

LIMITED SITE DATA DOCUMENT

BATAVIA IRON & METAL COMPANY, INC.

CITY OF BATAVIA, GENESEE COUNTY, NEW YORK

SITE NO. 819018

These documents that follow are NOT part of the Contract Documents for the remedial work at the Batavia Iron & Metal Company, Inc. Site. The Department neither represents that the Site conditions will be the same as in the attached document nor considers the attached documents as being comprehensive and an actual description of the site conditions. The Contractor shall be responsible for performing the remediation work based on the existing conditions at the Site.

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INVESTIGATION SUMMARY

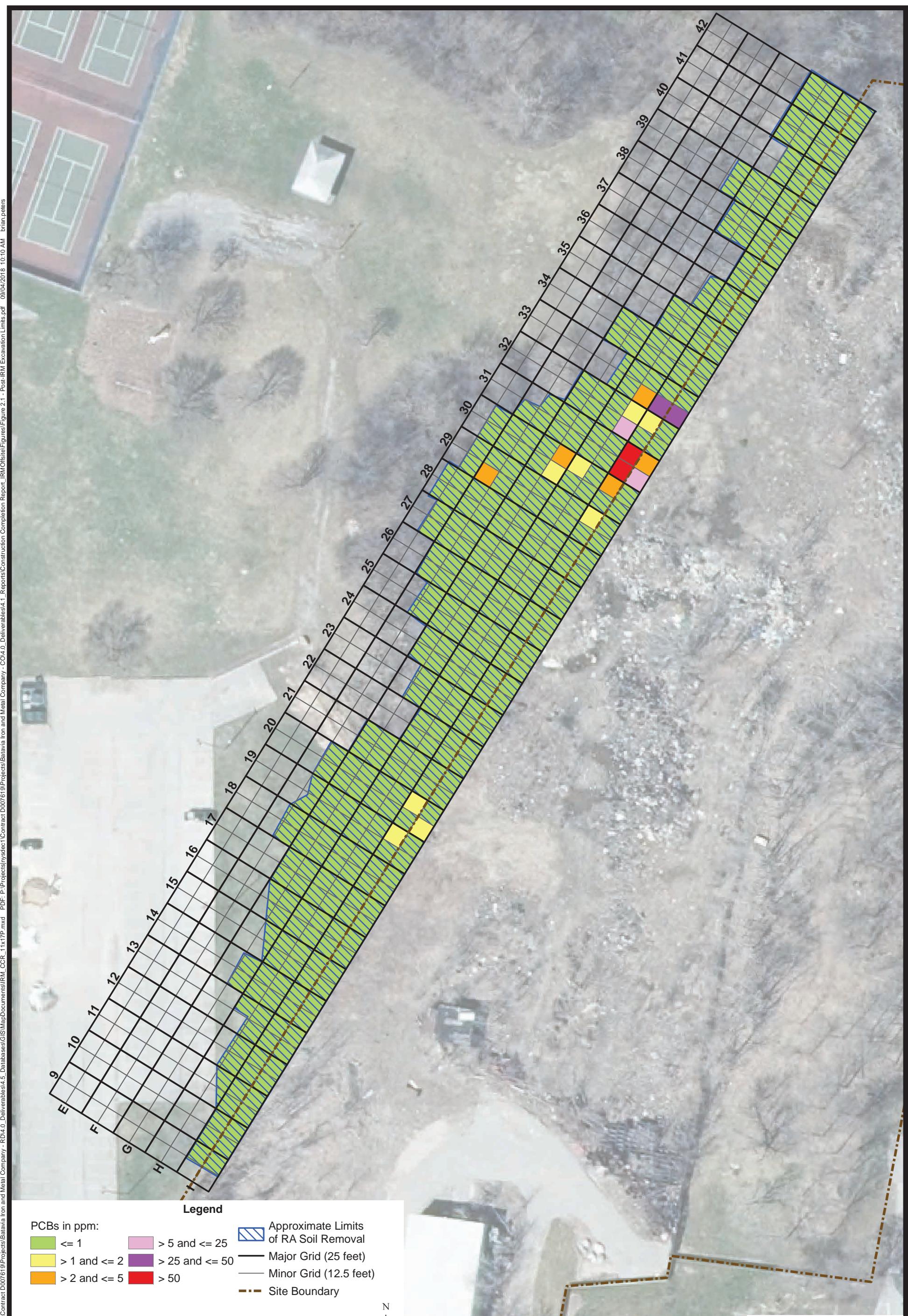
The following is a summary of reports describing historical investigations conducted at the Batavia Iron & Metal Company, Inc. Site between 2012 and 2019. These are provided in chronological order.

- GZA, 2012. Site Investigation and Remedial Alternatives Report, Batavia Iron & Metal Site, 301-305 Bank Street, Batavia, New York, NYSDEC Site #819018; Prepared by GZA GeoEnvironmental of New York; Prepared for the City of Batavia, New York. June 2012.
- MACTEC, 2015. Construction Completion Report, Area of Concern (AOC) 4, Remedial Action, Batavia Iron and Metal, State Superfund Site Number: 819018. Prepared for the New York State Department of Environmental Conservation, Albany, New York. June 2015.
- MACTEC Engineering and Consulting, P.C. (MACTEC), 2019. Construction Completion Report – Off-Site Interim Remedial Measure (City of Batavia Owned Property) Batavia Iron and Metal, State Superfund Site Number: 819018. Prepared for the New York State Department of Environmental Conservation, Albany, New York. February 2019.
- MACTEC, 2019. Pre-Design Investigation Report – Batavia Iron and Metal Company, NYSDEC Site 819018. Prepared for the New York State Department of Environmental Conservation, Albany, New York. April 26, 2019.
- MACTEC, 2020. Field Activities Report – Phase II Pre-Design Investigation, Batavia Iron & Metal Co. (NYSDEC Site #819018). Prepared for the New York State Department of Environmental Conservation, Albany, New York. September 10, 2020.

This information is being offered for consideration when planning the Site remedial work. Electronic copies of these reports can be provided upon request.

FIGURES

The enclosed figures are taken from the 2019 IRM Construction Completion Report and the Phase 1 and Phase 2 Pre-Design Investigation Reports. In general, the figures depict on- and off-Site area conditions observed during the 2018 and 2019 investigations, to some extent summarize additional data collected between 2018 and 2019, and include locations that need additional excavation after completion of the off-site IRM. The intent of each figure is to illustrate site conditions as they were known to have existed at the time the figure was generated. This information is being offered for consideration when planning the remedial work.



Document P:\Projects\ydec1\Contract D007619\Projects\Batavia Iron and Metal Company - RD4.0 Deliverables\4.5 Databases\GIS\MapDocuments\IRM_CCR_11x17P.mxd PDF: P:\Projects\ydec1\Contract D007619\Projects\Batavia Iron and Metal Company - RD4.0 Deliverables\4.1 Reports\Construction Completion Report_IRMOffsiteFigures\Figure 2.1 - Post-IRM Excavation Limits.pdf 09/04/2018 10:10 AM brian.peters

Genesee County color digital orthoimagery (2015) obtained from New York State GIS Clearinghouse at: gis.ny.gov

0 30 60 Feet

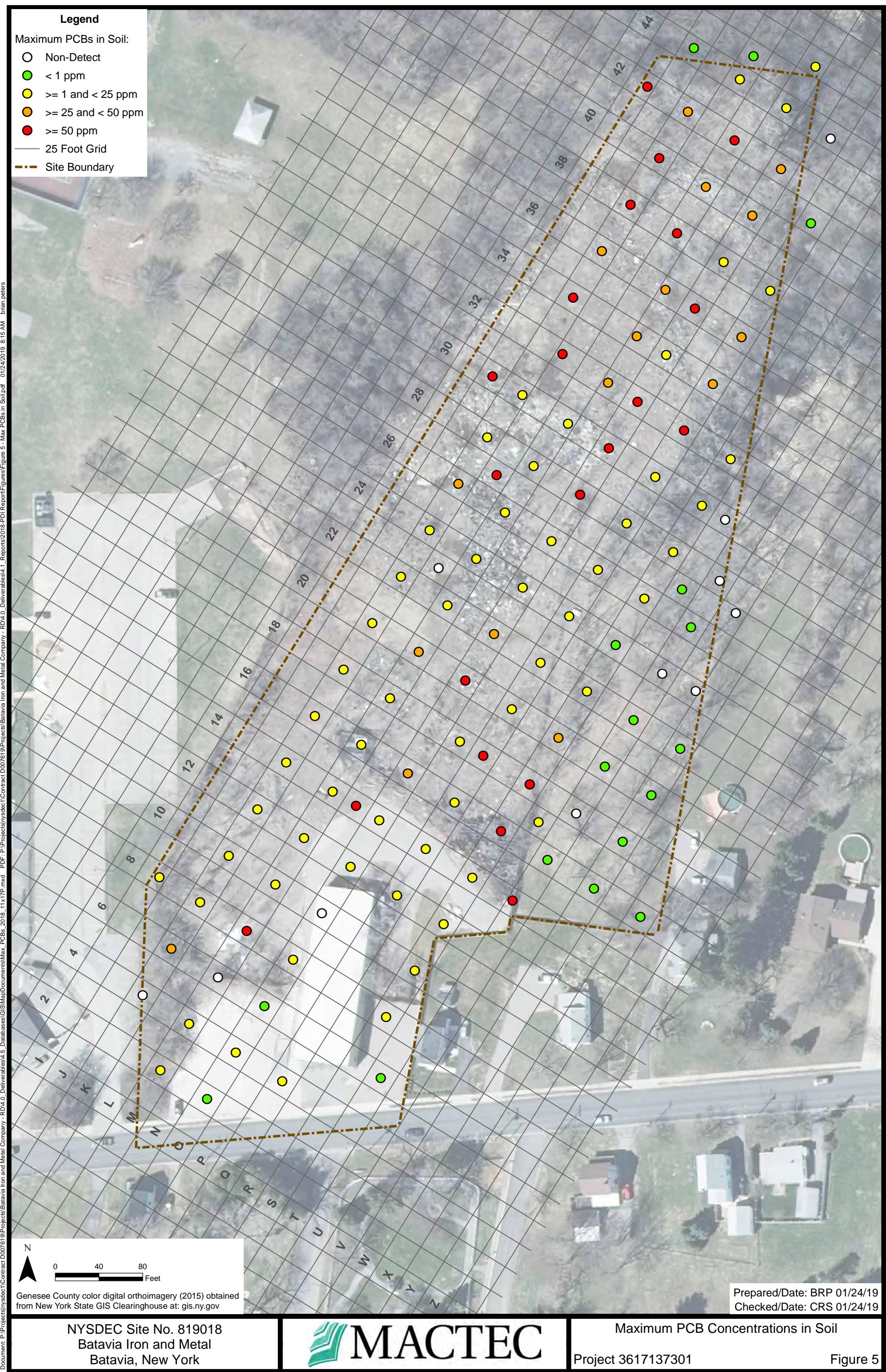
Prepared/Date: BRP 08/31/18
Checked/Date: CRS 08/31/18

