

### BARRON & ASSOCIATES, P.C.

#### **Geotechnical Consulting and Special Inspections**

10440 Main Street Clarence, NY 14031 (716) 759-7821 www.barronandassociatespc.com

April 8, 2024 Job No: 23-610

Uniland Construction Corporation University Corporate Centre 100 Corporate Parkway, Suite 500 Amherst, New York 14226

ATTN: Mr. David J. Reilly

RE: Geotechnical Engineering Report

Proposed Industrial Complex

2 Bailey Avenue

City of Buffalo, Erie Co., New York 14206

Mr. Reilly:

This report presents the findings of the subsurface investigation program and geotechnical engineering recommendations for the above referenced project. The geographic orientation of the project site is illustrated on the U.S. Geologic Survey (USGS) site location map in Figure No. 1. The project site is illustrated in Figure No. 2, entitled "Test Boring and Monitoring Well Location Plan", which includes: the approximate location of 30 test borings and three monitoring wells that were drilled by Buffalo Drilling Company, Inc. (BDC); relative ground surface elevations; and additional site details.

#### **EXPLORATION METHODS**

<u>Sampling Method</u>: An ATV-mounted CME-550 rotary drill rig was used to drill 30 borings each to auger refusal using 2-1/4 inch inside diameter (ID), continuous flight hollow stem augers. Samples were recovered by driving a standard split-spoon sampler (2-foot long by 1-3/8 inch inside diameter) 24 inches with a 140-pound hammer falling 30 inches per blow per the American Society of Testing and Materials (ASTM) Standard D1586. The number of blows from six to 18 inches of penetration is defined as the Standard Penetration Test (SPT) N-value. Auger refusal, which may infer the top of bedrock, was encountered at each boring, from approximately nine to 21 feet below ground surface.

Three groundwater observation wells were installed at test borings B-1, B-25, and B-30 in general accordance with ASTM Method D5092. The observation wells were constructed with two-inch inside diameter, flush joint threaded schedule 40 PVC screen and riser pipe. The well screen was a number 10 slot (i.e., 0.010-inch-wide) and backfilled with a uniformly graded silicon sand (i.e., Ricci No. 00N) that was placed to approximately one foot above the top of the screen. The well was sealed with a two-foot minimum thickness of bentonite chips. The remaining borehole annulus was sealed to ground surface with auger cuttings. A locking J-plug was installed at the top of each well. For convenience, the boring/well combination will be referred to as boring or well B/MW-1, B/MW-25, and B/MW-30.

Job No: 23-610

<u>Classification</u>: The retrieved soil and fill samples were initially logged in the field by the driller, and a portion of each sample was placed and sealed in a glass jar. The boring logs and monitoring well schematics, which are included in Appendix A, were based upon the field logs and a second visual classification of recovered samples in the laboratory by a geologist. Classification/identification of samples, as noted on the boring logs, is based on the Unified Soil Classification System (USCS) in ASTM D2487/D2488. Refer to Appendix B entitled, "Geotechnical Reference Standards", for an explanation of the terminology that is used for soil and rock descriptions.

<u>Laboratory Testing</u>: Laboratory soil testing was undertaken on several retrieved split spoon samples. The overall laboratory testing program consisted of the following test methods:

Particle Size Analysis
 ASTM D421

Water (Moisture) Content of Soil
 ASTM D2216

Liquid Limit, Plastic Limit, and Plasticity Index of Soils
 ASTM D4318

Table No. 1 is a summary of subsurface exploration and Table No. 2 presents the tabulated results of the physical/soil index properties. The associated graphical illustration of the data is included in Appendix C.

#### SITE AND SUBSURFACE CONDITIONS

General: The proposed site for development is a 53.7 ± acre former railroad yard, addressed as 2 Bailey Avenue, in the City of Buffalo, Erie County, New York. The project will include two large footprint garage-type structures, 120,000 and 180,000 ± square-foot respectively, auto impound building, salt barn, solar farm, several laydown/storage yard areas, and associated asphalt pavement parking and roadway areas. The site topography is generally slopes to the southeast with ground surface elevations varying by about nine feet across the boring locations. A mounded area exists at the south-central portion of the site and will likely require leveling for the Broadway Streets & Sanitation Garage building pad.

<u>Subsurface Soil Conditions</u>: In general, subsurface conditions, underlying a very thin, indistinguishable topsoil layer, consists of three to eight feet of fill material atop naturally deposited, lacustrine silty clay which mantles cohesive glacial till that extends to auger refusal.

Beneath the topsoil, primarily granular fill material was encountered and extends three eight feet below ground surface. This fill consists of fine to coarse-grained sand and gravel with little to trace amounts of silt, clay, slag, concrete, brick, construction lumber, and roots/organic matter. Relative density of the fill is generally medium dense with pockets of both loose and dense material. Moisture contents of retrieved samples are in a moist to wet state. This material appears to be locally imported or site-derived soil that was reworked and placed during the operation of the railway. In general, fill materials are typically placed in uncontrolled and uncompacted manners. As such, the variable strengths and erratic material

characteristics, both vertically and laterally at and below the proposed foundation bearing grades, make them unsuitable in providing reliable direct supporting media for footings/shallow foundations.

Beneath the fill, a naturally deposited silty clay exists and extends eight to 14 feet below ground surface. This soil consists of moderately plastic silty clay with trace amounts of sand and gravel. Consistency of the silty clay is primarily very stiff with moisture contents of retrieved samples in a moist state.

Beneath the lacustrine silty clay, a naturally deposited cohesive/fine-grained glacial till exists and extends to auger refusal. The till generally consists of silt and clay with lesser amounts of fine-sized to coarse-sized sand and gravel. Plasticity is noted as slightly plastic with consistency of this soil primarily being very stiff to hard. Moisture contents of retrieved samples are noted to be moist to wet.

<u>Bedrock</u>: Auger refusal, which is generally inferred to be the top of the apparent bedrock surface, was encountered at each boring approximately nine to 21 feet below ground surface. Based upon the regional geology, the bedrock type beneath the site would be the Onondaga Limestone.

Groundwater: No appreciable amounts of groundwater were encountered during or at the completion of drilling efforts at the test borings. Note that groundwater readings were taken at completion of drilling efforts and, therefore, an adequate amount of time for the groundwater level to recharge to static conditions was probably not allowed. Ground water levels where recorded after installation of the three monitoring wells, February 6, 14, and 15, 2024, and ranged from approximately 12 to 18 feet below ground surface. Development of the monitoring wells took place on March 26, 2024 and the static groundwater depths recorded approximately two to 12 feet below the ground surface. Refer to Table No. 1, "Summary of Subsurface Exploration" for actual elevations. This data suggests a south-easterly groundwater flow direction and two main groundwater tables. Firstly, the presence of the upper granular fill layer atop the less permeable silty clay creates localized, seasonal perched water table conditions and is represented at monitoring wells B/MW-1 and B/MW-25. At monitoring well B/MW-30, a second deeper water table with minor artesian flow exists near the till/bedrock interface. This groundwater condition was not apparent until fully penetrating the dense/hard glacial till. Fluctuations in the groundwater level may occur due to other factors than those present during field operations.

#### **EARTHQUAKE/SEISMIC CONSIDERATIONS**

<u>Site Class Definition</u>: For the given site conditions, the most applicable site definition is Site Class D, in accordance with Section 1613.2.2 of <u>Building Code of New York State © 2020</u> and listed in Table 20.3-1 of Chapter 20 of <u>American Society of Civil Engineers (ASCE) Standard 7-16</u>.

<u>Liquefaction Potential</u>: For the Site Class D, the design spectral response acceleration parameters  $S_{DS}$ , at 0.2 seconds, and  $S_{D1}$ , at one second, are 0.174g and 0.071g (g = 32.2 feet/sec<sup>2</sup>), respectively, for this part of Erie County, New York (see Appendix D). These values have a two percent probability of being exceeded in 50 years. Based upon the above conditions and an approximate magnitude 6.0 earthquake on the Richter Scale, the potential for liquefaction or settlement of Site Class D soil is considered low.

#### FOUNDATION DESIGN AND CONSTRUCTION RECOMMENDATIONS

<u>General</u>: This section will present and discuss recommendations on foundation design and construction and placement of controlled fills, and subgrade and base layer requirements for concrete floor slabs, paved parking and roadway areas.

As shown in Figure No. 2, the proposed development will include two large footprint garage-type structures, 120,000 and  $180,000 \pm \text{square-foot}$  respectively, auto impound building, salt barn, solar farm, several laydown/storage yard areas, and associated asphalt pavement parking and roadway areas. The existing ground surface generally slopes to the southeast with ground surface elevations varying by about nine feet across the boring locations. Finish floor elevations for the buildings and grading plans have yet to be finalized. For the purpose of this report, those elevations are assumed close to  $603.5 \pm \text{feet}$  to minimize potential earthwork. The final exterior grades are generally assumed about one-half foot below finished floor at elevation,  $603.0 \pm \text{feet}$ . Based upon the conceptual understanding of buildings planned to be constructed maximum design bearing wall and column loads are not expected to exceed 15 kips per lineal foot and 200 kips, respectively. The maximum live floor load is taken as 150 pounds per square foot (psf).

Site Preparation and Earthwork: General site preparation will include the demolition and complete removal of any encountered foundations and floor slabs, and removing all unsuitable surficial material (i.e., asphalt pavement, concrete, brick, expansive slag, organic or topsoil layer, and construction and demolition-like fill) to a depth where firm, granular or cohesive fills or naturally occurring soils are encountered. It is recommended, particularly in the location of the proposed structure, that any slag and/or cinder/ash containing material, if encountered, should be tested to determine the expansion and corrosion characteristics (where applicable for direct or potential contact with foundation elements and buried utilities) of these materials prior to their use on-site. Any rubble-like and brick fill, boulders, or wood fill in particular, if encountered, will require undercutting. The proposed building pads and any paved areas are to be proof rolled with a fully loaded ten-wheel dump truck. All encountered soft and disturbed zones should be undercut and stabilized with granular fill that is placed in compacted lifts prior to placement of additional fill materials above.

Refer to Appendix E entitled "General Earthwork Specification" for definition of the fill types and gradations, recommended minimum compaction requirements for various site developments, and placement and compaction methods. The NYS Department of Transportation (NYSDOT) specification numbers for typical aggregate subbase/base course components that are included in the select granular fill category as are as follows: Item No. 304.12 (Metric) Subbase Course, Type 2 (< 2-inch maximum size), which is preferred; or Item No. 304.14 (Metric) Subbase Course, Type 4 (< 2-inch maximum size).

It is unknown if any former residential/commercial structures existed at the project site or if any former footings and foundation walls may be buried. Septic tank and system locations and the abandonment conditions or practices are normally unknown. If encountered during site development, it is recommended that any tanks and systems should be properly removed/treated/remediated relative to the proposed development and under the applicable local and state regulations. The remaining cavities, from the aforementioned items, should be backfilled with select/approved granular fill that is placed in thin lifts and compacted to the minimum recommendation, as presented in this report, for the proposed development at that cavity location.

<u>Shallow Foundation</u>: The recommended foundation type is shallow footings. Continuous strip and spread exterior footings and interior spread footings would bear at an elevation of four feet or lower below final grade, at relative elevation  $599.00 \pm \text{feet}$ . A four-foot minimum footing depth (as required by local or New York State code) is needed to provide adequate protection from frost for exterior footings.

The exterior and interior footings are recommended to bear at the same relative elevation on stable, naturally deposited very stiff (Nvalues 15 blows or greater) silty clay or on thin layers of thoroughly compacted (minimum of 95 percent of the maximum dry density by ASTM D 1557) select/approved granular fill that is placed on an approved subgrade. If needed, a geotextile strength and filter fabric (such as, Mirafi 600X) may be used to line the excavation bottom for the purpose of stabilizing the excavation and placement of the select/approved granular fill.

Minimum one to four foot undercuts will be necessary within portions of all building footprints to remove random fill material and/or softer natural soils that is unsuitable for direct foundation bearing. The unsuitable fills/soils vary in both vertical and lateral distribution. See Appendix D, "Profile of Borings" for schematic and additional details. Undercut areas beneath proposed foundations must extend laterally beyond each vertically projected edge of the foundation by a minimum distance equal to one-half the total depth of the undercut or equating to a slope of two vertical to one horizontal from the bottom foundation corner. The undercuts and placement of compacted select/approved granular fill are required to ensure a suitable and more uniform bearing media for the footings, and to prevent unacceptable differential settlements.

Wall footings should have a minimum two-foot width and column footings should have a minimum three-foot width. Based on the above described conditions, the recommended maximum net allowable foundation bearing pressure is 3,000 pounds per square foot (psf) of bearing area. All footings for the proposed expansion are recommended to be designed near the same contact pressure. The recommended maximum net allowable foundation bearing pressure is based on generally accepted design methods for cohesive soil conditions. Based on the provisions of the above recommendations and estimated design requirements and utilization of proper construction procedures and experienced field supervision and testing personnel, total and differential settlements are estimated to be less than one inch and 3/4's inch, respectively. Refer to Appendix D for additional foundation design and construction details.

<u>Additional Foundation Considerations</u>: In addition to the above, the following recommendations will provide additional assurances with regard to proper foundation construction.

- a) All fill placed beneath, adjacent, or above foundations must comply with the "General Earthwork Specification", included as Appendix E.
- b) Backfill of foundations with approved select granular fill must be completed prior to placement of substantial superstructure loads, except for basement walls or substructure areas that may additionally require superstructure loads and possibly internal bracing.
- c) The upper silty materials may rut and "pump" if exposed to excessive surface water and repeated construction traffic. Proper site management and fill placement operations are needed to minimize costly undercuts and subgrade repairs prior to placement of concrete slabs and asphalt pavement. It is noted that construction during wet/rainy and/or Fall and Spring conditions may require added precautions and possibly a thicker base layer to maintain a stable subgrade condition.
- d) Step footings, if utilized, should have a rise to run ratio of 1:2, with a two-foot maximum rise and a four-foot minimum run between steps or as recommended by the design structural engineer.
- e) The recommended at rest (rigid wall), active, and passive static earth pressure coefficients for unsaturated, select granular sandy gravel fill against an earth retaining structure/wall are 0.76, 0.24, and 2.8 (with an ultimate value of 4.2), respectively. The respective equivalent static lateral fluid pressures are recommended to be 90, 30 and 330 (with an ultimate value of 500) pounds per square foot (psf) per foot depth which are based upon a moist, compacted unit backfill weight of 120 pounds per cubic foot (pcf). The at rest value would account for the average expected compaction induced stresses and/or the potential influences of hydrostatic pressure. The static lateral fluid pressures can be directly proportioned for other unit weights.

<u>Stabilization of Excavations</u>: The trench/excavation sidewall stability concerns can be addressed with the Occupational Safety and Health Act (OSHA) requirements as set forth in Subpart P of 29 CFR Part 1926, Sections 1926.650 to 1926.652. In lieu of a properly designed shoring system, side slopes of the trench excavation should be one on one (vertical to horizontal distance) or flatter in cohesive soils or one on one and one-half or flatter in the granular materials, as required by OSHA.

Water must not be allowed to accumulate or pond on exposed foundation bearing grades. Surface water and groundwater from within the excavation must be either pumped, diverted or channelized by gravity flow to effectuate the construction of the proposed foundation. Pockets of localized perched groundwater may seasonally be expected to be encountered at footing and/or footing undercut bearing grades. At these locations, dewatering with surface sumps may be required to maintain stable side slopes and excavation bottom.

Concrete Interior Floor Slab(s)/Pad: For the most part and based on test results for similar soils/fills, moderately plastic cohesive natural soils/fills may be difficult to compact in a controlled manner considering the varying soil plasticity and natural moisture contents that are estimated to be wet of optimum, at the time of this investigation. Excavated and approvable cohesive soil/fill types are not expected to be available in any reasonable quantities for placement within the building pad or beneath foundation elements due to their compressibility and sensitivity to poor weather conditions during the bulk of the construction season. These soils/fills may be expected to be suitable only for re-use as general fill or beneath lightly loaded pavement areas with the implementation of a very stringent and uniformly applied soil conditioning (i.e., drying and blending) and compaction methods, and "full time" inspection and testing by qualified geotechnical personnel.

Excavated and approvable granular (i.e., sand or gravel and non-plastic silt and sand) soil/fill are expected to be available in limited quantity. However, substantial efforts will be required to dry, blend, and screen/remove of deleterious material from the fill prior to use beneath load bearing portions of the site. For the most part, granular (i.e., sand or gravel) soil/fill is expected to be acceptable for on-site re-use, as general/ordinary fill without substantial reworking and/or modification, while silty fine-sized sand may first require drying and blending.

Dissimilar excavated materials should not be commingled prior to their use elsewhere on-site, unless designated for a green/vegetation area. General fill material is also recommended to be placed on prepared and approved subgrade and in accordance with previous recommendations.

A geotextile fabric (such as, Mirafi 600X or equal) that separates the subgrade and the approved/select granular base layer is particularly recommended for sensitive cohesive/fine-grained/silty clay subgrade soil/fills. This approach will stabilize and provide a workable building pad condition with minimal required repairs.

The approved subgrades will most likely consist of medium dense or better/thoroughly compacted (i.e., minimum 92 percent of the maximum dry density by ASTM D 1557) granular fill and/or a thoroughly compacted imported select/approved granular fill. Above the approved subgrades, a minimum 12-inch thick select granular fill (i.e., number two crusher run stone or equal) layer is recommended as the base course for the proposed concrete floor slabs (heavy duty). The NYSDOT specification numbers for typical aggregate subbase/base course components are Item No. 304.12 (Metric) Subbase Course, Type 2 (< 2-inch maximum), which is preferred, or Item No. 304.14 (Metric) Subbase Course, Type 4 (< 2-inch maximum). This select granular base layer would be compacted to a minimum 95 percent of the maximum dry density by ASTM D 1557. The floor slabs for the proposed buildings are recommended to be a sixinch minimum thickness (heavy duty) or is as determined by the design structural engineer. The floor slab reinforcement should be provided through placement of wire mesh or plastic fibers and is also as determined by the design structural engineer.

At the discretion of the design architect, a vapor barrier may be considered for use in the proposed structures. The use of a thin gravel cushion, as a capillary break, or a thin sand cushion over a vapor barrier that is placed beneath the concrete slab(s) are at the discretion of the design architect/engineer or as required by local code. Gradations of gravels that are satisfactory capillary breaks include 1 ¼-inch or ¾-inch crushed stone or aggregate per the ASTM D2321 Types IA, IB and II with less than 5 % fines. A number two crusher run stone may also be applicable, but the gradation and material property specifications must qualify.

