



Geotechnical
Environmental
Site Civil

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**DRAFT PRELIMINARY
GEOTECHNICAL INVESTIGATION REPORT**

For

**LRC Construction, LLC
5 Renaissance Square, 42nd Floor
White Plains, NY 10601**

**Proposed Parking Garage
East Post Road, White Plains, NY**

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Prepared for:
LC Post Road Garage, LLC

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SESI Project No:
14228

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DRAWING

FIGURE 1 Exploration Location Plan

APPENDICES

APPENDIX A	SESI Boring Logs (total of 17)
	SESI Soil Classification and Exploration Log Key
APPENDIX B	Geologic Cross Sections (total of 5)
APPENDIX C	Boring Logs from McLaren Geotechnical Report, dated May 7, 2020

1.0 INTRODUCTION

SESI Consulting Engineers (SESI) recently completed a preliminary geotechnical investigation for the proposed project. The proposed project consists of a 10-story parking structure with eight levels above grade, one ground-level, and one below-grade level with a total of 1,950 parking spaces proposed. The parking structure will be directly across East Post Road from the White Plains Hospital, and it is intended to serve the Hospital. This report summarizes SESI's preliminary geotechnical investigation, our findings, and presents preliminary foundation design recommendations for the proposed development. The report also presents a preliminary discussion of groundwater control and support of the mass excavation required to achieve the lowest level.

This report is both preliminary and draft because the design plans for the parking structure are not complete and are in progress. Also, the project site is expected to be a New York State Department of Environmental Conservation Brownfield Cleanup Program site, and the environmental investigation is still on-going. Our recommendations may need to be revised based on the required clean-up work.

2.0 PROJECT DESCRIPTION

2.1 Previous Work Completed

A Geotechnical investigation was completed by McLaren Engineering Group in 2020, and the results were presented in a report "Geotechnical Report – White Plains Hospital Center East Post Road Parking Garage," dated May 7, 2020. McLaren performed 13 soil borings throughout the property to depths between 7.5- and 39-feet below grade (ft-bg). McLaren used hollow stem augers to drill these borings, which is different than the method used by SESI, which was mud-rotary drilling. The results of the McLaren investigation are generally consistent with the results of our investigation. The boring logs from the McLaren report are included as **Appendix C** to this report.

2.2 Site and Surrounding Conditions

According to the topographic survey, which we obtained from several drawings including AKRF "East Post Road Parking Garage," Civil Site Plan Sheets ZB101, ZB201, C501 (listed below), the overall site covers approximately 2.4 acres. The site consists of six (6) separate parcels of land, which abut each other. The six (6) parcels currently consist of vacant land, a parking lot, and vacant commercial structures. The overall shape of the combined parcels is rectangular, approximately 500-feet by 210-feet. The site is bounded to the south by East Post Road with the White Plains Hospital beyond, to the west by developed land, to the north by the MLK Jr. Blvd. Housing Authority property, and to the east by Winchester Street. Topographically, the site is gently sloping, with grades ranging from elevation 200 to 204 in the northern half of the tract, and from elevation 207 to 213 in the southern half of the tract. A 5- to 6- foot high embankment, running east - west, exists between a portion of the north and south portions of the tract.

2.3 Proposed Development

SESI has reviewed the following documents in preparation of this report:

- 1) Walker Consultants "East Post Road Parking Structure," a preliminary design set of drawings dated May 2020. This design set is being changed to reflect a larger structure.
- 2) Walker Consultants "East Post Road Parking Structure," 11 Sheets including S-230 Preliminary Structural Elevations, dated 8/25/2025.

- 3) AKRF "East Post Road Parking Garage," Civil Site Plan Sheets ZB101, ZB201, C501. The topography referenced in this report was taken from Sheet ZB201, in this set.
- 4) McLaren Engineering Group "Geotechnical Report," dated 5/07/2020.

The proposed project currently consists of a parking structure, eight above-grade levels, one ground-level, and one below-grade level. A total of 1,950 parking spaces are proposed. The design drawings are in progress and preliminary at this time.

This current design calls for the lowest level slab at elevation 195.7 (this is 11- to 18±-feet below East Post Road). The ground level slab is at elevation 206.7, the eighth level slab is at elevation 286.4, and the top elevation of the structure is at elevation 304.

The bottom of the proposed structure (below the lower tier slab) is proposed to be near elevation 193, although the drawings indicate some elements may extend a little deeper. Also, the western one-third (approximate) of the structure may be higher than elevation 193, although the details are not clear.

Based on discussions with the Structural Engineer, this will be a heavily loaded structure, with individual column loads ranging from 2,000 kips to 4,000 kips.

3.0 AVAILABLE INFORMATION REVIEW

We obtained and reviewed available historic topographic maps, historic aerial photographs, geologic maps, and FEMA flood map for the Site.

- FEMA Flood Insurance Rate Map - Based on a search of the FEMA Flood Insurance Rate Maps available, this area does not appear to have a preliminary FEMA flood insurance rate map. It does, however, have an effective FEMA panel, map number (36119C0269F), (effective on 9/28/2007), which indicates that the Site is located within zone X, which is determined to be outside the 0.2% annual chance floodplain. This should be confirmed by the site civil engineer.
- Historic Aerial Photographs – Historic aerial photographs indicate that a majority of the existing site and surrounding development pre-date the 1990s. The one exception is a former structure that was located on 26 East Post Road but was demolished between 2016 and 2018. That site is now vacant.
- NY State Geologic Survey, "Bedrock Geology of the White Plains 7.5-Minute Quadrangle," dated 2024 – Published site geologic information maps the site as containing Inwood Marble.

4.0 PRELIMINARY SUBSURFACE INVESTIGATION

4.1 Field Investigation

Our engineering study consisted of a review of existing soils and geologic data, and a field investigation consisting of seventeen (17) Geotechnical borings drilled during July and August 2025 using a B-29 Mobile ATV drilling rig contracted directly by the Client. The boring locations are spaced throughout the footprint of the structure; however, there are gaps because either

access was not permitted, or because existing structures presented access barriers. The borings were drilled to depths ranging from 15 to 65 ft-bg. Boring B-11 was terminated at 8 ft-bg.

In addition to the 17 Geotechnical borings, SESI has conducted an Environmental drilling program consisting of approximately 100 borings. The results of this drilling program (boring logs, boring locations) will be submitted at a later time as an addendum to this report.

The approximate locations of the 17 SESI Geotechnical borings are shown on the **Exploration Location Plan - Figure 1**. Individual boring logs, which describe the materials encountered along with a key to SESI soil terminology, are presented in **Appendix A**. Geologic cross-sections through the Site are included in **Appendix B**.

Rock cores were obtained in 11 of the 17 borings; due to the limited schedule there was not sufficient time to obtain a core at every boring. All borings were drilled using mud-rotary drilling techniques, and every boring (except B-11) was drilled to refusal on either competent or decomposed bedrock.

The locations of Geotechnical borings conducted by the McLaren Engineering Group in 2020 are also included on **Figure 1**, and the logs are included in **Appendix C**. It is noted that the McLaren borings drilled in 2020 utilized hollow stem auger drilling, not mud-rotary drilling techniques. Comparisons of boring logs using these two different techniques should consider that hollow stem augers do not have as much penetration capacity as the tri-cone bit used in mud rotary drilling; therefore, drilling refusal depths between the two techniques are not equivalent. Regardless, we have compared the results where the two types borings are proximal, and the results are similar.

Soil and decomposed bedrock samples suitable for identification purposes were extracted from the borings at closely spaced intervals in accordance with the Standard Penetration Test (SPT, ASTM D1586-11). For this test, a standard split-spoon sampler (2-inches outside diameter, one and three-eighths inches inside diameter) is driven into the soil by a 140-pound weight falling 30-inches. After discounting the initial 6-inches of penetration due to possible disturbance of the material resulting from the drilling operation, the number of blows required to advance the sampler a distance of 12-inches is recorded and designated as the standard penetration resistance or "N" value. The "N" value is an indication of the relative compactness of the soil in-situ.

The 11 bedrock cores were also brought to our laboratory and recoveries, and rock quality designations (RQDs) were measured. The RQD of a core sample is an estimate of the quality of a rock mass. It is calculated as a percentage by summing the lengths of intact, sound core pieces that are 4-inches or longer and dividing by the total length of the core run. A higher RQD value indicates less fracturing and more sound bedrock.

RELATIONSHIP OF RQD AND ROCK QUALITY:

<u>ROCK QUALITY DESIGNATION (RQD)</u>	<u>DESCRIPTION OF ROCK QUALITY</u>
0 – 25	VERY POOR
25 – 50	POOR
50 – 75	FAIR
75 – 90	GOOD
90 – 100	EXCELLENT

The field work was performed under the direct technical observation of a Geotechnical Engineer from SESI. Our representative located the explorations in the field, maintained continuous logs of the explorations as work proceeded, and coordinated the soil sampling operations in order to develop the desired subsurface information. The borings were located in the field using plans provided by the Client. Ground surface elevations at each of the boring locations were approximated from plans (the plans depicted the topography) provided by the Client.

4.2 Geotechnical Laboratory Program

Soil and bedrock core samples were extracted from the borings and brought to our soil mechanics laboratory for additional classification.

4.3 Soil and Bedrock Conditions

The subsurface conditions at the Site generally consist of surficial fill, underlain by glacial till, over decomposed bedrock, over sound bedrock. The NY State Geologic Survey has mapped the site bedrock as Inwood Marble ("Bedrock Geology of the White Plains 7.5-Minute Quadrangle," dated 2024). There are other bedrock types mapped very close-by, including Yonkers Gneiss, Manhattan Schist, and Phlogopitic Marble. The bedrock geology in this area is complicated by intense tectonic activity in the past including contact zones and thrust faults. The following strata are described in the order of increasing depth. Refer to the geologic cross sections in **Appendix B**.

4.3.1 Uncontrolled Fill

Uncontrolled fills were encountered at the ground surface in most of the boring locations. The fill generally consists of a variable mixture of gravel, sand, silt and clay, with some debris mixed in. The fill generally extends to a depth of 5- to 15 ft-bg. The proposed construction will almost completely extend below the depth of the fill. Thus, the thickness and condition of the uncontrolled fill, with regards to the structural support of the proposed construction is not important; however, it will be important with regards to the design of the support of excavation system. In addition, the environmental condition of the fill is currently being evaluated separately by SESI.

4.3.2 Glacial Till Deposits

Glacial till is present throughout much of the site below the uncontrolled fill. It typically exists as a wedge. It is approximately 10- to 17-feet thick at the east end of the site and it pinches out (it is not present) at about the location of Brookfield Street. The glacial till consists of silty and gravelly sands and is in a dense to very dense condition, with "N" values 30 and above. Where present, the glacial till directly overlies either decomposed bedrock or hard, sound bedrock.

