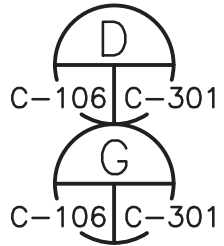


- NOTES:
- SEE NOTES, LEGEND AND ABBREVIATIONS DRAWING G-002.
 - SEE EROSION AND SEDIMENT CONTROL NOTES DRAWING C-301, AND EROSION AND SEDIMENT CONTROL DETAILS DRAWING C-302.



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(207) 775-5401

Department of
Environmental
Conservation
NEW YORK
Conservation

REMEDIAL ACTION
SARANAC LAKE GAS CO., INC.
SARANAC LAKE, NEW YORK
NYSDEC SITE NUMBER - 516008

CIVIL

EROSION AND SEDIMENT CONTROL PLAN
OU02

VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING.

DATE AUG 2017
PROJ 3611-16-1193
DWG C-106
SHEET 8 OF 29

PERMITTING
REVISION
CHK
WJW

NO. DATE
A 8/22/17
DSGN

DR
MAP
MJS

CHL BY APVD
MJS

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Consolidatory

REMEDIAL ACTION
SARANAC LAKE GAS CO., INC.
SARANAC LAKE, NEW YORK
NYSDEC SITE NUMBER - 516008

CIVIL

EROSION AND SEDIMENT CONTROL PLAN
OU03

VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING.
0 1 2

DATE JULY 2017
PROJ 3611-16-1193
DWG C-107
SHEET 9 OF 29

PERMITTING
REVISION
CHK
WJW

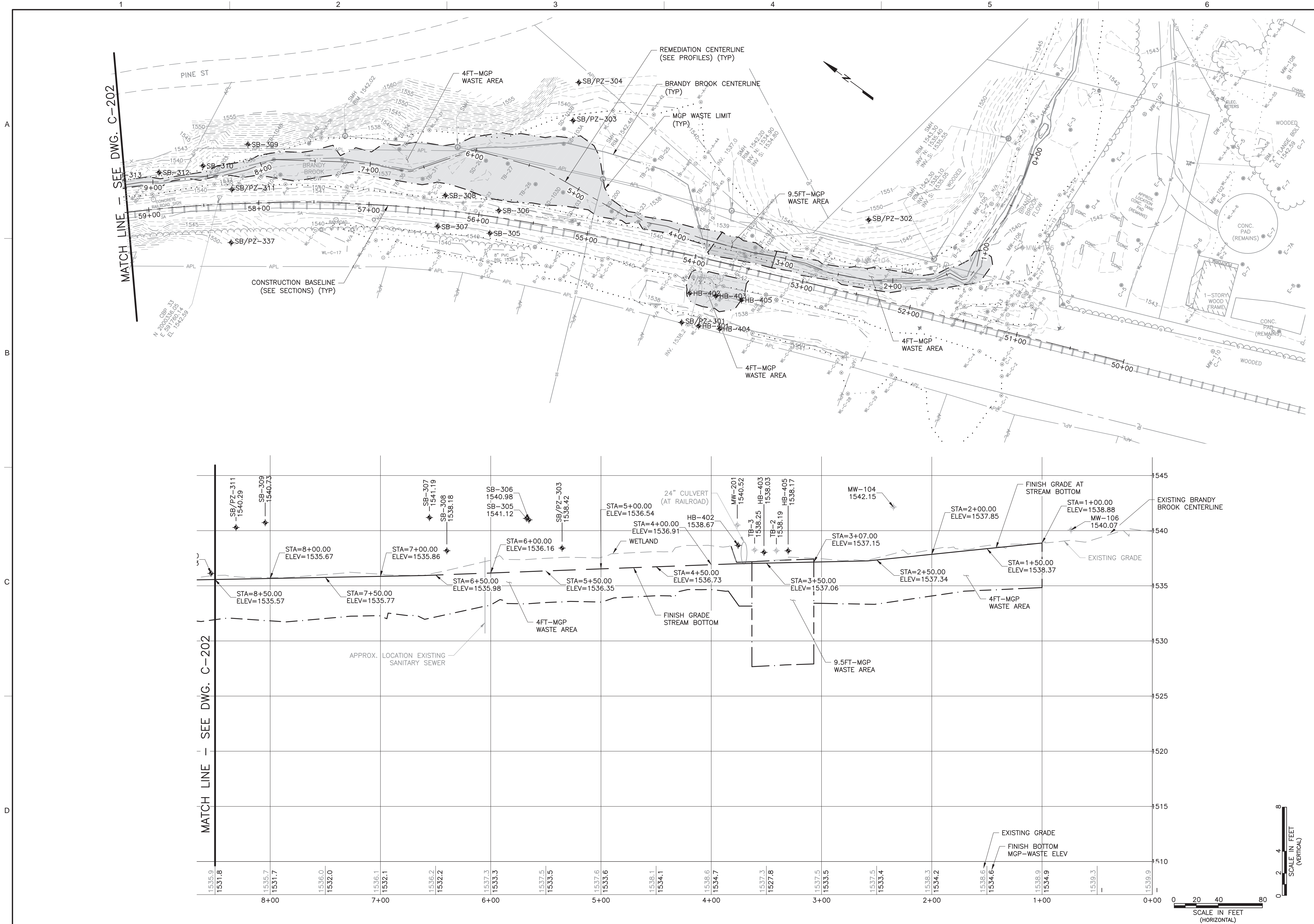
NO. DATE
A 8/22/17

DR MAP
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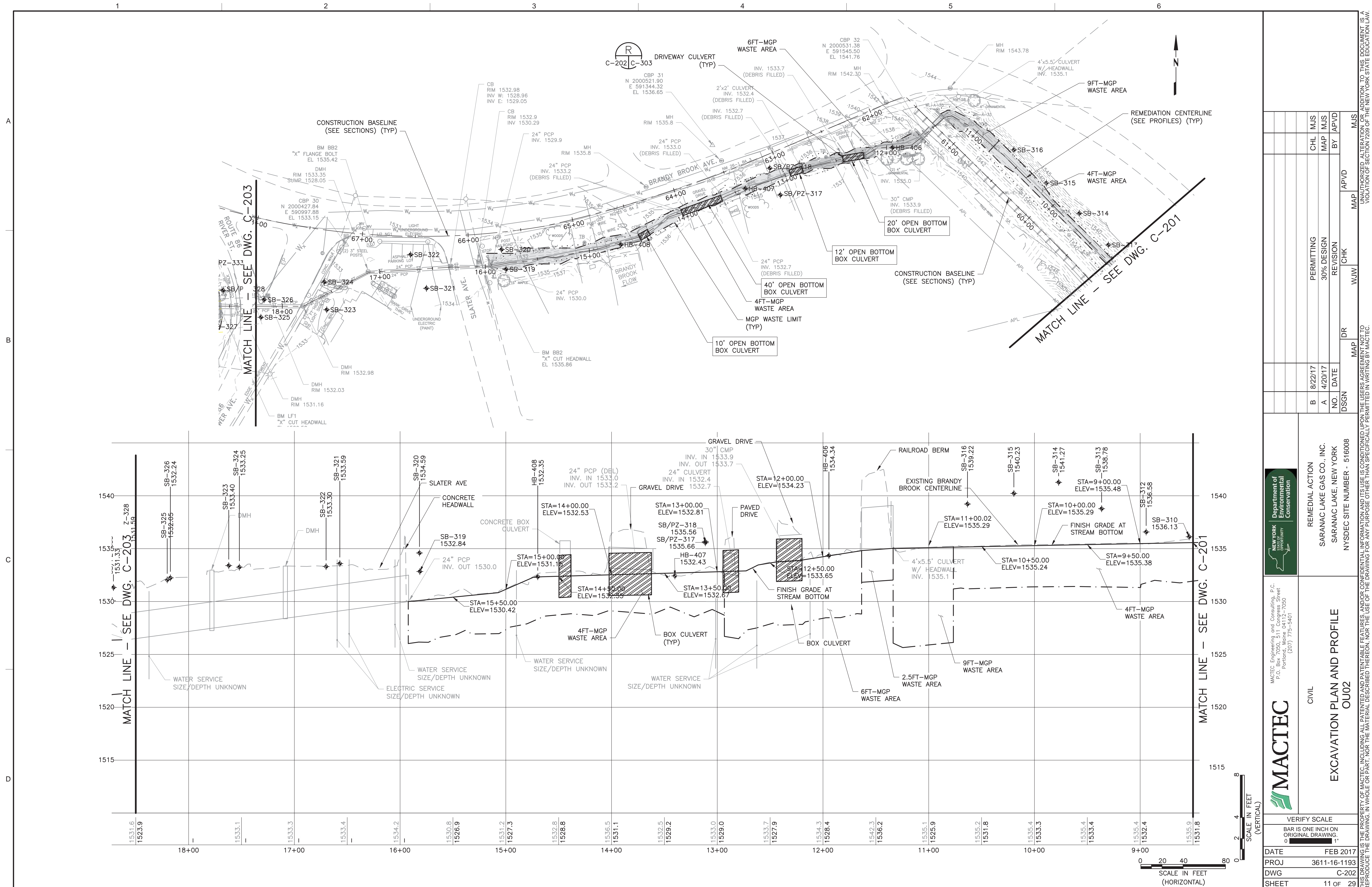
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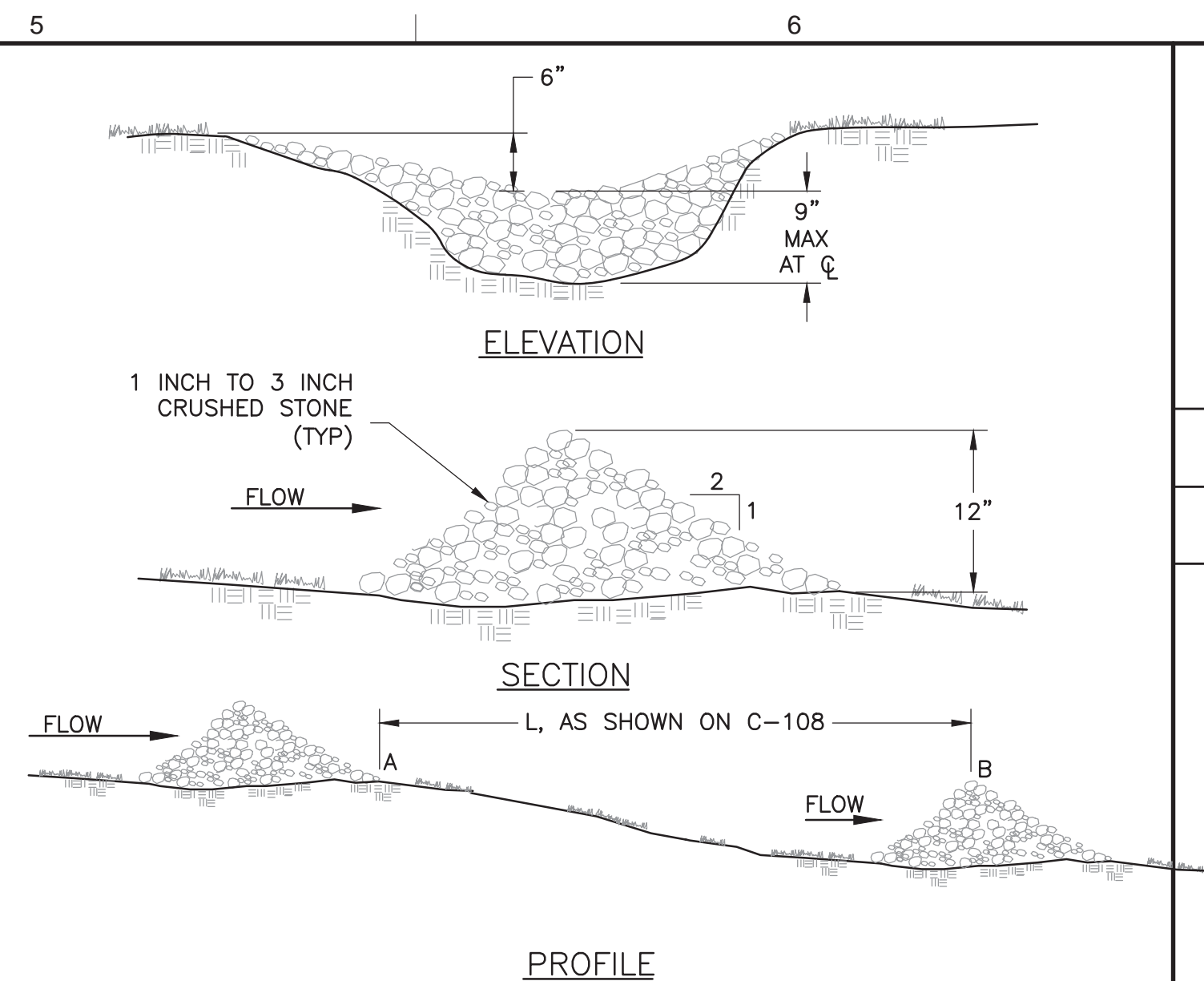
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MATCH LINE - SEE DWG. C-106

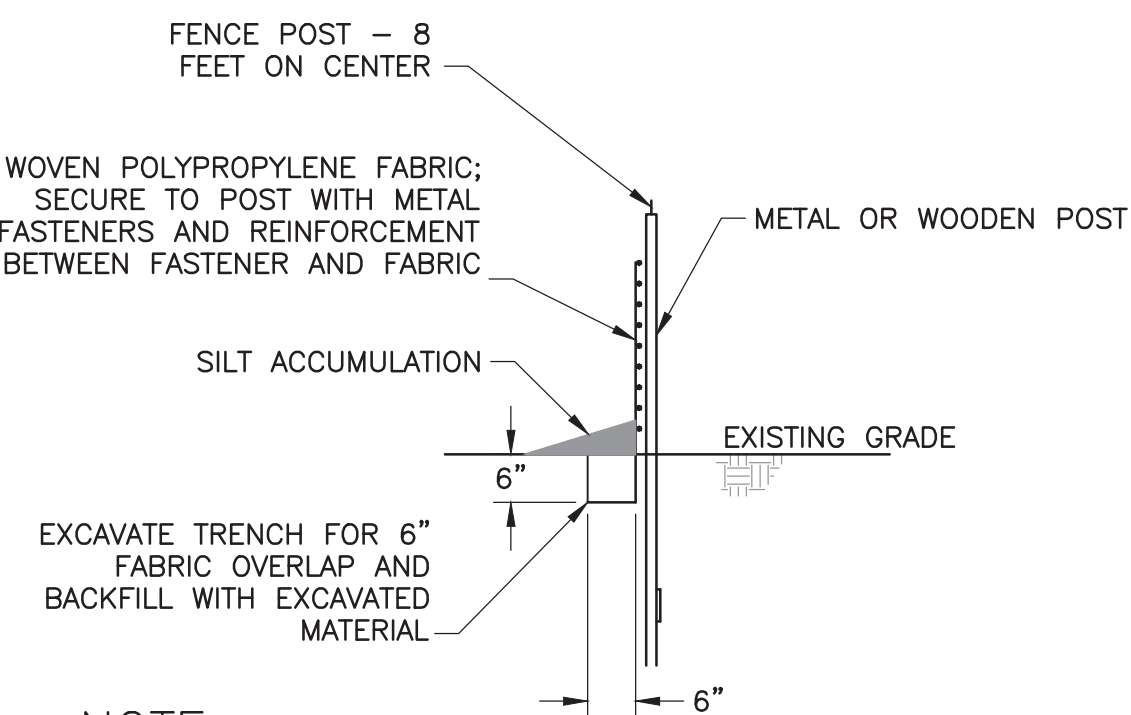


MACTEC Engineering and Consulting, P.C. P.O. Box 7050, 511 Congress Street Portland, Maine 04107-7050 (207) 775-5401		Department of Environmental Conservation NEW YORK		REMEDIAL ACTION SARANAC LAKE GAS CO., INC. SARANAC LAKE, NEW YORK NYSDEC SITE NUMBER - 516008		DATE FEB 2017	
CIVIL		EXCAVATION PLAN AND PROFILE OU02		VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING.		PROJ 3611-16-1193	
DATE FEB 2017		REVISION 30% DESIGN		NO. DATE		DWG C-201	
PERMITTING 8/22/17		CHL MJS		BY APVD		SHEET 10 OF 29	
30% DESIGN 4/20/17		MAP MJS		CHK APVD		UNAUTHORIZED ALTERATION OR ADDITION TO THIS DOCUMENT IS A VIOLATION OF SECTION 7209 OF THE NEW YORK STATE EDUCATION LAW.	
REVISION		WJW		DR			
DGN		MAP		MJS			