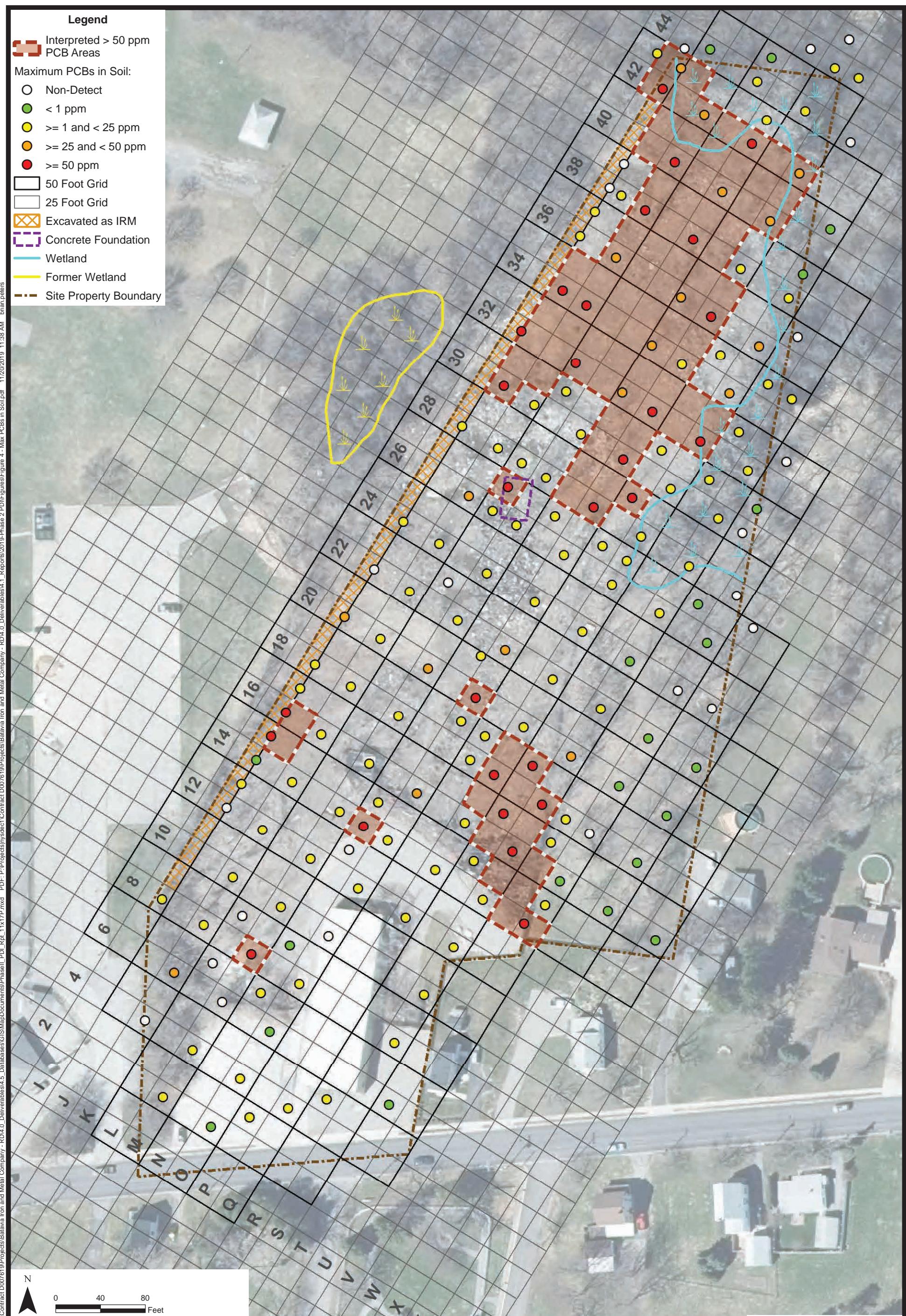
NYSDEC Site No. 819018
Batavia Iron and Metal Co.
Batavia, New York

 MACTEC

Post-IRM Excavation Limits
and Exceedances
Project 3617137301

Figure 2.1





TABLES

The enclosed tables are taken from the Phase 1 and Phase 2 Pre-Design Investigation Reports, and do not include all historical site data. Additional site data is available in the site investigation reports listed above. In general, the tables depict on- and off-Site area conditions observed during the Pre-Design Investigations and the off-site Construction Activities. The intent of the tables is to depict contaminant concentrations for soil and groundwater.

Contaminant concentrations presented on the tables are those reported at the time the table was generated. This information is being offered for consideration when planning the remedial work.

Table 6: Monitoring Well Data and Groundwater Elevations

Monitoring Location	Northing	Easting	Surveyor	Top of Riser or Monitoring Point Elevation	Casing	Ground	Depth of Well (feet btor)	Length of Well Screen (feet)	Top of Well Screen Elevation (feet)	Bottom of Well Screen Elevation (feet)	DTW-7/26/18 (feet btor)	Groundwater Elevation 7/26/18
MW-1	1097427.7	1257398.2	GZA-Corrected	900.65	902.45	898.85	24.5	NA	NA	876.2	4.08	896.6
MW-2	1097508.9	1257271.6	GPI	899.45	NA	897.21	22.2	NA	NA	877.3	3.13	896.3
MW-3	1097423.2	1257072.3	GZA-Corrected	901.23	901.18	898.23	23.7	NA	NA	877.6	4.33	896.9
MW-4	1096757.5	1256841.6	Patriot	903.00	903.40	901.88	11.5	5.0	896.5	891.5	10.80	892.2
MW-5	1097104.5	1257193.7	GPI	901.81	901.95	899.25	8.5	4.0	897.3	893.3	6.75	895.1
MW-7	1097337.9	1257159.8	GPI	902.89	903.07	900.77	10.0	5.0	897.9	892.9	5.69	897.2
MW-8	1096833.6	1257030.6	GPI	907.69	907.87	905.24	15.0	10.0	902.7	892.7	14.57	893.1
MW-9	1096610.6	1256993.1	Patriot	902.21	902.71	902.66	11.9	5.0	895.4	890.4	10.33	891.9
MW-101	1097272.0	1257072.0	Patriot	901.26	901.39	898.92	10.5	5.0	895.8	890.8	5.74	895.5
MW-102	1097232.3	1257103.4	Patriot	901.33	NA	899.23	9.9	5.0	896.4	891.4	5.69	895.6
MW-103	1097031.9	1257038.8	Patriot	901.42	NA	900.33	11.8	5.0	894.6	889.6	6.78	894.6
MW-104	1097004.2	1257260.6	Patriot	899.79	NA	899.32	10.7	5.0	894.1	889.1	6.74	893.1
MW-105	1096839.3	1256957.6	Patriot	904.97	905.38	905.29	10.0	5.0	900.0	895.0	dry	<894.97
MW-106	1096795.6	1257134.5	Patriot	903.72	NA	903.38	11.2	5.0	897.5	892.5	dry	<893.72
MW-107	1096573.1	1256792.8	Patriot	899.95	900.1	899.94	9.9	5.0	895.1	890.1	8.02	891.9
SP-12	1097231.3	1257195.4	Patriot	901.26	NA	899.12	9.9	5.0	896.4	891.4	5.12	896.1

Notes:

1. Monitoring points have been established at the top of the PVC riser, with the exception of MW-1, MW-2 and MW-3, which are top of steel casing.
2. The horizontal and vertical data is New York State Planes Coordinates, West Zone, North American Datum 83 and North Atlantic Vertical Datum 88, (US Survey Foot) respectively.
3. Survey completed by: Patriot Design and Consulting-October 2018; GPI Engineering, Landscape Architecture & Surveying, LLP-August 2018, and GZA-2012 that was corrected based on relative elevations at MW-4.

btor = below top of riser

DTW = depth to water

NA = Not available

Table 3: Soil PFAS Results

Parameter	Location	HA-N36	HA-T33	TP-J34	TP-L26	TP-P27	TP-P27	TP-R17
	Sample Date	8/1/2019	8/1/2019	7/29/2019	7/30/2019	7/30/2019	7/30/2019	7/31/2019
Sample Depth (ft bgs)	819018-HAN3600	819018-HAT3300	819018-TPJ3402	819018-TPL2600	819018-TPP2700	819018-TPP2700D	819018-TPR1700	
QC Code	0 FS	0 FS	2 FS	0 FS	0 FS	0 FD	0 FS	
Parameter	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result	Qualifier
Per- and polyfluoroalkyl substances (ug/kg)								
N-ethyl perfluorooctanesulfonamidoacetic acid (N-EtFOSA)	2.7 U		2.9 U	1.4 J	0.7 J	2.9 U	2.9 U	2.2 U
Perfluorobutanoic acid (PFBA)	2.1		0.27 J	0.26 J	0.24	0.22 J	0.23 J	1.4
Perfluorodecanesulfonic acid (PFDS)	0.13 J		0.29 U	0.35	0.088 J	0.29 U	0.29 U	1.9
Perfluorodecanoic acid (PFDA)	0.5		0.06 J	0.15 J	0.077 J	0.17 J	0.18 J	0.62
Perfluorododecanoic acid (PFDoA)	0.27 U		0.29 U	0.33 U	0.23 U	0.29 U	0.29 U	0.29
Perfluoroheptanesulfonic acid (PFH ₇ S)	0.047 J		0.29 U	0.23 J	0.23 U	0.29 U	0.29 U	0.22 U
Perfluoroheptanoic acid (PFHpA)	0.08 J		0.14 J	0.14 J	0.066 J	0.092 J	0.12 J	0.19 J
Perfluorohexanesulfonic acid (PFHxS)	0.12 J+		0.16 J+	0.31 J+	0.1 J+	0.29 U	0.29 U	0.15 J+
Perfluorohexanoic acid (PFHxA)	0.27 U		0.11 J	0.33 U	0.23 U	0.09 J	0.12 J	2.2 U
Perfluorononanoic acid (PFNA)	0.29		0.16 J	0.23 J	0.044 J	0.18 J	0.25 J	0.24
Perfluorooctanesulfonic acid (PFOS)	10		2.2	15	2.8	1.5	2.1	6.7
Perfluorooctanoic acid (PFOA)	0.7		0.89	1.1	0.18 J	0.23 J	0.35	0.59
Perfluorotetradecanoic acid (PFTeDA)	0.27 U		0.29 U	0.33 U	0.23 U	0.29 U	0.29 U	0.15 J
Perfluoroundecanoic acid (PFUnDA)	0.15 J		0.08 J	0.17 J	0.058 J	0.12 J	0.13 J	0.29
Perfluorooctanesulfonamide (FOSA)	0.27 U		0.29 U	0.25 J	0.23 U	0.29 U	0.29 U	0.095 J

Notes:

ft bgs = feet below ground surface

QC code: FS= field sample, FD = field duplicate

Samples analyzed for:

Per-and polyfluoroalkyl substances (PFAS) by USEPA Modified Method 537

Results in micrograms per kilogram (ug/kg)

Only detected compounds shown (detections in bold)

Qualifiers: U = not detected; J = estimated value (+ = biased high)

There are no New York soil standards or guidance values for PFAS compounds

Table 4: Boring/Test Pit Select Data

Site Type	Sample Location							Analytical Results																
	Location ID	Grid ID	Easting	Northing	Ground Elevation (ft)	Field Sample ID	Sample Date	Fill Thickness ¹ (ft)	Observed Impacts ² (Y/N)	Comment [Staining, Odors, Types of Fill [Metal, Ceramics, Bricks]]	Sample Depth (ft bgs)	PCB (total) 1 (mg/kg)	Arsenic 16 (mg/kg)	Barium 350 (mg/kg)	Cadmium 2.5 (mg/kg)	Chromium 22 (mg/kg)	Copper 270 (mg/kg)	Lead 400 (mg/kg)	Manganese 2000 (mg/kg)	Mercury 0.81 (mg/kg)	Nickel 140 (mg/kg)	Selenium 36 (mg/kg)	Silver 36 (mg/kg)	Zinc 2200 (mg/kg)
						819018-TPP2908	7/29/2019				8	0.25 U	0.65 J	27.2	0.074 J	8.6	8.5	5.7	244	0.012 J	7.3	4.7 U	0.71 U	38.3

Table 4: Boring/Test Pit Select Data

Site Type	Sample Location							Analytical Results															
	Location ID	Grid ID	Easting	Northing	Ground Elevation (ft)	Field Sample ID	Sample Date	Fill Thickness ¹ (ft)	Observed Impacts ² (Y/N)	Comment (Staining, Odors, Types of Fill [Metal, Ceramics, Bricks])	Sample Depth (ft bgs)	PCB (total) 1 (mg/kg)	Arsenic 16 (mg/kg)	Barium 350 (mg/kg)	Cadmium 2.5 (mg/kg)	Chromium 22 (mg/kg)	Copper 270 (mg/kg)	Lead 400 (mg/kg)	Manganese 2000 (mg/kg)	Mercury 0.81 (mg/kg)	Nickel 140 (mg/kg)	Selenium 36 (mg/kg)	Silver 36 (mg/kg)
HA	HA-U15	U15	1257124	1096745	905.1	819018-HAU1500 819018-HAU1501 819018-HAU1502	8/1/2019 8/1/2019 8/1/2019	0	N None	0 1 2	4.2 10 4.7 J+	5.3 7.8 5.2	75.5 218 92.5	4.2 10.6 3.7	34.4 68.1 29.2	1620 3230 1270	371 806 359	594 770 586	0.31 0.81 0.33	41.5 56.1 33	1.4 J 2.6 J 1.8 J	0.68 1.6 0.66 J	688 1250 554

Notes:

1 = Fill thickness as determined by visual observations noted in Attachment 2, test pit logs.