4.3.3 Decomposed Rock

Weathered and decomposed bedrock is present throughout much of the site, and it is thickest towards the south. It is not present in the northern portion of the site, where hard sound bedrock exists directly below the glacial till. The decomposed bedrock is weathered granite. It is in a very dense condition ("N" values greater than 50/1-inch) and was difficult to drill through. Samples obtained are silty and gravelly sands, and are the result of the tricone bit grinding through the decomposed bedrock. During our investigation, we extended many of the borings through this stratum to hard competent bedrock so it could be cored.

4.3.4 Competent Bedrock

Three (3) different types of bedrock were encountered on this site: gneiss, schist, and granite. We did not encounter Inwood Marble, which is the mapped bedrock type. In the north and west portions of the site, competent bedrock consists of Fordham Gneiss, although in the northeast

portion of the site, decomposed mica schist bedrock was encountered. In the southwest portion of the site, competent bedrock consists of Fordham Gneiss, but this changes to granite towards the east. In the east and southeast portions of the site, the competent bedrock includes granite, Fordham Gneiss, and mica schist. This tri-mixture of bedrock types in such a small area is a result of the intense tectonic activity in the past. The gneiss and schist are metamorphic rocks and the granite is an intrusive igneous rock. All of these bedrock types are very competent bearing materials; however, the decomposed and weathered bedrock (granite, an igneous rock) and metamorphic rocks (gneisses, schists) can be highly variable and vary over short distances. They can contain bands of more highly weathered materials and the bedding planes can vary significantly.

4.3.5 Groundwater

Groundwater, as measured in temporary wells at the site, was present at a depth of 5- to 7 ft-bg in August 2025; these wells are now abandoned. It is very difficult to determine the groundwater level when using mud-rotary drilling techniques, due to the use of drilling fluid; thus, the current known depth to groundwater is based on measurements in wells. This groundwater depth does not likely reflect the seasonal high water table as the area is in a drought. The water table could be several feet higher than measured in the wells, as it is a function of the amount of recent precipitation.

The proposed parking structure is to extend significantly below the water table; therefore, it is important to determine the design depth to groundwater for groundwater control and ultimately, for the placement of the waterproofing liner. As discussed below, since the structure will be constructed significantly below the water table, the lowest level slab(s) will need to be designed to resist hydraulic uplift pressures. Groundwater monitoring wells are currently being installed at the site. They should be monitored over several periods of wet weather. When this data is obtained this report will be updated.

Refer to Section 7.1, below, for a discussion of groundwater control during construction.

5.0 EVALUATION AND RECOMMENDATIONS

5.1 Introduction

This current design calls for a lower tier slab at elevation 195.7. The bottom of the proposed structure (below the lowest level slab) is proposed to be at approximately elevation 193, and the drawings indicate some elements extend a little deeper. A support of excavation (SOE) system will need to be designed to support East Post Road and the surrounding properties. Groundwater, as measured in wells in August 2025, was present near elevation 195 to 197. A temporary dewatering system will need to be installed as part of the SOE system design. Also, since the structure will be constructed significantly below the water table, the lowest level slab(s) will need to be designed as pressure slabs to resist upward hydraulic forces - this is discussed further as it applies to each of the three options for foundation support.

The Structural Engineer has indicated this will be a heavily loaded structure, with individual column loads ranging from 2,000 kips to 4,000 kips. The investigation has determined that the site soil and bedrock conditions are variable. Along the western portion of the site, decomposed bedrock and/or competent bedrock exists near elevation 193; however, throughout most of the site, very dense glacial till is present near and below elevation 193. To support the high structural loads on the variable geology, we have considered three (3) foundation options:

- 1) Mat foundation bearing on glacial till and/or decomposed or competent bedrock.
- 2) Spread foundations/piers bearing on glacial till or decomposed bedrock.
- 3) Caisson piles (drilled shafts) bearing within competent bedrock.

The columns that will support the structure will have variable loading conditions. As the design of the structure is in progress, we do not currently know the magnitude and location of the proposed column loads.

5.2 Site Preparation Procedures

The site preparation procedures for this project will likely require the following tasks:

1. Demolition of existing structures.
2. Installation of a groundwater control system and a support of excavation system around the site perimeter to support the mass excavation. These items are discussed in greater depth below.
3. Mass excavation of soil within the SOE. The depth of the excavation will extend to at least elevation 193± (the bottom of the proposed structure). However, based on the results of the environmental investigation it may need to extend deeper in places.

5.3 Foundation Recommendations

Following is a discussion of the three (3) foundation options discussed above.

5.3.1 Mat Foundation Bearing on Glacial Till/Decomposed and Competent Bedrock

Preliminarily, a mat foundation is the recommended option for foundation support. The mat would support the various columns, which will impose different loads on the mat, and which will be irregularly spaced. The underlying geologic materials are variable; however, almost of the bearing materials would be glacial till or decomposed bedrock. A small area in the northwestern portion of the site might bear on more competent bedrock. It is possible that a small area of uncontrolled fill will need to be removed in the northeastern portion of the site and replaced with structural fill. Finally, it is not known if the environmental remediation will require deeper excavations which, if required, could result in additional areas of structural fill construction.

These variables – column loads and spacing, the nature of the bearing material – would require structural analyses so that the mat does not experience unacceptable differential settlements or experience damaging stresses. Ground improvement elements could be considered under those portions of the mat containing the highest column loads, although the glacial till and decomposed bedrock are very dense so this may not be feasible, or necessary.

A mat foundation could minimize differential settlements of the structure, as opposed to individual foundations supporting columns. An added advantage of a mat foundation is that it could serve a dual purpose as a pressure slab to counter the upward hydraulic forces (assuming the weight of the structure on the mat is sufficient). The bottom of the mat slab will be below the water table; The design groundwater elevation has not been determined as groundwater monitoring wells have not yet been installed and sufficiently monitored.

We recommend a modulus of subgrade reaction value of 120-pounds per cubic-inch (pci) be used for initial mat thickness and reinforcement design. Once the initial mat foundation layout has been determined, we will need to perform an iterative settlement analysis in collaboration with the Project Structural Engineer to assist them with final mat foundation design, and to confirm that the total and differential foundation settlements can be expected to remain within tolerable limits.

The results of this analysis will also need to be reviewed to determine the magnitude of mat foundation loading and settlements along the site perimeter to confirm any induced settlements at the adjacent structures will remain within tolerable limits. If the analyses results indicate significant localized overstressing of the mat subgrade, or significant mat foundation or neighboring structure settlements, we anticipate that drilled-in settlement-reducing elements can be utilized at strategic locations within the mat footprint to control the mat foundation contact stresses and resulting settlements.

5.3.2 Spread Foundations/Piers Bearing on Glacial Till or Decomposed Bedrock

As an alternative to a mat foundation, the columns could be supported separately, or in groups, on the glacial till or decomposed bedrock strata. A disadvantage of this option is that a system of independent foundations, carrying a range of different loads, and bearing on different geologic media could result in unacceptable differential settlements of the structure. Also, the lowest level slab would need to be separately designed as a pressure slab with tie-down anchors (see Section 7.3) due to the high hydraulic head.

Columns with smaller loads could be supported on spread footings bearing on the glacial till or decomposed bedrock. For example, the glacial till has an allowable bearing capacity on the order of 3.0 to 4.0 tons per square foot (the decomposed bedrock has a higher allowable bearing capacity). The allowable bearing capacity is dependent on the design groundwater elevation, foundation depth and dimensions, and other factors. Assuming an allowable bearing capacity of 3.5 tsf, a 2,000 kip column would require a 17-foot square spread footing. This option would likely not be practical for the heavier loaded columns, and even this might not be cost effective or practical.

The decomposed bedrock has a higher allowable bearing capacity than the glacial till. It is similar to Dense Sandy Gravel Soil – Class 2a material (2022 NYB BC Table 1806.1) which has a presumptive allowable bearing capacity of 10 tsf.

5.3.3 Caisson Piles (Drilled Shafts) Bearing Within Competent Bedrock

Alternately, columns could be supported separately, or in groups, on caisson piles socketed into the competent bedrock strata. Similar to the spread foundation option, a disadvantage is that a system of independent foundations, carrying a range of different loads, and bearing within bedrock types with different characteristics could result in unacceptable differential settlements of the structure. The lowest level slab would also need to be separately designed as a pressure slab with tie-down anchors to counter hydraulic head.

The competent bedrock on site, which is either gneiss or granite, has an allowable bearing capacity of 20 tsf. For caisson piles, an allowable side-adhesion of 200 pounds per square inch between the competent bedrock and the sides of the caisson pile rock socket may be used for design. These strength values assume the caisson piles are socketed into hard, sound, competent bedrock, and this would need to be confirmed by the Geotechnical Engineer at the time of installation.

6.0 FOUNDATION DESIGN CRITERIA

6.1 Allowable Bearing Capacities of Geologic Media

Following is a summary of the maximum allowable bearing capacity of the geologic materials on the project site. We have referenced the NYC Building Code because the values in the Code are widely accepted as being valid and reliable, and the site is proximal to NYC.

- 1) Glacial Till – 3.0 to 4.0 tons per square foot, depending on final design parameters.
- 2) Decomposed bedrock – It is similar to Dense Sandy Gravel Soil – Class 2a, which has a presumptive allowable bearing capacity of 10 tsf (2022 NYB BC Section 1803.3 and Table 1806.1)
- 3) Competent bedrock – It is classified as Intermediate Rock - Class 1c, which has an allowable bearing capacity of 20 tsf (2022 NYB BC Section 1803.3 and Table 1806.1). Also, the allowable bond stress between the competent bedrock and the sides of a caisson pile rock socket is 200 pounds per square inch (2022 NYC Building Code Section 1812.7.3).

6.2 Seismic Design

The site glacial till soils and decomposed bedrock, below the layer of uncontrolled fill, have been classified as Site Class C for seismic design purposes in accordance with ASCE 7-16 and the 2020 New York State Building Code. Based on a structural occupancy/risk category of IV and information provided by the ASCE 7 Hazard Tool, the following seismic design criteria should be used for this project:

Mapped Spectral Response Acceleration for Short Periods	$S_S = 0.28g$
Mapped Spectral Response Acceleration for 1-Second Period	$S_1 = 0.051g$
Site Coefficient	$F_a = 1.6$
Site Coefficient	$F_v = 2.4$
Spectral Response for short periods	$S_{MS} = 0.31g$
Spectral Response for 1 second period	$S_{M1} = 0.11g$
Design Spectral Response Acceleration for Short Periods	$S_{DS} = 0.21g$
Design Spectral Response Accelerations for 1-Second Period	$S_{D1} = 0.071g$

6.3 Summary of Soil Design Parameters

Table I, below, presents a summary of the soil design parameters.