STABILIZED CONSTRUCTION ENTRANCE

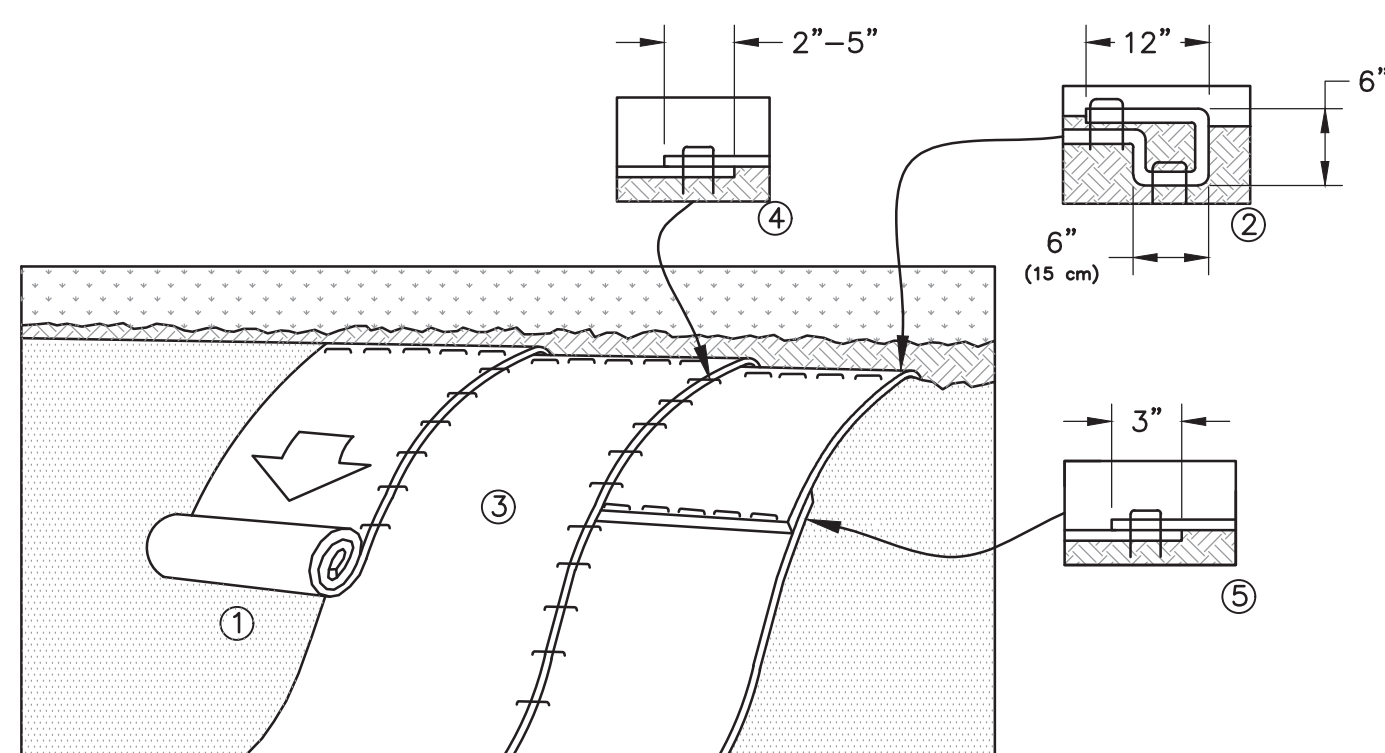


AUGMENTED SILTATION FENCE

NTS

C
TYP C-302

TEMPORARY STONE CHECK DAM



SILT FENCE

NTS

TYP C-302

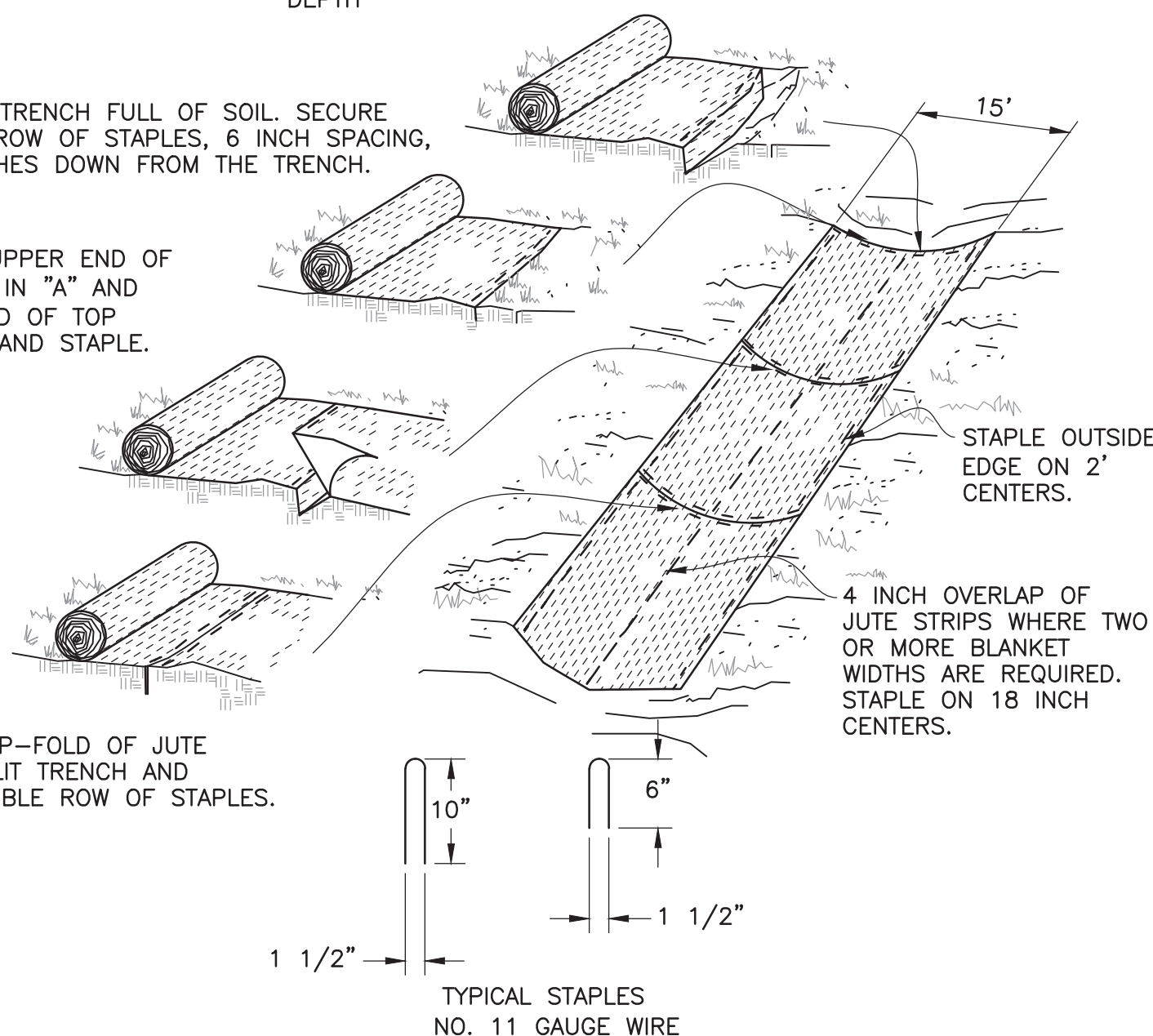
B

- A. BURY THE TOP END OF THE JUTE STRIPS
IN A TRENCH 6 INCHES OR MORE IN
DEPTH

- B. TAMP TRENCH FULL OF SOIL. SECURE WITH ROW OF STAPLES, 6 INCH SPACING. 4 INCHES DOWN FROM THE TRENCH.

- C. OVERLAP-BURY UPPER END OF LOWER STRIP AS IN "A" AND "B". OVERLAP END OF TOP STRIP 4 INCHES AND STAPLE.

- D. EROSION STOP—FOLD OF JUTE
BURIED IN SLIT TRENCH AND
TAMPED; DOUBLE ROW OF STAPLES.

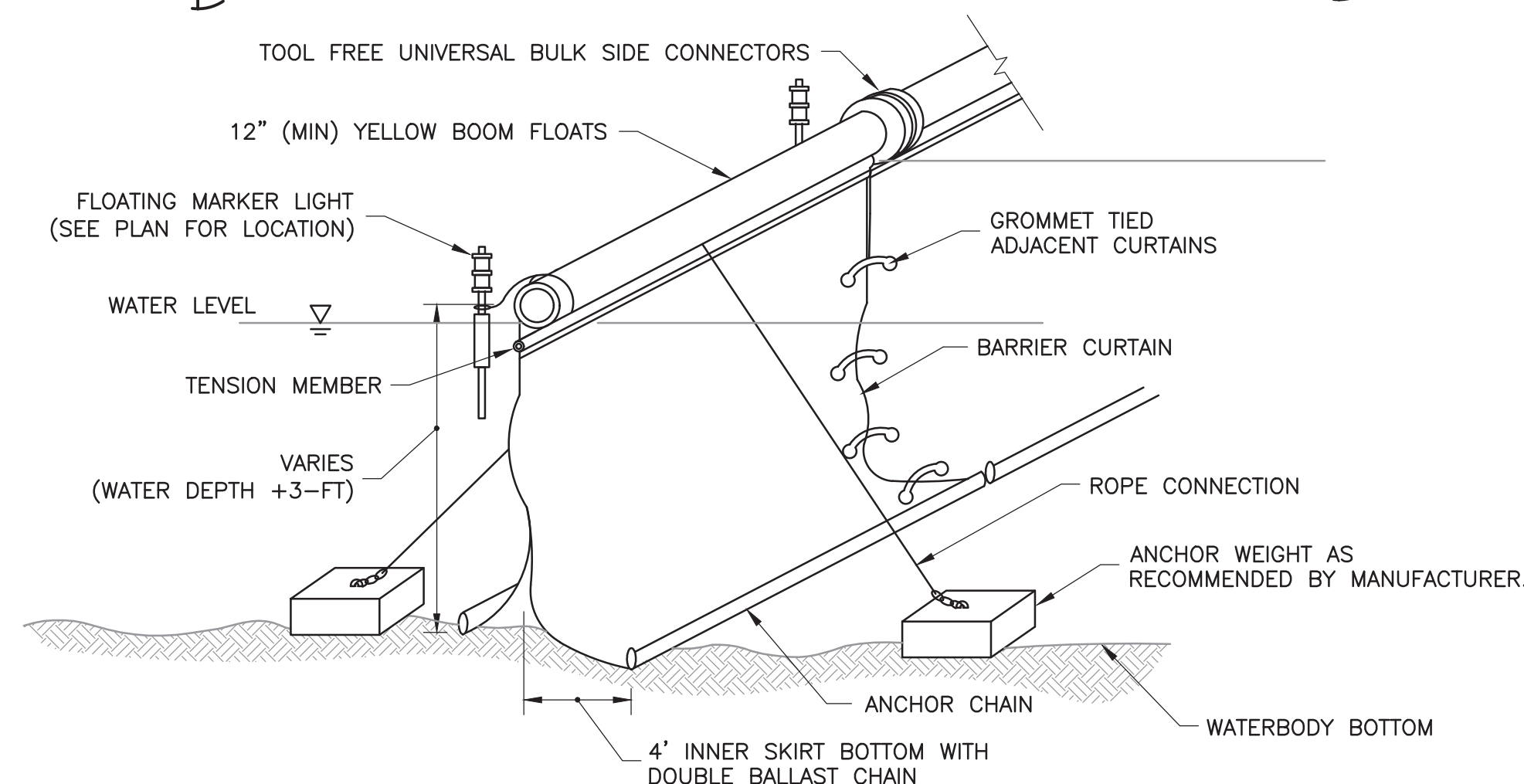


EROSION CONTROL BLANKET ON SWALE

NTS

F
TYP C-302

- NOTES:
1. GEOMEMBRANE COVER IS NOT REQUIRED FOR STOCKPILES OF CERTIFIED CLEAN MATERIAL; THEY MAY BE USED AS DUST CONTROL MEASURE.
 2. SOIL/SEDIMENT STOCKPILE SHALL BE COVERED TO THE EXTENT PRACTICABLE.
 3. SOIL/SEDIMENT STOCKPILE SHALL BE LOCATED WITHIN TEMPORARY CONTRACTOR STAGING AND STORAGE AREA.