At the assumed final site grade and based upon the thickness and character of the underlying fill and natural soils, the subgrade modulus is recommended not to exceed 150 pounds per cubic inch (pci). A Poisson Ratio of 0.4 is recommended for design purposes. Isolation of the floor slabs from the footings-piers-columns and walls do appear to be warranted. Based upon the subgrade modulus and slab mechanical properties and thickness, the design structural engineer may accordingly adjust the granular aggregate base thickness.

Exterior Concrete Slabs-On-Grade/Pad/Parking and Roadway Facilities: The characteristics of the fill and natural soils and the known frost penetration in Western New York require that specific attention is provided to the design and construction of paved roadway and parking areas. For new pavement sections, isolated pockets of surficial silty/cohesive/fine-grained soil/fill may be encountered and may be too soft and wet in the proposed parking and roadway areas. These types of soil/fill may be adequately conditioned (i.e., dried and blended) and compacted (i.e., minimum 90 percent compaction) to support necessary construction equipment and normal pavement section. Otherwise, the removal/undercutting of the silty/cohesive/fine-grained soil/fill to a firm, approved subgrade and subsequent placement and compaction of select or approved granular fill will be required in order to accommodate the recommended pavement sections.

For new pavement sections, it is recommended that the subgrade surface is adequately graded and/or underdrains are installed to prevent water accumulation. Above the approved subgrade surface (i.e., minimum 90 percent compaction), a minimum eight-inch thick select granular layer is recommended as the base course for lightly traveled roadway and parking areas (standard duty section). A geotextile filter and strength fabric (such as, Mirafi 600X or equal) and minimum 12-inch thick base course are recommended for all truck routes and heavily traveled roadways (heavy duty section). If "pumping" of the silty/cohesive/fine-grained soil subgrade occurs or is difficult to stabilize during construction, an increase in the base thickness to that of the heavy-duty section and/or a geotextile filter and strength fabric is recommended to be placed on the prepared and approved subgrade for the standard duty section. The NYS Department of Transportation (NYSDOT) specification numbers for typical aggregate subbase/base course components are Item No. 304.12 (Metric) Subbase Course, Type 2 (< 2-inch maximum size), which is preferred, or Item No. 304.14 (Metric) Subbase Course, Type 4 (< 2-inch maximum size). The granular aggregate base layer is recommended to be compacted to at least 95 percent of maximum dry density by ASTM D1557.

The thickness of top and binder course layers should be determined in accordance with AASHTO methods. In summary, the minimum recommended asphalt thicknesses for the heavy-duty section and standard duty section are four inches and three inches, respectively. The top and binder course layers are recommended to be designed and constructed in accordance with New York State Department of Transportation <a href="Standard Specification">Standard Specification</a>.

<u>Limitations</u>, Field Inspections and Monitoring: This report is based on the preliminary information that is provided by project representatives and the subsurface conditions that were encountered at the test boring locations. Due to the nature of the investigation method, test pit excavation will provide a greater level of delineation of the subsurface soil/fill/rock conditions than can be defined by the test boring data alone. As detailed in Appendix F "Limitations", modification regarding proposed building/structure locations and other site developments can result in changes to provided recommendations. It is recommended that the geotechnical engineer be provided the opportunity to generally review the final detailed design and contract specifications. Required earthwork and foundation construction should be done under the supervision of experienced construction personnel and in a manner consistent with proven methods. All site work should be carefully monitored and tested by experienced geotechnical personnel to assure compliance with earthwork and foundation construction specifications.

Thank you for the opportunity to assist on this project. If questions should arise, please call the undersigned at your earliest convenience.

AROFESSIONAL

Very truly yours,

**BARRON & ASSOCIATES, INC.** 

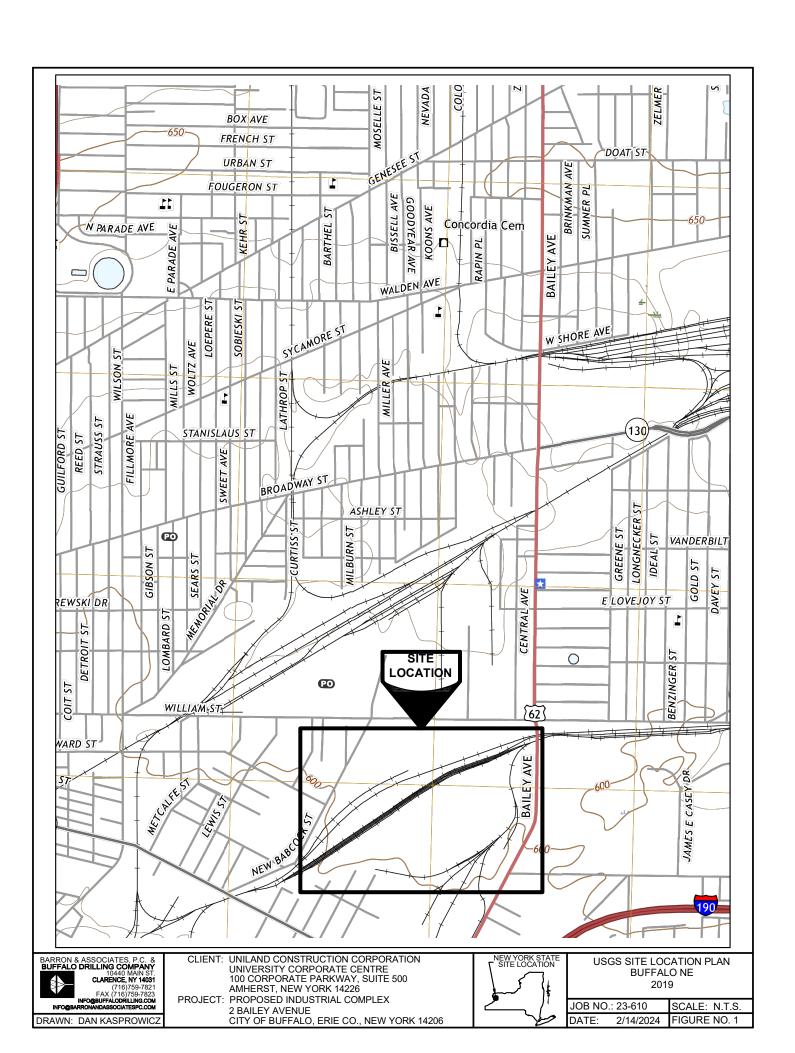
and

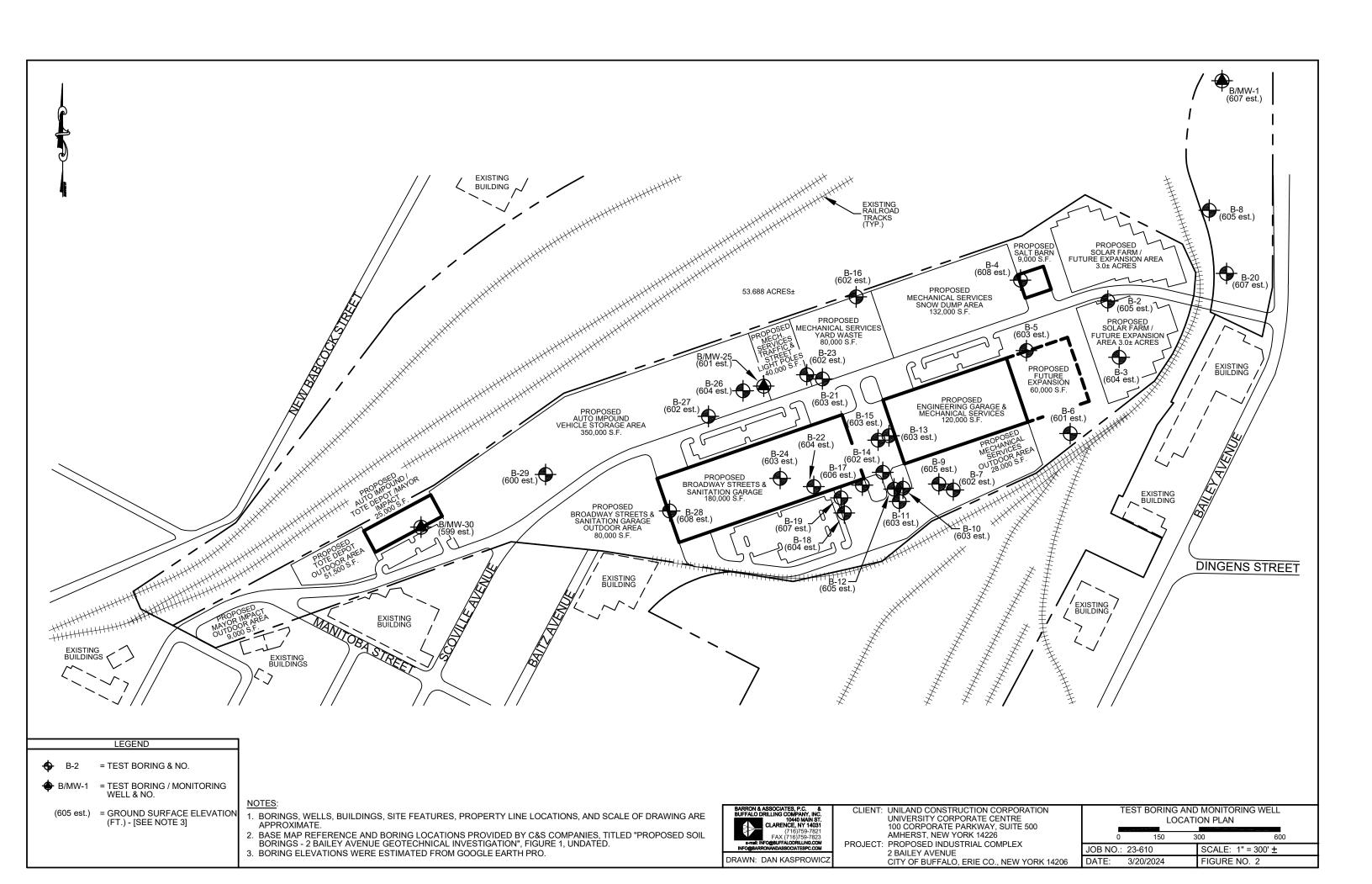
**BUFFALO DRILLING COMPANY, INC.** 

Andrew J. Camping, PG Senior Geologist

James S. Barron, PE

President/Geotechnical Engineer





B&A JOB NO: 23-610

CLIENT: Uniland Construction Corporation PROJECT: Proposed Industrial Complex

2 Bailey Avenue

City of Buffalo, Erie Co., New York 14206

### TABLE NO. 1 SUMMARY OF SUBSURFACE EXPLORATION

Boring No.	Ground	Tops	soil <sup>1</sup>	Fi	ill <sup>2</sup>	Ground	dwater <sup>3</sup>	Apparent	Top Rock <sup>4</sup>	Total
	Elevation (est.)	Thickness	To Elev.	Depth	Elev.	Depth	Elev.	Depth	Elev.	Depth Drilled
	(ft.)	(in.)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)
B/MW-1	607.0	-	-	6.0	601.0	4.8	602.2	15.0	592.0	15.0
B-2	605.0	-	-	7.0	598.0	-	-	15.6	589.4	15.6
B-3	604.0	-	-	6.0	598.0	-	-	16.5	587.5	16.5
B-4	608.0	-	-	6.0	602.0	-	-	17.0	591.0	17.0
B-5	603.0	-	-	4.0	599.0	-	-	15.0	588.0	15.0
B-6	601.0	-	-	5.0	596.0	-	-	19.5	581.5	19.5
B-7	602.0	-	-	3.0	599.0	-	-	20.0	582.0	20.0
B-8	605.0	-	-	5.0	600.0	-	-	15.5	589.5	15.5
B-9	605.0	-	-	6.0	599.0	-	-	20.0	585.0	20.0
B-10	603.0	-	-	6.0	597.0	-	-	19.0	584.0	19.0
B-11	603.0	-	-	4.0	599.0	-	-	17.0	586.0	17.0
B-12	605.0	-	-	6.0	599.0	-	-	17.0	588.0	17.0
B-13	603.0	-	-	4.0	599.0	-	-	21.0	582.0	21.0
B-14	602.0	-	-	6.0	596.0	-	-	20.5	581.5	20.5
B-15	603.0	-	-	5.0	598.0	-	-	21.0	582.0	21.0
B-16	602.0	-	-	4.0	598.0	-	-	15.0	587.0	15.0
B-17	606.0	-	-	4.0	602.0	-	-	19.5	586.5	19.5
B-18	604.0	-	-	5.0	599.0	-	-	10.0	594.0	10.0
B-19	607.0	-	-	4.0	603.0	-	-	9.1	597.9	9.1
B-20	607.0	-	-	8.0	599.0	-	-	20.5	586.5	20.5
B-21	603.0	-	-	5.0	598.0	-	-	17.0	586.0	17.0
B-22	604.0	-	-	6.0	598.0	-	-	13.0	591.0	13.0
B-23	602.0	-	-	5.0	597.0	-	-	14.5	587.5	14.5
B-24	603.0	-		6.0	597.0	-	-	14.0	589.0	14.0
B/MW-25	601.0	-	-	3.0	598.0	2.0	599.0	12.1	588.9	12.1
B-26	604.0	-	-	4.0	600.0	-	-	13.5	590.5	13.5
B-27	602.0	-	-	4.0	598.0	-	-	16.5	585.5	16.5
B-28	608.0	-	-	4.0	604.0	-	-	11.7	596.3	11.7
B-29	600.0	-	-	4.0	596.0	-	-	10.0	590.0	10.0
B/MW-30	599.0	-	-	6.0	593.0	12.3	586.7	18.0	581.0	18.0

- 1. All measurements are approximate and taken in the field during drilling or sampling efforts. Topsoil very thin and not visually distinguishable from fill material below.
- 2. In many cases, fill material is comprised of indigenous soil and is visually indistinguishable from naturally deposited material. Fill depths, in most cases have been approximated.
- 3. At the boring locations, groundwater readings were taken at the completion of drilling efforts and therefore adequate time for groundwater elevations to stabilize to static conditions was probably not allowed. B/MW-1, B/MW-25, and B/MW-30 are monitoring wells with stabilized groundwater readings.
- 4. Auger refusal depth identified during subsurface exploration efforts and is inferred as top of bedrock.



B&A JOB NO: 23-610

CLIENT: Uniland Construction Corporation PROJECT: Proposed Industrial Complex

2 Bailey Avenue

City of Buffalo, Erie Co., New York 14206

### TABLE NO. 2 LABORATORY PHYSICAL SOIL TEST RESULTS

Boring No.	Sample No.	Depth	Moisture Content ASTM D2216	Organic Matter Content ASTM D2974	Unconfined Compressive Strength ASTM D2166	G	rain Size	e Analys	sis		rberg Li STM D43		USCS Soil Classification ASTM D2487 / ASTM D2488 *
						Gravel	Sand	Silt	Clay	LL	PL	PI	
		(ft.)	(%)	(%)	(psf)	(%)	(%)	(%)	(%)	(%)	(%)	(%)	( - )
B-4	S-1	0-2	12.8	-	-	-	-	-	-	-	-	-	SM-Fill
	S-2	2-4	11.6	-	-	ı	ı	1	-	ı	ı	-	SM-Fill
	S-3	4-6	25.8	-	-	-	ı	-	-	ı	-	-	SM-Fill
	S-4	6-8	27.0	-	-	ı	ı	ı	-	ı	ı	-	CL
	S-5	8-10	20.9	-	-	ı	-	-	-	32	20	12	CL
	S-6	10-12	25.1	-	-	ı	ı	ı	-	ı	ı	-	CL
	S-7	14-16	12.9	-	-	ı	ı	ı	-	ı	ı	-	CL/ML-Till
B-8	S-1	0-2	17.8	-	-	26.9	58.1	1.5	)	ı	ı	-	SM-Fill
	S-2	2-4	17.6	-	-	20.9	50.1	15.0		ı	ı	-	SM-Fill
	S-3	4-6	-	-	-	ı	ı	ı	-	ı	ı	-	NO SAMPLE
	S-4	6-8	22.8	-	-	1	ı	-	-	ı	•	-	CL
	S-5	8-10	26.3	-	-	-	-	-	-	-	•	-	CL
	S-6	10-12	23.0	-	-	ı	ı	ı	-	ı	ı	-	CL
	S-7	14-16	11.8	-	-	ı	ı	ı	-	ı	ı	-	CL/ML-Till
B-10	S-1	0-2	12.7	-	-	1	ı	-	-	ı	•	-	SM-Fill
	S-2	2-4	20.0	-	-	ı	ı	•	-	ı	•	-	SM-Fill
	S-3	4-6	19.6	-	-	ı	ı	ı	-	ı	ı	-	SM-Fill
	S-4	6-8	24.0	-	-	-	-	-	-	32	18	14	CL
	S-5	8-10	20.5	-	-	-	ı	-	-	ı	-	-	CL
	S-6	10-12	21.0	-	-	-	-	-	-	-	-	-	CL
	S-7	14-16	23.8	-	-	ı	ı	ı	-	ı	ı	-	CL/ML-Till
B-15	S-1	0-2	15.1	-	-	-	ı	-	-	ı	-	-	SM-Fill
	S-2	2-4	23.6	-	-	ı	ı	ı	-	ı	ı	-	SM-Fill
	S-3	4-5	20.7	-	-	ı	-	-	-	-	-	-	SM-Fill
	5-5	5-6	30.3	-	-	-	•	-	-	•	-	-	CL
	S-4	6-8	25.2	-	-	-	-	-	-	-	-	-	CL
	S-5	8-10	19.7	-	-	-	-	-	-	-	-	-	CL
	S-6	10-12	18.3	-	-	-	1	-	-	1	-	-	CL
	S-7	14-16	17.0	-	-	-	1	-	-	27	16	11	CL
	S-8	19-21	8.5	-	-	-	-	-	-	-	-	-	CL/ML-Till

<sup>\*</sup> Soil classification based on visual identification and soil classification of adjacent samples (as applicable).