PARAMETER	VALUE
1. Glacial Till Allowable Soil Bearing Capacity (net)	3.0 to 4.0 tsf
Decomposed Bedrock Presumptive Allowable Bearing Capacity	10 tsf
Competent Bedrock Presumptive Allowable Bearing Capacity	20 tsf
2. Glacial Till Total Unit Weight	130 pcf
3. Glacial Till Angle of Internal Friction -	32 degrees
4. Earth Pressure Coefficient (See Note 1)	
Active Earth Pressure (Ka)	0.33
Earth Pressure @ Rest (Ko)	0.50
Passive Earth Pressure (Kp)	3.00
5. Coefficient of Sliding Friction (concrete over soil)	0.45
6. Subgrade Modulus for Floor Slab Design – Glacial Till	175 pci
Preliminary Subgrade Modulus for Mat Sab Design	120 pci
7. Permanent Slopes (above groundwater)	
Maximum Cut and Fill Slope in Soil	2H:1V
8. Seismic Design Criteria- Site Class	C

7.0 ADDITIONAL CONSTRUCTION RECOMMENDATIONS

7.1 Control of Groundwater

The design depth to groundwater has not yet been established; monitoring wells are currently being installed. These wells should be monitored through several periods of wet weather and during the wet season to develop a design groundwater elevation. A construction dewatering system will need to be installed. This system could be an integral part of the support of excavation system (e.g., soldier piles with lagging and grouting behind the wall, secant or tangent piles) which would minimize groundwater from entering the excavation. Alternately, if a pervious SOE is installed, then a groundwater pumping system will be required. The site fill, and sand layer (below the fill in some areas of the site) are moderately transmissive to groundwater. The glacial till has a high silt and clay content and should have low groundwater transmissivity. We do not know, but would anticipate, that the decomposed bedrock will be moderately transmissive.

The foundation will need to be designed to resist uplift hydraulic pressures. If a mat foundation is selected, then the weight of the mat and subsequent construction should be sufficient to resist the uplift pressures. The groundwater dewatering system must remain functional until the structure is sufficiently constructed, and the weight of the structure is sufficient to resist the uplift.

If a mat foundation is not constructed and individual elements are constructed (e.g., spread footings, caisson piles), then the lowest level slab should be designed as a pressure slab. It may be necessary to install rock anchors to resist uplift of the slab.

We understand a PREPRUFE waterproofing membrane system will be installed beneath the structure and around the foundation walls so post construction groundwater control may not be necessary.

7.2 Support of Excavation

A support of excavation system needs to be designed and constructed. Preliminarily, there are two (2) general SOE types that should be considered for this project:

- 1) Soldier piles and lagging. This SOE system would also require one to two levels of soil anchors to support the SOE, depending on the excavation depth. A disadvantage of this system is that it is pervious and would allow groundwater infiltration if the pervious soils/decomposed bedrock strata are not grouted.
- 2) Secant or tangent piles. Secant or tangent piles would similarly require one to two levels of soil anchors for support. An advantage of these options is that they would provide for a more impervious barrier to groundwater infiltration; note that the tangent piles are more impervious than secant piles. These options are generally more costly than soldier piles and lagging.
- 3) A diaphragm wall is an option but has not been considered further for this site as it is usually selected for deeper SOE systems and it is costly.

7.3 Post Tensioned Rock Anchors

If post-tensioned tie down anchors are considered for one of the foundation options, the anchors should be designed as rock anchors, since competent bedrock is not that deep. A maximum allowable bond strength between the competent bedrock and the anchor of 75 pounds per square inch should be utilized. This is consistent with recommended design values provided in the Post-Tensioning Institute Design Manual.

8.0 TESTING AND INSPECTION REQUIREMENTS

8.1 Inspection

The recommendations presented in the previous sections of this report assume that the construction procedures will be done under engineering inspection by a representative of this office.

SESI should inspect and approve the bottom of the mat slab, spread foundations prior to placement of concrete or drilled caisson piles to determine that the founding materials are capable

of supporting the anticipated foundation loads. SESI should also observe the installation of the SOE and groundwater dewatering system.

9.0 LIMITATIONS

The subsurface investigation identifies the subsurface conditions only at the locations of the explorations and at the depths where the samples were taken. Building locations were not provided prior to the performance of the subsurface exploration. Borings were located by your office to provide general information for the site and may not represent the subsurface conditions in the anticipated building footprint. SESI Consulting Engineers reviews the published geologic data and the field and laboratory data and uses their professional judgment and experience to render an opinion on the subsurface conditions throughout the Site. Because the actual subsurface conditions may differ, we recommend that SESI be retained to provide construction inspection in order to minimize the risks associated with unanticipated conditions. This report should not be used:

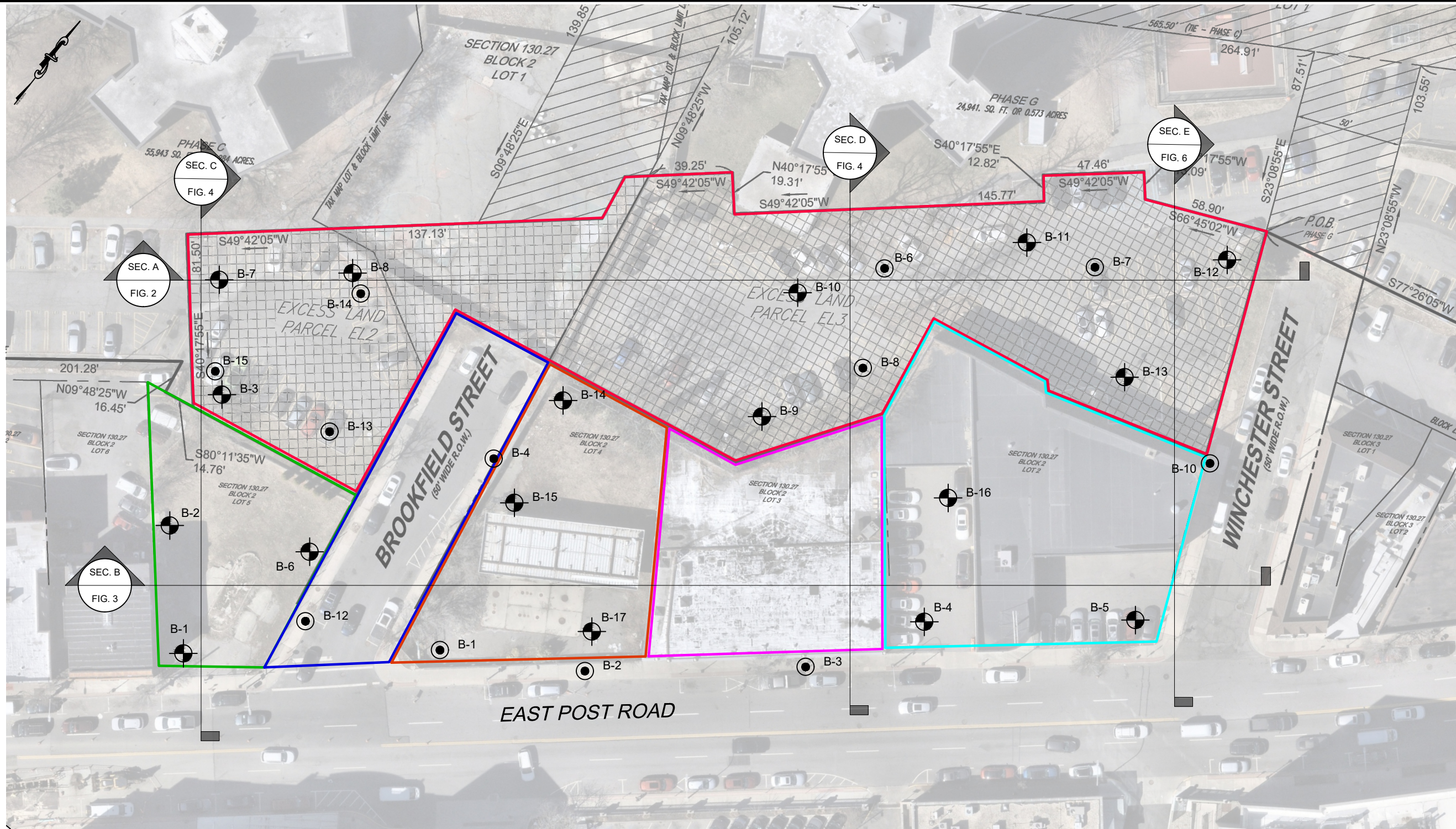
- When the nature of the proposed building(s) are changed;
- When the size or configuration of the proposed building is altered;
- When the location or orientation of the proposed building is modified;
- When there is a change in ownership; or
- For application to an adjacent or any other site.

SESI shall not accept any responsibility for problems, which may occur if SESI is not consulted when there are changes to the factors considered in this report's development. The soil logs should not be separated from the Engineering Report in order to minimize the possibility of soil log misinterpretation.

10.0 DISCLAIMER

This Report was prepared by SESI for the sole and exclusive use of . Nothing under the Professional Services Agreement between SESI and its client, LC Post Road Garage, LLC shall be construed to give any rights or benefits to anyone other than Client and SESI, and all duties and responsibilities undertaken pursuant to the Agreement will be for the sole and exclusive benefit of Client and SESI and not for the benefit of any other party. This Report has been prepared and issued subject to the express condition that same is not to be disseminated to anyone other than Client, without the advance written consent of SESI (which SESI, in its sole discretion, is free to grant or withhold). Use of the Report by any other person is unauthorized and such use is at the sole risk for the user.

N:\ACAD\14228\CAD\ENVIRONMENTAL\14228.DWG.EXPLORATION - V3.DWG 09/10/25 02:37:34PM, .dpg, LAYOUT:FIG 1



NOTE:
THIS PLAN IS FOR LOCATING SOIL BORINGS ONLY.

LEGEND:

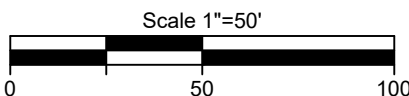
- | | | | | | | |
|--|-----|---|--|--|---|--|
| | B-1 | - | BORING NUMBER & APPROX. LOCATION | | - | 42 EAST POST ROAD, STORE FRONTS |
| | B-2 | - | BORING NUMBER & APPROX. LOCATION
(McCLAREN ENGINEERING GROUP -
APRIL 2020) | | - | BROOKFIELD STREET |
| | | - | | | - | 34 EAST POST ROAD, FORMER MARIANINA SITE |
| | | - | | | - | 60 EAST POST ROAD, FORMER TIRE SHOP |
| | | - | | | - | 26-28 EAST POST ROAD, FORMER GETTY |
| | | - | | | - | MLK JR BLVD, HOUSING AUTHORITY |

REFERENCE

- EXISTING SITE INFORMATION PROVIDED BY CONTROL POINT ASSOCIATES, INC., "EXHIBIT SKETCH", DATED JUNE 7, 2022.
- McCLAREN SOIL BORING LOCATIONS TAKEN FROM McCLAREN ENGINEERING GROUP, "GEOTECHNICAL REPORT", DATED MAY 7, 2020.

NYS Education Law
Unauthorized alterations or additions to this plan are a violation of section 7209 (2) of the New York State Education Law. Copies of this map not having the seal of the engineer shall not be valid.