2 = Impacts are defined as staining, odor, or PID hits over 1.0 part per million (ppm).

Site Type: TP=test pit; HA=test pit

Latitude and longitude in New York State Plane West; survey data from hand held global positioning system device.

Elevation in feet (ft) above mean sea level North American Vertical Datum 1988

f bgs = feet below ground surface

Analytical Results: Results shown for PCBs and metals detected at concentrations greater than the New York State Part 375 Soil Cleanup Objective (SCO) for Residential Use.

PCB = polychlorinated biphenols

Number immediately below PCB (total) and individual metals is the New York State Part 375 SCO for Residential Use for that analyte

Results in milligrams per kilogram (mg/kg)

Bold results indicate analyte detected

Shaded result indicates concentration exceeds the respective SCO

J = estimated value

U = not detected

LOGS AND RECORDS

The enclosed logs and records are select logs taken from the Phase 1 and Phase 2 Pre-Design Investigation Reports and do not include all historical logs and records. Additional logs and records are available in the site investigation reports listed above. In general, the logs and records depict information from soil and groundwater sampling records to provide general conditions observed during the Pre-Design investigations.

PHASE 1 PRE-DESIGN INVESTIGATION FIELD DATA RECORDS

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

Project Name:	Batavia Iron and Metal Company	Test Pit ID:	TPK16
Project Location:	Batavia NY	Page No.	1
Project No.:	3617137301.03.****	Client:	NYSDEC
		of:	1
Test Pit Location:	K16	Monitoring Equipment:	Mini-Rae 3000 PID
Weather:	70, mostly cloudy	Photographs (Y/N):	Y
Surface Conditions:	Soil/metal/debris	Protection Level:	D
Subcontractor:	Nothnagle	Length of Exc:	6.2 ft
Date Started:	7/30/18	Width of Exc:	4.2 ft
Operator:	Thom Mangefrida	Date Completed:	7/30/18
Logged By:	Nate Vogan	Checked By:	HEP
Equipment:	Bobcat E45	Refusal Depth:	NA
Reference Elevation NA		Total Depth:	7.0 ft
Water Level:	Dry	Time:	1145



Depth (ft. bgs)	Sample No. & Type	Monitoring				Lab Sample ID	Sample Description and Classification	USCS Group Symbol	Remarks
		Pocket Pen/ Torrane (Kg/cm ²)	PID Field Scan	PID Headspace	Lab Tests Performed				
0			0.0	0.5			Moist, soft, brown, SILT, little f sand and fmc gravel, some metal, little ceramics. No odor	FILL ML SW SW	
1.5			NA	NA			Hard, moist, medium dark brown, fm SAND, mostly bricks, some CD, no odors.		
4.5			0.0	0.8			Moist, soft, brownish black, SILT, little fine sand and organics, noncohesive, slight organic odor.		
5.5			0.0	0.7			Soft, moist, medium brown, FM SAND, little fmc subrounded gravel, some subrounded to subangular cobbles, no odors.		
7			0.0	0.9			SAA		
							EOP at 7 ft bgs		

PLAN VIEW

CROSS-SECTIONAL VIEW



NOTES:

Sample 0-0.2, 2.3 (SVOC), 4.5, 7.0

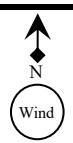
TEST PIT RECORD

PHASE 2 PRE-DESIGN INVESTIGATION FIELD DATA RECORDS

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

Project Name: Batavia Iron and Metal Company		Test Pit ID: TPP35
Project Location: Batavia NY		Page No. 1
Project No.: 3617137301.03.**** Client: NYSDEC		of: 1
Test Pit Location: TPP35	Monitoring Equipment: Mini-Rae 2000 PID	Location Sketch 
Weather: 80, Sunny	Photographs (Y/N): Y Protection Level: D	
Surface Conditions: Soil/metal/debris	Length of Exc: 8' Width of Exc: 4'	
Subcontractor: Nothnagle	Date Started: 7/29/19 Date Completed: 7/29/19	
Operator: Thom Mangefrida	Logged By: Katie Amann Checked By: Haley Plante	
Equipment: Back Hoe	Refusal Depth: NA Total Depth: 8'	
Reference Elevation: NA	Water Level: 2.7' Time: 1305	

Sample Information			Monitoring				Sample Description and Classification		USCS Group Symbol	Remarks				
Depth (ft. bgs)	Sample No. & Type	Pocket Pen/ Torvane (Kg/cm ²)	PID Field Scan	PID Headspace	Lab Tests Performed	Lab Sample ID								
0	1 2 3 4 5	0.4 0.0 0.0 0.0 0.0	000 002 004 006 008	Brown SILT, soft, some fine sand, some fine gravel, some metal debris, rubber, plastic and glass, organics throughout, moist. Brown SILT, little clay, little fine sand, little fine gravel, some metal debris, trace bricks, dark grey staining, wet. Gray and brown mottled SILT, little clay, trace fine sand, moist. Gray and brown mottled SILT, little clay, trace fine sand, moist. Gray and brown mottled SILT, little clay, trace fine sand, moist.	FILL ML ML ML ML	EOP 8 ft bgs								
2														
4														
6														
8														

PLAN VIEW



CROSS-SECTIONAL VIEW



NOTES:

- 1 - metals, mercury, PCBs, TCLP metals
- 2 - metals, mercury, PCBs, TCLP metals
- 3 - metals, mercury, PCBs

- 4 - metals, mercury, PCBs
- 5 - metals, mercury, PCBs

TEST PIT RECORD

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

Project Name: Batavia Iron and Metal Company

Test Pit ID: TPR19

Project Location: Batavia NY

Page No. 1

Project No.: 3617137301.03.**** Client: NYSDEC

of: 1

Test Pit Location: TPR19

Monitoring Equipment: Mini-Rae 2000 PID

Weather: 77F, mostly cloudy, windy

Photographs (Y/N): Y Protection Level: D

Surface Conditions: Soil/metal/debris

Length of Exc: 8' Width of Exc: 4'

Subcontractor: Nothnagle

Date Started: 7/30/19 Date Completed: 7/30/19

Operator: Thom Mangefrida

Logged By: Katie Amann Checked By: Haley Plante

Equipment: Back Hoe

Refusal Depth: NA Total Depth: 8'

Reference Elevation: NA

Water Level: Dry Time: 1525

Location Sketch



Sample Information			Monitoring				Sample Description and Classification	USCS Group Symbol	Remarks
Depth (ft. bgs)	Sample No. & Type	Pocket Pen/Torvane (Kg/cm²)	PID Field Scan	PID Headspace	Lab Tests Performed	Lab Sample ID			
0	1		0.0			000	Brown fine SAND and SILT, trace coarse sand, little fine gravel, organic material, dry (fill material), trace glass, metal, plastic fragments.	FILL	
2			0.0			002	Brown fine SAND and SILT, trace coarse sand, some fine gravel, trace coarse gravel, organic material, dry, trace glass fragments.		
4			0.0			004	Orange-brown fine SAND and SILT, trace coarse sand, trace fine gravel, trace clay, moist.		
6			0.0			006	Same as 4-ft. description.		
8			0.0			008	Same as 4-ft. description.		
							EOP 8 ft bgs		

PLAN VIEW



CROSS-SECTIONAL VIEW



NOTES:

1 - PCBs, metals, mercury (Duplicate , MD, MSD collected)

2 - PCBs, metals, mercury

3 - PCBs, metals, mercury

4 - PCBs

5 - PCBs

TEST PIT RECORD

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

Project Name: Batavia Iron and Metal Company

Test Pit ID: TPR33

Project Location: Batavia NY

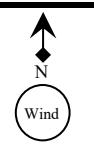
Page No.

1

Project No.: 3617137301.03.**** Client: NYSDEC

of:

1

Test Pit Location: TPR33		Monitoring Equipment: Mini-Rae 2000 PID		Location Sketch 	
Weather: 68F, sunny, wind to east @ 3 mph		Photographs (Y/N): Y Protection Level: D			
Surface Conditions: Soil/metal/debris		Length of Exc: 8' Width of Exc: 4'			
Subcontractor: Nothnagle		Date Started: 8/1/19 Date Completed: 8/1/19			
Operator: Thom Mangefrida		Logged By: Katie Amann Checked By: Haley Plante			
Equipment: Back Hoe		Refusal Depth: NA Total Depth: 6'			
Reference Elevation: NA		Water Level: 4.5' Time: 0855			

Depth (ft. bgs)	Sample Information		Monitoring			Sample Description and Classification	USCS Group Symbol	Remarks
	Sample No. & Type	Pocket Pen/ Torvane (Kg/cm ²)	PID Field Scan	PID Headspace	Lab Tests Performed	Lab Sample ID		
0	1 2 3 4	0.0 0.0 0.0 0.0	000 002 004 006	Dark brown SILT, some fine sand, trace clay, little fine gravel, some organic material, trace brick and glass fragments, moist. Gray and brown mottled SILT and fine SAND, little clay, moist. Gray and brown mottled CLAY, some silt, trace fine sand, moist. Gray-brown SILT, some fine sand, little clay, wet.	EOP 6 ft bgs	ML ML CL ML		
2								
4								
6								

PLAN VIEW



CROSS-SECTIONAL VIEW



NOTES:

1 - PCBs

2 - PCBs

3 - PCBs

4 - PCBs

TEST PIT RECORD

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

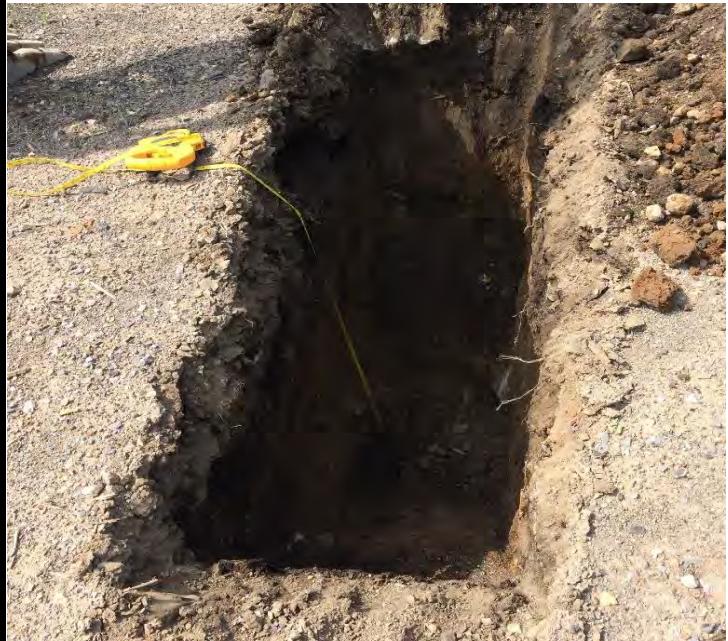
Project Name:	Batavia Iron and Metal Company	Test Pit ID:	TPN15
Project Location:	Batavia NY	Page No.	1
Project No.:	3617137301.03.****	Client:	NYSDEC
		of:	1
Test Pit Location:	TPN15	Monitoring Equipment:	Mini-Rae 2000 PID
Weather:	71F, partly cloudy	Photographs (Y/N):	Y
Surface Conditions	Soil/metal/debris	Length of Exc:	8'
Subcontractor:	Nothnagle	Width of Exc:	4'
Operator:	Thom Mangefrida	Date Started:	7/31/19
Equipment:	Back Hoe	Date Completed:	7/31/19
Reference Elevation:	NA	Logged By:	Katie Amann
		Checked By:	ANC
		Refusal Depth:	NA
		Total Depth:	6'
		Water Level:	Dry
		Time:	1030