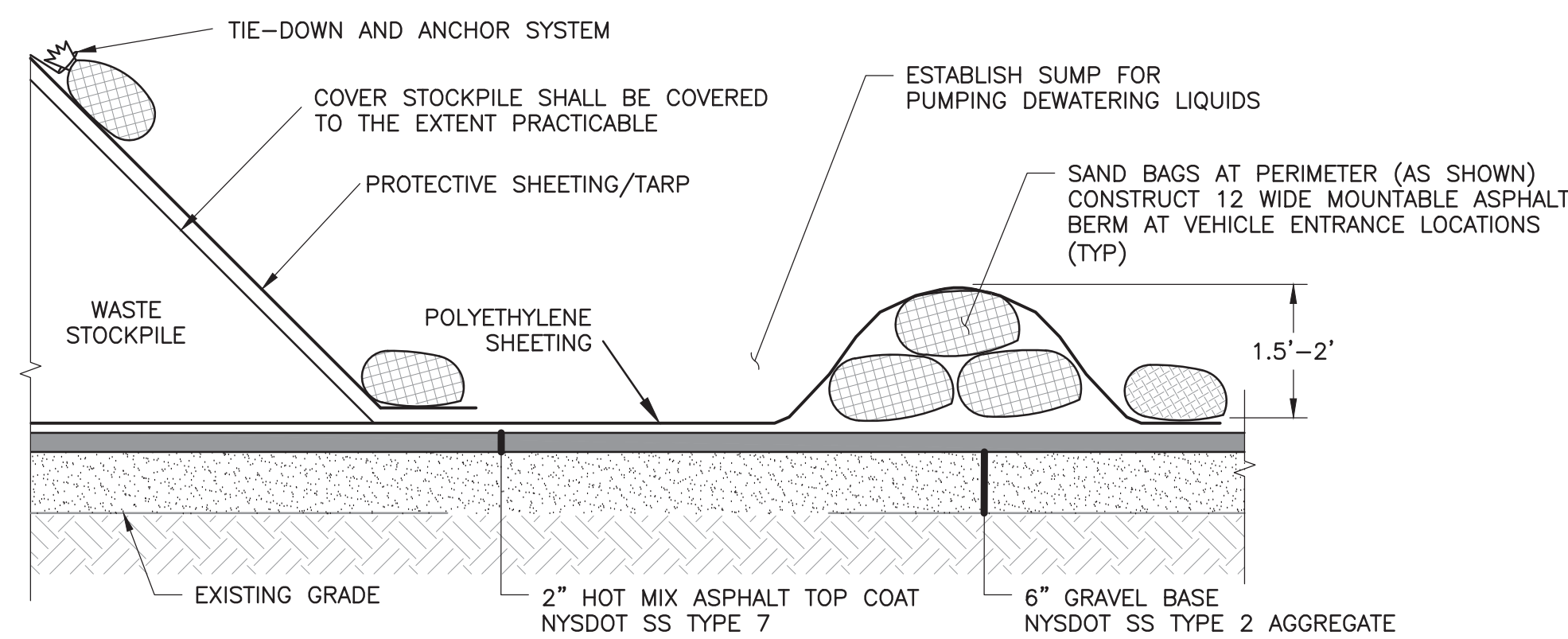


- NOTES:
1. PLACE TURBIDITY BARRIER IMMEDIATELY DOWNSTREAM OF BYPASS DISCHARGE.
 2. TURBIDITY BARRIER SHALL BE DEPLOYED PRIOR TO DREDGING OR DISTURBING UPLAND SOILS.

TURBIDITY CURTAIN

NTS

G
TYP C-302




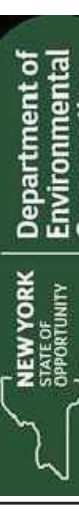

STOCKPILE CONTAINMENT AREA

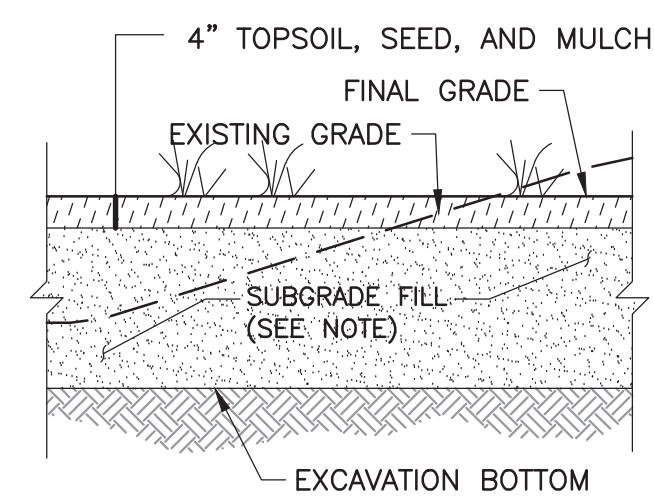
NTS

H
TYP C-302

- ## NOTES:
1. PREPARE TOPSOIL BEFORE INSTALLING MATTING, INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED.
 2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" DEEP X 6" WIDE TRENCH WITH APPROXIMATELY 12" OF BLANKET EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH. ANCHOR THE BLANKET WITH A ROW OF STAPLES/STAKES APPROXIMATELY 12" APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED TOPSOIL AND FOLD REMAINING 12" PORTION OF BLANKET BACK OVER SEED AND COMPACTED TOPSOIL. SECURE BLANKET OVER COMPACTED GROWING MEDIUM WITH A ROW OF STAPLES/STAKES SPACED APPROXIMATELY 12" (30cm) APART ACROSS THE WIDTH OF THE BLANKET.
 3. ROLL THE MATTING DOWN THE SLOPE. MATTING WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE TOPSOIL SURFACE. ALL MATTING MUST BE SECURELY FASTENED TO TOPSOIL SURFACE BY PLACING STAPLES/STAKES IN APPROPRIATE LOCATIONS AS SHOWN IN THE STAPLE PATTERN GUIDE. WHEN USING MANUFACTURER STAPLE PATTERN MARKING, STAPLES/STAKES SHOULD BE PLACED THROUGH EACH OF THE COLORED MARKS CORRESPONDING TO THE APPROPRIATE STAPLE PATTERN.
 4. THE EDGES OF PARALLEL MATTING MUST BE STAPLED WITH APPROXIMATELY 2"-5" OVERLAP DEPENDING ON MATTING TYPE. TO ENSURE PROPER SEAM ALIGNMENT, PLACE THE EDGE OF THE OVERLAPPING MATTING (MATTING BEING INSTALLED ON TOP) EVEN WITH THE SEAM ON THE PREVIOUSLY INSTALLED MATTING.
 5. CONSECUTIVE MATTING SPLICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 3" OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" APART ACROSS ENTIRE MATTING WIDTH.
 6. DETAIL PROVIDED BY NORTH AMERICAN GREEN ®.
 7. INSTALL ON ALL FINAL STABILIZED UPLAND SURFACES TO BE VEGETATED WITH SLOPES STEEPER THAN 10H:1V AND UPLAND VEGETATED DRAINAGE SWALES.

BIODEGRADABLE EROSION CONTROL MATTING

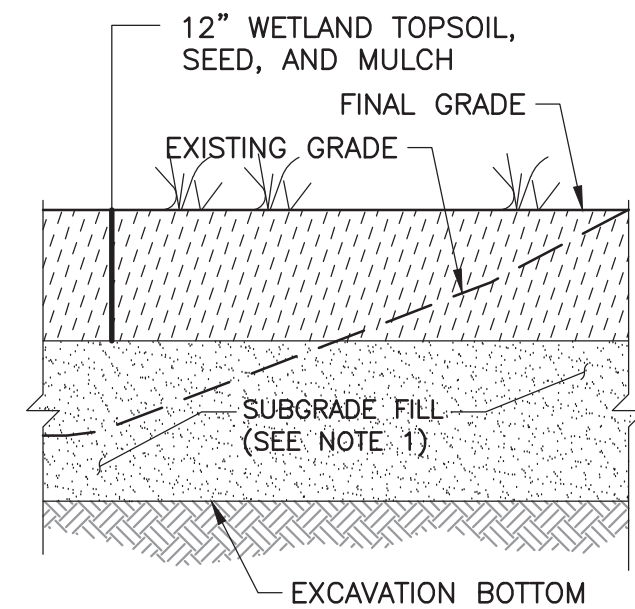
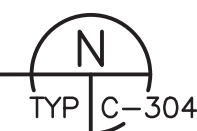
		MACTEC Engineering and Consulting, P.C. P.O. Box 7050, 511 Commercial Street Portland, Maine 04112-7050 (207) 775-5401									
CIVIL				REMEDIAL ACTION SARANAC LAKE GAS CO., INC. SARANAC LAKE, NEW YORK NYSDEC SITE NUMBER - 516008							
EROSION AND SEDIMENT CONTROL DETAILS											
VERIFY SCALE											
BAR IS ONE INCH ON ORIGINAL DRAWING.											
0"  1"											
DATE	FEB 2017										
PROJ	3611-16-1193										
DWG	C-302										
SHEET	14 OF 29										



NOTE:
DEPTHS OF SUBGRADE FILL VARIES
BASED ON ACTUAL EXCAVATION BOTTOM
AND FINAL GRADE ELEVATIONS.

VEGETATED UPLAND
RESTORATION

NTS

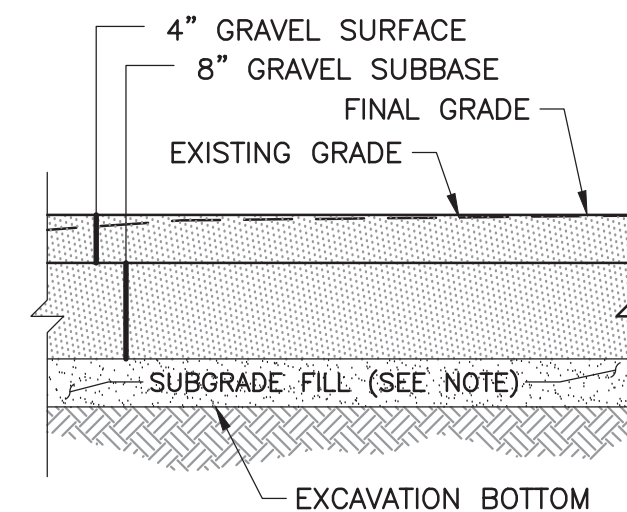
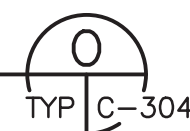


NOTES:

1. DEPTHS OF SUBGRADE FILL VARIES BASED ON ACTUAL EXCAVATION BOTTOM AND FINAL GRADE ELEVATIONS.
2. INSTALL PLANTINGS AS SHOWN ON DRAWINGS.

VEGETATED WETLAND
RESTORATION

NTS

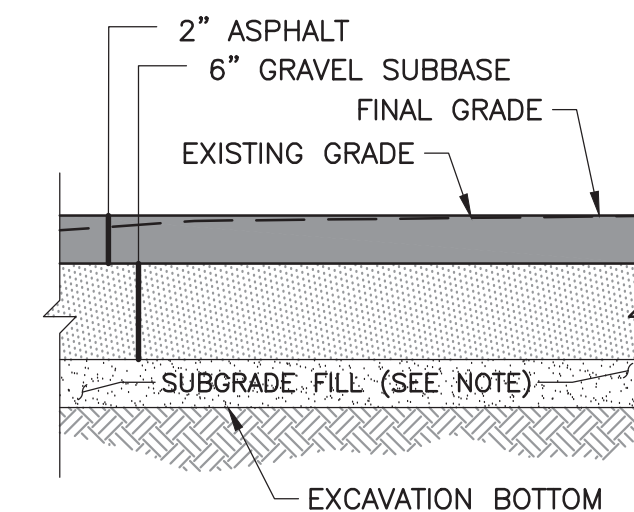
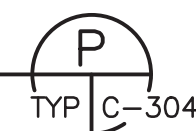


NOTE:

- NOTE:
DEPTHS OF SUBGRADE FILL VARIES
BASED ON ACTUAL EXCAVATION BOTTOM
AND FINAL GRADE ELEVATIONS.

GRAVEL DRIVE RESTORATION

NTS

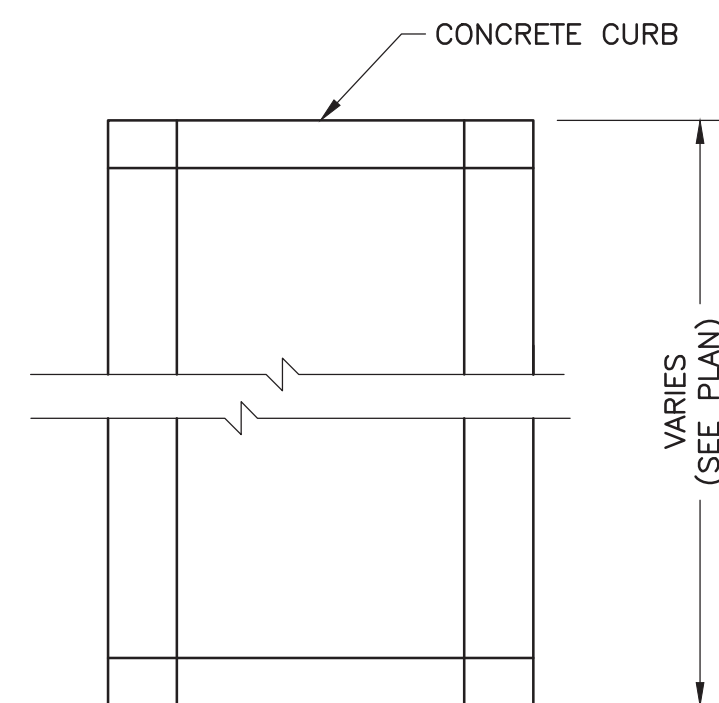
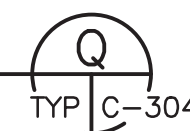


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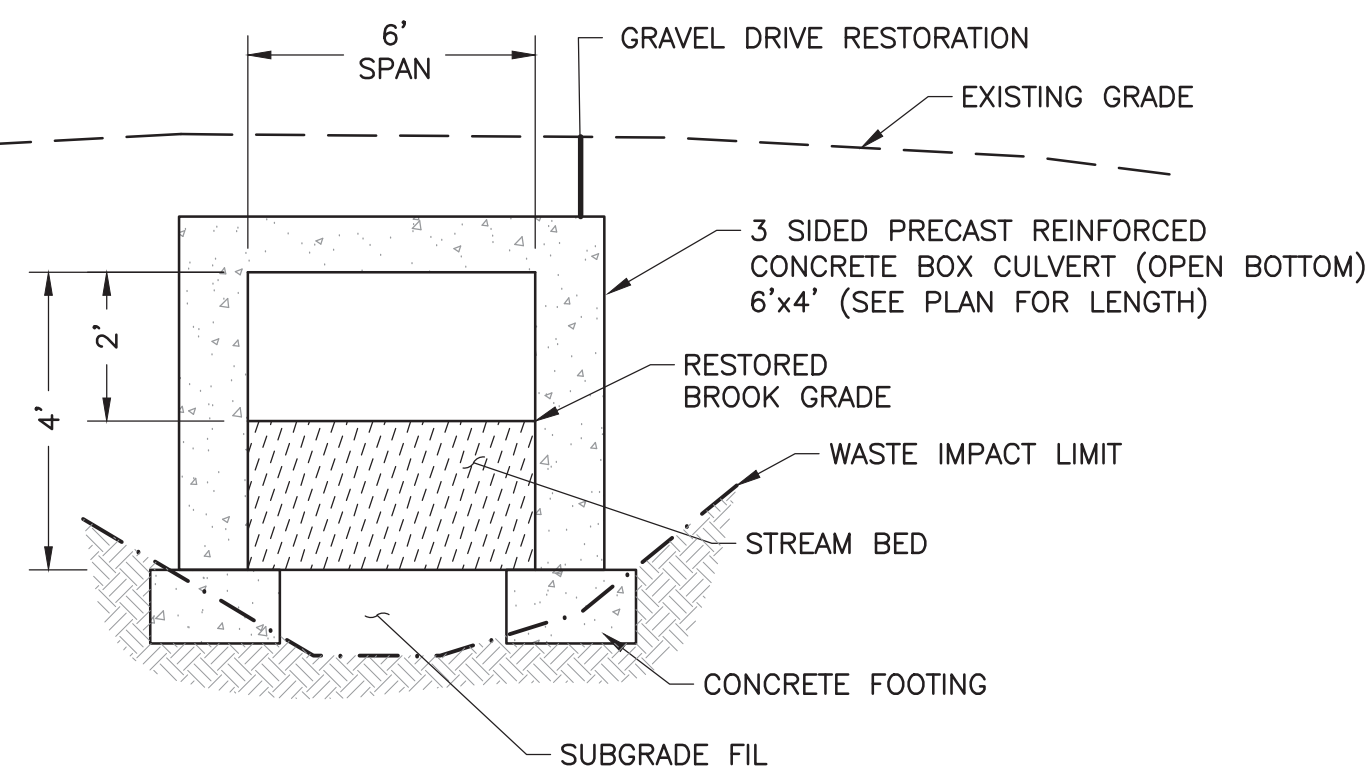
- NOTE:
DEPTHS OF SUBGRADE FILL VARIES
BASED ON ACTUAL EXCAVATION BOTTOM
AND FINAL GRADE ELEVATIONS.