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project:
EPR PARKING GARAGE
40 EAST POST ROAD
WHITE PLAINS, NY 10601

job no: 14228
drawing no:

title:
EXPLORATION LOCATION PLAN

dwg by: APG
chk by: CZ
scale: AS NOTED
date: 09/10/2025

SESI CONSULTING ENGINEERS
GEOTECHNICAL | ENVIRONMENTAL | SITE CIVIL
959 ROUTE 46E, 3RD FLOOR, PARSIPPANY, NJ 07054 PH: 973.808.9050

FIG. 1
1 of 1

APPENDIX A

SESI Boring Logs (total of 17)

SESI Soil Classification and Exploration Log Key

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	207.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.00 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH	10.0 ft		AT END OF DRILLING
FINAL DEPTH	44.5± ft		24 HR AFTER DRILLING

[illegible]

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	207.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in	▽ AT TIME OF DRILLING	6.00± ft
CASING DEPTH	10.0 ft	▼ AT END OF DRILLING	
FINAL DEPTH	20.0± ft	▽ 24 HR AFTER DRILLING	

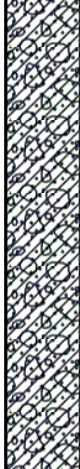
[illegible]

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	202.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH	15.0 ft		AT END OF DRILLING
FINAL DEPTH	24.0± ft		24 HR AFTER DRILLING

[illegible]

PROJECT NAME EPR Parking Garage
PROJECT NO. 14228
DATE STARTED 07-16-2025 **COMPLETED** 07-16-2025
DRILLING CONTRACTOR AARCO
SAMPLER 2-inch split spoon
EQUIPMENT B-29 Mobil Drill
DRILLING FOREMAN Selden **HELPER** Frank
LOGGED BY A. Daus **CHECKED BY** C. Zwingle
LATITUDE 41.026767 **LONGITUDE** -73.769289

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88 **GROUND ELEVATION** 212.0±
DRILLING METHOD Mud Rotary
SAMPLE HAMMER Auto
AUGER INNER DIAMETER _____ **OUTER DIAMETER** _____
ROTARY BIT DIAMETER _____ **DEPTH TO GROUNDWATER:** _____
CASING DIAMETER _____ **AT TIME OF DRILLING** 18.00± ft
CASING DEPTH _____ **AT END OF DRILLING** _____
FINAL DEPTH 47.5± ft **24 HR AFTER DRILLING** _____

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data							Remarks																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
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PROJECT NAME EPR Parking Garage **PROJECT LOCATION** 40 East Post Road, White Plains, NY
PROJECT NO. 14228 **ELEVATION DATUM** NAVD-88 **GROUND ELEVATION** 212.0 ft ±

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data							Remarks		
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)				
									20	40		60	80
		...same as above		S-7	⊗	4		50/4"					
	172	No Recovery	40	S-8	≡	0		50/1"					
	167	Bedrock Core from 45'-47.5' Brown, tan, very coarse grained, hard, moderately weathered, Granite bedrock	45	RC-1		REC = 67%; RQD = 10%		05:38 05:32 04:03					
		BORING COMPLETED AT 47.5± FEET											Drilled to 45 Coring inconsistent first feet Cored 2.5 feet Rock was breaking and core stopped catching.
	162		50										
	157		55										
	152		60										
	147		65										
	142		70										

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PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	212.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in	▽ AT TIME OF DRILLING	10.00± ft
CASING DEPTH	30.0 ft	▼ AT END OF DRILLING	
FINAL DEPTH	42.0± ft	▽ 24 HR AFTER DRILLING	

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data						Remarks										
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)											
									20		40	60	80							
		Fill: 13" DGA: Gray fine Gravel, and coarse to fine Sand, trace Silt																		
		Fill: Brown coarse to fine SAND, little fine Gravel, little Silt																		
		Fill: Brown coarse to fine SAND, little fine Gravel, little Silt, with Schist fragments																		
	207-	No Recovery	5	S-1	X	19				14-7-4-3 (11)										
				S-2	X	11				3-3-2-2 (5)										
				S-3	X	0				4-9-11-3 (20)										
		Fill: Brown coarse to fine Sand, some fine Gravel, little Silt		S-4	X	4				4-2-2-3 (4)										
	202	Black/orange-brown fine GRAVEL (Possible Fill)	10	S-5	X	2				18-8-4-3 (12)										
		Brown/black coarse to fine SAND, some Clayey Silt, trace fine Gravel (Possible Fill)		S-6	X	2				2-1-2-1 (3)										
	197-	Gray SILT, little fine Sand, trace fine Gravel; Glacial till	15	S-7	X	10				6-14-21-22 (35)										
	192	Gray-brown SILT, little coarse to fine Sand; Glacial till	20	S-8	X	8				47-50/5"										
	187	Gray-brown fine SAND and Silt; Glacial till	25	S-9	X	15				16-26-33-48 (59)										
	182	...same as above, with 3" seam of coarse to fine SAND, some Clayey Silt (extremely weathered rock)	30	S-10	X	17				26-35-43-32 (78)										

GROUND ELEVATION 212.0 ft ±

[illegible]

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data							Remarks		
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)				
									20	40		60	80
	203	2" Topsoil Fill: Gray-brown coarse to fine SAND, little fine Gravel, little Silt		S-1	4			6-7-6-4 (13)					0 ppm
		Fill: Orange-brown medium to fine SAND, trace Silt		S-2	18			6-5-6-6 (11)					0 ppm
		Fill: Orange-brown coarse to fine SAND, little Clayey Silt	5	S-3	12			8-12-8-9 (20)					0 ppm
		Fill: Orange-brown coarse to fine Sand, little Silt		S-4	18			9-5-5-7 (10)					0 ppm saturated
	198	Fill: Dark-brown/brown coarse to fine SAND, little Silt	10	S-5	9			4-6-6-11 (12)					0
		White/gray/gray-brown coarse to fine SAND, little Silt (highly weathered, residual Granitic bedrock)		S-6	12			13-17-17-16 (34)					11.5 ppm Casing to 12'
		...same as above		S-7	6			8-22-48-49 (70)					0 ppm
	193		15	S-8	0			21-50/5"					
	188	...same as above BORING COMPLETED AT 20± FEET	20	S-9	4			50/4"					
	183		25										
	178		30										

PROJECT NAME EPR Parking Garage
PROJECT NO. 14228
DATE STARTED 08-14-2025 **COMPLETED** 08-14-2025
DRILLING CONTRACTOR AARCO
SAMPLER 2-inch split spoon
EQUIPMENT Mobile B29
DRILLING FOREMAN Jose **HELPER** Julio
LOGGED BY N. Canto **CHECKED BY** C. Zwingle
LATITUDE 41.026558 **LONGITUDE** -73.770594

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88 **GROUND ELEVATION** 202.0±
DRILLING METHOD Mud Rotary
SAMPLE HAMMER Auto
AUGER INNER DIAMETER _____ **OUTER DIAMETER** _____
ROTARY BIT DIAMETER 3.88 in **DEPTH TO GROUNDWATER:**
CASING DIAMETER 4.00 in ▽ AT TIME OF DRILLING 3.95± ft
CASING DEPTH _____ ▼ AT END OF DRILLING _____
FINAL DEPTH 15.0± ft ▼ 24 HR AFTER DRILLING _____

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data							Remarks																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
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PROJECT NAME EPR Parking Garage
PROJECT NO. 14228
DATE STARTED 08-14-2025 **COMPLETED** 08-14-2025
DRILLING CONTRACTOR AARCO
SAMPLER 2-inch split spoon
EQUIPMENT Mobile B29
DRILLING FOREMAN Jose **HELPER** Julio
LOGGED BY N. Canto **CHECKED BY** C. Zwingle
LATITUDE 41.026669 **LONGITUDE** -73.770419

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88 **GROUND ELEVATION** 203.0±
DRILLING METHOD Mud Rotary
SAMPLE HAMMER Auto
AUGER INNER DIAMETER _____ **OUTER DIAMETER** _____
ROTARY BIT DIAMETER 3.88 in **DEPTH TO GROUNDWATER:** _____
CASING DIAMETER 4.00 in **AT TIME OF DRILLING** _____
CASING DEPTH _____ **AT END OF DRILLING** _____
FINAL DEPTH 17.0± ft **24 HR AFTER DRILLING** _____

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data						Remarks																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	206.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH	15.0 ft		AT END OF DRILLING
FINAL DEPTH	18.5± ft		24 HR AFTER DRILLING

[illegible]

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	204.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING 7.80± ft
CASING DEPTH	15.0 ft		AT END OF DRILLING
FINAL DEPTH	28.0± ft		24 HR AFTER DRILLING

[illegible]

PROJECT LOCATION		40 East Post Road, White Plains, NY	
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	203.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in	▽	AT TIME OF DRILLING
CASING DEPTH	5.0 ft	▼	AT END OF DRILLING
FINAL DEPTH	8.0± ft	▼	24 HR AFTER DRILLING

[illegible]

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	203.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH			AT END OF DRILLING
FINAL DEPTH	30.0± ft		24 HR AFTER DRILLING

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data								Remarks			
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)						
									20	40	60		80		
		Asphalt layer at surface													
	198	Brown coarse to fine SAND, some coarse to fine Gravel, trace Silt; possible fill	5	S-1	12	Moist	3-2-4-7 (6)								0 ppm Mottled
	193	Gray-brown-black coarse to fine SAND; some medium to fine Gravel, little Silt; possible fill	10	S-2	13	Wet	8-16-12-18 (28)								0 ppm Grinding at 11'
	188	Gray-tan-brown coarse to fine Sand, some coarse to fine Gravel, some Silt; Glacial till	15	S-3	17		14-18-20-27 (38)								0 ppm
	183	Gray-brown coarse to fine Sand, some Silt, little medium to fine Gravel; Glacial till	20	S-4	17		20-24-29-31 (53)								0 ppm Grinding at 23.25'
	178	12" Dark gray, very hard, moderately weathered gneiss, over 22" gray friable, moderately strong and weathered mica schist	25	R-1				06:31						Start core at 25'	
								06:21							
								04:19							
								03:45							
								03:07							
	173	BORING COMPLETED AT 30± FEET		30											

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	205.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH	15.0 ft		AT END OF DRILLING
FINAL DEPTH	32.0± ft		24 HR AFTER DRILLING

[illegible]

PROJECT NAME EPR Parking Garage
PROJECT NO. 14228
DATE STARTED 08-19-2025 **COMPLETED** 08-19-2025
DRILLING CONTRACTOR AARCO
SAMPLER 2-inch split spoon
EQUIPMENT Mobile B29
DRILLING FOREMAN Selvin **HELPER** Julio
LOGGED BY N. Canto **CHECKED BY** C. Zwingle
LATITUDE 41.026564 **LONGITUDE** -73.769964

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88 **GROUND ELEVATION** 209.0±
DRILLING METHOD Mud Rotary
SAMPLE HAMMER Auto
AUGER INNER DIAMETER _____ **OUTER DIAMETER** _____
ROTARY BIT DIAMETER 3.88 in **DEPTH TO GROUNDWATER:** _____
CASING DIAMETER 4.00 in **AT TIME OF DRILLING** _____
CASING DEPTH 15.0 ft **AT END OF DRILLING** _____
FINAL DEPTH 65.0± ft **24 HR AFTER DRILLING** _____