B&A JOB NO: 23-610

CLIENT: Uniland Construction Corporation PROJECT: Proposed Industrial Complex

2 Bailey Avenue

City of Buffalo, Erie Co., New York 14206

# TABLE NO. 2 (cont'd) LABORATORY PHYSICAL SOIL TEST RESULTS

Boring No.	Sample No.	Depth	Moisture Content ASTM D2216	Organic Matter Content ASTM D2974	Unconfined Compressive Strength ASTM D2166	G		e Analys	sis		rberg Li STM D43		USCS Soil Classification ASTM D2487 / ASTM D2488 *
						Gravel	avel Sand Silt Clay		LL	PL	PI		
		(ft.)	(%)	(%)	(psf)	(%)	(%)	(%)	(%)	(%)	(%)	(%)	( - )
B-24	S-1	0-2	18.5	-	-					-	-	-	SM-Fill
	S-2	2-4	21.8	-	-	13.5	65.1	21	.4	-	-	-	SM-Fill
	S-3	4-6	35.4	-	-					-	-	-	SM-Fill
	S-4	6-8	23.3	-	-	-	-	-	-	-	-	-	CL
	S-5	8-10	20.5	-	-	-	-	-	-	-	-	-	CL
	S-6	10-12	20.9	-	-	-	-	1	-	-	-	-	CL
B/MW-25	S-1	0-2	41.4	=	-	1	ı	•	•	•	-	-	SM-Fill
	S-2	2-3	21.3	=	-	1	ı	•	•	•	-	-	SM-Fill
	3-2	3-4	24.4	=	-	1	ı	•	•	•	-	-	CL
	S-3	4-6	27.2	=	-	1	ı	•	•	•	-	-	CL
	S-4	6-8	26.1	=	-	1	ı	•	•	•	-	-	CL
	S-5	8-10	10.5	-	-	ı	ı	-	-	-	-	-	CL/ML-Till
	S-6	10-12	11.5	-	-	ı	ı	ı	ı	18	11	7	CL/ML-Till
B-28	S-1	0-2	19.7	-	-	ı	ı	ı	ı	ı	-	-	SM-Fill
	S-2	2-4	36.1	-	-	ı	ı	ı	ı	ı	-	-	SM-Fill
	S-3	4-6	21.3	-	-	ı	ı	ı	ı	44	20	24	CL
	S-4	6-8	22.7	-	-	ı	ı	•	•	•	-	-	CL
	S-5	8-10	14.4	-	-	ı	ı	-	-	-	-	-	CL/ML-Till
	S-6	10-12	10.8	-	-	ı	ı	ı	ı	ı	-	-	CL/ML-Till
B/MW-30	S-1	0-2	21.7	-	-					ı	-	-	SM-Fill
	S-2	2-4	12.9	=	-	12.4	61.6	26	6.0	•	-	-	SM-Fill
	S-3	4-6	19.9	-	-					-	-	-	SM-Fill
	S-4	6-8	18.4	-	-	-	-	-	-	-	-	-	CL
	S-5	8-10	21.7	-	-	-	-	-	-	-	-	-	CL
	S-6	10-12	22.9	-	-	-	-	-	-	-	-	-	CL
	S-7	14-16	24.1	-	-	-	-	-	-	-	-	-	CL

<sup>\*</sup> Soil classification based on visual identification and soil classification of adjacent samples (as applicable).

### **BARRON & ASSOCIATES, P.C.**

### **Geotechnical Consulting and Special Inspections**

10440 Main Street

(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX A**

**TEST BORING LOGS/MONITORING WELL SCHEMATICS** 

### MONITORING WELL SCHEMATIC

**BORING No.:** B/MW-1

PROJECT:

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**Proposed Industrial Complex** 

2 Bailey Avenue, City of Buffalo, New York 14220

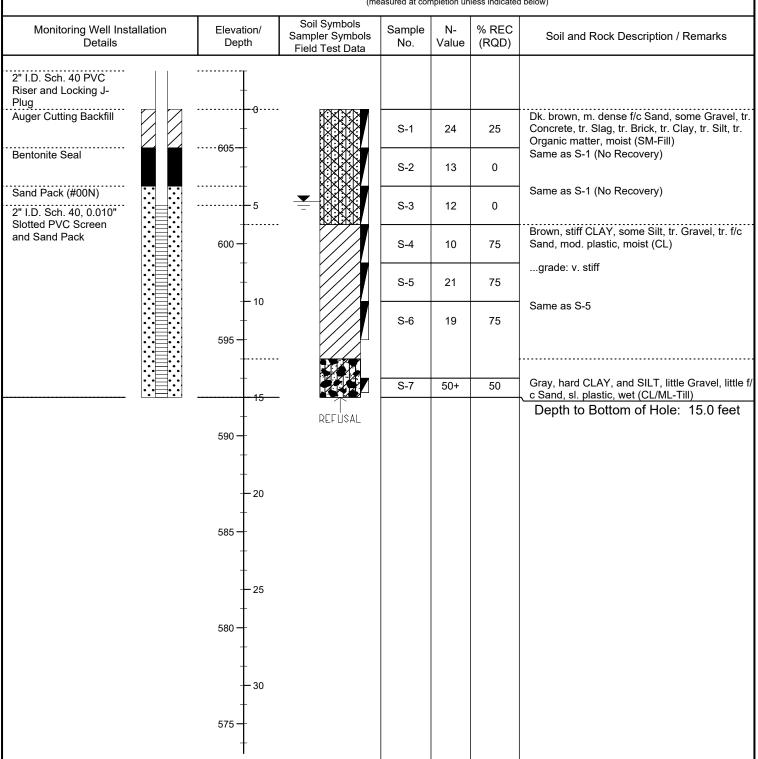
TYPE OF DRILL RIG: **DRILLER:** D. Rimbeck CME-550 (ATV) 4 1/4" I.D. H.S.A. **SAMPLING METHODS: ASTM D1586 SIZE AND TYPE OF BIT:** 

**DATE STARTED:** 2/6/24 SURFACE ELEVATION (FT): 607 est.

**DATE COMPLETED:** 2/6/24 **GROUNDWATER DEPTH (FT):** 4.8 (See Notes)

(measured at completion unless indicated below)

JOB No.: 23-610



- 1. Groundwater level at 13.0 feet below ground surface (BGS) after installation on 2/6/24.
- 2. Well developed and groundwater levels recorded again on 3/26/24.
- 3. Groundwater level at 4.8 feet BGS (elevation 602.2 +/- feet) on 3/26/24.
- 4. Groundwater recharging approximately 12 inches per five minutes.

#### DRILLING COMPANY, INC. 10440 MAIN STREET

10440 MAIN STREET CLARENCE, NEW YORK 14031

(716) 759-7821 FAX: (716) 759-7823

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

**TEST BORING LOG** 

**BORING No.:** B-2

**JOB No.**: 23-610

DATE STARTED:2/7/24SURFACE ELEVATION (ft.):605 est.DATE COMPLETED:2/7/24GROUNDWATER DEPTH (ft.):None

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
605 — 0	2 3 7 9	S-1 : 0.0'- 2.0'	10	50	Dk. brown, m. dense f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
<u> </u>	6 7 5 7	S-2 : 2.0'- 4.0'	12	65	grade: Brown, little Gravel
600 + 5	12 12 12 6 4	S-3 : 4.0'- 6.0'	18	50	grade: Dk. brown, some Gravel, wet
†	5 5 8 13	S-4 : 6.0'- 8.0'	13	65	grade: little Gravel  Brown, stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist
<u> </u>	9 9 15	S-5 : 8.0'- 10.0'	24	65	(CL)grade: v. stiff
595 + 10	23 3 8 12 18	S-6 : 10.0'- 12.0'	20	90	Same as S-5
	18				
590 <del></del> 15	3 3 50/4"	S-7 : 14.0'- 15.3'	50+	50	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
+	REFUSAL				Depth to Bottom of Hole: 15.6 feet
585 - 20					
Ī					
†					
580 — 25					
<u> </u>					
575 — 30					
570 - 35					
†					

# C (7

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

### **TEST BORING LOG**

**JOB No.**: 23-610 **BORING No.**: B-3

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/8/24 SURFACE ELEVATION (ft.): 604 est.

DATE COMPLETED: 2/8/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
T 0	2 3 5 4	S-1 : 0.0'- 2.0'	8	0	Brown, m. stiff Clay, some Silt, little f/c Sand, tr. Gravel, tr. Slag, mod. plastic, moist (CL-Fill) (No Recovery)
<b>T</b>	2 2 3 4	S-2 : 2.0'- 4.0'	5	30	Same as S-1
600 + 5	2 3 2 3 2 3 2 3 3 2 3 3 2 3 3 3 3 3 3 3	S-3 : 4.0'- 6.0'	5	25	Dk. brown, loose f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)
	3 4 8 9	S-4 : 6.0'- 8.0'	12	50	Brown, stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)
595 <del>-</del>	4 9 11	S-5 : 8.0'- 10.0'	20	70	grade: v. stiff
<del>-</del> 10	4 5 9 11	S-6 : 10.0'- 12.0'	14	75	grade: stiff
590 -	4 6 8	S-7 : 14.0'- 16.0'	14	90	Gray, stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
	8	3-7 . 14.0 - 10.0	14	90	(==,
†	REFU\$AL				Depth to Bottom of Hole: 16.5 feet
585 - 20					
<b> </b>					
580 —					
25					
575					
-30					
†					
570 - 35					
+					

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-4

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/8/24 SURFACE ELEVATION (ft.): 608 est.

DATE COMPLETED: 2/8/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(mea.	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
Το	4 6 2 4 4 6 6 6 7	S-1 : 0.0'- 2.0'	10	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
605	\$\begin{align*} 2 & \text{2} & \text{4} & \text{5} & \text{7} & \text{7} & \text{9} & \t	S-2 : 2.0'- 4.0'	12	50	Same as S-1
-5	22 17 8 9	S-3 : 4.0'- 6.0'	25	50	grade: wet
†	13 7 4 6	S-4 : 6.0'- 8.0'	11	50	Gray, stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)
600 +	7 8 10 15	S-5 : 8.0'- 10.0'	18	65	grade: Brown, v. stiff, tr. f/c Sand
<del>-</del> 10	5 8 18 22	S-6 : 10.0'- 12.0'	26	100	Same as S-5
595 —					
+ - 15	7 11 13 16	S-7 : 14.0'- 16.0'	24	100	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)
590 —	REFUSAL				Depth to Bottom of Hole: 17.0 feet
20					
585 —					
+					
- 25					
580					
30					
† 					
575 —					
<del>-</del> 35					

# 10440 MAIN STREET

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**PROJECT: Proposed Industrial Complex** 

2 Bailey Avenue, City of Buffalo, New York 14220

D. Rimbeck **DRILLER: TYPE OF DRILL RIG:** CME-550 (ATV) **SAMPLING METHODS: ASTM D1586 SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A.

**TEST BORING LOG** 

**BORING No.:** B-5

**JOB No.**: 23-610

DATE STARTED: 2/8/24 **SURFACE ELEVATION (ft.):** 603 est. DATE COMPLETED: 2/8/24 **GROUNDWATER DEPTH (ft.):** None

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
T <sup>o</sup>	\$\begin{array}{cccccccccccccccccccccccccccccccccccc	S-1 : 0.0'- 2.0'	23	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
600 -	20 20 22 24 27	S-2 : 2.0'- 4.0'	44	70	grade: dense, little Slag, little Brick
+ +5	6 6 8 12	S-3 : 4.0'- 6.0'	14	75	Gray, stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)
+	15 20 23 25	S-4 : 6.0'- 8.0'	43	75	grade: Brown, hard, tr. f/c Sand,
595 <del>-</del> -	9 9 11	S-5 : 8.0'- 10.0'	20	70	grade: v. stiff
<del>-</del> 10	6 9 23 24	S-6 : 10.0'- 12.0'	31	75	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
590 —					
† <del> </del> 15	11 50/5"	S-7 : 14.0'- 14.9'	50+	20	Same as S-6
+	Z ` REFU\$AL				Depth to Bottom of Hole: 15.0 feet
585 —					
+					
20					
580 —					
<b> </b>					
- 25					
†					
<u> </u>					
575 —					
-30					
+					
570 —					
+					
- 35					
+					

### 10440 MAIN STREET

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 TEST BORING LOG

**JOB No.**: 23-610 **BORING No.**: B-6

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/8/24 SURFACE ELEVATION (ft.): 601 est.

DATE COMPLETED: 2/8/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
600 + 0	3 3 3 7 7	S-1 : 0.0'- 2.0'	15	50	Dk. brown, m. dense f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
†	2 2 2 4 5 4 2 2 2 3 3	S-2 : 2.0'- 4.0'	9	50	grade: loose
-5	2 2 2 4 7	S-3 : 4.0'- 6.0'	6	50	Same as S-2  Brown, m. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic,
595 —	9 12 18 15	S-4 : 6.0'- 8.0'	30	90	moist (CL)grade: v. stiff
†	7 15 18 21	S-5 : 8.0'- 10.0'	33	90	grade: hard
590 -	7 12 15 19	S-6 : 10.0'- 12.0'	27	90	grade: v. stiff
					Gray, m. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet
<del>-</del> 15	4	S-7 : 14.0'- 16.0'	7	100	(CL/ML-Till)
†					
<b>l</b> †	50/6"	S-8 : 19.0'- 19.5'	50+	100	grade: hard
580 -	REFUSAL				Depth to Bottom of Hole: 19.5 feet
<del>-</del> 25					
575 —					
570 - 30					
35					
565 —					

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 TEST BORING LOG

**JOB No.**: 23-610 **BORING No.**: B-7

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/9/24 SURFACE ELEVATION (ft.): 602 est.

DATE COMPLETED: 2/9/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
T 0	6 6 6 5 5 5	S-1 : 0.0'- 2.0'	11	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
600 —	3 4 4 4 6	S-2 : 2.0'- 4.0'	8	50	grade: loose  Dk. brown, stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic,
+ +5	4 6 9 11	S-3 : 4.0'- 6.0'	15	50	moist (CL)grade: Brown, v. stiff, tr. f/c Sand
595 —	11 15 18 22	S-4 : 6.0'- 8.0'	33	75	grade: hard
<b>†</b>	6 7 15 17	S-5 : 8.0'- 10.0'	22	75	grade: v. stiff
590	6 11 18 21	S-6 : 10.0'- 12.0'	29	100	Same as S-5
390 —					
15	5.5.5.5.5.5.5.5.5.5.5.5.5.5.5.5.5.5.5.	S-7 : 14.0'- 16.0'	10	100	Gray, stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
585 —					
-20	5 50/4"	S-8 : 19.0'- 19.8'	50+	50	grade: hard
580	REFUSAL				Depth to Bottom of Hole: 20.0 feet
<del>-</del> 25					
575 —					
- -30					
570 —					
+					
<del>-</del> 35					



10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-8

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/7/24 SURFACE ELEVATION (ft.): 605 est.

DATE COMPLETED: 2/7/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
605 — 0	2 2 5 7 7 9	S-1 : 0.0'- 2.0'	12	50	Dk. brown, m. dense f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
†	10 11 11 15	S-2 : 2.0'- 4.0'	22	50	grade: m. dense
600 - 5	7 4 6 8 8	S-3 : 4.0'- 6.0'	10	0	Same as S-2 (No Recovery)
Ī	9 11 14 14	S-4 : 6.0'- 8.0'	25	25	Dk. brown, v. stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)
595 — 10	6 8 10 11	S-5 : 8.0'- 10.0'	18	90	grade: Brown, tr. f/c Sand
393 — 10	6 9 9 9	S-6 : 10.0'- 12.0'	18	25	Same as S-5
590 — 15	50/4"	S-7 : 14.0'- 15.3'	59+	25	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
	REFÜSAL				Depth to Bottom of Hole: 15.5 feet
585 - 20					
+					
580 <del>-</del> 25					
<u> </u>					
575 - 30					
<u> </u>					
†					
570 — 35					

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

### **TEST BORING LOG**

**JOB No.:** 23-610 **BORING No.:** B-9

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/9/24 SURFACE ELEVATION (ft.): 605 est.

DATE COMPLETED: 2/9/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
605 — 0	3 6 14 7	S-1 : 0.0'- 2.0'	20	50	Dk. brown, m. dense f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
†	6 6 7 6 6 7 6	S-2 : 2.0'- 4.0'	13	50	grade: Brown, little Gravel
600 $\frac{1}{1}$ 5	9 4 4 4 4	S-3 : 4.0'- 6.0'	8	50	grade: Dk. brown, loose, wet
	5 7 12 15	S-4 : 6.0'- 8.0'	19	75	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)
595 — 10	4 10 15 15	S-5 : 8.0'- 10.0'	25	75	Same as S-4
	11 14 17	S-6 : 10.0'- 12.0'	25	75	Same as S-4
					Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic,
590 — 15	9 9 12	S-7 : 14.0'- 16.0'	18	100	moist (CL/ML-Till)
<b> </b>					
585 - 20	28 50/4"	S-8 : 19.0'- 19.8'	50+	20	grade: hard, wet
	 REFU\$AL				Depth to Bottom of Hole: 20.0 feet
580 - 25					
575 - 30					
570 - 35					
I '			•		•

#### 10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

DRILLING COMPANY, INC.

10440 MAIN STREET

CLARENCE, NEW YORK 14031

TEST BORING LOG

JOB No.: 23-610

BO

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/9/24 SURFACE ELEVATION (ft.): 603 est.

DATE COMPLETED: 2/9/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

				(meas	sured at completion unless indicated below)
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
T 0	5 6 7 6 7	S-1 : 0.0'- 2.0'	13	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
600 —	\$\begin{array}{cccccccccccccccccccccccccccccccccccc	S-2 : 2.0'- 4.0'	5	50	grade: loose, wet
<u>+</u> 5	7 4 4 4 5	S-3 : 4.0'- 6.0'	8	50	grade: Brown
505	6 8 15 17	S-4 : 6.0'- 8.0'	23	100	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)
595 —	4 9 9 12	S-5 : 8.0'- 10.0'	18	75	Same as S-4
+ 10 +	8 10 14 17	S-6 : 10.0'- 12.0'	24	75	Same as S-4
590 —					
15	9 11 11 11	S-7 : 14.0'- 16.0'	20	100	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)
585 —					
-20	REFUSAL				Depth to Bottom of Hole: 19.0 feet
+					Depth to Bottom of Hole. 19.0 feet
580 —					
<del>-</del> 25					
575 <del>-</del>					
-30					
570 —					
<del>-</del> 35					

Logged by: E. Zinni

**BORING No.:** B-10

### 10 C

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

### **TEST BORING LOG**

**JOB No.**: 23-610 **BORING No.**: B-11

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/12/24 SURFACE ELEVATION (ft.): 603 est.