PAVED DRIVE
RESTORATION

NTS



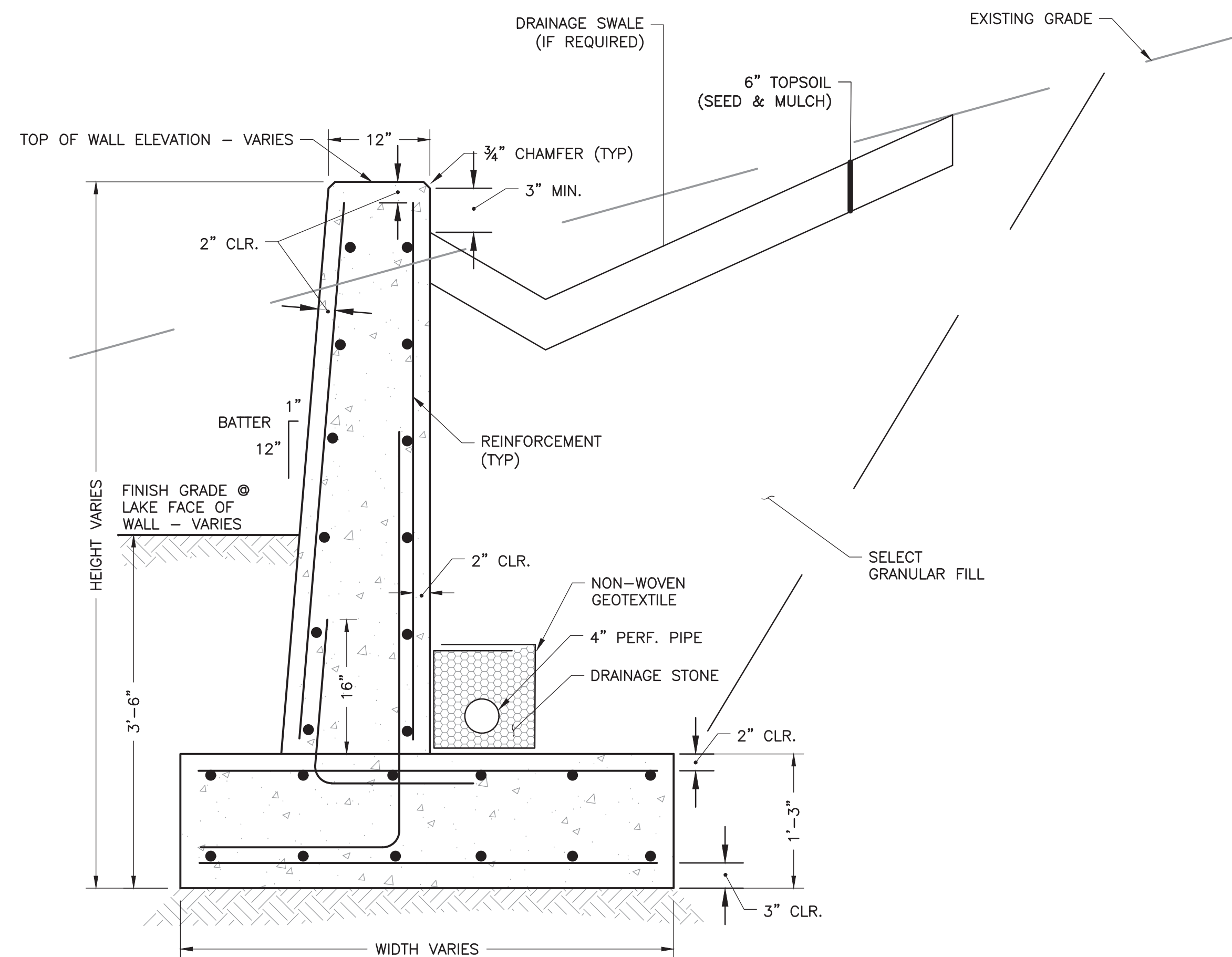
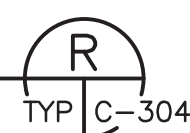
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SECTION

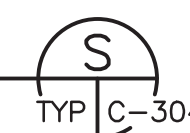
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NTS

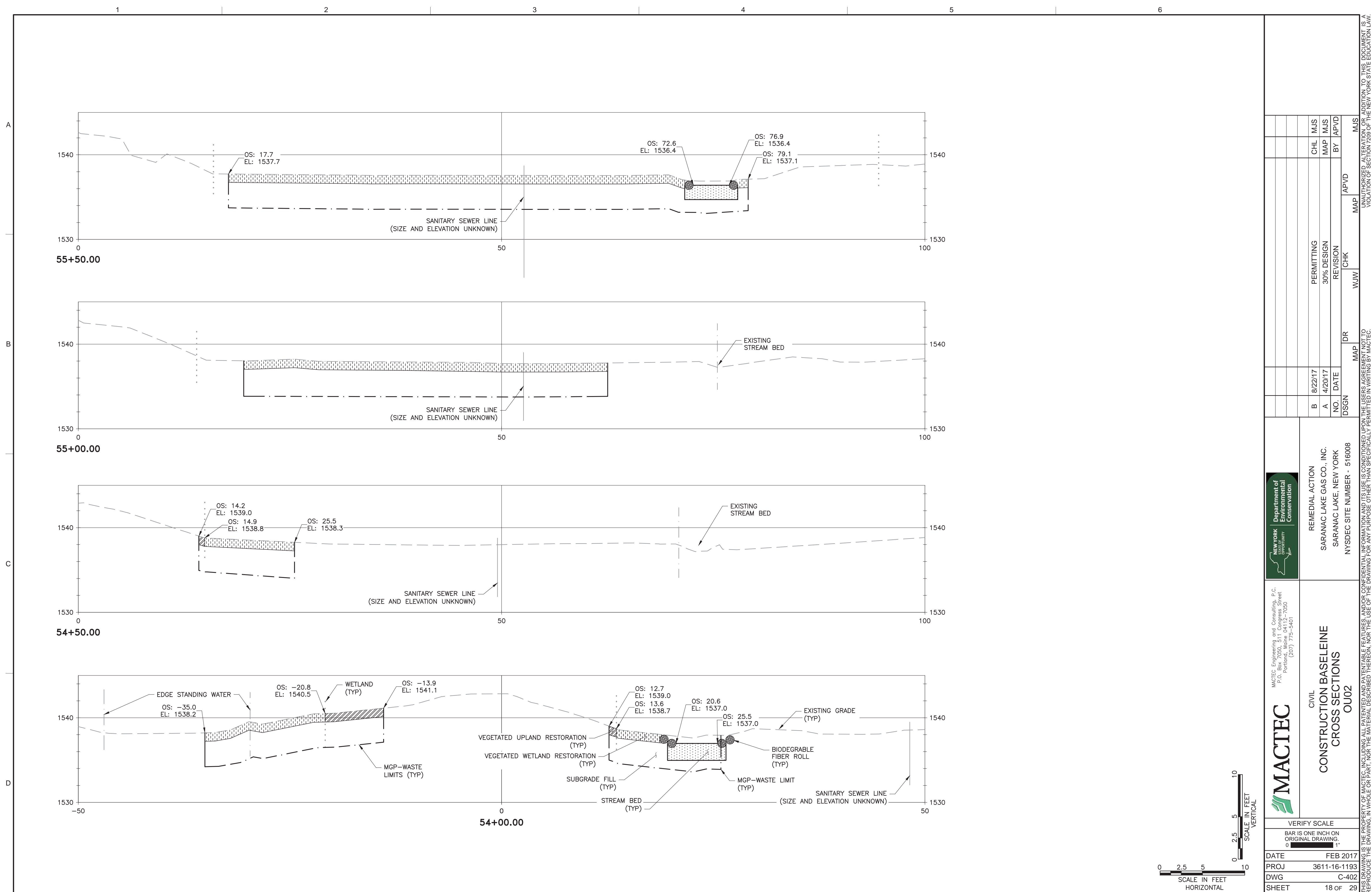


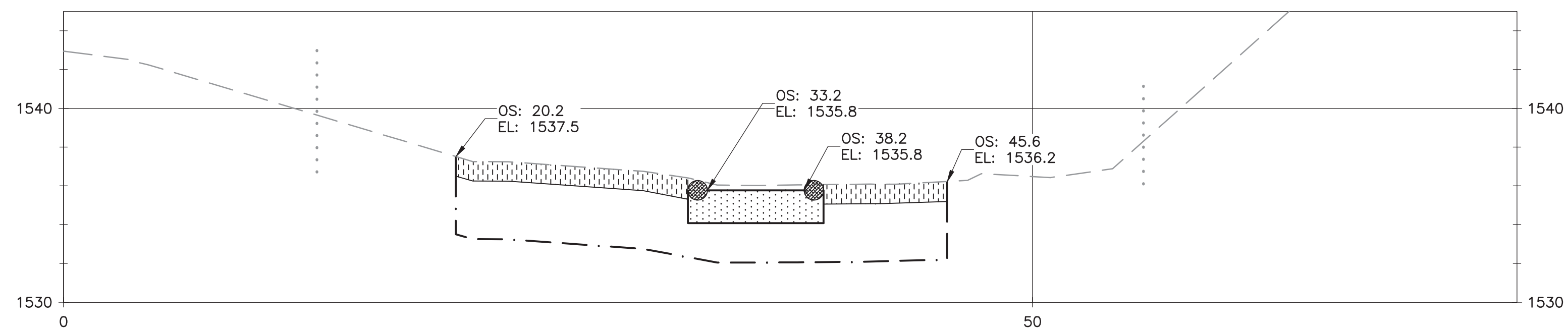
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NTS

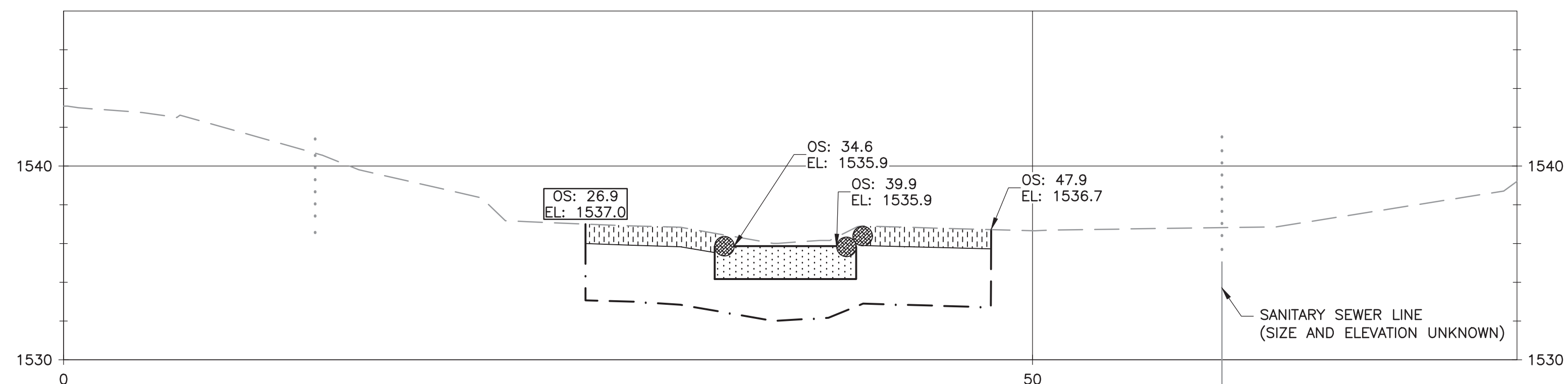


 <div> MACTEC Engineering and Consulting, P.C. P.O. Box 70500, 511 Congress Street Portland, Maine 04107-7050 (207) 775-5401 </div>				<div>CIVIL</div> <div>REMEDIAL ACTION</div> <div>SARANAC LAKE GAS CO., INC.</div> <div>SARANAC LAKE, NEW YORK</div> <div>NYSDEC SITE NUMBER - 516008</div>									
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DATE	JULY 2017												
PROJ	3611-16-1193												
DWG	C-304												
SHEET	16 OF 29												
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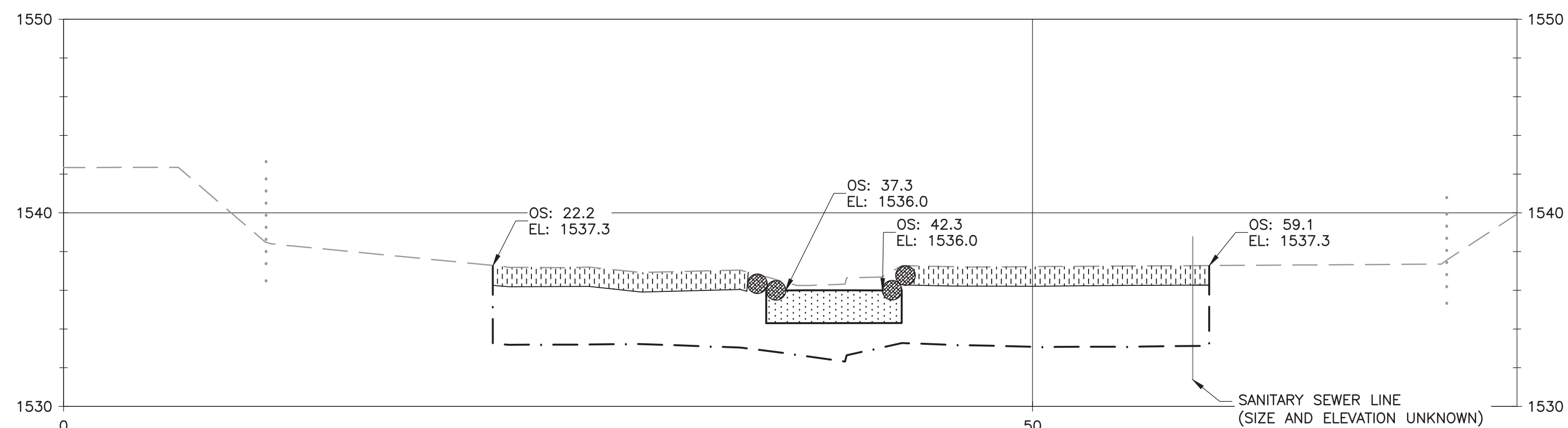