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data								Remarks		
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)					
									20	40	60		80	
	204	Gray-brown coarse to fine Sand, some fine Gravel, little Silt; possible fill	5	S-1		15	Wet	20-11-10-13 (21)						6.2 ppm Petroleum like odor
	199	Gray-brown coarse to fine SAND, little medium to fine Gravel, trace Silt; possible fill	10	S-2		9	Wet	17-16-16-15 (32)						3.3 ppm Slight petroleum odor
	194	Gray-brown coarse to fine Gravel, and coarse to fine Sand, trace Silt	15	S-3		6	Wet	50-50/1"						Rock in spoon tip 2.1 ppm Slight petroleum odor
	189	Tan-brown coarse to fine Gravel, some medium to fine Sand, little Silt; highly weathered Granite bedrock	20	S-4		6	Wet	70-50/0"/6"						Slow drilling to 20' 70 ppm
	184	Gray-white coarse to fine SAND, trace fine Gravel, trace Silt; highly weathered Granite bedrock	25	S-5		1		50/1"						Slow drilling to 25' 1.2 ppm
	179	Tan-brown coarse to fine SAND, little fine Gravel, trace Silt; highly weathered Granite bedrock	30	S-6		1		50/1"						Grinding at 29-30' 1 ppm

PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	211.0±
DRILLING METHOD	Mud Rotary		
SAMPLE HAMMER	Auto		
AUGER INNER DIAMETER		OUTER DIAMETER	
ROTARY BIT DIAMETER	3.88 in	DEPTH TO GROUNDWATER:	
CASING DIAMETER	4.00 in		AT TIME OF DRILLING
CASING DEPTH	15.0 ft		AT END OF DRILLING
FINAL DEPTH	28.0± ft		24 HR AFTER DRILLING

[illegible]

PROJECT NAME EPR Parking Garage
PROJECT NO. 14228
DATE STARTED 08-19-2025 **COMPLETED** 08-19-2025
DRILLING CONTRACTOR AARCO
SAMPLER 2-inch split spoon
EQUIPMENT Mobile B29
DRILLING FOREMAN Selvin **HELPER** Julio
LOGGED BY N. Canto **CHECKED BY** C. Zwingle
LATITUDE 41.026492 **LONGITUDE** -73.769722

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88 **GROUND ELEVATION** 210.0±
DRILLING METHOD Mud Rotary
SAMPLE HAMMER Auto
AUGER INNER DIAMETER _____ **OUTER DIAMETER** _____
ROTARY BIT DIAMETER 3.88 in **DEPTH TO GROUNDWATER:** _____
CASING DIAMETER 4.00 in **AT TIME OF DRILLING** _____
CASING DEPTH 15.0 ft **AT END OF DRILLING** _____
FINAL DEPTH 39.0± ft **24 HR AFTER DRILLING** _____

Material Symbol	EL (ft)	Sample Description	Depth (ft)	Sample Data							Remarks				
				Number	Type	Rec. (in)	Moisture	Blows/6-in Core time/ft	N-Value (Blows/ft)						
									20	40		60	80		
		Asphalt layer at surface													
	205	Fill: Orange-brown coarse to fine SAND, some medium to fine Gravel, trace Silt	5	S-1		14	Wet	6-7-7-6 (14)							0 ppm
	200	Gray coarse to fine SAND, little medium to fine Gravel, little Silt; Glacial till	10	S-2		11	Moist	10-17-18-13 (35)							0.3 ppm
	195	Gray coarse to fine SAND, little fine Gravel, trace Silt; Glacial till	15	S-3		13	Moist	17-37-42-35 (79)							0.6 ppm
	190	Gray coarse to fine SAND, some coarse to fine Gravel, little Silt; Glacial till	20	S-4		10	Moist	37-55-60/3"							0 ppm
	185	Gray coarse to fine SAND, trace fine Gravel, trace Silt; very decomposed Granite bedrock	25	S-5		3	Wet	35-50/1"							0.9 ppm
	180	Brown-white coarse to fine Sand, and Silt, trace fine Gravel; very decomposed Granite bedrock	30	S-6		2		70/3"							1 ppm
		RC-1 Brown, tan, very coarse grained, hard, moderately weathered granite bedrock						02:42							Hard grinding at 33' Rotary refusal at 34' Attempt rock core at 34' NQ Core barrel

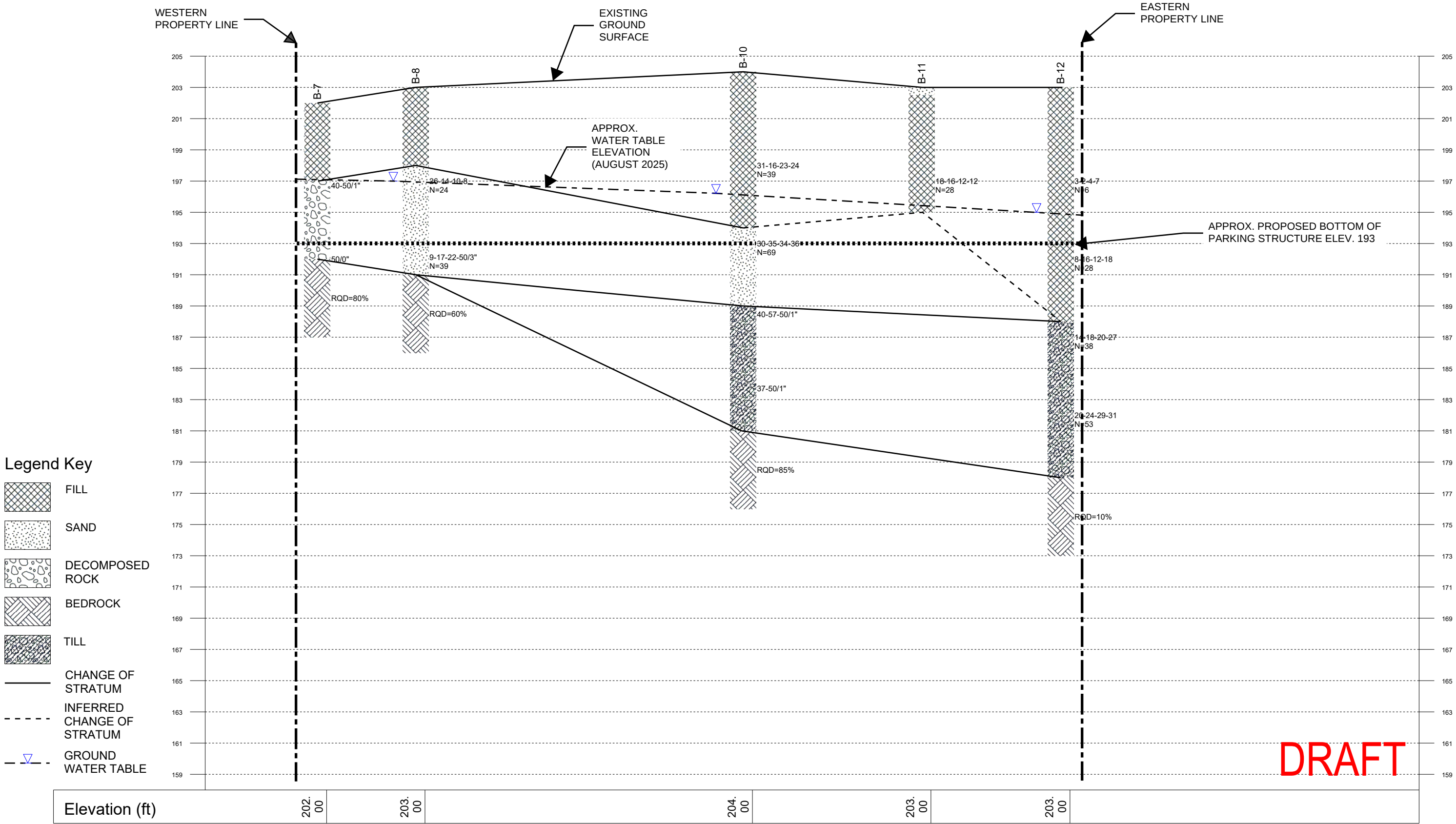
PROJECT LOCATION	40 East Post Road, White Plains, NY		
ELEVATION DATUM	NAVD-88	GROUND ELEVATION	210.0 ft ±

[illegible]

APPENDIX

Geologic Cross Sections (total of 5)

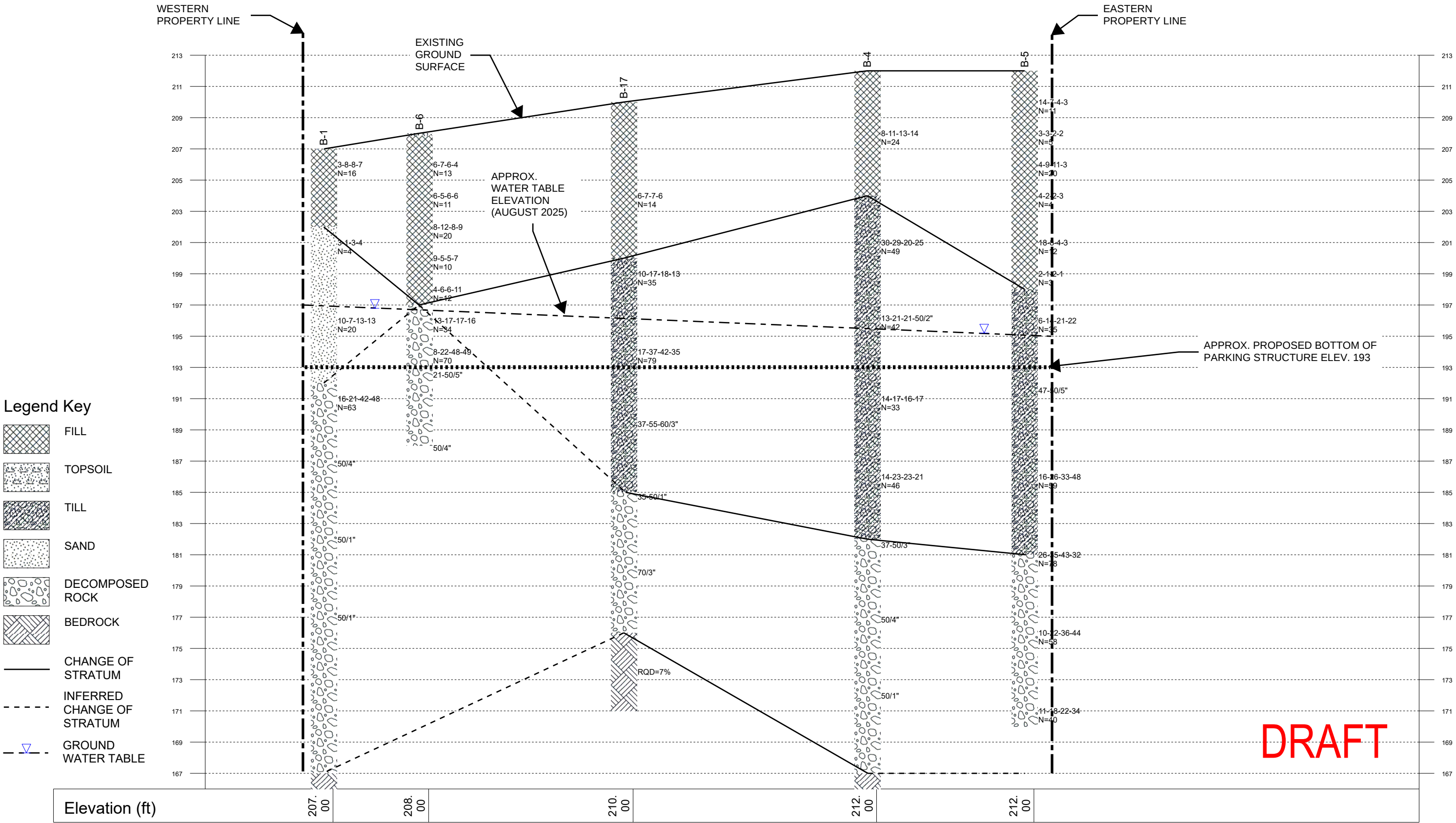
PROJECT NAME	EPR Parking Garage	PROJECT LOCATION	40 East Post Road, White Plains, NY	VERTICAL SCALE	1"=6'
PROJECT NO.	14228	ELEVATION DATUM	NAVD-88	HORIZONTAL SCALE	1"=60'