DATE COMPLETED: 2/12/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
T 0	4 5 5 5 6	S-1 : 0.0'- 2.0'	10	10	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)			
600 +	\$ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	S-2 : 2.0'- 4.0'	10	0	Same as S-1 (No Recovery)			
-5	6 6 11 12	S-3 : 4.0'- 6.0'	17	50	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)			
†	6 8 11 11	S-4 : 6.0'- 8.0'	19	50	Same as S-3			
595 —	7 13 19 19 24	S-5 : 8.0'- 10.0'	32	75	Brown, hard CLAY, some Silt, little Gravel, little f/c Sand, mod. plastic, moist (CL-Till)			
+ 10 +	8 12 12 18 23	S-6 : 10.0'- 12.0'	30	75	grade: v. stiff			
590								
— 15 —	9 12 21 50/5"	S-7 : 14.0'- 16.0'	33	75	grade: Gray, hard, and SILT, sl. plastic (CL/ML-Till)			
585 <del>-</del>	REFUSAL				Depth to Bottom of Hole: 17.0 feet			
20								
†								
580 —								
<del>-</del> 25								
575 —								
30								
†								
570 —								
+ 35								

# 10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-12

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/9/24 SURFACE ELEVATION (ft.): 605 est.

DATE COMPLETED: 2/9/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

			sured at completion unless indicated below)		
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
605 — 0	2 2 10 6 6 6 6 4	S-1 : 0.0'- 2.0'	16	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)
+	3 3 3 3 3 3 3 4 4	S-2 : 2.0'- 4.0'	6	65	grade: Brown, loose
600 + 5	5 5 3 5	S-3 : 4.0'- 6.0'	8	75	Dk. brown, stiff Clay, some Silt, little f/c Sand, tr. Gravel, tr. Slag, mod. plastic, moist (CL-Fill)
	4 5 11 13	S-4 : 6.0'- 8.0'	16	75	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)
595 - 10	6 9 13 17	S-5 : 8.0'- 10.0'	21	75	Same as S-4
393   10	8 13 17 17	S-6 : 10.0'- 12.0'	30	75	Same as S-4
590 — 15	8 9 50/5"	S-7 : 14.0'- 15.9'	17	95	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)
	REFUSAL				Double Dobon (Units 47.0) of
585 - 20	REFUVAL				Depth to Bottom of Hole: 17.0 feet
580 25					
+					
575 - 30					
+					
570 - 35					

### TEST BORING LOG

**JOB No.**: 23-610

**BORING No.:** B-13



**PROJECT:** 

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/12/24 SURFACE ELEVATION (ft.): 603 est.

DATE COMPLETED: 2/12/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

,		sured at completion unless indicated below)			
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks
T 0	2 2 3 3 4 7	S-1 : 0.0'- 2.0'	7	75	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)
600 -	3 4 4 4 8 8	S-2 : 2.0'- 4.0'	8	75	Same as S-1
5	4 5 5 6	S-3 : 4.0'- 6.0'	10	75	Gray, v. stiff CLAY, some Silt, some f/c Sand, tr. Gravel, mod. plastic, wet (CL)
505	6 8 11 13	S-4 : 6.0'- 8.0'	19	100	grade: Brown, v. stiff, tr. f/c Sand
595 —	6 12 18 26	S-5 : 8.0'- 10.0'	30	100	Same as S-4
10	8 15 22 28	S-6 : 10.0'- 12.0'	37	100	grade: hard
590 —					
15	5 6 9 12	S-7 : 14.0'- 16.0'	15	100	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)
585					
1 +	8 8 8 8	S-8 : 19.0'- 20.9'	18	75	Same as S-7
<del>-</del> 20	50/5"	3-0 . 19.0 - 20.9	10	73	
580 <del>-</del>	 REFU\$AL				Depth to Bottom of Hole: 21.0 feet
25					
575 —					
30					
570 —					
-35					

# 10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

### **TEST BORING LOG**

**JOB No.**: 23-610 **BORING No.**: B-14

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/12/24 SURFACE ELEVATION (ft.): 602 est.

DATE COMPLETED: 2/12/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
T <sub>o</sub>	3 3 50/4"	S-1 : 0.0'- 1.3'	53+	20	Dk. brown, v. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)			
600 —	50/3" 50/3" 50/3"	S-2 : 2.0'- 2.3'	50+	10	grade: and Construction lumber			
5	5 6 8 8	S-3 : 4.0'- 6.0'	14	50	Dk. brown, stiff Clay, some Silt, little f/c Sand, tr. Gravel, tr. Slag, tr. Brick, mod. plastic, moist (CL-Fill)			
595	9 12 17 24	S-4 : 6.0'- 8.0'	29	75	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)			
+ + 10	8 10 11 16	S-5 : 8.0'- 10.0'	21	75	Same as S-4			
590	9 13 17	S-6 : 10.0'- 12.0'	22	75	Same as S-4			
	7				Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic,			
15	10 10 12 13 13	S-7 : 14.0'- 16.0'	22	100	moist (CL/ML-Till)			
585 —								
20	17 50/5'	S-8 : 19.0'- 19.9'	50+	75	grade: hard			
580 —	REFUSAL				Depth to Bottom of Hole: 20.5 feet			
- 25								
575								
30								
570 —								
35								
+								

### 10440 MAIN STREET

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **JOB No.**: 23-610 **BORING No.:** B-15

**TEST BORING LOG** 

**PROJECT: Proposed Industrial Complex** 

2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck **TYPE OF DRILL RIG:** CME-550 (ATV) **ASTM D1586 SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A. **SAMPLING METHODS:** 

DATE STARTED: 2/12/24 603 est. **SURFACE ELEVATION (ft.):** DATE COMPLETED: 2/12/24 **GROUNDWATER DEPTH (ft.):** None (measured at completion unless indicated below)

	(measured at completion unless indicated below)						
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
T 0	6 6 6 7 7 8 8 8 8 8 12 12 12 12 12 12 14	S-1 : 0.0'- 2.0'	24	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)		
600 +	2 7 13 15 16 16	S-2 : 2.0'- 4.0'	28	65	grade: wet		
-5	12 11 7 7	S-3 : 4.0'- 6.0'	18	75	grade: Brown Gray, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, wet		
†	6 7 8 8	S-4 : 6.0'- 8.0'	15	75	(CL) grade: Brown, moist		
595 <del>-</del> -	5 11 19 18	S-5 : 8.0'- 10.0'	30	100	grade: hard		
10	7 12 18 22	S-6 : 10.0'- 12.0'	30	100	Same as S-5		
590 —							
15	8 13 25 17	S-7 : 14.0'- 16.0'	38	100	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)		
- 585 <del>-</del>							
20	7 8 12 50/4"	S-8 : 19.0'- 20.8'	20	100	grade: v. stiff		
580 —	REFUSAL				Depth to Bottom of Hole: 21.0 feet		
- - 25							
†							
575 <del>-</del> -							
-30							
570 <del>-</del>							
35							
I T							

#### 1044 CLAI (716

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-16

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/8/24 SURFACE ELEVATION (ft.): 602 est.

DATE COMPLETED: 2/8/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
	3 6 6 9 9 15	S-1 : 0.0'- 2.0'	15	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)		
600 +	12 14 7 6	S-2 : 2.0'- 4.0'	21	50	Same as S-1		
+ +5	7 7 8 9	S-3 : 4.0'- 6.0'	15	75	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)		
595 —	8 15 19 20	S-4 : 6.0'- 8.0'	34	75	grade: hard		
† †	8 18 20 22	S-5 : 8.0'- 10.0'	38	75	Same as S-4		
<del>-</del> 10	8 11 20 22	S-6 : 10.0'- 12.0'	31	10	Same as S-5		
590 —	22						
+ + 15	7 50/2"	S-7 : 14.0'- 14.7'	50+	50	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)		
585 —	REFUSAL				Depth to Bottom of Hole: 15.0 feet		
-							
20							
580 —							
†							
<del>-</del> 25							
575 —							
-30							
570 —							
35							
†							

#### 10440 CLAR (716)

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-17

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/12/24 SURFACE ELEVATION (ft.): 606 est.

DATE COMPLETED: 2/12/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion unless indicated below)						
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
605 - 0	2 3 4 4 5 5	S-1 : 0.0'- 2.0'	7	50	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)		
†	3 4 7 7 7	S-2 : 2.0'- 4.0'	11	50	grade: m. dense		
-5	4 4 4 6	S-3 : 4.0'- 6.0'	8	75	Brown, stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)		
600 +	3 5 7 7	S-4 : 6.0'- 8.0'	12	75	grade: tr. f/c Sand		
†	6 15 30 37	S-5 : 8.0'- 10.0'	45	75	grade: hard		
<del>-</del> 10	10 18 26 40	S-6 : 10.0'- 12.0'	44	100	Same as S-5		
					Construction of Class and		
- 15 590 -	10 13 13	S-7 : 14.0'- 16.0'	23	100	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)		
++							
+	50/2"	S-8 : 19.0'- 19.2'	50+	0	grade: hard (No Recovery)		
585 <del>-</del>	REFUSAL				Depth to Bottom of Hole: 19.5 feet		
580 -							
†							
30							
575 —							
35							
570							

### 10440 MAIN STREET CLARENCE, NEW YOR

TEST BORING LOG

CLARENCE, NEW YORK 14031 **JOB No.:** 23-610 **BORING No.:** B-18 (716) 759-7821 FAX: (716) 759-7823

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck **TYPE OF DRILL RIG:** CME-550 (ATV) **SAMPLING METHODS:** ASTM D1586 **SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A.

DATE STARTED: 2/13/24 SURFACE ELEVATION (ft.): 604 est.

DATE COMPLETED: 2/13/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

		(measured at completion unless indicated below)						
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
T 0	3 6 11 6 11 6	S-1 : 0.0'- 2.0'	17	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)			
	6 3 3 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	S-2 : 2.0'- 4.0'	6	50	grade: loose, wet			
600 + 5	4 3 3 3 5	S-3 : 4.0'- 6.0'	6	50	Same as S-2  Brown, m. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic,			
	5 10 13 15	S-4 : 6.0'- 8.0'	23	75	moist (CL)grade: v. stiff			
595 <del>-</del>	7 7 11 50/1"	S-5 : 8.0'- 9.6'	18	75	Same as S-4			
- 10 -	REFUSAL				Depth to Bottom of Hole: 10.0 feet			
590 <del>-</del>								
15								
+								
585 — 20								
580 —								
25								
575 —								
570 —								
35								

### **TEST BORING LOG**

**BORING No.:** B-19

**JOB No.**: 23-610



**PROJECT:** 

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED:2/13/24SURFACE ELEVATION (ft.):607 est.DATE COMPLETED:2/13/24GROUNDWATER DEPTH (ft.):None

(measured	at	comp	letion	unless	indica	ted	below)	
-----------	----	------	--------	--------	--------	-----	--------	--

	(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
T 0	2 2 3 3 3 3 5 5 5 5 7 5 7 5 7 7 7 7 7 7 7 7	S-1 : 0.0'- 2.0'	6	50	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)			
605 —	3 5 7 5	S-2 : 2.0'- 4.0'	12	50	grade: m. dense, some Slag			
-5	2 2 3 5	S-3 : 4.0'- 6.0'	5	50	Brown, m. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)			
600 —	8 8 14 18	S-4 : 6.0'- 8.0'	22	65	grade: v. stiff			
†	6 6 50/1"	S-5 : 8.0'- 9.1'	56+	50	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till)			
<del>  1</del> 0	REFÜSAL				Depth to Bottom of Hole: 9.1 feet			
595 —								
+ + 15								
590 <del>-</del>								
+ +								
<del>-</del> 20								
585 —								
- 25								
580 —								
30								
575 —								
- 25								
+ 35								

**PROJECT:** 

### 10440 MAIN STREET

**TEST BORING LOG** 

**BORING No.:** B-20

**JOB No.:** 23-610

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**Proposed Industrial Complex** 

2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck **TYPE OF DRILL RIG:** CME-550 (ATV) **SAMPLING METHODS: ASTM D1586 SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A.

**DATE STARTED:** 2/7/24 **SURFACE ELEVATION (ft.):** 607 est. **DATE COMPLETED:** 2/7/24 **GROUNDWATER DEPTH (ft.):** None (measured at completion unless indicated below)

	(measured at completion unless indicated below)						
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
Γο	2 4 10 15	S-1 : 0.0'- 2.0'	14	75	Dk. brown, m. dense f/c Sand, some Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)		
605 —	19 12 23 23 25	S-2 : 2.0'- 4.0'	35	75	grade: dense, little Gravel		
-5	2 16 16 16 16 16 19	S-3 : 4.0'- 6.0'	32	70	Same as S-2		
600 -	9 8 5 4	S-4 : 6.0'- 8.0'	13	50	grade: m. dense, some Gravel, wet		
†	4 5 5 6	S-5 : 8.0'- 10.0'	10	10	Brown, stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)		
595	5 7 17 10	S-6 : 10.0'- 12.0'	24	50	grade: v. stiff		
1 393							
<del>-</del> 15	4 6 6 6	S-7 : 14.0'- 16.0'	10	100	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)		
590 —							
-20	7 7 7 50/5"	S-8 : 19.0'- 20.4'	57+	50	grade: hard		
585 —	REFUSAL				Depth to Bottom of Hole: 20.5 feet		
+							
<del>-</del> 25							
580 —							
-30							
575 —							
<del>-</del> 35							

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

### **TEST BORING LOG**

**JOB No.**: 23-610 **BORING No.**: B-21

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/14/24 SURFACE ELEVATION (ft.): 603 est.

DATE COMPLETED: 2/14/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion unless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks				
T <sup>o</sup>	3 7 11 10 10 10 10 10 10 10 10 10 10 10 10	S-1 : 0.0'- 2.0'	18	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)				
600 —	6 9 13 12	S-2 : 2.0'- 4.0'	22	50	Same as S-1				
+ +5	12 8 6 5	S-3 : 4.0'- 6.0'	14	70	grade: Gray, some Silt, tr. Gravel Brown, stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist				
†	3 7 11 11	S-4 : 6.0'- 8.0'	18	75	(CL) grade: v. stiff				
595 <del> </del>  -	8 11 20 24	S-5 : 8.0'- 10.0'	31	100	grade: hard				
<del>-</del> 10	8 10 19 23	S-6 : 10.0'- 12.0'	29	100	grade: v. stiff				
590 —									
+ + 15 +	17 17 17 17 17	S-7 : 14.0'- 16.0'	34	0	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till) (No Recovery)				
585 —	REFUSAL				Depth to Bottom of Hole: 17.0 feet				
-20									
580 —									
-25									
575 —									
-30									
570 —									
+ + 35									
†									

### **TEST BORING LOG**

**BORING No.:** B-22

**JOB No.**: 23-610



**PROJECT:** 

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck **TYPE OF DRILL RIG:** CME-550 (ATV) **SAMPLING METHODS:** ASTM D1586 **SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A.

DATE STARTED: 2/13/24 SURFACE ELEVATION (ft.): 604 est.

DATE COMPLETED: 2/13/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

,	(measured at completion unless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks				
T 0	2 5 5 7 6 8 8 8	S-1 : 0.0'- 2.0'	12	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)				
†	4 7 9 11	S-2 : 2.0'- 4.0'	16	50	grade: wet				
600 + 5	2 3 5 5 5	S-3 : 4.0'- 6.0'	8	0	grade: loose (No Recovery)				
	5 5 6 6	S-4 : 6.0'- 8.0'	11	25	Dk. brown, stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)				
595	3 4 5 5	S-5 : 8.0'- 10.0'	9	100	grade: Brown, some f/c Sand, wet				
<del>-</del> 10 + +	5 5 7 7	S-6 : 10.0'- 12.0'	12	100	Gray, v. stiff CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)				
590 —	REFUSAL				Depth to Bottom of Hole: 13.0 feet				
+									
585									
-20									
580 —									
25									
575 - 30									
+									
570 —									
+ 35									

### **TEST BORING LOG**

**BORING No.:** B-23

**JOB No.:** 23-610



**PROJECT:** 

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/14/24 SURFACE ELEVATION (ft.): 602 est.

DATE COMPLETED: 2/14/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion dilless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks				
	8 111 13 16	S-1 : 0.0'- 2.0'	24	50	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)				
600 —	5 2 2 2 3 11 12	S-2 : 2.0'- 4.0'	20	50	Same as S-1				
+ +5	13 20 11 3	S-3 : 4.0'- 6.0'	31	50	grade: dense Gray, v. stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic,				
595 —	4 10 14 14	S-4 : 6.0'- 8.0'	24	75	moist (CL) grade: Brown, v. stiff, tr. f/c Sand				
<u> </u>	7 13 19 26	S-5 : 8.0'- 10.0'	31	100	grade: hard				
<del>-</del> 10	6 15 23 25	S-6 : 10.0'- 12.0'	38	100	Same as S-5				
590 —									
+ + 15	50/1"	S-7 : 14.0'- 14.1'	50+	0	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, wet (CL/ML-Till) (No Recovery)				
585 —	REFÜ\$AL				Depth to Bottom of Hole: 14.5 feet				
-									
20									
580 —									
†									
<del>-</del> 25									
575 <del> </del>  -									
-30									
570 <del>-</del>									
35									
'									

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823 **TEST BORING LOG** 

**JOB No.**: 23-610 **BORING No.**: B-24

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck **TYPE OF DRILL RIG:** CME-550 (ATV) **SAMPLING METHODS:** ASTM D1586 **SIZE AND TYPE OF BIT:** 2 1/4" I.D. H.S.A.

DATE STARTED: 2/12/24 SURFACE ELEVATION (ft.): 603 est.

