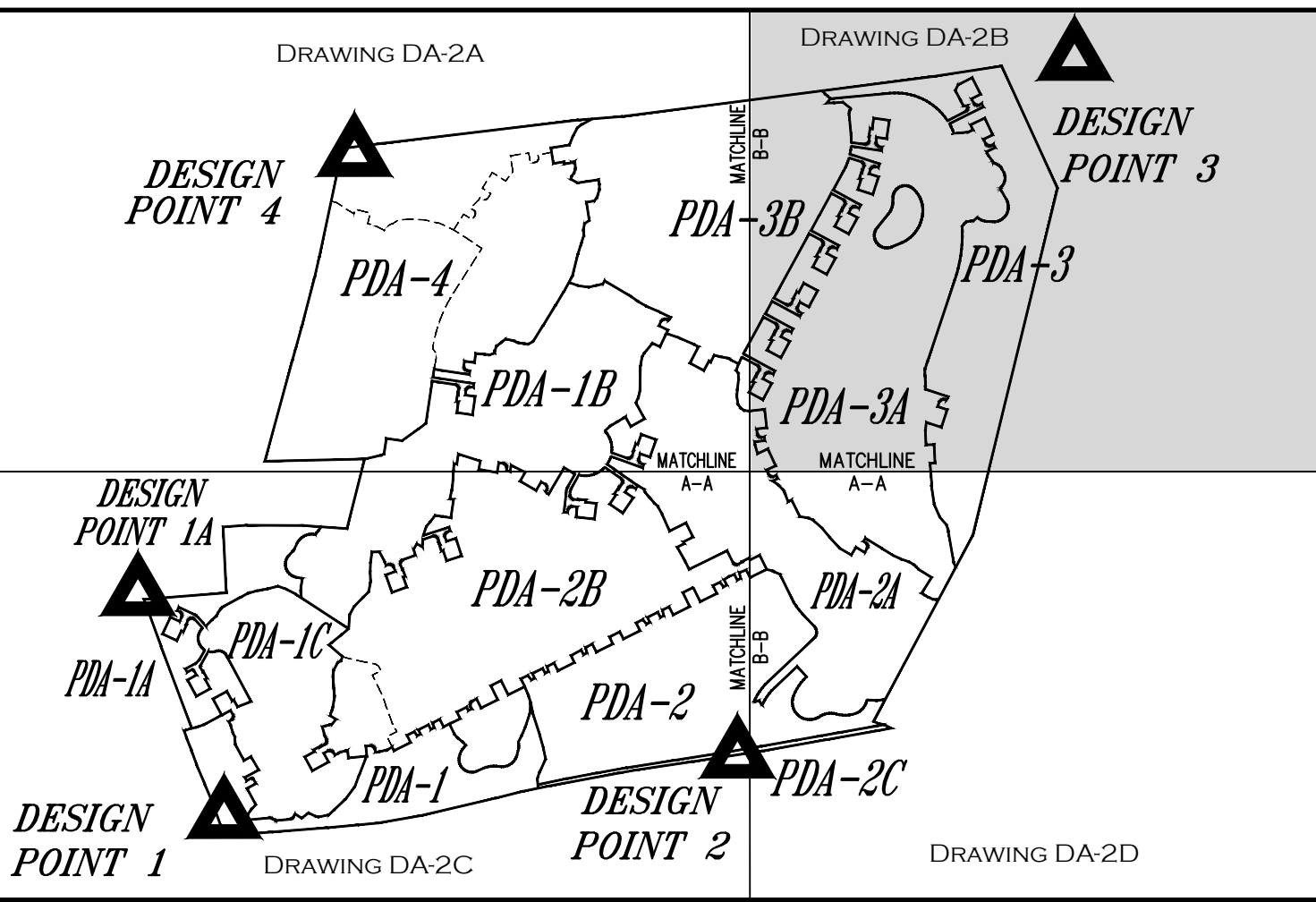
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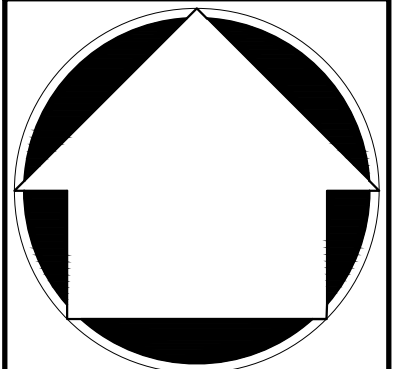
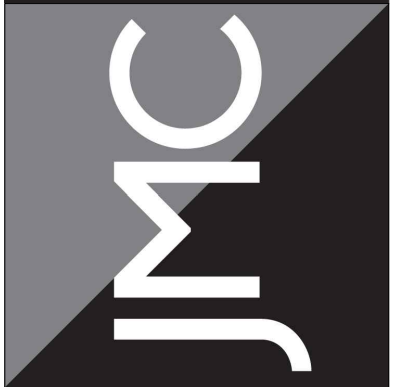


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No.	Revision	Date	By
1.	DRAINAGE REVISIONS	08/13/2020	RB
2.	DRAINAGE REVISIONS	06/24/2021	RB
3.	DRAINAGE REVISIONS	10/15/2021	RB
4.	LAYOUT UPDATES	11/11/2022	RB
5.	DRAINAGE REVISIONS	05/02/2023	RB
6.	DRAINAGE REVISIONS	06/09/2023	RB
7.	DRAINAGE REVISIONS	08/01/2023	RB