PROJECT NAME EPR Parking Garage
PROJECT NO. 14228

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88

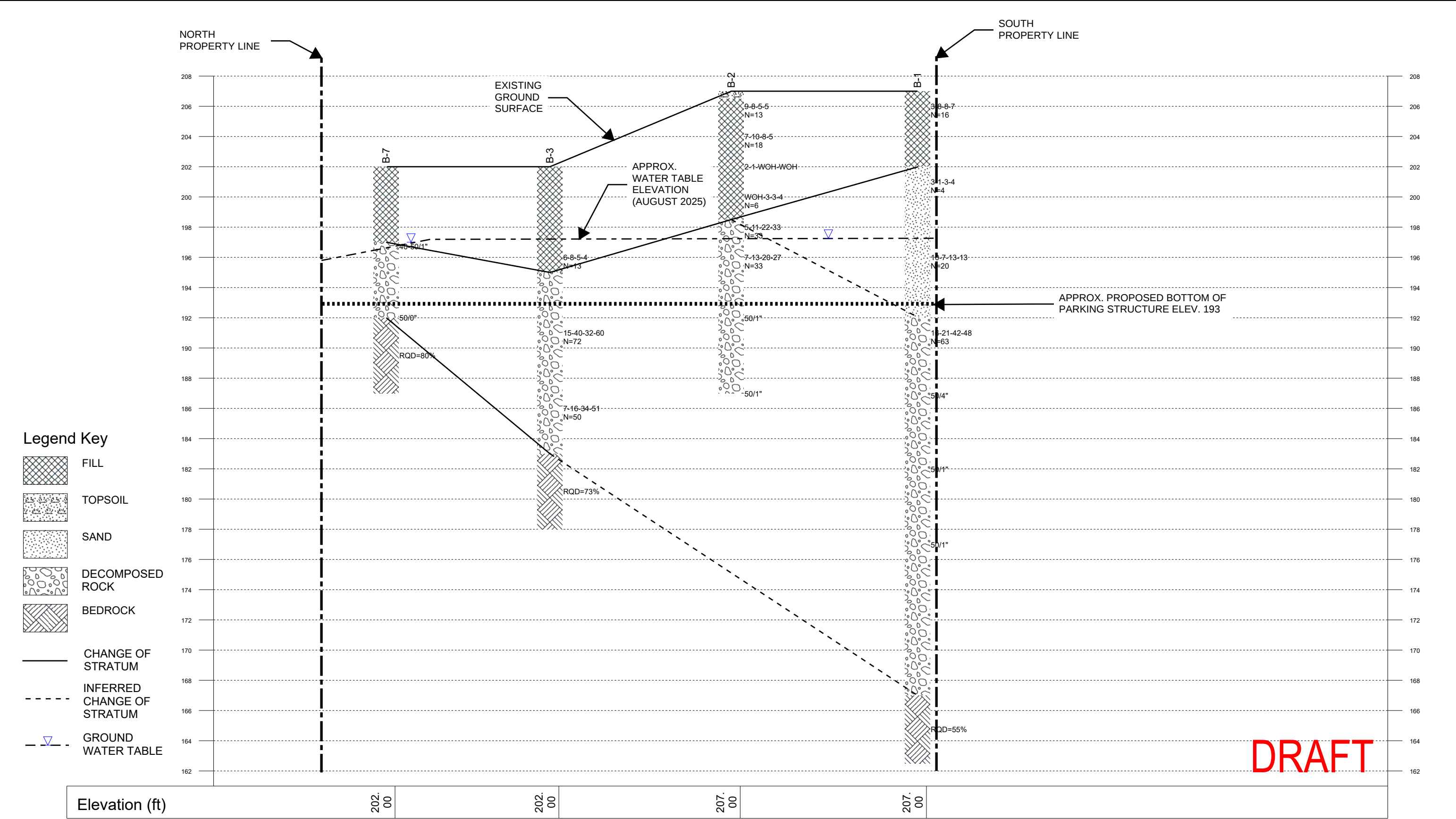
VERTICAL SCALE 1"=6'
HORIZONTAL SCALE 1"=60'



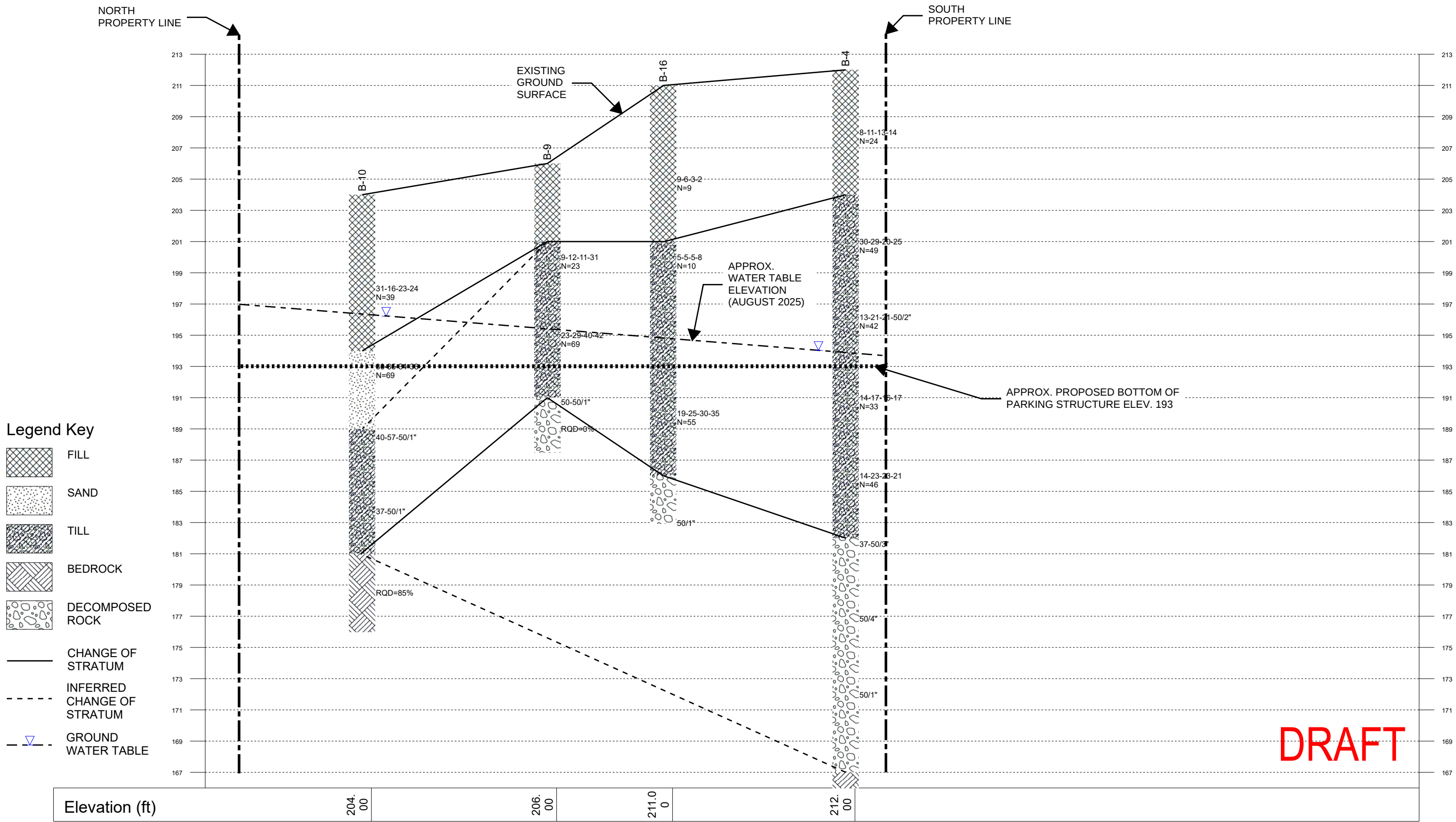
PROJECT NAME EPR Parking Garage
PROJECT NO. 14228