DATE COMPLETED: 2/12/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion dilless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks				
Το	8 10 10 13 13 13 18	S-1 : 0.0'- 2.0'	23	65	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)				
600 +	7 7 7 7 9 11	S-2 : 2.0'- 4.0'	16	50	Same as S-1				
-5	2 2 2 2 3 2 3 2	S-3 : 4.0'- 6.0'	5	50	grade: loose, little Clay, little Silt, saturated				
†	5 5 6 5	S-4 : 6.0'- 8.0'	11	50	Brown, stiff CLAY, some Silt, some f/c Sand, tr. Gravel, mod. plastic, wet (CL)				
595 <del> </del>  -	7 12 22 24	S-5 : 8.0'- 10.0'	34	100	grade: hard, tr. f/c Sand, moist				
<del>-</del> 10	8 11 20 25	S-6 : 10.0'- 12.0'	31	100	Same as S-5				
590 —					(Depth of glacial till based upon adjacent borings.)				
15	 REF∐\$AL				Depth to Bottom of Hole: 14.0 feet				
585 —									
-20									
+									
580 —									
25									
575 <del>-</del>									
30									
570 —									
35									
<b>'</b>									



10440 MAIN STREET

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

JOB No.: 23-610 **BORING No.:** B/MW-25

MONITORING WELL SCHEMATIC

PROJECT: **Proposed Industrial Complex** 

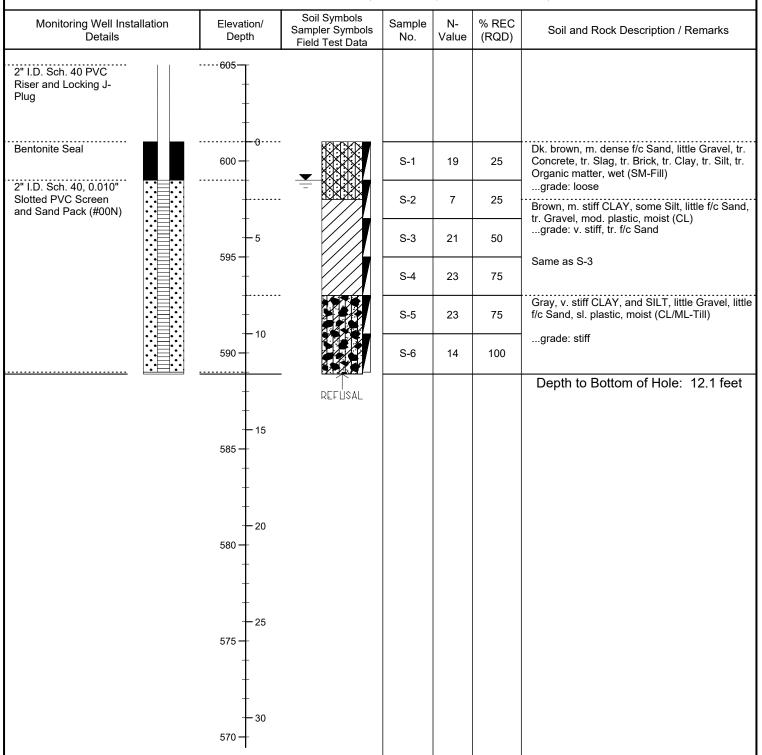
2 Bailey Avenue, City of Buffalo, New York 14220

**DRILLER:** D. Rimbeck TYPE OF DRILL RIG: CME-550 (ATV) 4 1/4" I.D. H.S.A. **SAMPLING METHODS: ASTM D1586 SIZE AND TYPE OF BIT:** 

**DATE STARTED:** 2/14/24 SURFACE ELEVATION (FT): 601 est.

**DATE COMPLETED:** 2/14/24 **GROUNDWATER DEPTH (FT):** 2.0 (See Notes)

(measured at completion unless indicated be



- 1. Groundwater level at 11.8 feet below ground surface (BGS) after installation on 2/14/24.
- 2. Well to be developed and groundwater levels recorded again on 3/26/24.
- 3. Groundwater level at 2.0 feet BGS (elevation 599.0 +/- feet) on 3/26/24.
- 4. Groundwater recharging approximately 6 inches per five minutes.

### 10440 MAIN STREET CLARENCE, NEW YORK

TEST BORING LOG

CLARENCE, NEW YORK 14031 **JOB No.:** 23-610 **BORING No.:** B-26 (716) 759-7821 FAX: (716) 759-7823

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/14/24 SURFACE ELEVATION (ft.): 604 est.

DATE COMPLETED: 2/14/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

(measured at completion unless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
Τ°	2 2 2 2 2 2 2 2 2 3 3 3 3 3 3 3 3 3 3 3	S-1 : 0.0'- 2.0'	4	25	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, wet (SM-Fill)			
1	2 3 4 4	S-2 : 2.0'- 4.0'	7	25	Same as S-1			
600 +	9 14 14 20	S-3 : 4.0'- 6.0'	28	75	Brown, v. stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)			
	10 19 21 33	S-4 : 6.0'- 8.0'	40	100	grade: hard			
595	8 12 20 25	S-5 : 8.0'- 10.0'	32	100	Same as S-4			
<del>-</del> 10	8 12 22 24	S-6 : 10.0'- 12.0'	34	100	Same as S-4			
590 —								
15	REFUSAL				Depth to Bottom of Hole: 13.5 feet			
585 —								
580 —								
25								
575 —								
570 —								
35								

### **TEST BORING LOG**

**BORING No.:** B-27

**JOB No.**: 23-610



**PROJECT:** 

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/15/24 SURFACE ELEVATION (ft.): 602 est.

DATE COMPLETED: 2/15/24 GROUNDWATER DEPTH (ft.): None (measured at completion unless indicated below)

	(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
Το	3 5 5 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	S-1 : 0.0'- 2.0'	10	25	Dk. brown, m. dense f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)			
600 —	17 50/5"	S-2 : 2.0'- 2.9'	50+	25	grade: v. dense			
+ +5	6 16 6 5	S-3 : 4.0'- 6.0'	22	25	Dk. brown, v. stiff CLAY, some Silt, little f/c Sand, tr. Gravel, mod. plastic, moist (CL)			
595 —	4 8 10 6	S-4 : 6.0'- 8.0'	18	25	grade: Brown, tr. f/c Sand, wet			
†	4 4 4 4	S-5 : 8.0'- 10.0'	8	75	grade: stiff			
<del>-</del> 10	4 5 4 5	S-6 : 10.0'- 12.0'	9	75	Same as S-5			
590 <del> </del>  -								
+ 15 	12 23 25 50/5"	S-7 : 14.0'- 15.9'	48	10	Gray, hard CLAY, and SILT, some Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)			
585 <del>-</del> -	REFUSAL				Depth to Bottom of Hole: 16.5 feet			
20								
580 —								
-								
- 25								
575 <del> </del>  -								
-30								
570 —								
35								
l '			1					

### 10440 MAIN STREET CLARENCE, NEW YO

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**JOB No.**: 23-610

**TEST BORING LOG** 

**BORING No.**: B-28

**PROJECT:** Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

DATE STARTED: 2/13/24 SURFACE ELEVATION (ft.): 608 est.

DATE COMPLETED: 2/13/24 GROUNDWATER DEPTH (ft.): None

(measured at completion unless indicated below)

(measured at completion unless indicated below)							
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
T 0	2 2 3 4 4 4 4	S-1 : 0.0'- 2.0'	7	30	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)		
605	2 2 4 5 5 5 5	S-2 : 2.0'- 4.0'	9	50	grade: saturated		
+ +5	6 8 14 15	S-3 : 4.0'- 6.0'	22	75	Brown, v. stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)		
	7 11 17 17	S-4 : 6.0'- 8.0'	28	75	Same as S-3		
600 —	9 16 18 18 23	S-5 : 8.0'- 10.0'	34	75	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)		
10	5 5 5 10 50/1"	S-6 : 10.0'- 11.6'	15	75	grade: v. stiff		
595 —	REFUSAL				Depth to Bottom of Hole: 11.7 feet		
15							
590 —							
<del>-</del> 20							
- 585 <del>-</del>							
+ + 25							
<b> </b>							
580 <del>-</del> -							
<del>-</del> 30							
575 <del>-</del>							
-35							
+							

## 10440 MAIN STREET CLARENCE, NEW YORK 14031

CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

PROJECT: Proposed Industrial Complex

2 Bailey Avenue, City of Buffalo, New York 14220

DRILLER:D. RimbeckTYPE OF DRILL RIG:CME-550 (ATV)SAMPLING METHODS:ASTM D1586SIZE AND TYPE OF BIT:2 1/4" I.D. H.S.A.

**TEST BORING LOG** 

BORING No.: B-29

**JOB No.**: 23-610

DATE STARTED:2/15/24SURFACE ELEVATION (ft.):600 est.DATE COMPLETED:2/15/24GROUNDWATER DEPTH (ft.):None

(measured at completion unless indicated below)								
Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks			
600 — 0	2 3 4 6	S-1 : 0.0'- 2.0'	7	50	Dk. brown, loose f/c Sand, little Gravel, tr. Concrete, tr. Slag, tr. Brick, tr. Clay, tr. Silt, tr. Organic matter, moist (SM-Fill)			
†	6 8 5 5	S-2 : 2.0'- 4.0'	13	50	Dk. brown, stiff Clay, some Silt, little f/c Sand, tr. Gravel, tr. Slag, mod. plastic, moist (CL-Fill)			
595 — 5	4 4 10 10	S-3 : 4.0'- 6.0'	14	75	Brown, stiff CLAY, some Silt, tr. Gravel, tr. f/c Sand, mod. plastic, moist (CL)			
†	16 24 24 26	S-4 : 6.0'- 8.0'	48	75	grade: hard			
†	11 15 17 17 17 50/2"	S-5 : 8.0'- 9.7'	32	75	Gray, hard CLAY, and SILT, little Gravel, little f/c Sand, sl. plastic, moist (CL/ML-Till)			
590 — 10	REFUSAL				Depth to Bottom of Hole: 10.0 feet			
585 — 15								
Ţ								
<u> </u>								
580 — 20								
†								
575 - 25								
†								
+								
570 + 30								
+								
565 - 35								
+								



CLARENCE, NEW YORK 14031

(716) 759-7821 FAX: (716) 759-7823

PROJECT: **Proposed Industrial Complex** 

2 Bailey Avenue, City of Buffalo, New York 14220

**TYPE OF DRILL RIG: DRILLER:** D. Rimbeck CME-550 (ATV) 4 1/4" I.D. H.S.A. **SAMPLING METHODS: ASTM D1586 SIZE AND TYPE OF BIT:** 

**DATE STARTED:** 2/15/24 SURFACE ELEVATION (FT): 599 est.

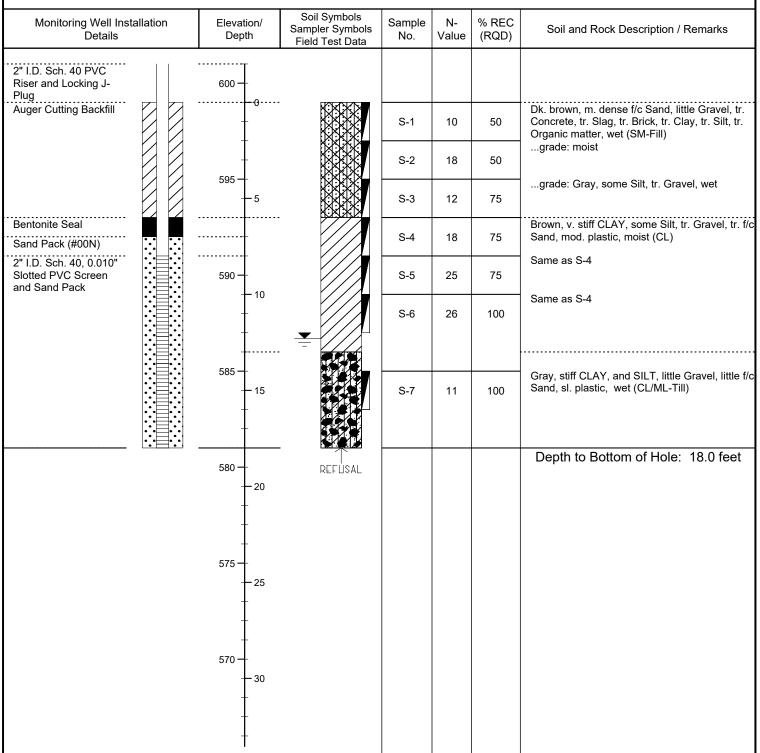
**DATE COMPLETED:** 2/15/24 **GROUNDWATER DEPTH (FT):** 12.3 (See Notes)

(measured at completion unless indicated be

JOB No.: 23-610

MONITORING WELL SCHEMATIC

**BORING No.:** B/MW-30



- 1. Groundwater level at 17.8 feet below ground surface (BGS) after installation on 2/15/24.
- 2. Well developed and groundwater levels recorded again on 3/26/24.
- 3. Groundwater level at 12.3 feet BGS (elevation 586.7 +/- feet) on 3/26/24.
- 4. Groundwater recharging approximately two inches per five minutes.



### Geotechnical Consulting and Special Inspections

10440 Main Street

(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX B**

### **GEOTECHNICAL REFERENCE STANDARDS**



### **Geotechnical Consulting and Special Inspections**

10440 Main Street Clarence, NY 14031 (716) 759-7821 www.barronandassociatespc.com

### **Column Header Descriptions**

BARRON & ASSOCIATES, P.C. & BUFFALO DRILLING COMPANY, INC.

10440 MAIN STREET CLARENCE, NEW YORK 14031 (716) 759-7821 FAX: (716) 759-7823

**PROJECT:** 

DRILLER:

SAMPLING METHODS: DATE STARTED: DATE COMPLETED:

### **TEST BORING LOG**

JOB No.:

**BORING No.:** 

PAGE: 1 of 3

EXAMPLE

TYPE OF DRILL RIG: SIZE AND TYPE OF BIT: SURFACE ELEVATION (ft.): GROUNDWATER DEPTH (ft.):

(measured at completion unless indicated below)

Elevation/ Depth (feet)	Soil Symbols Sampler Symbols Field Test Data	Sample No. : Range	N- Value	% REC (RQD)	Soil and Rock Description / Remarks		
100 — 0							
	1 1 1 2	S-1 : 0.0'- 2.0'	2	50	Dk. brown, soft, CLAY, some Silt, little f. Sand, tr. Gravel, tr. Roots, tr. Organic matter, mod. plastic, wet (CL)		
	6 8 11 14	S-2 : 2.0'- 4.0'	19	50	Brown, m. dense f. SAND, some Silt, tr. Gravel, tr. Clay, tr. Roots, non-plastic, moist (SM)		

Elevation/Depth: The depth column provides the vertical scale of the boring log in feet below ground surface.

<u>Sample No.: Range</u>: Disturbed samples are identified with "S" preceding the sample number. Undisturbed (Shelby-tube) samples are identified with "U" preceding the sample number. Rock core samples identified with "C" preceding the core run. The range of elevation/depth where the sample was taken is identified in one-tenth foot increments.

N-value: The Standard Penetration Test N-value, as specified by ASTM D1586, is defined as the number of blows required by a 140-pound hammer falling 30 inches, each blow, to drive a 2-in outside diameter split-spoon sampler 12 inches.

% REC (RQD): "Percent Recovery" is the length of samples recovered divided by the total length sampled. The result is numerically expressed as a percent. The Rock Quality Designation (RQD) is the total length of places >4 inches divided by the total length of core run.

Soil and Rock Description / Remarks: This column denotes the exact depth of recovery and general documentation of drilling efforts. Description and classification are based on visual inspection of samples and boring operations. The stratum lines shown on the boring logs are based upon interpretation and may not represent precise subsurface conditions. Water-level readings have been made in the drill holes at time under conditions stated on the boring logs. Fluctuations in the water level may occur due to other factors than those present the time measurement was taken. See attached sheet.