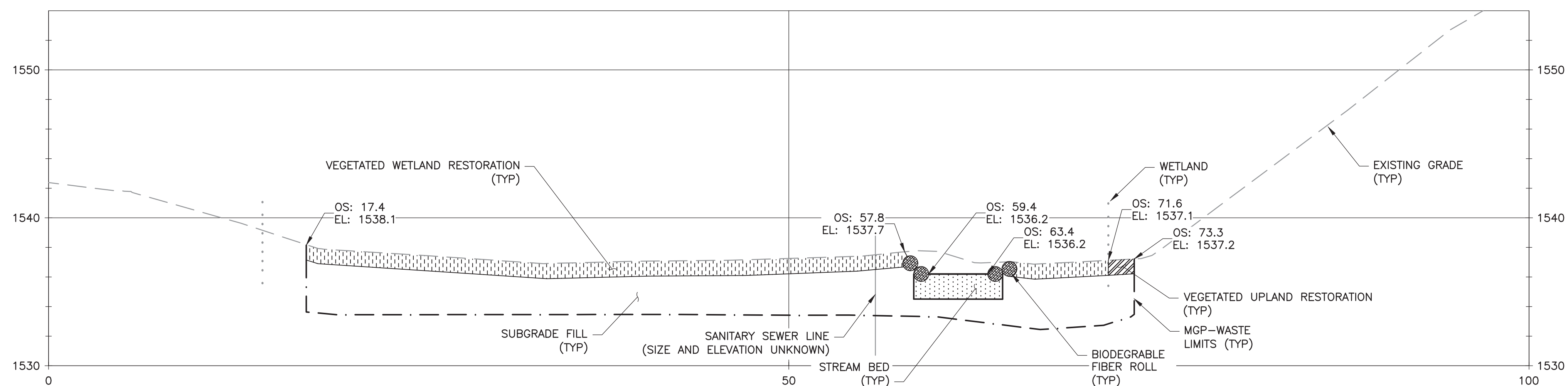
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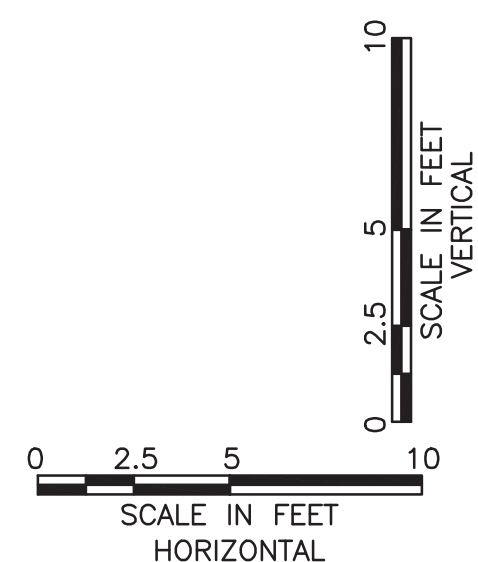
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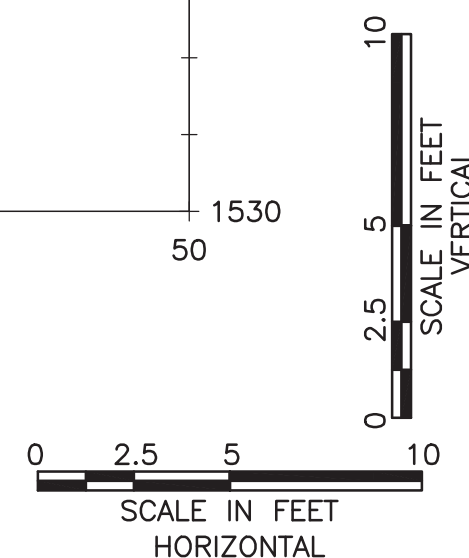
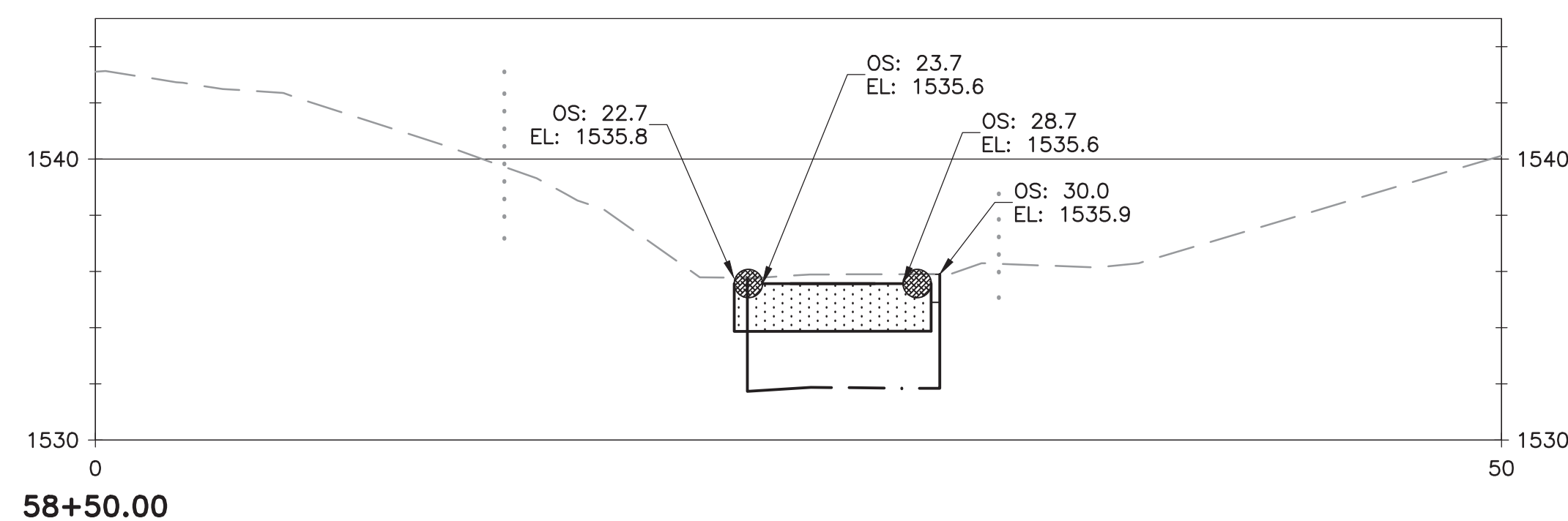
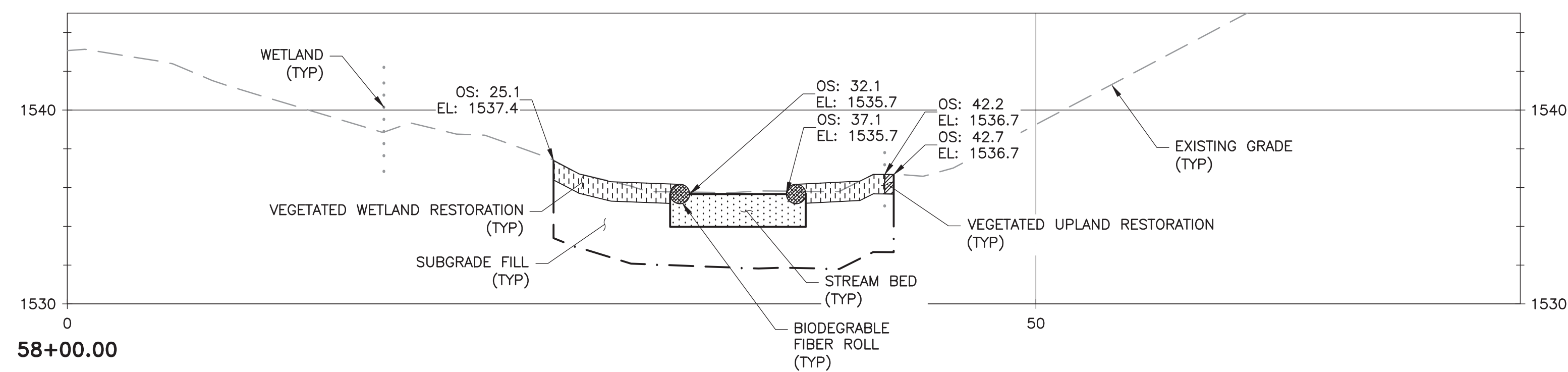
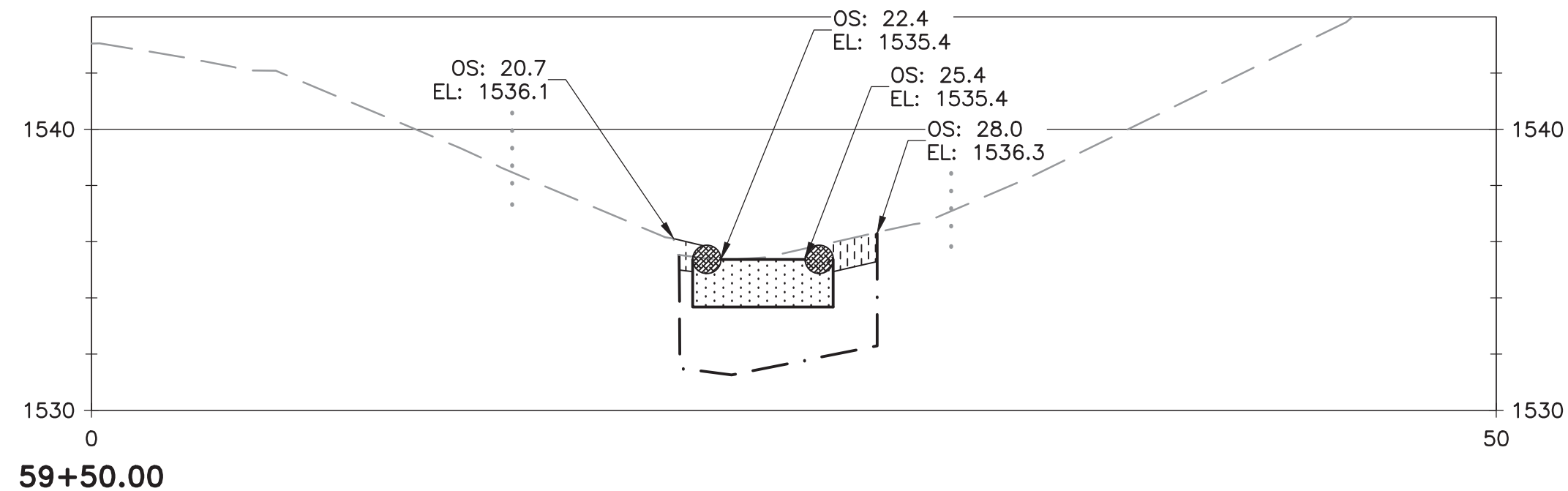
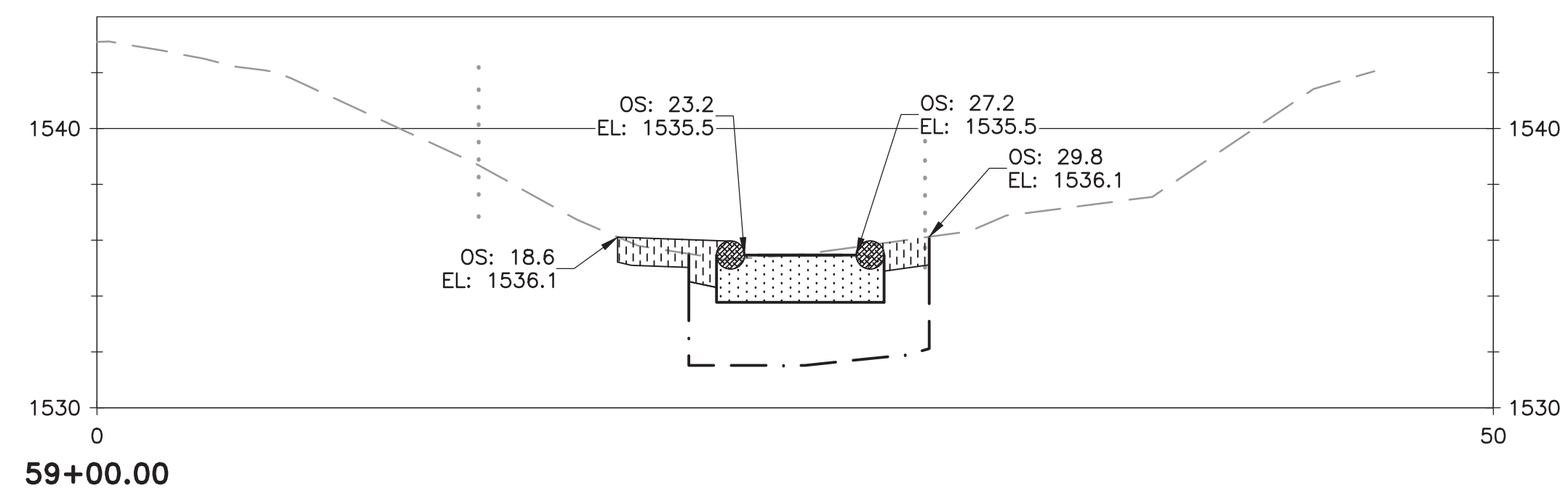
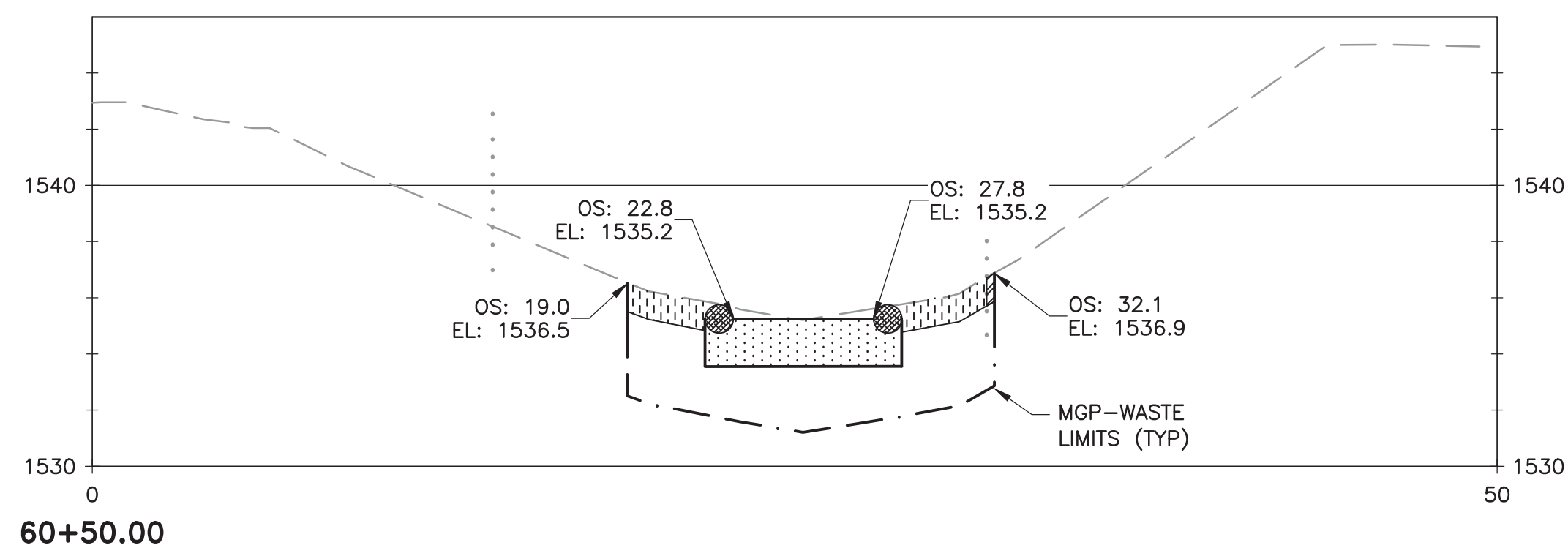
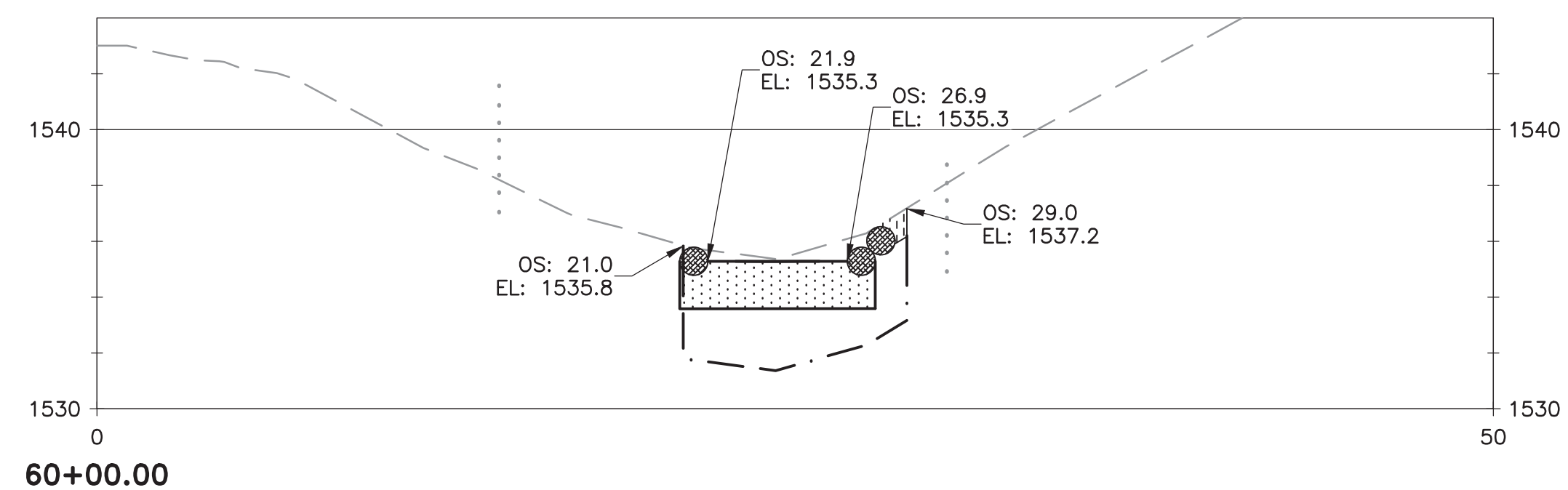
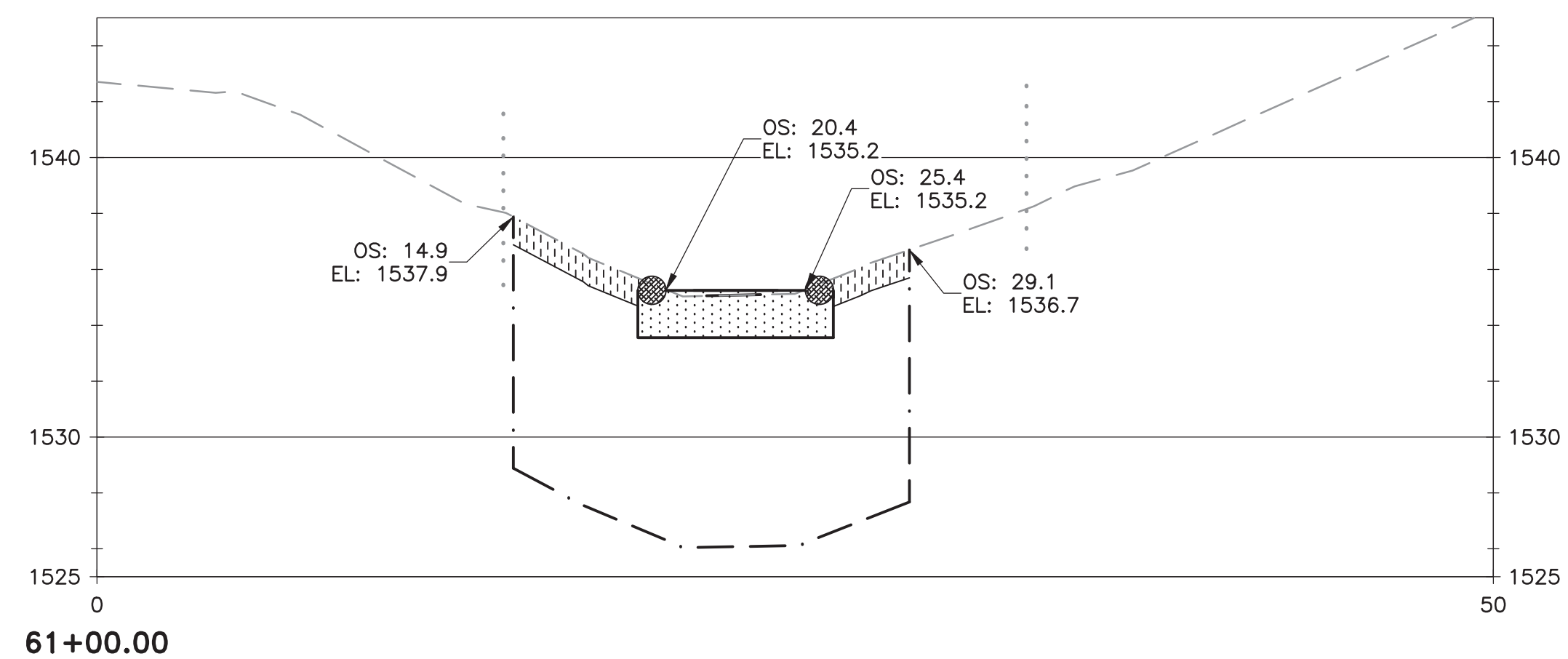
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CIVIL CONSTRUCTION BASELINE CROSS SECTIONS OU02															
VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING 0"  1"															
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SHEET		19 OF 22													