OWNER: RIDGEWOOD ELMWOOD OWNER, LLC.
25A HANOVER ROAD, SUITE 310
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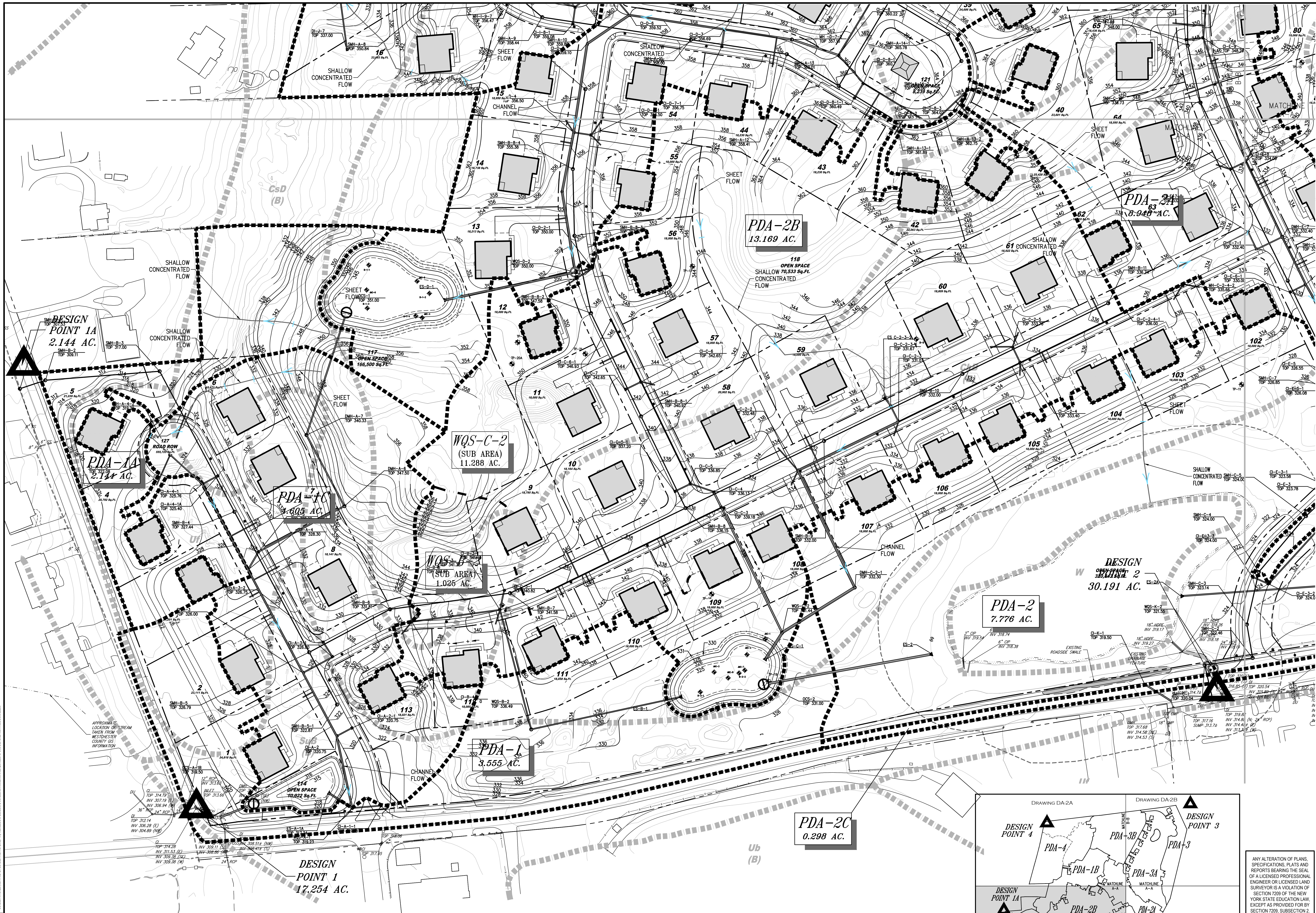


PROPOSED DRAINAGE
AREA MAP

ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

Drawn: EKG	Approved: DV
Scale: 1" = 50'	
Date: 04/02/2020	
Project No: 17099	
17099-DRAINAGE	PDA-2
PDA-1s	
DA-2B	

NOT FOR CONSTRUCTION



No.	Revision	Date	By
1.	DRAMA REVISIONS	06/13/2020	RB
2.	DRAMA REVISIONS	06/24/2021	RB
3.	DRAMA REVISIONS	10/15/2021	RB
4.	LAYOUT UPDATES	11/17/2022	RB
5.	DRAMA REVISIONS	05/02/2023	RB
6.	DRAMA REVISIONS	06/09/2023	RB
7.	DRAMA REVISIONS	08/07/2023	RB

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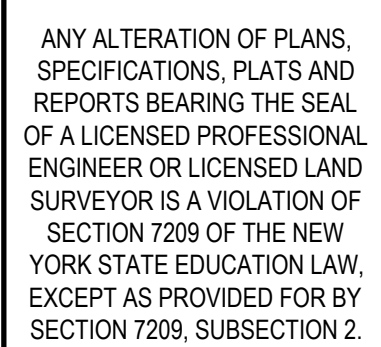
PROPOSED DRAINAGE AREA MAP

ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

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Date: 04/02/2020
Project No: 17099
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DA-2C



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Date:	04/02/2020		
Project No:	17099		
17099-DRAINAGE	PDA-4		PDA

NOT FOR CONSTRUCTION

PROPOSED DRAINAGE
AREA MAP

ELMWOOD PRESERVE
850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK

EROSION AND SEDIMENT CONTROL STORMWATER POLLUTION PREVENTION

ELMWOOD PRESERVE – PARK PARCEL

**850 DOBBS FERRY ROAD
TOWN OF GREENBURGH, NEW YORK**

*Applicant/Operator/
Owner:* **Ridgewood Elmwood Owner, LLC**
(973) 593-0003
bowings@ridgewoodrep.com

Prepared by:



JMC Project 17099

Dated: 10/31/2025
Revised: **05/13/2026**

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APPENDICES

APPENDIX DESCRIPTION

- A. Soil Testing Data
- B. Temporary Erosion and Sediment Control Inspection and Maintenance Checklist
- C. Owner / Contractor's Certification

REFERENCED DRAWINGS FOR SWPPP DESIGN AND DETAILS

JMC SITE PLANS

<u>Dwg. No.</u>	<u>Title</u>	<u>Rev. No. / Date</u>
C-014	“Existing Conditions Map (Park Parcel)”	02 05/13/2026
C-210	“Final Grading Plan (Park Parcel)”	02 05/13/2026
C-410	“Remediation Erosion and Sediment Control Plan (Park Parcel)”	02 05/13/2026
C-920	“Construction Details (Park Parcel)”	02 05/13/2026

I. INTRODUCTION

This Stormwater Pollution Prevention Plan for Erosion & Sediment Control has been prepared for the 14.209 Park Parcel of the Elmwood Preserve Site, located in the Town of Greenburgh, Westchester County, New York (hereinafter referred to as the "Site"). The site is bordered by residences to the north, Dobbs Ferry Road to the south, the Sprain Brook Parkway and Rum Brook Park to the east, and the Elmwood Preserve Development to the west. The development has been designed in accordance with the following:

- Requirements of the New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-25-001, effective January 29, 2025.
- Chapter 248 "Stormwater Management" of the Town of Greenburgh Zoning Code.
- New York State Standards and Specifications for Erosion and Sediment Control (Blue Book), effective November 2016.

This report addresses the Erosion and Sediment Control requirements for the proposed remediation occurring on the site. The project site is currently developed with paths and lawn areas associated with the former Elmwood Country Club golf course. The proposed action will include a proposed curb cut and small portion of a driveway apron into the site, removal of contaminated soils and the existing paths. The improvements require an approximate disturbance area of $\pm 618,925$ square feet and will result in a decrease in impervious areas on site, and as such, no stormwater management practices are proposed. Accordingly, since the project falls under Table I within Appendix B of the General Permit, an NOI will be filed for Erosion and Sediment Control only.

II. EXISTING CONDITIONS

The existing conditions of the project site consist of existing drive / pathways, lawn, landscaped areas, and sand dunes that were associated with the Former Elmwood Country Club Golf Course. The site is currently unoccupied. The site generally slopes to the north with slopes towards the West and the East. The site does not contain existing stormwater management practices. After stormwater runoff exits the project site, it flows towards Rum Brook.

III. PROPOSED CONDITIONS

The proposed action will include removal of the contaminated soils at the project site. Impacted soils on site will be tested and characterized for reuse and/or disposal. Soils characterized for reuse will be used as fill onsite; soils characterized for disposal will be removed from the site in accordance with all state and local requirements. Clean fill (topsoil) may be added for landscaping purposes. In addition to the removal of the potentially contaminated soils, the existing paths associated with the former golf course will be removed and a new curb cut, and portion of a driveway apron will be constructed off of Dobbs Ferry Road to provide future access to the site. The proposed improvements require $\pm 618,925$ square feet of disturbance to the site with a net decrease in impervious surface. Existing drainage patterns, except for flattening of slopes, are not being changed and will be maintained and protected during construction. As such no stormwater management practices are proposed for this project.