Location Sketch



Sample Information			Monitoring				Lab Sample ID	Sample Description and Classification	USCS Group Symbol	Remarks
Depth (ft. bgs)	Sample No. & Type	Pocket Pen/Torvane (Kg/cm ²)	PID Field Scan	PID	Headspace	Lab Tests Performed				
0	1	0.0	0.0				000	Gray-brown fine SAND and SILT and fine GRAVEL, dry (Non-native road base material), trace glass, metals, brick, plastic, slag fragments. 1.0-2.0 ft.: Brown/black foundry sand.	FILL	
2			0.0							
4			0.0							
6			0.0							
								EOP 6 ft bgs		

PLAN VIEW



CROSS-SECTIONAL VIEW



NOTES:

- 1 - PCBs, metals, mercury, TCLP metals
- 2 - PCBs, metals, mercury, TCLP metals
- 3 - PCBs, metals, mercury
- 4 - PCBs, metals, mercury

TEST PIT RECORD

TEST PIT RECORD



511 Congress Street, Portland Maine 04101

Project Name:	Batavia Iron and Metal Company	Test Pit ID:	TPP27
Project Location:	Batavia NY	Page No.	1
Project No.:	3617137301.03.****	Client:	NYSDEC

Sample Information		Monitoring					Sample Description and Classification		USCS Group Symbol	Remarks
Depth (ft. bgs)	Sample No. & Type	Pocket Pen/ Torvane (Kg/cm ²)	PID Field Scan	PID Headspace	Lab Test Performed	Lab Sample ID				
0	1	0.6 0.4 0.0 0.0 0.0 0.0 0.0 0.0	000 002 004 006 008	Dark brown SILT, soft, some fine sand, trace glass, brick, wood debris, organic material throughout, moist. Dark brown SILT, soft, some fine sand, trace glass, brick, wood debris, organic material throughout, moist. Gray and gray-brown SILT, some f sand, little clay; occasional 1-2" lenses of orange-brown clay, trace organic material, moist (This lithology starts at 2-ft). Same as 3-ft description. Brownish gray fine SAND, little clay, trace organic material, damp. Brownish gray fine SAND, little clay, trace coarse sand, wet.	ML ML ML ML SM SM	EOP 8 ft bgs				
2										
3										
4										
6										
8										

PLAN VIEW



CROSS-SECTIONAL VIEW



NOTES:

1 - PCBs, metals, mercury, PFAS (Duplicate samples collected)

2 - PCBs, metals, mercury

3 - PCBs, metals, mercury

4 - PCBs, metals, mercury

(MS/MSD samples collected)

5 - PCBs, metals, mercury

TEST PIT RECORD

APPENDIX A

PHOTOGRAPHS

Appendix A contains select photographs of the Site taken during the execution of the Phase 2 Pre-Design Investigation in 2019 and wetland delineation activities in 2019. This information is offered for consideration when planning the remedial work.

Site Photographs

Client:	NYDEC	Project Number:	3617137301
Site Name:	Batavia Iron and Metal Co., Inc.	Site Location:	Batavia, New York
Photographer: Charles Lyman			
Date: 11/03/2019			
Photograph: 1			
Direction: North			
Description: View of isolated wetland on west side of property			
Photographer: Charles Lyman			
Date: 11/03/2019			
Photograph: 2			
Direction: West			
Description: View of isolated wetland on west side of property			

Site Photographs

Client:	NYDEC	Project Number:	3617137301
Site Name:	Batavia Iron and Metal Co., Inc.	Site Location:	Batavia, New York
Photographer:	Charles Lyman		
Date:	11/03/2019		
Photograph:	3		
Direction:	North		
Description:	View of swale connecting isolated wetland to forested wetland north of the site.		
Photographer:	Charles Lyman		
Date:	11/03/2019		
Photograph:	4		
Direction:	Northwest		
Description:	View of forested wetland along the back (north) side of the site.		
	 A photograph showing a small, shallow stream or swale flowing through a mix of wetland and forested areas. The water is clear and reflects the surrounding environment. Fallen leaves are scattered along the banks. In the background, there's a dense stand of trees, some with autumn foliage and others bare. The date '11-03-2019' is visible in the bottom right corner of the photo.  A photograph of a forested wetland area. The foreground is filled with tall, dense clumps of reeds growing out of a body of water. Behind them, a stand of bare trees rises, their trunks vertical against the sky. The date '11-03-2019' is visible in the bottom right corner of the photo.		

Site Photographs

Client:	NYDEC	Project Number:	3617137301
Site Name:	Batavia Iron and Metal Co., Inc.	Site Location:	Batavia, New York
Photographer:			
Date:	11/03/2019		
Photograph:	5		
Direction:	South		
Description:	<p>View along back of property, note edge of forested wetland (picture left).</p> 		
Photographer:	Charles Lyman		
Date:	11/03/2019		
Photograph:	6		
Direction:	North		
Description:	<p>View of swale between isolated wetland and forested wetland located north of the site.</p>		

APPENDIX B

SITE SURVEY

Appendix B contains the most recent surveys of the Site. This information is offered for consideration when planning the remedial work.

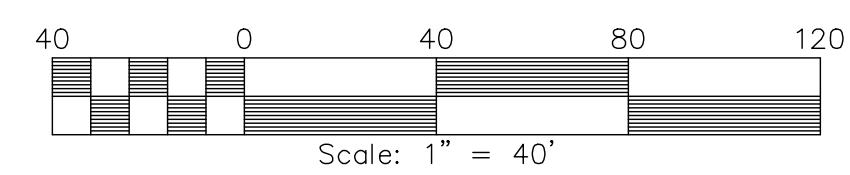


NOTES:

1.) FINAL GRADING TOPOGRAPHIC DATA WAS COLLECTED BY GPI ON JUNE 1, 2018

2.) THIS MAP REFERENCES A BOUNDARY SURVEY PERFORMED BY LU ENGINEERS
ON JULY 10, 2014.