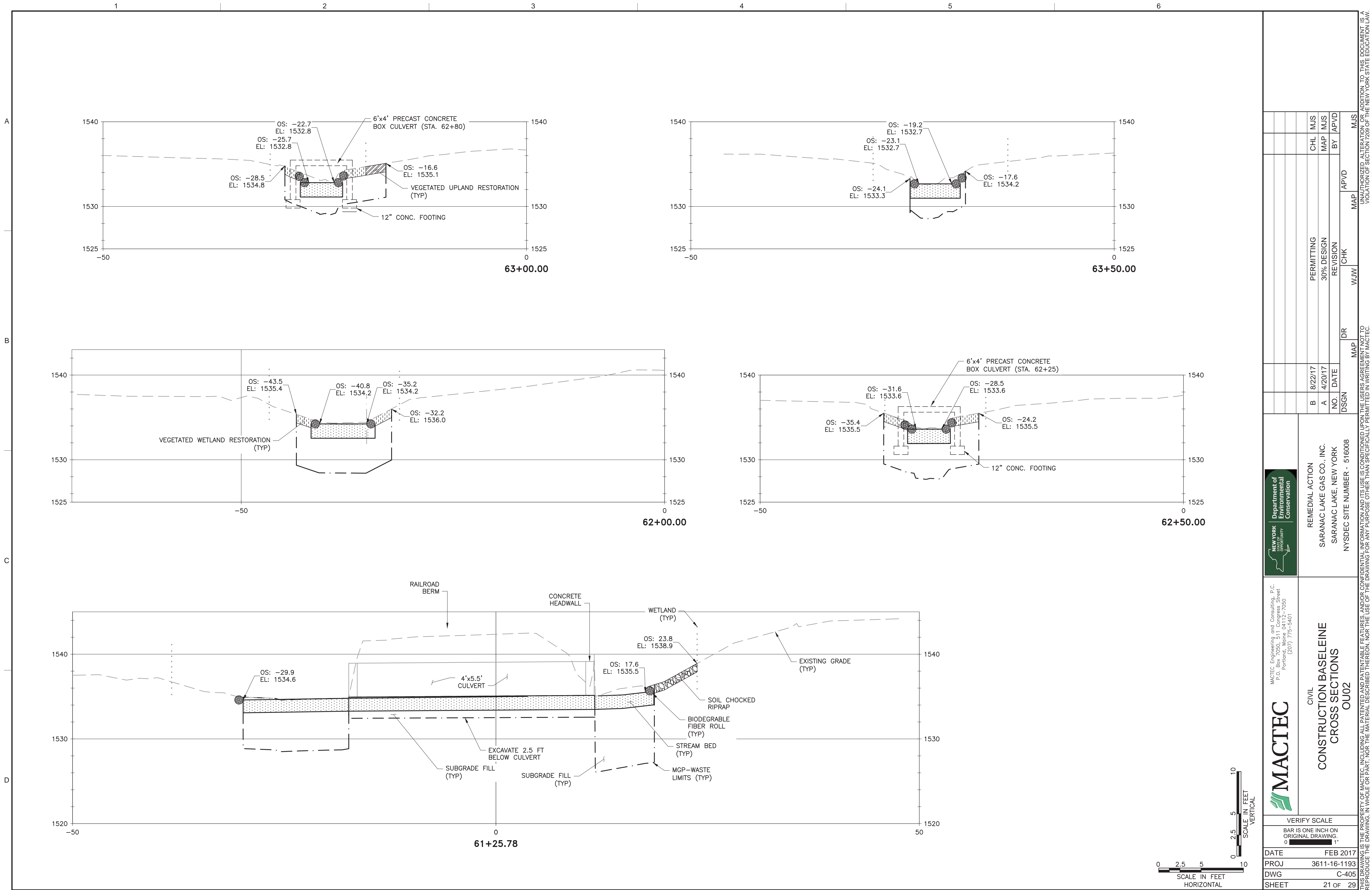
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NEW YORK
 STATE OF
 DEPARTMENT of
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 Conservation
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 REMEDIAL ACTION
 SARANAC LAKE GAS CO., INC.
 SARANAC LAKE, NEW YORK
 NYSDEC SITE NUMBER - 516008

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NEW YORK

Department of
Environmental
Conservation

CIVIL

CONSTRUCTION BASELINE
CROSS SECTIONS
OU02

REMEDIAL ACTION

SARANAC LAKE GAS CO., INC.
SARANAC LAKE, NEW YORK
NYSDEC SITE NUMBER - 516008

VERIFY SCALE

BAR IS ONE INCH ON
ORIGINAL DRAWING.

DATE

FEB 2017

PROJ

3611-16-1193

DWG

C-405

SHEET

21 OF 29

PERMITTING

CHL

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30% DESIGN

REVISION

NO.