IV. SOIL EROSION & SEDIMENT CONTROL

A potential impact of the proposed development on any soils or slopes will be that of erosion and transport of sediment during construction. An Erosion and Sediment Control Management Program will be established for the proposed development, beginning at the start of construction and continuing throughout its course, as outlined in the "New York State Standards and Specifications for Erosion and Sediment Control," November 2016. A continuing maintenance program will be implemented for the control of sediment transport and erosion control after construction and throughout the useful life of the project.

The Operator shall have a qualified professional assess the site prior to the commencement of construction and certify that the appropriate erosion and sediment controls, as shown on the Sediment & Erosion Control Plans, have been adequately installed to ensure overall preparedness of the site for the commencement of construction. In addition, the Operator shall have a qualified professional conduct one site inspection at least every seven calendar days.

Prior to the commencement of construction activity, the owner or operator must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The owner or operator shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the trained contractor. The owner or operator shall ensure that at least one trained contractor is on site on a daily basis when soil disturbance activities are being performed. The owner or operator shall have each of the contractors and subcontractors identified above sign a copy of the certification statement provided in Appendix 'C' before they commence any construction activity.

Soil Description

As provided by the United States Department of Agriculture, Soil Conservation Service "Web Soil Survey," soil classifications which exist on the subject site are described below.

Soils are placed into four hydrologic groups: A, B, C, and D. In the definitions of the classes, infiltration rate is the rate at which water enters the soil at the surface and is controlled by the surface conditions. Transmission rate is the rate at which water moves in the soil and is controlled by soil properties. Definitions of the classes are as follows:

- A. (Low runoff potential). The soils have a high infiltration rate even when thoroughly wetted. They chiefly consist of deep, well drained to excessively drained sands or gravels. They have a high rate of water transmission.
- B. The soils have a moderate infiltration rate when thoroughly wetted. They chiefly are moderately deep to deep, moderately well drained to well drained soils that have moderately fine to moderately coarse textures. They have a moderate rate of water transmission.

- C. The soils have a slow infiltration rate when thoroughly wetted. They chiefly have a layer that impedes downward movement of water or have moderately fine to fine texture. They have a slow rate of water transmission.
- D. (High runoff potential). The soils have a very slow infiltration rate when thoroughly wetted. They chiefly consist of clay soils that have a high swelling potential, soils that have a permanent high water table, soils that have a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. They have a very slow rate of water transmission.

A soil's tendency to erode is also described in the USDA web soil survey. The ratings in this interpretation indicate the hazard of soil loss from unsurfaced areas. The ratings are based on soil erosion factor K, slope, and content of rock fragments. The hazard is described as "slight," "moderate," or "SEVERE." A rating of "slight" indicates that little or no erosion is likely; "moderate" indicates that some erosion is likely, that the temporarily unsurfaced / un-stabilized during construction may require occasional maintenance, and that simple erosion-control measures are needed; and "SEVERE" indicates that significant erosion is expected, that the roads or trails require frequent maintenance, and that erosion-control measures are needed.

Per the Soil Survey, the following soils listed below are present at the site. Following this list is a detailed description of each soil type found on the property:

<u>Symbol</u>	<u>Hydrologic Soil Group</u>	<u>Description</u>
PnB	C	Paxton Fine Sandy Loam, 3-8% slopes
PnC	C	Paxton Fine Sandy Loam, 8-15% slopes
Ub	B	Udorthents, smoothed

PnB, Paxton Fine Sandy Loam, 3 TO 8 PERCENT SLOPE

This soil is comprised of Paxton, Woodbridge, Ridgebury, and Charlton soils. The parent material (Paxton) consists of coarse-loamy lodgment till derived from gneiss, granite, and/or schist. This soil is defined as being well drained.

Hydrologic group: **C**
Erosion Hazard Rating: Slight

PnC, Paxton Fine Sandy Loam, 8 TO 15 PERCENT SLOPE

This soil is comprised of Paxton, Charlton Woodbridge, and Ridgebury. The parent material (Paxton) consists of coarse-loamy lodgment till derived from gneiss, granite, and/or schist. This soil is defined as being well drained.

Hydrologic group: **C**
Erosion Hazard Rating: Moderate

Ub, Udorthents, Smoothed

This soil is comprised of Udorthents, Urban Land, Leicester, Hollis, Charlton, Riverhead, and Sun. The parent material consists mainly of Udorthents which consists of soil materials in alternating layers ranging from sand to silt loam. This soil is defined as being moderately well drained.

Hydrologic group: **B**

On-Site Pollution Prevention

There are temporary pollution prevention measures used to control litter and construction debris on site, such as:

- Silt Fence
- Stabilized Construction Access
- Sediment Traps
- Erosion Control Matting

Temporary Control Measures

Temporary control measures and facilities will include silt fences, stabilized construction access, sediment traps, stockpiles, temporary seeding and mulching.

Throughout the construction of the proposed development, temporary control facilities will be implemented to control on-site erosion and sediment transfer. Construction ditches, if required, will be used to direct stormwater runoff to temporary sediment traps for settlement. The sediment traps constructed as part of this project will serve to remove sediment and pollutants from the stormwater runoff produced during construction.

Descriptions of the temporary sediment & erosion controls that will be used during the development of the site including silt fence, stabilized construction access, seeding, mulching and temporary sediment traps are as follows:

1. Silt Fence is constructed using a geotextile fabric. The fence will be either 18 inches or 30 inches high. The height of the fence can be increased in the event of placing these devices on uncompacted fills or extremely loose undisturbed soils. The fences will not be placed in areas which receive concentrated flows such as ditches, swales and channels nor will the filter fabric material be placed across the entrance to pipes, culverts, spillway structures, sediment traps or basins.
2. Haybales may be used along existing paved areas to intercept sediment from stormwater runoff generated by upstream soil disturbance. Haybales may also be placed around temporary soil stockpile areas on pavement.
3. Stabilized Construction Access consists of AASHTO No. 1 rock. The rock entrance will be a minimum of 50 feet in length by 24 feet in width by 8 inches in depth.

4. Seeding will be used to create a vegetative surface to stabilize disturbed earth until at least 80% of the disturbed area has a perennial vegetative cover. This amount is required to adequately function as a sediment and erosion control facility. Grass lining will also be used to line temporary channels and the surrounding disturbed areas.
5. Mulching is used as an anchor for seeding and disturbed areas to reduce soil loss due to storm events. These areas will be mulched with straw at a rate of 3 tons per acre such that the mulch forms a continuous blanket. Mulch must be placed after seeding or within 48 hours after seeding is completed.
6. Erosion Control Matting will be utilized on slopes and within swales, where applicable, to provide stabilization in advance of vegetation being established. Such matting will be biodegradable to facilitate long term growth of vegetation in swales, on slopes and within stormwater management facilities.
7. Sediment Traps will be constructed to intercept sediment laden runoff and trap and retain the sediment. The sediment traps area sized to provide a sediment storage volume of 3,600 cubic feet per acre draining to the basin. The traps will utilize an overflow spillway set at an elevation above the required sediment storage volume. Once stabilized the sediment traps will be removed and final grading/planting will be completed.