### **Geotechnical Consulting and Special Inspections**

10440 Main Street Clarence, NY 14031 (716) 759-7821 www.barronandassociatespc.com

PAGE: 2 of 3

### Soil and Rock Description / Remarks: Terminology Used for Soil Description

Number of Blows per ft., N   Relative Density   1-fine   V-very	Density Descri	iption of Granular Soil	Abbreviations Us	ed in Soil Sample Classification					
4 - 10	Number of Blows per ft., N	Relative Density	f - fine	v - very					
10 - 30	0 - 4	Very Loose	m - medium	gr - gray					
Number of Blows per ft N	4 - 10	Loose	c - coarse	bn - brown					
Over 50         Very Dense         tr - trace         dk - dark           Number of Blows per ft., N         Consistency         Parting         Less than 0.02 ft.           2 - 4         Soft         Thin bed         0.2 - 0.5 ft.           4 - 8         Medium         Medium bed         0.5 - 1.0 ft.           8 - 15         Stiff         Thick bed         1.0 - 2.0 ft.           5 - 30         Very Stiff         Meassive         Over 2.0 ft.           Very 30 ft Hard         Passing / Retained on         Very Stiff Very St	10 - 30	Medium	f/m - fine to medium	yel - yellow					
Consistency Description of Cohesive Soil         Parting         Less than 0.02 ft.           Number of Blows per ft., N         Consistency         Parting         Less than 0.02 ft.           2 - 4         Soft         Thin bed         0.2 - 0.5 ft.           4 - 8         Medium         Medium bed         0.5 - 1.0 ft.           4 - 8         Medium         Medium bed         1.0 - 2.0 ft.           15 - 30         Very Stiff         Massive         Over 2.0 ft.           15 - 30         Very Stiff         Massive         Over 2.0 ft.           0ver 30         Hard         Very Stiff         Can be scratched with from the stiff of the	30 - 50	Dense f/c - fine to coarse sl - slight							
Parting	Over 50	Very Dense	tr - trace	dk - dark					
Below 2	Consistency Des	cription of Cohesive Soil		Bedding					
2 - 4	Number of Blows per ft., N	Consistency	Consistency Parting Less than 0.02 ft.						
4 - 8 8 - 15 8 - 15 8 - 15 8 - 16 8 - 15 8 - 17 8 - 18 8 - 15 8 - 18 8	Below 2	Very Soft	Band	0.02 - 0.2 ft.					
8 - 15   Stiff   Thick bed   1.0 - 2.0 ft.   15 - 30   Very Stiff   Massive   Over 2.0 ft.   15 - 30   Very Stiff   Massive   Over 2.0 ft.   16 - 30   Very Stiff   Massive   Over 2.0 ft.	2 - 4	Soft	Thin bed	0.2 - 0.5 ft.					
15 - 30	4 - 8	Medium	Medium bed	0.5 - 1.0 ft.					
Over 30         Hand           Grain Size         Hardness           Boulder         LARGE / 1/2-in sieve         Soft Can be indented with thumb           Cobble         12-in / 3-in sieve         Moderately Hard         Can be scratched with fingernall           Cobble         12-in / 3-in sieve / No. 4 sieve         Hard         Difficulty to scratch with knife           Gravel         No. 40 sieve / No. 10 sieve         Very hard         Cannot be scratched with knife           Sand         No. 10 sieve / No. 40 sieve         Very hard         Cannot be scratched with knife           Sand         No. 10 sieve / No. 200 sieve         Some         Cannot be scratched with knife           Smaller than 0.005 mm           Percentage Terminology Used in Soil Classification           Trace         0 - 10 %         Some         20 - 35 %           Little         10 - 20 %         And         35 - 50 %           We will be specified to search the place of moisture, soil usually above groundwater level.           We will be specified to search the plasticity           Plasticity           Non-plastic Small part in thread cannot be fouch. Soil may be below groundwater level.           Sight plasticity         An 1/8-in thread c	8 - 15	Stiff	Thick bed	1.0 - 2.0 ft.					
Over 30         Hard           Fassing / Retained on         Very Soft or Plastic         Can be indented with thumb           Boulder         LARGE / 1/2-in sieve         Soft         Can be scratched with fingernall           Cobble         12-in / 3-in sieve         Moderately Hard         Can be scratched with knife           Gravel         3-in sieve / No. 4 sieve         Hard         Difficulty to scratch with knife           Sand         No. 10 sieve / No. 40 sieve         Very hard         Cannot be scratched with knife           Sand         No. 10 sieve / No. 200 sieve         Sore         Cannot be scratched with knife           Smaller than 0.005 mm           Percentage Terminology Used in Soil Classification           Trace         0 - 10 %         Some         20 - 35 %           Little         10 - 20 %         And         35 - 50 %           We with simple of moisture, dusty, dry to the touch.           Moisture         Small quantity of moisture, Soil usually above groundwater level.           Wet         Moisture noticeable to the touch. Soil may be below groundwater level.           Saturated         Visible free water, usually soil is below groundwater level.           Stight plasticity         Thread can be barley rolll	15 - 30	Very Stiff	Massive	Over 2.0 ft.					
Boulder LARGE / 12-in sieve Soft Can be indented with thumb Cobble 12-in / 3-in sieve Moderately Hard Can be scratched with fingernail Cobble 12-in / 3-in sieve Moderately Hard Can be scratched with Knife Gravel 3-in sieve / No. 4 sieve Hard Difficulty to scratch with knife No. 4 sieve / No. 10 sieve Very hard Cannot be scratched with knife No. 10 sieve / No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No.	Over 30	•							
Boulder LARGE / 12-in sieve Soft Can be indented with thumb Cobble 12-in / 3-in sieve Moderately Hard Can be scratched with fingernail Cobble 12-in / 3-in sieve Moderately Hard Can be scratched with Knife Gravel 3-in sieve / No. 4 sieve Hard Difficulty to scratch with knife No. 4 sieve / No. 10 sieve Very hard Cannot be scratched with knife No. 10 sieve / No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No. 200 sieve No. 40 sieve No.	G	Grain Size		Hardness					
Boulder LARGE / 12-in sieve Soft Can be scratched with fingermail Cobble 12-in / 3-in sieve Moderately Hard Can be scratched with knife Gravel 3-in sieve / No. 4 sieve Hard Difficulty to scratch with knife No. 4 sieve / No. 10 sieve No. 40 sieve No. 40 sieve No. 40 sieve / No. 200 sieve No. 40 sieve / No. 200 sieve Sitt No. 200 sieve / No. 200 sieve / No. 200 sieve Smaller than 0.005 mm  **Percentage Terminology Used in Soil Classification**  Trace 0.10 % Some 20.35 % And 35 - 50 % ***  **Precentage Terminology Used in Soil Classification**  Trace 0.10 % Some 20.35 % And 35 - 50 % ***  **Precentage Terminology Used in Soil Classification**  Trace No. 20 sieve / No. 20 sieve No. 40 siev			Very Soft or Plastic						
Cobble 12-in / 3-in sieve Moderately Hard Difficulty to scratched with knife Gravel 3-in sieve / No. 4 sieve Hard Difficulty to scratch with knife No. 4 sieve / No. 10 sieve Very hard Cannot be scratched with knife No. 10 sieve / No. 40 sieve / No. 40 sieve / No. 200 sieve Sitt No. 200 sieve / No. 200 sieve Sitt No. 200 sieve / No. 200 sieve Sitt No. 200 sieve / No. 200 sieve Smaller than 0.005 mm Some 20 - 35 % Little 10 - 20 % And 35 - 50 % Some 20 - 35 % Sieve Smaller than 0.005 mm Sieve Small quantity of moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level. Saturated Visible free water, usually soil is below groundwater level. Small quantity of moisture, dustly soil is below groundwater level. Saturated Non-plastic An 1/8-in thread cannot be rolled at any water content. Slight plasticity Thread can be barley rolled. Moderate plasticity Thread can be barley rolled. Moderate plasticity Thread can be barley rolled. Moderate plasticity Thread can be is required to reach the plastic limit (PL). Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL. Crystalline Crystals are to small they cannot be distinguished with the naked eye. Very Fine Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pithed Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug	Boulder	ŭ	•						
Gravel 3-in sieve / No. 4 sieve No. 10 sieve Very hard Cannot be scratched with knife No. 4 sieve / No. 10 sieve / No. 200 sieve No. 40 sieve / No. 200 sieve No. 40 sieve / No. 200 sieve / No. 200 sieve				_					
Sand No. 4 sieve / No. 10 sieve Very hard Cannot be scratched with knife No. 10 sieve / No. 40 sieve / No. 40 sieve / No. 200 sieve   Silt No. 200 sieve / 0.005 mm sieve   Clay Smaller than 0.005 mm   Percentage Terminology Used in Soil Classification   Trace			·						
Sand No. 10 sieve / No. 40 sieve No. 40 sieve / No. 200 sieve No. 40 sieve / No. 200 sieve Silt No. 200 sieve / 0.005 mm sieve Clay Smaller than 0.005 mm  Percentage Terminology Used in Soil Classification  Trace 0 - 10 % Some 20 - 35 % Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch. Moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level. Saturated Visible free water, usually soil is below groundwater level.  Vet Plasticity Non-plastic An 1/8-in thread cannot be rolled at any water content. Slight plasticity Thread can be barley rolled. Moderate plasticity Thread can be barley rolled. Considerable time is required to reach the plastic limit (PL). Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL. Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye. Crystalline Crystals barely discernable with the naked eye. Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline Smaller than pinhead. Their presence is indicated by the degree of absorbency. Plinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Glavei			•					
Silt No. 40 sieve / No. 200 sieve  No. 200 sieve / 0.005 mm sieve  Clay Smaller than 0.005 mm  Percentage Terminology Used in Soil Classification  Trace 0 - 10 % Some 20 - 35 %  Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch.  Moisture Small quantity of moisture. Soil usually above groundwater level.  Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystals are to small they cannot be distinguished with the naked eye.  Crystalline Crystals barely discernable with the naked eye.  Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Plithed Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Sand		very flate	Carnot be seraterica with Killie					
Silt No. 200 sieve / 0.005 mm sieve Smaller than 0.005 mm  Percentage Terminology Used in Soil Classification  Trace 0-10 % Some 20 - 35 % Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch. Moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Very Coarsely Crystalline Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Curia								
Clay  Percentage Terminology Used in Soil Classification  Trace 0 - 10 % Some 20 - 35 % Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch. Moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level. Saturated Visible free water, usually soil is below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Slight plasticity Non-plastic An 1/8-in thread cannot be rolled at any water content. Slight plasticity Thread can be barley rolled. Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL). Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye. Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Cilt								
Percentage Terminology Used in Soil Classification  Trace 0 - 10 % Some 20 - 35 % Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch. Moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level. Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content. Slight plasticity Thread can be barley rolled. Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL). Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye. Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pirted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.									
Trace 0 - 10 % Some 20 - 35 % Little 10 - 20 % And 35 - 50 %  Moisture  Dry Absence of moisture, dusty, dry to the touch. Moisture Small quantity of moisture. Soil usually above groundwater level. Wet Moisture noticeable to the touch. Soil may be below groundwater level. Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content. Slight plasticity Thread can be barley rolled. Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL). Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye. Crystalline Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Clay								
Little 10 - 20 % And 35 - 50 %    Moisture									
Dry Absence of moisture, dusty, dry to the touch.  Moisture Small quantity of moisture. Soil usually above groundwater level.  Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic  An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Plasticity Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline  Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.									
Dry Absence of moisture, dusty, dry to the touch.  Moisture Small quantity of moisture. Soil usually above groundwater level.  Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Little	10 - 20 %	And	35 - 50 %					
Moisture Small quantity of moisture. Soil usually above groundwater level.  Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystalsline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	<u>-</u>								
Wet Moisture noticeable to the touch. Soil may be below groundwater level.  Saturated Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystal line Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pithed Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	•	•							
Saturated  Visible free water, usually soil is below groundwater level.  Plasticity  Non-plastic  An 1/8-in thread cannot be rolled at any water content.  Slight plasticity  Thread can be barley rolled.  Moderate plasticity  Thread is easy to roll and little time is required to reach the plastic limit (PL).  Plasticity  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense  Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline  Crystals barely discernable with the naked eye.  Crystalline  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline  Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug  1/4 inch to the diameter of the core. The upper limit will vary with core size.									
Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Wet			evel.					
Non-plastic An 1/8-in thread cannot be rolled at any water content.  Slight plasticity Thread can be barley rolled.  Moderate plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Saturated	Visible free water, usually soil is	below groundwater level.						
Slight plasticity  Moderate plasticity  Thread is easy to roll and little time is required to reach the plastic limit (PL).  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense  Crystals are to small they cannot be distinguished with the naked eye.  Crystalline  Crystals barely discernable with the naked eye.  Crystalline  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline  Crystals larger than 1/8-in diameter.  Voids  Porous  Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted  Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug  1/4 inch to the diameter of the core. The upper limit will vary with core size.		l	Plasticity						
Moderate plasticity Plasticity Thread is easy to roll and little time is required to reach the plastic limit (PL). Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye. Crystals barely discernable with the naked eye. Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Non-plastic	An 1/8-in thread cannot be rolled	at any water content.						
Plasticity  Considerable time is required to reach PL. Thread can be re-rolled several times after reaching the PL.  Crystallinity or Texture  Dense  Crystals are to small they cannot be distinguished with the naked eye.  Crystals barely discernable with the naked eye.  Crystalline  Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline  Crystals larger than 1/8-in diameter.  Voids  Porous  Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted  Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug  1/4 inch to the diameter of the core. The upper limit will vary with core size.	Slight plasticity	Thread can be barley rolled.							
Crystallinity or Texture  Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Moderate plasticity	Thread is easy to roll and little tir	ne is required to reach the plastic	limit (PL).					
Dense Crystals are to small they cannot be distinguished with the naked eye.  Very Fine Crystalline Crystals barely discernable with the naked eye.  Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Plasticity	Considerable time is required to	reach PL. Thread can be re-rolled	several times after reaching the PL.					
Very Fine Crystalline Crystals barely discernable with the naked eye. Crystalline Crystals are medium size, up to 1/8-in diameter. Very Coarsely Crystalline  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency. Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb. Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.		Crystal	linity or Texture						
Crystalline Crystals are medium size, up to 1/8-in diameter.  Very Coarsely Crystalline Crystals larger than 1/8-in diameter.  Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Dense	Crystals are to small they cannot	be distinguished with the naked e	eye.					
Very Coarsely Crystalline  Crystals larger than 1/8-in diameter.  Voids  Porous  Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted  Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug  1/4 inch to the diameter of the core. The upper limit will vary with core size.	Very Fine Crystalline	Crystals barely discernable with	the naked eye.						
Voids  Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Crystalline	Crystals are medium size, up to	1/8-in diameter.						
Porous Smaller than pinhead. Their presence is indicated by the degree of absorbency.  Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Very Coarsely Crystalline	Crystals larger than 1/8-in diame	ter.						
Pitted Pinhead size to 1/4 in. If thin walls separate the individuals pits, the core may be described as honeycomb.  Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.			Voids						
Vug 1/4 inch to the diameter of the core. The upper limit will vary with core size.	Porous	Smaller than pinhead. Their pres	ence is indicated by the degree of	absorbency.					
	Pitted	Pinhead size to 1/4 in. If thin wal	Is separate the individuals pits, the	e core may be described as honeycomb.					
	Vug	1/4 inch to the diameter of the co	ore. The upper limit will vary with co	ore size.					
	· ·								



**Geotechnical Consulting and Special Inspections** 

10440 Main Street Clarence, NY 14031 (716) 759-7821 www.barronandassociatespc.com

PAGE: 3 of 3

### Soil Classification Chart: Unified Soil Classification System (USCS)

	Major Divisions		<u>Pattern</u>	USCS ID	Typical Descriptions
	Clean Gravels			GW	Well-graded gravels, gravel-sand mixtures, little or no fines
	<b>Gravels</b> : More than 50% of coarse	(little or no fines)		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
	fraction larger than No. 4 sieve	Gravels with appreciable		GM	Silty gravels, gravel-sand-silt mixtures
Coarse-Grained Soils: More than 50% of material		amounts of fines		GC	Clayey gravels, gravel-sand-silt mixtures
larger than No. 200 sieve		Clean sands		SW	Well-graded sands, gravelly sands, little or no fines
	Sands: Less than 50% of coarse	(little or no fines)		SP	Poorly-graded sands, gravelly sands, little or no fines
	fraction larger than No. 4 sieve	Sand with appreciable amount of fines		SM	Silty sands, silt-sand mixtures
				SC	Clayey sands, sand-clay mixtures
			ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity	
	Silts and Clays, Low plasticity: Liquid Limit < 50%			CL	Inorganic clays of low to medium plasticity, gravely clays, sandy clays, silty clays, lean clays
Fine-Grained Soils: Less than 50% of material			OL	Organic silts and organic silty clays of low plasticity	
larger than No. 200 sieve				МН	Inorganic silts, micaceous or diatomaceous fine sand or silty soils
	Silts and Clays, Liquid Lir			СН	Inorganic clays of high plasticity, fat clays
				ОН	Organic clays of medium to high plasticity, organic silts
	Highly Organic Soils			Pt	Peat, humus, swamp soils with organic contents
	Miscellaneous Fill			FILL	Miscellaneous fill may belong in any division but is identified as FILL



### Geotechnical Consulting and Special Inspections

10440 Main Street

(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX C**

LABORATORY SOIL TEST RESULTS



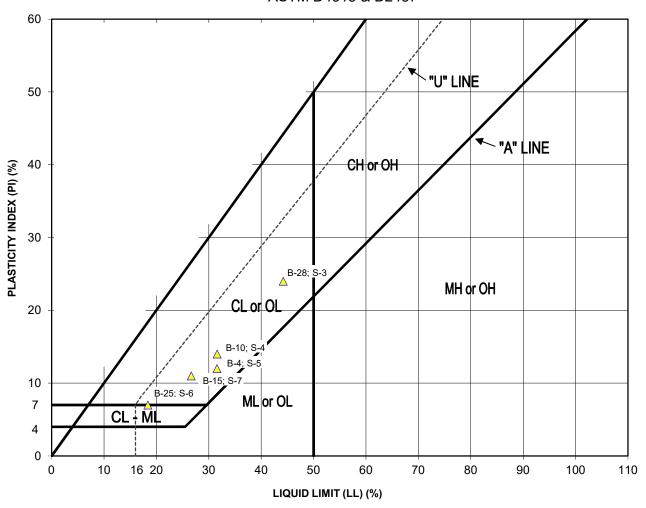
Job No: 23-610

Project: Proposed Industrial Complex

2 Bailey Avenue

City of Buffalo, Erie Co., New York 14206

### Plasticity Chart ASTM D4318 & D2487



Boring	Sample	Depth	LL	PL	PI
No.	No.	(ft.)	(%)	(%)	(%)
B-4	S-5	8-10	32	20	12
B-10	S-4	6-8	32	18	14
B-15	S-7	14-16	27	16	11
B-25	S-6	10-12	18	11	7
B-28	S-3	4-6	44	20	24

# BARRON & ASSOCIATES, P.C. Geotechnical Consulting and Special Inspections 10440 Main Street (716) 759-7821

Clarence, NY 14031

(716) 759-7821 www.barronandassociatespc.com Job No: 23-610

Project: Proposed Industrial Complex

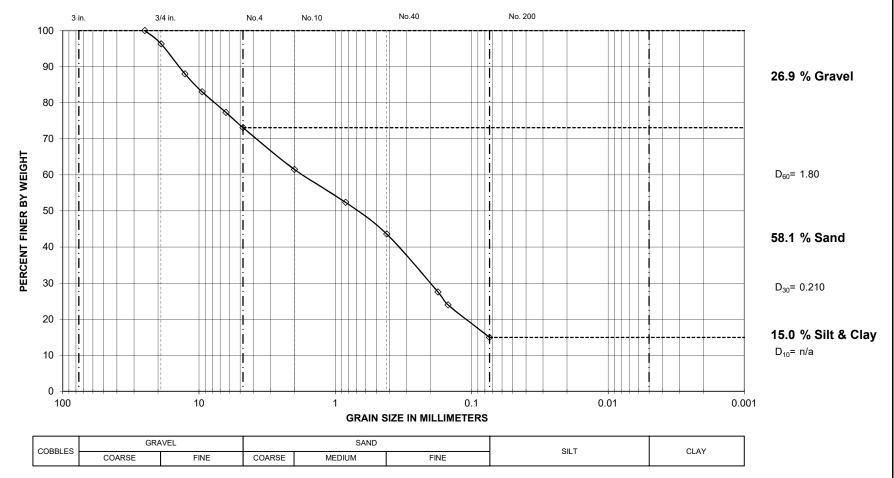
2 Bailey Avenue

City of Buffalo, Erie Co., NY 14206

**GRAIN SIZE ANALYSIS ASTM C136** 

C<sub>C</sub>= NA C<sub>U</sub>= NA LL= NA PL= NA PI= NA USCS= SM-Fill

#### U.S. STANDARD SIEVE SIZE



Date Tested: March 15, 2024 Boring No.: B-8 Sample No.: S-1 to S-3 Depth: 0 to 6 ft.

# Barron & Associates, P.C. Geotechnical Consulting and Special Inspections 10440 Main Street (716) 759-7821

10440 Main Street (716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com Job No: 23-610

Project: Proposed Industrial Complex

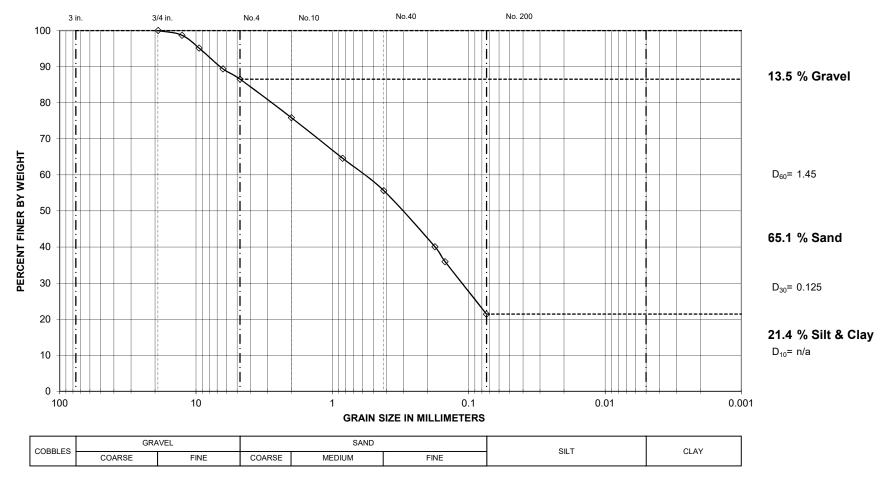
2 Bailey Avenue

City of Buffalo, Erie Co., NY 14206

**GRAIN SIZE ANALYSIS ASTM C136** 

C<sub>C</sub>= NA C<sub>U</sub>= NA LL= NA PL= NA PI= NA USCS= SM-Fill

#### U.S. STANDARD SIEVE SIZE



Date Tested: March 15, 2024 Boring No.: B-24 Sample No.: S-1 to S-3 Depth: 0 to 6 ft.