PROJECT LOCATION 40 East Post Road, White Plains, NY
ELEVATION DATUM NAVD-88

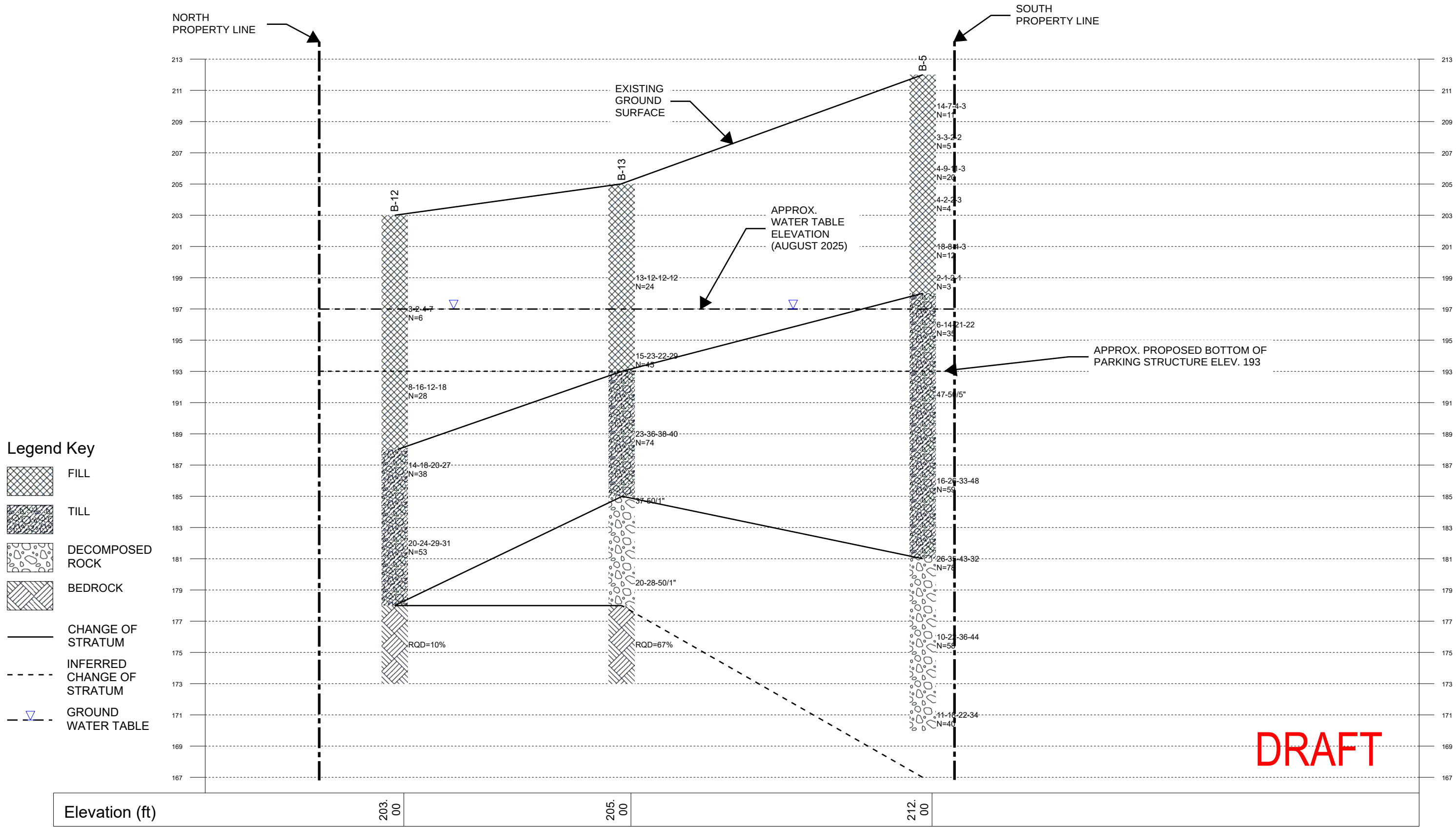
VERTICAL SCALE 1"=6'
HORIZONTAL SCALE 1"=30'



PROJECT NAME	EPR Parking Garage	PROJECT LOCATION	40 East Post Road, White Plains, NY	VERTICAL SCALE	1"=6'
PROJECT NO.	14228	ELEVATION DATUM	NAVD-88	HORIZONTAL SCALE	1"=30'



PROJECT NAME	EPR Parking Garage	PROJECT LOCATION	40 East Post Road, White Plains, NY	VERTICAL SCALE	1"=6'
PROJECT NO.	14228	ELEVATION DATUM	NAVD-88	HORIZONTAL SCALE	1"=30'



APPENDIX C

**Boring Logs Excerpted From McLaren Geotechnical Report
May 7, 2020**



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BORING NUMBER B-1

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/16/20 COMPLETED 4/16/20

GROUND ELEVATION 209 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 9.00 ft / Elev 200.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					0.8 9" ASPHALT	208.3
	SS 1	42	15-7-10-12 (17)	SM		(SM) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
	SS 2	58	10-14-15- 36 (29)	SM		(SM) MEDIUM DENSE GRAY AND BROWN DENSE FINE TO MEDIUM SAND, TRACE SILT	206.0
5	SS 3	67	22-27-25- 20 (52)	SM		(SM) VERY DENSE GRAY AND BROWN FINE TO MEDIUM SAND, TRACE SILT	
	SS 4	13	50	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
10	SS 5	42	29-50-50 (100)	SP		▽ (SP) VERY DENSE GRAY AND BROWN FINE SAND, TRACE DECOMPOSED ROCK	
				SP			
15	SS 6	8	50	SP		(SP) VERY DENSE GRAY FINE SAND, TRACE SILT	
				SP			
20	SS 7	8	100			20.0 VERY DENSE GRAY FINE SAND AND DECOMPOSED ROCK	189.0
						AUGER REFUSAL AT 22.0 FT	
						22.0	187.0

Refusal at 22.0 feet.
Bottom of borehole at 22.0 feet.

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\2001\200113\0118 - TECHNICAL\REPORTS\GINT\GINT200113 EPR GARAGE - WPH.GPJ



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BORING NUMBER B-2

PAGE 1 OF 2

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/21/20 COMPLETED 4/21/20

GROUND ELEVATION 209 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 9.00 ft / Elev 200.00 ft

LOGGED BY ARIEL FRIAS

CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
0.7	SS 0			SM		8" CONCRETE SURFACE	208.3
	SS 1	17	12-16-30-25 (46)	SM		(SM) GRAVEL	
	SS 2	58	11-22-22-18 (44)	SM		(SM) GRAVEL SUBBASE, BROWN FINE TO MEDIUM SAND	
5	SS 3	8	100	SM		(SM) DENSE BROWN FINE TO MEDIUM SAND, TRACE FINE GRAVEL	
	SS 4	58	10-26-32-30 (58)	SM		(SM) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE FINE GRAVEL	
7.0	SS 5	58	33-31-60-33 (91)	SM		(SM) VERY DENSE BROWN FINE TO VERY FINE SAND, TRACE SILT, TRACE MEDIUM SAND	202.0
10				SP		▽ (SM) VERY DENSE BROWN FINE TO VERY FINE SAND, TRACE SILT, TRACE ROCK FRAGMENTS	
				SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME COARSE SAND, TRACE ROCK FRAGMENTS	
15	SS 6	50	40-45-50 (95)	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME COARSE SAND, TRACE ROCK FRAGMENTS	
20	SS 7	17	50-50	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, ROCK FRAGMENTS	
25	SS 8	38	60-50	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE FINE GRAVEL, TRACE ROCK FRAGMENTS	
30	SS 9	17	50			DECOMPOSED ROCK	179.0
	RC 1					BEDROCK (WEATHERED) - RC1	
35						1ST FT/1MIN 2ND FT/2MIN 3RD FT/2MIN	

(Continued Next Page)



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BORING NUMBER B-2

PAGE 2 OF 2

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
35						
	RC 2	100 (90)				BEDROCK - RC2: RQD = 90% 1ST FT/2MIN 2ND FT/2MIN 3RD FT/2MIN 4TH FT/2MIN 5TH FT/2MIN (continued)

39.0

170.0

Refusal at 31.0 feet.
Bottom of borehole at 39.0 feet.



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BORING NUMBER B-3

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/27/20

COMPLETED 4/27/20

GROUND ELEVATION 209 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 9.00 ft / Elev 200.00 ft

LOGGED BY ARIEL FRIAS

CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 18:21 - P:\PROJ\2001\200113.01\8 - TECHNICAL\REPORTS\GEO\TECH\GINT\200113 EPR GARAGE - WPH.GPJ

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0			SM		6" BRICK SURFACE	208.5
	SS 1	17	100			(SM) VERY DENSE GRAY FINE TO MEDIUM SAND, SOME FINE GRAVEL	
	SS 2	42	5-6-6-14 (12)	SM		(SM) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, ROCK FRAGMENTS	
5	SS 3	42	10-9-7-9 (16)	SM		(SM) MEDIUM DENSE BROWN VERY FINE TO FINE SAND, TRACE SILT, TRACE FINE GRAVEL	
	SS 4	50	10-9-15-24 (24)	SM		(SM) MEDIUM DENSE BROWN MEDIUM TO COARSE SAND AND GRAY VERY FINE TO FINE SAND, TRACE SILT	
	SS 5	83	18-23-49- 50 (72)	SM		(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, TRACE SILT, SOME COARSE SAND	201.0
10	SS 6	50	29-39-48- 65 (87)	SM		(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, TRACE SILT	
						(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, TRACE SILT	
15	SS 7	42	60-75-50 (125)	SM			
						(SP) VERY DENSE GRAY FINE TO MEDIUM SAND, TRACE COARSE SAND	
20	SS 8	33	41-50	SP			
						(SP) VERY DENSE GRAY FINE TO MEDIUM SAND, DECOMPOSED ROCK	
25	SS 9	13	100			DECOMPOSED ROCK	184.0
	RC 1	38 (0)				BEDROCK - RC1	
30							179.0

Refusal at 30.0 feet.
Bottom of borehole at 30.0 feet.



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BORING NUMBER B-4

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/15/20 COMPLETED 4/15/20

GROUND ELEVATION 206 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

∇ AT TIME OF DRILLING 5.00 ft / Elev 201.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\2001\200113\0118 - TECHNICAL\REPORTS\GEO\TECH\GINT\200113 EPR GARAGE - WPH.GPJ

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					8" CONCRETE WALKWAY	206.3
	SS 1	21	50			GRAVEL GRAVEL SUBBASE WITH WHITE CONCRETE POWDER	
							203.0
5	SS 2	33	11-15-18- 19 (33)	SP		(SP) DENSE GRAY FINE TO MEDIUM SAND, TRACE FINE GRAVEL	
	SS 3	50	11-12-14- 14 (26)	SP		∇ (SP) MEDIUM DENSE GRAY FINE TO MEDIUM SAND, SOME COARSE SAND	
	SS 4	58	11-22-33- 26 (55)	SP		(SP) VERY DENSE GRAY FINE TO MEDIUM SAND	
10	SS 5	38	39-50			(SP) VERY DENSE GRAY FINE TO MEDIUM SAND, DECOMPOSED ROCK	
				SP			
15							191.0
	SS 6	8	50			(SP) DECOMPOSED ROCK, SOME FINE TO MEDIUM SAND	
				SP			
20							
	SS 7	4	50			SAME AS ABOVE AUGER REFUSAL AT 20.5 FT	
	RC 1	100 (90)				BEDROCK - RC1: RQD = 90% 1ST FT/1 MIN 2ND FT/1 MIN 3RD FT/1 MIN 4TH FT/1 MIN 5TH FT/1 MIN	
25							180.5

Refusal at 25.5 feet.
Bottom of borehole at 25.5 feet.



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BORING NUMBER B-6

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/24/20 COMPLETED 4/24/20

GROUND ELEVATION 203 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

∇ AT TIME OF DRILLING 7.00 ft / Elev 196.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 1	50	11-24-25-16 (49)	SP		6" ASPHALT	202.5
	SS 2	67	19-21-41-40 (62)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND	
	SS 3	67	12-11-9-10 (20)	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	
5	SS 4	50	20-13-24-38 (37)	SM		(SP) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
	SS 5	42	60-50	SM		(SM) MEDIUM DENSE BROWNISH GRAY FINE TO MEDIUM SAND, SOME VERY FINE SAND, TRACE SILT	197.0
10				SM		(SM) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME VERY FINE SAND, TRACE SILT	
				SM		(SM) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME VERY FINE SAND, TRACE SILT	
15	SS 6	42	48-50	SM		(SM) VERY DENSE BROWNISH GRAY VERY FINE TO FINE SAND, TRACE SILT, ROCK FRAGMENTS	
						AUGER REFUSAL AT 19 FT	184.0

Refusal at 19.0 feet.
Bottom of borehole at 19.0 feet.