The contractor shall be responsible for maintaining the temporary sediment and erosion control measures throughout construction. This maintenance will include, but not be limited to, the following tasks:

1. For dust control purposes, moisten all exposed graded areas with water at least twice a day in those areas where soil is exposed and cannot be planted with a temporary cover due to construction operations or the season (December through March).
2. Inspection of erosion and sediment control measures shall be performed at the end of each construction day and immediately following each rainfall event. All required repairs shall be immediately executed by the contractor.
3. Sediment deposits shall be removed when they reach approximately $\frac{1}{3}$ the height of the silt fence. All such sediment shall be properly disposed of in fill areas on the site, as directed by the Owner's Field Representative. Fill shall be protected following disposal with mulch, temporary and/or permanent vegetation and be completely circumscribed on the downhill side by silt fence.
4. Rake all exposed areas parallel to the slope during earthwork operations.
5. Following final grading, the disturbed area shall be stabilized with a permanent surface treatment (i.e. turf grass, pavement or sidewalk). During rough grading, areas which are not to be disturbed for seven or more days shall be stabilized with the temporary seed mixture, as defined on the plans. Seed all piles of dirt in exposed soil areas that will not receive a permanent surface treatment.

Construction Site Chemical Control

The purpose of this management measure is to prevent the generation of nonpoint source pollution from construction sites due to improper handling and usage of nutrients and toxic substances, and to prevent the movement of toxic substances from the construction site.

Many potential pollutants other than sediment are associated with construction activities. These pollutants include pesticides; fertilizers used for vegetative stabilization; petrochemicals; construction chemicals such as concrete products, sealers, and paints; wash water associated with these products; paper; wood; garbage; and sanitary waste.

Disposal of excess pesticides and pesticide-related wastes should conform to registered label directions for the disposal and storage of pesticides and pesticides containers set forth in applicable Federal, State and local regulations that govern their usage, handling, storage, and disposal.

Pesticides should be disposed of through either a licensed waste management firm or a treatment, storage and disposal (TSD) facility. Containers should be triple rinsed before disposal, and rinse waters should be reused as product. Other practices include setting aside a locked storage area, tightly closing lids, storing in a cool, dry place, checking containers periodically for leaks or deterioration, maintaining a list of products in storage, using plastic sheeting to line the storage areas, and notifying neighboring property owners prior to spraying.

When storing petroleum products, follow these guidelines:

- Create a shelter around the area with cover and wind protection;
- Line the storage area with a double layer of plastic sheeting or similar material;
- Create an impervious berm around the perimeter with a capacity of 110 percent greater than that of the largest container;
- Clearly label all products;
- Keep tanks off the ground; and
- Keep lids securely fastened.

Post spill procedure information and have persons trained in spill handling on site or on call at all times. Materials for cleaning up spills should be kept on site and easily available. Spills should be cleaned up immediately and the contaminated material properly disposed of. Maintain and wash equipment and machinery in confined areas specifically designed to control runoff.

Thinners or solvents should not be discharged into sanitary or storm systems when cleaning machinery. Use alternative methods for cleaning larger equipment parts, such as high-pressure, high-temperature water washes, or steam cleaning. Equipment-washing detergents can be used, and wash water may be discharged into sanitary sewers if solids are removed from the solution first. (This practice should be verified with the local sewer authority.) Small parts can be cleaned with degreasing solvents, which can then be reused or recycled.

Solid Waste Management and Portable Sanitary Management

The purpose of this management measure is to prevent the potential for solid waste such as construction debris, trash, etc. from construction sites due to improper handling and storage. Debris and litter should be removed periodically from the BMP's and surrounding areas to prevent clogging of pipes and structures. All construction material shall be stored in designated staging areas. Roll-off containers shall be placed on site and all empty containers, construction debris and litter shall be placed in the containers.

Portable sanitary units may be utilized on-site or bathrooms will be provided within construction trailers. A sanitation removal company will be hired to pump/remove any sanitary waste. In the event that portable sanitary units are used and then cleaned after being emptied, the rinse water may not be disposed of to the storm drain system. It shall be contained for later disposal if it can't be disposed of on-site. Remove paper and trash before cleaning the portable sanitary units. The portable sanitary units shall be located away from the storm drain system if possible. Provide over head cover for wash areas if possible. Maintain spill response material and equipment on site to eliminate the potential for contaminants and wash water from entering the storm drain system.

Specifications for Soil Restoration

Prior to the final stabilization of the disturbed areas, soil restoration will be required for all vegetated areas to recover the original properties and porosity of the soil. Soil Restoration Requirements are provided on Table I below:

Table I

Soil Restoration Requirements

Type of Soil Disturbance	Soil Restoration Requirement		Comments/Examples
No soil disturbance	Restoration not permitted		Preservation of Natural Features
Minimal soil disturbance	Restoration not required		Clearing and grubbing
Areas where topsoil is stripped only – no change in grade	HSG A&B	HSG C&D	Protect area from any ongoing construction activities
	apply 6 inches of topsoil	Aerate* and apply 6 inches of topsoil	
Areas of cut or fill	HSG A&B	HSG C&D	Clearing and grubbing
	Aerate and apply 6 inches of topsoil	Apply full Soil Restoration**	
Heavy traffic areas on site (especially) in a zone 5-25 feet around buildings but not	Apply full Soil Restoration (decompaction and compost enhancement)		

within a 5 foot perimeter around foundation walls)		
Areas where Runoff Reduction and/or Infiltration practices are applied	Restoration not required, but may be applied to enhance the reduction specified for appropriate practices.	Keep construction equipment from crossing these areas. To protect newly installed practice from any ongoing construction activities construct a single phase operation fence area.
Redevelopment projects	Soil Restoration is required on redevelopment projects in areas where existing impervious area will be converted to pervious area.	

* Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

** Per "Deep Ripping and De-compaction, DEC 2008."

During periods of relatively low to moderate subsoil moisture, the disturbed subsoils are returned to rough grade and the following full soil restoration steps applied:

1. Apply 3 inches of compost over subsoil.
2. Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils.
3. Rock-pick until uplifted stone/rock materials of four inches and larger size are cleaned off the site.

Specifications for Final Stabilization of Graded Areas

Final stabilization of graded areas consists of the placement of topsoil and installation of landscaping (unless the area is to be paved, or a building is to be constructed in the location). Topsoil is to be spread as soon as grading operations are completed. Topsoil is to be placed to a minimum depth of six inches on all embankments, planting areas and seeding/sod areas. The subgrade is to be scarified to a depth of two inches to provide a bond of the topsoil with the subsoil. Topsoil is to be raked to an even surface and cleared of all debris, roots, stones and other unsatisfactory material.

Planting operations shall be conducted under favorable weather conditions as follows:

- Permanent Lawns - April 15 (provided soil is frost-free and not excessively moist) to May 15; August 15 to October 15.
- Temporary Lawn Seeding - if outside of the time periods noted above, the areas shall be

seeded immediately on completion of topsoil operations with annual ryegrass (Italian rye) at a rate of six pounds per 1,000 square feet. Temporary lawn installation is permitted provided the soil is frost-free and not excessively moist. The permanent lawn is to be installed the next planting season.