# BARRON & ASSOCIATES, P.C. Geotechnical Consulting and Special Inspections

10440 Main Street (716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com Job No: 23-610

Project: Proposed Industrial Complex

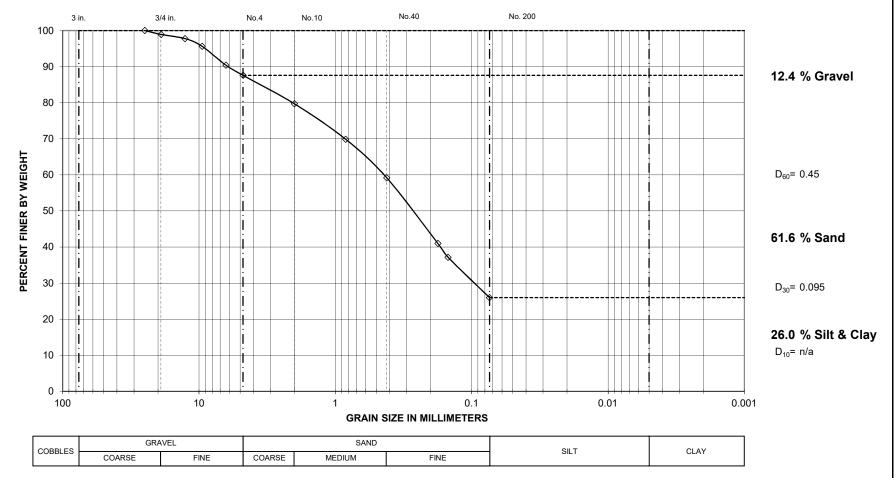
2 Bailey Avenue

City of Buffalo, Erie Co., NY 14206

### **GRAIN SIZE ANALYSIS ASTM C136**

C<sub>C</sub>= NA C<sub>U</sub>= NA LL= NA PL= NA PI= NA USCS= SM-Fill

#### U.S. STANDARD SIEVE SIZE



Date Tested: March 15, 2024 Boring No.: B-30 Sample No.: S-1 to S-3 Depth: 0 to 6 ft.

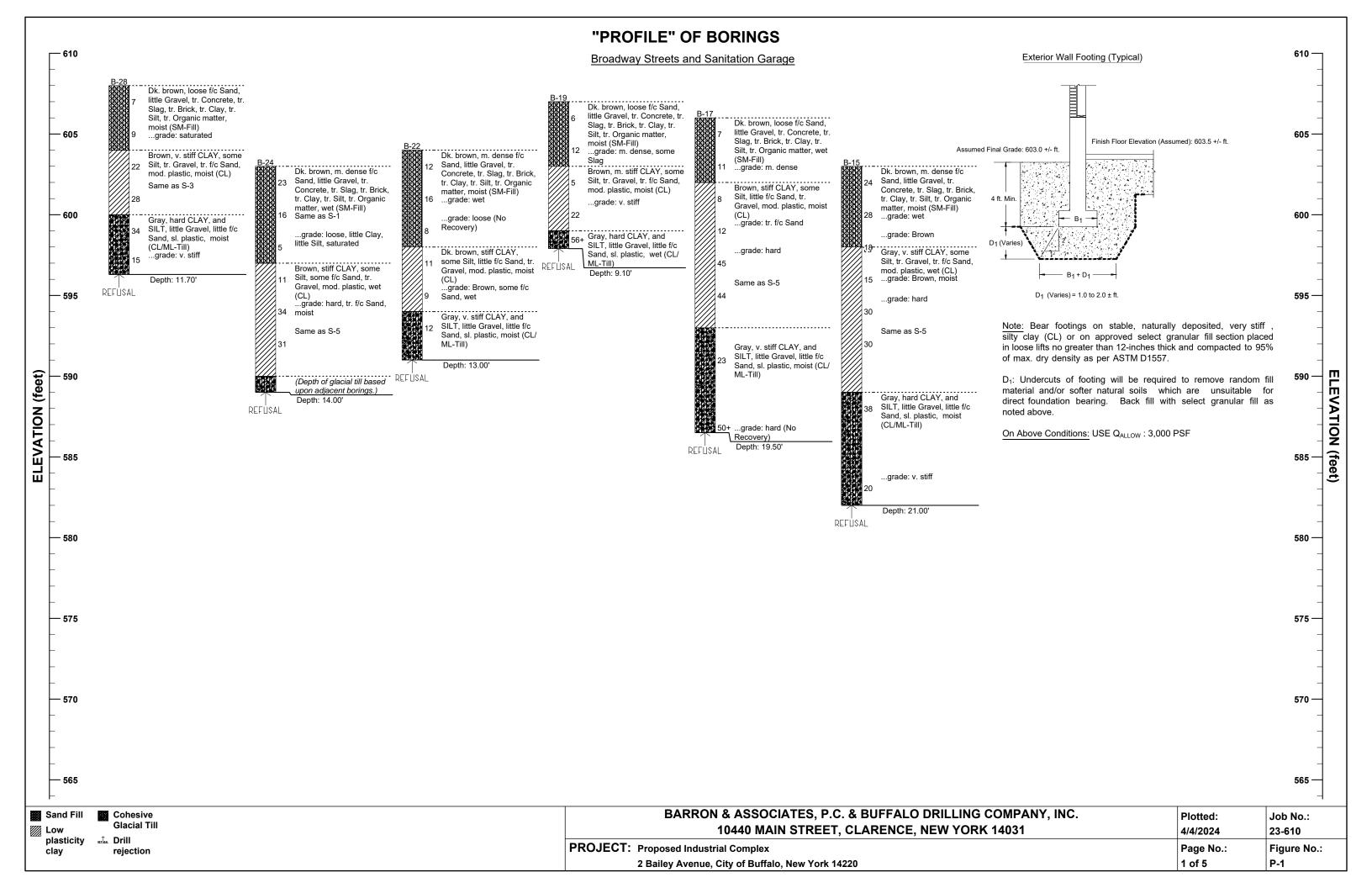
### Geotechnical Consulting and Special Inspections

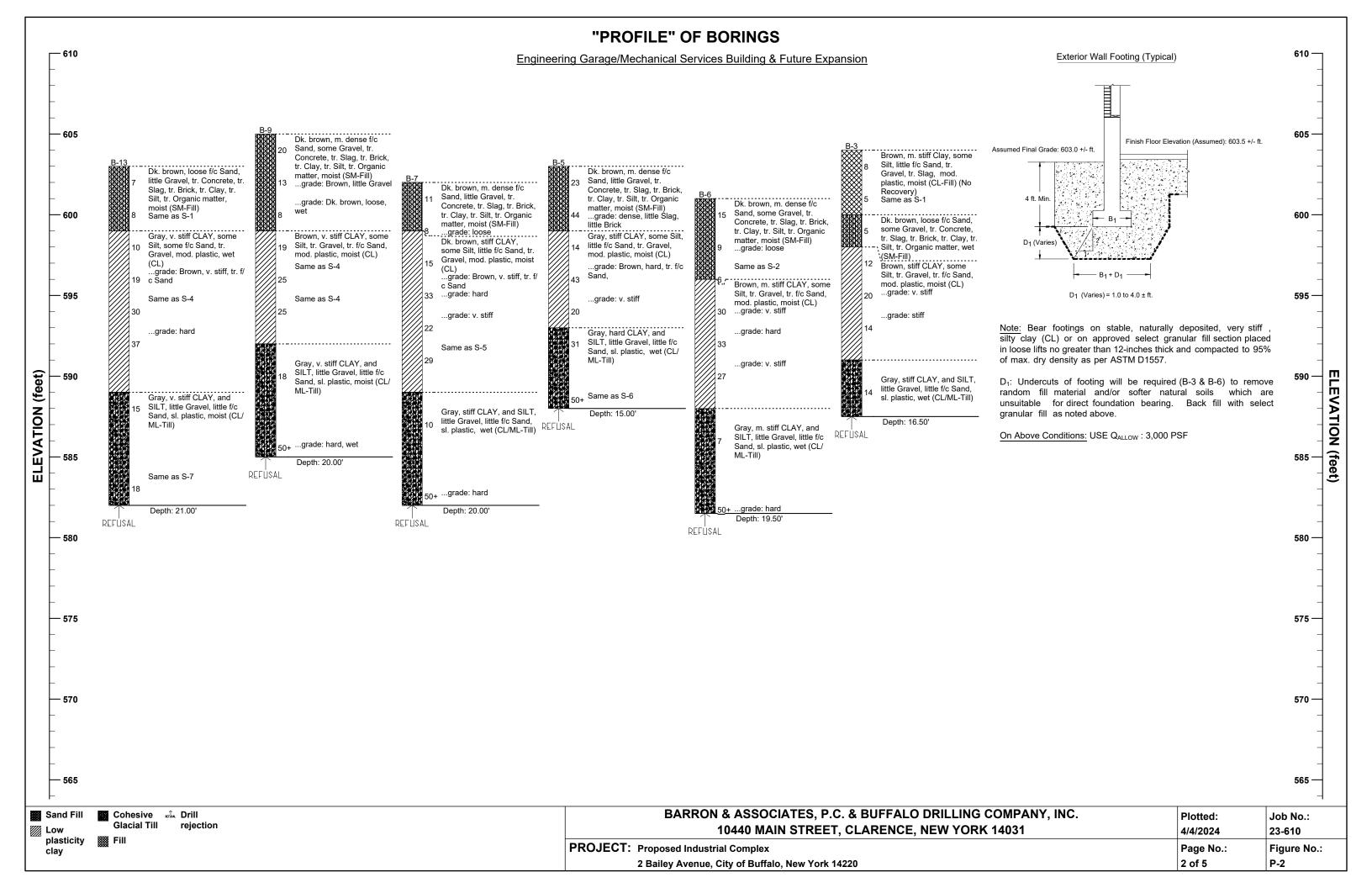
10440 Main Street

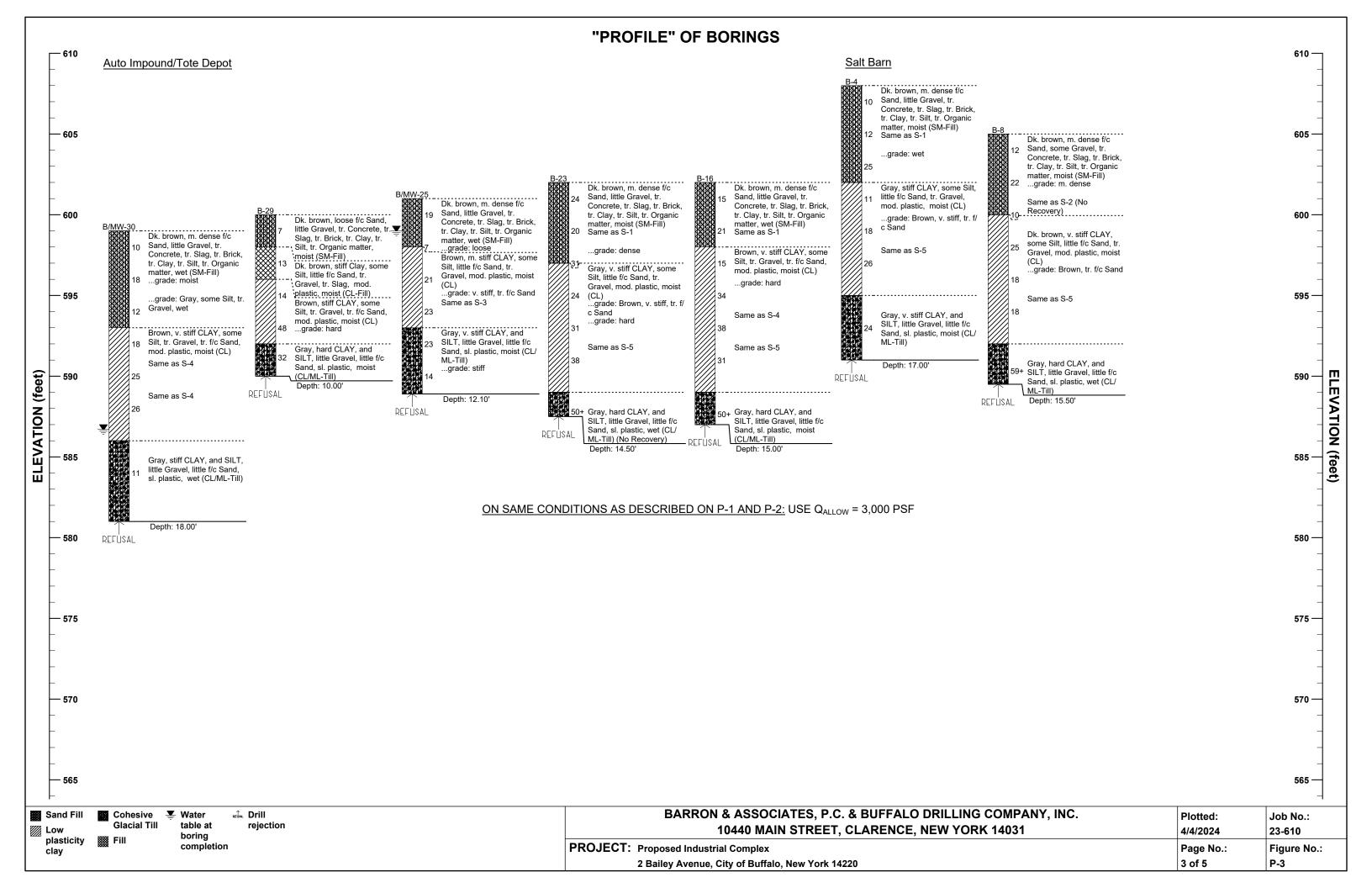
(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX D**

### **ENGINEERING COMPUTATIONS AND SCHEMATICS**





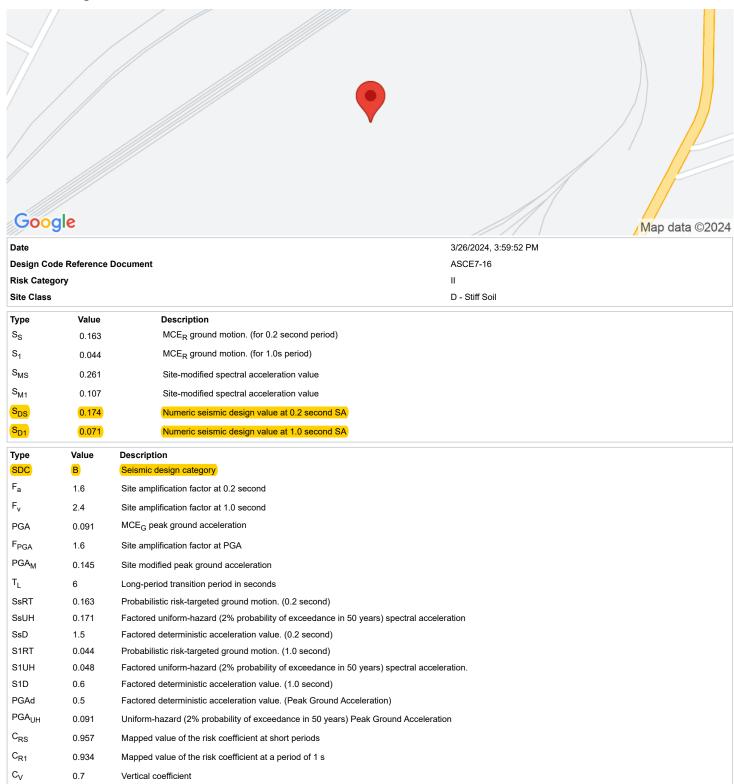






### Proposed Industrial Complex, Bailey Ave., Buffalo, NY 14206

Latitude, Longitude: 42.8800, -78.8205



B&A JOB NO.:	23-610		
DATE:	03/20/24		

PAGE: 5 of 5

## LATERAL EARTH PRESSURE ON GENERIC BLOCK FOUNDATIONS FOR SIGNS, FREE-STANDING RETAINING WALLS, OR BELOW GRADE/BASEMENT/TANK/POOL RETAINING WALLS (Less Than 20 Feet High)

- a) Porous filter media, in contact with the basement/below grade foundation wall or retaining walls, protects and is in contact with a minimum 4 inch diameter perforated drainage pipes at the footing/base of the foundation/structural wall (exterior backfill side and interior basement side) and/or weep pipes through the wall, as needed and as applicable.
- b) Where recommended, a geotextile filter fabric will protect the gravel filter media from the earth backfill. Overlap unsewn seams as per the manufacturer's recommendations.
- c) Waterproof earth side of wall, as is customarily provided in practice.