DATE

8/22/17

4/20/17

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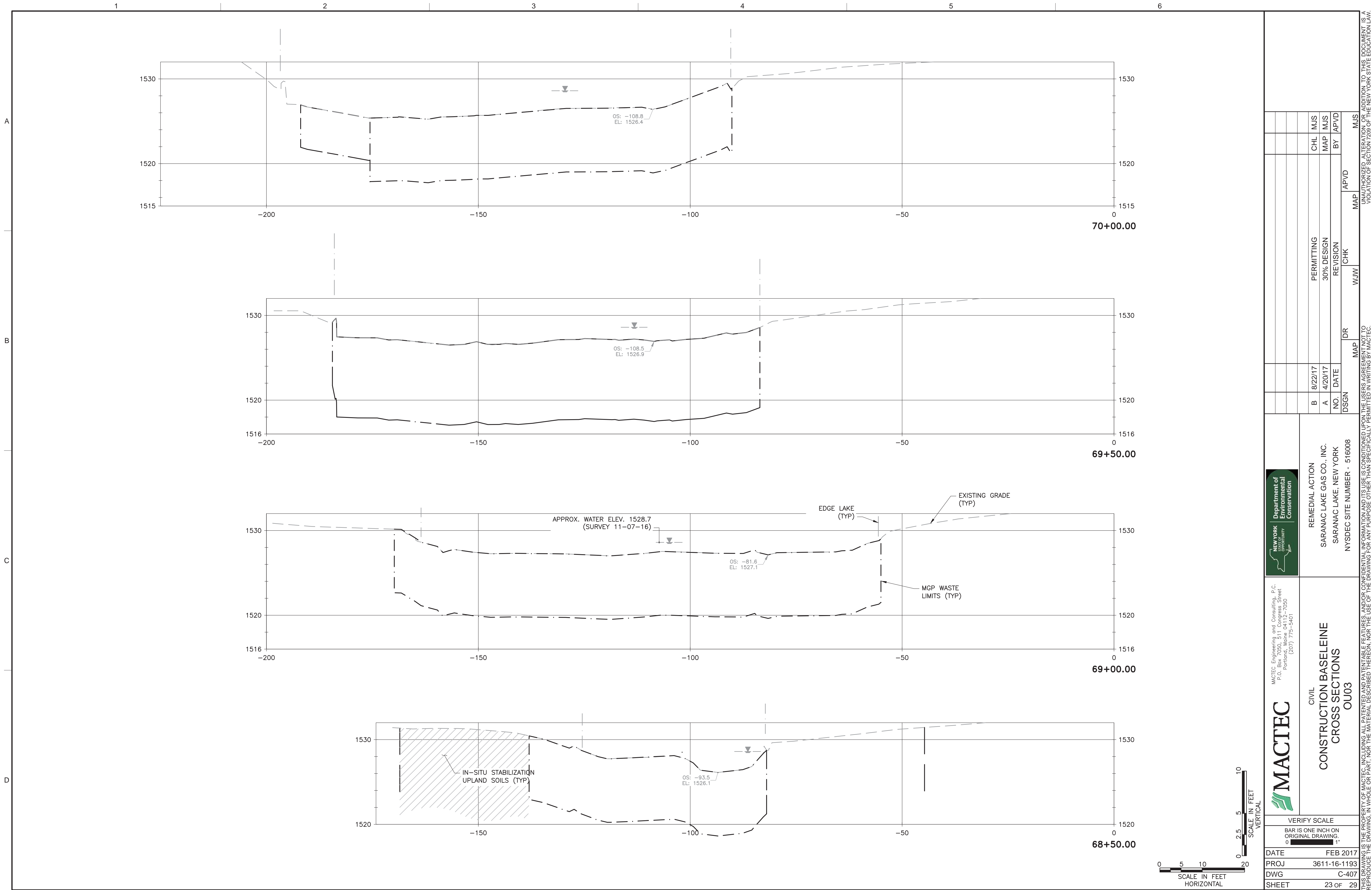
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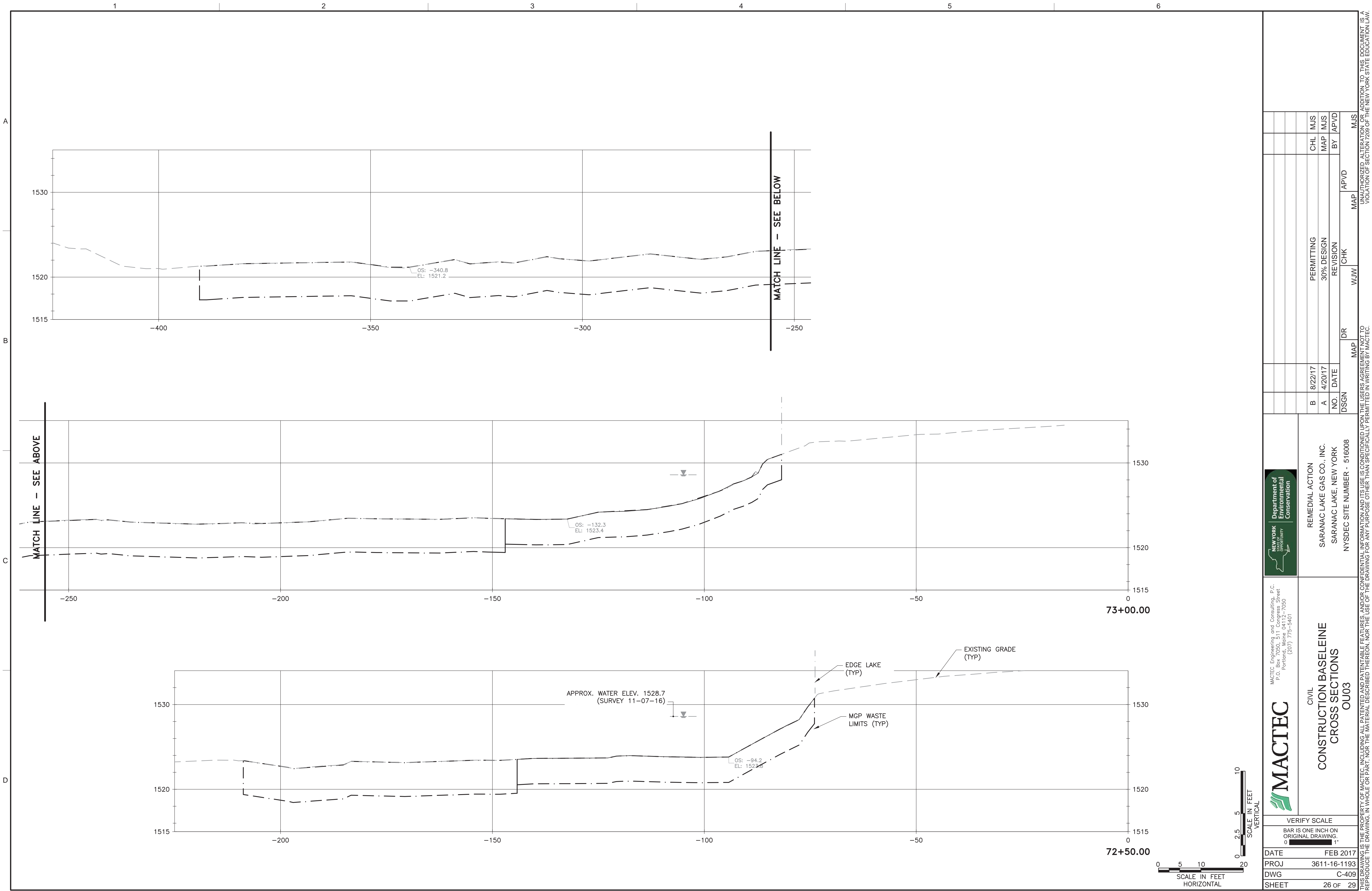
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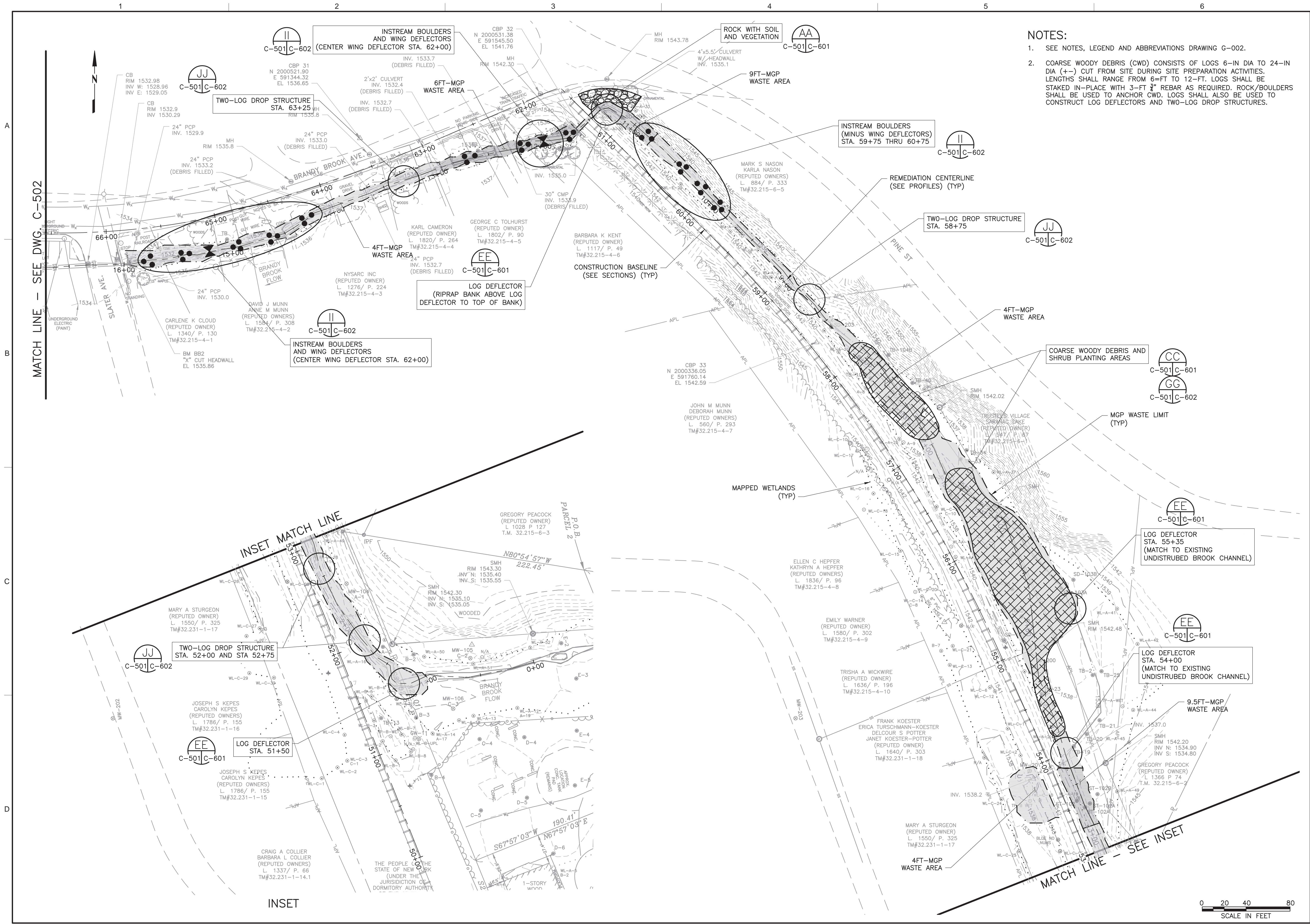
MJS

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- NOTES:
- SEE NOTES, LEGEND AND ABBREVIATIONS DRAWING G-002.
 - COARSE WOODY DEBRIS (CWD) CONSISTS OF LOGS 6-IN DIA TO 24-IN DIA (+/-) CUT FROM SITE DURING SITE PREPARATION ACTIVITIES. LENGTHS SHALL RANGE FROM 6=FT TO 12=FT. LOGS SHALL BE STAKED IN-PLACE WITH 3=FT 3" REBAR AS REQUIRED. ROCK/BOULDERS SHALL BE USED TO ANCHOR CWD. LOGS SHALL ALSO BE USED TO CONSTRUCT LOG DEFLECTORS AND TWO=LOG DROP STRUCTURES.

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Department of Environmental Conservation
NEW YORK
Conservation

CIVIL
BRANDY BROOK
RESTORATION/LANDSCAPING PLAN
OU02

REMEDIAL ACTION
SARANAC LAKE GAS CO., INC.
SARANAC LAKE, NEW YORK
NYSDEC SITE NUMBER - 516008

DATE	JULY 2017
PROJ	3611-16-1193
DWG	C-501
SHEET	26 OF 29

VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING.
0 20 40 80
SCALE IN FEET

PERMITTING
30% DESIGN
REVISION
CHK
WJW
APVD
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CHL
MJS
BY
APVD

NO.
DATE
DSGN

9/22/17
4/20/17
NO.

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A

FILE NAME: Z:\Projects\insdec1\Saranac Lake\C-501-Restoration OU2.dwg PLOT DATE: Fri, 18 Aug 2017 PLOT TIME: 1:32 PM



- NOTES:
- SEE NOTES, LEGEND AND ABBREVIATIONS DRAWING G-002.
 - PLANT 25 RED CEDAR (JUNIPERUS VIRGINIANA) IN AREAS LABELED TREE PLANTING.

MACTEC
Civil
PONTIAC BAY
RESTORATION/LANDSCAPING PLAN
OU03

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NEW YORK
Department of
Environmental
Conservation

REMEDIAL ACTION
SARANAC LAKE GAS CO., INC.
SARANAC LAKE, NEW YORK
NYSDEC SITE NUMBER - 516008

DATE	JULY 2017
PROJ	3611-16-1193
DWG	C-502
SHEET	27 OF 29

VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING.
0 20 40 80
SCALE IN FEET

MATCH LINE - SEE DWG. C-501

PERMITTING	CHL	MJS
30% DESIGN	MAP	MJS
REVISION	BY	APVD
CHK	APVD	MJS
DR	MAP	MJS
DSGN	NO.	DATE
	B	8/22/17
	A	4/20/17

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ATTACHMENT F
PRE DESIGN INVESTIGATION FIELD ACTIVITIES REPORT

**PRE-DESIGN INVESTIGATION
FIELD ACTIVITIES REPORT
OPERABLE UNITS OU02 AND OU03**

**SARANAC LAKE GAS COMPANY, INC.
NYSDEC SITE NO. 516008**

WORK ASSIGNMENT NO. D007619-39

Prepared for:

**New York State Department of Environmental
Conservation Albany, New York**

Prepared by:

**MACTEC Engineering and Consulting, P.C.
Portland, Maine**

MACTEC: 3611161193

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GLOSSARY OF ACRONYMS AND ABBREVIATIONS

ACOE	Army Corps of Engineers
ASP	analytical services protocols
ASTM	American Society for Testing and Materials
bgs	below ground surface
DNAPL	dense nonaqueous-phase liquid
DUSR	data usability summary report
EPA	United States Environmental Protection Agency
FAR	Field Activities Report
FDR	field data records
ft	foot/feet
GPR	ground penetrating radar
HSA	hollow stem augers
IDW	investigation-derived wastes
MACTEC	MACTEC Engineering and Consulting, P.C.
MGP	manufactured gas plant
No.	number
NYLD	New York Leak Detection
NYS	New York State
NYSDEC	New York State Department of Environmental Conservation
OU	operable units

GLOSSARY OF ACRONYMS AND ABBREVIATIONS (CONTINUED)

PAH	polycyclic aromatic hydrocarbons
PCB	polychlorinated biphenyls
PDI	pre-design investigation
PER	Protection of Ecological Resources
PID	photoionization detector
PVC	polyvinyl chloride
RD	remedial design
RI	remedial investigation
SCG	standards, criteria and guidance values
SCO	soil cleanup objectives
SGV	sediment guidance values
Site	Saranac Lake Gas Company
SPT	standard penetration testing
SVOC	semi-volatile organic compounds
SY	square yards
TCLP	Toxicity Characteristics Leaching Procedure
UST	underground storage tank
VOC	volatile organic compound
WA	work assignment

1.0 INTRODUCTION

MACTEC Engineering and Consulting, P.C. (MACTEC), under contract to the New York State Department of Environmental Conservation (NYSDEC), is submitting this Pre-Design Investigation (PDI) Field Activities Report (FAR) for the Saranac Lake Gas Company, Inc. Site – NYSDEC Site 516008 (Site) in the Village of Saranac Lake, in Essex County, New York (Figure 1.1). The Site is listed as a Class 2 Inactive hazardous waste site in the Registry of Hazardous Waste Sites in New York State (NYS). This report has been prepared in accordance with the NYSDEC requirements in work assignment (WA) No. D007619-39 dated September 26, 2016, and with the July 2011 Superfund Standby Contract between MACTEC and the NYSDEC.

The Site, a former manufactured gas plant (MGP) facility, is in a residential setting off Payeville Lane in the Village of Saranac Lake, Essex County. The Site has been separated into three Operable Units (OUs).

The Site facility, OU01, is approximately 4.5 acres in size and is located east of and adjacent to the Adirondack Scenic Railroad right of way. Residential properties border OU01 to the north, east, and part of the west side, and North Country Community College soccer fields and facilities border OU01 to the south. An access road extends from Payeville Lane west to OU01. The Site and surrounding area is serviced by public water; groundwater is not believed to be used as a source of drinking water. Currently, OU01 is a vacant lot with an open, unoccupied one story brick building. Remedial Design (RD) activities for OU01 are not included in WA D007619-39.

Brandy Brook (OU02) flows through OU01 and continues in a northerly direction for approximately 1,000 feet, then turns to the west and flows for 700 feet, where it discharges to Pontiac Bay in Lake Flower. The section of the brook that turns to the west is culverted under a railroad crossing and culverted again under three driveway crossings and below Slater Avenue and Lake Flower Avenue. The driveway crossings range from 8 to 20 feet long and the culverted section below Slater Avenue and Lake Flower Avenue is approximately 250 feet long. The brook channel is approximately 3 to 5 feet wide. The bottom is scoured fine to medium sand, with pockets of mucky organic material in low lying depositional areas. In places of high water flow and where flow through the brook is more channelized, the bottom of the brook is comprised of predominantly gravel and cobbles. The channel is well entrenched with undercut banks for nearly its

entire length. Trees, branches, woody debris, and detritus were observed in the stream channel in multiple locations along the brook.