On slopes with a grade of 3 horizontal to 1 vertical or greater, and in swales, a geotextile netting or mat shall be installed for stabilization purposes as shown on the Plans. Seeded areas are to be mulched with straw or hay at an application rate of 70-90 pounds per 1,000 s.f. Straw or hay mulch must be spread uniformly and anchored immediately after spreading to prevent wind blowing. Mulches must be inspected periodically and in particular after rainstorms to check for erosion. If erosion is observed, additional mulch must be applied. Netting shall be inspected after rainstorms for dislocation or failure; any damage shall be repaired immediately.

All denuded surfaces which will be exposed for a period of over two months or more shall be temporarily hydroseeded with (a) perennial ryegrass at a rate of 40 lbs per acre (1.0 lb per 1000 square feet); (b) Certified "Aroostook" winter rye (cereal rye) @ 100 lb per acre (2.5 lb/1000 s.f.) to be used in the months of October and November.

Permanent turfgrass cover is to consist of a seed mixture as follows:

(a) Sunny sites

Kentucky Bluegrass	2.0-2.6 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet
Fine Fescue	0.4-0.6 pounds/1000 square feet

(b) Shady sites

Kentucky Bluegrass	0.8-1.0 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet
Fine Fescue	2.6-3.3 pounds/1000 square feet

All plant materials shall comply with the standards of the American Association of Nurserymen with respect to height and caliper as described in its publication American Standard for Nursery Stock, latest edition.

V. CONSTRUCTION PHASE AND POST-CONSTRUCTION MAINTENANCE

During the construction phase and following construction of the project, a number of maintenance measures will be taken with respect to the site maintenance. Measures to be taken included the following:

I. During Construction

A comprehensive sediment and erosion control plan will be in place during the construction period. Maintenance measures for sediment and erosion controls will include:

A qualified professional acceptable to the municipality will be hired by the owner or operator to monitor the installation and maintenance of the sediment and erosion control plans. The qualified professional shall report directly to the Engineering Consultant and shall be responsible for ensuring compliance with the design of the sediment and erosion control plans.

The qualified professional so hired will inspect all sediment and erosion control measures at least every seven calendar days. In the event that there has been a variance with the design of the sediment and erosion control measures so that the ability of the measures to adequately perform the intended function is lessened or compromised and/or the facilities are not adequately maintained, the qualified professional shall be required to report such variance to the Engineering Consultant within 48 hours and shall be empowered to order immediate repairs to the sediment and erosion control measures.

The qualified professional will also be responsible for observing the adequacy of the vegetation growth (trees, shrubs, groundcovers and turfgrasses) in newly graded areas and for ordering additional plantings in the event that the established plant materials do not adequately protect the ground surface from erosion.

2. Following Construction

Site maintenance activities on the property will include:

- Grounds maintenance, including mowing of lawns;
- Planting of trees, shrubs and groundcovers; pruning of trees and shrubs;
- Application of fertilizer and herbicides;
- Maintenance of stormwater management area;

Grounds maintenance on the site will be performed by landscaping contractor.

Fertilizer is typically applied twice in the year - once in the spring and once in the fall. The application of fertilizer is usually necessary to maintain healthy lawn growth due to competition for nutrients with trees and shrubs and since the clippings are often removed. It is not recommended that fertilizer be applied during the summer. It is at this time that lawns are typically dormant.

Fertilizers come in three basic types: (1) Organic; (2) Soluble synthetic and (3) Slow release.

Organic fertilizers are derived from plant or animal waste. Since they are heavier and bulkier than other fertilizers, it is necessary to apply a much greater amount at one time. Soluble synthetic fertilizers are predictable with determining the exact impact on a lawn. However more applications are necessary since their effect is often short term. Slow release fertilizers have a high percentage of nitrogen so quantities that need be handled at one time are smaller. Slow release fertilizers will be utilized by the project.

Per Chapter 863, Article XXVIII of Westchester County Code of Ordinances, no fertilizer containing more than 0% phosphorus shall be used, unless testing confirms the need for additional phosphorus.

Fertilizer shall be applied by the landscape contractor in accordance with the manufacturer's instructions. The application of fertilizer does require some skill on the part of the operator. Should there be a spill of fertilizer, the landscape contractor shall be required to scrape or vacuum it up. The area will then be watered in accordance with the manufacturer's instructions to ensure that the fertilizer becomes soluble and available to plants and does not run off.

VI. CONCLUSION

This Stormwater Pollution Prevention Plan has been prepared to describe the project's sediment and erosion control measures to be utilized during construction. The proposed interim improvements to be utilized during construction have been designed in accordance with the requirements of the:

- New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-25-001, effective January 29, 2025.
- Chapter 248 "Stormwater Management" of the Town of Greenburgh Zoning Code.
- New York State Standards and Specifications for Erosion and Sediment Control (Blue Book), effective November 2016

Based on the foregoing, it is our professional opinion that the proposed temporary erosion and sediment control measures will provide the mitigation described above and are not anticipated to have any adverse impacts to the site or any surrounding areas.

APPENDIX A
WEB SOIL SURVEY DATA



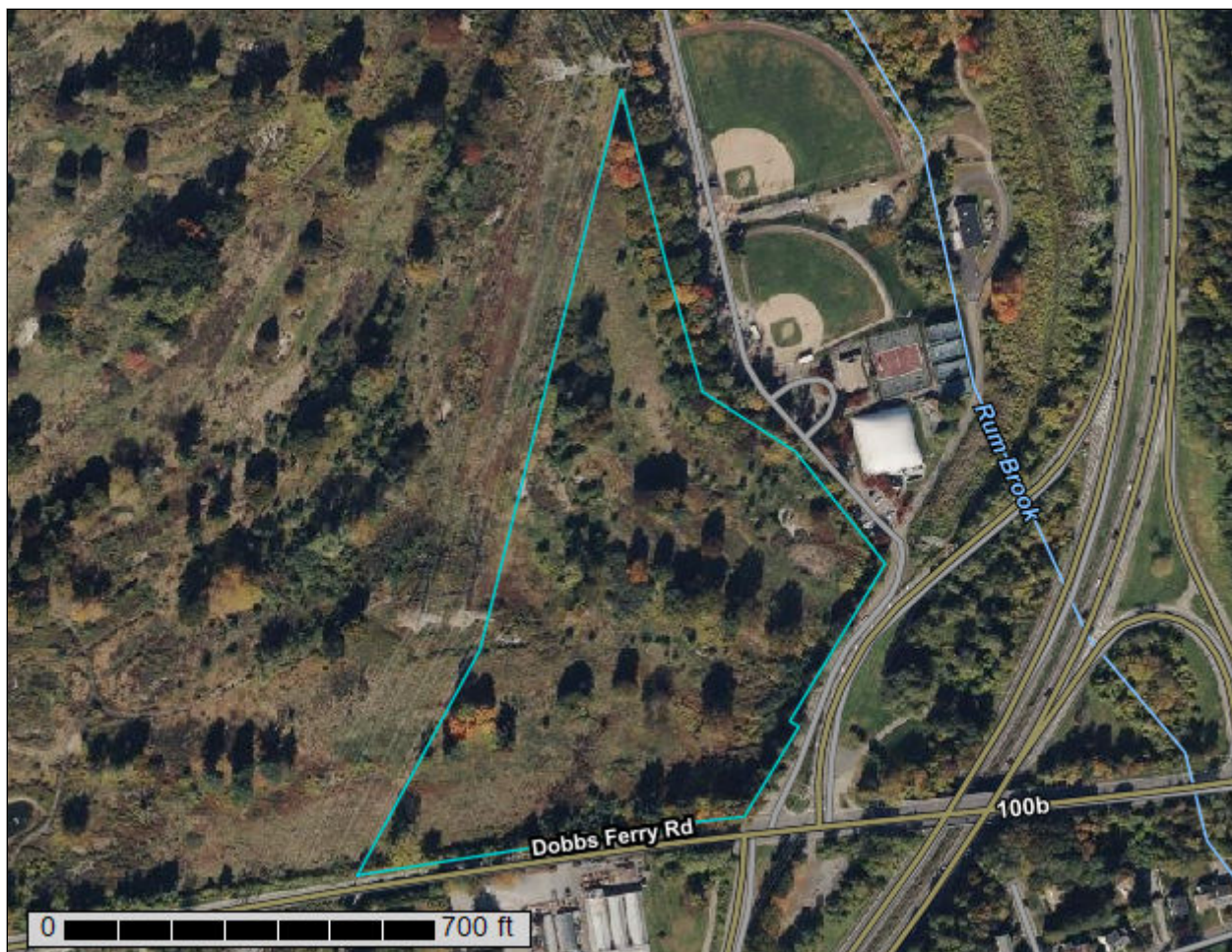
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for **Westchester County, New York**