Clarence, NY 14031

- d) Drainage pipes are connected to an appropriately designed collector pipe, conveyance, and/or sump pump system as is applicable for the intended purpose of the wall and as customarily provided/installed in practice.
- e) For potential groundwater table conditions above the top of the basement slab-on-grade condition, install continuous waterstops (with no joints in stop) at wall and floor construction joints, as is customarily provided in practice. Interior intermediate drainage pipes beneath the slab, that are spaced on-center and in both directions, do appear to be needed.
- f) Assume a uniformly graded, clean coarse sand or sandy gravel backfills:
  - \* equivalent N-value in a dense state:  $(N_1)_{60} = 40$  blows/foot
  - \* friction angle:  $\phi' = 38$  degrees {Teng, pg 12}

www.barronandassociatespc.com

- average in-place densities: moist  $\gamma_m$  = 120 pcf
  - saturated  $\gamma_{sat}$  = 132 pcf submerged  $\gamma$ ' =  $(\gamma_{sat} \gamma_{w})$  = 70 pcf
- g) Assume at base of wall/footing, coefficient of friction against sliding ( $f_s$ ) at base of wall (Refer to Teng, pg. 320-1):
  - $f_s = tan(0.58 \times \phi^{\perp}) = 0.40$  (AREA silty soil to silty coarse-grained soil)
- h) Use equivalent fluid pressure design approach (Hough, pg. 249 and NAVFAC pg. 7-10-9):
  - \* at rest pressure coefficient  $K_0 = 1 \sin(\phi') \neq 0.38$ \* effective lateral pressure of soil -  $\gamma'_1 = K_0 \times \gamma'_1 \in 26.6$  pcf
  - \* hydrostatic pressure  $\gamma_{\rm w}$  = 62.4 pef.
  - \* equivalent fluid pressure with water level  $\gamma_{eo} = \gamma'_1 + \gamma_{eo} = 89 \text{ pcf (say 90 pcf)}$  at the top of the grade at the wall
  - \* equivalent fluid pressure with compaction  $\gamma_{eo} = 2 \times K_0 \times \gamma_m = 91 \text{ pcf (say 90 pcf)}$  induced lateral stress increase (W&F, pg 409)

  - $\gamma_{ep} = K_p \times \gamma_m = 504 \text{ pcf (say 330 pcf with a F.S. = 1.5)}$

	<u>I hore</u>	oughly Compacted ⇒⇒ ·	٠,	Uniformly Graded & Clean	Non-Plastic Silty Sa	nd
				Coarse Sand or Sandy Gravel Fill	or Sandy Silt Fill	
USE:	Earth Pressure Coefficient	Static Active	=	0.24	0.33	
		Static At-Rest	=	0.76	1.00	
		Static Passive	=	2.80	2.00	(with F.S. = $1.5$ )
		Static Passive	=	4.20	3.00	(with F.S. = $1.0$ )
USE:	Equivalent Fluid Pressure	Static Active	=	30 pcf		
		Static At-Rest	=	90 pcf (for rigid walls)		
		Static Passive	=	330 pcf (with F.S. = 1.5)	[ 500 pcf with (with I	F.S. = 1.0)]

[For earthquakes, structural engineer may elect to use the above Static Passive case instead of the below Earthquake Lateral Load for Non-Yielding Wall movement into the soil backfill.]

USE: Simplified Model for Earthquake Lateral Load/Ft. Wall Length

 $H_{bw}$  = Earth Height Behind Wall (feet)

@ 0.6  $H_{\text{bw}}$  above base. Loads for Non-Yielding Wall.

Reduce load by 33% for Yielding Wall (active case) =  $(6.8 \text{ psf/foot}) \times H_{bw}^2$  (NYS,  $S_{ds} = 0.25 \text{ g}$ )  $(\gamma_m = 120 \text{ pcr}^2 \text{ for } S_{ds}^2 \text{ value}$ . Add to Static At Rest/ =  $(13.5 \text{ psf/foot}) \times H_{bw}^2$  (NYS,  $S_{ds} = 0.50 \text{ g}$ ) Active Pressure/Load for unsaturated backfill case) =  $(17.5 \text{ psf/foot}) \times H_{bw}^2$  (NEern NYS,  $S_{ds} \le 0.65 \text{ g}$ )

[Above for Site Class C to E soils. Interpolate for other S<sub>ds</sub> values.

Use 1,75 × values for walls on Class B/A rock or on rigid foundation base. (FEMA NEHRP Guidelines)]

Saturated/Liquified Soil During Earthquake = 132 pcf

(Equivalent Fluid Pressure. Add to inertial hydrodynamic pressure, not presented here.)

For looser/denser backfills, adjust above pressures by the ratio = new density / 120 pcf or / 132 pcf (for saturated case)}

USE: Coefficient of Friction Against Sliding ( $f_s$ ) = 0.45 (on compacted NYSDOT Item #304.12 or #304.14 gravels)

(use-lowest f<sub>s</sub> with no underlying weaker layers) = 0.35 (on compacted granular soil & non-plastic silt) = 0.20 (slab-on-grade on polyethylene on granular fill)

[with a F.S. = 1.0] = 0.60/0.50 (on clean, rough, & sound bedrock/smooth bedrock)

Min. Factor of Safety Against Sliding = 1.5 [Rev.9-30-09]



### Geotechnical Consulting and Special Inspections

10440 Main Street

(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX E**

### **GENERAL EARTHWORK SPECIFICATION**



### GENERAL EARTHWORK SPECIFICATIONS

### 1. GENERAL

### 1.1. SITE AND SUBSURFACE CONDITIONS

#### 1.1.1. Overview

This specification is included as a courtesy to the clients of *Barron & Associates, PC* and addresses earthwork site preparation. Additions and modifications are necessary to create a job-specific specification. This specification may serve as a basis for the development for a technical specification under Division 31 (Earthwork) and Division 32 (Exterior Improvements).

#### 1.1.2. Site Conditions

The site-specific conditions are described under separate cover or may be available from the OWNER.

### 1.1.3. Subsurface Conditions

The site-specific subsurface conditions are described under separate cover or may be available from the OWNER.

#### 1.2. REFERENCES

### 1.2.1. American Standard for Testing and Measurement (ASTM)

•	ASTM C136	Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
•	ASTM D6938	Standard Test Methods for In-Place Density and Water Content of Soil and
		Soil-Aggregate by Nuclear Methods (Shallow Depth)
•	ASTM D7928	Standard Test Method for Particle-Size Distribution (Gradation) of Fine-
		Grained Soils Using the Sedimentation (Hydrometer) Analysis
•	ASTM D1140	Standard Test Methods for Determining the Amount of Material Finer than
		75-µm (No. 200) Sieve in Soils by Washing
•	ASTM D1557	Standard Test Methods for Laboratory Compaction Characteristics of Soil
		Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³))
•	ASTM D2216	Standard Test Methods for Laboratory Determination of Water (Moisture)
		Content of Soil and Rock by Mass
•	ASTM D2487	Standard Practice for Classification of Soils for Engineering Purposes (Unified
		Soil Classification System)

 ASTM D4318 Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

 ASTM D1883 Standard Test Method for California Bearing Ratio (CBR) of Laboratory-Compacted Soils

### 1.2.2. Other

- 2018 International Building Code (IBC)
- 2020 Building Code of New York State
- 2020 Residential Code of New York State
- 2021 MasterSpec

#### 1.3. DEFINITIONS

### 1.3.1. Unacceptable Material

Soil material containing debris, wood, scrap material, vegetation, refuse, soft unsound particles, and other organic, frozen, deleterious, or objectionable materials. Contaminated soils shall be properly documented and removed or remediated on site. If necessary, remediation procedure will be defined by the OWNER.

#### 1.3.2. Unsuitable Material

Brown, organic topsoil and underlying soft pockets of organic silt or wet, reworked silty clay.

### 1.3.3. Ordinary Fill

Friable soil containing no stone greater than two-thirds loose lift thickness and no unacceptable or unsuitable materials. In general, existing random fill is expected to be acceptable for reuse as ordinary fill given proper sorting, blending, drying, and controlled placement methods.

#### 1.3.4. Granular Fill

Ordinary fill meeting the designation of ASTM D2487 classification of GW with a maximum of 10 percent by weight passing ASTM D1140, No. 200 sieve.

#### 1.3.5. Select Granular Fill

Clean, uncoated soil which contains no unacceptable materials and conforms to the gradation requirements defined in <u>Table A: Select Granular Fill</u>.

Table A: Select Granular Fill			
Sieve Size	Percent Finer by Weight		
<sup>2</sup> ∕₃ of the loose lift thickness	100		
No. 10	30 - 95		
No. 40	10 - 70		
No. 200	0 - 15		

4/23/2021 Page 2 of 8

#### 1.3.6. Sand and Gravel

Clean, hard, durable, uncoated particle of sand and gravel, free from lumps of clay, containing no unacceptable matter, and conforming to gradation requirements of <u>Table B: Sand and Gravel</u>.

Table B: Sand and Gravel			
Sieve Size	Percent Finer by Weight		
*	100		
No. 4	50 - 85		
No. 10			
No. 40	10 - 35		
No. 100			
No. 200	0 - 8		

<sup>\*</sup> Job-Specific. To be determined by the ENGINEER

#### 1.3.7. Crushed Stone

Clean, durable, sharp-angled fragments of rock or crushed gravel stone of uniform quality, containing no unacceptable matter, free from coatings, and conforming to gradation requirements of <u>Table C: Crushed Stone</u>.

Table C: Crushed Stone					
Sieve Size	Percent Fir	Percent Finer by Weight			
	3/4-inch Stone	1 1/4-inch Stone			
1 ½-inch		100			
1 1/4-inch		85 - 100			
1-inch	100				
¾-inch	90 - 100	10 - 40			
5%-inch					
½-inch	10 - 50	0 - 8			
³%-inch	0 - 20				
No. 4	0 - 5				

### 1.3.8. Flowable Fill

Also known as Controlled Low Strength Material – Controlled Density Fill (CLSM-CDF), this material is available under a variety of producer names (e.g., K-Krete©, M-Crete, Flash Fill©, Flowable Mortar, Unshrinkable Fill, etc.). This non-settling backfill mixture is most commonly used for its flowable characteristics, its support strength under traffic loads, and its removability at a later date. The material may be produced on-site or off-site. In either case, the producer

4/23/2021 Page 3 of 8

of such materials and the product must meet certain certification criteria. Such information is beyond the scope of this specification and will be considered on a site-specific basis.

Flowable fill may be acceptable for use as a backfill for utility trenches of other low-lying areas which require a compacted granular fill. Its use and warranty of performance is left to the CONTRACTOR in such applications.

The use of flowable fill under load-bearing structural components in place of properly placed and compacted granular fill is NOT common and is questionable. The localized use of such material may have profound effects on the performance of a foundation system. Site-specific conditions and the extent of anticipated use of flowable fill must be examined by geotechnical engineer. Cost of such consultation shall be borne by the CONTRACTOR unless specifically directed by the OWNER to seek such consultation. Without such consultation, warranty of performance for such use is left to the CONTRACTOR.

#### 1.4. SUBMITTALS

The following submittals shall be provided in accordance with approved submittals procedures.

- (1) Fill Source: Provide name and source locations of fill material.
- (2) Field Test Reports: Field tests will be performed by OWNER's Representative as needed. CONTRACTOR may be required to perform such tests on proposed off-site fill materials.
  - (a) Fill material grain size analyses per ASTM C136, D7928, D1140, D2487
  - (b) Moisture/Density test results per ASTM D2216
  - (c) Liquid limit, plastic limit, and plasticity index per ASTM D4318
  - (d) Compaction/Density test results per ASTM D6938 and D1557
- (3) Sample: Geotextile fabric

#### 2. PRODUCTS

Geotextile Fabric: Mirafi 600X or equal. (Also referred to as synthetic fabric).

### 3. EXECUTION

#### 3.1. PROTECTION

#### 3.1.1. General

Manner of excavation shall minimize disturbance of underlying natural ground. If deemed necessary by the Engineer, alter construction procedures to reduce subgrade disturbance.

4/23/2021 Page 4 of 8

Excavate areas which have been excessively disturbed to firm ground and backfill with properly compacted granular fill.

### 3.1.2. Roads and Walks

Keep roads and walks free of dirt and debris at all times.

### 3.1.3. Trees, Shrubs, and Existing Facilities

Protect from any damage all vegetation and facilities identified to remain.

#### 3.1.4. Utility Lines

Locate all utilities within the area of disturbance prior to the start of work. Show locations on initial plans. Protect utility lines from damage. Notify the ENGINEER immediately of damage to or an encounter with an unknown utility. Damage to utility lines are to be repaired by the CONTRACTOR at no additional cost. The CONTRACTOR shall have underground utility owners stake out utility locations prior to the start of clearing and excavation operations.

### 3.2. VERIFICATION OF CONDITIONS / PROOF-ROLLING

Prior to placement of the initial layer of fill over the natural ground, proof-roll the exposed natural ground above the groundwater table elevation by making two passes with a fully-loaded ten-wheel truck. Excavate unstable areas detected by this process and replace with compacted granular fill.

#### 3.3. PREPARATION

#### 3.3.1. Surface Preparation

Within the site limits indicated on the drawings, excavate all unsuitable material to firm natural ground in the manner specified herein. Follow a construction procedure which permits visual identification of firm natural ground. In the even that groundwater is encountered, the ENGINEER may require that the size of the open excavation be limited to that which can be handled by open pumping to allow visual inspection of the excavation bottom and the performance of backfill operations to be conducted in a dry state.

Excavation of unsuitable material shall be limited to the greater of the following:

- A distance of 5 feet beyond building lines or
- The area defined by a one-horizontal to one-vertical line sloping down from the outside bottom edge of exterior footings to firm natural ground.

4/23/2021 Page 5 of 8

#### 3.4. PLACEMENT AND COMPACTION

#### 3.4.1. General

Place fill in accordance with <u>Table D: Compaction Alternatives</u>. These alternatives are provided as minimum compaction standards only and in no way relieve the CONTRACTOR of his obligation to achieve any specified degree of compaction by whatever means may be necessary.

Grade to provide positive drainage and a smooth surface which will readily shed water. To the extent practicable, compact each layer to the specified density on the same day placed. Place fill in horizontal layers. Where horizontal layers meet a natural slope, key layer into slope by cutting a bench.

Fill that is too wet for proper compaction: Disc, harrow, or otherwise dry to proper moisture content for compaction to the required density.

Fill that is too dry for proper compaction: Uniformly apply water over the surface of the loose layer in sufficient quantity to allow compaction to the required density.

Table D: Compaction Alternatives					
L Compaction Method L	Maximum Stone Size	Maximum Loose Lift Thickness (inches)		Maximum Number of Passes	
		Below Structure and Pavements	Less-Critical Areas	Below Structure and Pavements	Less-Critical Areas
Hand operated vibratory plate of light roller in confined areas.	3	4	4	4	4
Hand operated vibratory drum rollers weighing at least 1,000 pounds in confined areas.	4	6	8	4	4
Loaded 10-wheel truck or D-8 crawler.	6	10	12	4	2
Light vibratory drum roller; Min. weight at drum 8,000lbs; Min. dynamic force 10,000lbs.	8	12	12	6	2
Minimum vibratory drum; Min. weight at drum 10,000lbs; Min. dynamic force 20,000lbs.	8	18	18	6	4

4/23/2021 Page 6 of 8

### 3.4.2. Dewatering

Provide adequate pumping and drainage facilities to keep excavated areas sufficiently dry of groundwater and surface run-off. Dewatering shall avoid adversely affection construction procedures or causing excessive disturbance of underlying natural ground. Drain all pumped water in such a manner as to avoid damage to adjacent property.

If requested by the ENGINEER, place a 6-inch to 12-inch layer of sand and gravel or crushed stone over the natural underlying soil to stabilize area which have been disturbed due to groundwater seepage pressures and to expedite dewatering operations. Particular attention shall be given areas under proposed foundations.

### 3.5. FIELD QUALITY CONTROL

### 3.5.1. Compaction Requirements

Allow the ENGINEER sufficient time to make necessary observations and tests. Base the degree of compaction on maximum dry density as determined by ASTM D1557. The minimum degree of compaction for placed fill shall be as indicated in <u>Table E: Compaction</u> Requirements.

Table E: Compaction Requirements			
<u>Area</u>	Minimum Degree of Compaction (%)		
Below foundation.	95		
Pavement and building subbase and base courses.	95		
Below building slab base course and above bottom of foundation.	92		
Below pavement subbase and base courses.	90		
Trench backfill outside of building.	90		
Trench backfill inside of building.	Refer to one of the above- listed categories		
Ordinary fill within 5 feet of grade.	90		
Vegetated areas below 5 feet of grade.	85		

#### 3.5.2. Testing

Site work should be monitored and tested by geotechnical ENGINEER or his representative and in accordance with requirements of the design team to assure compliance with earthwork and foundation construction specifications.

4/23/2021 Page 7 of 8

The owner will retain a geotechnical ENGINEER or his representative to perform on-site observations and testing during this phase of construction operations. The geotechnical ENGINEER or his representative will:

- Observe excavation and dewatering of building and controlled fill areas;
- Observe backfill and compaction within building and controlled fill areas;
- Laboratory test and analyze fill material; and
- Observe construction and performing water content, gradation, and compaction tests.

On a timely basis, the CONTRACTOR will receive copies of test results submitted to the OWNER. In addition, during construction the geotechnical ENGINEER will advise the OWNER and CONTRACTOR in writing of conditions which fail to conform to the Contract Documents. The CONTRACTOR shall take immediate action to remedy indicated deficiencies.

The geotechnical ENGINEER or his representative will not supervise or direct the actual work of the CONTRACTOR or employees and representatives of the CONTRACTOR. The presence of, observations by, and testing performed by the geotechnical ENGINEER or his representatives shall not excuse the CONTRACTOR from defects discovered in the work.

4/23/2021 Page 8 of 8



### **Geotechnical Consulting and Special Inspections**

10440 Main Street

(716) 759-7821 Clarence, NY 14031 www.barronandassociatespc.com

### **APPENDIX F**

**LIMITATIONS** 

#### LIMITATIONS

- 1. This report is based on the data that was obtained from the subsurface explorations and on the design of the proposed industrial complex as submitted to the geotechnical engineer. A geotechnical engineer, who is experienced in foundation construction and earthwork, should be engaged to review the final design and specifications in order to determine whether any change in concept may have any effect on the validity of the conclusions presented herein, and whether these conclusions have, in fact, been implemented in the design and specifications.
- 2. The subsurface conditions, including thickness, between the exploration locations are approximate and simplified representations of the strata and transitions. There is the possibility that variations in soil and rock conditions and boundaries will be encountered during construction. In order to permit correlation between the exploratory soil data and the actual soil conditions encountered during construction and so as to assess conformance with the plans and specifications as originally contemplated, it is recommended that a geotechnical engineer, who is experienced in foundation construction and earthwork monitoring, should be retained to perform continuous construction review during the site preparation and foundation construction operations.
- 3. The subsurface exploration logs and subsurface conditions may aid in estimating material quality and quantities, such as topsoil/organic matter, fills, natural soils, and rock, but are not to be relied upon as the exclusive means for bid preparation purposes. It is the responsibility of the contractor to perform any additional site examinations and explorations and to prepare an accurate bid.

#### 4. Disclaimers:

- a. In the event that any changes in the nature, design or location of the structure are planned, the conclusions that are contained in this report shall not be considered valid unless the changes are reviewed and the conclusions of this report are modified or verified in writing.
- b. The geotechnical engineering report has been prepared for this project by Barron & Associates, P.C. This report is for assistance in design only and is not a sufficient basis on which to prepare an accurate bid.
- c. This report has been prepared for the exclusive use of Uniland Construction Corporation of Amherst, New York, their client(s), and their designated design representatives, for specific application to the construction of an industrial complex at 2 Bailey Avenue, in the City of Buffalo, Erie County, New York and in accordance with generally accepted geotechnical engineering practice. No other warranty, expressed or implied, is made.