3.) HORIZONTAL DATUM: NAD 83 NY STATE PLANE WEST ZONE
VERTICAL DATUM: NAVD 88



LEGEND	
□	CATCH BASIN
□	GAS METER
□	MOTOR VEHICLE
○	SANITARY PLUG
○	SANITARY MANHOLE
●	TELEPHONE POLE
●	REBAR
○	IRON PIPE
◆	DECIDUOUS TREE
—	WOOD FENCE
—	CHAINLINK FENCE
~~~~~	WOODS LINE

THIS MAPPING WAS PREFORMED FOR:  
CONTRACTOR: NATURE'S WAY ENVIRONMENTAL  
ADDRESS: 3553 CRITTENDEN ROAD  
ALDEN, NY 14004  
PHONE: (716) 937-6527 EXT. 105

## **APPENDIX C**

### **RECORD OF DECISION**

Appendix C contains the Record of Decision (ROD) issued for the Site (April 2013) under the NYSDEC Superfund Program. This information is offered for consideration when planning the remedial work.

ROD is located at this link: <https://www.dec.ny.gov/data/DecDocs/819018/>

## **APPENDIX D**

### **Dewatering Calculations**

Appendix D contains a memo summarizing the dewatering calculations that can assist in sizing the construction water treatment system to be used during the Work. This information is offered for consideration when planning the remedial work.



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## Batavia Iron and Metal Co., Inc.

Batavia, New York

### Dewatering Calculations

October 6, 2020

#### **Introduction:**

Shallow soils at the Batavia Iron and Metal Site (Site) have been contaminated primarily with relatively low concentrations of polychlorinated biphenyl (PCB) compounds. While most excavation areas across the Site are proposed for the removal of the upper 1 to 6 feet of soil, there are a few areas on-site that are proposed for excavation up to 8 feet below the natural ground surface. Depth to groundwater over much of the Site area to be excavated varies seasonally. For the purposes of this dewatering evaluation, the July 26, 2018 synoptic set of depth to groundwater measurements has been used. Based on these data, the depth to groundwater is estimated to vary from ground surface (at the wetland area on the northern portion of the site) to 8 feet or more below natural ground surface at the south end of the site. Hence, dewatering only needs to be implemented for areas with proposed excavation depths below the water table. In general, where these areas occur, excavations do not exceed 4 feet below the water table although two areas (GW-6 and GW-7) extend 5 to 6 feet below the water table.

The Site has been gridded with a spacing of 50 feet x 50 feet, and the grid has been used to assign expected depths of excavation within a grid block. Based on considerations of Site workflow, access, and similar project experience, it has been assumed that no more than 6 grid blocks will be dewatered and excavated at a time (a Work Area), while other grid blocks are yet to be excavated or have already been excavated and backfilled. Work Areas (also referred to as cells) range in size from about 2 to 6 of these 50-foot x 50-foot grid blocks.

Because the deepest excavation depth within each Work Area has been assumed for the purposes of estimating dewatering flows, an additional dewatering depth below this has not been considered. In practice, assuming reasonable success with dewatering to the target depths, the adjacent shallower excavations will be dewatered well below the target bottom elevations. These conditions are anticipated to provide for a stable working surface on which to place and compact backfill materials.

#### **Hydrogeology:**

Depth to groundwater measurements were made at 5 monitoring well and 7 piezometer locations to generate an interpreted groundwater contour map of the Site. Slug test data from a prior contractor (GZA) was reviewed and considered along with test pit and boring log soil descriptions to estimate the hydraulic conductivity (K) of the soils underlying the Site.



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These calculations provided estimates of K ranging from an arithmetic mean of 0.015 feet per day (ft/d), a geometric mean of 0.0080 ft/d, and a maximum of 0.037 ft/d. These estimates are consistent with soil descriptions at the Site: shallow fill and silty sand with significant portions of silt and/or clay at depth, and direct push refusal around 14 feet below ground surface. Based on these conditions, a saturated thickness of 10 feet was assumed to be capable of contributing groundwater to excavations at flow rates requiring management.

As with many dewatering projects, it is possible that layers of higher permeability material are present in the subsurface which could yield substantial volumes of groundwater. To account for this uncertainty an assumed more conservative horizontal K of 0.40 ft/d has been used in the calculations. In addition, upward flow into the bottom of the excavation was considered based on the bottom area of each Work Area, a unit upward hydraulic gradient and a vertical K of 0.04 feet/day (10% of the estimated horizontal K).

Of note, without a barrier around the wetland at the north end of the site, the wetland could provide a constant head of water and/or deliver large flows to any nearby Work Areas excavated below the elevation of the wetland. We understand that permitting for wetland work has been obtained to accommodate control of inflow from the wetland to adjacent excavation Work Areas.

Based on metered flow rates from Frac tanks during a prior Interim Remedial Measure at the immediately abutting city property that occurred steadily over a six-month time frame, flows averaged 6 gallons per minute (gpm) with a maximum daily flow of 15 gpm. These data provide a useful benchmark against which dewatering estimates may be compared and to confirm the estimates used in **Table 1**.

### Dewatering Estimates:

Dewatering flow rate estimates were prepared assuming the 50-foot x 50-foot grid blocks are excavated to the depths shown on Drawings C-106 to 108 Excavation Plans with Groundwater Contours and Work Areas in **Attachment 1**. Work Areas (consisting of up to 6 of these 50-foot x 50-foot grid blocks) are labelled to indicate whether each Work Area is subject to Stormwater Only (e.g., **SO-1**) or Stormwater and Groundwater (e.g., **GW-1**) construction flows.

The calculation method follows that described in Construction Dewatering (Powers, 1992, pages 95 - 104), and assumes that dewatering of the excavation will behave similar to a large diameter well. The diameter or radius of the equivalent well is determined using the perimeter or area encompassing the excavation. Simple well equations then can be used to determine the requisite rate to produce a given drawdown using an estimated K of the aquifer. This method estimates only the pumping rates necessary to keep the excavation dewatered to the desired depth. Higher initial rates will likely be required as the head differential between the center of the Work Area being dewatered and the surrounding undisturbed soils will be greater than toward the end of the dewatering period when these head differences will be smaller.

Groundwater dewatering equations and calculations are included in **Table 1**.



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The results suggest that the required dewatering rate for Groundwater Work Areas GW-1 through GW-11 may range from 2 to 6 gpm to maintain dewatered conditions within individual Work Areas. The amount of free draining water in the volume of soil within Work Areas that would need to be initially removed before maintenance pumping is estimated to range from less than 10,000 gallons to just over 100,000 gallons, assuming a specific yield of 0.2.

Assuming a well point and sump dewatering network is installed at sufficient spacing within one Work Area at a time (anticipated to require a maximum well point or sump spacing of 20 feet on center) dewatering time frames of 2 to 12 days or more may be required for each Work Area.

While the above estimates are lower than the range observed during the Interim Remedial Measures dewatering (6 to 15 gpm), they are of the same magnitude.

The rainfall and subsequent stormwater flows that will require management during construction are estimated and summarized in **Tables 2 through 4**. Period of record storm events for Batavia NY were obtained along with the maximum daily rainfall recorded for the area over the anticipated construction period of May through December. These values were considered along with contributing areas that may deliver stormwater runoff within a maximum of 6 of the 50-foot x 50-foot blocks. Based on the maximum observed daily storm of record, an approximate 26,810 gallons of runoff from a 15,000 square foot Work Area would be collected, or an average of 19 gpm over the 24-hour period.

Combined groundwater and stormwater flows (construction water flows) from up to 3 Work areas at various stages of excavation, sampling and analyses, and backfilling, and storm water flows are estimated in **Tables 5 through 7**.

Based on a 20 gpm maximum treatment plant rate, an estimate of Frac tank storage requirements is provided in **Tables 8 and 9**.

#### **Limitations:**

These estimates should be considered approximate. The range of K values used in these calculations was taken from limited slug test estimates and inspection of soil boring and test pit logs; no aquifer pumping test data were available. Treatment of the area's dewatering as equivalent to a large-diameter well is also approximate given that dewatering depths may approach or exceed assumptions used in developing the equations employed.