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

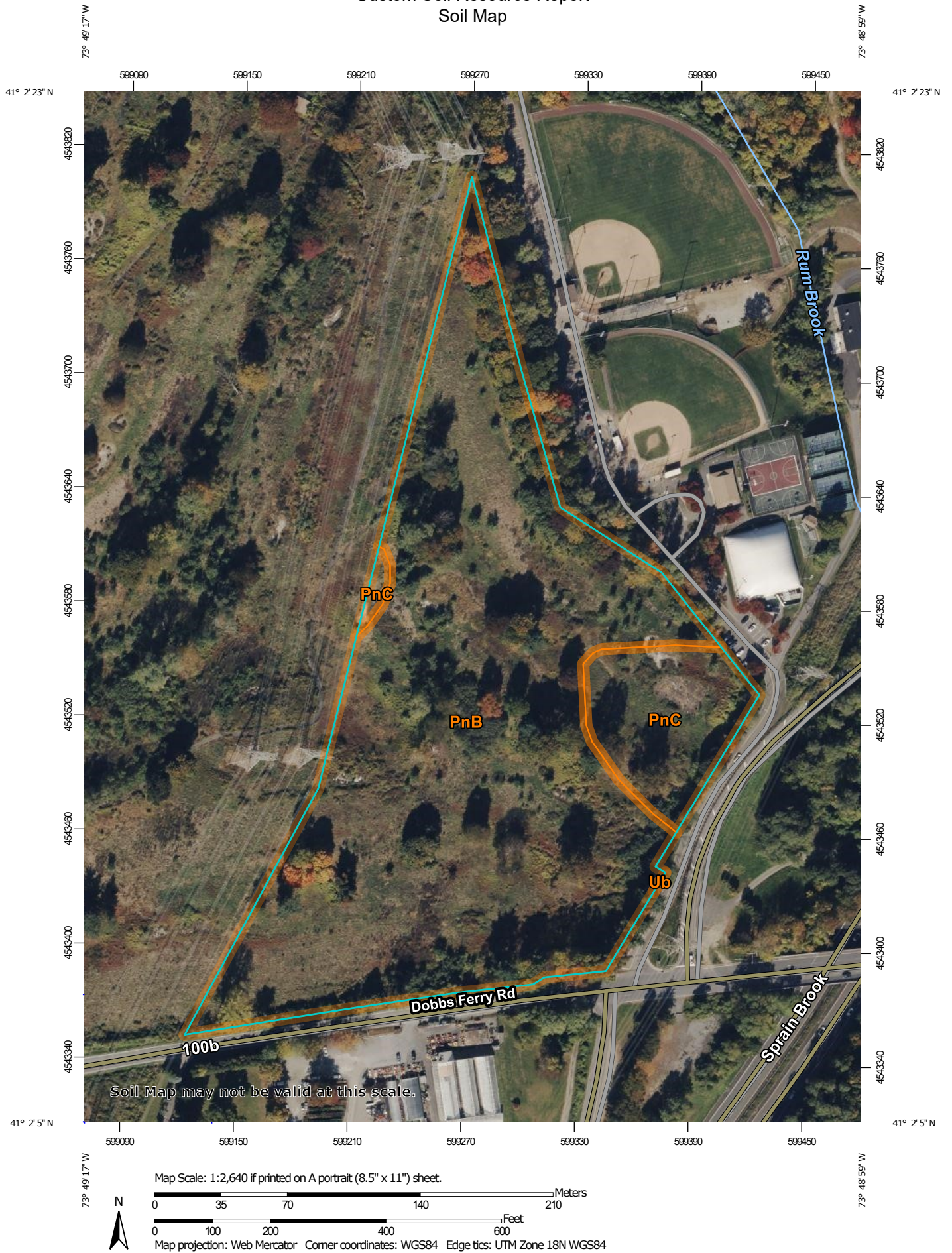
Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



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MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 20, Aug 26, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 21, 2022—Oct 27, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	12.6	89.0%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	1.6	11.0%
Ub	Udorthents, smoothed	0.0	0.0%
Totals for Area of Interest		14.2	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The

delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Westchester County, New York

PnB—Paxton fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2t2qp
Elevation: 0 to 1,570 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: All areas are prime farmland

Map Unit Composition

Paxton and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Paxton

Setting

Landform: Hills, drumlins, ground moraines
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

Ap - 0 to 8 inches: fine sandy loam
Bw1 - 8 to 15 inches: fine sandy loam
Bw2 - 15 to 26 inches: fine sandy loam
Cd - 26 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 18 to 39 inches to densic material
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 18 to 37 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2s
Hydrologic Soil Group: C
Ecological site: F144AY007CT - Well Drained Dense Till Uplands
Hydric soil rating: No

Minor Components

Woodbridge

Percent of map unit: 9 percent
Landform: Hills, drumlins, ground moraines
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

Ridgebury

Percent of map unit: 6 percent
Landform: Drainageways, hills, ground moraines, depressions
Landform position (two-dimensional): Backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, base slope, dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Charlton

Percent of map unit: 5 percent
Landform: Hills
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

PnC—Paxton fine sandy loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2w66y
Elevation: 0 to 1,320 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Paxton and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Paxton

Setting

Landform: Drumlins, hills, ground moraines
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex, linear
Across-slope shape: Convex

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Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

Ap - 0 to 8 inches: fine sandy loam
Bw1 - 8 to 15 inches: fine sandy loam
Bw2 - 15 to 26 inches: fine sandy loam
Cd - 26 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 20 to 39 inches to densic material
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 18 to 37 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: C
Ecological site: F144AY007CT - Well Drained Dense Till Uplands
Hydric soil rating: No