OU03 includes Pontiac Bay and an adjacent area within Lake Flower. Pontiac Bay (approximately 4 acres) is located along the northeast portion of Lake Flower adjacent to the intersection of Lake Flower Avenue, Brandy Brook Road, and River Street. Lake Flower is a Class A designated water body.

This FAR describes PDI field work conducted in support of the combined RD for OU02 and OU03. The field programs were completed following the procedures described herein and in general accordance with the PDI Field Activities Plan (MACTEC, 2016), Quality Assurance Program Plan (MACTEC, 2011a) and Program Health and Safety Plan (MACTEC, 2011b). This report is organized as follows:

- Section 2.0 describes the field activities conducted at the Site as part of the PDI.
- Section 3.0 presents the results of the PDI activities.
- Section 4.0 refines the current understanding of the extent of Site-related MGP waste-impacts.
- Section 5.0 includes the references discussed in this report.

2.0 PRE-DESIGN INVESTIGATION FIELD ACTIVITIES

Based on the WA and discussions with the NYSDEC, as well as the full-scale remediation proposal presented in the Record of Decision (ROD) for OU02 (NYSDEC, 2016) and OU03 (NYSDEC, 2015), the objective of the 2016/2017 PDI was to collect data to supplement the information gathered during the Remedial Investigation (MACTEC, 2015) to support the combined ROD for OU02 and OU03.

The technical objectives of the investigation were to:

- Complete an ecological study, bathymetric study, and wetland delineation at OU02 and OU03
- Complete a topographic and property boundary survey of OU02
- Complete a topographic survey of OU03 shoreline areas and determination of property lines at OU03
- Complete a hydrogeological investigation of OU02 and OU03 to evaluate the depth of groundwater, the potential for groundwater discharge into Pontiac Bay, and to assist with ROD dewatering specifications
- Complete an environmental and geotechnical investigation of OU02 to refine the horizontal and vertical extent of MGP waste impact to sediments, to support excavation support design, and to assess the slope stability of Brandy Brook
- Complete an environmental and geotechnical investigation of OU03 to refine the horizontal and vertical extent of MGP waste impact to sediments, to support shoring design, and to identify options available to render saturated sediment suitable for off-site disposal during the remedial action.

The remainder of Section 2.0 describes the field activities conducted during the PDI.

2.1 GENERAL FIELD OPERATIONS

Field activities were conducted in level D personal protection and in accordance with the MACTEC Field Activities Plan (MACTEC, 2016).

Subcontractors chosen to support the field activities included:

- Underground utility clearance was completed by New York Leak Detection, Inc. (NYLD)
- Site topographic, bathymetric, and property boundary surveys were completed by Prudent Engineering, LLP

- Direct push soil borings and piezometer installations were completed by Precision Environmental Services, Inc.
- Land geotechnical auger borings and piezometer installations were completed by Aztech Technologies, Inc.
- Barge-mounted geotechnical drilling was completed by Atlantic Testing Laboratories
- Soil and sediment chemical analyses were provided by TestAmerica, Inc.
- Soil geotechnical analyses were completed by Atlantic Testing Laboratories
- Transport and disposal of investigation derived waste (IDW) was conducted by NRC/OP-TECH

Prior to conducting field work, NYLD used ground penetrating radar (GPR) and power/radio techniques to identify underground utilities near the Site. NYLD's utility survey reports are included in Appendix A.

The Site topographical and bathymetric survey is included in Appendix B. Additionally, a location and elevation survey was performed for the fourteen piezometers installed during the investigation. Horizontal locations were tied to the NYS Plane Coordinate System using North American Datum of 1983, and measured to an accuracy of 0.1 foot. Vertical elevations of wells were tied to mean sea level, using National Geodetic Vertical Datum of 1988, and measured to an accuracy of 0.01 foot (Appendix B).

IDW soil and water was containerized in United States Department of Transportation-approved 55-gallon drums. The IDW was removed from the Site by NRC/OP-TECH and disposed at the ENPRO Services of Vermont, Inc. facility in Williston, Vermont. Waste Profile Forms and Waste Manifest are included in Appendix C.

2.2 SOIL AND SEDIMENT SAMPLING

Soil and sediment sample collection methods are described in the following subsections. Each sample was screened using a photoionization detector (PID) MiniRae 2000 with a 10.6 eV lamp, and soils and sediments were classified using the Unified Soil Classification System and the NYSDEC Guidance, “*Field Descriptions of Samples for Former MGP Sites*”. Field observations were recorded on field data records (FDRs) included in Appendix D. Sample locations are shown on Figures 2.1 and 2.2.

2.2.1 Hand Borings

Nine hand borings were collected utilizing Geoprobe® hand tools at selected locations throughout Brandy Brook to further evaluate the nature and extent of MGP-related waste present within the Brook sediments and underlying soils. Hand boring samples were collected at the locations depicted on Figure 2.1.

2.2.2 Direct Push Borings

A total of 43 direct push soil borings were advanced throughout Brandy Brook and along the shoreline of Pontiac Bay using Geoprobe® tooling mounted on a Bobcat® to further delineate the horizontal and vertical extent of MGP waste impacts. A subset of these 43 locations were advanced along the northern shoreline of Pontiac Bay with the purpose of investigating petroleum (gasoline) impacts to soils that were observed in this area. Soil samples were collected continuously using a four-foot long, two-inch diameter core sampler lined with acrylic liners for the collection of discrete subsurface soil samples. Direct push rods were advanced to refusal or field-determined depths. One-inch piezometers were installed at 14 direct push boring locations, as described in further detail in Subsection 2.3.

2.2.3 Hollow Stem Auger Borings

Hollow stem auger (HSA) borings were advanced at seven locations with a Geoprobe® 3230 DT track mounted drill rig. The borings were installed to gather geotechnical data, to further delineate the horizontal and vertical extent of MGP waste impacts, and to install groundwater piezometers.

Either continuous or standard 5-foot interval Standard Penetration Testing (SPT), in accordance with ASTM D 1586, was conducted using 2-inch outside diameter split-spoon samplers. One-inch piezometers were installed at four HSA boring locations, as described in Subsection 2.3.

2.2.4 Barge-mounted Sediment Borings

Six geotechnical borings (SB-501 to SB-506, Figure 2.2) were advanced in Pontiac Bay using a barge-mounted CME-450 drill rig to collect geotechnical data to support shoring design associated with the proposed Pontiac Bay sediment excavation. The six borings were completed to approximately 40 feet below top of sediments, and one (SB-505) was advanced to 60 feet below top of sediments. SPTs were conducted at five foot intervals from top of sediments to the termination of each boring.

2.3 PIEZOMETER INSTALLATION AND HYDRAULIC CONDUCTIVITY TESTING

To evaluate the depth of groundwater at Brandy Brook and Pontiac Bay, and to assist with RD dewatering specifications, a total of 14 piezometers were installed as shown on Figures 2.1 and 2.2 (designated by soil boring/piezometer [SB/PZ] labeling). Each piezometer was installed with a one-inch diameter polyvinyl chloride (PVC) casing with either a five or ten foot, schedule 40 PVC 0.010 inch slotted well screen. Following installation, piezometers were developed by over pumping for twenty minutes with a peristaltic pump. Well construction diagrams and development activities were recorded on FDRs (Appendix D).

Groundwater elevation measurements were collected from a subset of existing monitoring wells and the newly installed piezometers following their installation and development. Additionally, hydraulic conductivity testing was completed on two existing monitoring wells (MW-104 and -106) and on four of the newly installed piezometers. Testing was completed as slug tests using a solid mass of steel rebar (the slug) and a data logger. One falling head test was completed for each well, and one rising head test was completed at PZ-301. The test data was analyzed by the methods of Hvorslev (1951) and Bouwer and Rice (1976) and is included with the FDRs in Appendix D.

2.4 FILTER BAG TESTING AND GEOTECHNICAL LABORATORY ANALYSIS

Filter Bag Testing

Four onsite filter bag dewatering tests were conducted on Pontiac Bay sediments. Test locations are shown on Figure 2.2. Sediment samples were collected from two to three feet below top of sediment and placed in a hanging bag made of geotextile filter material and suspended over a 5-gallon collection bucket. The samples were observed for the ability and ease of the deposits to dewater. Test results are discussed in Subsection 3.3.

Geotechnical Laboratory Analysis

During HSA and barge drilling activities, a subset of soil samples were collected and submitted for the analysis of select geotechnical parameters by Atlantic Testing Laboratories. The borings, sample collection depths, and analytical parameters are presented in Table 2.1. Test results are discussed in Subsection 3.3.

2.5 CHEMICAL LABORATORY ANALYSIS

Three sediment samples and one soil sample were submitted in November 2016 for the analysis of t(34) PAHs as defined by the NYSDEC guidance, *NYSDEC Screening and Assessment of Contaminated Sediments* (NYSDEC, 2014).

Six additional soil samples were submitted in March/April 2017 for the analysis of :

- Volatile Organic Compounds (VOCs) by United States Environmental Protection Agency (EPA) Method 8260C
- Semivolatile Organic Compounds (SVOCs) by EPA Method 8270D
- Petroleum Hydrocarbons by New York State Analytical Services Protocols (ASP) Method 310.13

Additionally, two composite sediment samples from borings HB-401 through HB-405 and from borings SB-501 through 506 were submitted for full suite analysis for predisposal characterization, including the following analyses:

- Total polychlorinated biphenyls (PCBs) by method 8082
- Toxicity Characteristics Leaching Procedure (TCLP) VOCs by method 1311/8260
- TCLP SVOCs by method 1311/8270
- TCLP Pesticides by method 8081

- TCLP metals by methods 1311/6010 and 7471
- Ignitability by method 1030A
- pH by method 9045D
- percent solids test by method 160.3
- Reactivity by methods 9010 and 9030.

Validation of laboratory analytical results was completed by MACTEC in accordance with DER-10 (NYSDEC, 2010) and presented in the Data Usability Summary Reports (DUSR) included in Appendix E. Analytical results are discussed in Subsection 3.2.

2.6 ECOLOGICAL AND WETLAND SURVEY

An ecological and wetlands delineation survey was conducted within OU02 and OU03 to delineate wetlands and collect data on wetland functions and values. Wetland delineation activities included hand dug test pits, completion of wetland FDRs, identification and flagging of wetland boundaries, and an evaluation of the wetland functions and values. Wetlands delineation was conducted in accordance with Army Corps of Engineers (ACOE) Wetland Delineation Guidance including the Corps of Engineers Wetland Delineation Manual (ACOE, 1987) and the Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region, revised January, 2012. Wetlands were classified using the US Fish and Wildlife Service guidance Classification of Wetlands and Deepwater Habitats of the United States (Cowardin et al, 1979). The delineation survey is presented as the Wetland Delineation and Functional Assessment Report in Appendix F.

3.0 PRE-DESIGN INVESTIGATION RESULTS

Field observations and analytical results for the soil and sediment samples collected during the PDI are discussed in this section.

3.1 FIELD OBSERVATIONS AND MEASUREMENTS

Evidence of MGP-waste impacts (including visual, olfactory, and/or PID response) observed during soil sampling were identified and recorded on FDRs in accordance with the NYSDEC Field Descriptions of Samples for Former MGP Sites (included in Appendix D). A summary of MGP-waste impacts observed during sampling is summarized in Table 3.1.

3.2 SOIL AND SEDIMENT CHEMICAL ANALYTICAL RESULTS

Validation of the laboratory analytical results was completed by MACTEC in accordance with DER-10 and is presented in the DUSR included in Appendix E.

PAH Analysis

Three sediment samples and one soil sample were analyzed for the NYSDEC Sediment Guidance Target Compound List of 34 PAHs (t(34)PAH). As shown in Table 3.2, PAHs were detected in all three sediment samples, at concentrations within the Class B sediment guidance values (SGV) category in HB-409, and in the Class C SGV category in HB-406. PAHs were also detected above the Restricted Residential Use SCO and the Protection of Ecological Resources (PER) SCO in the one soil sample submitted (SB-330, Table 3.3).

VOC, SVOC, and Petroleum Hydrocarbon Analysis

Six additional soil samples from the shoreline of Pontiac Bay were analyzed for SVOCs by EPA Method 8270D. As shown in Table 3.4, SVOCs were detected above the Restricted Residential Use SCO and the PER SCO in four of the six submitted samples.

One of the six shoreline samples (SB-611) was analyzed for VOCs by EPA Method 8260C, in which three VOCs were detected above the Restricted Residential Use SCO and the Protection of Ecological Resources (PER) SCO (Table 3.4).

Additionally, to further characterize the petroleum impacts that were observed along the northern shoreline of Pontiac Bay, samples SB-607 and SB-611 were analyzed for the Petroleum Hydrocarbon footprint by New York State ASP Method 310.13. The results of this analysis revealed that both samples contained petroleum products which closely resemble highly degraded gasoline and motor oil (Table 3.4).

Predisposal Characterization

Two composite samples were submitted for the full suite analysis for predisposal waste characterization. The results of these analyses are presented in Table 3.5.

3.3 GEOTECHNICAL TESTING RESULTS

Filter Bag Testing

MACTEC performed four onsite filter bag dewatering tests on Pontiac Bay soils (locations shown on Figure 2.2). Observations from these four tests are provided in Table 3.6 below.

Table 3.6 Filter Bag Testing Results

Test ID	Start Time	Finish Time	Initial Volume of Soil	Final Volume of Water	Comments
FBT-1	13:53	14:23	1 Pint	131 mL	Soils cohesive after 30 minutes, fracture easily, dry, not much free water
FBT-2	13:58	14:28	1 Pint	170 mL	Soils cohesive, fracture easily, wetter than FBT-1
FBT-3	14:04	14:34	1 Pint	155 mL	Soils are coarser, traces of fine silt, wetter than FBT-2 and 3, higher organic content as well
FBT-4	14:08	14:38	1 Pint	155 mL	Highest content of fine soils

The filter bag testing indicated that the deposits would not likely be difficult to dewater during the remediation activities and would satisfy disposal facility water content, workability, and strength requirements subsequent to completion of the dewatering program.

Geotechnical Laboratory Testing

MACTEC submitted 12 soil samples to Atlantic Testing Laboratories for the analysis of grain size (American Society for Testing and Materials [ASTM] D422) and direct shear (ASTM D 3080). The

intent of the testing was to provide data to support the design of the sediment removal from Pontiac Bay. Direct shear and grain size testing was conducted to aid in the development of sheet pile cofferdam design should it be determined that the bay would be dewatered during sediment excavation. Grain size data will also be used to support the determination of the potential excavation/dredging methods as well as to further assess the most efficient methods to dewater and/or amend excavated sediments prior to offsite disposal. The results of the testing are summarized in Table 3.7. Individual laboratory testing results are presented in Appendix G.

The results of the geotechnical testing of the 12 samples showed that sample SB-506 (S-7) was characterized as silt with lesser amounts of sand and contained approximately 74% material finer than the Standard Number (No.) 200 sieve (characterized as brown silt with some fine sand). The remaining 11 samples were characterized as fine to coarse sand with lesser quantities (6.6 to 34 percent) of material finer than the Standard No. 200 sieve.