These methods also do not provide estimates of rates that may be encountered during the initial phases of the dewatering process. As indicated above, the estimates rest on actual Ks of the aquifer, its heterogeneity, and the possibility of rapid draining from sand or gravel seams that the excavations may encounter. The extent of these more freely draining seams, if present, is unknown, so it is not known if they would yield high rates for an extended period of time or would run dry in a time frame of hours to days.



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Depending on the time of year that excavation will occur, temporary hydraulic barriers around the Limit of Work at the wetland at the north end of the site may be required to contain water in the wetland and prevent it from flowing directly into excavations. Such barriers are not considered here and will be the responsibility of others.

Prepared by: JBR 09-03-20, RAL comments addressed 09-24-20

Checked by: RAL 09-17-20, MJS 9-25-20



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**TABLES 1-9  
Construction Dewatering Flow Estimates**

Table 1: Estimate of Groundwater Dewatering Flows

Work Area ID Note 1	Area of Work Area, A (sf)	Perimeter of Work Area, P (ft)	Depth of Excavation (ft bgs) Note 2	Depth to GW (ft bgs) Note 3	Excavation Depth Below GW Depth (ft)	Volume of Water in the saturated Volume of Soil (gal) Note 4	Estimated Dewater Rate (gpm) Note 4	Volume of Water Inflow During 24- hr Hold Time (gal) Note 5	Volume of Dewater During 10-hr Backfill Operation (gal) Note 6	Total Dewater Volume Per Excavated Work Area (gal)
<b>SO-1</b>	12,100	N/A	6	10.0	None	0	N/A	0	0	0
<b>SO-2</b>	5,750	N/A	2	8.0	None	0	N/A	0	0	0
<b>SO-3</b>	5,000	N/A	2.5	10.0	None	0	N/A	0	0	0
<b>SO-4</b>	15,000	N/A	5.5	10.0	None	0	N/A	0	0	0
<b>SO-5</b>	11,000	N/A	7	15.0	None	0	N/A	0	0	0
<b>SO-6</b>	15,000	N/A	8	9.0	None	0	N/A	0	0	0
<b>SO-7</b>	15,000	N/A	6	10.0	None	0	N/A	0	0	0
<b>SO-8</b>	15,000	N/A	9	10.0	None	0	N/A	0	0	0
<b>SO-9</b>	15,000	N/A	5	6.0	None	0	N/A	0	0	0
<b>SO-10</b>	15,000	N/A	3	7.0	None	0	N/A	0	0	0
<b>SO-11</b>	15,000	N/A	1	2.0	None	0	N/A	0	0	0
<b>GW-1</b>	15,000	500	7	3.5	3.5	78,550	<b>5.7</b>	8,220	3,420	90,190
<b>GW-2</b>	15,000	500	8	4.0	4.0	89,770	<b>5.7</b>	8,140	3,390	101,300
<b>GW-3</b>	15,000	500	6	3.0	3.0	67,320	<b>5.8</b>	8,290	3,450	79,060
<b>GW-4</b>	15,000	500	7	2.5	4.5	100,990	<b>5.6</b>	8,060	3,360	112,410
<b>GW-5</b>	5,000	300	5	1.0	4.0	29,920	<b>2.6</b>	3,800	1,580	35,300
<b>GW-6</b>	5,000	300	7	1.0	6.0	44,880	<b>2.5</b>	3,630	1,510	50,020
<b>GW-7</b>	5,000	300	6	1.0	5.0	37,400	<b>2.6</b>	3,720	1,550	42,670
<b>GW-8</b>	3,125	300	2	0.0	2.0	9,350	<b>2.3</b>	3,370	1,400	14,120
<b>GW-9</b>	3,750	250	2	0.0	2.0	11,220	<b>2.2</b>	3,180	1,320	15,720
<b>GW-10</b>	5,000	300	4.5	0.0	4.5	33,660	<b>2.6</b>	3,760	1,570	38,990
<b>GW-11</b>	5,000	350	1	0.0	1.0	7,480	<b>3.0</b>	4,380	1,820	13,680
<b>Total</b>	230,725	-	-	-	-	510,540	-	58,550	24,370	
<b>Estimated Total Groundwater Dewatering Volume Generated (gal):</b>										<b>593,460</b>

**Table 1: Estimate of Groundwater Dewatering Flows**

Work Area ID Note 1	Area of Work Area, A (sf)	Perimeter of Work Area, P (ft)	Depth of Excavation (ft) Note 2	Depth to GW bgs)	Excavation Depth Below GW Depth (ft) Note 3	Volume of Water in the saturated Volume of Soil (gal) Note 4	Estimated Dewater Rate (gpm) Note 4	Volume of Water Inflow During 24- hr Hold Time (gal) Note 5	Volume of Dewater During 10-hr Backfill Operation (gal) Note 6	Total Dewater Volume Per Excavated Work Area (gal)
------------------------	---------------------------	--------------------------------	------------------------------------	------------------	------------------------------------------------	-----------------------------------------------------------------	----------------------------------------	----------------------------------------------------------------	-------------------------------------------------------------------	----------------------------------------------------

**Summary of Minimum, Maximum, and Average Calculated Areas, Rates, and Volumes**

Maximum Work Area (Cell):	15000.0 sf
Minimum Cell Area:	3125.0 sf
Average Cell Area:	10487.5 sf
Maximum Estimated Dewater Rate per Cell:	5.8 gpm
Minimum Estimated Dewater Rate per Cell:	2.2 gpm
Average Estimated Dewater Rate per Cell:	3.7 gpm
Max Estimated Volume (V) of Water in a Saturated V of Soil per Cell:	100,990 gal
Min Estimated Volume (V) of Water in a Saturated V of Soil per Cell:	0 gal
Avg Estimated Volume (V) of Water in a Saturated V of Soil per Cell:	46,413 gal (only cells with excavation below GW)

**Specific Yield: 0.20**

Assumed Excavation Open Time (hr): 24 Hold 10 Backfill

Maximum Perm Test Hydraulic Conductivity, K =	0.037 ft/d	1.3E-05 cm/s	GZA
Boring Log/Test Pit Soils Estimated Hydraulic Conductivity, K =	0.40 ft/d	1.4E-04 cm/s	
Assumed Hydraulic Conductivity, K =	<b>0.40 ft/d</b>	1.4E-04 cm/s	
Assumed Hydraulic Conductivity, K =	<b>3.0 gpd/ft^2</b>		
Assumed Hydraulic Conductivity, K =	1.4 microns/s		
Assumed Vertical Hydraulic Conductivity, K =	<b>0.04 ft/d</b>		

Notes:

1. Work Area IDs correspond to excavation cell layout and labels on the Construction Drawings. SO = Stormwater Only Work Area; GW = Stormwater and Groundwater Work Area.
2. Excavation depth per Pre-Design Investigation and historical Remedial Investigation data for PCBs. Excavation depths were set at minimum 1 foot intervals.
3. Depth of groundwater below ground surface estimated as the shallowest depth crossing the Work Area to the nearest 0.5 foot interval (one half the groundwater contour interval). Groundwater contours interpreted from Figure 4 *Groundwater Sampling Locations and Interpreted Groundwater Contours - July 2018*, MACTEC in July 2019.
4. Calculations based on the *Dewatering Estimate for Soil Removal at the Oneida Street Site* completed by MACTEC in January 2012, and *Construction Dewatering, Powers 1992*. Updated for the Batavia Site July 2020.
5. Each cell must be confirmed clean prior to backfilling. The specifications will require confirmation sample analysis to have a 24-hour turnaround, therefore, the cell will remain open for a minimum of 24 hours hold period.
6. It is assumed that the combination of additional hold time and backfilling will occur over a 10 hour work day. Each cell will require dewatering to keep the cell open and dry during this period.
7. Abbreviations: GW, groundwater; ft, feet; bgs, below ground surface; gal, gallon; gpm, gallon per minute; hr, hour; A, area; P, perimeter; avg, average; min, minimum; max, maximum; d, day; V, volume.
8. Estimate of maximum groundwater flow upward through floor of excavation Work Area assumes a unit gradient and K = 0.04 ft/day.

Prepared By: JRand  
Checked By: RLewis

Date: 9/2/2020  
Date: 9/9/2020

**Page 3 - Supporting Equations and Calculations**

$$R_o = 3(H-h)^*K^0.5 \quad Q = ((\Pi^*K)(H^2-h^2))/\ln(R_o/r_s)$$

**Basic Equations**

Radial Flow to Water Table Aquifer,  
Assumes  $R_o = R_o + r_s$  and  $r_w = r_s$  (Powers pg 95, 104):

$r_s = P/(2^*Pi)$ (ft)	$R_o$	$R_o + r_s$	Q (cf/day)	Q (gpm)	Q (gpm) from upwelling	Q (gpm) Total Dewater Rate
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
N/A	N/A	N/A	N/A	N/A	NA	NA
79.58	12.47	92.05	498.41	<b>2.6</b>	<b>3.1</b>	<b>5.7</b>
79.58	14.25	93.83	488.08	<b>2.5</b>	<b>3.1</b>	<b>5.7</b>
79.58	10.69	90.27	508.40	<b>2.6</b>	<b>3.1</b>	<b>5.8</b>
79.58	16.04	95.61	477.43	<b>2.5</b>	<b>3.1</b>	<b>5.6</b>
47.75	14.25	62.00	307.85	<b>1.6</b>	<b>1.0</b>	<b>2.6</b>
47.75	21.38	69.13	285.24	<b>1.5</b>	<b>1.0</b>	<b>2.5</b>
47.75	17.82	65.57	297.18	<b>1.5</b>	<b>1.0</b>	<b>2.6</b>
47.75	7.13	54.87	325.15	<b>1.7</b>	<b>0.6</b>	<b>2.3</b>
39.79	7.13	46.92	274.54	<b>1.4</b>	<b>0.8</b>	<b>2.2</b>
47.75	16.04	63.78	302.68	<b>1.6</b>	<b>1.0</b>	<b>2.6</b>
55.70	3.56	59.27	385.02	<b>2.0</b>	<b>1.0</b>	<b>3.0</b>

**Table 2: Normal Monthly Precipitation Summary**

Location	Data Set (years)	Normal Monthly Precipitation (inches) (Note 1)												Annual
		Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	
Batavia, New York	30	2.11	1.97	2.23	2.98	3.31	3.65	3.44	3.16	3.83	3.14	2.96	2.56	35.34

Notes:

- Source: National Oceanic and Atmospheric Administration (NOAA) National Climatic Data Center (NCDC) [ <https://www.ncdc.noaa.gov/cdo-web> ] Data Set from 1981 to 2010
- Estimated Construction Period

Prepared By: AFlynn _____ Date: 7/13/2020  
 Checked By: MPeters _____ Date: 9/22/2020

**Table 3: Maximum Daily Precipitation Summary**

Location	Data Set (years)	Maximum Daily Precipitation (inches) (Note 1)												Maximum Daily Precipitation during Construction
		Jan 08 1998	Feb 19 1981	Mar 04 1991	Apr 05 1982	May 23 2001	Jun 26 1998	Jul 15 1992	Aug 31 2005	Sep 16 2005	Oct 21 1995	Nov 23 2007	Dec 04 2007	
Batavia, New York	30	2.25	3.40	1.85	4.10	2.00	3.10	2.35	2.16	2.25	2.32	2.04	3.01	3.10

Notes:

- Source: National Oceanic and Atmospheric Administration (NOAA) National Climatic Data Center (NCDC). [ <https://www.ncdc.noaa.gov/cdo-web>] Data set from 1981 to 2010
- Estimated Construction Period

Prepared By: AFlynn _____ Date: 7/13/2020  
 Checked By: MPeters _____ Date: 9/22/2020