Minor Components

Charlton

Percent of map unit: 7 percent
Landform: Hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Woodbridge

Percent of map unit: 6 percent
Landform: Ground moraines, drumlins, hills
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

Ridgebury

Percent of map unit: 2 percent
Landform: Hills, ground moraines, depressions, drainageways, drumlins
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave, linear
Across-slope shape: Concave, linear
Hydric soil rating: Yes

Ub—Udorthents, smoothed

Map Unit Setting

National map unit symbol: bd7f
Elevation: 0 to 2,400 feet
Mean annual precipitation: 46 to 50 inches
Mean annual air temperature: 46 to 52 degrees F
Frost-free period: 115 to 215 days
Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, smoothed, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents, Smoothed

Typical profile

H1 - 0 to 4 inches: gravelly loam
H2 - 4 to 70 inches: very gravelly loam

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: 40 to 60 inches to lithic bedrock
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high
(0.06 to 5.95 in/hr)
Depth to water table: About 18 to 48 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 15 percent
Available water supply, 0 to 60 inches: Low (about 4.6 inches)

Minor Components

Udorthents, wet substratum

Percent of map unit: 5 percent
Hydric soil rating: No

Urban land

Percent of map unit: 5 percent
Hydric soil rating: Unranked

Leicester

Percent of map unit: 2 percent
Hydric soil rating: No

Hollis

Percent of map unit: 2 percent
Hydric soil rating: No

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Charlton

Percent of map unit: 2 percent

Hydric soil rating: No

Riverhead

Percent of map unit: 2 percent

Hydric soil rating: No

Sun

Percent of map unit: 2 percent

Landform: Depressions

Hydric soil rating: Yes

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

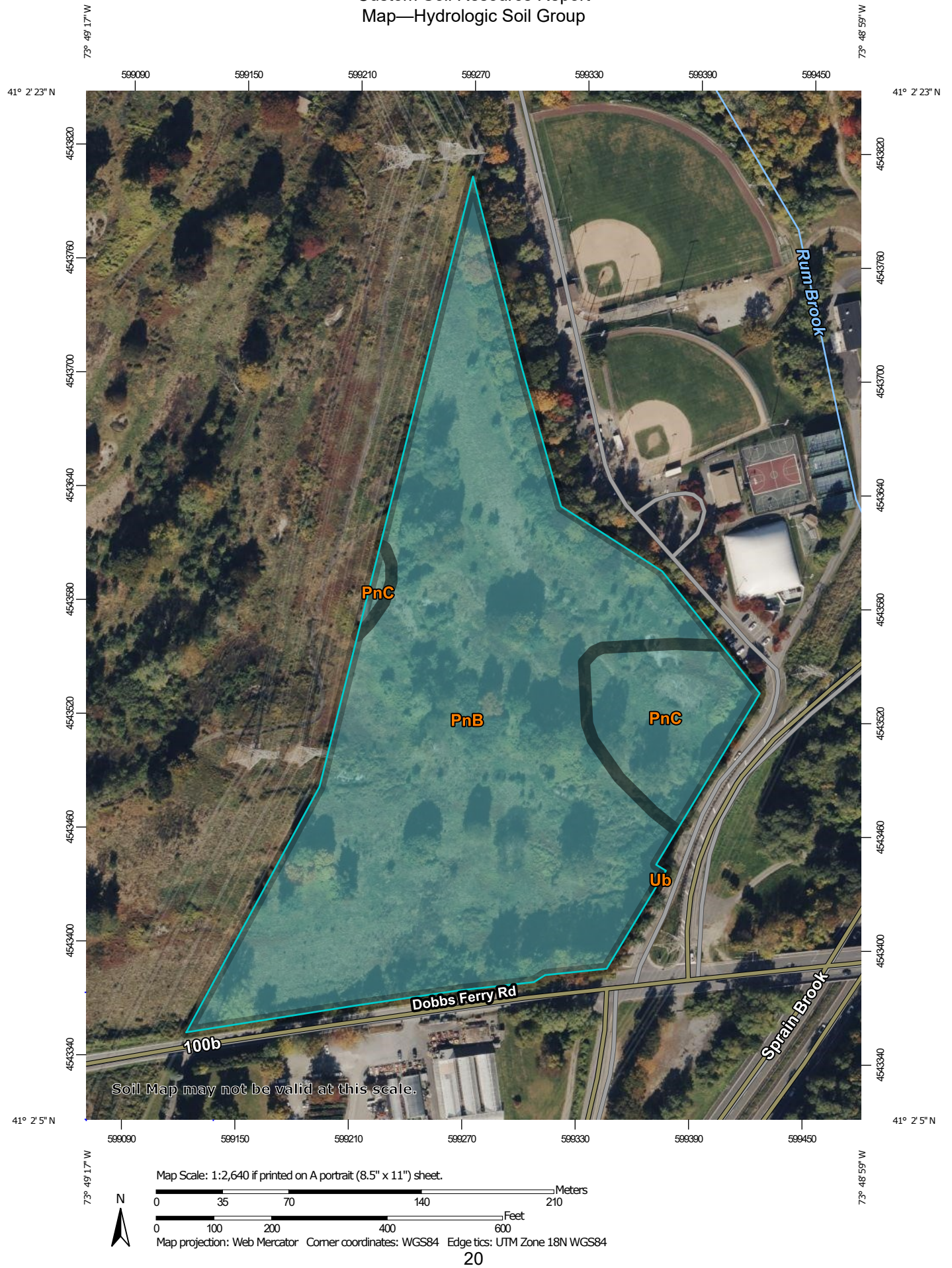
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Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

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Map—Hydrologic Soil Group



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MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 20, Aug 26, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 21, 2022—Oct 27, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	C	12.6	89.0%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	C	1.6	11.0%
Ub	Udorthents, smoothed	B	0.0	0.0%
Totals for Area of Interest			14.2	100.0%