GENERAL BH / TP / WELL - CINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\2001\200113\018_TECHNICALREPORTS\GEOTECH\200113 EPR GARAGE - WPH.GPJ



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BORING NUMBER B-7

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/23/20

COMPLETED 4/23/20

GROUND ELEVATION 203 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 7.00 ft / Elev 196.00 ft

LOGGED BY ARIEL FRIAS

CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

GENERAL BH / TP / WELL - GINT STD US LAB GDT - 5/1/20 16:21 - P:\PROJ\200113\018 - TECHNICAL\REPORTS\GEOTECH\GINT\200113 EPR GARAGE - WPH.GPJ

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					0.8 8" ASPHALT SURFACE	202.3
	SS 1	58	25-14-21-18 (35)	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, BRICK FRAGMENTS	
	SS 2	58	9-7-8-26 (15)	SP		(SP) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
5	SS 3	58	13-18-16-17 (34)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
	SS 4	67	27-33-17-15 (50)	SP		▽ (SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
10	SS 5	83	10-11-22-40 (33)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND, TRACE FINE GRAVEL	
						(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, TRACE SILT	
15				SM			
	SS 6	58	24-36-60-50 (96)			(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, SOME SILT	
20							
	SS 7	50	30-65-50 (115)	SM			
25							
	SS 8	100	100/2"	SM		(SM) VERY DENSE GRAY VERY FINE TO FINE SAND, SOME SILT	
						DECOMPOSED ROCK	
						AUGER REFUSAL AT 26 FT	

Refusal at 26.0 feet.
Bottom of borehole at 26.0 feet.



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BORING NUMBER B-8

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/24/20 COMPLETED 4/24/20

GROUND ELEVATION 205 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 7.00 ft / Elev 198.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					7" ASPHALT SURFACE	204.4
	SS 1	50	6-17-23-32 (40)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND, ROCK FRAGMENTS	
	SS 2	33	17-22-27- 35 (49)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT	
5	SS 3	58	13-17-49- 26 (66)	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND AND GRAY VERY FINE TO FINE SAND	
	SS 4	58	17-1-50 (51)	SM		(SM) VERY DENSE BROWNISH GRAY VERY FINE TO FINE SAND, SOME MEDIUM SAND, TRACE SILT	198.0
10	SS 5	0	50	SM		(SM) NO RECOVERY, SAME AS ABOVE AT SPOON TIP AUGER REFUSAL AT 10.5 FT	194.5

Refusal at 10.5 feet.
Bottom of borehole at 10.5 feet.

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\200\200113\0118 TECHNICALREPORTS\GEO\TECHGINT\200113 EPR GARAGE - WPH.GPJ



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BORING NUMBER B-10

PAGE 1 OF 1

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/16/20 COMPLETED 4/20/20

GROUND ELEVATION 208 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 5.00 ft / Elev 203.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

GENERAL BH / TP / WELL - GINT STD US LAB GDT - 5/1/20 1621 - P:\PROJ\200113\13.018 TECHNICAL\REPORTS\GEOTECH\GINT\200113 EPR GARAGE - WPH.GPJ

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 1	25	6-9-6-6 (15)	SP		6" CONCRETE SIDEWALK	207.5
	SS 2	33	6-3-2-2 (5)	SP		(SP) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, TRACE FINE GRAVEL	
	SS 3	17	4-12-21-12 (33)	SP		(SP) LOOSE BROWN FINE TO MEDIUM SAND, TRACE ROCK FRAGMENTS	
	SS 4	17	11-11-15-50 (26)	SP		▽ (SP) DENSE BROWN FINE TO MEDIUM SAND, ROCK FRAGMENTS	
						(SP) DENSE BROWN FINE TO MEDIUM SAND, ROCK FRAGMENTS	
						DRILLED DOWN (BOULDER)	200.0
10	SS 5	17	8-47-50 (97)	SM		(SM) VERY DENSE GRAY VERY FINE SAND, TRACE SILT	198.0
						(SM) VERY DENSE GRAY VERY FINE SAND, LITTLE SILT	
15	SS 6	25	76-50	SM			
						(SP) GRAY VERY FINE TO FINE SAND TRACE TO LITTLE SILT	
20	SS 7	38	49-50	SP		(SP) NO RECOVERY, ROCK FRAGMENTS AT THE TIP OF THE SPOON	
						(SP) BROWN TO LIGHT BROWN FINE TO MEDIUM SAND, DECOMPOSED ROCK	
25	SS 8	100	50/2"	SP		BEDROCK, AUGER REFUSAL	182.0

Refusal at 26.0 feet.
Bottom of borehole at 26.0 feet.



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BORING NUMBER B-12

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CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/16/20 COMPLETED 4/16/20

GROUND ELEVATION 208 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

∇ AT TIME OF DRILLING 7.00 ft / Elev 201.00 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\2001\200113\0118 - TECHNICAL\REPORTS\GEO\TECHGINT\200113 EPR GARAGE - WPH.GPJ

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					9" CONCRETE SIDEWALK	207.3
	SS 1	17	6-10-5-4 (15)	SP		(SP) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	
	SS 2	33	4-4-6-15 (10)	SP		(SP) LOOSE TO MEDIUM DENSE BROWN FINE TO MEDIUM SAND, SOME SILT	
5	SS 3	42	12-14-15-27 (29)	SM		(SM) MEDIUM DENSE BROWN AND GRAY FINE TO MEDIUM SAND, TRACE SILT	203.0
	SS 4	50	33-18-23-23 (41)	SM		∇ (SM) DENSE BROWN FINE SAND, SOME FINE GRAVEL	
10	SS 5	75	15-17-33-50 (50)	SM		(SM) DENSE TO VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, SOME DECOMPOSED ROCK	
							11.0
				SM		(SM) DENSE TO VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, SOME DECOMPOSED ROCK	197.0
15	SS 6	100	50/5"			DECOMPOSED ROCK, AUGER REFUSAL AT 17 FT	

Refusal at 17.0 feet.
Bottom of borehole at 17.0 feet.



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BORING NUMBER B-13

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CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/23/20

COMPLETED 4/23/20

GROUND ELEVATION 203 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 8.00 ft / Elev 195.00 ft

LOGGED BY ARIEL FRIAS

CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					4" ASPHALT SURFACE	202.7
	SS 1	67	19-30-35- 38 (65)	SP		(SP) VERY DENSE DARK BROWN FINE TO MEDIUM SAND WITH FINE TO COARSE SAND, COBBLES AND SOME SILT	
	SS 2	63	26-31-50 (81)	SP		(SP) DENSE DARK BROWN FINE TO MEDIUM SAND WITH FINE TO COARSE SAND, COBBLES AND SOME SILT	
5	SS 3	67	15-18-18- 28 (36)	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, ROCK FRAGMENTS	198.0
	SS 4	63	21-32-50 (82)	SM		▽ (SM) NO RECOVERY (BROWN FINE TO MEDIUM SAND)	
10	SS 5	100	50/3"	SP		(SP) VERY DENSE BROWN FINE TO MEDIUM SAND, TRACE SILT, DECOMPOSED ROCK AUGER REFUSAL AT 10 FT	193.0

Refusal at 10.0 feet.
Bottom of borehole at 10.0 feet.

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:21 - P:\PROJ\200200113\0118 - TECHNICAL REPORTS\GEO\TECHGINT\200113 EPR GARAGE - WPH.GPJ

CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/15/20

COMPLETED 4/15/20

GROUND ELEVATION 202 ft NAVD 88 **HOLE SIZE** 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 5.00 ft / Elev 197.00 ft

LOGGED BY ARIEL FRIAS

CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 0					5" ASPHALT COVER	201.6
	SS 1	33	10-18-15 (33)	SP		(SP) DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	
	SS 2	33	10-8-7-6 (15)	SP		(SP) MEDIUM DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	
5	SS 3	17	4-3-2-9 (5)	SP		(SP) LOOSE BROWN FINE TO MEDIUM SAND, SOME GRAVEL, ROCK FRAGMENTS	
	SS 4	17	8-50	SP		(SP) VERY DENSE GRAY FINE TO MEDIUM SAND, ROCK FRAGMENTS	
	SS 5	50	20-45-50 (95)	SM		(SM) VERY DENSE GRAY FINE TO MEDIUM SAND, TRACE SILT, TRACE DECOMPOSED ROCK	194.0
10	SS 6	13	50	SM		(SM) VERY DENSE GRAY FINE TO MEDIUM SAND, TRACE SILT, TRACE DECOMPOSED ROCK	191.5
	RC 1	100 (95)				BEDROCK - RC1: RQD = 95% 1ST FT/1 MIN 2ND FT/1 MIN 3RD FT/1 MIN 4TH FT/1 MIN 5TH FT/1 MIN	
15	RC 2					BEDROCK - RC2 1ST FT/1 MIN 2ND FT/1 MIN	184.5

Refusal at 10.5 feet.
Bottom of borehole at 17.5 feet.

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 5/1/20 16:24 - P:\PROJ\2001\200113.01\8_TECHNICAL\REPORTS\GEO\TECH\GINT\200113 EPR GARAGE - WPH.GPJ



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BORING NUMBER B-15

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CLIENT WHITE PLAINS HOSPITAL CENTER

PROJECT NAME EAST POST ROAD PARKING GARAGE

PROJECT NUMBER 200113

PROJECT LOCATION WHITE PLAINS, NY

DATE STARTED 4/23/20 COMPLETED 4/23/20

GROUND ELEVATION 202 ft NAVD 88 HOLE SIZE 2 inches

DRILLING CONTRACTOR SOILTESTING, INC.

GROUND WATER LEVELS:

DRILLING METHOD HOLLOW STEM AUGER

▽ AT TIME OF DRILLING 4.50 ft / Elev 197.50 ft

LOGGED BY ARIEL FRIAS CHECKED BY LUKE A. DAUR

AT END OF DRILLING ---

NOTES

AFTER DRILLING ---

DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0							
	SS 1	58	14-36-36- 26 (72)	SP		4" ASPHALT SURFACE (SP) VERY DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	201.7
	SS 2	50	33-38-30- 25 (68)	SP		(SP) SAME AS ABOVE	
5	SS 3	25	8-10-28-14 (38)	SP		▽ (SP) DENSE BROWN FINE TO MEDIUM SAND, SOME FINE GRAVEL	
	SS 4	100	50/4"	SP		(SP) VERY DENSE GRAY AND BROWN FINE TO MEDIUM SAND, DECOMPOSED ROCK AUGER REFUSAL AT 7.5 FT	196.0 194.5

Refusal at 7.5 feet.
Bottom of borehole at 7.5 feet.