**Table 4: Stormwater Runoff from Probabilistic, 24-Hour Duration, Storm Events During Construction**

24-hr storm	Precipitation Note 1	Runoff Area, A	RCN	Runoff, Q	Total Volume of Stormwater Runoff, V _r	
			Note 2			
1-yr	1.96 in	15,000 sf	98	1.73 in	2,168 cf	16,220 gal
2-yr	2.30 in	15,000 sf	98	2.07 in	2,590 cf	19,370 gal
Max	3.10 in	15,000 sf	98	2.87 in	3,585 cf	26,810 gal

Notes:

- Source: Technical Paper No. 40, Rainfall Frequency Atlas of the United States for Durations from 30 Minutes to 24 Hours and Return Periods from 1 to 100 Years, US. Department of Commerce, Weather Bureau, 1963. [ [http://www.nws.noaa.gov/oh/hpsc/PF_documents/TechnicalPaper_No40.pdf](http://www.nws.noaa.gov/oh/hpsc/PF_documents/TechnicalPaper_No40.pdf) ]
- A weighted curve number (RCN) of 98 for the estimated disturbed area was used based on assuming the bottom of the excavation is saturated. Runoff depth and volume calculated using TR-55.
- Abbreviations: yr, year; hr, hour; A, runoff area; sf, square feet; Q, runoff; in, inches; cf, cubic feet; V, runoff volume; gal, gallon; RCN, runoff curve number; max, maximum daily precipitation.

Prepared By: AFlynn _____ Date: 7/13/2020  
 Checked By: MPeters _____ Date: 9/23/2020

**Table 5: Estimated Daily Construction Water Volume Generation**

Assumed Site Conditions Contributing to Construction Water Generation	Minimum Dewater Rate per Cell (gpm)	Average Open Area (sf)	Average Dewater Rate per Cell (gpm)	Maximum Open Area (sf)	Maximum Dewater Rate per Cell (gpm)	Min Daily Total Volume (gal)	Average Daily Total Volume (gal)	Max Daily Total Volume (gal)
Assume 1 cell is open on a given day awaiting confirmation sample analytical results	2.2	10,487.5	3.7	15,000.0	5.8	Note 1	Note 1	Note 1
Assume the excavated soil/sediment volume from 1 cell is being dewatered in the staging/storage area on a given day	-	-	-	-	-	0	46,410	100,990
Assume 1 confirmed clean cell is being backfilled and requires dewatering during the 10-hr work day	-	-	-	-	-	1,323	2,217	3,455
Assume 1 cell is being excavated during the 10-hr work day	-	-	-	-	-	0	46,410	100,990
Total Estimated Daily Groundwater Dewater Generation:						1,323	95,037	205,435
Total Estimated Likely Daily Stormwater Generation (Note 2):						16,220	16,220	16,220
Total Estimated Maximum Daily Stormwater Generation (Note 3):						26,810	26,810	26,810
<b>Total Estimated Daily Construction Water to be Managed (Lower Limit, Note 4):</b>						<b>1,323</b>	<b>95,037</b>	<b>205,435</b>
<b>Total Estimated Daily Construction Water to be Managed (Upper Limit, Note 5):</b>						<b>17,543</b>	<b>111,257</b>	<b>221,655</b>
<b>Total Estimated Additive Daily Contingency Construction Water to be Managed (Note 6):</b>						<b>10,590</b>	<b>10,590</b>	<b>10,590</b>
<b>Total Estimated Number Range of 20,000 gal Frac Tanks (Note 7, 8):</b>						<b>1</b> <b>1</b>	<b>5</b> <b>6</b>	<b>11</b> <b>12</b>
<b>Total Estimated Number of 20,000 gal Contingency Frac Tanks (Note 9):</b>						<b>1</b>	<b>1</b>	<b>1</b>

Notes:

1. GW infiltration is left to accumulate in open cells during the hold period provided the sidewalls of the excavation can remain stable.
2. For the likely total daily stormwater volume, assume the 1-year, 24-hour duration storm event (Table 4) occurs over an open area equal to a 15,000 sf excavation area.
3. For the maximum daily stormwater volume, assume the maximum recorded daily precipitation (Table 4) occurs over an open area equal to a 15,000 sf excavation area.
4. Lower limit of daily construction water generation assumes no contribution from a storm event (i.e., no rainfall occurs on that day).
5. Upper limit of daily construction water generation assumes a probabilistic 1-yr, 24-hr duration storm event occurs (i.e., rainfall occurs on that day).
6. Contingency daily construction water generation assumes that the maximum recorded daily precipitation event occurs during the construction period. This volume is greater than and added to the probabilistic likely volume associated with the 1-year, 24-hour duration storm.
7. The range of frac tanks is based on the estimated lower and upper limit daily construction water volumes.
8. Assumes total storage requirements without ongoing on-site treatment and disposal.
9. The additional contingency frac tanks required to store the additional stormwater volume associated with the maximum daily precipitation.

Prepared By: AFlynn Date: 7/13/2020  
Checked By: MPeters Date: 9/23/2020

**Table 6: Estimated Total Stormwater Volume Generation**

Construction Period	Normal Monthly Precipitation (inches), Note 1	Open Excavation Area (sf), Note 2	Volume of Stormwater Generated (cf), Note 3	Volume of Stormwater Generated (gal), Note 3
May	3.31	57,680	15,910	119,010
June	3.65	57,680	17,540	131,200
July	3.44	57,680	16,530	123,640
August	3.16	57,680	15,190	113,620
September	3.83	57,680	18,410	137,710
October	3.14	57,680	15,090	112,870
November	2.96	57,680	14,230	106,440
December	2.56	57,680	12,310	92,080
<b>Estimated Total Stormwater Volume Generated (gal)</b>				<b>936,570</b>

Notes:

1. Normal monthly precipitation value from Table 3.
2. Assume that a maximum of 25 percent of the total excavation area calculated in Table 1 is open or contributing runoff into open areas during any given storm event. The volume of precipitation for this contributing area will be collected as construction water.
3. Stormwater volume assumes 100 percent generation of runoff with no evapotranspiration or infiltration hydrologic components.
4. Abbreviations: sf, square feet; cf, cubic feet; gal, gallon

Prepared By: AFlynn Date: 7/13/2020  
Checked By: MPeters Date: 9/23/2020

**Table 7: Estimated Total Construction Water Volume Generation**

Description	Volume Generated (gal)
Estimated Total Groundwater Dewater Volume Generated (gal):	593,460 (Note 3)
Estimated Total Stormwater Volume Generated (gal):	936,570 (Note 4)
<b>Estimated Total Construction Water Generated (gal):</b>	<b>1,530,030 (Note 5)</b>

Notes:

1. Assumes an 8 month construction period.
2. Assumes any single cell is not open and active for a period longer than 34 hours (24-hour hold plus 10-hour backfill duration). Longer open/active durations may increase the groundwater dewater volume generation.
3. Calculated in Table 1 based on the volume of saturated soil below the estimated groundwater depth, groundwater inflow generated during the 24-hour hold period while confirmatory testing is completed, and a 10-hour dewatering duration during backfilling.
4. Calculated in Table 6 based on the collection of stormwater runoff for an open and active area equivalent to 25 percent of the total excavation area on any given day of any given month. The volume of stormwater generation assumes 100 percent capture of the normal monthly precipitation total without consideration of hydrologic losses associated with evapotranspiration or infiltration.
5. Total construction water generated during the construction period is considered to be the sum total of groundwater dewater and stormwater.

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**Table 8: Estimated On-Site Treatment System**

On-Site Treatment System Discharge Rate (Note 1)	20 gpm
Gallons Treated and Disposed per Hour	1,200 gph
Gallons Treated and Disposed per 24 hr Day	28,800 gpd
Equivalent 20,000 gal Tank Volumes Treated and Disposed per Day	1.44 tanks

Notes:

1. Maximum discharge rate of 20 gpm per municipal discharge permit.

**Table 9: Estimated Daily Construction Water Storage**

Description	Min Daily Total Volume (gal)	Average Daily Total Volume (gal)	Max Daily Total Volume (gal)
Total Estimated Daily Construction Water to be Managed (Lower Limit, Note 1):	1,323	95,037	205,435
Total Estimated Daily Construction Water to be Managed (Upper Limit, Note 1):	17,543	111,257	221,655
Total Estimated Additive Daily Contingency Construction Water to be Managed (Note 1):	10,590	10,590	10,590
<b>Total Estimated Number Range of 20,000-gal Frac Tanks (Note 2, 3):</b>	1	1	3
<b>Total Estimated Number of 20,000-gal Contingency Frac Tanks (Note 4):</b>		1	4
			8
			9

Notes:

1. Calculated in Table 5.
2. The range of frac tanks is based on the estimated lower and upper limit daily construction water volumes.
3. Assumes total storage requirements are offset with ongoing treatment and disposal calculated in Table 8.
4. The additional contingency frac tanks required to store the additional stormwater volume associated with the maximum daily precipitation

Prepared By: AFlynn Date: 7/13/2020  
Checked By: MPeters Date: 9/23/2020

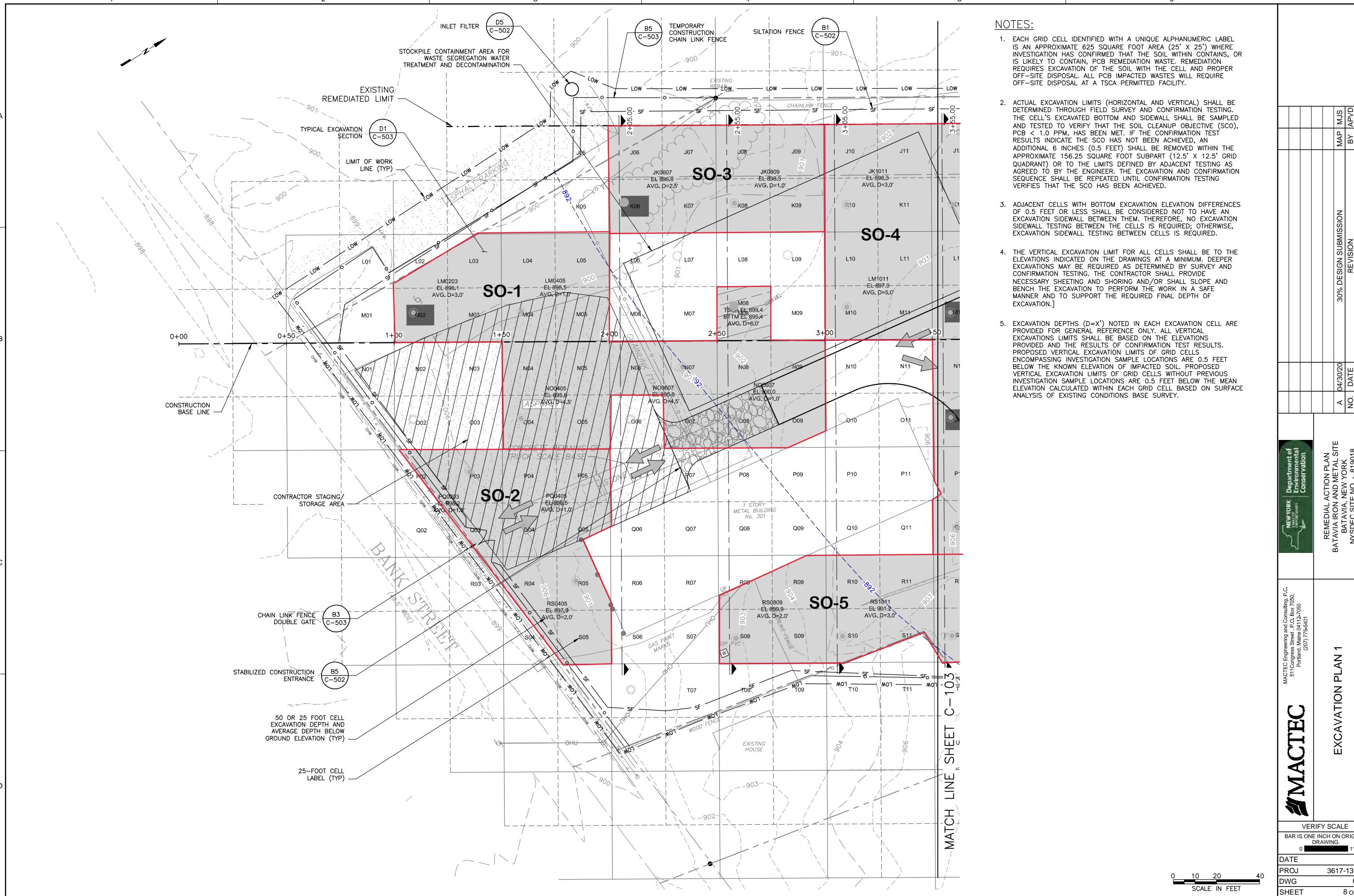


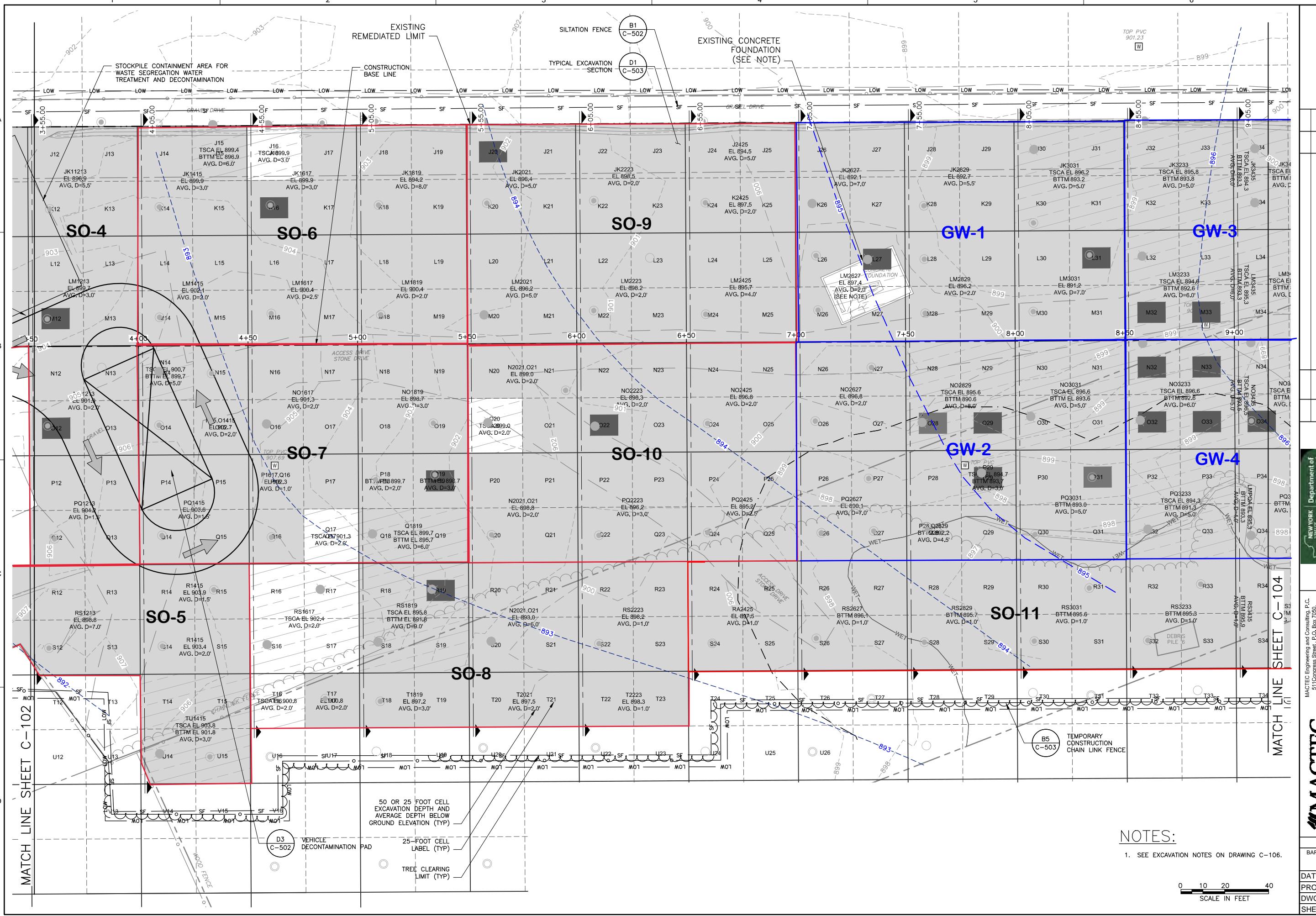
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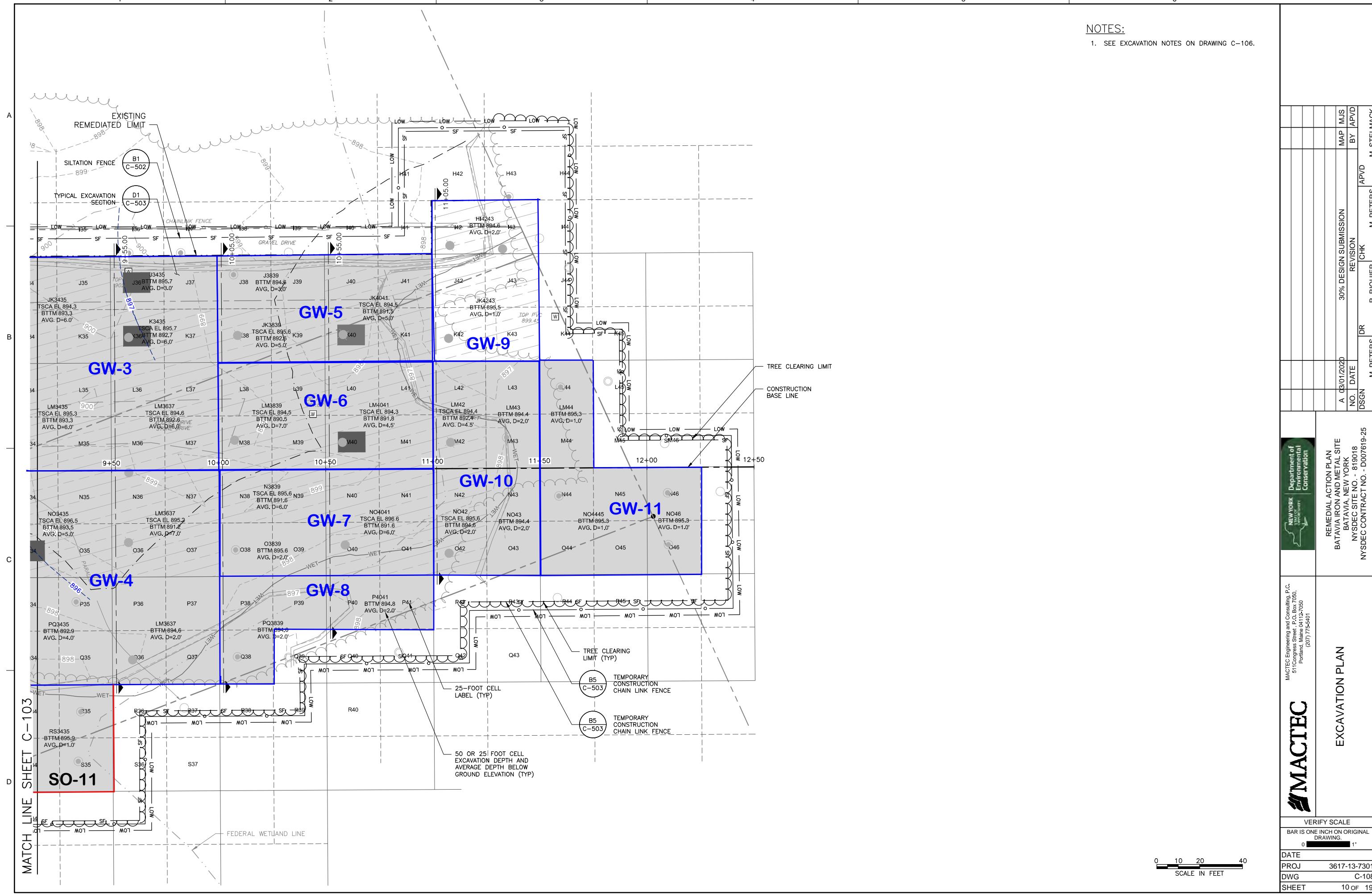
#### **ATTACHMENT 1**

#### **Excavation Plans with Groundwater Contours and Work Areas**





EXCAVATION PLAN		REMEDIAL ACTION PLAN	
		BATAVIA IRON AND METAL SITE BATAVIA, NEW YORK NYSDC SITE NO. - 819018 NYSDC CONTRACT NO. - D007619-25	
MACTEC		Department of Environmental Conservation	
Match Line Sheet C-104		Match Line Sheet C-104	
<b>DATE</b> A 03/01/2020 <b>NO.</b> NO. DATE <b>DSGN</b> R. RIQUIER <b>M. PETERS</b> <b>APVD</b> M. STELMACK		<b>30% DESIGN SUBMISSION</b> <b>REVISION</b> <b>CHK</b> <b>MJS</b> <b>APVD</b> <b>BY</b> <b>M. STELMACK</b>	
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## **APPENDIX E**