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition

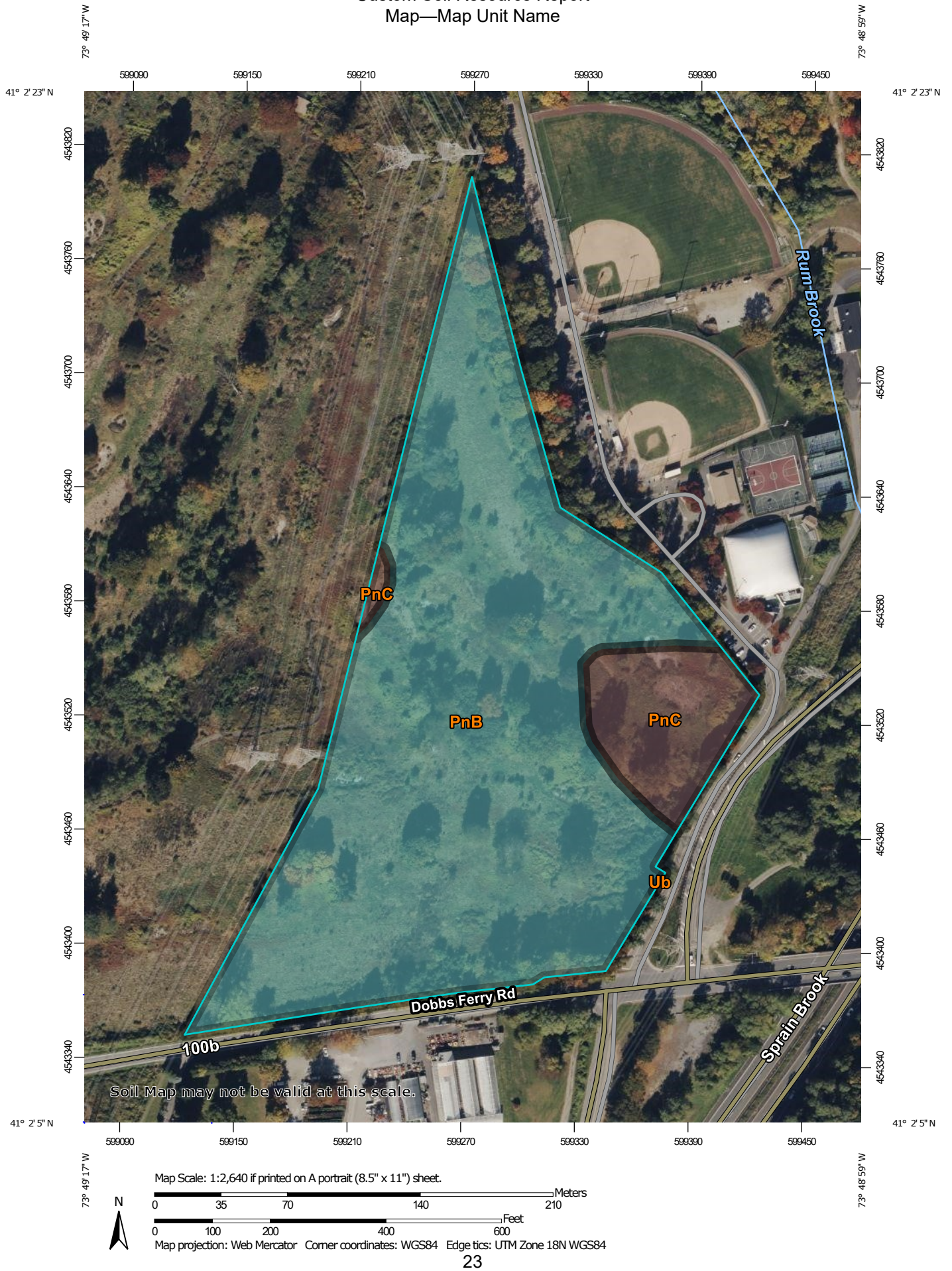
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Map Unit Name

A soil map unit is a collection of soil areas or nonsoil areas (miscellaneous areas) delineated in a soil survey. Each map unit is given a name that uniquely identifies the unit in a particular soil survey area.

Custom Soil Resource Report
Map—Map Unit Name



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

-  Paxton fine sandy loam, 3 to 8 percent slopes
-  Paxton fine sandy loam, 8 to 15 percent slopes
-  Udorthents, smoothed
-  Not rated or not available

Soil Rating Lines

-  Paxton fine sandy loam, 3 to 8 percent slopes
-  Paxton fine sandy loam, 8 to 15 percent slopes
-  Udorthents, smoothed
-  Not rated or not available

Soil Rating Points

-  Paxton fine sandy loam, 3 to 8 percent slopes
-  Paxton fine sandy loam, 8 to 15 percent slopes
-  Udorthents, smoothed
-  Not rated or not available

Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York
Survey Area Data: Version 20, Aug 26, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 21, 2022—Oct 27, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Map Unit Name

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	Paxton fine sandy loam, 3 to 8 percent slopes	12.6	89.0%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	Paxton fine sandy loam, 8 to 15 percent slopes	1.6	11.0%
Ub	Udorthents, smoothed	Udorthents, smoothed	0.0	0.0%
Totals for Area of Interest			14.2	100.0%

Rating Options—Map Unit Name

Aggregation Method: No Aggregation Necessary

Tie-break Rule: Lower

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- United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084>

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APPENDIX B

TEMPORARY EROSION AND SEDIMENT CONTROL INSPECTION AND MAINTENANCE CHECKLIST

Temporary Erosion and Sediment Control Inspection and Maintenance Checklist

Erosion and Sediment Control Measure	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Stabilized Construction Entrance	Daily	• Periodic top dressing with additional aggregate as required • Clean sediment in public right-of-ways immediately
Silt Fence	Weekly + After Each Rain	• Remove & redistribute sediment when bulges develop in the silt fence.
Haybales (Baled Fence)	Daily	• Remove sediment and debris • Check that all bales are tight against each other and the ground surface.
Temporary Swale	Weekly + After Each Rain	• Immediately remove any sediment and reestablish vegetative cover as required. • Inspect entire swale and surrounding area for rilling and repair as required with the placement of additional topsoil, then seeding and mulching, as outlined in Figure No. 3.14 of the NYSDEC New York State Standards and Specifications for Erosion and Sediment Control.
Temporary Sediment Trap	Weekly + After Each Rain	• Remove and redistribute sediment when it reaches an elevation indicated on the construction documents. • Check for rilling within and around the sediment basin and repair as required. • Remove all sediment and debris from the outlet control structure as maybe required. • Dewater as necessary to FracTank

The owner/operator responsible for inspection and maintenance as outlined above:

Ridgewood Elmwood Owner, LLC
Contact Name:
254 Hanover Road, Suite 310
Florham Park, NY 07932
Phone:
Fax:
Email:

APPENDIX C

OWNER'S / CONTRACTOR'S CERTIFICATION



Site Planning
Civil Engineering
Landscape Architecture
Land Surveying
Transportation Engineering

Environmental Studies
Entitlements
Construction Services
3D Visualization
Laser Scanning

JMC Project 17099
Elmwood Preserve - Park
850 Dobbs Ferry Road
Town of Greenburgh, Westchester County, NY

OWNER'S CERTIFICATION

"I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that the Notice of Intent (NOI) and corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting the NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting the NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which the NOI is being submitted."

Company Name: _____

Address: _____

Telephone Number: _____

Name and Title: _____

Signature: _____ Date: _____

Permit Identification No.: _____



Site Planning
Civil Engineering
Landscape Architecture
Land Surveying
Transportation Engineering

Environmental Studies
Entitlements
Construction Services
3D Visualization
Laser Scanning

JMC Project 17099
Elmwood Preserve – Park
850 Dobbs Ferry Road
Town of Greenburgh, Westchester County, NY

CONTRACTOR'S CERTIFICATION

"I hereby certify under penalty of law that I understand and agree to comply with the requirements of the SWPPP and agree to implement any corrective actions identified by the qualified inspector during a site inspection. I also understand that the owner or operator must comply with the requirements of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") Construction General Permit (CGP) for Stormwater Discharges from Construction Activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

Company Name: _____

Address: _____

Telephone Number: _____

Name and Title: _____

Signature: _____ Date: _____

Permit Identification No.: _____

Name and Title of Trained Contractor: _____

Elements of the SWPPP Contractor is responsible for: _____

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JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC | JMC Site Development Consultants, LLC

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