### **TCLP Lead Comparison**

Appendix E contains a memo summarizing the comparison between total lead concentrations and lead concentrations using the toxicity characteristic leaching procedure (TCLP) method. This information is offered for consideration when planning the remedial work.

# Memo

To: Contract/Project File

Prepared By: Charles Staples, P.G., MACTEC Engineering and Geology, PC (MACTEC) 12/2/20 CRS

Checked By: Mark Stelmack, P.E., MACTEC 12/11/20 MJS

Subject: Lead TCLP results, Batavia Iron and Metals Site, Batavia, NY

Toxicity Characteristic Leaching Procedure (TCLP) results for metals in soil at the Batavia Iron and Metals Site were reviewed to evaluate the potential for soil at the property to be considered a characteristic hazardous waste due to metals exceeding their TCLP limits. Samples for TCLP analysis were collected during the 2019 sampling event. Of the samples collected for total metals analysis, a subset of 33 were analyzed for TCLP metals to evaluate the reported range of total metals concentrations. Total lead concentrations in the compared samples ranged from 9.4 milligrams per kilogram [mg/Kg] to 70,000 mg/kg).

Within the 33 samples analyzed, lead was the only compound to exceed its TCLP criteria of 5 milligrams per liter (mg/L); however, there was not a clear linear relationship between total lead and TCLP lead. Sixteen of the 33 samples had concentrations of total lead greater than 1,000 mg/kg. Only samples with concentrations of lead greater than 5,000 mg/kg had corresponding TCLP results that failed for TCLP (i.e., greater than 5 mg/L). The attached Table 1 shows a comparison of total lead results to TCLP lead results.

Based on this comparison, it can be assumed that total lead concentrations greater than 5,000 mg/kg have the potential to exceed TCLP criteria and be considered a characteristic hazardous waste. The attached Figure 1 shows the 2018 and 2019 sample locations indicating the PCB concentration ranges and highlights in purple grids where total lead concentrations exceed 5,000 mg/kg. Metals sample locations coincide with the PCB sample locations. Therefore, grids that show PCB results, but are not highlighted in purple, contained lead at concentrations less than 5,000 mg/kg. Exceptions to this are the six grids that contain results for PCBs, but where metals samples were not collected, i.e., grids L09, N07, N09, N21, O19, and R33.

Lead was only detected at concentrations greater than 5,000 mg/kg at depths shallower than three feet below ground surface (bgs); most lead detections were at ground surface. The one exception was in the historic building foundation (sample collected in grid L27), where lead was greater than 5,000 mg/kg at the bottom of the foundation, located around at between 4-5 ft bgs.

Table 2 presents the 25-foot by 25-foot gridded areas within which lead was detected at concentrations greater than 5,000 mg/kg. Table 2 includes the estimated depth from ground surface where lead exceeds 5,000 mg/kg and the resulting estimated volume of soil where lead exceeds 5,000 mg/kg. Where applicable based on sampling results, the volume amount of soil exceeding 5,000 mg/kg within a grid is also designated as being in exceedance of 50 mg/kg PCBs. The volumes include only those grids where lead was sampled and do not extrapolate volumes for non-sampled grids.

**Table 1: Soil Total and TCLP Metals Results**

Parameter	Location		HA-L27	HA-N36	HA-O32	HA-R05	HA-R35	HA-R35	HA-T31	TP-J34	TP-J34
	Sample Date	Sample ID	8/1/2019	8/1/2019	8/1/2019	8/1/2019	8/1/2019	8/1/2019	8/1/2019	7/29/2019	7/29/2019
	Sample Depth (ft bgs)	QC Code	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result
Total Metals (mg/kg)	RES	COM									
Lead - Total (mg/Kg)	400	1000	<b>20300</b>		<b>4980</b>	<b>70000</b>	<b>385</b>	<b>343</b>	<b>9.4</b>	<b>466</b>	<b>1200</b>
Lead - TCLP (mg/L)	5	--	<b>58.7</b>		<b>1.1</b>	<b>8</b>	<b>0.57</b>	<b>0.067</b>	0.02 U	<b>0.11</b>	<b>0.73</b>

Notes:

ft bgs = feet below ground surface

QC code: FS= field sample, FD = field duplicate

Samples analyzed for:

Metals by USEPA Method 6010

Toxicity Characteristic Leaching Procedure (TCLP)

metals by USEPA Method 1311

Results in milligrams per kilogram or liter (mg/kg or mg/L)

Only detected compounds shown (detections in bold);

Qualifiers: U=not detected;

J=estimated value (+/- = biased high/low)

Criteria:

RES = Part 375 Protection for Residential Use

COM = Part 375 Protection for Commercial Use

**Highlighted cell exceeds criteria**

Blue highlighted result exceeds TCLP criteria

**Table 1: Soil Total and TCLP Metals Results**

Location	TP-L07	TP-L26	TP-L26	TP-L26	TP-L28	TP-L28	TP-L28	TP-L31	TP-M43	TP-M43
Sample Date	7/31/2019	7/30/2019	7/30/2019	7/30/2019	7/30/2019	7/30/2019	7/30/2019	7/30/2019	7/29/2019	7/29/2019
Sample ID	819018-TPL0700	819018-TPL2600	819018-TPL2602	819018-TPL2608	819018-TPL2800	819018-TPL29018-TPL2	819018-TPL3100	819018-TPM4300	819018-TPM4304	
Sample Depth (ft bgs)	0	0	2	8	0	2	2	0	0	4
QC Code	FS	FS	FS	FS	FS	FS	FS	FS	FS	FS
Parameter	RES	COM	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result	Qualifier
<b>Total Metals (mg/kg)</b>										
Lead - Total (mg/Kg)	400	1000	<b>11.5</b>		<b>2050</b>		<b>46.5</b>		<b>3.9</b>	
Lead - TCLP (mg/L)	5	--	0.02 U		<b>0.37</b>		<b>0.0058 J</b>		0.02 U	

**Notes:**

ft bgs = feet below ground surface

QC code: FS= field sample, FD = field duplicate

Samples analyzed for:

Metals by USEPA Method 6010

Toxicity Characteristic Leaching Procedure (TCLP)

metals by USEPA Method 1311

Results in milligrams per kilogram or liter (mg/kg or mg/L)

Only detected compounds shown (detections in bold);

Qualifiers: U=not detected;

J=estimated value (+/- = biased high/low)

Criteria:

RES = Part 375 Protection for Residential Use

COM = Part 375 Protection for Commercial Use

**Highlighted cell exceeds criteria**

Blue highlighted result exceeds TCLP criteria

**Table 1: Soil Total and TCLP Metals Results**

Parameter	Location	TP-N15	TP-N15	TP-N27	TP-N27	TP-N27	TP-P29	TP-P29	TP-P29	TP-P31
Sample Date	7/31/2019	7/31/2019	7/30/2019	7/30/2019	7/30/2019	7/29/2019	7/29/2019	7/29/2019	7/29/2019	7/29/2019
Sample Depth (ft bgs)	0	2	0	2	4	0	2	4	0	0
QC Code	FS	FS	FS	FS	FS	FS	FS	FS	FS	FS
RES	COM	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result	Qualifier	Result
<b>Total Metals (mg/kg)</b>										
Lead - Total (mg/Kg)	400	1000	<b>2230</b>	<b>857</b>	<b>1060</b>	<b>20.3</b>	<b>118</b>	<b>5060</b>	<b>324</b>	<b>5.9</b>
Lead - TCLP (mg/L)	5	--	<b>1.8</b>	<b>0.79</b>	<b>0.95</b>	0.02 U	<b>0.074</b>	<b>26.9</b>	<b>0.025</b>	0.02 U

Notes:

ft bgs = feet below ground surface

QC code: FS= field sample, FD = field duplicate

Samples analyzed for:

Metals by USEPA Method 6010

Toxicity Characteristic Leaching Procedure (TCLP)

metals by USEPA Method 1311

Results in milligrams per kilogram or liter (mg/kg or mg/L)

Only detected compounds shown (detections in bold);

Qualifiers: U=not detected;

J=estimated value (+/- = biased high/low)

Criteria:

RES = Part 375 Protection for Residential Use

COM = Part 375 Protection for Commercial Use

**Highlighted cell exceeds criteria**

Blue highlighted result exceeds TCLP criteria

**Table 1: Soil Total and TCLP Metals Results**

Location	TP-P31	TP-P35	TP-P35	TP-R15	TP-R17	TP-R31
Sample Date	7/29/2019	7/29/2019	7/29/2019	7/31/2019	7/31/2019	7/29/2019
Sample ID	819018-TPP3102	819018-TPP3500	819018-TPP3502	819018-TPR1500	819018-TPR1700	819018-TPR3100
Sample Depth (ft bgs)	2	0	2	0	0	0
QC Code	FS	FS	FS	FS	FS	FS
Parameter	RES	COM	Result	Qualifier	Result	Qualifier
<b>Total Metals (mg/kg)</b>						
Lead - Total (mg/Kg)	400	1000	<b>7020</b>		<b>3880</b>	<b>864</b>
Lead - TCLP (mg/L)	5	--	<b>18.1</b>		4.2	<b>0.53</b>
					<b>1.7</b>	<b>2.2</b>
					<b>0.64</b>	

Notes:

ft bgs = feet below ground surface

QC code: FS= field sample, FD = field duplicate

Samples analyzed for:

Metals by USEPA Method 6010

Toxicity Characteristic Leaching Procedure (TCLP)

metals by USEPA Method 1311

Results in milligrams per kilogram or liter (mg/kg or mg/L)

Only detected compounds shown (detections in bold);

Qualifiers: U=not detected;

J=estimated value (+/- = biased high/low)

Criteria:

RES = Part 375 Protection for Residential Use

COM = Part 375 Protection for Commercial Use

**Highlighted cell exceeds criteria**

Blue highlighted result exceeds TCLP criteria

**Table 2: Soil Volume for Cells with Lead Detected Greater than 5,000 mg/kg**

Grid/Cell ID	Estimated Depth (ft) of Lead Concentrations > 5,000 mg/kg	PCBs > 50 mg/kg Within Grid	Volume (cubic yard)
J20	2	No	46.3
J36	3	No	69.4
K06	1	No	23.1
K16	3	No	69.4
K36	3	Yes	69.4
K40	1	Yes	23.1
L27	4.5	Yes	104.2
L31	3	No	69.4
M02	2	No	46.3
M12	4	No	92.6
M32	1	Yes	23.1
N32	3	Yes	69.4
N33	1	Yes	23.1
O12	2	No	46.3
O22	1	No	23.1
O28	1	Yes	23.1
O29	1	Yes	23.1
O32	1	Yes	23.1
O33	1	Yes	23.1
O34	2	Yes	46.3
P19	1	No	23.1
P29	1	Yes	23.1
P31	3	No	69.4
R19	1	Yes	23.1
<b>Total soil volume where PCBs are less than 50 mg/kg and lead is greater than 5,000 mg/kg :</b>			<b>578.4</b>
<b>Total soil volume where PCBs are more than 50 mg/kg and lead is greater than 5,000 mg/kg :</b>			<b>497.2</b>

Notes:

mg/kg = milligrams per kilogram

PCBs = polychlorinated biphenyls

Depth = depth of lead exceedence from ground surface in feet (ft).

Grid is surface is 25 by 25 feet

Volume estimate is only for grids with lead sample results. Adjacent cells that were not sampled may also contain lead concentrations greater than 5,000 mg/kg.